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December 13, 1999

Mr. William C. Huffman
Project Manager
Decommissioning Section
Projects Directorate IV & Decommissioning
U.S. Nuclear Regulatory Commission
Mail Stop 11 D19
Washington, DC 20555-0001

Dear Mr. Huffman:

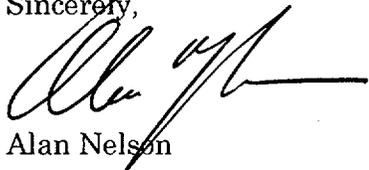
On July 15-16, 1999, the NRC held a workshop on spent fuel accidents at decommissioning plants. During the course of the workshop, presentations by the NRC and the industry concluded that spent fuel pools possess substantial capability beyond their design basis to withstand seismic events but that variations in seismic capacity existed due to plant specific designs and locations.

NEI forwarded "Seismic Screening Criteria for Assessing Potential Pool Vulnerabilities at Decommissioning Plants, to the NRC" August 18, 1999 for review and comment. Based on NRC review, the staff proposed additional details to the submitted checklist. Detailed NRC comments were made available on December 3, 1999 "Screening Criteria for Assessing Potential Seismic Vulnerabilities of Spent Fuel Pools at Decommissioning Plants."

Enclosed is the revised screening criteria addressing the December 3, 1999 NRC memorandum. We believe the revision addresses the deficiencies identified. We request that the revised checklist be considered as the NRC prepares its draft report to be issued in January 2000.

Please contact me at (202) 739-8110 or by e-mail (apn@nei.org) if you have any questions or if you would like to schedule a meeting to discuss industry's response to the staff's recommendations.

Sincerely,



Alan Nelson

APN/dc
Enclosure

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Per
Bill Huffman*



Seismic Screening Criteria
for
Assessing Potential Fuel Pool Vulnerabilities
at
Decommissioning Plants

December 13, 1999
Revision 1

Background

To increase the efficiency and effectiveness of decommissioning regulations, the NRC staff has engaged in rulemaking activities that would reduce the need to routinely process exemptions once a plant is permanently shut down. With this goal in mind, members of the NRC staff, industry representatives and other stakeholders held a two-day workshop on risk related spent fuel pool accidents at decommissioning plants.

At this workshop, based upon presentations by the NRC staff (Goutam Bagchi et al.) and the nuclear industry (T. O'Hara - DE&S), it was concluded that a large seismic event (in the range of three times the design level earthquake) would represent a risk of exceeding the structural capacity of the spent fuel pool and thus potentially result in draining the pool.

Although the methodologies presented by the NRC staff and the industry differed somewhat, they both concluded that, in general, spent fuel pools possess substantial capacity beyond their design basis but that variations in seismic capacity existed due to plant specific details (i.e. "Differences in seismic capacity due to spent fuel location and other details.").

The consensus was that the risk was low enough that precise quantification was not necessary to support exemption requests but that this needed to be confirmed on a plant specific basis with deterministic criteria. It was recommended that a simple spent fuel pool (SFP) vulnerability check list be developed to provide additional assurance that no beyond-design-basis seismic structural vulnerabilities exist at decommissioning plants. A draft seismic screening checklist was provided to the Staff by NEI in August 1999. Comments on this draft were discussed during a conference call held on December 7, 1999 and the following draft screening checklist has been revised to address the issues raised.

Spent Fuel Pool Seismic Vulnerability Check List

Purpose of Checklist

As discussed briefly in the "Background" section, the purpose of this checklist is to identify and evaluate specific seismic characteristics which might result in a specific spent fuel pool from not being capable of withstanding, without catastrophic failure, a beyond-design-basis seismic event equal in magnitude to approximately three times its design basis. Completion of the requirements will be performed by a qualified seismic engineer. This effort will include a thorough SFP walkdown and a review of appropriate SFP design drawings.

DRAFT CHECKLIST

Item 1:

Requirement: Identify Preexisting Concrete and Liner Plate Degradation

Basis: A detailed review of plant records concerning spent fuel pool concrete and liner plate degradation should be performed and supplemented by a detailed walkdown of the accessible portions of the spent fuel pool concrete and liner plate. The purpose of the records review and visual inspection activities is to accurately assess the material condition of the SFP concrete and liner in order to assure that these existing material conditions are properly factored into the remaining seismic screening assessments.

Design Feature: The material condition of the SFP concrete and liner, based upon the records review and the walkdown inspection, will be documented and used as an engineering input to the following seismic screening assessments.

Item 2:

Requirement: **Assure Adequate Ductility of Shear Wall Structures**

Basis: The expert panel involved with the development of Reference 1 concluded that, "For the Category 1 structures which comply with the requirements of either ACI 318-71 or ACI 349-76 or later building codes and are designed for an SSE of at least 0.1g pga, as long as they do not have any special problems as discussed below, the HCLPF capacity is at least 0.5g pga." This conclusion was based upon the assumption that the shear wall structure will respond in a ductile manner. The "special problems" cited deal with individual plant details which could prevent a particular plant

Spent Fuel Pool Seismic Vulnerability Check List

from responding in the required ductile fashion. Examples cited in Reference 1 included an embedded structural steel frame in a common shear wall at the Zion plant (which was assumed to fail in brittle manner due to a potential shear failure of the attached shear studs) and large openings in a "crib house" roof (also at the Zion plant) which could interrupt the continuity of the structural slab.

Other examples which could impact the ductility of the spent fuel pool structure include large openings which are not adequately reinforced or reinforcing bars that are not sufficiently embedded to prevent a bond failure before the yield capacity of the steel is reached.

Design Feature: This design feature requirement will be documented based on a review of drawings and a SFP walkdown.

Item 3:

Requirement: **Assure Design adequacy of Diaphragms (including roofs)**

Basis: In the design of many nuclear power plants, the seismic design of roof and floor diaphragms has often not received the same level of attention as have the shear walls of the structures. Major cutouts for hatches or for pipe and electrical chases may pose special problems for diaphragms. Since more equipment tends to be anchored to the diaphragm compared to shear walls, moderate amounts of damage may be more critical for the diaphragm compared to the same amount of damage in a wall.

Based upon the guidance provided in Reference 1, diaphragms for Category I structures designed for a SSE of 0.1g or greater do not require an explicit evaluation provided that: (1) the diaphragm loads were developed using dynamic analysis methods; (2) they comply with the ductility detailing requirements of ACI 318-71 or ACI 349-76 or later editions. Diaphragms which do not comply with the above ductility detailing or which did not have loads explicitly calculated using dynamic analysis should be evaluated for a beyond-design-basis seismic event in the 0.45-0.5g pga range.

Design Feature: This design feature requirement will be documented based on a review of drawings and a SFP walkdown.

Spent Fuel Pool Seismic Vulnerability Check List

Item 4: _____

Requirement: Verify the Adequacy of the SFP Walls and Floor Slab to Resist Out-of-Plane Shear and Flexural Loads

Basis: For PWR pools that are fully or partially embedded, an earthquake motion that could cause a catastrophic out-of-plane shear or flexural failure is very high and is not a credible event. For BWR pools (and PWR pools that are not at least partially embedded), the seismic capacity is likely to be somewhat less and the potential for out-of-plane shear and/or flexural wall or base slab failure, at beyond-design-basis seismic loadings, is possible.

A structural assessment of the pool walls and floor slab out-of-plane shear and flexural capabilities should be performed and compared to the realistic loads expected to be generated by a seismic event equal to approximately three times the site SSE. This assessment should include dead loads resulting from the masses of the pool water and racks, seismic inertial forces, sloshing effects and any significant impact forces.

Credit for out-of-plane shear or flexural ductility should not be taken unless the reinforcement associated with each failure mode can be shown to meet the ACI 318-71 or ACI 349-49 requirements.

Design Feature: Compliance with this design feature will be documented based upon a review of drawings (in the case of embedded or partially embedded PWR pools) or based upon a review of drawings coupled with the specified beyond-design-basis shear and flexural calculations outlined above.

Item 5:

Requirement: **Verify the Adequacy of Structural Steel (and Concrete) Frame Construction**

Basis: At a number of older nuclear power plants, the walls and roof above the top of the spent fuel pool are constructed of structural steel. These steel frames were generally designed to resist hurricane and tornado wind loads which exceeded the anticipated design basis seismic loads. A review of these steel (or possibly concrete) framed structures should be performed to assure that they can resist the seismic forces resulting from a beyond-design-basis

Spent Fuel Pool Seismic Vulnerability Check List

seismic event in the 0.45-0.5g pga range. Such a review of steel structures should concentrate on structural detailing at connections. Similarly, concrete frame reviews should concentrate on the adequacy of the reinforcement detailing and embedment.

Failure of the structural steel superstructure should be evaluated for its potential impact on the ability of the spent fuel pool to continue to successfully maintain its water inventory for cooling and shielding of the spent fuel.

Design Feature: This design feature requirement will be documented based on a review of drawings and a SFP walkdown.

Item 6:

Requirement: **Verify the Adequacy of Spent Fuel Pool Penetrations**

Basis: The seismic and structural adequacy of any spent fuel pool (SFP) penetrations whose failure could result in the draining or syphoning of the SFP must be evaluated for the forces and displacements resulting from a beyond-design-basis seismic event in the 0.45-0.5g pga range. Specific examples include SFP gates and gate seals and low elevation SFP penetrations, such as, the fuel transfer chute/tube and possibly piping associated with the SFP cooling system. Failures of any penetrations which could lead to draining or syphoning of the SFP should be considered.

Design Feature: This design feature requirement will be documented based on a review of drawings and a SFP walkdown.

Item 7:

Requirement: **Evaluate the Potential for Impacts with Adjacent Structures**

Basis: Structure-to-structure impact may become important for earthquakes significantly above the SSE, particularly for soil sites. Structures are usually conservatively designed with rattle space sufficient to preclude impact at the SSE level but there are no set standards for margins above the SSE. In most cases, impact is not a serious problem but, given the potential for impact, the consequences should be addressed. For impacts at earthquake levels below 0.5g pga, the most probable damage includes the potential for electrical equipment malfunction and for local

Spent Fuel Pool Seismic Vulnerability Check List

structural damage. As cited previously, these levels of damage may be found to be acceptable or to result in the loss of SFP support equipment. The major focus of this impact review is to assure that the structure-to-structure impact does not result in the inability of the SFP to maintain its water inventory.

Design Feature: This design feature requirement will be documented based on a review of drawings and a SFP walkdown.

Item 8:

Requirement: **Evaluate the Potential for Dropped Loads**

Basis: A beyond-design-basis seismic event in the 0.45-0.5g pga range has the potential to cause the structural collapse of masonry walls and/or equipment supports systems. If these secondary structural failures could result in the accidental dropping of heavy loads which are always present (i.e. not loads associated with cask movements) into the SFP, then the consequences of these drops must be considered. As in previous evaluations, the focus of the drop consequence analyses should consider the possibility of draining the SFP. Additionally, the evaluation should evaluate the consequences of any resulting damage to the spent fuel or to the spent fuel storage racks.

Design Feature: This design feature requirement will be documented based on a review of drawings and a SFP walkdown.

Item 9:

Requirement: **Evaluation of Other Failure Modes**

Basis Experienced seismic engineers should review the geotechnical and structural design details for the specific site and assure that there are not any design vulnerabilities which will not be adequately addressed by the review areas listed above. Soil-related failure modes including liquefaction and slope instability should be screened by the approaches outlined in Reference 1 (Section 7 & Appendix C).

Design Feature: This design feature requirement will be documented based on a review of drawings and a SFP walkdown.

Spent Fuel Pool Seismic Vulnerability Check List

Item 10: Potential Mitigation Measures

Although beyond the scope of this seismic screening checklist, the following potential mitigation measures may be considered in the event that the requirements of the seismic screening checklist are not met at a particular plant.

- a.) Delay requesting the licensing waivers (E-Plan, insurance, etc.) until the plant specific danger of a "zirc-fire" is no longer a credible concern.
- b.) Design and install structural plant modifications to correct/address the identified areas of non-compliance with the checklist. (It must be acknowledged that this option may not be practical for significant seismic failure concerns.)
- c.) Perform plant-specific seismic hazard analyses to demonstrate that the seismic risk associated with a catastrophic failure of the pool is at an acceptable level. (The exact "acceptable" risk level has not been precisely quantified but is believed to be in the range of 1.0E-06.)

Item 11: Required Documentation

A simple report describing the results of the seismic engineer's walkdown and drawing review findings is judged to provide sufficient documentation to rule out a beyond-design-basis seismic event as a significant risk contributor to a decommissioned nuclear power plant.

References:

1. "A Methodology for Assessment of Nuclear Power Plant Seismic Margin Revision 1), (EPRI NP-6041-SL), August 1991

Comments on NRC Draft Screening Criteria for Assessing Potential Seismic Vulnerabilities of Spent Fuel Pools at Decommissioning Plants – December 3, 1999 NRC Memorandum

Summary of NRC Draft

To increase the efficiency and effectiveness of decommissioning regulations, the NRC staff has engaged in rulemaking activities that would reduce the need to routinely process exemptions once a plant is permanently shut down. The December 3, 1999 memorandum from W. Huffman to S. Richards (Reference 1) provides a summary of the staff's current concerns regarding a screening criteria for assessing potential seismic vulnerabilities to spent fuel pools (SFP) at decommissioning plants. Attachments to this memorandum contain suggested enhancements to the proposed seismic checklist and also excerpts from an independent technical review by Dr. Robert Kennedy. The report by Kennedy endorsed the feasibility of the use of a seismic screening concept. The Kennedy report identified eight sites for which the seismically induced probability of SFP failure is greater than 3.0×10^{-6} using the LLNL 93 hazard data.

The seismic risk of failure of the spent fuel pool can be estimated by rigorously convolving a family of fragility curves with a family of seismic hazard curves (Reference 2), or by simplified approximation methods. Two simplified methods are described in the attachments to the December 3, 1999 memorandum (Reference 1).

The first simplified method was presented by the Staff in their preliminary draft of June 16, 1999 (Reference 3). This method is based on use of the SFP high confidence low probability of failure (HCLPF) value and the simplifying assumption that the conditional probability of SFP failure is about a factor of 20 less than the annual probability of exceeding the SFP HCLPF value. Given that the SFP HCLPF value is more than or equal to three times the SSE (and less than 10^{-5}) then the SFP failure frequency should be less than 5×10^{-7} . This simplified method is based on use of peak ground acceleration (PGA) curves.

The second simplified method was suggested by Kennedy and is based on use of spectral acceleration (S_a) rather than PGA. Kennedy states that damage to structures, systems, and components (SSCs) does not correlate well to PGA ground motions but correlates much better with spectral accelerations between 2.5 and 10 Hz at nuclear power plants. Based on previous studies Kennedy proposes to screen SFPs based on use of the peak spectral acceleration (PSA) HCLPF seismic capacity of 1.2g. This value is equivalent to 0.5g PGA. This simplified approach is based on calculating the 10% conditional probability of failure capacity ($C_{10\%}$) given the PSA value of 1.2g. Using Equation 6 in the Reference 1 attachment results in a $C_{10\%} S_a$ value of 1.82g. The annual probability of exceeding this value at 10, 5 and 2.5 Hz is then calculated using the LLNL hazard results. These value are then multiplied by 0.5 and the highest of the 10, 5, and 2.5 Hz results is used as the SFP failure probability. For example, the $C_{10\%}$ at 5 Hz is 1.82g or about 56.8 cm/sec spectral velocity. For LLNL site 1, the annual probability of exceeding 56.8 cm/sec is about 2.0×10^{-6} . This value is multiplied by 0.5 which results in a SFP failure probability for

site 1 of about 1.0×10^{-6} . This same calculation is performed at 10 and 2.5 Hz.

Based on comparisons made by Kennedy he concludes that simplified method 1 (Reference 3) underestimates the seismic risk by factors of 2.3 and 3.5 for Vermont Yankee and Robinson respectively. Using simplified method 2 the seismic risk is overestimated by 20% and 5% respectively for these two cases.

Kennedy noted that in his judgement it will be necessary to have seismic fragility HCLPF computations performed on at least six different aboveground SFPs with walls not supported by soil before HCLPF screening levels can be established for these SFPs.

Recommendation Number 4 of the December 3, 1999 memorandum requested that industry provide input concerning:

- a. the list of high hazard sites,
- b. a credible ground motion description at which the seismic hazard frequency is low enough at these sites, and
- c. plant specific seismic capacity evaluations using credible ground motion descriptions at these sites.

Recommendation Number 5 requests that industry propose treatment of sites West of the Rocky Mountains.

Preliminary Industry Comments

Industry concurs that use of a seismic screening checklist is an excellent approach to plant-specific seismic assessments. In addition, we will incorporate into our earlier seismic checklist those suggestions presented in Recommendation numbers 1, 2, and 3 to the December 3, 1999 memorandum.

With respect to the simplified methods to estimate seismic failure frequency of SFP failure the method proposed by Kennedy appears to be reasonable.

In the recommendations section of the 12/3/99 memorandum (Reference 1) some actions by industry are proposed. Recommendation Number 4.b requests that industry recommend a credible ground motion description at which the seismic hazard frequency is low enough at these "high" hazard sites. These "high" hazard sites were identified based on use of the Kennedy simplified SFP failure methodology and the LLNL 1993 hazard results. The response to Recommendation Numbers 4.a and 4.c are dependent on the resolution of 4.b.

Comments on Recommendation Number 4.b

1. Using the Kennedy simplified SFP failure methodology $C_{10\%}$ values are determined at 10, 5, and 2.5 Hz. At 5 Hz the spectral acceleration value is 1.82g or about 56.8 cm/sec.
2. The PSA values associated with these $C_{10\%}$ values are consistent with spectral values which describe the San Onofre and Diablo Canyon SSEs, i.e., large magnitude, near field earthquakes.
3. The issue of large earthquakes occurring near EUS NPPs was resolved by the Charleston Issue (SECY-91-135, Reference 4). As stated in SECY-91-135, "Large 1886 Charleston-size earthquakes, greater than or equal to magnitude 6.5, are not significant contributors to the seismic hazard for nuclear facilities along the eastern seaboard outside the Charleston region. This result is consistent with the results emerging from the ongoing studies of earthquake-induced liquefaction features along the eastern seaboard. These studies have found no evidence of large prehistoric earthquakes originating outside the South Carolina region. Thus the issue of the Charleston earthquake occurring elsewhere in the eastern seaboard is considered to be closed."
4. Credible, versus not credible in terms of annual probability, is typically associated with greater than about 10^{-6} (credible) and 10^{-6} or less (not credible). Within the context of the Kennedy simplified SFP failure methodology, if the annual probability of exceeding the screening level value (for example 56.8 cm/sec at 5 Hz) times 0.5 is less than 10^{-6} , then only the seismic checklist must be satisfied. Implicit in this approach is that the probabilistic estimates at the $C_{10\%}$ level are credible.
5. For a site to be screened out the $C_{10\%}$ value should be on the order of 10^{-6} . Figure 1 (attached) shows the 5 Hz spectral acceleration values associated with the 10^{-6} LLNL results at each of the 69 sites. As can be seen, for site number 36 (which in Table 3 of the Kennedy report is the site with the highest SFP failure frequency) the 10^{-6} spectral acceleration is about 7,700 cm/sec² or about 245 cm/sec. As stated previously, 57 cm/sec is consistent with 5 Hz spectral velocities associated with a magnitude 6.6 earthquake 8 km from the site (San Onofre SSE), therefore these predicted groundmotions must be associated with a very large earthquake, greater than magnitude 6.5, very near to the site – which is counter to the conclusions of SECY-91-135. Other values at other sites are equally incredible. Based on these results, it is concluded that the LLNL results, at the probability/ground motion levels of interest, are deterministically incredible and therefore their use in screening is questionable. Figure 2 (attached) shows the 5 Hz spectral acceleration values associated with the 10^{-6} EPRI results. As can be seen, the EPRI results, at the probability/ground motion levels of interest, are credible, and consistent with SECY-91-135.
6. Figure 3 (Figure 2 from NUREG-1488, Reference 5) illustrates the problems associated with the LLNL results at high ground motions/low annual probabilities. As can be seen from Figure 3, at high probabilities there is reasonable agreement between LLNL and

EPRI. However, the slope of the LLNL results at high ground motions is too shallow. The effect of this shallow slope is to predict incredible ground motions at credible probability levels.

7. Based on this review, industry contends that it would be appropriate to only use EPRI results in the SFP seismic screening analysis. We believe this to be reasonable in light of the difficulties associated with the LLNL results at low probabilities. The effect of using only the EPRI results is shown in column 3 of Table 3 in the Kennedy report (Reference 1). As can be seen, only 1 plant would be required to perform further analyses. However, because both LLNL and EPRI are considered to provide valid results, it is proposed that the results from each study be geometrically averaged such that equal weight is provided the results from each study. Arithmetic averaging is considered unacceptable in light of the difficulties associated with the LLNL results. Figure 4 provides the results of geometrically averaging the LLNL and EPRI results.

Comments on Recommendation Number 4.a

Based on Figure 4 about 6 sites would be preliminarily screened in due to exceeding the 10^{-6} criterion. One of the 6 sites is Shoreham. If these screened in SFPs are above ground then further analyses will be required.

Comments on Recommendation Number 4.c

It is industry's understanding of Section 4.2 of the Kennedy report that given that a plant satisfies the seismic screening checklist then the SFP is likely to have a seismic capacity higher than the screening level capacity. If plant-specific information is conveniently available, additional seismic capacity values will be developed in a manner similar to that described in NUREG/CR-5176.

Comments on Recommendation Number 5

A response to the NRC Recommendation Number 5 requesting industry to provide "Proposed treatment of sites West of the Rocky Mountains" will be provided later. However, as a result of detailed deterministic investigations at and around each site, a better understanding of the sources and causes of earthquakes is developed in the licensing of Western U.S. (WUS) plants. Therefore, it would be reasonable to describe the credible ground motion for WUS sites deterministically.

References:

1. Memorandum, W. Hauffman to S. A. Richards, USNRC, Screening Criteria for Assessing Potential Seismic Vulnerabilities of Spent Fuel Pools at Decommissioning Plants, December 3, 1999.
2. NUREG/CR-5176, Seismic Failure and Cask Drop Analyses of the Spent Fuel Pools at Two Representative Nuclear Power Plants, Lawrence Livermore National Laboratory, January 1989.
3. USNRC, Preliminary Draft Technical Study of Spent Fuel Pool Accidents for Decommissioning Plants, June 16, 1999.
4. SECY-91-135, Conclusions of the Probabilistic Seismic Hazard Studies Conducted for Nuclear Power Plants in the Eastern United States, May 14, 1991.
5. NUREG-1488, Revised Livermore Seismic Hazard Estimates for 69 Nuclear Power Plant Sites East of the Rocky Mountains, October, 1993.

LLNL 5 Hz Spectral Acceleration at 1.0E-6

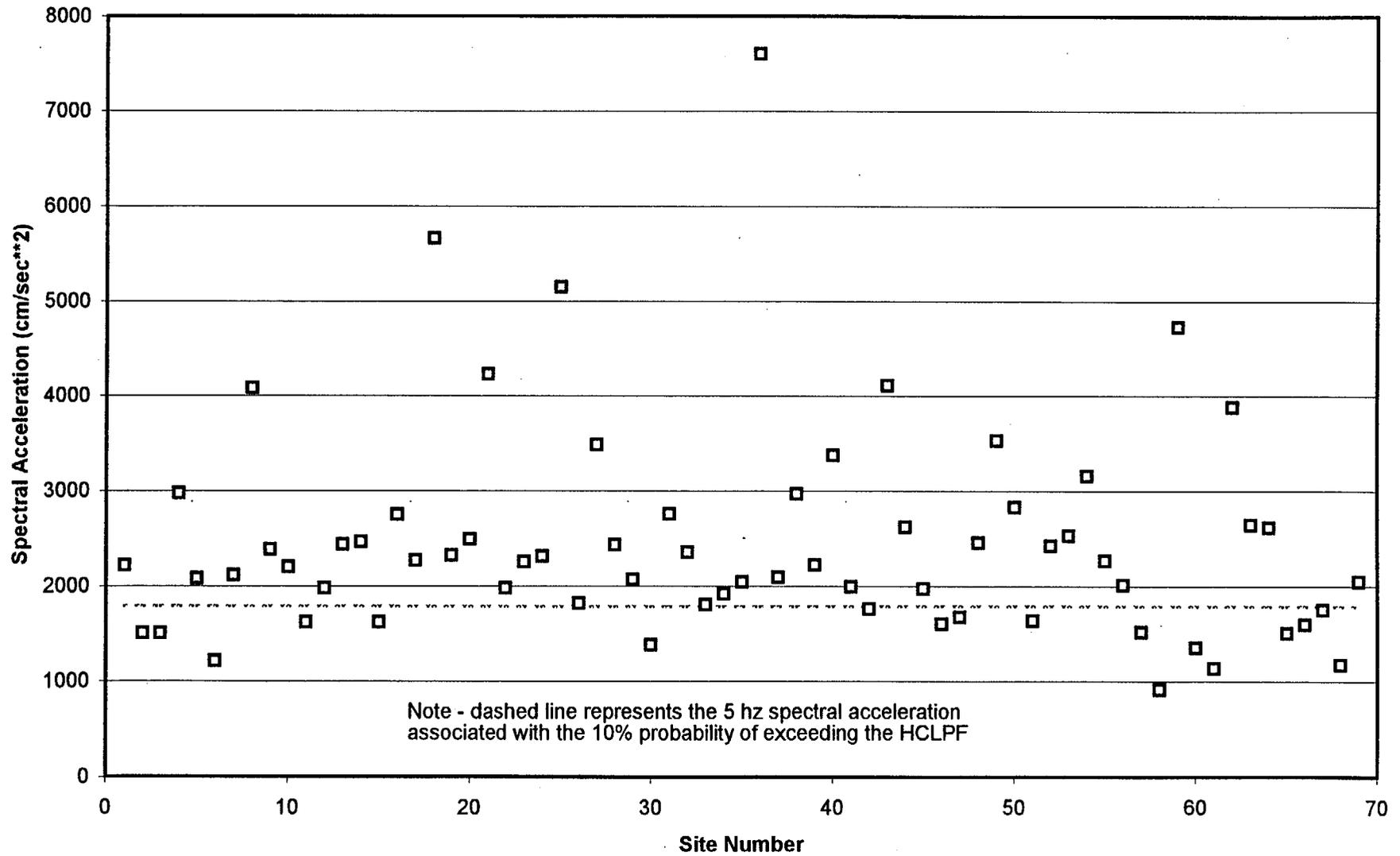


Figure 1

EPRI 5 Hz Spectral Acceleration at 1.0E-6

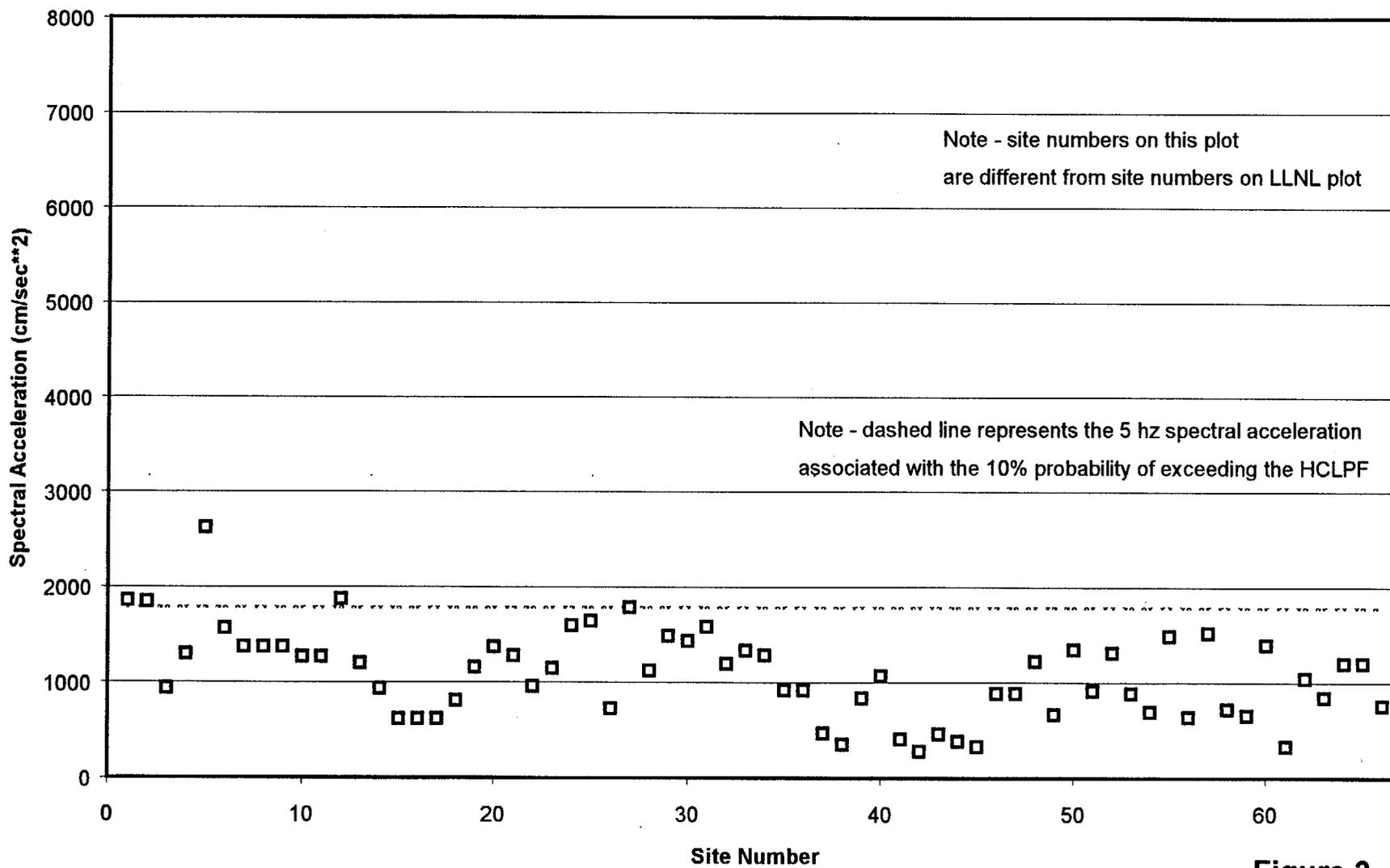


Figure 2

Comparison of 1989 LLNL, 1992 LLNL and EPRI Estimates of Probability of Exceeding Peak Ground Acceleration per Year versus Acceleration - Pilgrim site

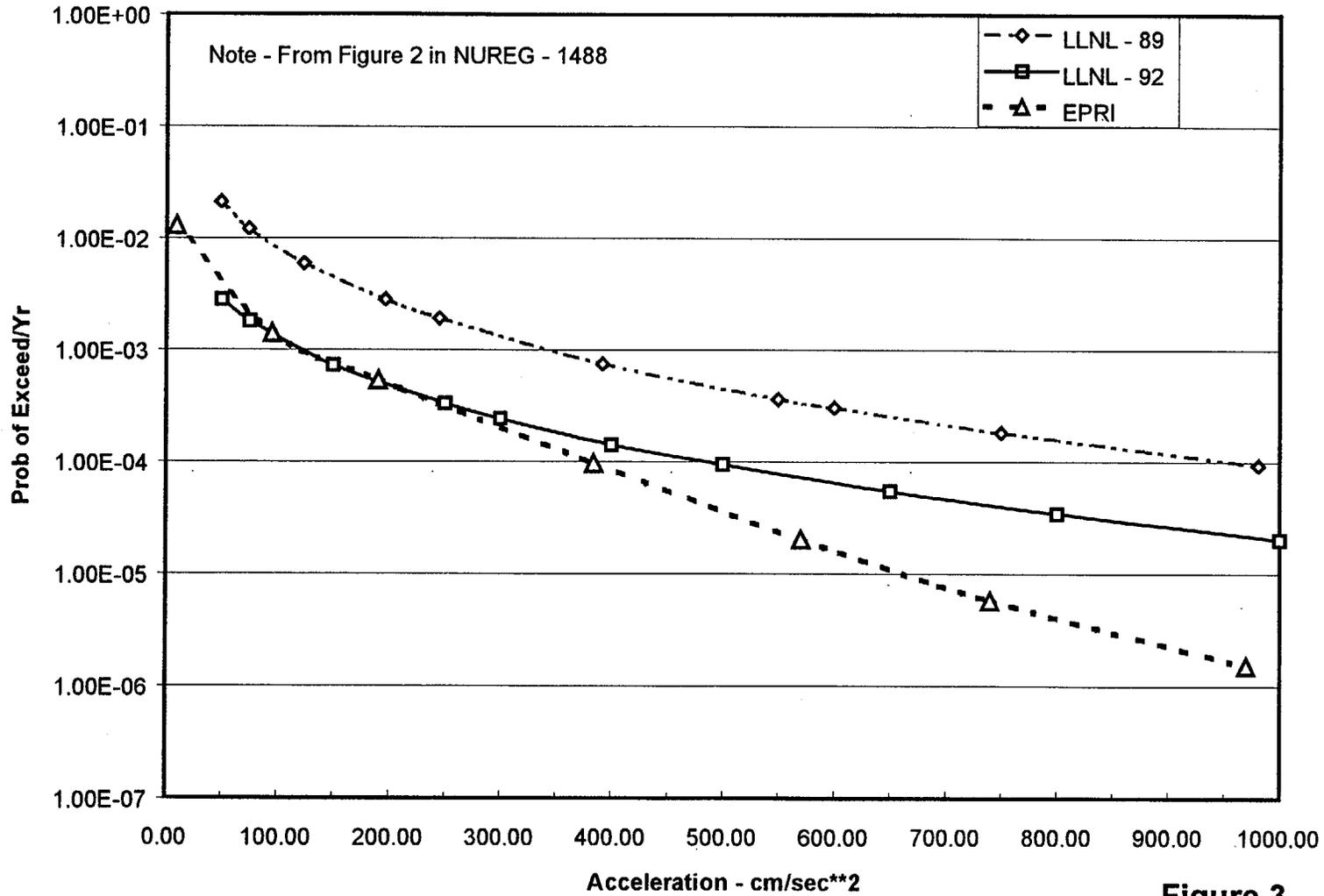


Figure 3

Geometric Mean (LLNL & EPRI)

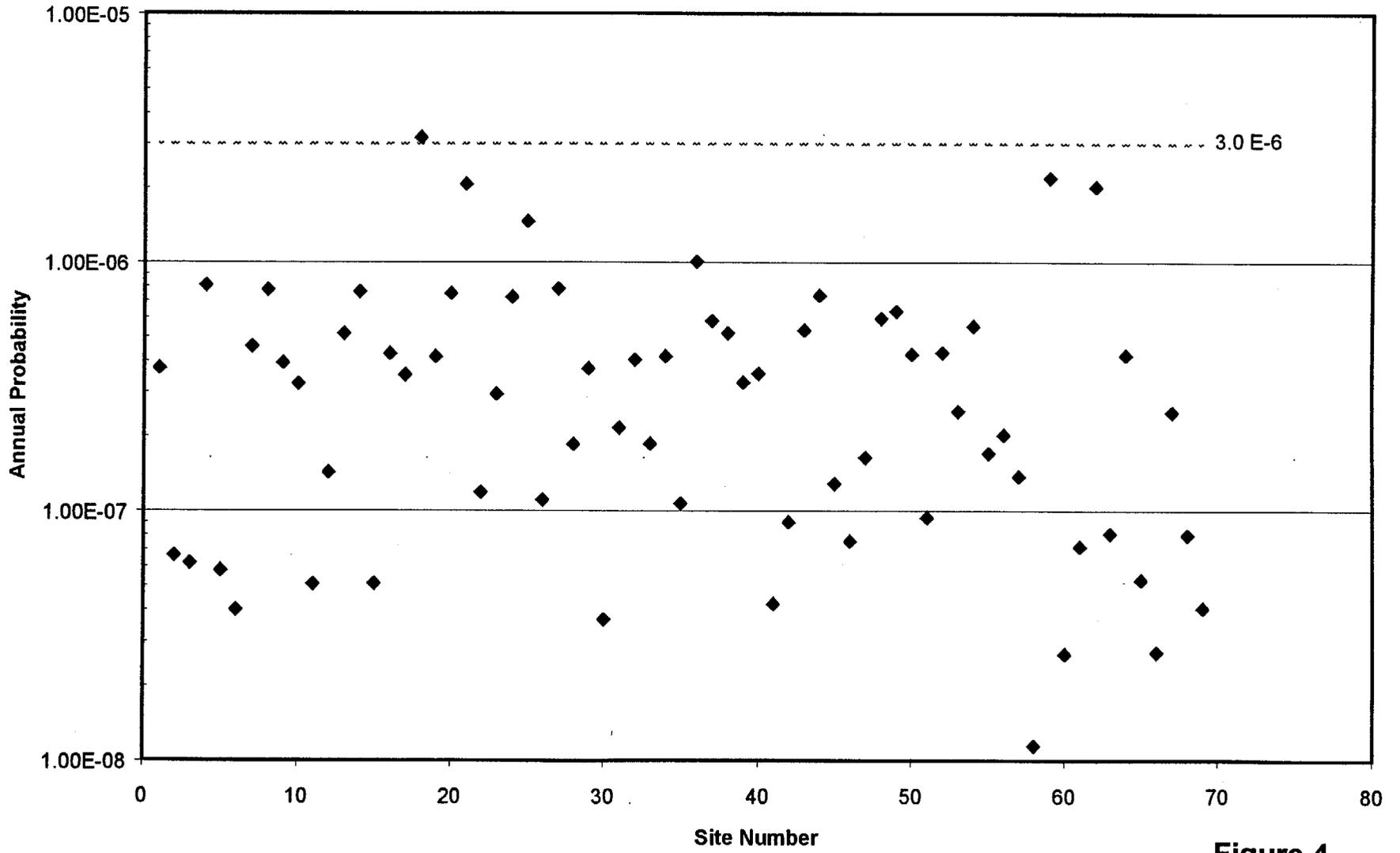


Figure 4