

# Integrated Nonlinear SSI Analysis: the CGD Challenge

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# Acknowledgement

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U.S. DEPARTMENT OF  
**ENERGY**

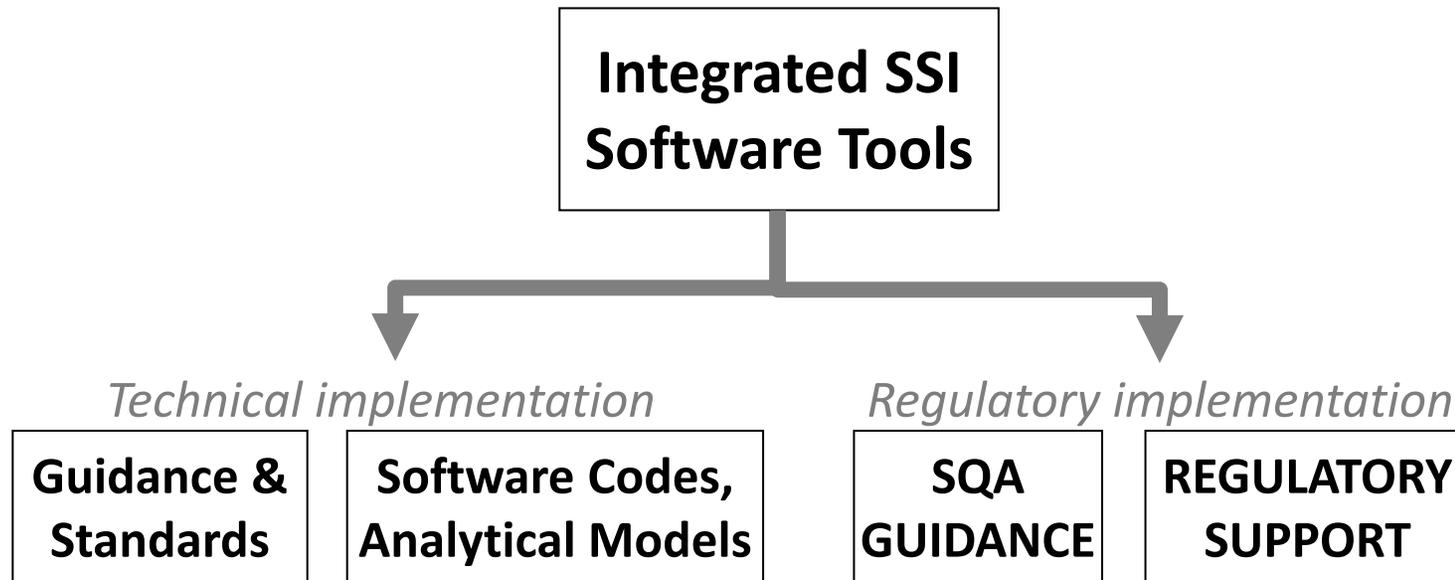
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**REDUCE COST** of new reactors  
by enabling  
modern **SEISMIC ANALYSIS**

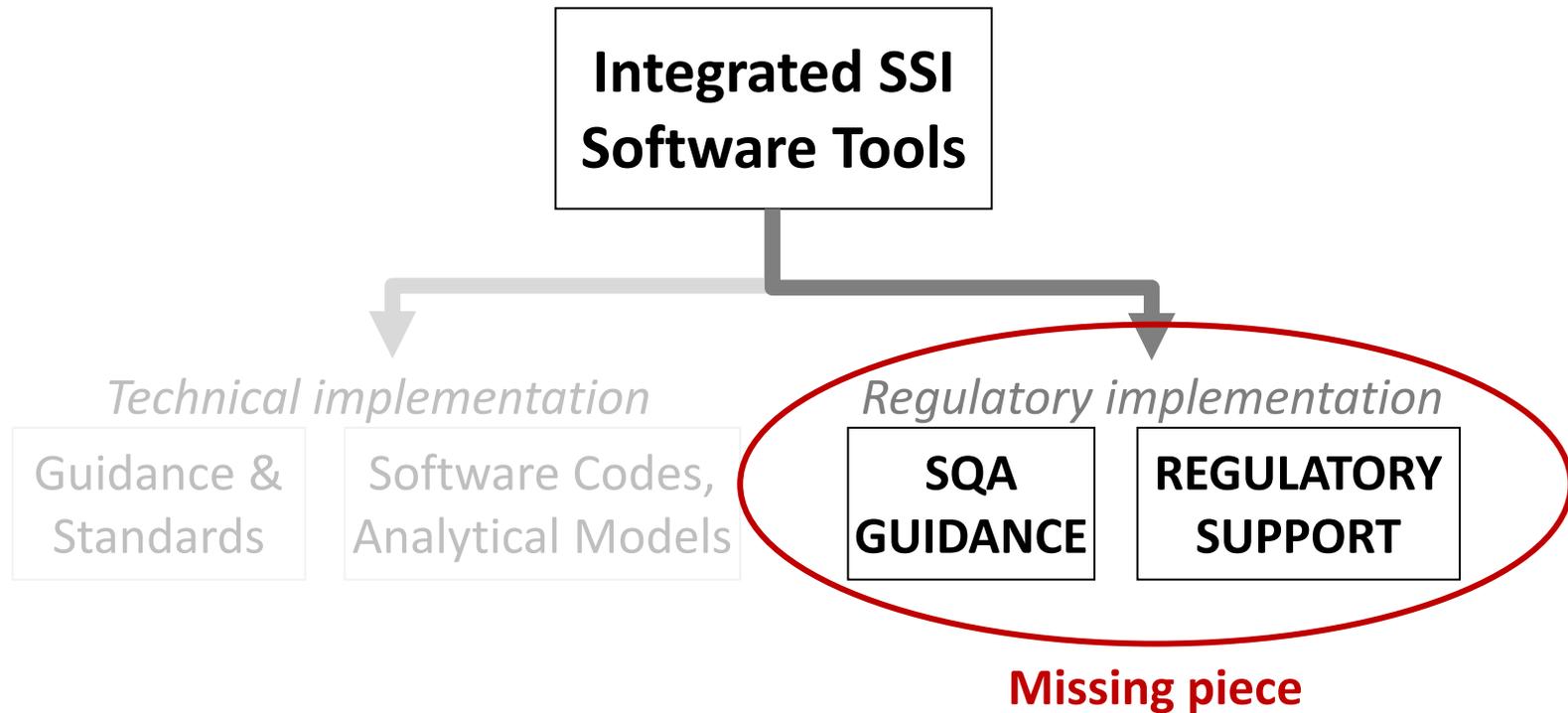
through **SOFTWARE CGD** guidance  
and increased

**REGULATOR CONFIDENCE.**

# Licensing framework



# Licensing framework



# Project outcomes

U.S. Department of Energy  
Federal Identification Number: DE-NE0008857

## CGD Guidance Document

Test Matrix			
Nonlinear Behavior	Software Feature	Response Mode	Test Cases
Seismic Isolation	Elastomeric isolator element	Horizontal cyclic shear	Test Cases
		Vertical axial stiffness	
	Sliding isolator element	Horizontal cyclic shear	
		Vertical axial stiffness	

**Hysteretic response of a lead-rubber isolator subjected to harmonic and three-directional earthquake time series.**

**Objective:**  
To verify the capability of an isolator element to simulate the nonlinear hysteretic response of a lead-rubber (LR) isolator subjected to sinusoidal and three-directional earthquake inputs.

**Problem description:**  
The LR bear lead core is modeled as a combination of a central lead core,  $T_b$  and extended by Naggaraja  $s$ , and a central  $e$  modeled as a  $r$  of the central  $6$ ) and extended the basic hysteretic model of the LR bearing in shear. It is characterized by the strength at zero-displacement ( $Q_0$ ), (i.e., characteristic strength), post-yield stiffness ( $k_1$ ), and the elastic stiffness ( $k$ ). The yield strength and yield displacement are denoted by  $F_y$  and  $u_y$ , respectively.

**Figure 1. Hysteretic behavior of the LR bearing.**

Kumar et al. (2015) developed a mathematical model for LR isolator with hysteretic response in shear, strength degradation due to heating of the lead core in axial tension, and buckling in axial compression. Herein, the isolator element need simulate only bilinear hysteretic response shown in Figure 1, and exhibit linear elastic behavior in the axial, rotational and torsional directions. For the test cases, the isolator is to be modeled as a two-node element with nodes at the ends. The axial direction (i.e., vertical orientation) of the isolator element directions are the local- $y$  and local- $z$  axes.

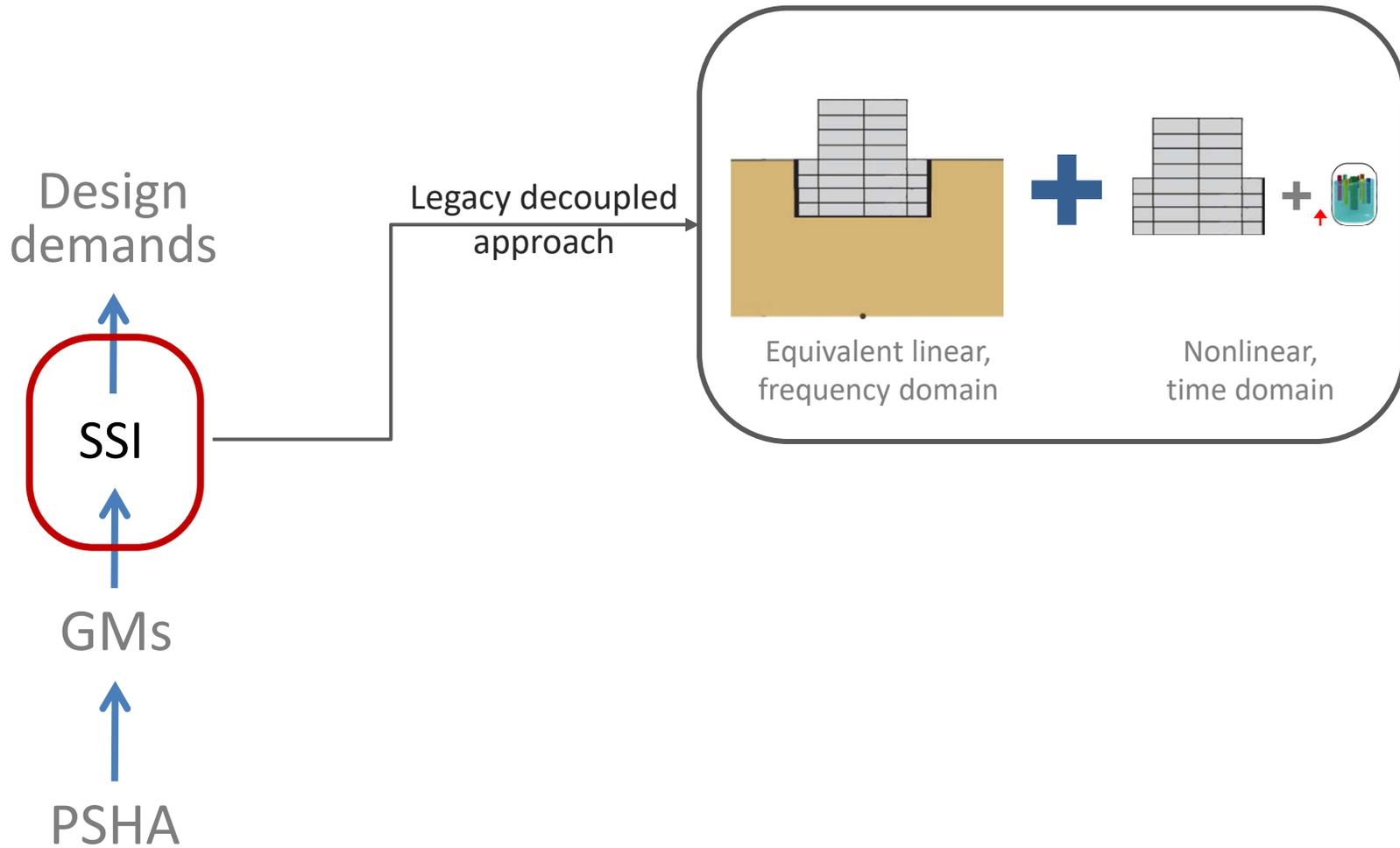
**Case 1:** A massless LR isolator element is fixed at node 1. A three-cycle with increasing amplitudes of 100 mm, 150 mm, and 200 mm, is applied in the  $x$ -direction (see Figure 2a), at a frequency of 0.5 Hz. The rotational degrees of freedom are restrained.

**Case 2:** A massless LR isolator element is subjected to three-directional earthquake inputs at node 1 as shown in Figure 2b. The rotational degrees of freedom are restrained.

## Outreach

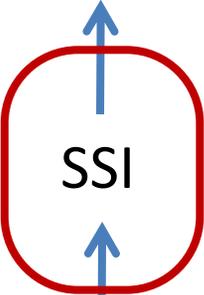


# Legacy SSI approach



# Integrated SSI approach

Design demands

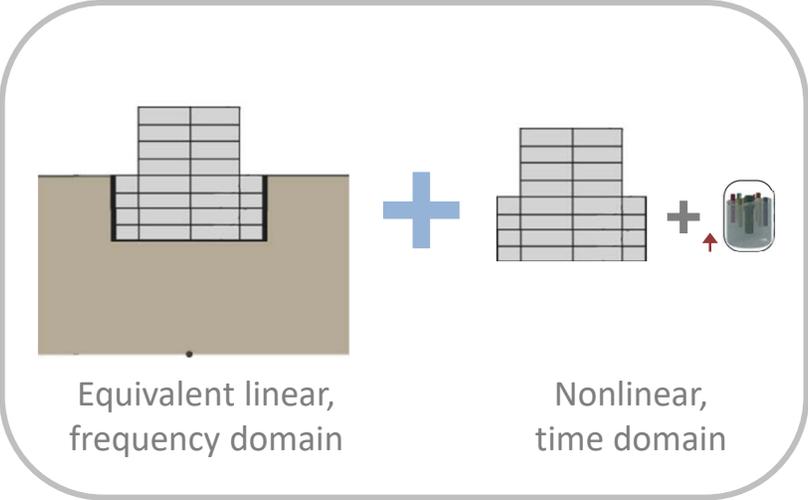


SSI

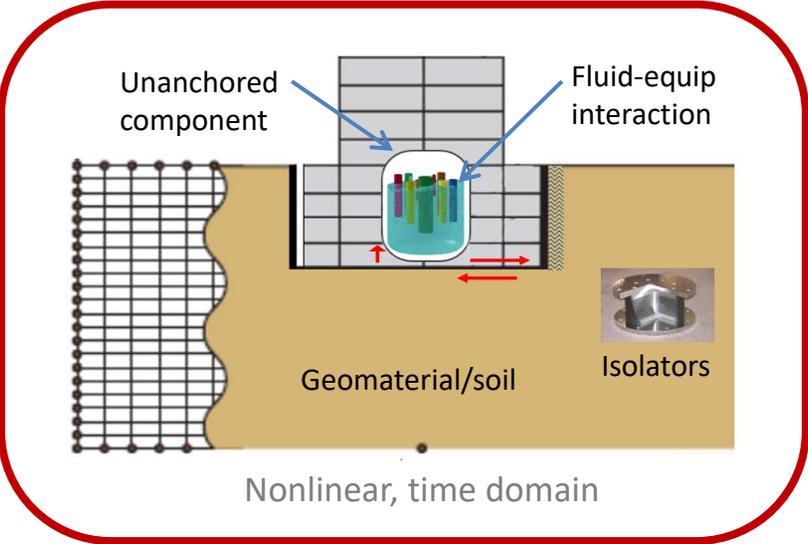
GMs

PSHA

Legacy decoupled approach



Integrated approach



# Commercial grade dedication

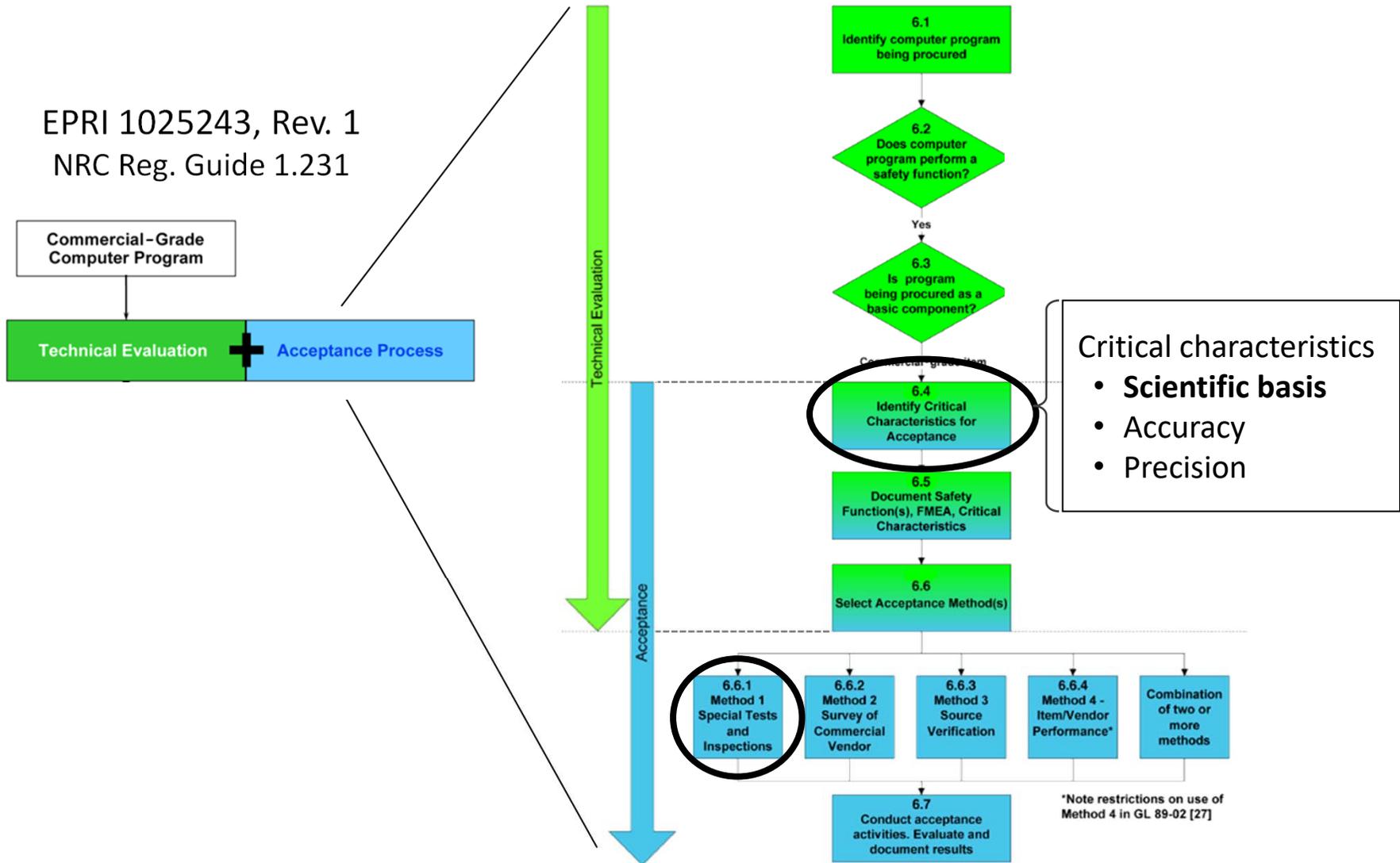
## **10 CFR Part 21: Dedication**

Provide reasonable assurance item will perform  
its intended safety function

Achieved by identifying and verifying acceptability of  
critical characteristics

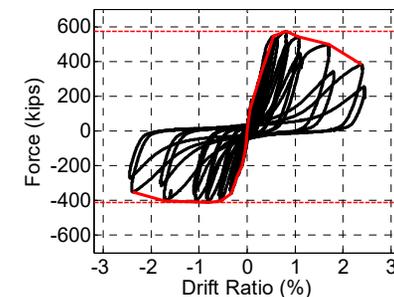
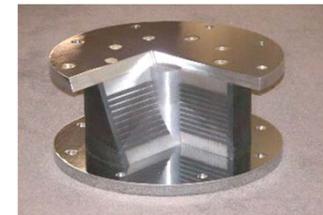
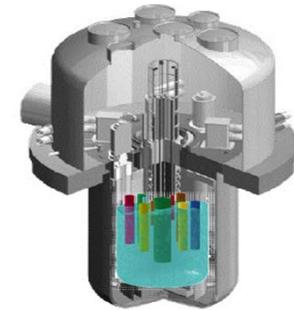
# CGD of analysis software

EPRI 1025243, Rev. 1  
NRC Reg. Guide 1.231



# Prioritized nonlinear behaviors

- Fluid-structure interaction
- Seismic isolation and damping systems
- Geomaterials (i.e., soil)
- Reinforced concrete
- Structural steel
- Interface/contact (incl. RV support)
- Deep embedment



# Test matrix excerpt

Test Matrix Excerpt			
Nonlinear Behavior	Software Feature	Response Mode	Test Cases
Seismic Isolation	Elastomeric isolator element	Horizontal cyclic shear	2, 3, 12
		Vertical axial stiffness	3, 12
	Sliding isolator element	Horizontal cyclic shear	3, 13
		Vertical axial stiffness	3, 13

## Test Cases

34. Response of a uniaxial nonlinear fluid viscous damper-steel brace assembly subjected to unidirectional, harmonic and earthquake time series.

Objective:

To verify the capability of a uniaxial damping element to simulate the cyclic response of a nonlinear fluid viscous damper subjected to unidirectional horizontal sinusoidal and earthquake inputs

Problem description:

The internal axial resistance of a nonlinear fluid viscous damper (FVD) is proportional to a fractional power of the relative velocity across the dashpot:

$$F = C|V|^{\alpha}$$

where  $F$  is the internal axial force,  $V$  is the relative velocity across the dashpot,  $C$  is the damping

coefficient, and  $\alpha$  is a coefficient, and  $\alpha$  is 1.995 have shown spring in series with dampers. Verification

Case 1: A massless,  $x(t)$ , with an amplitude boundary condition properties are: damping exponent ( $\alpha$ ) = 0.6,

Case 2: A massless, frame, 3 m long, are identified in Table formulation). The its properties are:  $c = 1$  the frame. The nodes two beam-column of 1D horizontal groups

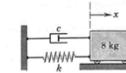
### 3. Free vibration of a linear spring-damper-mass system

Objective

To verify the uniaxial behavior of a linear viscous damper and spring

Problem description

Free-vibration response of a damper-spring-mass system (Figure 6) due to initial displacement is studied. The system dynamic properties are summarized in Table 6.



The finite element properties shown released from rest 2, and 3 seconds.

Reference solution

The equation of a Figure 7:

Hysteretic response of a lead-rubber isolator subjected to harmonic and three-directional earthquake time series.

Objective:

To verify the capability of an isolator element to simulate the nonlinear hysteretic response of a lead-rubber (LR) isolator subjected to sinusoidal and three-directional earthquake inputs.

Problem description:

The LR bearing is composed of alternating layers of natural rubber and steel shim plates, and a central lead core that is added to dissipate energy. The shear resistance of the LR bearing can be modeled as a combination of the viscoelastic behavior of the rubber layers and the hysteretic behavior of the central lead core. The hysteretic component is based on the model proposed by Park *et al.* (1986) and extended by Nagayama *et al.* (1989) for analysis of seismic isolation systems. Figure 1 presents the basic hysteretic model of the LR bearing in shear. It is characterized by the strength at zero-displacement ( $Q_0$ ), (i.e., characteristic strength), post-yield stiffness ( $k_p$ ), and the elastic stiffness ( $k_e$ ). The yield strength and yield displacement are denoted by  $F_y$  and  $u_y$ , respectively.

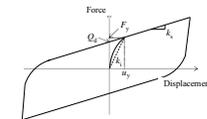


Figure 1. Hysteretic behavior of the LR bearing in shear

Kumar *et al.* (2015) developed a mathematical model for LR isolator with the capability to capture hysteretic response in shear, strength degradation due to heating of the lead core, axial-shear interaction, cavitation in axial tension, and buckling in axial compression.

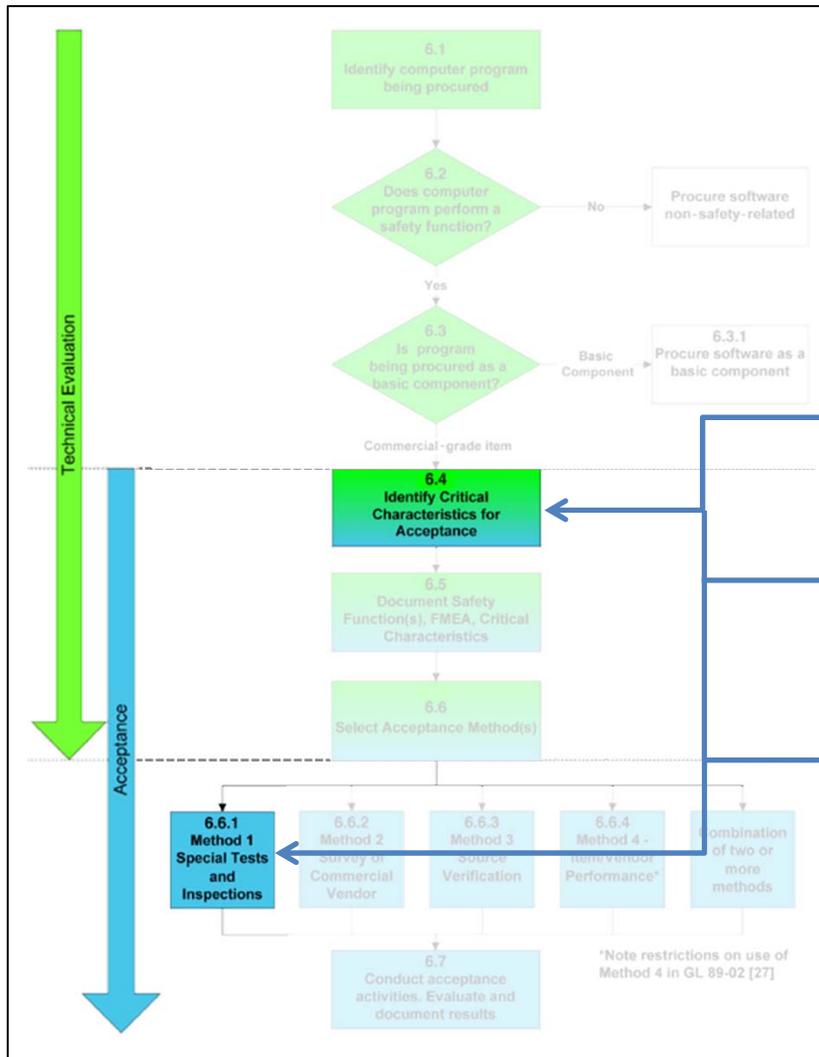
Herein, the isolator element need simulate only bilinear hysteretic response (Park *et al.*, 1986) in shear as shown in Figure 1, and exhibit linear elastic behavior in the axial, rotational, and the torsional directions. For the test cases, the isolator is to be modeled as a two-node element with six degrees of freedom at each node. The axial direction (i.e., vertical orientation) of the isolator element is the local-x axis, and the shear directions are the local-y and local-z axes.

Case 1: A massless LR isolator element is fixed at node 1. A three-cycle sinusoidal displacement history, with increasing amplitudes of 100 mm, 150 mm, and 200 mm, is applied at node 2 along shear (y) direction (see Figure 2a), at a frequency of 0.5 Hz. The rotational degrees of freedom at node 2 are restrained.

Case 2: A massless LR isolator element is subjected to three-directional (vertical and two horizontal) earthquake inputs at node 1 as shown in Figure 2b. The rotational degrees of freedom at nodes 1 and 2 are

# End-user software CGD

## Software CGD



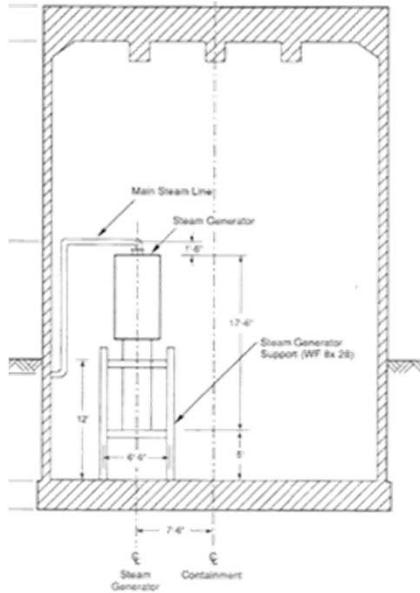
## Test matrix and problems

#	Software Feature	Response Mode	Response Parameter	Test Case
1	Fluid element	Impulsive (acoustic)	Frequency and magnitude of pressure waves	(8), (9), (10), (11), (20)
		Convective (sloshing)	Frequency and magnitude of fluid displacement	1, (8), (9)
2	Fluid-structure coupling algorithm	Normal	Force/stress resultant on flexible structure	(8), (9), (10), (11), (20)
3	Elastomeric isolator element	2D horizontal (cyclic shear)	Cyclic force-displacement	2 (12)
		Vertical/axial stiffness	Force-displacement	2 (12)
4	Sliding isolator element	2D horizontal (cyclic shear)	Cyclic force-displacement	(13)
		Vertical/axial stiffness	Force-displacement	(13)
5	Viscous damper	uniaxial, linear		3
6	Metallic yield damper	uniaxial	Cyclic force-displacement	(14)
		Cyclic pure shear (Simple shear)	Cyclic force-displacement	4
7	Geomaterial constitutive model	Wave propagation/SRA, equivalent linear	Free-field response spectra, strain/acc. profile with depth	(15), (21), (25?), (26), (27)
		Inertial/kinematic SSI response, equivalent linear	In-structure response	(21), (26)
		Wave propagation/SRA, nonlinear	Free-field response spectra, strain/acc. profile with depth	(16), (25?), (27?), (28)
		Inertial/kinematic SSI response, nonlinear	In-structure response	(26?)
		Lateral soil pressure	Pressure resultant on structure	(17)
		Uniaxial compression Triaxial compression		

# Example Test Cases

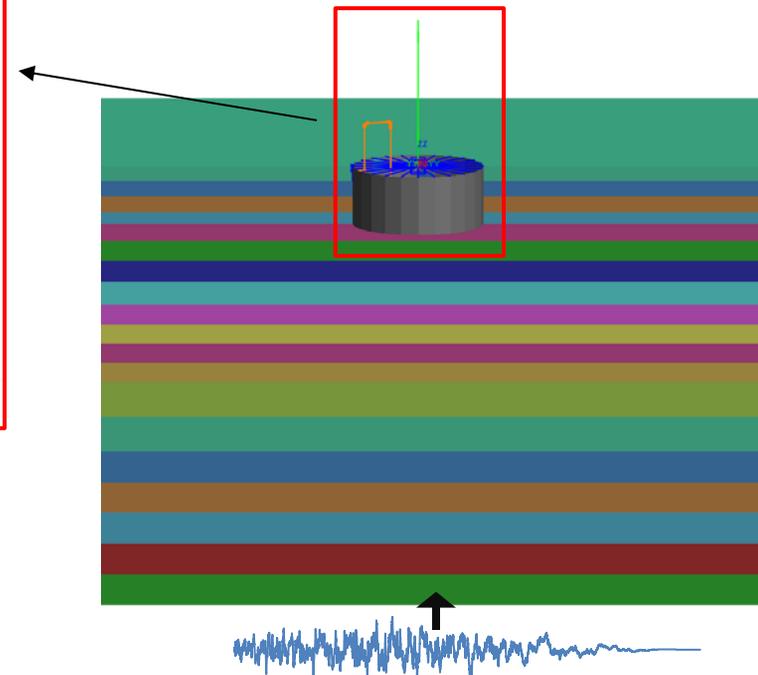
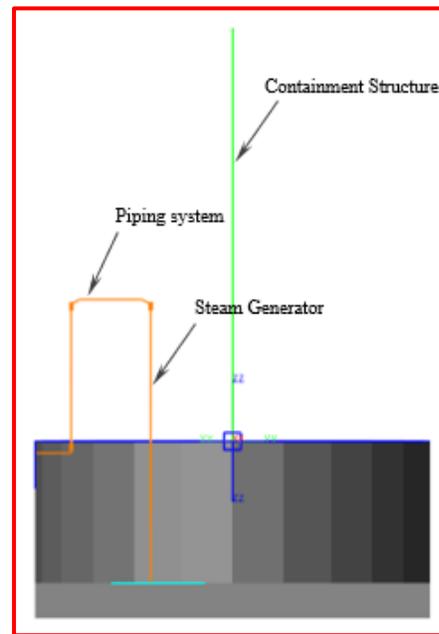
Feature	Software Feature	Response Mode	Response Parameter	Test Case
1	Fluid element	Impulsive (acoustic)	Frequency and magnitude of pressure waves	8, 9, 10, 11, 20, 22
		Convective (sloshing)	Frequency and magnitude of fluid displacement	1, 8, 9
2	Fluid-structure coupling algorithm	Normal	Force/stress resultant on flexible structure	8, 9, 10, 11, 20, 22
3	Elastomeric isolator element	2D horizontal (cyclic shear)	Cyclic force-displacement	12, 2
		Vertical/axial stiffness	Force-displacement	12, 3
4	Sliding isolator element	2D horizontal (cyclic shear)	Cyclic force-displacement	13
		Vertical/axial stiffness	Force-displacement	13, 3
5	Viscous damper	uniaxial, linear		34, 3
6	Metallic yield damper	uniaxial	Cyclic force-displacement	14
		Cyclic pure shear (Simple shear)	Cyclic shear stress-strain	4, 25, 37
		Wave propagation/SRA, equivalent linear	Free-field response spectra, strain/acc. profile with depth	15, 21, 26, 27, 36
7	Geomaterial constitutive model	Inertial/kinematic SSI response, equivalent linear	In-structure response	21, 26
		Wave propagation/SRA, nonlinear	Free-field response spectra, strain/acc. profile with depth	16, 27, 28, 37
		Inertial/kinematic SSI response, nonlinear	In-structure response	26
		Lateral soil pressure	Pressure resultant on structure	17
		Uniaxial compression	TBD	TBD
		Triaxial compression	TBD	TBD
8	Beam fiber section	Axial	Force-displacement	6
		Flexure	Force-displacement	5, 6
		Shear	Force-displacement	5, 6
9	Beam with plastic hinge	Axial	Force-displacement	5, 31
		Flexure	Force-displacement	5, 31
		Shear	Force-displacement	5, 31
10	Shell layered section	In plane membrane actions	Force-displacement	19, 32
		Flexure	Force-displacement	19, 32
11	Concrete material model (Nonlinear/cyclic)	Uniaxial cyclic	Cyclic force-displacement	6
		In-plane membrane cyclic	Cyclic force-displacement	19, 32
12	Rebar material model	Uniaxial cyclic	Cyclic force-displacement	6, 14
13	Structural steel material model	Uniaxial cyclic	Cyclic force-displacement	14
14	Contact algorithm	Normal	Nonlinear compressive and tensile response	23, 24, 35
		Tangential Friction	Static and dynamic contact force	7, 24, 35
15	1D spring elements	uniaxial, linear	Force-displacement	3
		Uniaxial, cyclic nonlinear	Force-displacement	14

# Test case: soil-structure response



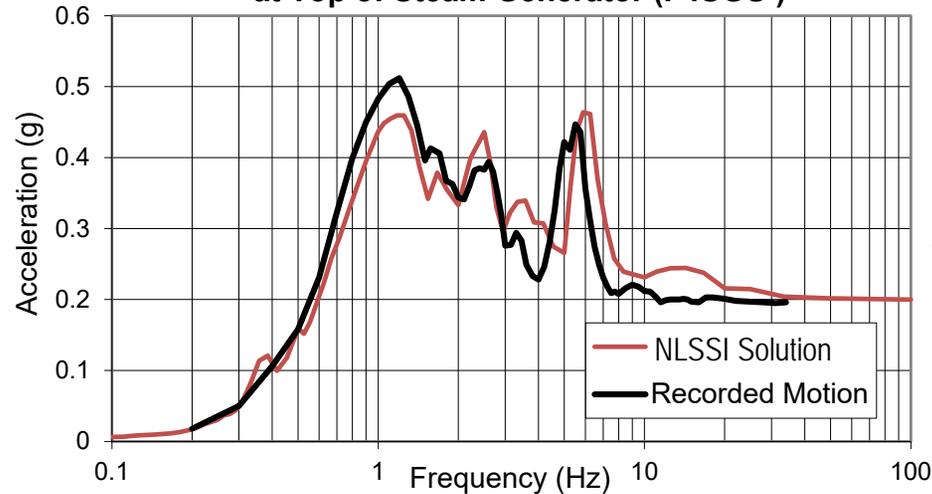
## Lotung large scale seismic test SSI experiment

- Equivalent-linear soil properties
- Comparison with recorded response during seismic event

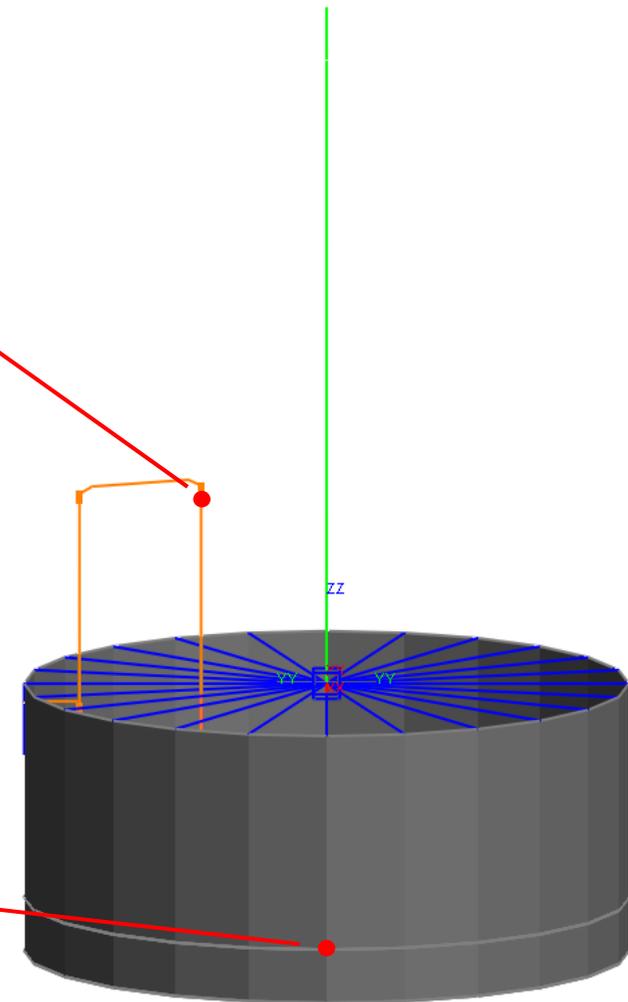
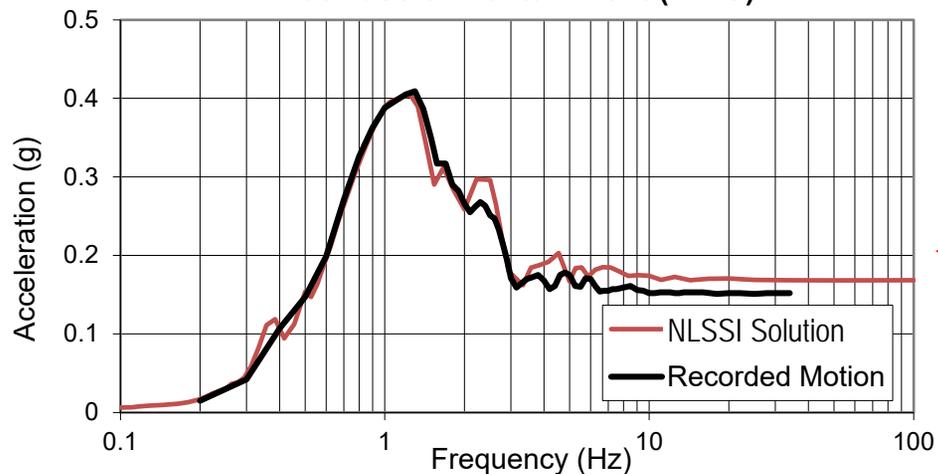


# Test case: soil-structure response

LSST Acceleration Response Spectra 5% Damping at Top of Steam Generator (F4SGU)



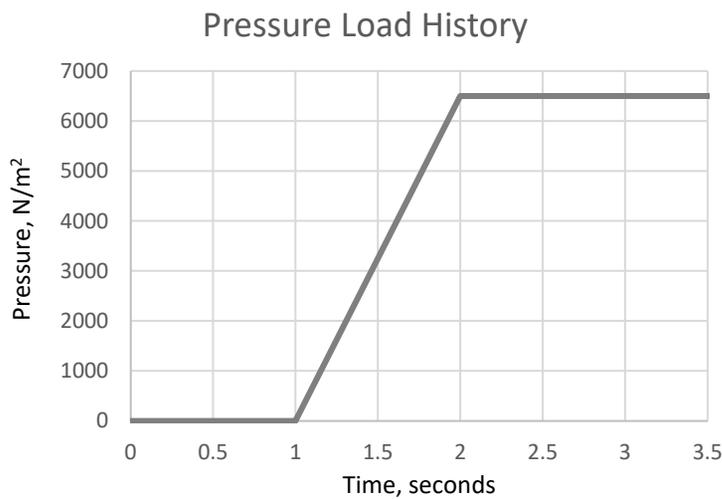
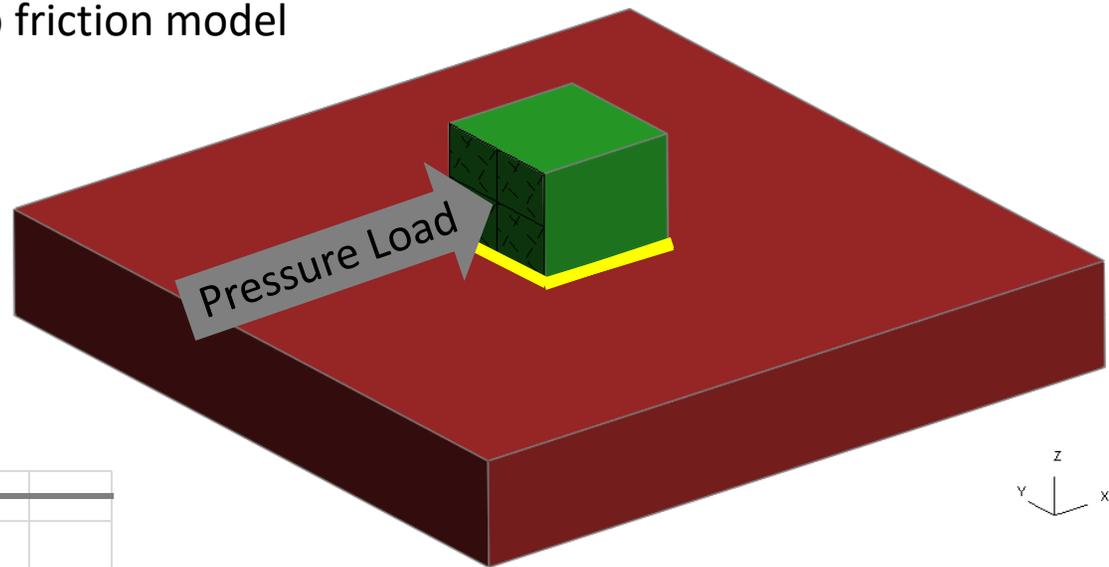
LSST Acceleration Response Spectra 5% Damping at Base of Containment (F4LS)



# Test case: contact surface sliding

## Sliding response of a solid cube

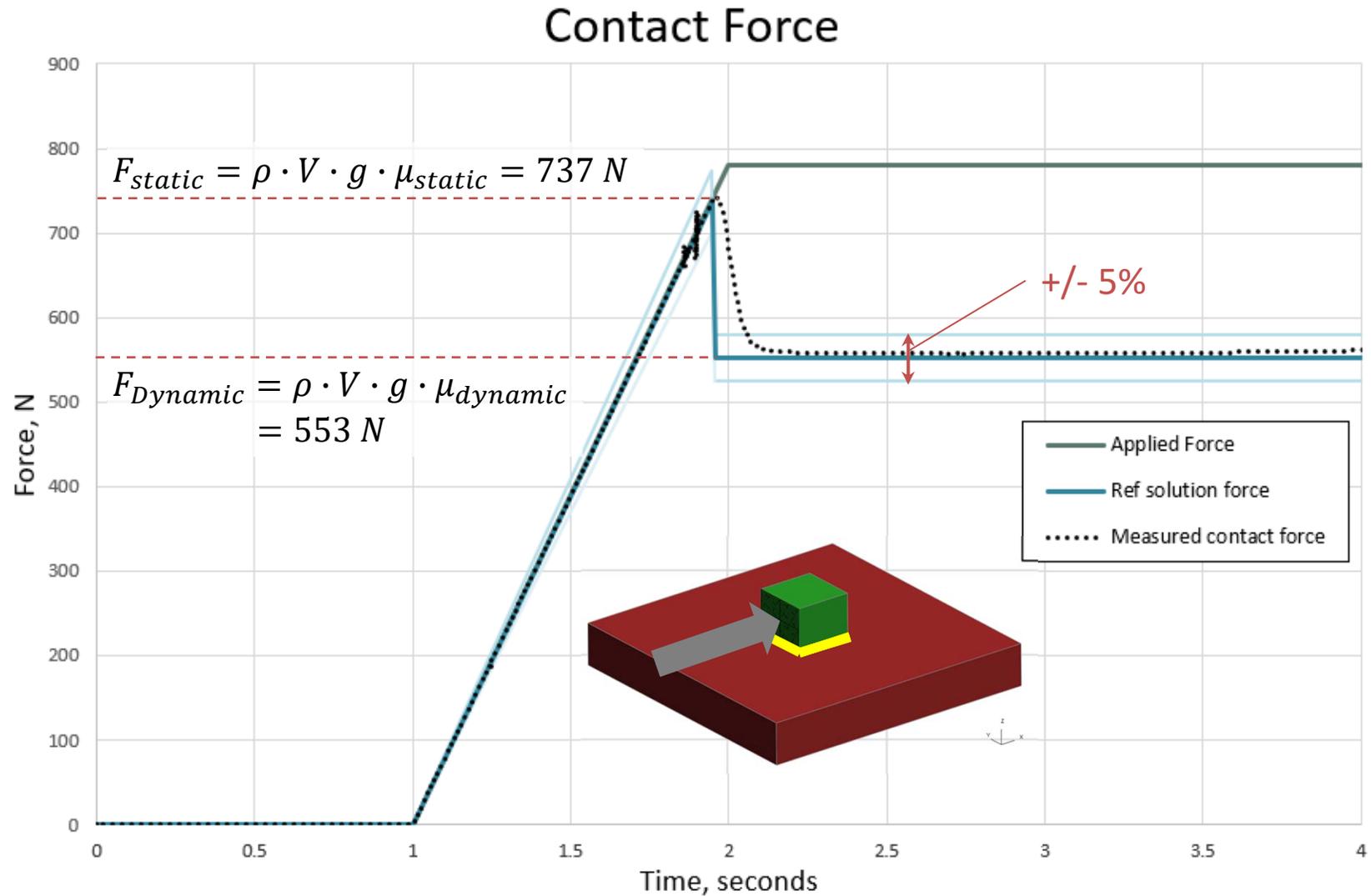
- Contact surface (penalty-based)
- Comparison with Coulomb friction model



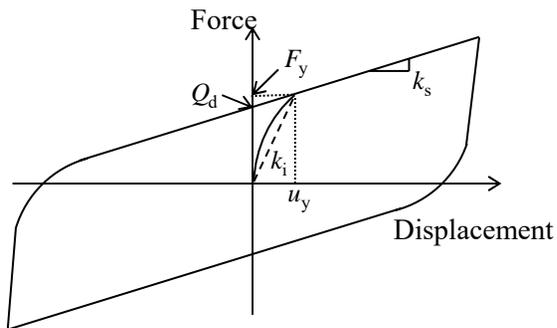
$$F_{static} = \rho \cdot V \cdot g \cdot \mu_{static} = 737 \text{ N}$$

$$F_{dynamic} = \rho \cdot V \cdot g \cdot \mu_{dynamic} = 553 \text{ N}$$

# Test case: contact surface sliding



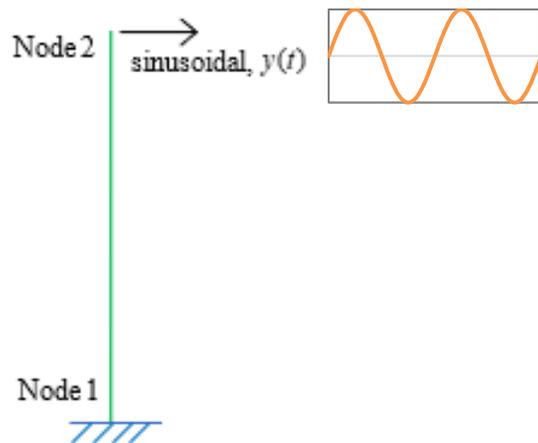
# Test case: elastomeric bearing response



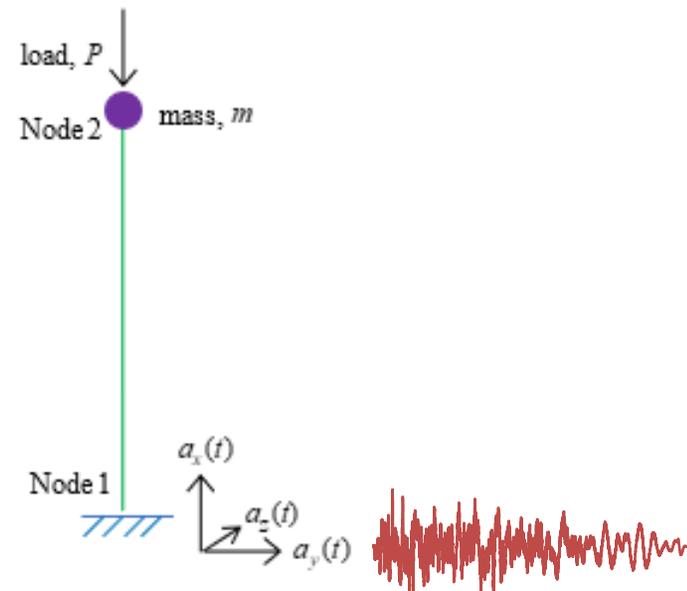
## Hysteretic response of lead-rubber isolator

- Bilinear hysteretic shear response
- Linear elastic axial response
- Code-to-code verification

### Case 1: 1D sinusoidal displacement

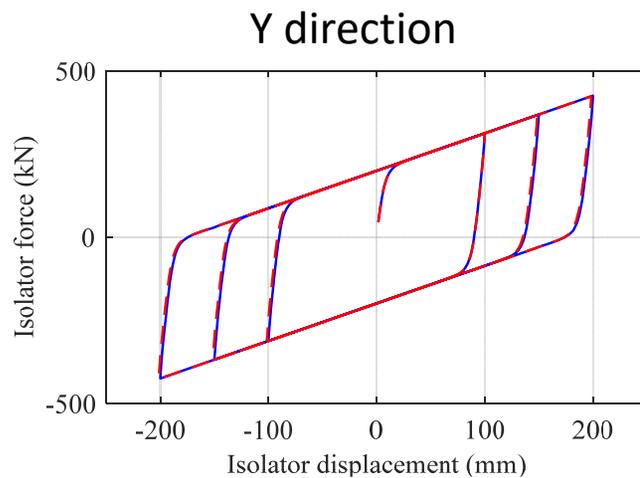
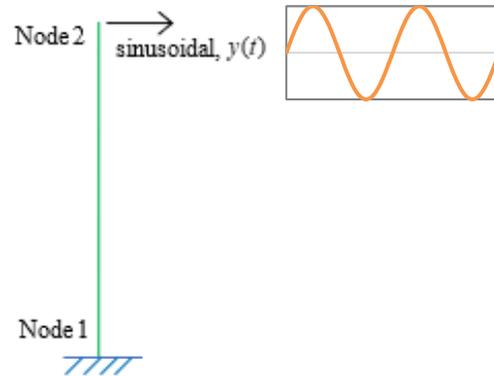


### Case 2: 3D EQ acceleration input



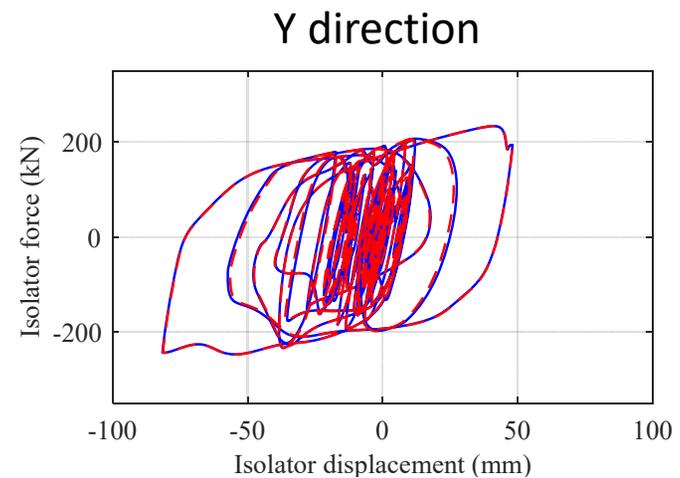
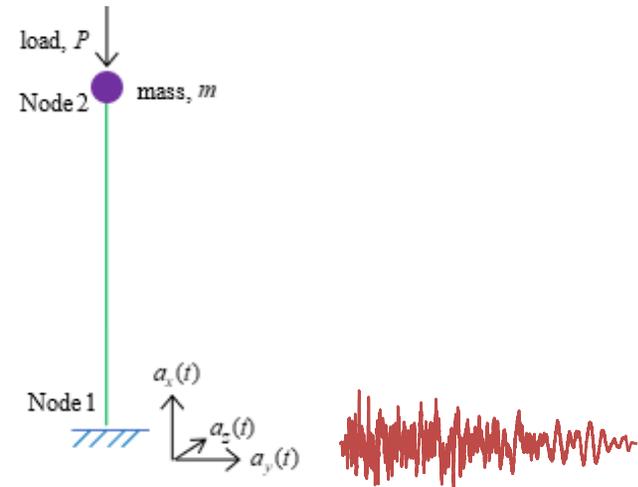
# Test case: elastomeric bearing response

## Case 1: 1D sinusoidal displacement



--- MASTODON — OpenSees

## Case 2: 3D EQ acceleration input



--- MASTODON — OpenSees

# Q&A + DISCUSSION

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