

# Natural Phenomena Hazards October 2020 Meeting

## RISK-CONSISTENT VERTICAL GROUND MOTION SPECTRA

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# Outline

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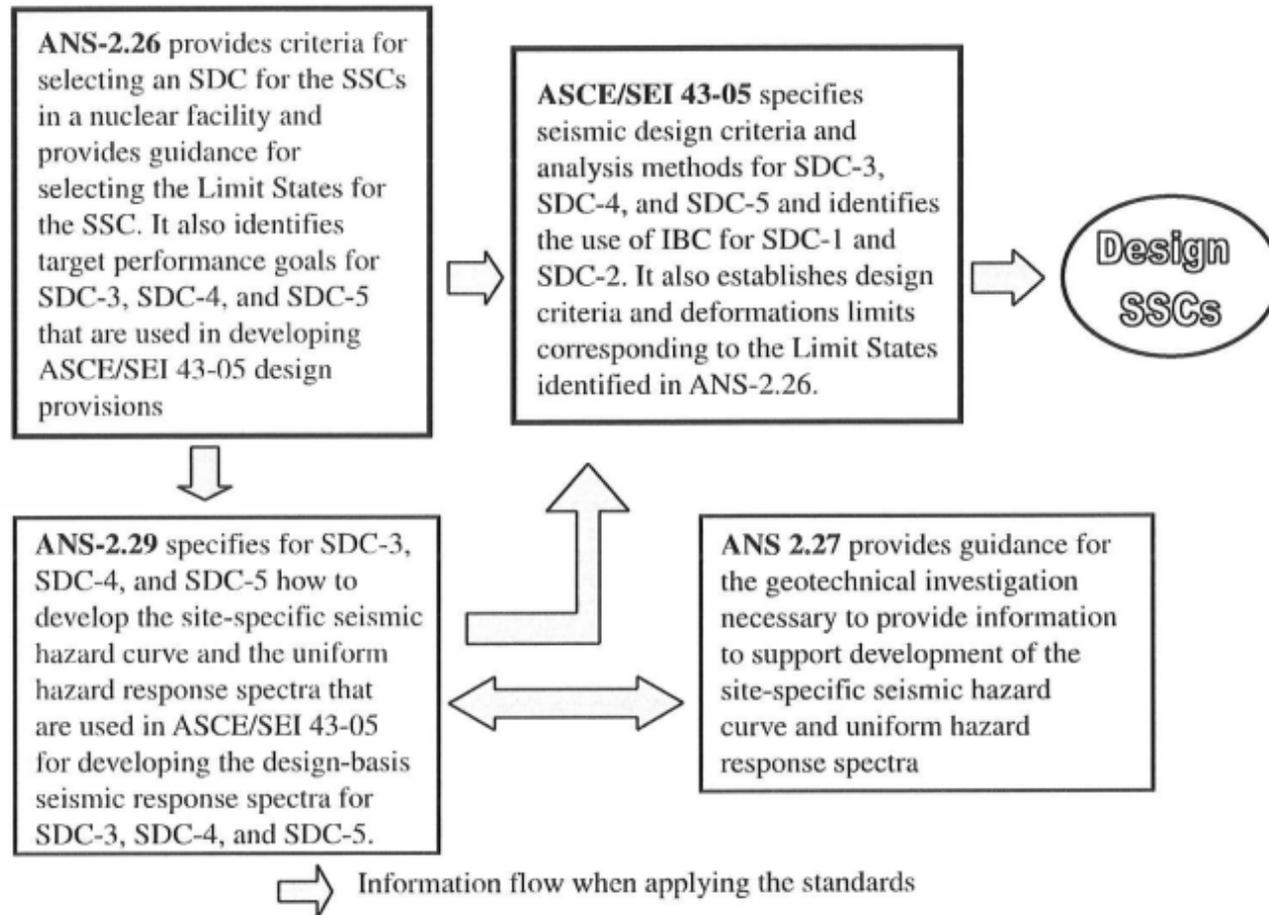
- Background on performance-based design response spectrum (DRS)
- Current Practice for Vertical DRS
- Motivation for Risk-Consistent Vertical DRS
- Proposals for Risk-Consistent Vertical DRS
- Development of Risk-Consistent Adjustment Factor
- Conclusions

# Input Motion for Performance-Based Design

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- Seismic performance for SSCs with serious failure consequences is recognized by the designation of a seismic design basis (SDB), which varies by SSC
- The SDB for nuclear facilities is defined by
  - (1) acceptable level of damage or limit state (LS), and
  - (2) a seismic design category (SDC) 1 through 5
- SDC determines the annual frequency of exceedance (AFE) of acceptable performance level (*Target performance goal ( $P_F$ ) – defined in ANS 2.26*)
- More serious consequences of unacceptable performance → Higher SDCs  
Higher SDC requires lower  $P_F$  to achieve approximately balanced risk through different SDCs
- Lower  $P_F$  is achieved by higher design basis earthquake (DBE)

# Nuclear Seismic Design Standards



(Figure extracted from ANS-2.26)

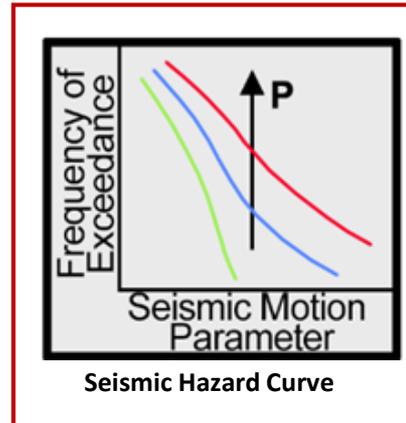
# Performance Objective

Performance computed by risk convolution integral

- Probabilistic seismic hazard curve for  $A_{REF}$
- Seismic fragility function

$A_{REF}$ : Reference ground motion spectrum parameter, e.g., horizontal PGA

The risk convolution integral is strongly influenced by the slope of the hazard curve

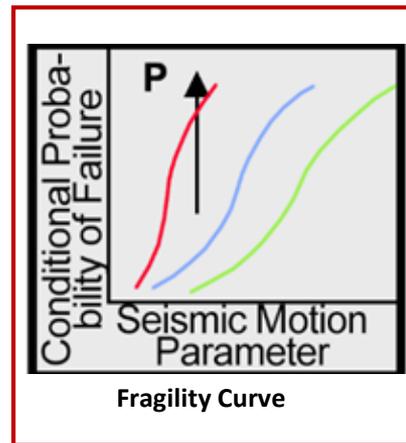


“Linear” in log-log space. Defines the UHRS shape, which is used in fragility.

Hazard curve slope

$$P_f = \int_0^{\infty} \left( \frac{-dH(a)}{da} \right) P_{f/a} da$$

Fragility function



$$A = A_m \epsilon_R \epsilon_U$$

$$A_m = F_{Sm} A_{REF}$$

$$\beta = \text{SQRT}[\beta_R^2 + \beta_U^2]$$

(Adapted from EPRI 3002012994)

# Input Ground Motion Spectrum

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***Selection of the seismic ground motion (DRS for Design, RE for SPRA) is critical***

The goal is:

- Identify the design response spectra (DRS) representing DBE that achieves the performance goal

To achieve the performance goal, ASCE 43 defines the DRS as:

$$\text{DRS} = \text{SF} \times \text{UHRS}_{H_p}$$

where  $\text{UHRS}_{H_p}$  is the UHRS determined for exceedance frequency  $H_p$  and SF is

the scale factor, which is function of slope factor ( $A_R$ ), a function of  $\frac{-dH(a)}{da}$

# Vertical DRS Ground Motion

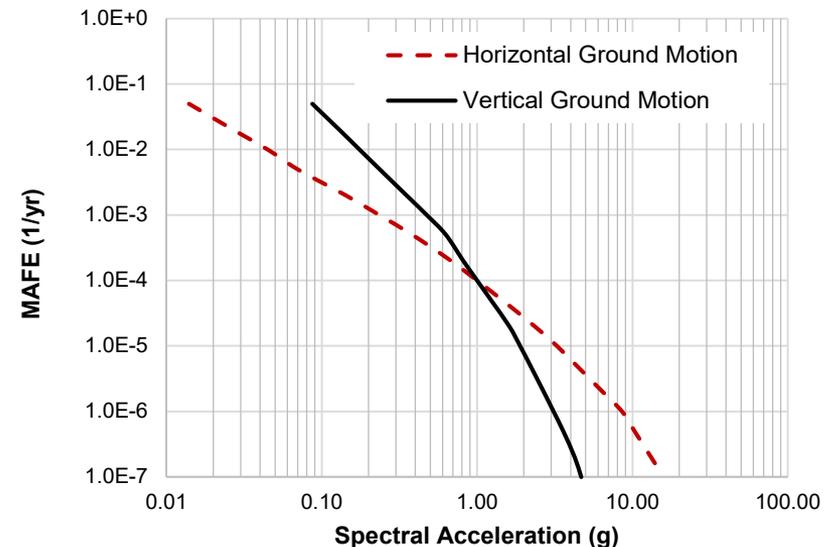
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## Common Methods in Practice:

- Apply vertical-to-horizontal (V/H) ratios to the horizontal DRS
- Apply the SF factors calculated for the horizontal DRS to the vertical UHRS
- Both approaches result in ***hazard-consistent*** vertical motion, and ***not risk-consistent***
- Vertical ground motion hazard can have very different ***slope***
- It is a common simplification since vertical ground motion is usually a secondary contributor to limit-states for most components
- If vertical motion governs limit-state of a component, e.g., shear in a beam, the vertical DRS **may not result in** meeting the ***target performance goal***

# Horizontal vs Vertical Hazard Curves

- Slope of horizontal and vertical hazard curves are dissimilar
- Hazard-consistent vertical DRS introduces performance bias for components **governed** or strongly **influenced** by **vertical motion (conservative or unconservative)**
- Bias can be up to ~50% in mean risk for these components
- Magnitude of bias depends on difference in hazard curve slopes



Example horizontal and vertical hazard curves

# Methods for Risk-Consistent Vertical Spectrum

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## Starting from a Clean Slate

- Apply V/H ratios to the the horizontal UHRS at MAFEs =  $H_p$  and  $10H_p$
- Develop independent SF factors for the resulting vertical UHRS
- Results in risk-consistent vertical DRS that account for difference in slope

## Starting from an Existing Design / Analysis

- Many analyses already have DRS defined, ground motion suites developed and response analyses performed
- Redoing the ground motion selection and analyses is certainly not desired and may not be feasible
- Existing analyses can be backfitted to follow a risk-consistent vertical DRS by applying a calibrated adjustment factor to the original vertical spectrum
  - This corresponds to multiplying the vertical input motions by a constant
  - For equivalent linear analyses → multiply vertical response results by constant

# Risk-Consistent Adjustment Factor

Hazard-consistent vertical DRS × Adjustment Factor = Risk-consistent vertical DRS

## Criterion

- Find a scalar adjustment factor such that:

$P_F$  calculated using *risk-adjusted vertical DRS convolved w. horizontal hazard curve*  
=  
 $P_F$  calculated using *hazard-consistent vertical DRS convolved w. vertical hazard curve*

Multiple the adjustment factor by the hazard-consistent vertical DRS to **approximate** a risk-consistent vertical DRS

**Approximate** only since an average frequency-independent value is used

# Risk-Consistent Adjustment Factor (cont.)

The risk-consistent adjustment factor is given by  $\lambda$ :

$$\lambda = \{ (F_{Sm})^{K_{Hv}-K_{Hh}} \exp[0.5 \beta^2 (K_{Hh} - K_{Hv})] \}^{1/K_{Hh}}$$

- Scale factor:  $F_{Sm}$
- Variability / dispersion in the fragility function :  $\beta$
- Slopes of the horiz. and vert. hazard curves betw.  $H_p$  and  $10H_p$ :  $K_{Hh}$  &  $K_{Hv}$

$F_{Sm}$ , is expected margin to the **median** seismic capacity of the facility

$\lambda$  is not strongly sensitive to  $\beta$  (next slide)

$K_{Hh}$  &  $K_{Hv}$  are the primary factors in determining the adjustment factor

$\lambda$  is generally frequency-dependent as  $K_{Hh}$  &  $K_{Hv}$  are (similar to V/H ratio)

# Example Risk Estimate of Bias

## Potential out of plane or punching shear failure of slab

- The performance ( $P_F$  /yr) is quantified by convolving the seismic fragility and hazard in terms of the vertical ground motion directly,  $\beta = 0.40 \rightarrow 8.6E-6$ /yr
- Exact solution uses vertical hazard curve in the risk convolution integral
- Hazard-consistent DRS case convolves horizontal hazard curve and fragility from hazard-consistent slab vertical demand  $\rightarrow$  results in bias in  $P_F$
- Risk-consistent DRS case convolves horizontal hazard curve and fragility from risk-adjusted slab vertical demand ( $\lambda = 0.93$ )  $\rightarrow P_F$  matches the exact solution

$$(A_{R,v} / A_{R,h}) = 0.6$$

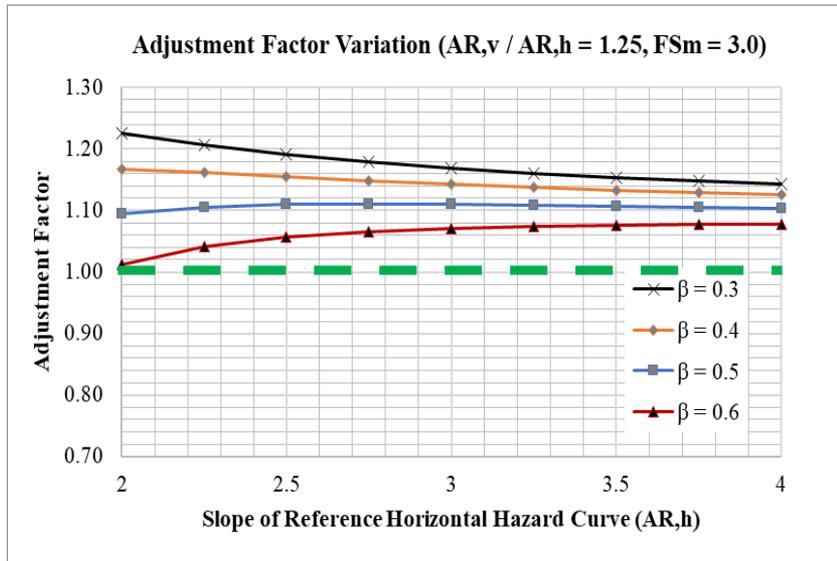
Vertical DRS	Calculated Risk (/yr)	Ratio to Exact Risk Estimate
Exact Solution	8.60E-06	1
Hazard-consistent DRS	1.10E-05	1.27
Risk-consistent DRS	8.60E-06	1

~30% bias

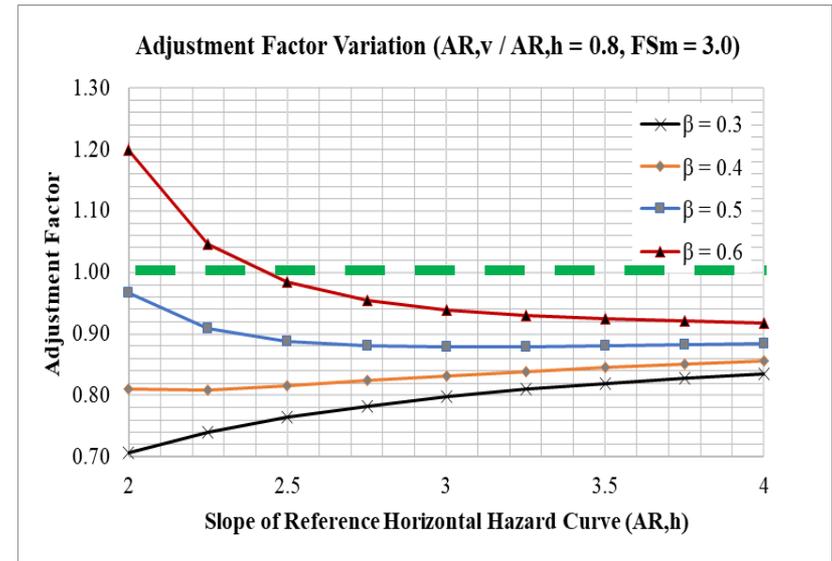
# Properties of $\lambda$

Plots show influence of horiz. hazard curve slope  $A_{R,h}$ , for a typical  $F_{Sm} = 3.0$

- Typical hazard curve slopes are between 2 and 4
- When the vertical hazard curve slope is 25% flatter than the horizontal (left),  $\lambda$  is practically between 1.0 and 1.2
- When the vertical hazard curve slope is 25% steeper than the horizontal (right),  $\lambda$  is largely between 0.75 and 1.0
- For practical cases, sensitivity to  $\beta$  is less than  $\pm 0.1$



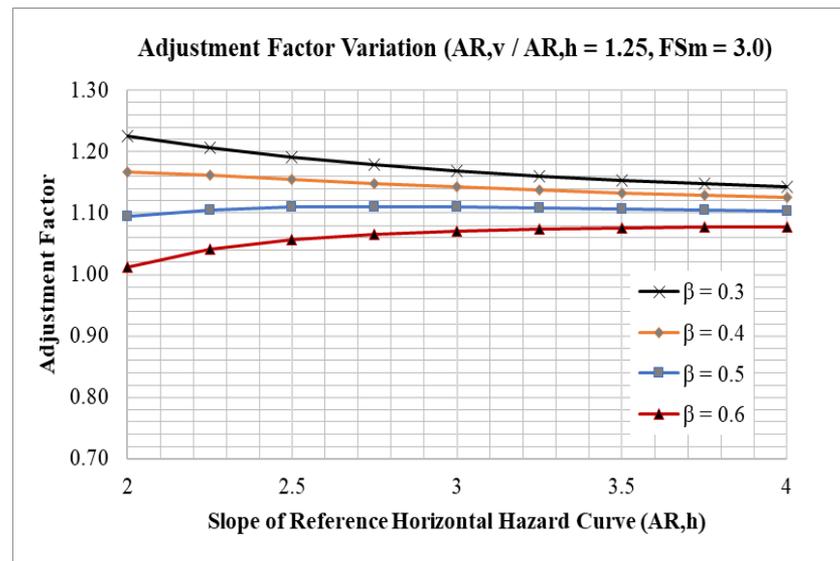
*Ignoring  $\lambda$  is unconservative*



*Ignoring  $\lambda$  is conservative*

# Sensitivity of $\lambda$ to $\beta$

- Dependence on  $\beta$  is typically mild with other parameters constant,  $< \pm 0.1$ 
  - A  $\lambda$  can be selected representative of values within the range of  $\beta$
  - Additional variability can be included in vertical ground motion to capture uncertainty in  $\lambda$  values within range of  $\beta$
  - Sensitivity study indicated that this variability is minor vs. variability in V/H ratios, and can be ignored



# Conclusions

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- Common approaches for developing vertical DRS may result in deviation from intended performance goal for some components
- The reason is the difference in slope of the horizontal and vert hazard curves, a key parameter in the risk convolution integral
- Risk-consistent vertical DRS can be developed by
  - Developing scale factors for the vertical DRS independent of the horizontal DRS
  - Adjusting hazard-consistent vertical DRS
- Adjustment has practical advantages for existing analyses
- Adjustment factor,  $\lambda$ 
  - Strongly influenced by slopes of hazard curves
  - Only mildly influenced by  $\beta$  for typical range of  $\beta$
  - Generally frequency-dependent but can be approximated as constant
- The developed adjustment factor method is practical and applicable to both performance-based design and SPRA applications

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**Thank you**