A. N. SHINPOCH
Secretary

STATE OF WASHINGTON
DEPARTMENT OF SOCIAL AND HEALTH SERVICES
Olympia, Washington 98504-0095

October 29, 1985

Mr. C.E. MacDonald, Chief
Transportation Certification Branch
Division of Fuel Cycle and Material Safety
U.S. Nuclear Regulatory Commission
Washington, D.C. 20555

Dear Mr. MacDonald:

Exxon Nuclear Company, Inc. requested that I, as State Health Officer, write to you to voice my objection to or acceptance of their proposal to transport certain transuranic materials not in strict conformance with 10 CFR 71. My staff has reviewed the exemption request made by Exxon on September 6, 1985. Since these boxes will be transported by knowledgeable staff from the Exxon facility adjacent to the Hanford Reservation to the US Ecology disposal site, on the reservation, with very limited use of public access roads, I see no problem with the use of these packages. Any other alternative would involve considerable occupational exposure.

Since most of the transportation would occur on U.S. Department of Energy roads, we contacted Mr. Gerald Yesberger at the Richland Operations office. He was also in agreement with the proposal.

If we can provide any additional information regarding this requst, please contact Nancy Kirner of this office.

Sincerely,

John Beare

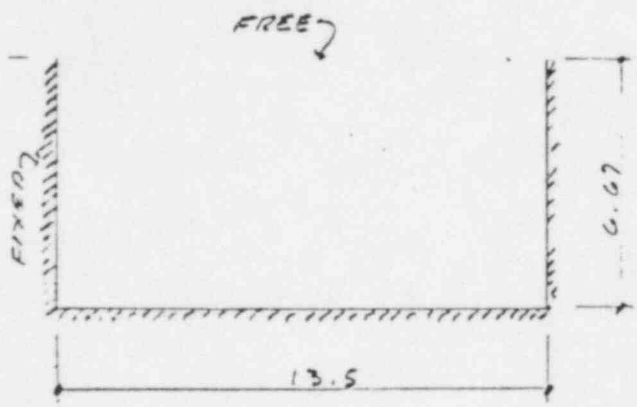
John A. Beare, M.D., M.P.H.
Director
Division of Health - ET-21

cc: C.W. Malody

PROJECT: EXXON NUCLEAR JOB NO. RC - 0015
CLIENT: G.L. GRANT
BY: T.E. MEIER
DATE: 6-25-85
J.B No.: 139

MEIER ASSOCIATES
1332 Grandridge Blvd., Suite 102
Kennewick, Washington 99336

GLOVE BOX 2A DESIGN



FLUID PRESSURE
= 153 × 6.7'
= 1025 PSF

$a/b = \frac{6.75}{6.67} \simeq 1$ $\quad a = \frac{13.5}{2} = 6.75$

$MOM_x = +.0644(1025)(6.67)^2$
$= 2937$ 1-lbs/FT.

$MOM_y = +.0865(1025)(6.67)^2$
$= 3853$ 1-lbs/FT

$R_y = .4584(1025)\,6.67$
$= 3134$

THE FLUID LOAD IS TEMPORARY
∴ TAKE STRESSES TO YIELD

$f'_c = 3,000$ PSI $\quad f_y = 60,000$ PSI

TRY $t = 6"$ ∴ $d = 3"$

$F = \frac{bd^2}{12000} = \frac{12 \times 3^2}{12000} = .009$

$k_u = \frac{M}{F} = \frac{3.9}{.009} = 433$

$a_u = 4.02$

$A_s = \frac{M_u}{a_u d} = \frac{3.9}{4.02 \times 3} = .32$ in²/FT

USE #5 @ 12
EACH WAY
$A_s = .31$ in²/FT

$f_v = \frac{3134}{.85 \times 12 \times 3} = 102$ PSI
O.K

USE #5 @ 12" E.W. ALL AROUND $t = 6"$

BASE DESIGN

THE PRESSURE IS 1025 PSF
$a/b = 6.75/6 \cong 1$

USE $T = 6"$ & #5 @ 12"

PROJECT: EXXON NUCLEAR JOB
No. ZC-0015

MEIER ASSOCIATES
1332 Grandridge Blvd., Suite 102
Kennewick, Washington 99336

CLIENT: G. A. GRANT
BY: T. E. MEIER
DATE: 6-25-85
JOB No.: 139

THE GLOVE BOX WILL BE PICKED UP WITH SLINGS WRAPPED UNDER THE BOX SO LIFTING FIXTURES ARE NOT REQUIRED

CHECK THE LOAD TRANSFER @ THE CONSTR. JOINT WHEN THE BOX IS FILLED:

$P = 5 \times 6.17 \times 12.5 \times 150$
$= 57,800$ lbs

PERIMETER = $13.5(2) + 5(2) = 37'$

LOAD/FT = $58,000/37 = 1568$ lbs

$f_s = P/A = \dfrac{1568 \times 1.7}{.31} = 8596$ PSI
O.K

ASSUME 3~ ROLLER SUPPORTS

6.75 6.75

21^k 40^k 21^k

$P = 6 \times 6.7 \times 13.5 \times 150$
$\approx 82^k$
$P/FT = 82/13.5$
$= 6.1^k/FT$

$40^k/6 = 6.7\ ^k/FT$ @ THE CENTER SUPPORT ROLLER

I RECOMMEND A MINIMUM OF 4~ ROLLER SUPPORTS @ 4.5' c/c TO REDUCE THE CONCENTRATED LINE LOAD ON THE FLOOR

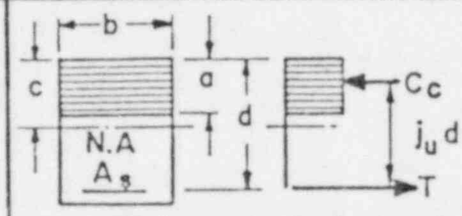
Flexure 1.1—Coefficients for rectangular sections with out compression reinforcement

$$\rho = \omega \frac{f'_c}{f_y} = \frac{A_s}{bd} \qquad K_u = \phi\, f'_c\, \omega(1 - .59\omega)$$

$$\frac{c}{d} = 1.18\, \frac{\omega}{\beta_1} \qquad a_u = \phi\, f_y\,(1 - .59\omega)/12{,}000$$

$$\frac{a}{d} = \beta_1\, \frac{c}{d} \qquad A_s = \frac{M_u}{a_u d} \qquad j_u = 1 - \frac{a}{2d}$$

$$\beta_1 = 0.85 \qquad F = \frac{M_u}{K_u}$$



A_s in square inches and M_u is in ft - kips

ω	K_u	f'_c = 3,000										
		f_y = 40,000		f_y = 50,000		f_y = 60,000		f_y = 80,000		c/d	a/d	j_u
		ρ	a_u	ρ	a_u	ρ	a_u	ρ	a_u			
.020	53	.0015	2.96	.0012	3.71	.0010	4.45	0.0008	5.93	.028	.024	.988
.030	80	.0023	2.95	.0018	3.68	.0015	4.42	0.0011	5.89	.042	.035	.982
.040	105	.0030	2.93	.0024	3.66	.0020	4.39	0.0015	5.86	.056	.047	.976
.050	131	.0038	2.91	.0030	3.64	.0025	4.37	0.0019	5.82	.069	.059	.971
.060	156	.0045	2.89	.0036	3.62	.0030	4.34	0.0022	5.79	.083	.071	.965
.070	181	.0053	2.88	.0042	3.60	.0035	4.31	0.0026	5.75	.097	.083	.959
.080	206	.0060	2.86	.0048	3.57	.0040	4.29	0.0030	5.72	.111	.094	.953
.090	230	.0068	2.84	.0054	3.55	.0045	4.26	0.0034	5.68	.125	.106	.947
.100	254	.0075	2.82	.0060	3.53	.0050	4.23	0.0038	5.65	.139	.118	.941
.110	278	.0083	2.81	.0066	3.51	.0055	4.21	0.0041	5.61	.153	.130	.935
.120	301	.0090	2.79	.0072	3.48	.0060	4.18	0.0045	5.58	.167	.142	.929
.130	324	.0098	2.77	.0078	3.46	.0065	4.15	0.0049	5.54	.180	.153	.923
.140	347	.0105	2.75	.0084	3.44	.0070	4.13	0.0052	5.50	.194	.165	.917
.150	369	.0113	2.73	.0090	3.42	.0075	4.10	0.0056	5.47	.208	.177	.912
.160	391	.0120	2.72	.0096	3.40	.0080	4.08	0.0060	5.43	.222	.189	.906
.170	413	.0128	2.70	.0102	3.37	.0085	4.05	0.0064	5.40	.236	.201	.900
.180	434	.0135	2.68	.0108	3.35	.0090	4.02	0.0068	5.36	.250	.212	.894
.190	455	.0143	2.66	.0114	3.33	.0095	4.00	0.0071	5.33	.264	.224	.888
.200	476	.0150	2.65	.0120	3.31	.0100	3.97	0.0075	5.29	.278	.236	.882
.210	497	.0158	2.63	.0126	3.29	.0105	3.94	0.0079	5.26	.292	.248	.876
.220	517	.0165	2.61	.0132	3.26	.0110	3.92	0.0082	5.22	.305	.260	.870
.230	537	.0173	2.59	.0138	3.24	.0115	3.89	0.0086	5.19	.319	.271	.864
.240	556	.0180	2.58	.0144	3.22	.0120	3.86	0.0090	5.15	.333	.283	.858
.250	575	.0188	2.56	.0150	3.20	.0125	3.84	0.0094	5.12	.347	.295	.853
.260	594	.0195	2.54	.0156	3.17	.0130	3.81	0.0098	5.08	.361	.307	.847
.270	613	.0203	2.52	.0162	3.15	.0135	3.78	0.0101	5.04	.375	.319	.841
.280	631	.0210	2.50	.0168	3.13	.0140	3.76	0.0105	5.01	.389	.330	.835
.290	649	.0218	2.49	.0174	3.11	.0145	3.73			.403	.342	.829
.300	667	.0225	2.47	.0180	3.09	.0150	3.70			.416	.354	.823
.310	684	.0233	2.45	.0186	3.06	.0155	3.68			.430	.366	.817
.320	701	.0240	2.43	.0192	3.04	.0160	3.65			.444	.378	.811
.330	718	.0248	2.42	.0198	3.02					.458	.389	.805
.340	734	.0255	2.40	.0204	3.00					.472	.401	.799
.350	750	.0263	2.38							.486	.413	.794
.360	766	.0270	2.36							.500	.425	.788
.370	781	.0278	2.35							.514	.437	.782

1. When $h_f / d \geq a/d$, beam is designed as rectangular section; when $h_f / d < a/d$, beam is designed as a T section (See Flexure 2).
2. Values of ρ above upper solid line are for percentages smaller than ρ min., ρ min. = $200/f_y$. See section 10.5, ACI 318-71.
3. Values of ρ below lower solid line would exceed ρ max. = 0.75 ρ_b. For values of ρ_{max} see values of $(\rho_w - 0.75\,\rho_f)$ max. in Flexure 2. See Section 10.3.2. ACI 318-71.
4. For deflection see Section 9.5 ACI 318-71 and "Deflection" chapter in this handbook.
5. Check for Crack Control where $f_y > 40{,}000$ See Section 10.6. ACI 318-71 and under chapter "Reinforcement" in this handbook.
6. The capacity reduction factor ϕ = 0.9 has been included in the table values.
7. For use of this Table, see Flexure Examples 1 to 10, 15, 16

a/b	y/b	R_x \ x/a	M_x 0	M_x 0.2	M_x 0.4	M_x 0.6	M_x 0.8	M_x 1.0	M_y 0	M_y 0.2	M_y 0.4	M_y 0.6	M_y 0.8	M_y 1.0
a/b = 1/8	1.0	+.0082	+.0004	+.0002	+.0000	-.0001	-.0002	-.0002	0	0	0	0	0	0
	0.8	+.0251	+.0011	+.0005	+.0000	-.0003	-.0005	-.0005	+.0002	+.0001	+.0000	-.0000	-.0001	-.0001
	0.6	+.0496	+.0021	+.0009	+.0001	-.0006	-.0009	-.0011	+.0004	+.0002	+.0000	-.0001	-.0002	-.0002
	0.4	+.0751	+.0031	+.0014	+.0001	-.0008	-.0014	-.0016	+.0006	+.0003	+.0000	-.0002	-.0003	-.0003
	0.2	+.0942	+.0038	+.0016	+.0000	-.0010	-.0017	-.0019	+.0008	+.0003	-.0000	-.0003	-.0005	-.0005
	0	+.0460	0	+.0001	+.0003	+.0005	+.0006	+.0006	0	+.0005	+.0014	+.0023	+.0028	+.0030
		R_x \ R_y	+.0460	+.0136	+.0543	+.0839	+.1004	+.1056						
a/b = 1/4	1.0	+.0147	+.0022	+.0012	+.0002	-.0006	-.0012	-.0014	0	0	0	0	0	0
	0.8	+.0523	+.0046	+.0022	+.0002	-.0012	-.0021	-.0024	+.0009	+.0005	+.0002	-.0000	-.0002	-.0002
	0.6	+.1015	+.0083	+.0037	+.0002	-.0023	-.0038	-.0042	+.0017	+.0007	-.0000	-.0005	-.0009	-.0010
	0.4	+.1514	+.0114	+.0049	+.0001	-.0032	-.0051	-.0057	+.0023	+.0008	-.0004	-.0013	-.0019	-.0021
	0.2	+.1494	+.0102	+.0037	-.0004	-.0030	-.0043	-.0047	+.0020	+.0004	-.0011	-.0022	-.0029	-.0031
	0	+.0304	0	+.0004	+.0010	+.0016	+.0020	+.0021	0	+.0020	+.0052	+.0081	+.0100	+.0107
		R_x \ R_y	+.0304	+.0309	+.1052	+.1563	+.1856	+.1950						
a/b = 3/8	1.0	+.0189	+.0066	+.0040	+.0008	-.0020	-.0039	-.0045	0	0	0	0	0	0
	0.8	+.0885	+.0117	+.0056	+.0006	-.0031	-.0054	-.0062	+.0023	+.0012	+.0004	-.0002	-.0005	-.0007
	0.6	+.1541	+.0176	+.0075	+.0001	-.0049	-.0079	-.0088	+.0035	+.0013	-.0006	-.0020	-.0029	-.0032
	0.4	+.2107	+.0208	+.0079	-.0007	-.0061	-.0090	-.0099	+.0042	+.0009	-.0019	-.0042	-.0056	-.0061
	0.2	+.1691	+.0145	+.0045	-.0012	-.0042	-.0057	-.0061	+.0029	+.0001	-.0022	-.0039	-.0048	-.0051
	0	+.0102	0	+.0008	+.0020	+.0030	+.0038	+.0040	0	+.0039	+.0099	+.0152	+.0188	+.0200
		R_x \ R_y	+.0102	+.0474	+.1488	+.2154	+.2526	+.2645						
a/b = 1/2	1.0	+.0326	+.0151	+.0088	+.0015	-.0046	-.0084	-.0097	0	0	0	0	0	0
	0.8	+.1315	+.0216	+.0099	+.0007	-.0059	-.0099	-.0112	+.0043	+.0020	+.0002	-.0011	-.0019	-.0022
	0.6	+.1972	+.0273	+.0108	-.0005	-.0079	-.0119	-.0132	+.0055	+.0015	-.0020	-.0047	-.0064	-.0070
	0.4	+.2421	+.0277	+.0092	-.0019	-.0082	-.0115	-.0125	+.0055	+.0004	-.0042	-.0076	-.0097	-.0104
	0.2	+.1607	+.0160	+.0041	-.0017	-.0044	-.0055	-.0058	+.0032	-.0002	-.0026	-.0039	-.0044	-.0046
	0	-.0045	0	+.0014	+.0033	+.0050	+.0061	+.0065	0	+.0068	+.0167	+.0252	+.0307	+.0325
		R_x \ R_y	-.0045	+.0744	+.1942	+.2699	+.3108	+.3236						
a/b = 3/4	1.0	+.1061	+.0406	+.0196	+.0013	-.0115	-.0190	-.0214	0	0	0	0	0	0
	0.8	+.2077	+.0433	+.0177	-.0003	-.0119	-.0184	-.0205	+.0087	+.0031	-.0012	-.0042	-.0061	-.0067
	0.6	+.2408	+.0426	+.0145	-.0026	-.0124	-.0174	-.0189	+.0085	+.0010	-.0055	-.0102	-.0130	-.0139
	0.4	+.2542	+.0349	+.0091	-.0039	-.0102	-.0130	-.0138	+.0070	-.0011	-.0075	-.0115	-.0137	-.0143
	0.2	+.1337	+.0163	+.0031	-.0017	-.0031	-.0033	-.0033	+.0033	+.0001	-.0000	+.0014	+.0029	+.0035
	0	-.0196	0	+.0028	+.0064	+.0093	+.0111	+.0117	0	+.0139	+.0320	+.0465	+.0554	+.0584
		R_x \ R_y	-.0196	+.1256	+.2666	+.3496	+.3923	+.4055						
a/b = 1	1.0	+.1985	+.0644	+.0253	-.0013	-.0172	-.0252	-.0276	0	0	0	0	0	0
	0.8	+.2564	+.0601	+.0210	-.0028	-.0161	-.0226	-.0245	+.0120	+.0034	-.0026	-.0065	-.0088	-.0095
	0.6	+.2485	+.0515	+.0149	-.0047	-.0145	-.0189	-.0201	+.0103	+.0003	-.0075	-.0125	-.0151	-.0159
	0.4	+.2411	+.0372	+.0078	-.0049	-.0100	-.0118	-.0122	+.0074	-.0021	-.0076	-.0099	-.0106	-.0107
	0.2	+.1108	+.0154	+.0025	-.0006	-.0006	-.0000	+.0003	+.0031	+.0018	+.0060	+.0116	+.0160	+.0175
	0	-.0241	0	+.0044	+.0096	+.0137	+.0161	+.0169	0	+.0220	+.0482	+.0683	+.0804	+.0845
		R_x \ R_y	-.0241	+.1691	+.3199	+.4038	+.4457	+.4584						
a/b = 3/2	1.0	+.3127	+.0857	+.0207	-.0087	-.0199	-.0232	-.0238	0	0	0	0	0	0
	0.8	+.2929	+.0730	+.0158	-.0086	-.0172	-.0194	-.0198	+.0146	+.0023	-.0042	-.0072	-.0082	-.0085
	0.6	+.2352	+.0560	+.0094	-.0083	-.0134	-.0142	-.0141	+.0112	-.0013	-.0077	-.0096	-.0096	-.0094
	0.4	+.2148	+.0359	+.0038	-.0053	-.0065	-.0057	-.0053	+.0072	-.0021	-.0023	+.0012	+.0046	+.0059
	0.2	+.0897	+.0132	+.0021	+.0025	+.0050	+.0069	+.0076	+.0026	+.0077	+.0220	+.0356	+.0444	+.0474
	0	-.0204	0	+.0079	+.0158	+.0212	+.0243	+.0252	0	+.0396	+.0791	+.1062	+.1214	+.1262
		R_y	-.0204	+.2452	+.3964	+.4668	+.4966	+.5047						

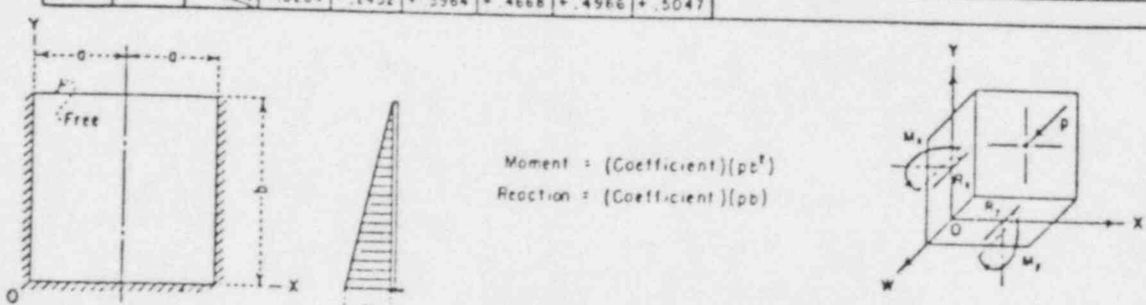


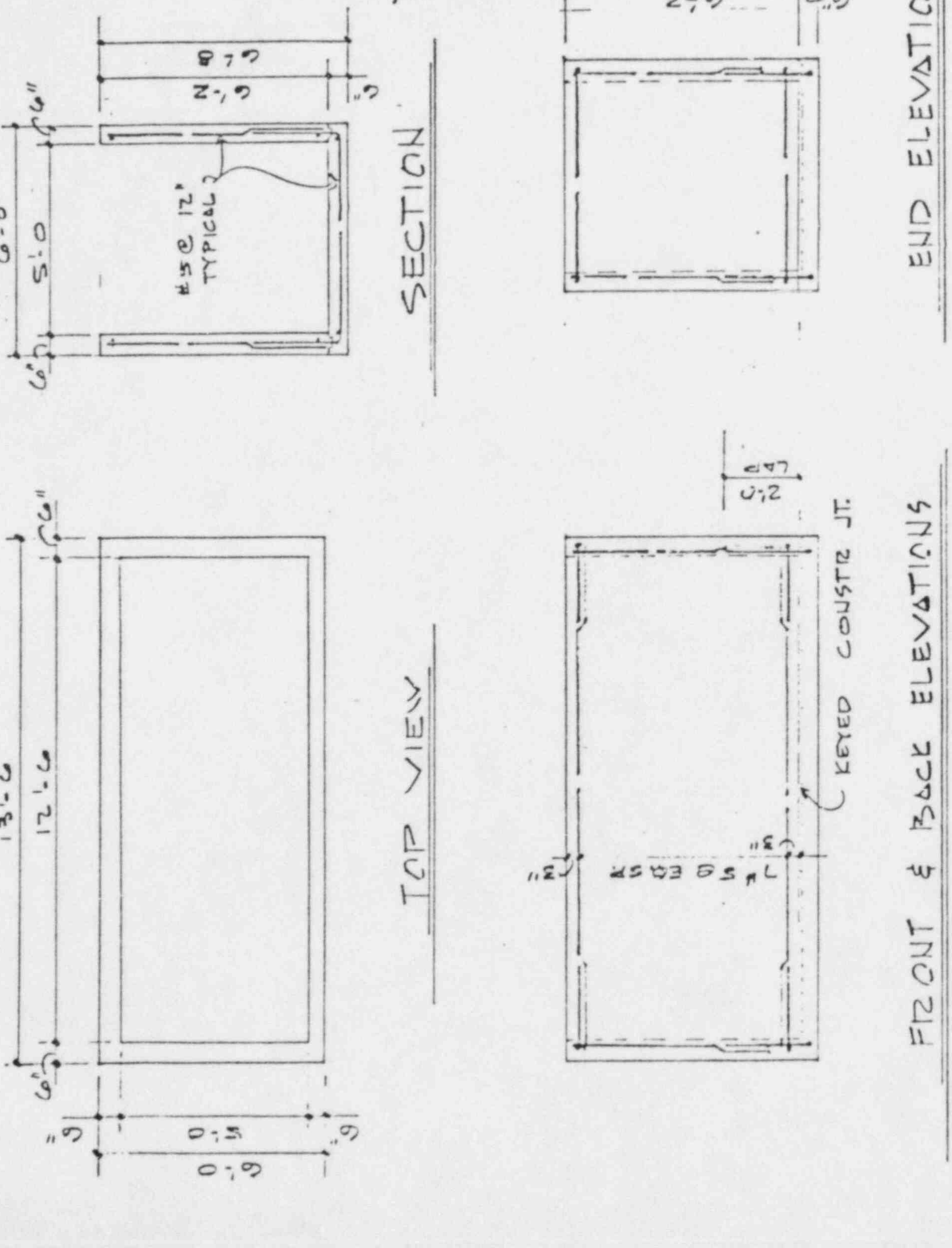
FIGURE 4.—*Plate fixed along three edges, moment and reaction coefficients, Load IV, uniformly varying load.*

MEIER ASSOCIATES
1332 Grandridge Blvd., Suite 102
Kennewick, Washington 99336

TERRANCE E. MEIER
STATE OF WASHINGTON
16492
REGISTERED
PROFESSIONAL ENGINEER

Terrance E. Meier
6/25/85

CONCRETE = f'c = 3,000 PSI
STEEL fy = 60,000 PSI



ACME CONCRETE COMPANY

June 24, 1985

George Grant, Inc.
P.O. Box 786
Richland, Wa. 99352

Attn: Dennis Carl

Re: Exxon Nuclear, Job #RC-0015, Line Item 24

Dear Mr. Carl:

The theoretical cubic foot weight of our 8 sk grout mix, Mix No. 7681 is 147.00 lbs. per cubic foot, with the following batch weights.

2750	lbs. of concrete sand
752	lbs. of cement
56	gal. of water
3969	

Assuming that the cement will use 3 gal of water per sack for hydration and the sand has an absorption of 1.5%, the approximate dried cubic foot weight would be 135.59 lbs. per cubic foot.

If I can be of further help, please feel free to call me.

Sincerely,

ACME CONCRETE CO.

CAVITY CONCRETE

Steve Wittstock
Quality Control

SJW:cd

JUN 25 1985

Dennis:

ACME CONCRETE COMPANY

June 20, 1985

George Grant, Inc.
P.O. Box 786
Richland, Wa. 99352

Attn: Dennis Carl

Re: Exxon Nuclear Job #RC-0015, Line Item 24

Dear Mr. Carl:

The theoretical unit weight of our P.C. concrete, Mix No. 6511, is 152.57 lbs. per cubic foot, with the following batch weights.

1452 lbs. of concrete sand
1820 lbs. of 3/4 aggregate
610 lbs. of cement
28.5 gal. of water
4119.4

Assuming that the cement will use 3 gal of water per sack for hydration and the aggregate has an absorption of ½% and the sand has an absorption of 1.5%, the approximate unit of a dried cubic foot would be 148.6 lbs. per cubic foot.

If I can be of further help, please feel free to call me.

Sincerely,

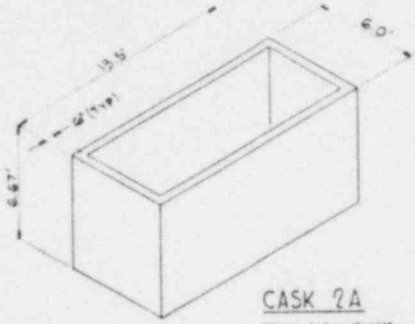
ACME CONCRETE CO.

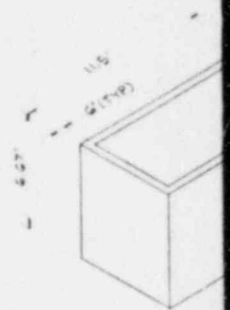
Steve Wittstock
Quality Control

SJW:cd

SHELL CONCRETE

RECEIVED
JUN 21 1985
George A. Grant, Inc.
Richland, Wash.

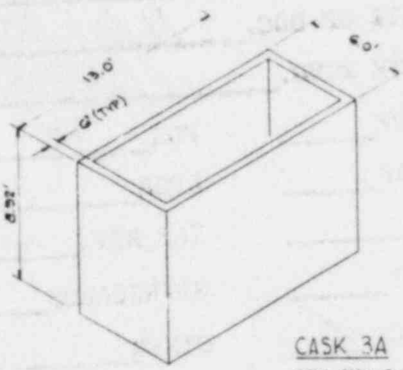


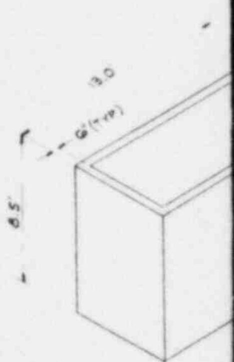


CASK 2A

TOTAL CASK VOLUME – 540.27 CU.FT.
CASK WEIGHT – 57.63 TONS
CASK CONTENTS:

RADIONUCLIDE	nCi
Pu – 238	5.40×10^8
Pu – 239	2.60×10^8
Pu – 240	2.26×10^8
Pu – 242	0.003×10^8
Am – 241	8.84×10^8
Pu – 241	2.21×10^{10}

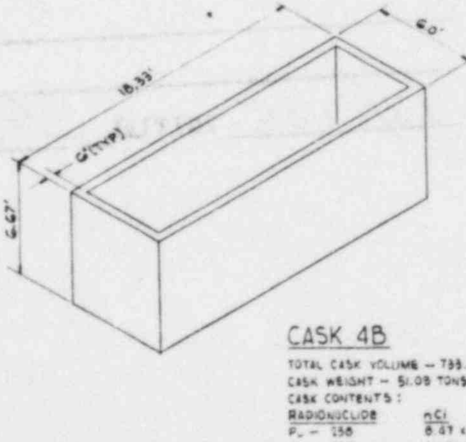


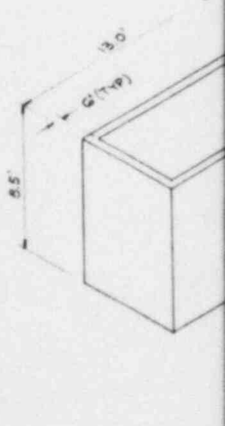


CASK 3A

TOTAL CASK VOLUME – 695.8 CU.FT.
CASK WEIGHT – 47.7 TONS
CASK CONTENTS:

RADIONUCLIDE	nCi
Pu – 238	5.98×10^8
Pu – 239	2.88×10^8
Pu – 240	2.50×10^8
Pu – 242	0.003×10^8
Am – 241	9.78×10^8
Pu – 241	2.44×10^{10}

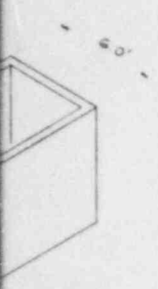




CASK 4B

TOTAL CASK VOLUME – 733.57 CU.FT.
CASK WEIGHT – 51.08 TONS
CASK CONTENTS:

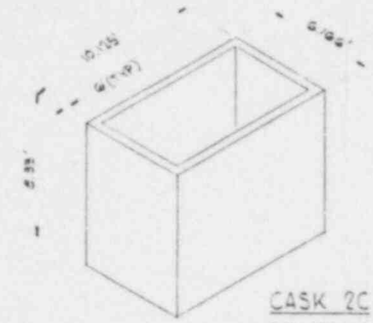
RADIONUCLIDE	nCi
Pu – 238	8.47×10^8
Pu – 239	4.08×10^8
Pu – 240	3.54×10^8
Pu – 242	0.004×10^8
Am – 241	13.85×10^8
Pu – 241	3.46×10^{10}



CASK 2B

TOTAL CASK VOLUME – 460.23 CU.FT.
CASK WEIGHT – 32.09 TONS
CASK CONTENTS:

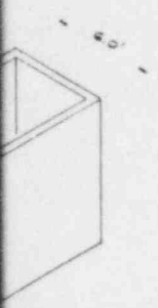
RADIONUCLIDE	nCi
Pu – 238	5.29×10^{8}
Pu – 239	2.57×10^{8}
Pu – 240	2.22×10^{8}
Pu – 242	0.003×10^{8}
Am – 241	8.67×10^{8}
Pu – 241	2.17×10^{10}



CASK 2C

TOTAL CASK VOLUME – 520.05 CU.FT.
CASK WEIGHT – 36.24 TONS
CASK CONTENTS:

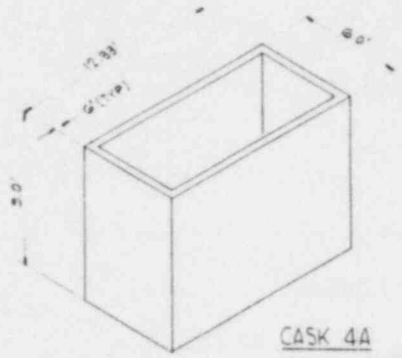
RADIONUCLIDE	nCi
Pu – 238	6.13×10^{8}
Pu – 239	2.95×10^{8}
Pu – 240	2.57×10^{8}
Pu – 242	0.006×10^{8}
Am – 241	10.03×10^{8}
Pu – 241	2.51×10^{10}



CASK 3B

TOTAL CASK VOLUME – 663 CU.FT.
CASK WEIGHT – 44.84 TONS
CASK CONTENTS:

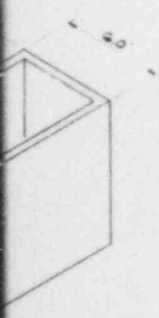
RADIONUCLIDE	nCi
Pu – 238	4.82×10^{8}
Pu – 239	2.32×10^{8}
Pu – 240	2.02×10^{8}
Pu – 242	0.002×10^{8}
Am – 241	7.89×10^{8}
Pu – 241	1.97×10^{10}



CASK 4A

TOTAL CASK VOLUME – 665.82 CU.FT.
CASK WEIGHT – 46.34 TONS
CASK CONTENTS:

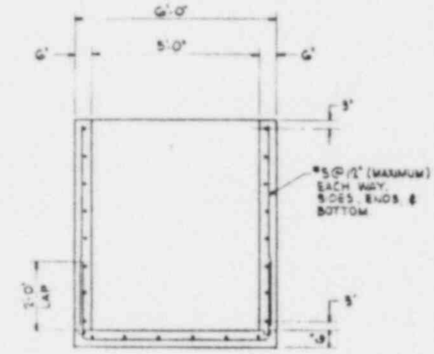
RADIONUCLIDE	nCi
Pu – 238	7.59×10^{8}
Pu – 239	3.66×10^{8}
Pu – 240	3.18×10^{8}
Pu – 242	0.004×10^{8}
Am – 241	12.42×10^{8}
Pu – 241	3.10×10^{10}



CASK 5

TOTAL CASK VOLUME – 663 CU.FT.
CASK WEIGHT – 45.72 TONS
CASK CONTENTS:

RADIONUCLIDE	nCi
Pu – 238	0.87×10^{8}
Pu – 239	0.42×10^{8}
Pu – 240	0.36×10^{8}
Pu – 242	0.0004×10^{8}
Am – 241	1.42×10^{8}
Pu – 241	3.54×10^{9}



CROSS SECTION
TYPICAL ALL CASKS

CONCRETE DENSITIES

CASK SIDES & BOTTOM – 148.6 #/FT³
ENTOMBMENT – 135.55 #/FT³

TI APERTURE CARD

Also Available On Aperture Card

QTY	ITEM	DESCRIPTION	REMARKS

EXXON NUCLEAR COMPANY, INC.

MO EQUIPMENT BURIAL CASKS

PROJECT ENG.: J. HELTON
DRAWN BY: COOK 8-29-85

ATTACHMENT No. 1

8512170386-01

DOCKET NO. 71-9207

CONTROL NO. 26157

DATE OF DOC. 11/04/85

DATE RCVD. 11/12/85

FCUF ______ PDR ✓

FCAF ______ LPDR ______

WM ______ I&E REF. ✓

WMUR ______ SAFEGUARDS ______

FCTC ✓ OTHER ______

DESCRIPTION:

Certificate of Compliance amendment request

12/12/85 INITIAL CEC