

#### UNITED STATES NUCLEAR REGULATORY COMMISSION WASHINGTON, D. C. 20555

#### OCT 21 1983

MEMORANDUM FOR: George Knighton, Chief

Licensing Branch No. 3 Division of Licensing

FROM:

George Lear, Chief

Structural and Geotechnical Engineering Branch

Division of Engineering

SUBJECT:

EVALUATION OF PG&E'S RESPONSE TO THE ANONYMOUS

ALLEGATIONS AT DIABLO CANYON, UNIT 1

Reference:

Letter from J. O. Schuyler of PG&E to George Knighton

dated July 1, 1983

As requested, R. Lipinski of Structural Engineering Section A, Structural and Geotechnical Engineering Branch evaluated responses of the Pacific Gas and Electric Company, (PG&E) to the allegation no. 6, contained in the enclosure to the reference.

We find that the PG&E's response is acceptable pending review of the two items, confirmatory in nature, namely:

- a) Test programs of the material (masonry block, grout and reinforcing bars) to demonstrate validity of the actual stresses use in design.
- b) Demonstration by the PG&E that the tornado parameters (wind and the associated atmospheric pressure drop) represents the upper bound of the loads which can be expected to act on the masonry walls located in the turbine building.

During the telephone conference involving the staff and the PG&E representatives on October 18, 1983, both of these items were discussed and it is expected that the licensee will provide the pertinent information by October 25, 1983.

8310310503XA Structural and Geotechnical

Engineering Branch Division of Engineering

Enclosure: As stated

cc: See page 2

Contact: R. Lipinski, x28428

cc: J. Knight
D. Jeng
B. Buckley
A. Vietti
N. Chokshi
R. Lipinski

# Evaluation of PG&E Response to Anonymous Allegation No. 6 Diablo Canyon Nuclear Plant, Unit 1

#### 1. Allegation

"Tornado Design Criteria: Masonry walls in the turbine building are not analyzed for the suction loads which might result from a tornado. Thus the tornado design criteria is (sic) incomplete."

#### 2. Project Position

In his response, the licensee stated that the switchgear and cable spreading rooms in the turbine building are capable of withstanding a 200 mph tornado wind and the associated differential pressure of 0.86 psi.

Furthermore, all masonry walls in proximity to safety related equipment are being re-evaluated for the loads resulting from a Hosgri earthquake using the appropriate response spectra. The design suction pressure of 0.86 psi is equivalent to 1.5g seismic load for an 8-in thick masonry wall. The walls in question are being reviewed for a seismic acceleration of no less than 1.5g. In view of the above, the licensee concluded, since the seismic loads are equal or greater than the postulated tornado loads the masonry walls located in the turbine building are adequately protected.

#### Extent of Review by the Staff

During OL review of Diablo Canyon plant the applicant evaluated the tornado resisting capabilities, in terms of safewind velocities, of the Category I

structures. The safewind velocity included wind pressure and the associated atmospheric pressure drop effects and was based on the element of the structure with the minimum capability.

The switchgear and cable spreading rooms are located in the turbine buildings and the separation between the individual rooms consists of eight-inch concrete block walls, with all cells full of grout, number four reinforcing bars vertically, on 16-inch centers and two number four reinforcing bars horizontally on 32-inch centers.

At the request of the staff the licensee performed additional analysis to determine capability of the walls to resist tornado loads. This analysis consisted of evaluation of the walls for a postulated 200 mph wind pressure plus one-half of the associated atmospheric pressure drop. Because of the location of the equipment within the turbine building it has been concluded that the probability of a tornado generated missiles striking the vulnerable areas is small enough to be negligible and missiles were not included in the analysis. In the analysis the licensee compared the required capacities of the walls with those which are available, using both criteria: those of IF Bulletin 80-11 and those of the SRP Section 3.8.4, Appendix A. The analysis included comparison of shear stresses as well as those produced by the bending moment, which is governed by rebar tension stress. The allowable stresses of the material

used in the evaluation were those obtained by the tests of the actual material installed instead of using code specified minimum material properties.

In all cases examined the available capacity of the walls exceed the required capacity by a safe margin.

#### 4. Remaining Open Items

The staff requested that the licensee provides additional information with regard to two items:

- a) Demonstrate, by means of test records, or otherwise, that the material properties used in the analysis are appropriate, and
- b) Demonstrate that the loads resulting from the tornado, which have been approved by the staff as appropriate for the site represent a upper boundary of the loads which would be experienced by the subject walls.

During a telephone conference, on October 18, 1983, the staff discussed these items with the licensee and the additional information is expected to be submitted for review by the staff by October 25, 1983. We regard these items as confirmatory information.

#### 5. Conclusion

The licensee demonstrated, using methods acceptable to the staff, that the walls are capable to resist the loads expected to be experienced by the subject walls. The analysis performed by the applicant contains one relaxation relative to the normal, or currently accepted design procedures. This is the use of actual material strengths rather than code specified minimum material strengths. In our review we found the use of actual material strengths acceptable since in spite of this relaxation some margin remains. In Ref. 1 for example, the licensee stated that the average compressive strength used for the grout filling the cells of the block walls is the tested strength of 3285 psi rather than minimum specified compressive strength of 2000 psi. The steel used in the reinforcing is Grade 40. The yield and the ultimate strength of the rebar steel was determined by tests at the Pittsburgh Testing Laboratory. The test reports compiled by J. A. Blume, for 80 percent of the turbine building rebars indicate that the average tested yield strength is 51,390 psi, which is about what has been used in the analysis (51,400 psi).

The method of the analysis has been found to be acceptable to the staff. In summary, the staff finds that pending satisfactory review of the two items listed in (4) above, the walls provide reasonable assurance that they will withstand the expected loads, and therefore we find that the allegation is unfounded.

#### References

- Report on Safety-Related Masonry Walls Diablo Canyon Power Plant, Unit 1, August 17, 1983, Pacific Gas and Electric Company.
- 2. Letter from J. O. Schuyler of PG&E to G. Knighton, NRC, dated July 1, 1983.
- Letter from J. O. Schuyler of PG&E to D. Eisenhut, NRC dated
   October 11, 1983.
- Letter from J. O. Schuyler of PG&E to D. Eisenhut, NRC, dated
   October 14, 1983.

SECTION IV

GUY F. ATKINSON COI AND Diablo Canyon Project Specification 5422

#### MATERIAL RECEIVING AND INSPECTION REPORT

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4.		e material on site						
5.		erial description a						
	_		A	ccept	Reject	N/A	Date	Signed
6.	Doc	umentation						
	a.	Shipping list			. 🗆	OCT	2 1978	A
	b.	Purchase order				MOCT	2 1978	1
	c.	Material certifica	tion			DCT	2 1978	2
	d.	OS & D Report				Voct	2 1978	42
7.	Ide	ntification .						
	a.	Heat/lot number				PET	2 1978	42
	b.	Stamping				POCT	2 1978	#
	c.	Other				₩8CT	2 1978	12
8.	Vis	ual Inspection						
	a.	Shipping damage		0		OCT	2 1978	1
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	c.	Configuration				OCT	2 1978	1
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	Eg	puipment PER 3	ub Con	TRACT.		GFACO =	QUALITY ASS	URANCE





H. T. WEDDLE, PRESIDENT

(a/c 805) 543-9290

2900 South Broad Street

San Luis Obispo

California 93401

June 29, 1978

Quy F. Atkinson Co. P. O. Box 99 Avila Beach, Calif. 93424

Gentlemen,

Please first enclosed letter of certification on Air Vol Block, Inc., with current laboratory tests on the blocks attached.

Sincerely yours,

H. T. WEDDLE President

RECEIVED

JUN 3 0 1978

Guy F. Atkinson Company PIN 2277

GFACO. GUALITY ASSURANCE ACCEPTED BY

OCT

2 1978

Q.A. INSPECTOR

DATE

June 26, 1978

## RECEIVED

JUN 3 0 1978

Faulstich Bros. 2900 S. Broad San Luis Obispo, Ca. 93401

Guy F. Atkinson Compar PIN 2277

Ref: Diablo Canyon, Guy F. Atkinson Job

Gentlemen:

This is to certify that all masonry units supplied to you by Air-Vol Block, Inc. for above referenced project shall comply to the requirements of the plans and specifications. This certification includes compliance to A.S.T.M.—90, Grade A.

I am enclosing a copy of recent test results for your

the second secon

Sincerely,

Robert J. Miller

Pres. Air Vol Block, Inc.

RJM; pwb

GFACO. QUALITY ASSURANCE
ACCEPTED BY

OCT 2 1978

Q.A. INSPECTOR DATE

#### Coast Laboratories

BUCKLEY BOAD , 544-3276 SAN LUIS OBISPO, CALIFORNIA 93401

93422

SOIL MECHANICS FOUNDATION ENGINEERING

Young Brothers Construction

7600 El Camino Real

Atascadero, California

DATE: December 19, 1977

LABORATORY #: CC03113

DATE

PROJECT: . Security Pacific Bank - Atascadero Branch

ARCHITECT: Kerr, Hall, Hurley, Deutsch

STRUCTURAL ENGINEER: Fred Schott

CONTRACTOR: Young Brothers Construction

SAMPLE:

Young Bros. Const

13- DHHC

Six 8 x 8 x 16 0E Standard and six 8 x 6 x 16 0.E. Slump concrete masonry blocks were sampled at the jobsite by Patrick Smith, of Central Coast Laboratories, on November 21 & 23, 1977.

SIZE: '			8 x 6 x	16	8 x 8 x 16	
	Length, inches		15.65		15.65	ece mac
	Width, inches	T.0 .	7.63		7.60	MASONAY 5
	Depth, inches	Hour &	x8x16 5.60		7.63	MANUAL
Sample	Compression (gross are Total Load Destruction,	(a) = 4 to	AREA *15.65 = 62 Lbs. per Sq. Inch	Absorption Lbs. per Cu. Foot	Moisture Content, % of Maximum	*Linear Shrinkaça Percent
1	138,500		1160	++16.0		
2	124,500			**16.0	22.1	0.051
3			1040	**17.4	24.4	0.048
	142,000		1200	**18.1	16.2	0.048
Average			1135	17.2	20.9	0.049
8 x 8 x 16						
1	144,500	2308.	1215	10.4	13.2	0.031
2	164,500	2628.	1375	10.4	19.5	0.034
3	- 158,000	2524.	1340	10.6	13.2	0.031
Average		2487.)	(1310)	10.5	15.3	0.032
Specification	on Requirements		1000	15.0 max		
* Saturated	to oven-dry come night prior	ndition.	ing	GFACO. QUALITY ACCEPTED Respect 74119	ASSURANCE BY	0.050

# Central Coust Laborar

376 AUCHITY POAD . 544-3776 SAN LUIS ONISPO, CALIFORNIA 93401



DATE: February 9, 1978

LAD NO: CC03202 .

FOR:

Young Brothers Construction

7600 El Camino Real

Atascadero, CA 93422

PROJECT: Security Pacific National Bank

Paso Robles Branch

ARCHITECT: . Priest, Richword, Wolf & Rossi

STRUCTURAL ENGINEER: Taylor & Gaines

CONTRACTOR: Young Brothers Construction

Six 8x8x16 O.E. concrete masonry blocks were sampled at Air-Vol Block Co. by B. Huggins of Central Coast Laboratories on January 16, 1978.

SIZE: . . . 15.6 Width, Depth,

	Lbs. per <u>Sy. Inch</u> 1,125  1,411  1,537	Absorption Lbs. per Cu. Foot  9.8 10.5	Moisture Content. Percent of Maximum 20.0	*Linear Shrinkage Percent .043
Average of three: 2574	~	12.0	13.0	.039
Specification Requirements *Saturated to oven-dry con	,,,,,	10.7 15.0 max.	16.6 40.0 max.	.038 .060 max.

CENTRAL CUAST LAGURATORIES

Copies to: PRHR

Taylor & Gaines Pac. S. W. Realty

Young Bros.

PAS:1s

GFACO. QUALITY ASSURANCE ACCEPTED BY

Patrick Alan Smith

Quality Control Director

#### PACIFIC GAS AND ELECTRIC COMPANY

7 3 W 3 -

245 MARNE" E"REET + SAN FRANCISCO, CALIFORNIA 44156 + 1415, 781-4211 + "WY BIT ST. 658"

P. O. Box 117 Avila Beach, California 93424

May 22, 1978

Mr. R. W. Wunderlich
Proj at Manager
Guy F. Atkinson Company
P. C. Box 99
Avila Beach, California 93424

Diablo Canyon Project Specification 5422 Concrete Block Walls

Dear Mr. Wunderlich:

In regard to the concrete block to be installed, the work shall be performed in accordance with Specification 8828 and shall meet Q. A. requirements of Specification 8828.

Sincerely,

M. R. TRESLER

Project Superintendent

Mik Lucien / Risk

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Guy F. Atkinson Company

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### PACIFIC GAS AND ELECTRIC COMPANT

PGWE +

245 MARKET STREET . SAN FRANCISCO, CALIFORNIA 94106 . (415) 781-4211 . TWX 910-377---

P. O. Box 117 Avila Beach, California 93424

May 8, 1978

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MAY 9 1978

Mr. R. W. Wunderlich
Project Superintendent
Guy F. Atkinson Company
P. O. Box 99
Avila Beach, California 93424

Guy F. Atkinson Company PIN 2277

Diablo Canyon Project Specification 5422 Your Letter A.4-30 Concrete Mix Design

Dear Mr. Wunderlich:

The 3/8" mix, furnished by San Luis Ready Mix, is acceptable for filler in your concrete block work. The design mix is for 2,000 P.S.I. in 28 days. The design mix for one yard of concrete (saturated and dry surface) is as follows:

	Cement	564 1bs.
4 1	Sand	1,907 lbs.
	3/8" Agg.	827 1bs.
	Water	49 Gal.
	S1 ump	5" + 1" at batch plant
Grout	Aid #2 '	6 1bs.*

\*Grout Aid #2 is to be added on site to the delivered mix - 6 lbs./yd. or per manufacturer's recommendation.

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File 6.4
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Ruunderlich
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Bruguier

Sincerely,

M. R. TRESLER

Project Superintendent

#### PACIFIC GAS AND ELECTRIC COMPANY

P. O. Box 117

Avila Beach, California 93424

93424

May 5, 1978

RECEIVE

MAY 8 1970

Guy F. Atkinson Com PIN 227

..... 6 IS/G

Diablo Canyon Project Specification 5422 Your Letter A.5-6 Masonry-Block Walls

Dear Mr. Wunderlich:

Mr. R. W. Wunderlich

Guy F. Atkinson Company

Avila Beach, California

Project Manager

P. O. Box 99

The letters of certification from your subcontractor Faulstich Bros. on Air Vol. Concrete Block, Kaiser Cement, Coast Rock and the special hydrated lime are acceptable.

A letter of certification (and/or test results) however, must be furnished on the concrete block units provided by Basalt Rock Company.

Sincerely,

M. R. TRESLER

Project Superintendent

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Demattei-SSF
W. Harris
R. Wunderlich
M. Walsh
BREGNIER/G. Menn

5-8-78

FILE: WASCHE!

GUY F. ATKINSON COMPANY

POST OFFICE BOX 99
AVILA BEACH, CALIFORNIA
93424

FILE: MASK

April 21, 1978

Mr. M.R. Tresler Pacific Gas and Electric Company Post Office Box 117 Avila Beach, CA 93424

Attention: Jim Cochran

Diablo Canyon Project - Specification 5422
Ready Mix Concrete for Concrete Block Work A4-30

Gentlemen:

With your approval we will use ready mix concrete for filler in our concrete block work.

The concrete to be furnished by San Luis Ready Mix. The mix will be 3/8" and designed for 2000 psi at 28 days.

Very truly yours,

GUY F. ATKINSON COMPANY

R.W. Wunderlich Project Manager

RWW/dh

Attachment

# GROUT MIX DESIGN FOR CONCRETE BLOCK CORE FILL

Design for one yard of concrete (saturated and dry surface): 2000 psi at 28 days.

cement 564 lb.

sand . 1907 lb.

3/8" agg. 827 lb.

water . 49 gal.

slump 5" + 1"

# Central Coast Laboratories

SAN LUIS OBISPO, CALIFORNIA 93401



Leborstory No. CC0155

Maked AIR VOL BLOCK, INC.

Semple 8 x 8 x 16" Open End Gray Concrete Blocks

Received January 19, 1972

winted by Air Vol Block, Inc.

SHRINKAGE TESTS:

• • •		Sample 1	Sample 2	Sample 3
	Saturated	10.0064	10.0070	10.0067
	Oven Dry	10.0010	10.0021	10.0018
	Shrinkage	.0054	.0049	.0049
	* Shrinkage	.054	.049	.049

AVERAGE SHRINKAGE = 0.051% SPECIFICATION = 0.06% Maximum

COMPRESSION TESTS:

	GROSS AREA	TOTAL LOAD	GROSS PSI
7 5/8 x 15 9/16"	118.7	222,500	1 874 47
7 5/8 x 15 9/16"	118.7	.221,000	1,874.47
7 5/8 x 15 9/16"	118.7	229,000	1,929.23
	FR105		.,,,,,,,

AVERAGE = 1,888.51

Respectfully submitted.

Robert E. Williams, Civil Engineer

REW:1

NOTE .