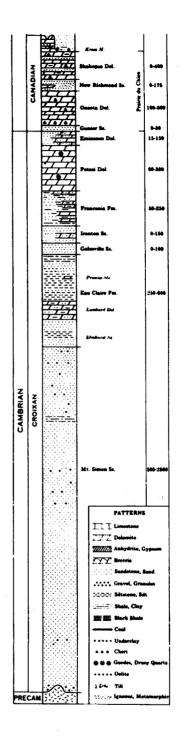


ķ	<u>س</u>	(:: :	Member (
ž	2		Glocial drift, losse, and allevial deposits	ı	0-500
QUATERNARY	LEISTOCENE				
à	7				
RE	GLF	HT-A	Baylia Pm.		9-100
	Г		Bond Fm		200
	MO.	100	-Short Creat La	ì	
	⊢		No 8 Coal Medicale Pin	MeLenneboro	146.05
			Lonedale Lo.	ž	-
7		Y	Sa 7124		
₹	Į				
₹	1		No 4 Coal		
PENNSYLVANIAN	DES MOINESIAN	$\mathbf{X}_{\mathbf{a}}$	Carbondale Fm.		125-23
Š	1 €		No. 6 Coal	ŧ	
Z	2			Keren	
O.	•	400	No # Conf	_	
•	1				
	ľ	HHOOF		į	2-110
	-		Scootle Lo Bernstutte Na	_	
	M.	144	Abbott Fm. Relates No. Cassaville For	نے خ	2-85
					49-98
			St. Louis La.		0-100
_	Z	1	Salem La.		0-00
₹	E		Source Sal		0-40
<u>a</u>	VALMEYERAN	10/4	Warsew SA.		10-200
55	1	24 1 1	Kookuk Le.		30-100
MISSISSIPPIAN	_	7111	Burlington Ls.		79-200
Ξ	1	180.100	Fern Gien Ls. Sedalia Ls.		
		leeleel e	Sedalia L Starra Cave La. "Prospect Hill Sts.	# =	0-10 0-12 0-12
	N N	11117	McCraney Ls.	North	3-30
_		initial .	Hannibal Sh. Louisiana Lo.	ķ	15-123 0-30
Ϋ́	1 2		Saverton Sh.	¥.	9-129
DEVONIAN	UPPER		Grassy Creek Sh. Sweetland Creek Sh.	New Albany	0-100 0-100
Ä	Q		Sylamore Sc. Cedar Valley Ls.	-	0-100 0-5 0-20
٥	Ī		Wagaipinicon La.		0-10
	İ				
	z	- 17			
Z	NIAGARAN	即、世	Recise Del.	1	0-500
SILURIAN	ò				
7	Ž				
S		75/74	Wookeoha Dol.		0-30
	\vdash	15.55	Jeliet Del.		40-164
	AL.		nantaine Del. Edgewood Del.		29-60 9-86
			Noda Fin.		9-15
	CINCIN	, , ,	Brainard Sh.	a de	0-100
	ž		Fort Atkinson La.	Maquoketa	5-50
	Ĺ	嘉嘉	Scolar Sh.	1	50-100
			Duffique Dol.		0-45
			Wise Lake Dol.		50-80
			Dunleith Dol.	Galena	100-144
	z	477		٠,	
	ž		Guttenberg Le. Spechts Ferry Sh		0-20 0-10 0-20
z	ζ.	477	Guttenborg Ls. Spechts Perry Sh. Quimbys Mill Ls. Nachuss Ls.		0-20 0-50
<u>₹</u>	CHAMPLAINIAN		Grand Detour Lo.	Platv	15-75
اَجَ	Į,		Mifflin Ls. Perstonics Dol. Glenwood Fm.	7	5-50 10-60 0-150
ORDOVICIAN					V-100
õ				Ancell	
			St. Peter Su.		



NOTE STRATIGRAPHIC COLUMN OF THE PLEISTOCENE DEPOSITS OF ILLINOIS IS SHOWN ON FIGURE 2.5-5.

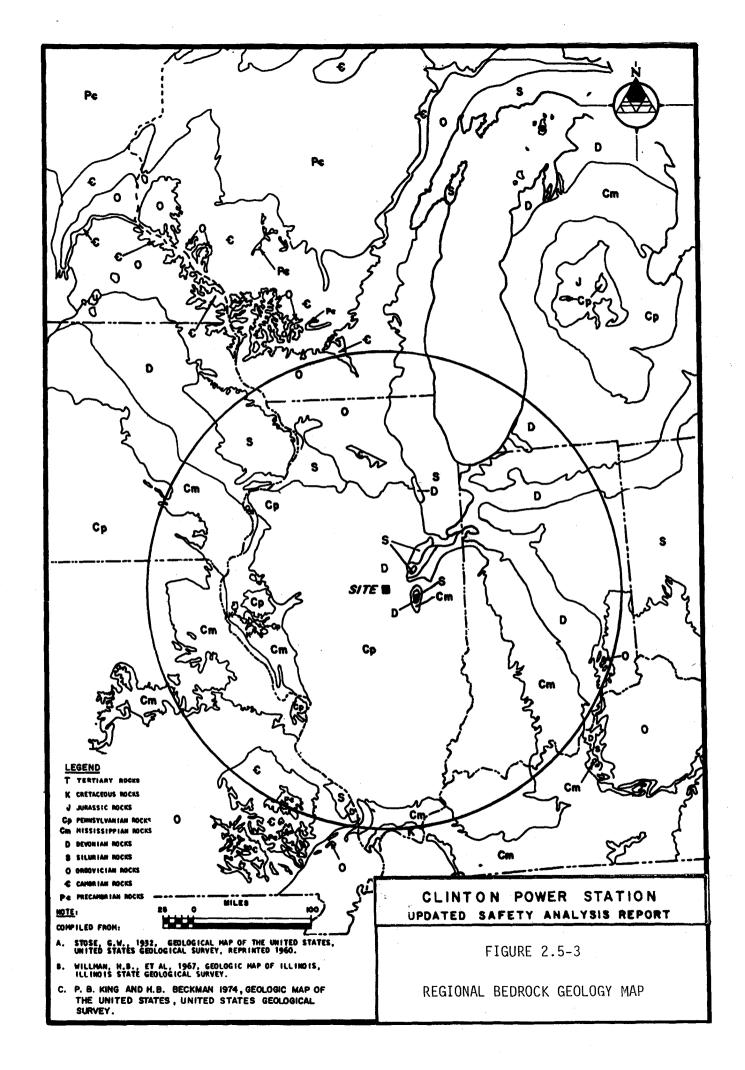
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

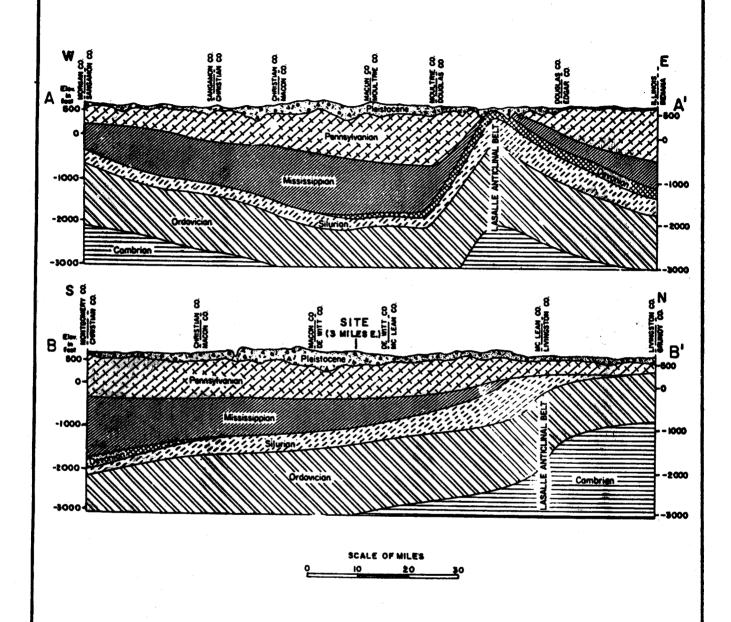
FIGURE 2.5-2

REGIONAL STRATIGRAPHIC COLUMN

REFERENCE:

ILLINOIS STATE GEOLOGICAL SURVEY, 1967, GEOLOGIC MAP OF ILLINOIS,





NOTES

- I. REFER TO FIGURE 2.5-7 FOR LOCATION OF REGIONAL GEOLOGIC CROSS SECTION.
- MODIFIED FROM: GROUNDWATER GEOLOGY IN EAST CENTRAL ILLINOIS BY L.F. SELKREGG AND J.P. KEMPTON, ILLINOIS STATE GEOLOGICAL SURVEY CIRCULAR 248, 1958.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-4

REGIONAL GEOLOGIC CROSS SECTIONS

т	IMI		RATIGRAPHY			ROCK STRATIGRAPHY	SOIL STRATIGRAPHY
		HOLOCENE STAGE				C Ravinio E Sand M. C Waukegon M. Q Loke Forest M. Winnetka M. Shehayaran M. Shehayaran M.	Modern Soil
	ľ		VALDERAN SUBSTAGE		Loess	Sheboygan M. o c o c	
		AGE	TWOCREEKAN SUBSTAGE	* * * * * * * * * * * * * * * * * * * *		Wodsworth T.M.	
Σ	S	INAN ST	WOODFORDIAN SUBSTAGE	Peorio L	Morton Loess	Haeger T.M. Yorkville T.M. Malden T.M. Tiskilwa T.M. Delayen T.M. Glenburn T.M. Lee Center T.M. Online T.M. College T.M. Solicy T.M. Solic	Jules Soil
<u> </u>	R E	ONS	FARMDALIAN SUBSTAGE		bein itt	Peddicord Fm.	Formdale Sail
Y S Y	ENE SEI	WISC	ALTONIAN SUBSTAGE	ono Si	feadow Loess M. AcDonough Loess M. Aarkham Silt M	Capren T.M. Plano Silt M. Argyle T.M. Capren T.M. Cap	Pleasant Grove Soil Chapin Soil
NA	10C		IGAMONIAN TAGE		-	. Berry Clay Member	Sangamon Soil
ATE	E I S.	E AN	JUBILEEAN SUBSTAGE	Sill	<u>.</u>	Radnor T.M. Sterling T.M. Toulon M. Winslöw T.M. Hagarstown M. Roby Silt M.	
	٦	STAGE	MONICAN SUBSTAGE	ovelond	Teneriffe Silt	Hulick T.M. Ogle T.M. Vandelia T.M.	
		ורנ	LIMAN SUBSTAGE	Lov	Peters- burg Silt	Ouncan Mills M. Mulberry Grave M. Kellerville T.M. Smithboro T.M.	Pike Soil
		KAR	MOUTHIAN TAGE ISAN TAGE			Lierte Clay Member Titton T.M. Willery T.M. Herkness Silt M. Belgium M. Sankety Mehamet Hageler T.M. Sand M. Sand M.	Yarmouth Soil
	ľ		ONIAN TAGE				Aften Soil
			RASKAN TAGE			Enion Formation Mounds Grover Gravel	

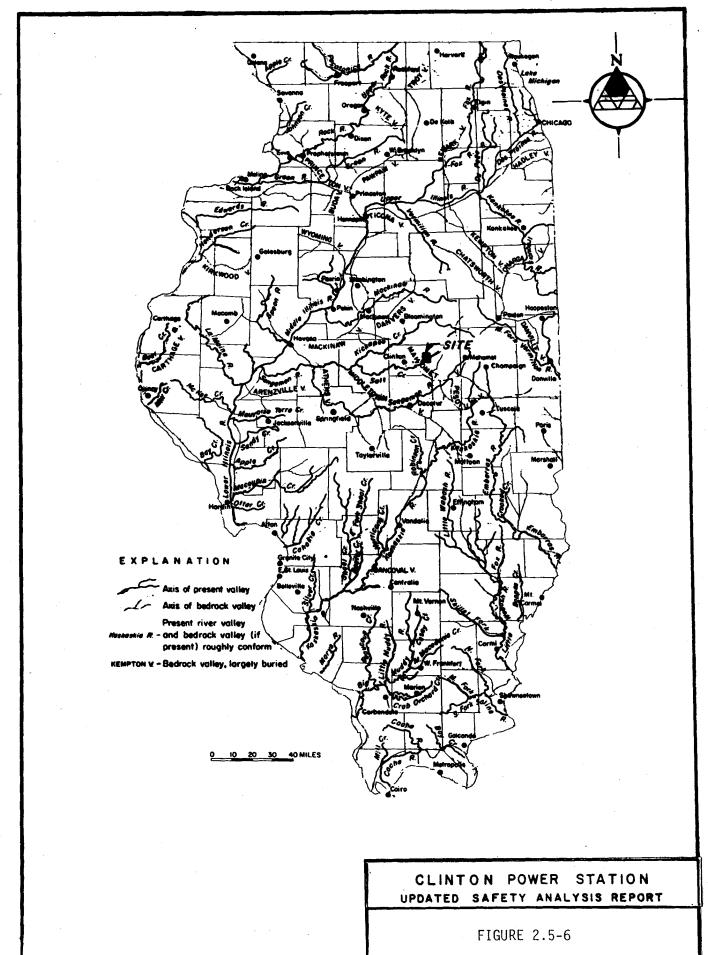
REFERENCE

I. M.B. WILLMAN, ET. AL., HANDBOOK OF ELHOIS STRATHGRAPHY, BULLETIN 95, ILLMOIS STATE GEOLOGICAL SURVEY, URBANA, 1975.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

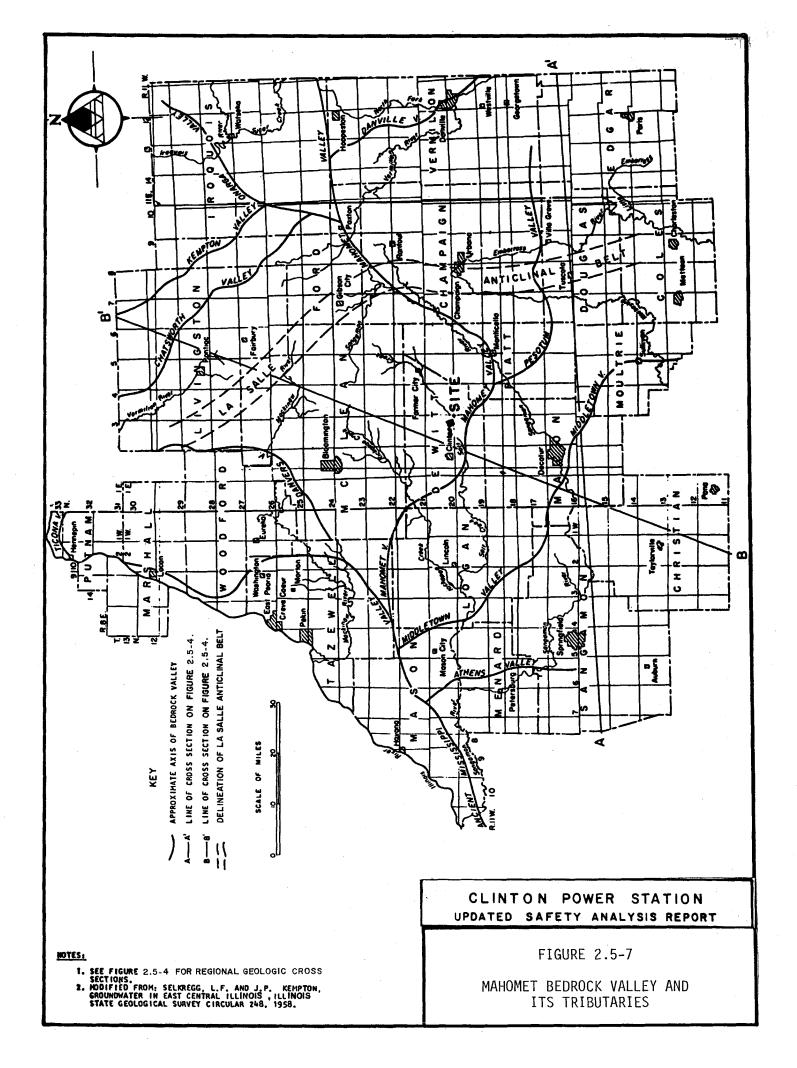
FIGURE 2.5-5

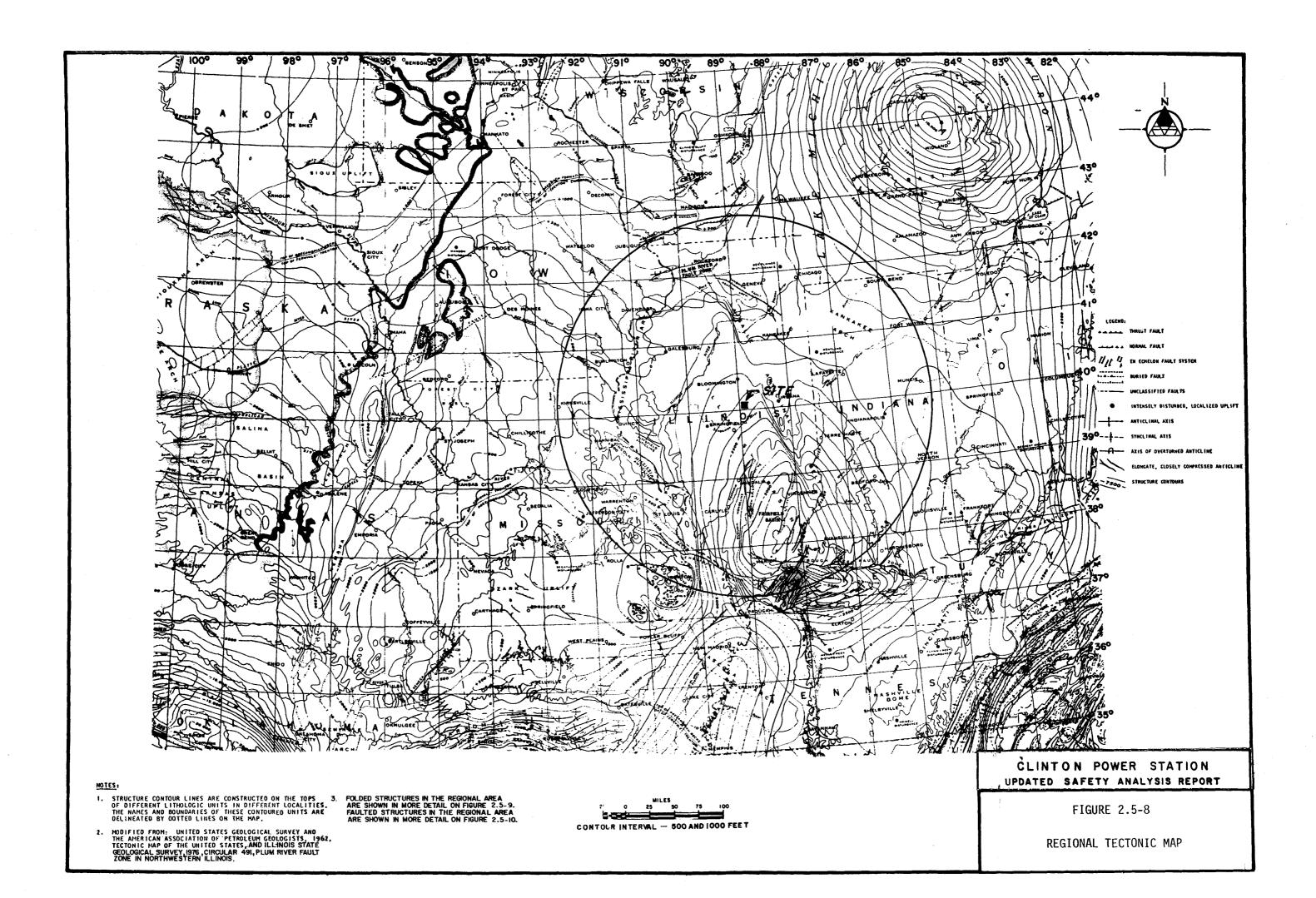
STRATIGRAPHIC COLUMN OF THE PLEISTOCENE DEPOSITS OF ILLINOIS

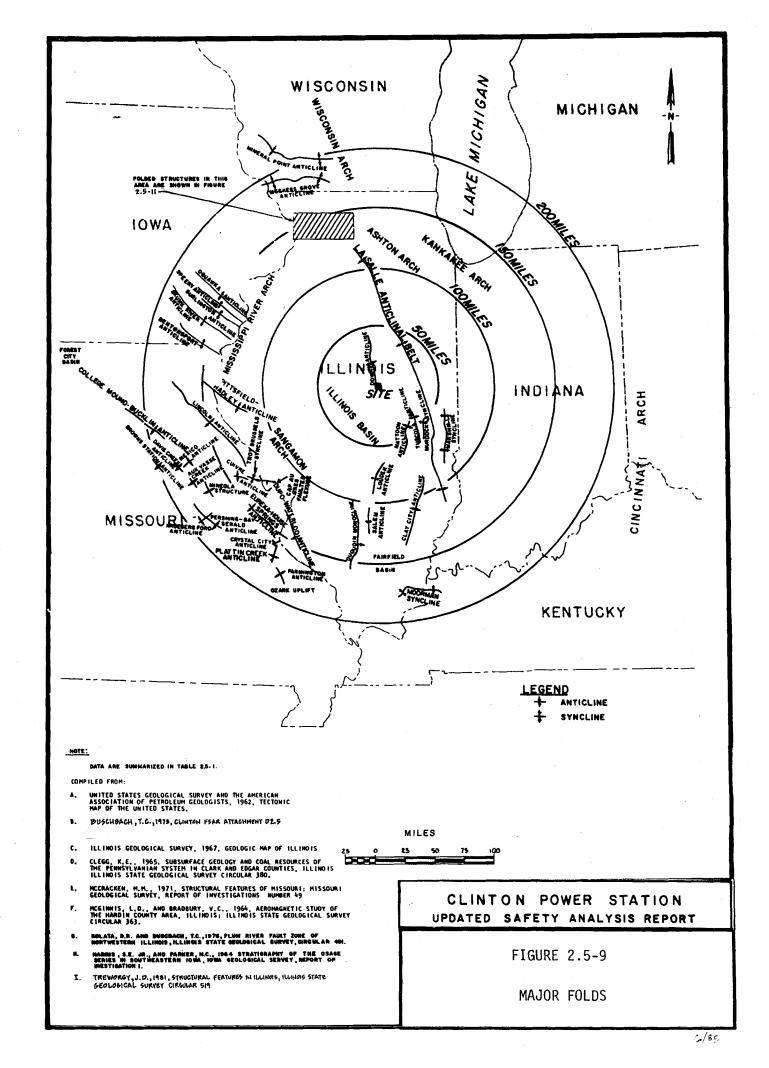


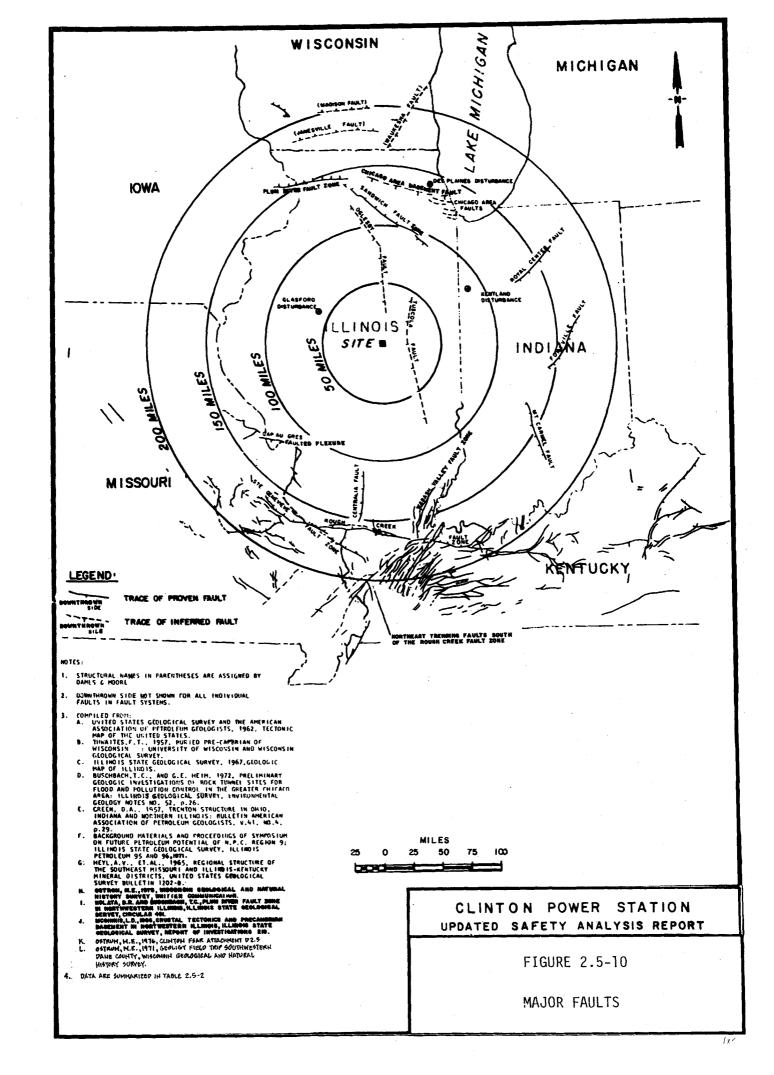
BEDROCK VALLEY MAP OF ILLINOIS

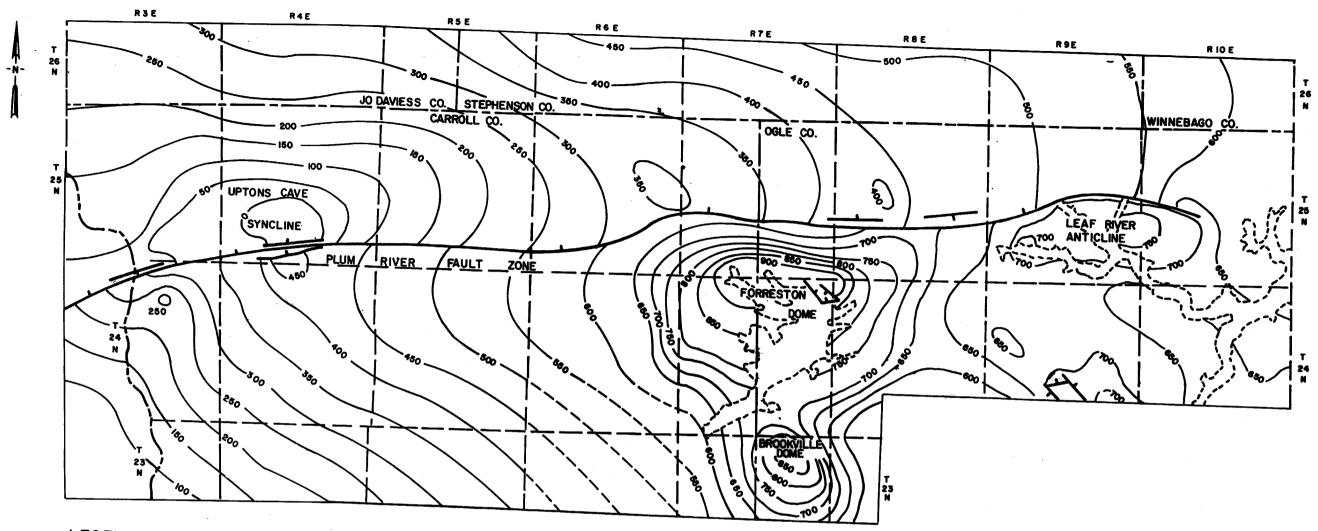
MODIFIED FROM: GLACIAL DRIFT IN ILLINOIS: THICKNESS AND CHARACTER, ILLINOIS STATE GEOLOGICAL SURVEY CIRCULAR 490, URBANA, 1975.











LEGEND

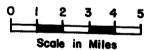
- ----- Top of Glenwood Formation eroded
- ---- State Line
- ——— Township Lines
- ——— County Lines
- Trace of Fault

Downthrust

Side

NOTE

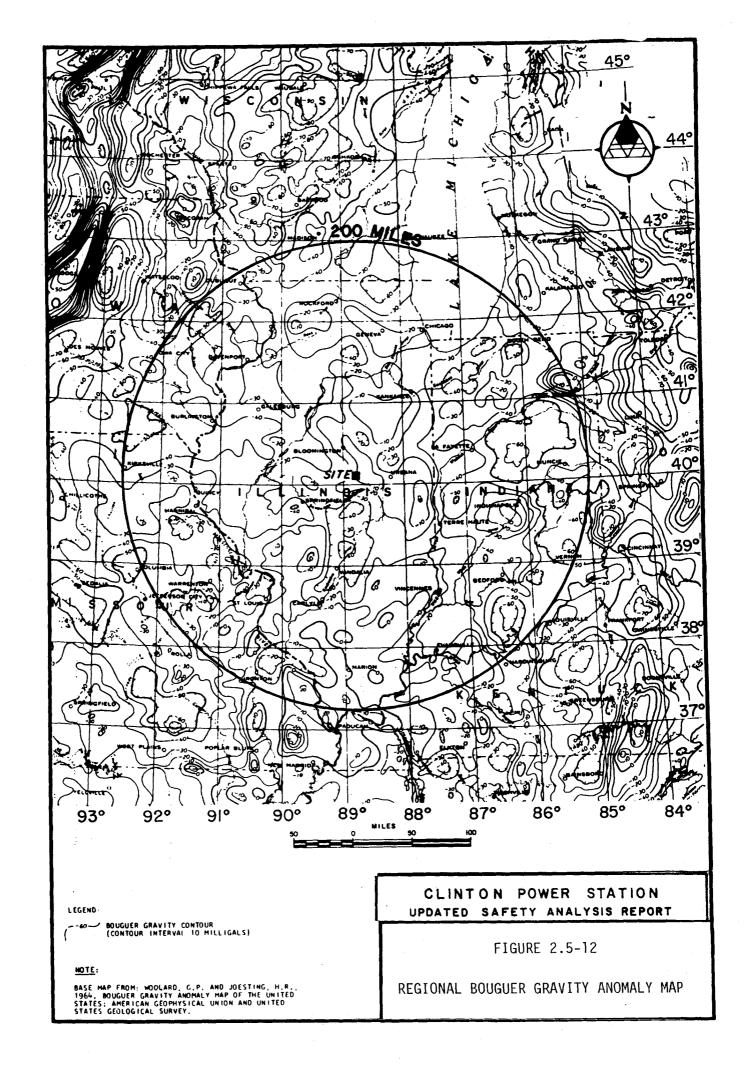
- I. Modified from Kolata D.R. and Buschbach T.C., Plum River Fault Zone of Northwestern Illinois, Illinois State Geological Survey Circular 491, 1976.
- 2. The location of this area with respect to the regional area is shown on Figure 2.5-9.

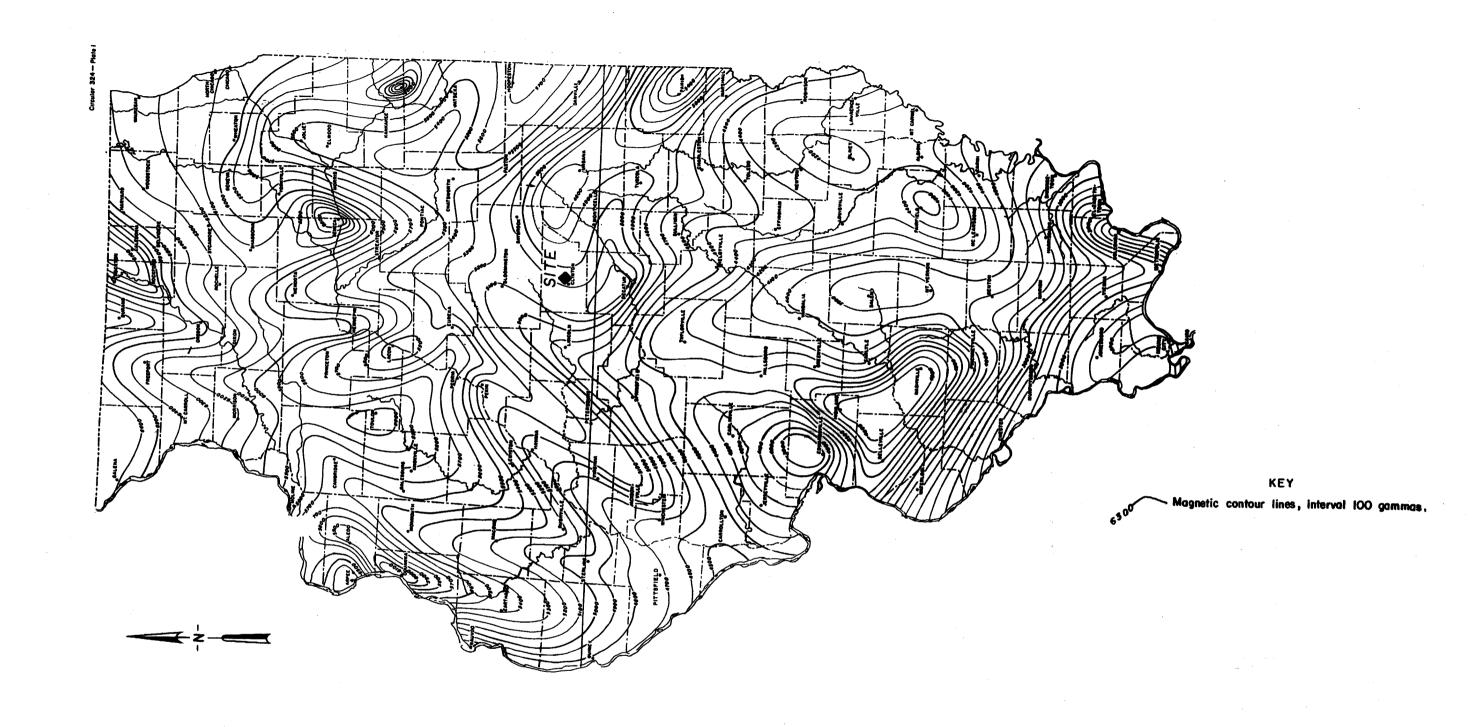


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-11

PLUM RIVER FAULT ZONE AND ASSOCIATED STRUCTURES IN ILLINOIS





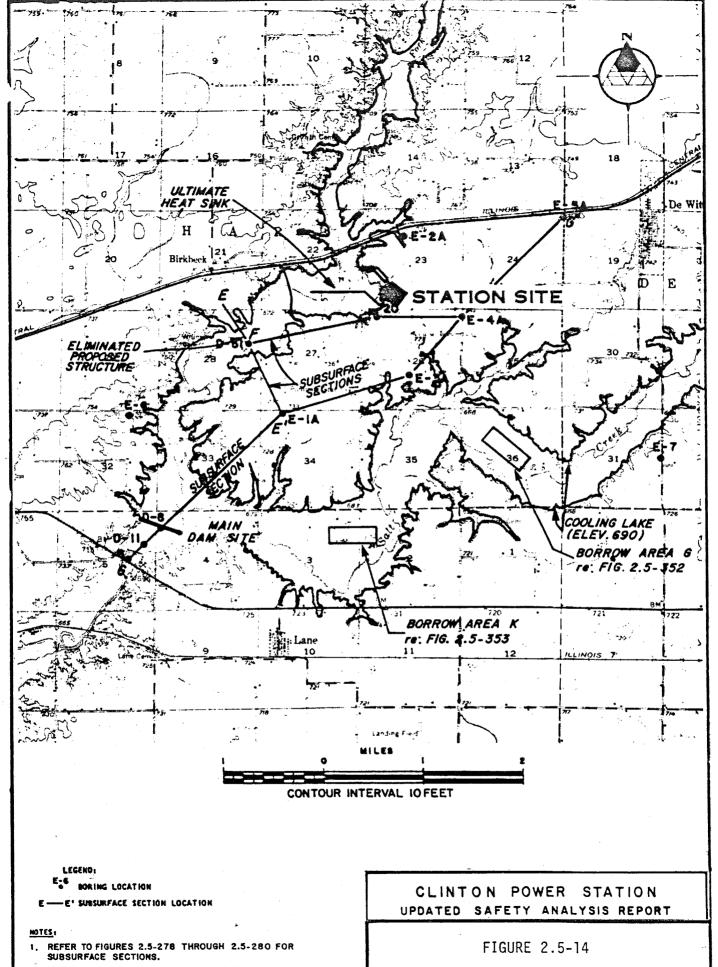
Scale in Miles

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-13

REGIONAL VERTICAL MAGNETIC ANOMALIES

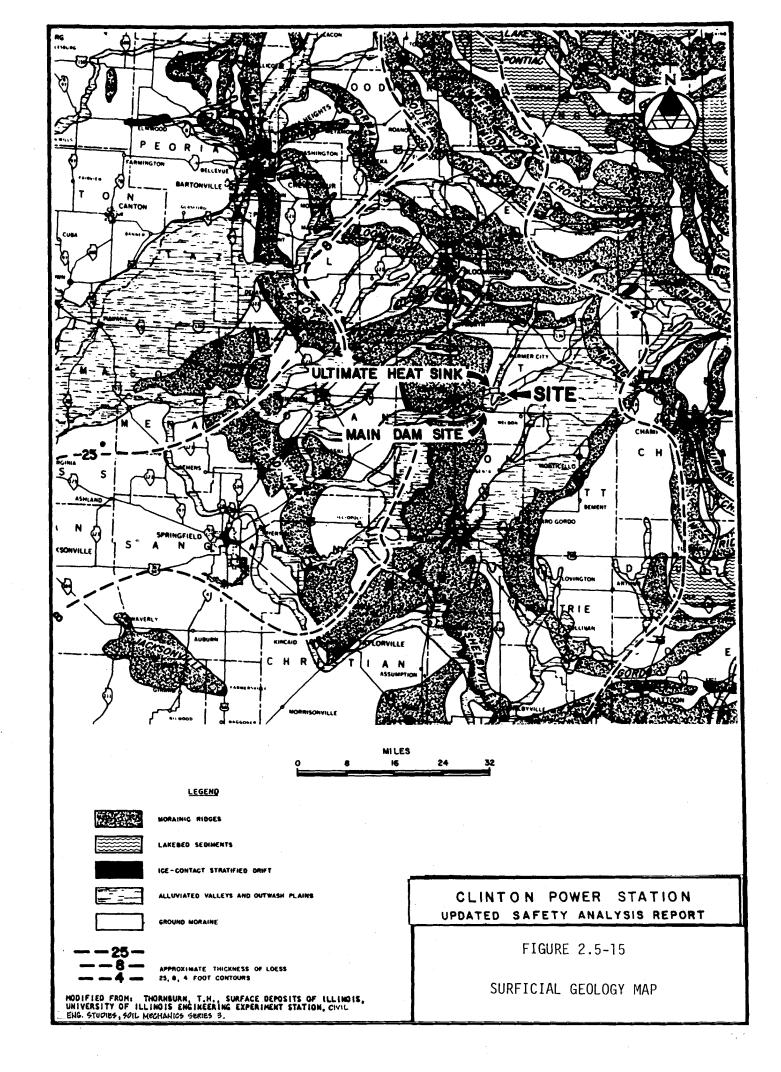
MODIFIED FROM: McGINNIS, L, D., AMD HEIGOLD, P.C., REGIONAL MAPS OF VERTICAL MAGNETIC INTENSITY IN ILLINOIS, ILLINOIS STATE GEOLOGICAL SURVEY CIRCULAR 324, 1961.

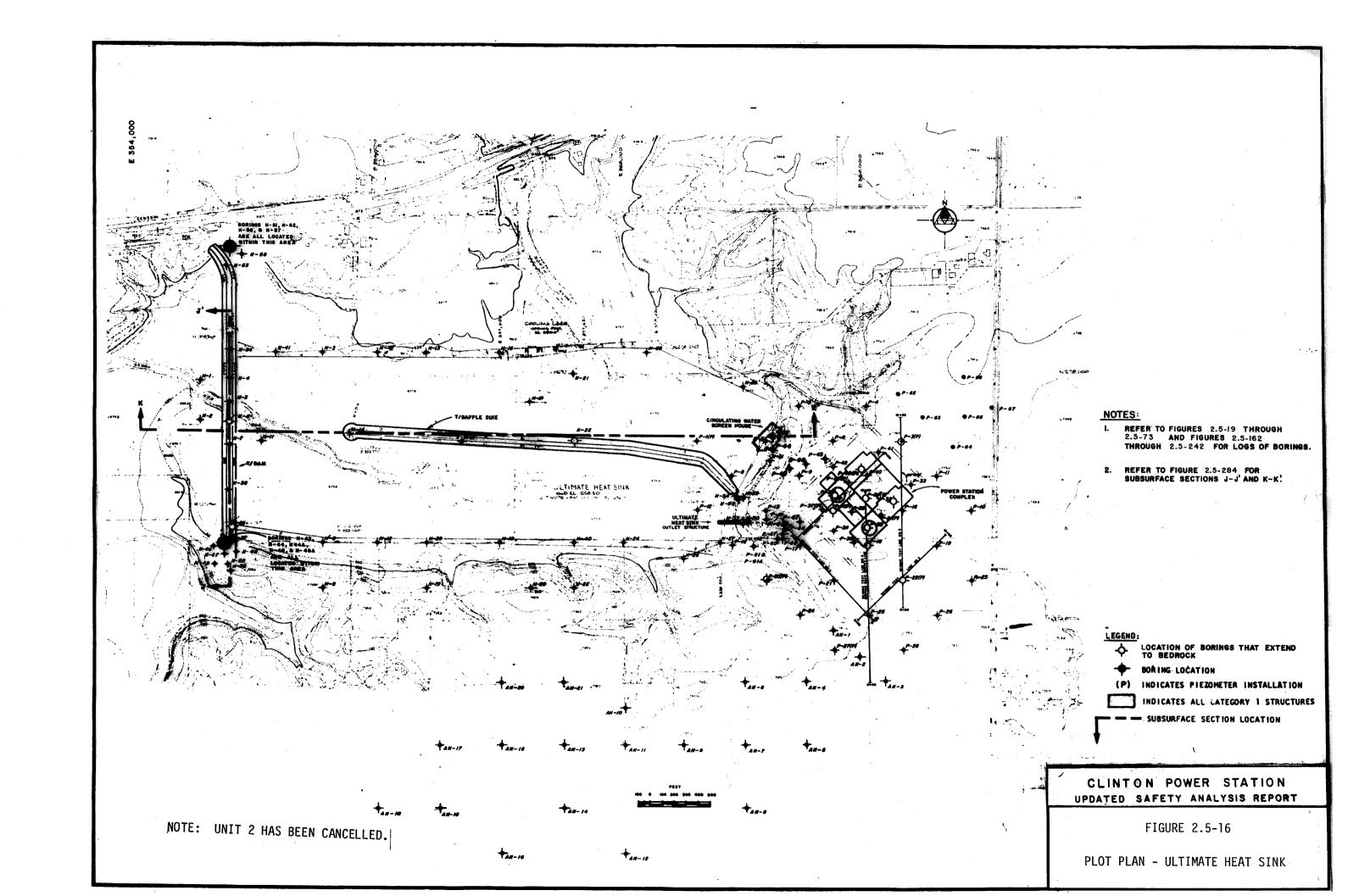


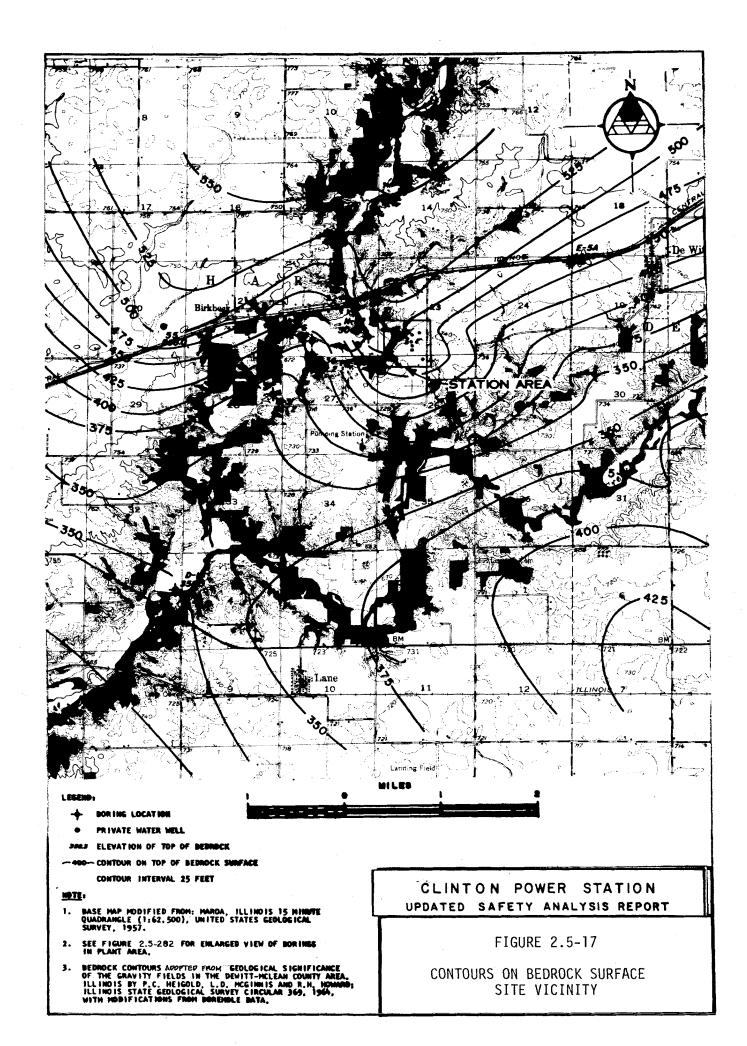
2. REFER TO FIGURES 2.5-80, 2.5-83, 2.5-96, AND 2.5-145 THROUGH 2.5-151 FOR LOGS OF BORINGS.

3. BASE MAP MODIFIED FROM: MAROA, ILLINOIS IS MIMUTE QUADRANGLE (1:62500) UNITED STATES GEOLOGICAL SURVEY, 1957.

SITE VICINITY MAP







			THE CONTACT OF A DAILY	STRATIGRA	PHIC UNITS
·		T	IME STRATIGRAPHY	UPLAND	VALLEY
•			Holocene Stage		Cahokia Peyton Colluvium Alluvium
		Stage	Valderan Substage	Richland L oe ss	
			Twocreekan Substage		Henry
System	Series	Wisconsinan	Woodfordian Substage	Wedron Formation	Formation
f		Wis	Farmdalian Substage	Robein Silt	
Quaternary	Pleistocene		Altonian Substage]
uate	leis	S	angamonian Stage		
ď	Ъ		Illinoian Stage	weathered Glasford Formation	
					tered l Formation
		Y	armouthian Stage	·	_
			Kansan Stage Uncon	formity Banner	Formation
				McLeansboro Group	Bond Formation
İ			Pennsylvanian	Medeansboro Group	Modesto Formation
			System	Kewanee Group	Carbondale Formation

STRATIGRAPHIC UNIT	APPROXIMATE THICKNESS*	GENERAL DESCRIPTION
Cahokia Alluvium	0-35 ft.	Alluvium and silty clay (CL,SM or ML)
Peyton Colluvium Richland Loess	0-10 ft. 0-10 ft.	Loess, clayey silt (ML or CL), may be leached, soft.
Henry Formation	0-35 ft.	Stratified sand and gravel (SP, GP, SM).
Wedron Formation	20-55 ft.	Till, clayey sandy silt till (ML or CL), stiff to very stiff, with lenses of stratified sand, gravel, or silt.
Robein Silt	0-2 ft.	Silt (ML or CL), black or dark brown, massive, soft.
weathered Glasford Formation	10-15 ft.	Silt and silty clay (ML or CL), weathered, soft; and till (ML or CL), weathered, soft with lenses of sand or silt; black, dark brown, green.
unaltered Glasford Formation	90-140 + ft .	Till, gray sandy silt (ML or SM), hard. Upper part may contain lenses of stratified sand, silt, or gravel.
Banner Formation	25-105 ft.	Complex sequence, variably consisting of glacio- lactustrine silt (ML or CL), hard; clay till (ML), hard; may be undelain by very dense sand (Mahomet Sand Member), O-140ft. thick.
McLeansboro Group and Kewanee Group	Not Completely Penetrated	Alternating beds of shale, siltstone, limestone, and coal bedrock.

STRATIGRAPHIC DESCRIPTION

NOTES:

- 1. The stratigraphic units are discussed in detail in subsection 2.5.1.2 and Attachment C2.5.
- 2. Figure 2.5-274 shows a comparison of stratigraphic nomenclature used in the FSAR, PSAR, and boring logs.
- 3. Excavations for the Clinton Power Station did not extend below the unaltered Glasford Formation.
- 4. Borings for the Clinton Power Station did not extend below rocks of the Carbondale Formation.
- 5. Illinoian-age till of the Glasford Formation was subjected to a significant period of weathering during the Sangamonian Stage and Altonian Substage.
- 6. Deposits of Cahokia Alluvium and Henry Formation were not differentiated; reported approximate thicknesses of each unit represents a combined thickness for both deposits. The Cahokia Alluvium is Holocene and quite possibly, in part, Valderan/Twocreekan in age; the Henry Formation is Woodfordian (probably early) in age. The Wedron Formation is probably early Woodfordian.

- 7. The Holocene Stage is represented by a significant period of weathering and development of agricultural soil profiles.
- 8. Vertical scale does not represent either relative thickness of stratigraphic units or relative duration of time interval.
- 9. Standard Unified Soil Classification symbols are used.
- 10. Locally, the Peyton Colluvium rests directly on Glasford Formation.

*Based on data from excavations and boring logs.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-18

SITE STRATIGRAPHIC COLUMN

				LAB	ORAT	ORY	TEST	DATA						
		TESTS REPORTED SEWHERE	ATTER LIN	BERG			STRENG		URE	Ţ	BLOW COUNTS		В	DRING P-I
		TES' EPOR SEWI	LIMIT	PLASTICITY INDEX		RESSION		POCKET PENETROMETER	MOISTURE CONTENT	DRY DENSITY pef	COC		SU	RFACE ELEVATION 675.9
	680	ER	*	*	9 bit	ы	C 94	d 5	ۆۆ	٥	BLOW CO			
											8 3		IBOLS	DESCRIPTIONS
	670							1500	. •		4 🗈		ML ML	BLACK CLAYEY SILT WITH TRACE OF SAND AND ORGANIC MATTER - (TOPSOIL) Top of Salt Creek Alluvium
	670								23.3	102	'	7.4 A.,	GP-	MOTTLED BROWN AND GRAY SANDY SILT WITH CLAY
		SA			! .				11.2		14 🗷	4 4	SΡ	BROWN GRAVELLY FINE TO COARSE SAND (MEDIUM DENSE)
	660				<u> </u>			4500+	9.9	131	23 ₺			Top of Illinoian Glacial TIII GRAY CLAYEY SILT WITH SAND AND OCCASIONAL GRAVEL (VERY STIFF)
		·						4500+			72 🗷		l	GRADES TO HARD
FEE T	650							4500+	8.5	137	120/6" 1			
FE	000					ľ		4500+			125/6" 12			
3	640							4500+	9.7	131	150/5" E		l	
								4500+			132 🛭		ML	FIL SEAM OF SPAVEL
ELEVATION	630							4500+	7.9	136	10046, 1			5" SEAM OF GRAVEL
Z	555							4500+			58 🛭			
ELE	620	<u> </u>						4500+	8.7	137	47 🗷			
								4500+			34 🛭			•
	610	МА						4500+	9.6		55			
								4500+			48 🛭			
	600	<u> </u>	<u> </u>		<u> </u>			4500+	7.8	139	136	•		
								4500+			89 🖟			BOD INC. COMPLETED AT 70 E SEET
1		1			1									BORING COMPLETED AT 79.5 FEET ON 6-26-72 CASING USED TO A DEPTH OF 6.0 FEET
1	590	<u></u>									┛・			CASING OSES TO A SELECT OF CONTRET

PIEZOMETER INSTALLED ON 6-26-72
PIEZOMETER TIP PLACED AT ELEVATION
609.9. PEA GRAVEL WAS PLACED FROM
ELEVATION 596.4 TO ABOVE THE TIP;
A BENTONITE SEAL; AND CEMENT GROUT
TO ELEVATION 675.9.
PIEZOMETER INSTALLED ON 6-26-72
BORING P-1B WAS DRILLED ADJACENT TO
P-1A, AND THE PIEZOMETER TIP WAS
PLACED AT ELEVATION 665.9.

WATER LEVEL READINGS

DEPTH BELOW GROUND
SURFACE IN FEET
TIP ELEVATION 609.9 TIP ELEVATION 665.9 DATE

1.9 2.9 8-3-72 1.8 3.1 8-15-7 2.5 3.8 8-6-72

REFER TO FIGURE 2.4-36 FOR WATER LEVEL OBSERVATIONS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-19

LOG OF BORING P-1

NOTE

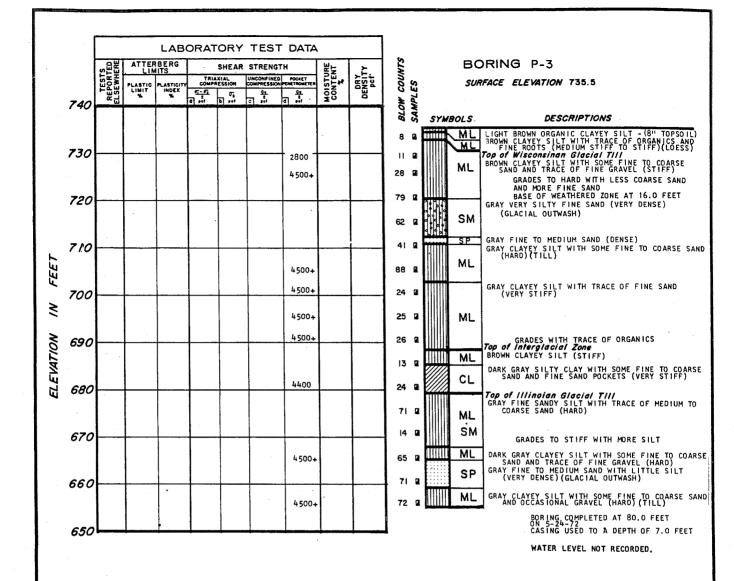


FIGURE 2.5-20

LOG OF BORING P-3

NOTE:

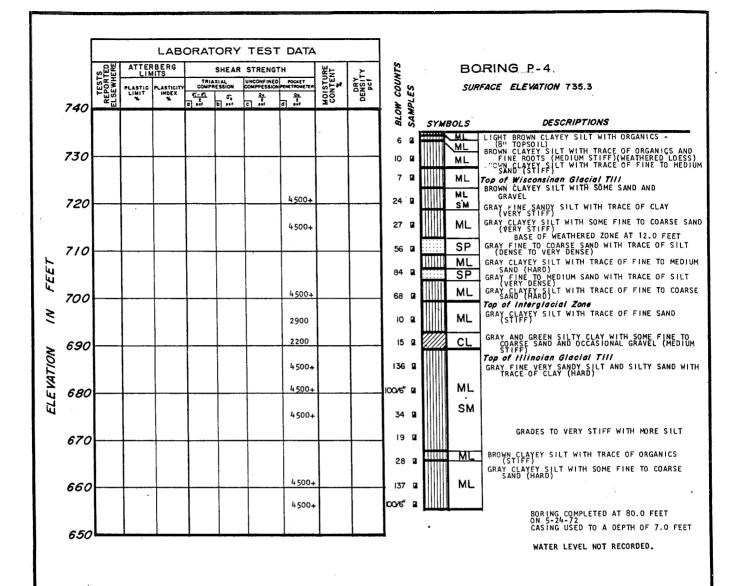


FIGURE 2.5-21

LOG OF BORING P-4

NOTE:

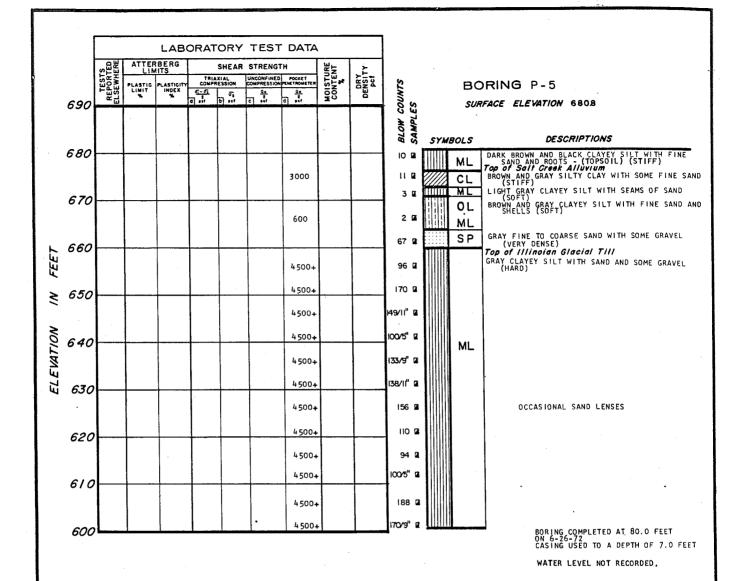


FIGURE 2.5-22

LOG OF BORING P-5

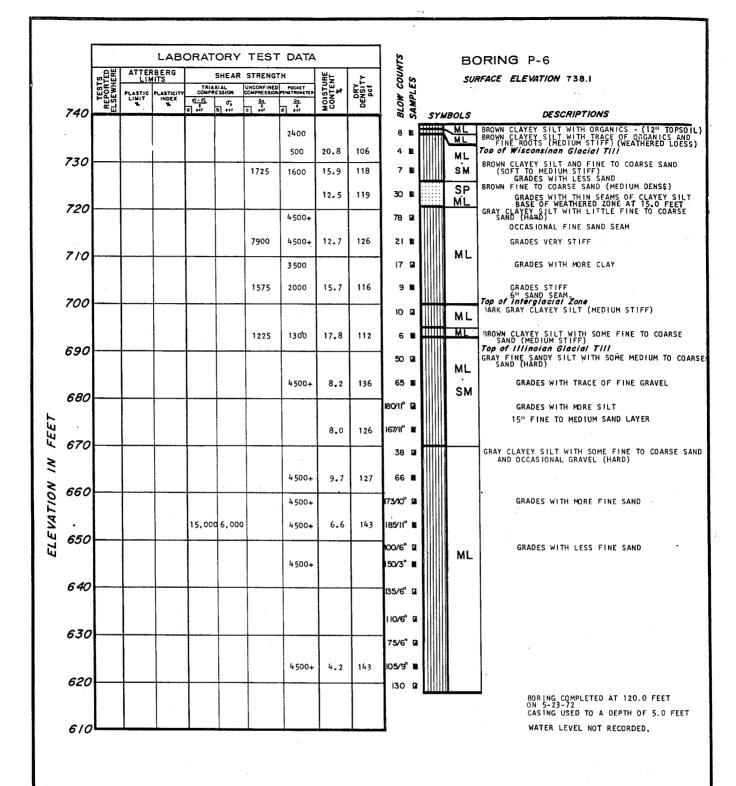


FIGURE 2.5-23

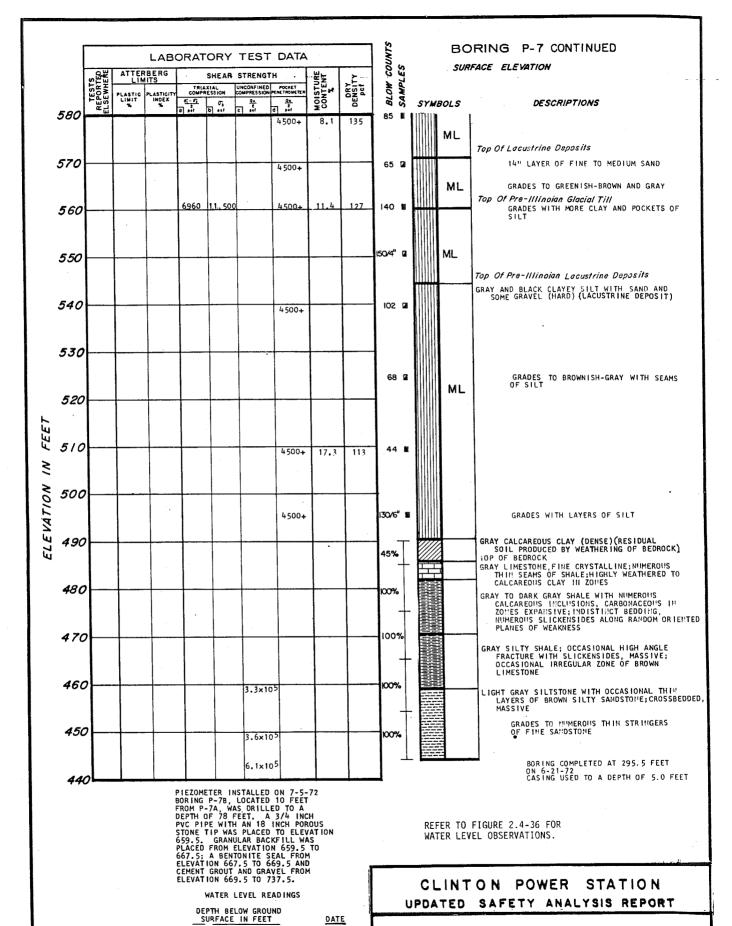
LOG OF BORING P-6

NOTE:

				ORAI	UKT	1531	DATA		1	BORING P-7
	TESTS REPORTED ELSEWHERE	ATTER	BERG			STRENG	POCKET PENETROMETER	MOISTURE CONTENT	mæn. ≿==	BORING P-7 SURFACE ELEVATION 737.5
	TE: REPO LSEW	PLASTIC LIMIT	PLASTICITY INDEX	4-6		COMPRESSION	94	CONT	DENSITY pcf	SURFACE ELEVATION T3T.5
740	_ ω			0 647	6 pri	C 941	d 64			ซี่ รั SYMBOLS DESCRIPTIONS
							2600	-		12 8 CL SHOWN CLAYEY SILT WITH FINE SAND AND ROOT (TOPSOIL) CL LIGHT BROWN SILTY CLAY (STIFF) (WEATHERED TOP OF Wisconsinan Glacial Till)
30								11.0	118	6 Top of Wisconsinan Glacial Till TAN AND BROWN FINE SANDY SILT (LOOSE)
							1700	11.5	120	IO E CL LIGHT ROWN SANDY CLAY WITH SILT AND SOME
							4500			APPROXIMATE BASE OF WEATHERED ZONE AT GRAY CLAYEY SILT WITH SAND AND SOME GRAVI (VERY STIFF)
720										31 Q LAYER OF REDDISH-BROWN FINE SAND W SOME GRAVEL LAYER OF BROWN SILTY CLAY WITH SOME
							4500+	7.4	133	I IIIII MAI I SAND AND GRAVEL
710	┣─			 		 	-		122	CAYER OF GRAY FINE TO MEDIUM SAND W
				1			4500+			90 2 OCCASIONAL LAYERS OF FINE SANDY SIL
700						3900	4500+	13.8	123	GRADES TO VERY STIFF Top of Interplacial Zone, GRAY CLAYEY STIFT NO STIFF SOME GRAVEL (MEDIUM STIFF TO STIFF)
							2700			8 2 ML SOME GRAVEL (MEDIUM STIFF TO STIFF)
						3050	3300	18.2	111	14 B CL
590			-	-	 		 		ļ	48 2 Company of Illinoian Glacial Till LIGHT GREENISH-GRAY SILTY SAND AND SANDY WITH OCCASIONAL GRAVEL (VERY DENSE TO
					1		4500+	5.1	142	78 B THE SM GRADES WITH MORE SILT
6 80	<u> </u>		-		 	-				
							4500+			
										94 8 SP GRAY FINE TO MEDIUM SAND WITH SILT AND S
670										32 A GRAY SANDY SILT WITH SOME GRAVEL AND TRA
								4.3	138	GRADES WITH MORE SAND SM GRADES WITH LAYERS OF DARK GRAY F TO MEDIUM SAND WITH SOME GRAVEL
660	┝─		 	-	-	-	+ •		1	= }
				1			4500+			94 2 GRAY CLAYEY SILT WITH SAND AND SOME GRAY (HARD) (SAND, SILT AND CLAY CONTENT VA WITH DEPTH)
650	<u> </u>	<u> </u>	<u> </u>	<u> </u>	 					
				1						175/If" Q
~ 40							4500+			200/5" 🗷
640										190/6" @
							4500+			155/10 ¹ @
630		 	 	 	-	 	4500+	╂		- IEO/IO" IR
										105 g ML
620						1	4500+	ļ		
•							4500+			3" LAYER OF GRAY FINE SAND
										~
610		1					4500+	1		112 0
600	<u>/</u> -	 	-	-	-	-	4500+		-	85 2
590	,	<u> </u>		$oldsymbol{ol}}}}}}}}}}}}}}}}}$						
										75 2 6" LAYER OF FINE TO MEDIUM SAND
580) 						_1			CLINTON POWER STATION

FIGURE 2.5-24

LOG OF BORING P-7 (SHEET 1 of 2)



NOTE

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

52.7

8-29-72

FIGURE 2.5-24

LOG OF BORING P-7
(SHEET 2 of 2)

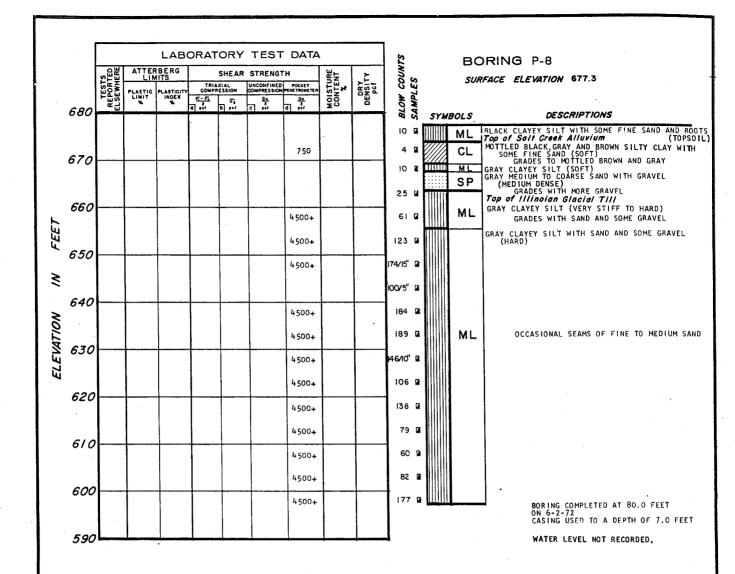


FIGURE 2.5-25

LOG OF BORING P-8

NOTE:

	٦,	REPORTED ELSEWHERE	ATTER	BERG			STRENG		3E NT	Ł	15		·E	BORING P-9
		E SE	PLASTIC	PLASTICITY INDEX		AXIAL RESSION		D POCKET ON PENETROMETER	MOISTURE CONTENT	DENSITY pcf) Š ,		_	SURFACE ELEVATION 744.1
75	: <u>,</u> L	ELS.	LIMIT	*	0 m	σ, b ***	Qu C 961	9 101 84	9 8	8	BLOW COUNTS	í	•	JOHN AGE LEE VALVOIT VIVIA
, ,	۲										107	Ì		
	- 1				1						80		MBOL	
					1						8 🗷	1	N.	(15" TOPSOIL)
74	7							1900			6 2		ML	
										1	10 12		CI	Top of Wisconsinan Glacial Till
					İ		-		ļ	1	" -		``	GRADES, WITH MORE SAND AND TRACE OF FINE GRAVEL
73	70							4500+			16 🛭	' 		BASE OF WEATHERED ZONE AT 12.0 FEET GRAY CLAYEY SILT WITH TRACE OF FINE TO CO SAND (STIFF TO VERY STIFF)
					ļ	1		4500+		\ '	39 🛭	1		GRADES TO HARD WITH LESS CLAY AND MOR
72	ام	_					<u> </u>	1			l		ML	SILT AND FINE SAND
12			-					4500+			27 0	·		
							İ	4500+		İ	152		М	GRAY CLAYEY SILT WITH SOME FINE TO COARSE AND TRACE OF FINE GRAVEL (HARD)
										1		Щ	141	Top of Interglacial Zone
71	0				1	†	1				25	•	М	GRAY FINE SANDY SILT WITH TRACE OF CLAY (
								ĺ			74 0	.		•
							-			1	'-	' ЩЩ	S	Top of Illinoian Glacial Till
70	20 F				 	-	+	4500+		 	33 0	z		GRAY CLAYEY SILT WITH TRACE OF FINE TO CO SAND (VERY STIFF)
70					1		1				ł			
								4500+		1	29 0	·	М	GRADES WITH MORE SILT
69	90			<u> </u>		-			ļ	ļ	19 6	.		
								4500+			'	· Ш		
	-							1600			12 (2	М	GRAY CLAYEY SILT (STIFF)
68	ام				<u> </u>]	. 1111	M	
00	77									1	53 1	⁴	III S	M GRAY CLAYEY SILT WITH TRACE OF FINE TO CO
						-		4500+			40 1	2	Шм	SAND (HARD) (TILL)
	ا ,								['''	1
67	7			T					T-		100	a	S	P GRAY FINE TO COARSE SAND WITH TRACE OF S
	-							4500+	1		104		M	IL IGRAY CLAYEY SILT WITH TRACE OF FINE TO CO
	-							7,500			""	ننبدو -		SAND (HARD) (TILL) BORING COMPLETED AT 80.0 FEET
66	50 L				—				<u> </u>		_			ON 5-22-72

FIGURE 2.5-26

LOG OF BORING P-9

NOTE:

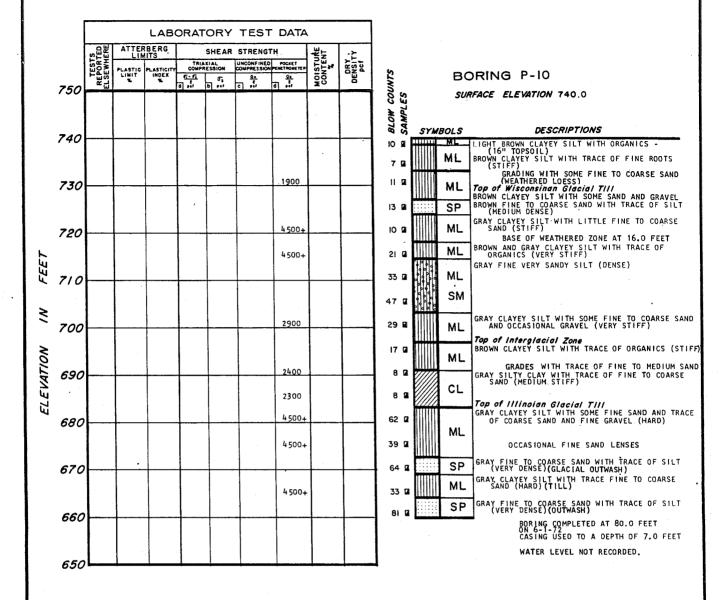


FIGURE 2.5-27

LOG OF BORING P-10

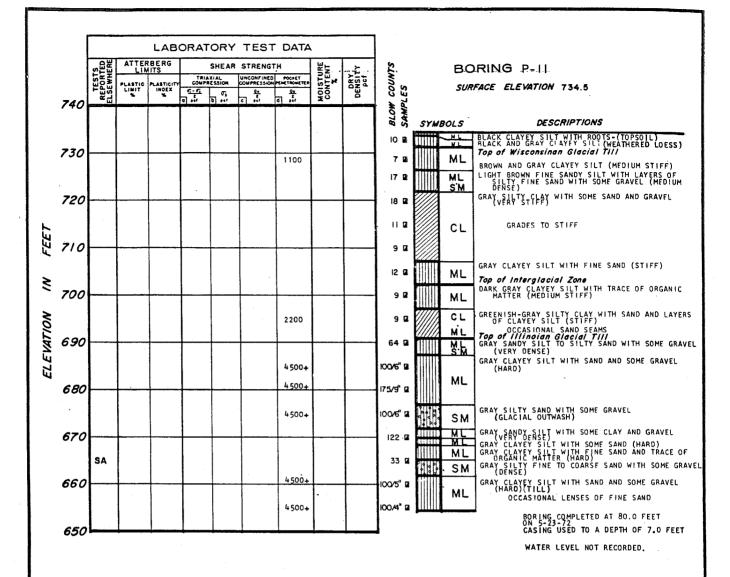


FIGURE 2.5-28

LOG OF BORING P-11

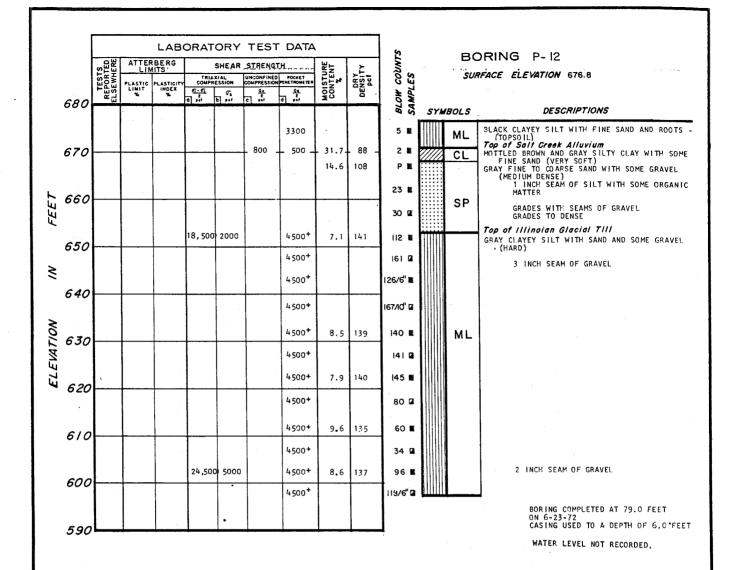


FIGURE 2.5-29

LOG OF BORING P-12

NOTE:

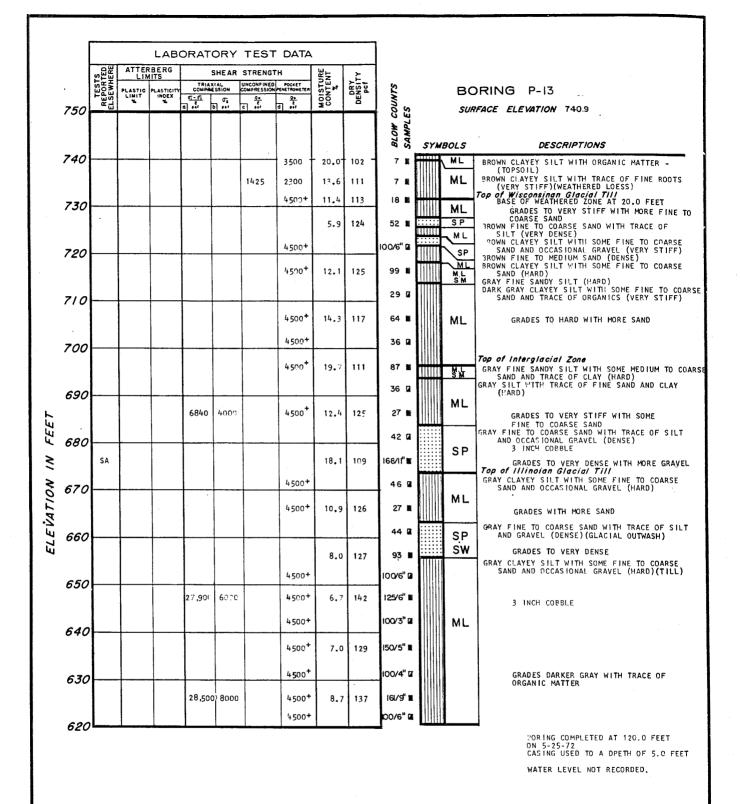


FIGURE 2.5-30

NOTE:

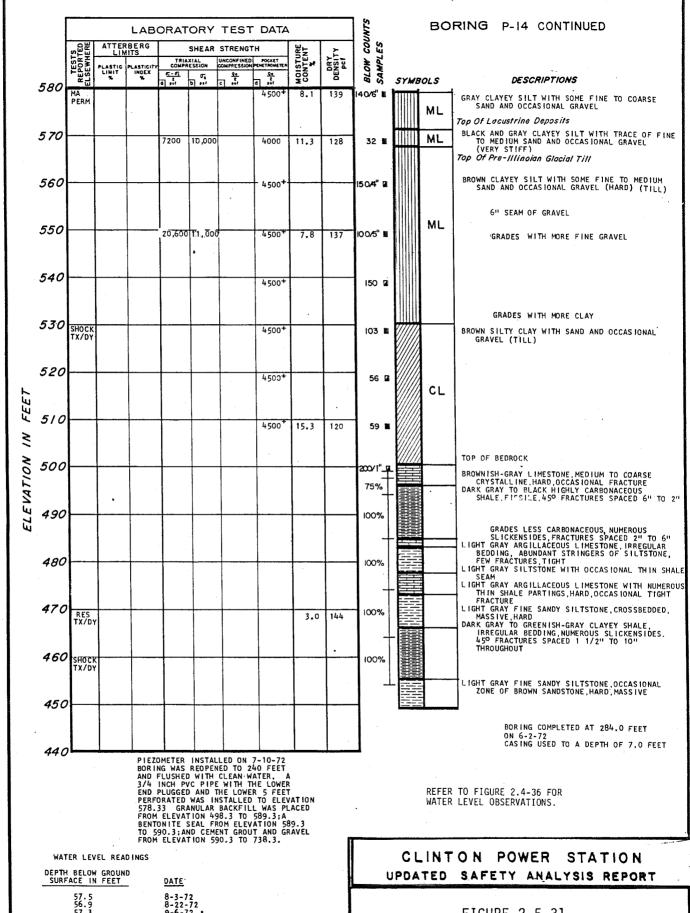
SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

LOG OF BORING P-13

	<u></u>			ORAT	ORY	TES	T DATA	<u> </u>		77		В	DRING P-14
	TESTS REPORTED ELSEWHERE	ATTE	RBERG	1		STRENG		NT	<u>}</u>	BLOW COUNTS SAMPLES		SU	RFACE ELEVATION 738.3
	TES EPOF SEW	PLASTIC LIMIT	PLASTICITY	COMP	EIAL ESSION		PUSPET In reset occurring to	MOISTURE CONTENT	DENSIT	100			
740	2.5	*	-3°		□	न 🏗	न ः —	. <u>₹</u> 8	. ä ,	BLO	SYA	BOLS	DESCRIPTIONS
							2400			8 12		HL.	SECOND CLAYEY SILT WITH SOME GREANICS - (1095
							400	18.4	109	2 1	15111	HL	MOTTLED TROWN AND GRAY CLAYEY SILT (VERY SI (WEATHERED LOESS) -Top of Wisconsinon Glacial Till INE TO COARSE
730	-			2320	1000		2100	14.6	121	8 8		ML	BROWN CLAYEY SILT WITH SOME FINE TO COARSE SAND AND OCCASIONAL GRAVEL (STIFF)
							1.00	17.0	'''	~-			BASE OF WEATHERED ZONE 17.0 FEET
				·						35 ⊯		ML	GRAY CLAYEY SILT WITH SOME FINE TO COARSE SAND (STIFF)
720	'			 			1		 	65 Q			7" SEAM OF GRAY FINE TO MEDIUM SAND GRAY CLAYEY SILT WITH TRACE OF FINE SAND (HARD)
	TV /DV									ł		ML	OCCASIONAL FINE SAND STRINGER
	TX/DY			880	2500		4500	16.6	117	27 ₪			GRAY CLAYEY SILT WITH SOME FINE TO MEDIUM
710		-		<u> </u>			1		 	33 🛭		ML	SAND (HARD) 2 INCH SEAM OF BROWN FINE SAND
							1200		1			CL	GRAY SILTY CLAY WITH TRACE OF FINE TO MEDIL
700							1200	}		10 🗷		OL.	SAND (STIFF)
700							4000		<u> </u>	11 22		ML	GRAY CLAYEY SILT WITH SOME FINE TO COARSE S AND OCCASIONAL GRAVEL (STIFF)
				5880	5000		4500+			16 ■			Top of Interglacial Zone
690							7,00	17.2	102	10 =		ML	DARK GRAYISH-BROWN CLAYEY SILT WITH ORGANIC ODOR
050										14 9	1111	IVIC	DARK GRAY SILTY CLAY WITH TRACE OF FINE SAN
	С	12.5	12.5				1600	16.2	115	12 🗷		CL	(STIFF) GRADES WITH SOME FINE TO MEDIUM SAND
680				L						100/6" 🖪			Top of Illinoian Glacial TIII
	CHEM+												GRAY FINE SANDY SILT WITH SOME MEDIUM TO CO SAND AND OCCASIONAL GRAVEL (HARD)
				21,700	4000		4.500+	8.1	139	77 €		ML	
670	\vdash						-		ļ	118 2		SM	OCCASIONAL FINE SAND SEAM
										110 4		0111	GRADES CLAYEY
		11.0	4.C				4500+	5.1	142	106 ■			
660	SA									35 0		sw	BROWN FINE TO MEDIUM SAND WITH TRACE OF SIL
	SA						4500+	9.5	129	100/5" ■		SM	(DENSE) (GLACIAL OUTWASH) GRAY CLAYEY SILT WITH SOME FINE TO COARSE S
650	PERM												AND OCCASIONAL GRAVEL (TILL)
650									 	100/6, 0			
							4500+			100AT 8			
640							4500+			140 000			
• .•							1,000			140/6" 13			
			>	15,000	9000		4500+	8.2	139	100/4" 🛢			
630				<u> </u>						1 .			
							4500*			IEONO, IT			
	TX/DY			20,500	9000	1	4500 ⁺	8.3	139	105∕6" ■			
620				 	 	<u> </u>	4500+		ļ	127 2		ML	_
										""			TRACE OF ORGANICS
_													
610	RES. TX/DY			26,300	9000	 	4500+	7.6	139	100/5"			
									•				· ·
c00													
600	SHOCK TX/DY						4500+			100⁄e, ■			GRADES WITH LESS COARSE SAND
590													
							4500+			155 Q			
580									<u></u>	1		CW	GRAY FINE TO CHARSE SAND WITH CHANCE AND TO
	_		ER SAMP							_		∍ S W	GRAY FINE TO COARSE SAND WITH GRAVEL AND TR OF SILT (VERY DENSE)

FIGURE 2.5-31

LOG OF BORING P-14 (SHEET 1 of 2)



SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

FIGURE 2.5-31

LOG OF BORING P-14 (SHEET 2 of 2)

				ORAT	UKY	1651	DATA		1	ည		P	ORING P-15	
	TESTS REPORTED ELSEWHERE	ATTER LIM	BERG ITS	L		STRENGT		URE		CN)	SURFACE ELEVATION TS6.3			
	TES POR SEWI	PLASTIC LIMIT	PLASTICITY INDEX	TRIA COMPR			POCKET PEHETROMETER	MOISTURE CONTENT	DRY ENSITY pcf	, coi		SUK	TAGE ELEVATION 130.3	
740	EL.S.	*	*		σ <u>.</u> Β :••	C pel	d 94 d 94	¥8	90	BLOW COUNTS SAMPLES	6617	BOLS .	DESCRIPTIONS	
							2600	25.2	98	7 ME	37 <i>W</i>	ML	LIGHT BROWN CLAYEY SLIT WITH SOME ORGANI	
730							1000	21.8	106	5 1⊠		ML	(TOPSOIL) BROWN CLAYEY SILT WITH TRACE OF FINE ROO' Top of Wisconsinan Glacial Till (STI	
				2390	1000		1600	13.9	123	8 №		ML	MOTTLED BROWN AND GRAY CLAYEY SILT WITH OF FINE TO MEDIUM SAND (STIFF) GRADES TO VERY STIFF	
720								21.2	105	l9-1≧		SP SM	BROWN FINE SAND WITH TRACE OF SILT (MEDI DENSE)	
, 20										14 12		- UIVI	GRAY SILTY FINE SAND (MEDIUM DENSE) GRAY CLAYEY SILT WITH SOME FINE SAND (MEDIUM STIFF) BASE OF WEATHERED ZONE 16.0 FEET	
710	С	14.5	8.5				2400	12.8	116	7 №		ML	GRADES WITH TRACE OF FINE TO	
	1									7 🖻		ļ	COARSE SAND AND WITH MORE CLAY	
				1530	3000		2000	32.1	92	7 🗷		<u> </u>	Top of Interglacial Zone	
700	<u> </u>	 	 	 	 	<u> </u>			 	1		۱	GRAY SILTY CLAY WITH TRACE OF FINE SAND (MEDIUM STIFF)	
		<u> </u>							Ì	6 🗷		CL		
690	TX/DY			1980	3000		3200	18.0	111	1118		ML	GRAY CLAYEY SILT WITH SOME FINE TO COARS	
		1								12 🗷		ML	Top of Illinoian Glacial TIII GRAY FINE SANDY SILT WITH TRACE OF CLAY	
•							4500+	9.7	135	33 ■			(STIFF) GRAY CLAYEY SHIT WITH SAND AND SOME GRAY	
680						 	+	1		1			(VERY STIFF) GRADES TO HARD	
•				11, 000	1,200		4500		127	47 Z		ML		
670	<u> </u>	-		14,000	4000	<u> </u>	4500+	8,6	133	1,7010				
							4500+		1	42 🗷			GRADES WITH LESS FINE SAND	
660	ŞA							12.6	123	134/11" 🖬		1 SM		
550							4500+		-	77,0			GLACIAL OUTWASH) GRAY CLAYEY SILT AND FINE TO COARSE SAN WITH OCCASIONAL GRAVEL (HARD)(TILL)	
650	<u> </u>	<u> </u>		<u> </u>	1		ļ	11.9	103	100/e, #		1		
					1		4500 ⁺			100/5" 🗷				
										100/3" 🛭			1/2" LENSE OF SHALE HARD	
640			1	1	1		4500+	1	1	100/6, 5		ML	3" COBPLE GRADES DARKER GRAY	
								9.4	130	115/6"			CHAPES PARTER WANT	
630	 	1		+-	+	1	4500+	+	1.,0	100/5" [2]			2" SEAM OF GRAY FINE TO MEDIUM	
							-	٠, .					SAND	
620	<u>'</u> -	 	-	36,00	0 8,000		4500+	-	140	100/5" 1				
	l						4500+			124 🛭	· EIIIII		BORING COMPLETED AT 120.0 FEET	
610	<u>,L</u>			1		1		1	1	1			ON 5/24/72 CASING USED TO A DEPTH OF 5.0	

FIGURE 2.5-32

LOG OF BORING P-15

NOTE:

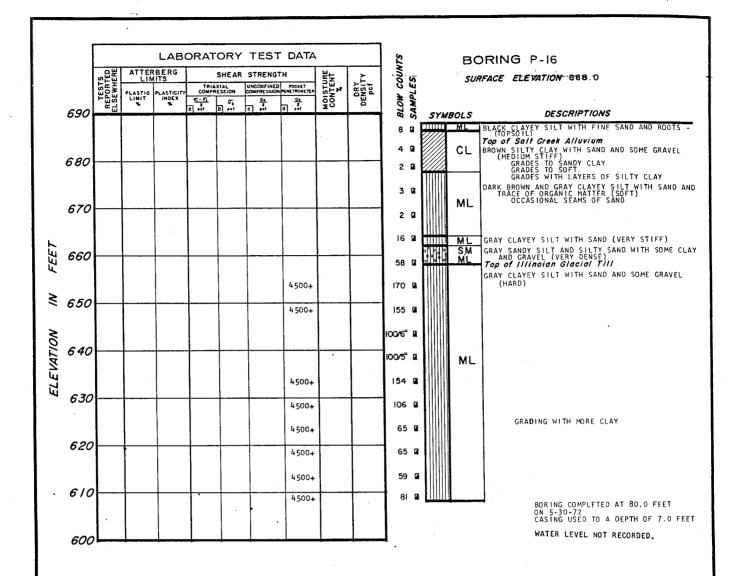


FIGURE 2.5-33

LOG OF BORING P-16

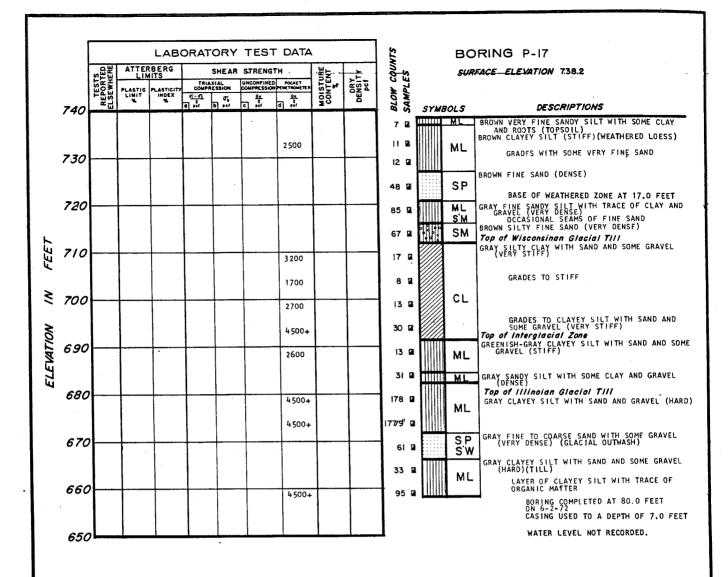


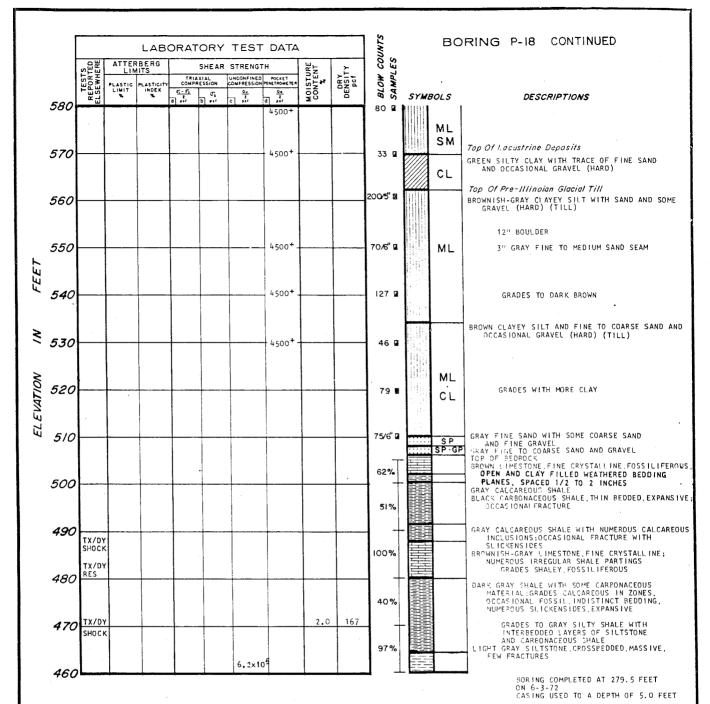
FIGURE 2.5-34

LOG OF BORING P-17

	- 101			ORAT	ORY	TEST	DATA		,	/N/3		ВС	DRING P-18
	STS RTED HERE	ATTER				STRENG	POCKET PENETROMETER	FURE	۶ <u>۴</u> ۔	COL		SUR	RFACE ELEVATION 738.2
	TESTS REPORTED ELSEWHERE	PLASTIC LIMIT	PLASTICITY INDEX	a-'a		COMPRESSION	PENETROBETER 94 d per	MOISTURE CONTENT	DENSITY Pcf	BLOW COUNTS SAMPLES			
740	<u> </u>			0) 144	01 941	C 987				1	SYA	IBOLS	DESCRIPTIONS
							2500	23.7	96	8 5		ML	LIGHT BROWN CLAYEY SILT WITH ORGANICS (TO BROWN CLAYEY SILT (STIFF) (WEATHERED LOESS
730							2700	L	114	- 6 E	439		Top of Wisconsinan Glacial Till Light Brown Silty Fine to medium sand (Li
							1	15.5	114			S M	
720				3360	1500			14.1	118	19 ቘ		M L S M	BROWN FINE SANDY SILT WITH TRACE OF CLAY (STIFF) BASE OF WEATHERED ZONE AT 19.0 FEET
120										25 🗷] GRAY SILT (STIFF) GRAY FINE TO MEDIUM SAND (DENSE)(GLACIAL
								11.8	115	42 ■		SP	OUTWASH) GRADES WITH OCCASIONAL FINE SAND
710							-			38 ₽			SEAM
	SHOCK		ļ		İ		3000			İ		M.L S.M	GRAY FINE SANDY SILT WITH TRACE OF CLAY (HARD)
700	SHOCK TX/DY						3000			15 ₺		ML	GRAY CLAYEY SILT WITH TRACE OF FINE TO CO
,,,,									İ	. 17 🖭		CL	Top of Interglacial Zone DARK GRAY CLAYEY SILT WITH TRACE OF ORGAN (VERY STIFF)
		16.5	13.5				2000	17.7	104	10 🗷		ML	GRADES TÓ STIFF LIGHT GRAY CLAYEY SILT WITH FINE TO COARS
690	 			-	-		 		 	9 2		ML	AND OCCASIONAL GRAVEL (STIFF)
	C SA						4500+	10.3	131	66 E		SM	Top of Illinoian Glacial TIII GRAY SANDY SILT WITH CLAY AND SOME GRAVEL
680	PERM				ļ			1.5.	',	100/4 "12			(HARD)
										1		ML	
							4500+	10.0	122	155/11" ■		SM	
670			 				4500*	 		44 🛭			
							4500 ⁺			56 ₺	Щ	1	GRAY CLAYEY SILT (HARD)
660			 	<u> </u>	 	ļ	4500+	 	 	29 2	Ш	ML	
• .						1			ŀ	175/6" 🗆			GRAY CLAYEY SILT WITH SOME FINE TO COARSI SAND AND OCCASIONAL GRAVEL (HARD)
650							1.			1			
650							4500+			100/6" 12			12" GRAVEL SEAM
				٠.						500/6, 8			
640	 		 	 		-	+	 	 	150/5" B			GRADES WITH MORE SAND
							4500+			119/6" 0		1	
630	 			<u> </u>						95/6" 2			6" BROWN FINE TO MEDIUM SAND SEAM
	1											l	
-							4500+			112/6" 12		ML	GRADES WITH MORE CLAY
620	T	 				 	4500≠	†	†	75 Q		SM	
		ľ											
610	·	<u> </u>		ļ		ļ		↓		75/6" 🛭			
					•								
~~~													
600				1			4500+		1	62/6"0			
													6" GRAVEL SEAM
590	<u>'</u>		-	+	-	1	4500+	-	-	65/6" 🛭	•		BORING CONTINUED
							.						
580	<u> </u>		1							1		11.	
550											CI	INT	ON POWER STATION

FIGURE 2.5-35

LOG OF BORING P-18 (SHEET 1 of 2)



WATER LEVEL NOT RECORDED.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-35

LOG OF BORING P-18

(SHEET 2 of 2)

NOTE:

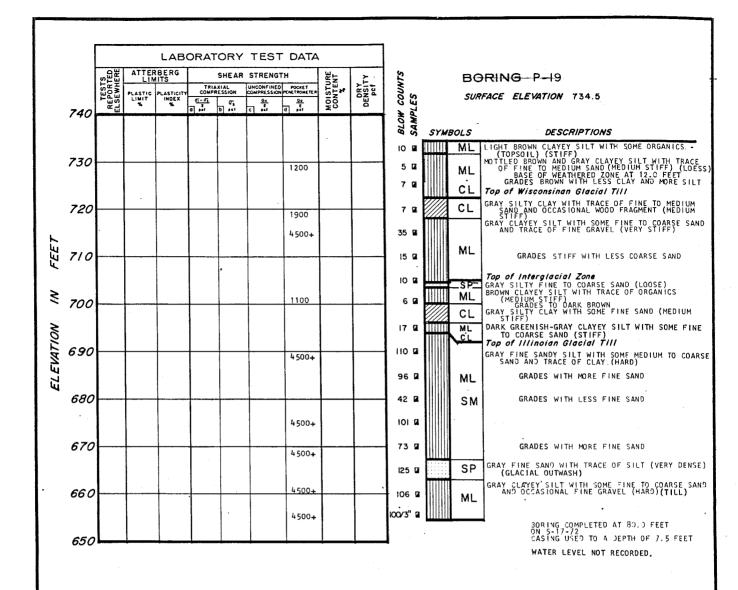


FIGURE 2.5-36

LOG OF BORING P-19

NOTE:

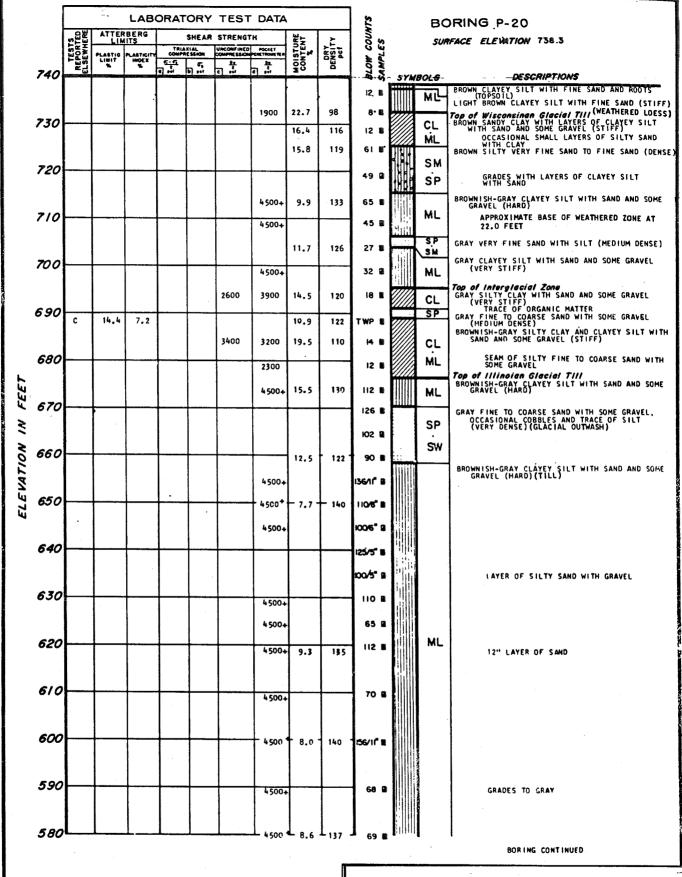
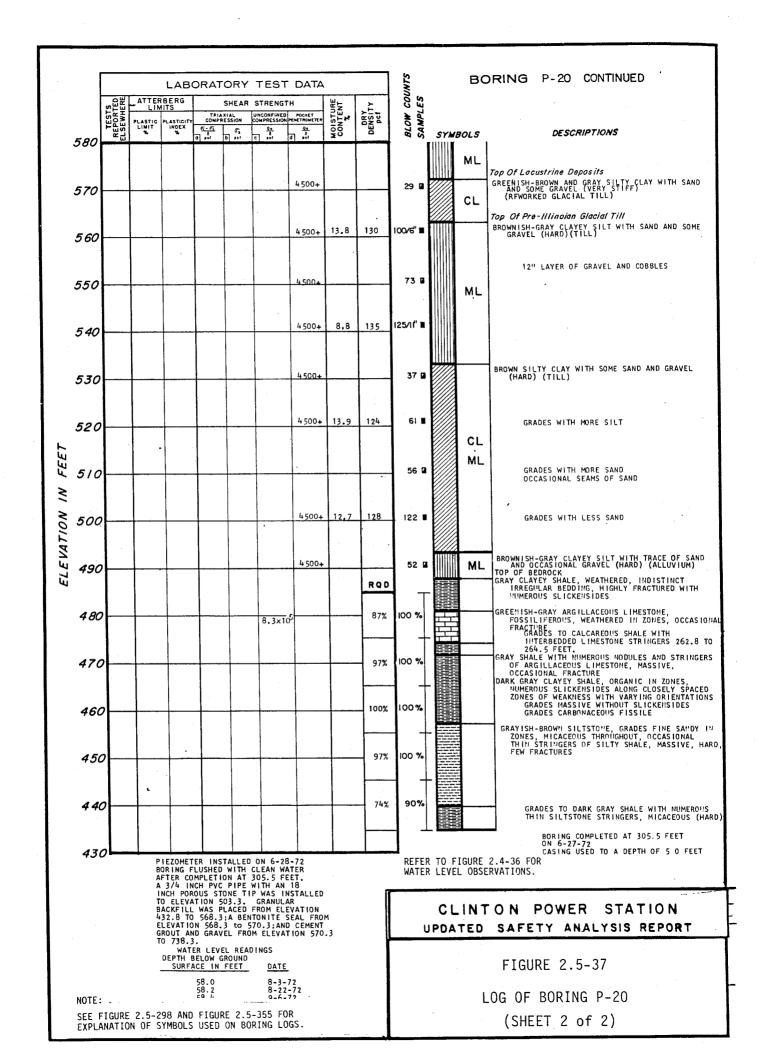


FIGURE 2.5-37
LOG OF BORING P-20
(SHEET 1 of 2)

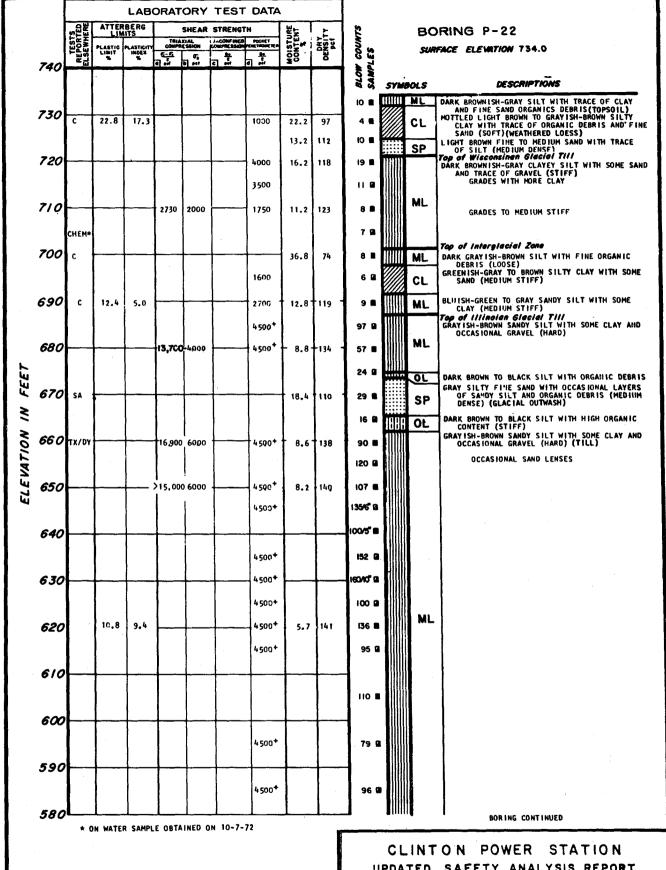


			LAB	ORAI	ORY	165	I DATA					
	TESTS REPORTED ELSEWHERE	ATTER LIN PLASTIC LIMIT	PLASTICITY	TRIA COMPR	XIAL RESSION		D POCKET N PENETROMETER	MOISTURE CONTENT	DENS!TY pcf	Š	ВС	PRING P-21
50	E.S.	LIMIT'		2 *** 2 - 2	D ***	Qu 0 901	Qu 2 2 2 2	<b>2</b> 0	8	BLOW COUNTS	SUR	FACE ELEVATION 740.2
										BLOW SAMPL	OI S	DESCRIPTIONS
40						<u> </u>	2300	24.7	94		ML	DARK BROWN CLAYEY SILT WITH ORGANICS AT
							1600	16 5	,,,		ML	SAND (STIFF) (WEATHERED LOESS)
				2080	1000		1600	16.5	113	5 L	ML	Top of Wisconsinan Glacial Till BROWN CLAYEY SILT WITH SOME SAND AND GRAVEL
30				2000	1000		1600	14.9	120		CL	
1								17.6	111	57 <b>u</b>	SM	BROWN SILTY FINE SAND (DENSE)  GRAY CLAYEY SILT WITH SOME FINE TO COARS
										58 🛮		AND OCCASIONAL GRAVEL (HARD)
20									<del>                                     </del>	.	ML	BASE OF WEATHERED ZONE 14.5 FEET
				6600	2000		4500+	10.6	128	31 🖪	141	
ا ، ، ۔										28 🛭		12" LAYER OF GRAY FINE TO COARSE S
10			†		<b> </b>	<del>                                     </del>				20 -	SP	AND GRAVEL
							4500+	10.1	130	27	GP	GRAY FINE TO COARSE SAND AND GRAVEL (MEDIUM DENSE)
		]								17 @	ML	GRAY CLAYEY SILT WITH TRACE OF FINE TO N SAND (VERY STIFF)
200				<b> </b>	1	<del>                                     </del>			<del> </del>	'' <b>"  </b>	CL	Top of Interglacial Zone DARK GRAY CLAYEY SILF WITH TRACE OF FINE
	С	16.6	4.4				2000	18.4	114	8 🛮	ML	(MEDIUM STIFF TO STIFF)
		ł							1			GRAY FINE SANDY SILT WITH TRACE OF MEDIL
590	$\vdash \vdash$				-	-			<del> </del>	13 🛭	ML	COARSE SAND AND OCCASIONAL GRAVEL (VERY STIFF)
	· !						4500+			90/6" 8		Top of Illinoian Glacial Till
- 1				1			4500+			100/5" 2		GRADES TO HARD WITH TRACE OF CLAY
580		<b> •</b>	-	<u> </u>	-	<del> </del>	4 500		-	100/3 2	ML	
1							4500	8.7	134	78 🛚	ŚМ	
				-							e n	BROWN SILTY FINE TO MEDIUM SAND (MEDIUM
570	<u> </u>		-	<del> </del>	<del> </del>	<del> </del>	-		-	46 🛚	SP ML	GRAY CLAYEY SILT WITH SOME FINE TO COARS
									1	35 ■		AND OCCASIONAL GRAVEL (VERY STIFF) GRAY AND BLACK CLAYEY SILT WITH FIME SAN
										111111	S.P SM	TRACE OF ORGANICS (DENSE)  GRAY SILTY FINE TO COARSE SAND (DENSE)
660	<b></b> -	<del> </del>		<del> </del>	<del>                                     </del>		+			42 🖟	SМ	
				12,600	6000		4500+	7.9	140	100/5" ■		GRAY CLAYEY SILT WITH SOME FINE TO COARSE AND OCCASIONAL GRAVEL (HARD)
							1			200/6" [2		
<i>550</i>	<del> </del>		-		+	<del> </del>	+			2000		
	l						4500+	6.8	146	100/5"		
							4500+			135/6" Q	ML	GRADES WITH MORE CLAY
540	<del> </del>	-	1	+	<del> </del>		4500		<del>  -</del>	130/6 14		
		10.6	3.9				4500 ⁺	6.5	138	100/4" 🗷		
	l						4500+			170 AM		
530	<b> </b>		-	-		-	4500	ļ	<del> </del>	130/6" ☑		
	l			20,400	8000		4500+	8.6	136	100/4" 🛚		GRADES WITH TRACE OF FINE SAND
I	ĺ						4500+			07.0		
620				<u></u>	<u> </u>		4500		<u> </u>	97		BORING COMPLETED AT 120.0 FEET
												ON 5-19-72 CASIMG USED TO A DEPTH OF 7.5 F

FIGURE 2.5-38

LOG OF BORING P-21

NOTE:

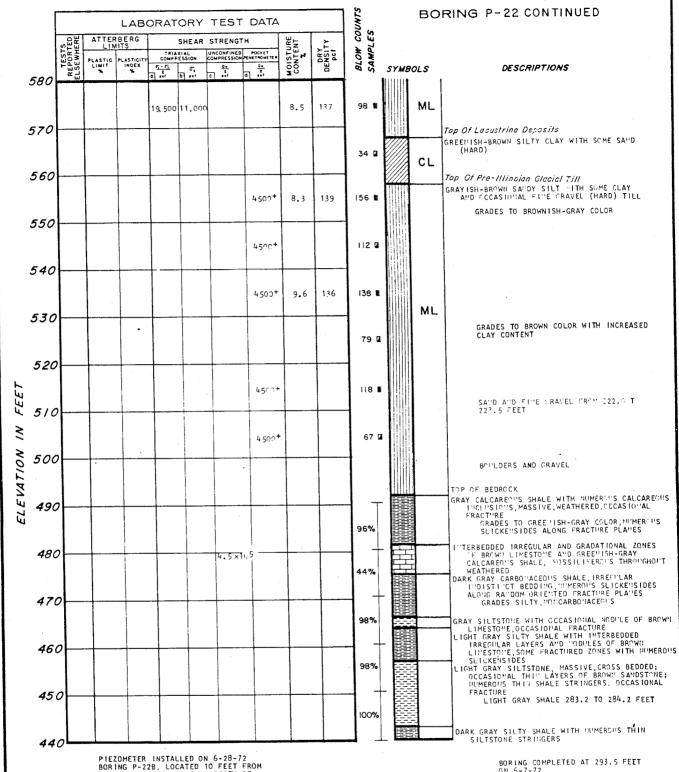


UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-39

LOG OF BORING P-22

(SHEET 1 of 2)



PIEZOMETER INSTALLED ON 6-28-72
BORING P-22B, LOCATED 10 FEET FROM
P-22A, WAS DRILLED TO A DEPTH OF
64 FEET, A 3/4 INCH PVC PIPE WITH
AN 18 INCH POROUS STONE TIP WAS
PLACED TO ELEVATION 670.0. GRANULAR
BACKFILL WAS PLACED FROM ELEVATION 670.0
TO 679.0.2 A BENTONITE SEAL FROM 679.0 TO
680.5; AND CEMENT GROUT AND GRAVEL FROM
ELEVATION 680.5 TO 734.0.

WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET DATE 8-15-72 18.2 18.4

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS:

REFER TO FIGURE 2.4-36 FOR WATER LEVEL OBSERVATIONS.

BORING COMPLETED AT 293.5 FEET ON 6-7-72 CASING USED TO A DEPTH OF 5.0 FEET

#### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-39

LOG OF BORING P-22 (SHEET 2 of 2)

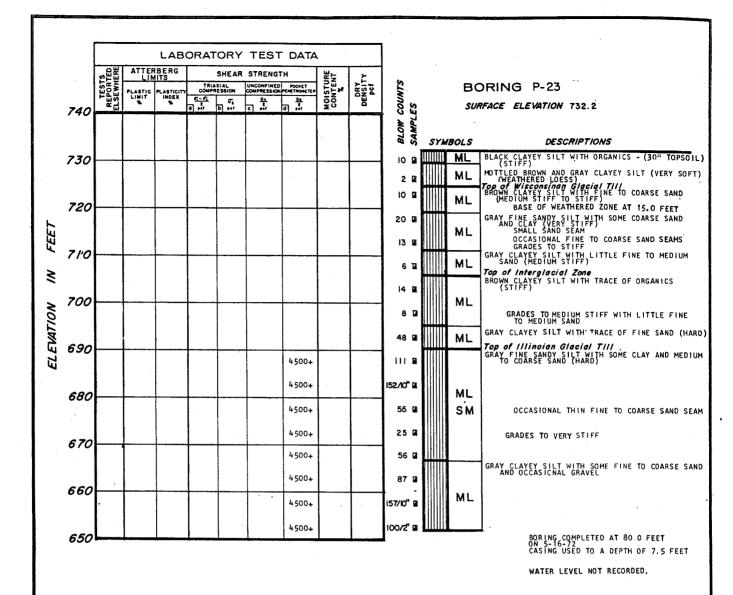


FIGURE 2.5-40

LOG OF BORING P-23

NOTE:

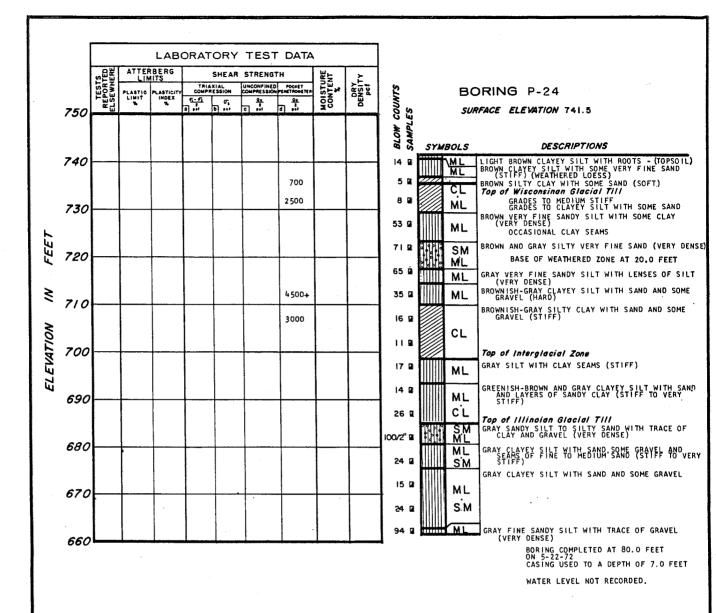


FIGURE 2.5-41

LOG OF BORING P-24

NOTE:

	,				· .						1
	.!		1		ORAT			T DATA		·	<u> </u>
	,	TESTS REPORTED ELSEWHERE	ATTER	RBERG MITS	TR		R STRENG		MOISTURE	\ <u></u>	
	7	TES EPO	PLASTIC LIMIT	PLASTICITY	$\sigma_i \cdot \sigma_i$	PRESSION	COMPRESSIO	ED POCKET ON PENETPOMETER	PNO:	DRY DENSITY pcf	BORING P-25
	750	<u> </u>	<del>_</del>	<del>  -</del>	<u> </u>	ь	e 55	न क	20	+	SURFACE ELEVATION 740.9
	ľ	1								1 '	SURFACE ELEVATION 740.9  SURFACE ELEVATION 740.9  SOURT OF THE SURFACE ELEVATION 740.9  DESCRIPTIONS
	,	'									S SYMBOLS DESCRIPTIONS
	740	<b> </b>	<u> </u>	+	+	+	+	2500	$\vdash$	+	9 2 ML LIGHT BROWN CLAYEY SILT WITH ORGANICS - (8" TOPSOIL)
	1									ļ. '	BROWN CLAYFY SILT (STIFF) (WEATHERED LOESS)
	770							2500			CL GRADES WITH TRACE OF FINE TO MEDIUM SANG
	730	ì				1				1	Top of Wisconsinan Glacial Till
L	!							2500			CL GRAY SILTY CLAY WITH SAND AND SOME GRAVEL
FEET	720	L						4500+			GRAY CLAYEY SILT WITH SAND AND SOME GRAVEL (VERY STIFF)
F	, 20							4500+			28 B
_								7,000			GRAY CLAYEY SILT WITH TRACE OF FINE TO MEDIUM
>	710	/ <b>-</b>	<del> </del>		+	<del> </del>		2600	┿	4-	SAND AND TRACE OF ORGANICS (STIFF)
							1	1800			8 2 CL
EL EVATION											GRAY SILTY CLAY WITH SAND AND SOME GRAVEL
471	700	/├─	₩	+	+	+-	+	4500+	+	+-	CL (VERY STIFF) Top of Interglacial Zone
EV											8 0 ML GRAY CLAYEY SILT WITH SAND AND SOME GRAVEL (STIFF)
73											DARK GRAY SILTY FINE SAND WITH SOME CLAY (LOOSE)  Top of ///ininian Glacial Till
-	690	<b>/</b>	+	+	+	+-	+	+	+		LIGHT GRAY SILTY FINE SAND WITH TRACE OF
		1									50 2 SM COARSE SAND AND CLAY (DENSE)
	201							4500+			SAND AND OCCASIONAL GRAVEL (HARD)
	680	1	†	+	1					1	GRADES WITH MORE FINE SAND
				1.							SP GRAY FINE TO MEDIUM SAND WITH TRACE OF SILT
	-670	<u>,                                    </u>							$\perp$		14 4 GRAY CLAYEY SILT WITH TRACE OF FINE TO MEDIUM
	·67, U	Τ	T	T	T	T	T	500	T	T ·	ML GRAY FINE SANDY SILT WITH TRACE OF MEDIUM TO COARSE SAND AND CLAY (HARD) (TILL)
ı				1				4500+			GRAY FINE SAND WITH TRACE OF SILT (VERY DENS
	660	, <b>L</b>			$\perp$	$\perp$					145 D SP GRAY FINE SAND WITH TRACE OF STEE (VERT DENS
i	•	-									ON 5-25-72 CASING USED TO A DEPTH OF 7.0 FEET
ĺ											WATER LEVEL NOT RECORDED.
											* · · · · · · · · · · · · · · · · · · ·

FIGURE 2.5-42-

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

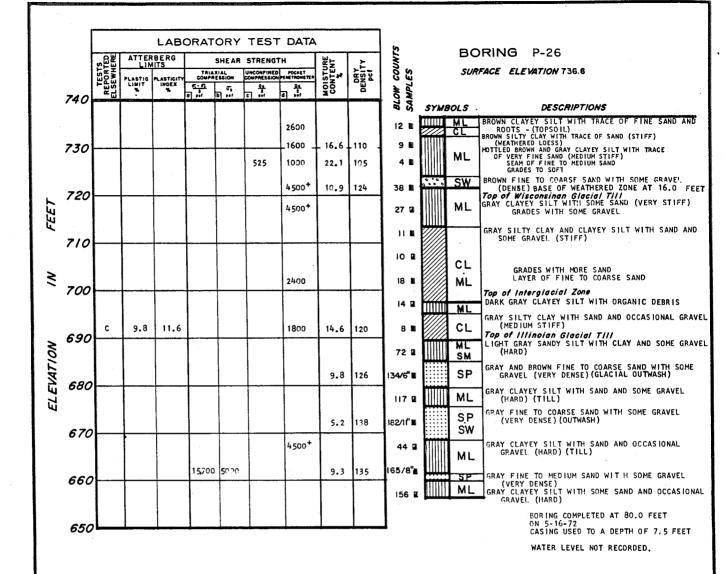


FIGURE 2.5-43

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

		STS		IITS	TRIA	XIAL	UNCONFINED	POGKET	E.W.	<u>د د ت</u>	77.		во	RING P-27
	750	TESTS REPORTE ELSEWHEI	PLASTIC LIMIT	PLASTICITY INDEX %	a-ta			Ou 2 3 5	MOISTUR CONTEN	DENSITY PCf	COUNTS		SURF	FACE ELEVATION 742.9
	, 00							•			BLOW CO.	SYMBO	LS	DESCRIPTIONS
	740										13 🖩	7777	ML.	BROWN CLAYEY SILT WITH TRACE OF SAND AND ROOTS - (TOPSOIL)
	, ,,								15.6	115	5 <b>%</b>		cL	Top of Wisconsinan Glacial TIII BROWN SILTY CLAY WITH SOME SAND (MEDIUM STIFF)
					1910	1000			13.9	120	7 ₺			GRADES TO GREENISH-FROWN GRADES WITH OCCASIONAL GRAVEL BASE OF WEATHERED ZONE AT 12 FEET GRAY SILTY CLAY WITH SAND AND OCCASIONAL
7	730	С	10.4	6.8	<del> </del>	<b>—</b>	<del>                                     </del>		11.2	126	8 1		CL	GRAY SILTY CLAY WITH SAND AND OCCASIONAL GRAVEL (MEDIUM STIFF)
FEET								į			41 2		SP	BROWN FINE TO MEDIUM SAND WITH OCCASIONAL GRAVEL (DENSE)
4	720		ļ	ļ	-	-			17.3	118	145 N		S.M	LAYER OF CLAYEY SILT GRAY SILTY SAND TO SANDY SILT WITH TRACE OF GRAYEL (HARD)
			•					-	'''	''			ML ML	TRACE OF ORGANIC MATTER
≥	710										127 🛭		ML	"  " LAYER OF GRAY FINE TO COARSE SAND  TOP OF Interglacial Zone  GRAY SILTY CLAY WITH SOME SAND AND OCCASIONAL
•	,,,						2200		12.1	124	12 🗷		CL	GRAVEL (STIFF)
		С	12.4	11.5					17.5	109	P■		-	TRACE OF ORGANIC MATTER
8	700	$\vdash$		ļ	$\dagger$	<del>                                     </del>	1			<u> </u>	15 🕱		ML	GRAY CLAYEY SILT WITH SOME VERY FINE SAND (STIFF)
ELEVATION							1475		15.9	114	P		CL	GRAY SILTY CLAY WITH SAND AND OCCASIONAL GRAVEL (STIFF)
£17	690	SA	ļ	-	ļ		ļ	-		101	ļ ,		ML	GRADES WITH MORE SAND  Top of ///inoian Glacial Till GRAY VERY FINE SANDY SILT TO SILTY SAND WITH
73		34							11.3	124				TRACE OF CLAY AND OCCASIONAL GRAVEL
	680										105 🗷			(HARD)
	600								7.6	139	149		ΜĻ	GRADES WITH MORE CLAY
					1						22 🖬		SM	GRADES TO VERY STIFF
	670	<u>'</u>	1	-		1	<del>                                     </del>		9.8	130	17 1			GRADES WITH LESS CLAY TRACE OF ORGANIC MATTER
											35			GRADES TO FINE SANDY SILT
	660	<u></u>			1					1	]	<del></del>		PORING COMPLETED AT 80.0 FEET
					PIEZOM TIP EL	ETER II EVAT 101	NSTALLED N 685.4	ON 6-6-	72					ON 6-6-72 CASING USED TO A DEPTH OF 7.0 FEET

WATER LEVEL READINGS

LABORATORY TEST DATA ATTERBERG SHEAR STRENGTH

DEPTH BELOW GROUND SURFACE IN FEET

DATE

REFER TO FIGURE 2.4-36 FOR WATER LEVEL OBSERVATIONS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-44

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

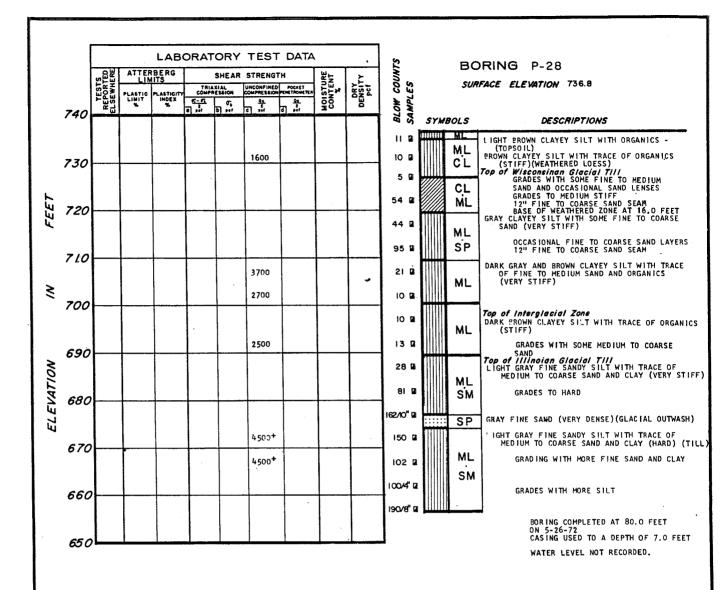
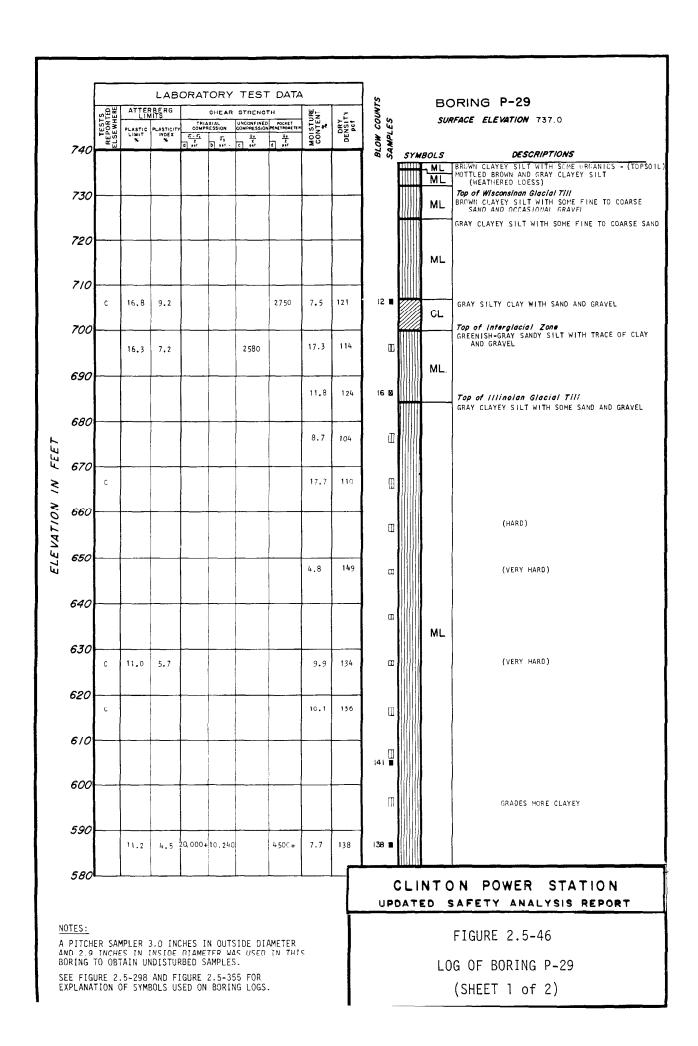
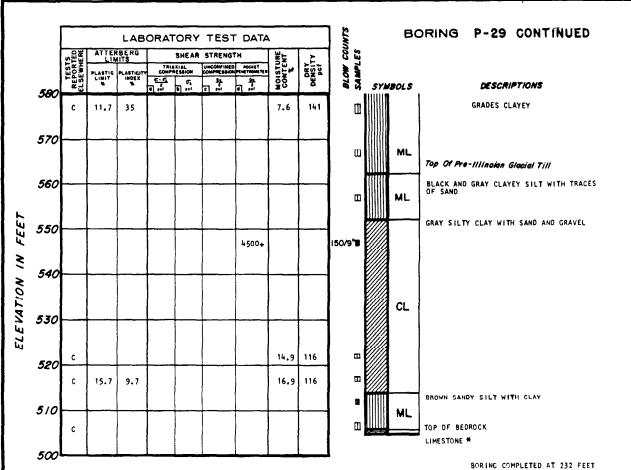


FIGURE 2.5-45

LOG OF BORING P-28

NOTE:





BORING COMPLETED AT 232 FEET NA 4-23-73 CASING USED TO A DEPTH OF 5 FEET GROWND WATER NOT MEASURED

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-46
LOG OF BORING P-29
(SHEET 2 of 2)

NOTE: USED TRI-COME ROLLER BIT -NO CORE SAMPLE OBTAINED.

16.3   15.3   15.3   17.0   24.6   99   5.8   ML   17.0   16.3   15.3   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0   17.0	ſ			LAB	ORAT	ORY	TEST	DATA			ی		B	ORING P-30
16.3   15.3   15.3   1700   24.6   99   99   16.3   15.3   15.3   15.3   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   24.6   99   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1700   1	Ī	ESTS ORTED EWHERE	PLASTIC	PLASTICITY				1	STURE ITENT	ASITY Per	COUN			·
16.3   15.3   1700   24.6   99   99   18   18   18   18   18   18	740	REP ELSE	LIMIT	# WOEX					ŞĞ Ş	OE	BLOW	5Y	WROLS	DESCRIPTIONS
			16.3	15.3		·		1700	24.6	99	8 ৳	11.00		DARK BROWN CLAYEY SILT WITH SOME ORGANICS (TOPSOIL) MOTTLED YELLOWISH-BROWN AND GRAY CLAYEY SIL
10.7   9.4   4500   12.5   127   19	730							4500+	10.1	134			ML	(WEATHERED LOESS) (LOOSE) GRADES WITH GRAVEL
10.7   9.4   4500   12.5   127   19	720													GRADES WITH OCCASIONAL THIN SAND
	710		10.7	9.4				4500	12.5	127	19 ቘ		ML	
700  13.8 9.7   900 2250 15.7 118 17   ML GARDES MITH GAN'S LILY FINE TO CARD  15.1 116 17   ML GARDES MITH GAN'S LILY FINE TO CARD  17.1 1.4   1.7   4500+ 11.8 126   24   ML GARDES MITH GAN'S LILY FINE TO GARDES MITH GAN'S LILY FINE TO GARDES MITH GAN'S LILY FINE TO GARDES MITH GAN'S LILY FINE TO GARDES MITH GAN'S LILY FINE TO GARDES MITH GAN'S LILY FINE TO GARDES MITH GAN'S LILY FINE TO GARDES MITH GAN'S LILY FINE TO GARDES MITH GAN'S LILY FINE TO GARD MITH TARGE OF FINE S GARD MID GCCAS GORAL FINE CARD  11.4 1.7   4500+   1500+   58   ML ML GARDES MITH GAN'S LILY FINE TO MEDIUM SAID WITH SOME FINE CARD GAN'S LINY FINE TO MEDIUM SAID WITH SOME FINE CARD GAN'S LILY FINE MITH SOME FINE CARD GAN'S LILY FINE MITH SOME FINE CARD GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FINE GAN'S LILY FIN	i							4500+	15.3	118				GRAY FINE TO COARSE SAND WITH TRACE OF SILT (MEDIUM DENSE) GRAY CLAYEY SILT WITH TRACE OF GRAVEL AND LAYERS OF GRAY SILTY FINE SAND (VERY DEN
13.8   9.7   16.1   116   17	700						900							Top of Interglacial Zone GRAY CLAYEY SILT WITH SOME FINE TO COARSE S
11	690		13.8	9.7					16.1	116	17 E	- 172 P		GRADES WITH GRAY SILTY FINE TO MEDIU
660  11.4 1.7 4500+ 9.9 136 27 M  ML  1720 4500+ 10.9 130 59 M  4500+ 560					962	3960		4500+	1128	126				GRAY FINE TO MEDIUM SAND WITH SOME SILT (LOOSE) Top of Illinoian Glacial Till
670  11.4 1.7	680										64 🗷			COARSE SAND AND OCCASIONAL FINE GRAVEL GRADES WITH OCCASIONAL POCKETS OF SANDY SILT WITH ORGANICS
660  4446 5400 2500 30.3 88 26	670		11.4	1.7					9.9	136	27 <b>≡</b>		ML	SILT POCKETS GRADE CUT
					4446	5400	1720							
650  8.0 8.5  8.0 8.5  7.5 146 100/8" 8  150/2" 0  4500+  150/4" 8  150/4" 8  150/4" 8  660  9.3 4.1  4500+ 6.8 150  150/8" 8  150/8" 8  4500+ 9.8 134  150/8" 8  660  10.4 4.5  8730 8640  4500+ 8.0 136  10.3/9" 8  10.4 4.5  00.8 8730 8640  4500+ 8.1 140  160 8  00.8 8730 8640  4500+ 8.1 140  160 8  00.8 8730 8640  4500+ 8.1 140  160 8  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 8640  00.8 8730 86	660					,,,,,			30.5		† •		SP	GRANICS (VERY STIFF) GRAY FINE SAND
150/2"	650				ļ				,,	,,,	1			AND OCCASIONAL GRAVEL (HARD) GRAY FINE TO MEDIUM SAND WITH TRACE OF SIL' (VERY DENSE) GRAY CLAYEY SILT WITH FINE TO COARSE SAMD
630  9.3 4.1 4500+ 6.8 150 150/8*   8584 4500+ 8.1 136 200   4500+ 9.8 134 157   670  10.4 4.5 8730 8640 4500+ 8.0 136 103/9*   4500+ 7.5 137 230   000  000 000 000 000 000 000 000 00	640		8.0	0.5					/.5	146				GRADES WITH STRINGERS GRAY CLAYEY S GRADES WITH OCCASIONAL COBBLES AND
630  9.3 4.1 4500+ 6.8 150 150/8*   8584 4500+ 8.1 136 200   4500+ 9.8 134 157   670  8730 8640 4500+ 8.0 136 103/9*   10.4 4.5 4500+ 7.5 137 230   4500+ 7.5 137 200   000 000 000 000 000 000 000 000 0	<del>04</del> 0							4500+			150/4" B			
620 9.3 4.1 4500+ 6.8 150 150/8" E 8584 4500+ 8.1 136 200 E 4500+ 9.8 134 157 E 10.4 4.5 4500+ 7.5 137 230 E 10.4 4.5 4500+ 8.0 1460 E 10.4 4.5 14500+ 8.1 140 160 E	630	-						4500+			1			
610  8730 8640 4500+ 8.1 136 200 E  10.4 4.5 4500+ 7.5 137 230 E  4500+ 8.1 140 160 E  98730 8640 4500+ 8.1 140 160 E	620		9.3	4.1			-		6.8	150			ML	
600  8730 8640 4500+ 8.0 136 103/9" E  10.4 4.5 4500+ 7.5 137 230 E  0RGANICS GRADE OUT  4500+ 8.1 140 160 E	020						8584	4500+	8.1	136	200 ■			
600   10.4 4.5   4500+ 7.5 137   230 M   ORGANICS GRADE OUT	610	-	-		8730	8640		<del> </del>		<del> </del>	-			GRADES WITH TRACE OF ORGANICS
590 4500+ 8.1 140 160 M	600		10.4	4.5						į				ORGANICS GRADE OUT
590	500							4500+			200 ■			SHOWING GOV
	590	-	-	-	_	<u> </u>		+	<del> </del>	140	1			
9.8 5.7 4500+ 7.9 139 160 <b>■</b>	EGA		9.8	5.7						139	1			

FIGURE 2.5-47

LOG OF BORING P-30 (SHEET 1 of 2)

NOTE:

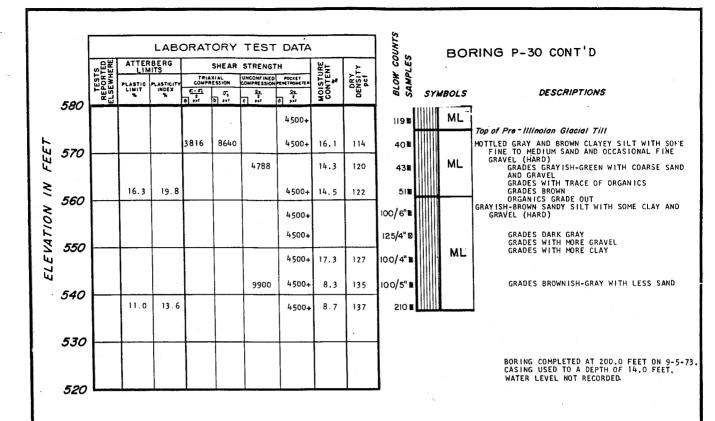


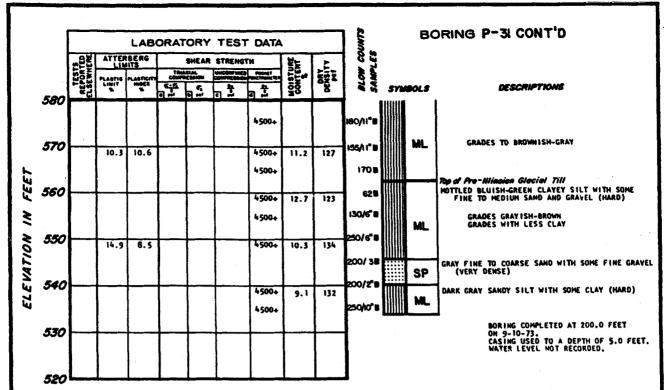
FIGURE 2.5-47
LOG OF BORING P-30
(SHEET 2 of 2)

			LAB	ORAT	ORY	TES"	T DATA			W
	TS TED TERE	ATTERI LIM	BERG ITS			STRENG		NTE	ř	BORING P-31
	TESTS REPORTED ELSEWHERE	PLASTIC I		TRIAL COMPRI		COMPRESSIO	D POCKET N PENETROMETER Qu	MOISTURE CONTENT	DRY DENSITY Pef	BORING P-31  SURFACE ELEVATION 736.8  SURFACE ELEVATION 736.8  DESCRIPTIONS
740	3			8  311	<b>5</b>	C 911	d  pu			TO SYMBOLS DESCRIPTIONS
		-								ML LIGHT BROWN CLAYEY SILT WITH SOME ORGANICS
730							3000			5 ML BROWN CLAYEY SILT WITH TRACE OF FINE SAND A
										20 M THE SM (WEATHERED LOESS) YELLOWISH-BROWN SILTY FINE SAND WITH OCCASION FINE GRAVEL
720										GRADES WITH TRACE OF CLAY  49
120										SP SILT AND GRAY CLAYEY SILT LAYERS (DENSE) GRADES WITH LAYERS OF GRAY SANDY SILT WITH CLAY
							2000			GRADES WITH TRACE OF FINE GRAVEL AND MORE SILT
710	<b>-</b>	11.8	11.3				3000	15.1	120	12 Top of Wisconsinan Glacidi Till DARK GRAY CLAYEY SILT WITH SOME FINE TO MED SAND AND OCCASIONAL FINE GRAVEL (VERY ST
,						2340	3000	13.4	122	13 1
700	ļ						3000	14.2	121	12 12 ML
							4500+			22 <b>(</b> HARD)
		15.5	11.0				2000	21.2	106	Top of Interglacial Zone
690									<b> </b>	CL DARK GREEN SILTY CLAY WITH SOME FINE TO MED
						:				36 SP GRAY FINE TO COARSE SAND WITH TRACE OF SILT OCCASIONAL FINE GRAVEL (DENSE) Top of Jilinolan Glaziai TIII
680	<u> </u>			<b> </b>	-		4500+	8.0	138	DARK GRAY SANDY SILT WITH SOME CLAY AND TRA
						ļ	4500+			48
670		15.2	4.5							LAYER OF GRAY SILT WITH TRACE OF CLAY
										GRADES WITH COARSE SAND AND OCCASIONAL
				9036	5000		4500+	9.8	134	COARSE GRAVEL GRADES WITH GRAY SAND LAYERS
660 650				<b></b>		1	+		<u> </u>	
						'			ŀ	LAYER OF GRAY MEDIUM TO COARSE SAND W TRACE OF SILT AND FINE SAND
650		9.6	6.9	ļ	_		4500+	7.1		_ 150/6" Ø
							4500+			I5O/6" 8
640										200/6" 2
040			:							THE COLORS WITH PROPERTY OF SOME THE
							4.500		•	GRADES WITH POCKETS OF GRAY FINE TO MEDIUM SAND
630		NON-P	LASTIC			ļ	4500+		<u> </u>	GRADES WITH MORE CLAY
							4500+			18O/a. ■
620	<u> </u>		<u> </u>	<u> </u>	<u> </u>	<del> </del>	1,500	ļ	<u> </u>	100/6" <b>1</b>
							4500+			140
e in							4500+			
610		10.5	7.3				4500+	8.2	137	ISO/6" FILE SP GRAY FINE SAND WITH TRACE OF SILT AND FINE
									1	GRAVEL DARK GRAY SANDY SILT WITH SOME CLAY AND FIN
600		<del> </del>		+	-		4500+	7.2	140	IO3/6"   GRAVEL (HARD)
							4500+			106 /6" <b>2</b>
590	<u> </u>	10,6	5.9	ļ	<u> </u>		4500+	7.4	141	195/i0" <b>■                                    </b>
				18500	> 8640		45004	ł	139	IBO/IO"
FOO				.000	0040		4500+		139	GRADES WITH MORE FINE SAND AND CLAY
580	*	· <del></del>	**				- 1			

FIGURE 2.5-48

LOG OF BORING P-31 (SHEET 1 of 2)

NOTE:



PIEZOMETER INSTALLED ON 9-11-73. BORING WAS FILLED WITM GRAVEL AND SEALED WITM BENTONITE TO 159.0 FEET AFTER FLUSHING WITM CLEAN WATER. A 2 INCH PVC PIPE WITM THE LOWER END PLUGGED AND THE LOWER 109.0 FEET PERFORATED WAS INSTALLED TO ELEVATION 577.8. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 577.8 TO 686.8. A BENTONITE SEAL FROM ELEVATION 686.8 TO 688.8; AND CEMENT GROUT FROM ELEVATION 688.8 TO 734.8.

### WATER LEVEL READINGS

SURFACE IN FEET	DATE
25.3	10-29-73
25.3	11-15-73
24.5	12-31-73

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-48

LOG OF BORING P-31

(SHEET 2 of 2)

l			LAB	DRAT	ORY	TEST	T DATA		ļ	5		В	ORING P-32
ĺ	STS RTED VHERE	ATTER LIM		TRIA:		STRENG'	D POCKET	TURE	<u>}</u>	SOUN			URFACE ELEVATION 737.4
740	TESTS REPORTED ELSEWHERE	PLASTIC I	PLASTICITY INDEX %	2-0		COMPRESSION G por	OH DOT	MOISTURE CONTENT	DENSITY pcf	BLOW COUNTS SAMPLES			
740										17	57A	1BOLS	
			. !			ĺ	1000					ML ML	BLACKISH-BROWN CLAYEY SILT WITH ORGANICS (TOPSOIL) BROWN CLAYEY SILT WITH TRACE OF FINE SAND
730	-			<del></del>	<del>                                     </del>	<del> </del>	2500			14 🖪	///// <b>/</b>	14.2	(STIFF) (WEATHERED LOESS)  Top of Wisconsinan Glacial Till
			, ,				2,000	13.9	123	//	//////		MOTTLED LIGHT AND DARK BROWN SILTY CLAY WI' SOME FINE TO COARSE SAND AND GRAVEL (MEDIUM STIFF)
720	<u> </u>				<u> </u> !	<u> </u>	4500+	<b>  </b>		20 %	//////	CL.	GRADES TO LIGHT BROWN
ļ			. !			,	2500	15.5	115	9 №	/////		GRADES DARK GRAY AND WITH MORE SAND
710									,!	90	//////		COBBLE LAYER AT 24.0'
//0							2250	15.9	119	9∎0		SM	GRAYISH-BROWN SILTY FINE SAND WITH TRACE OF CLAY AND POCKETS OF GRAY SANDY SILT (LOI
ļ							1500		, ,	7 <b>₽</b>	//////	Oiv.	Top of Interglacial Zone
700			<del></del>	$\vdash$	<del>                                     </del>				[	12			GRAY SILTY CLAY WITH SOME FINE TO MEDIUM S AND GRAVEL AND ORGANICS
	<b>i</b> !		i i				1500					CL	GRADES WITH OCCASIONAL COBBLE ORGANICS GRADE OUT
690	<b> </b>		<u> </u>	<del> </del>		<del> </del>	2500		<b> </b>	7 🛮			
				10437	3888			8.8	1 <b>3</b> 7	20 ■		SW	BROWNISH-GRAY FINE TO COARSE SAND WITH TRA OF SILT Top of Illinoian Glacial Till
680			<u> </u>				4500+		<u> </u>	50 N		ղCL ML	GREEN SILTY CLAY WITH TRACE OF FINE AND CO
							T	12.8 14.5	123 120	10≝		SP	GRAY SANDY SILT WITH SOME CLAY AND GRAVEL (HARD) BROWN AND GRAY MEDIUM TO COARSE SAND WITH
- 70	SA						4500+	1	. !	59 ■			TRACE OF FINE SAND AND SILT (LOOSE) LIGHT GRAY SANDY SILT WITH TRACE OF CLAY A
670							4500+			1 _			FINE GRAVEL (HARD) GRADES WITH OCCASIONAL SAND STRINGER 6" COBBLE AT 69.5"
	С	11.3	10.8					8.4	.136	115.00			STATES WITH TRACE OF ADCANGE INCHIS
660		<u></u>	<del> </del>	<del> </del>	-	-	4500+	<del>                                     </del>	1,35	115 =			GRADES WITH TRACE OF ORGANIC INCLUSI GRADES WITH MORE SAND AND GRAVEL
	TX/DY						4500+	7.9	135				
650 650	<u> </u>	<u> </u>	<u> </u>	<u> </u>			4500+	ļ	<u> </u>	140/6"■			COLORS LUTTI COCACIONAL CODDICE
ļ	С	11.3	6.6	7697	6480		4500+	6.7 6.4					GRADES WITH OCCASIONAL COBBLES
640		9.9	4.7				4500+	7.0	126	150/4"■			GRADES WITH LESS GRAVEL
•							4500+			80			COBBLE AT 101.01
230							4500+	7.3	135	163.2		ML	
630	С	10.9	8.9		<b> </b>	<b>†</b>	4500+	$\dagger$	140				
							4500+	1	139	167/11"			
620		<u> </u>	<del> </del>	+-	+	-	<del></del>	1		1			
	TX/DY						4500+		133				
610	<u> </u>		-	+-	+	+	4500+	7.7	139	131 /9" ■			GRADES WITH MORE CLAY
													3" LAYER OF SILTY FINE SANDY AT 130.
600		<u> </u>	ļ	<u> </u>	<u> </u>			<u> </u>	<u></u>	]			
				18720-	+80008+	+	4500+	25.2	120	154/II" <b>E</b>			GRADES WITH LESS CLAY
590													
330	С	10.6	7.8			1		8.5	136				
							4500+		سر ا				
580	L	<u> </u>	1	Т	1	1		<u> </u>	1		3 L I	NT	ON POWER STATION
			-										SAFETY ANALYSIS REPORT

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

FIGURE 2.5-49

LOG OF BORING P-32 (SHEET 1 of 2)

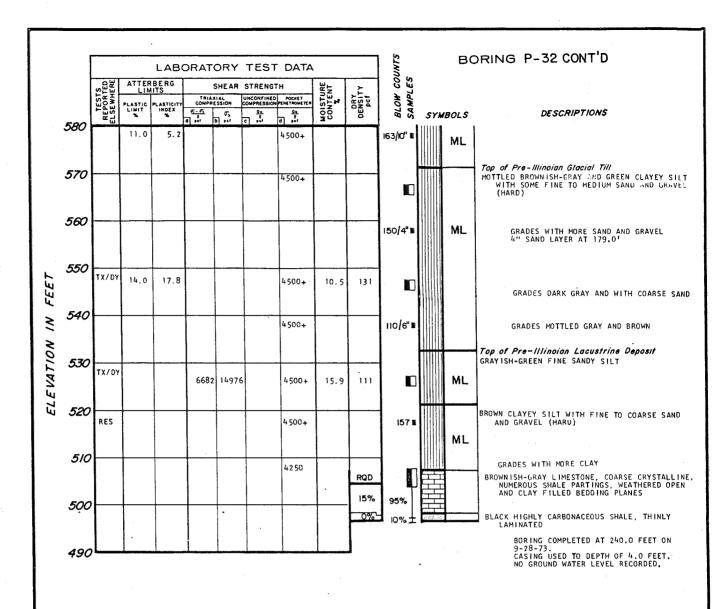


FIGURE 2.5-49
LOG OF BORING P-32
(SHEET 2 of 2)

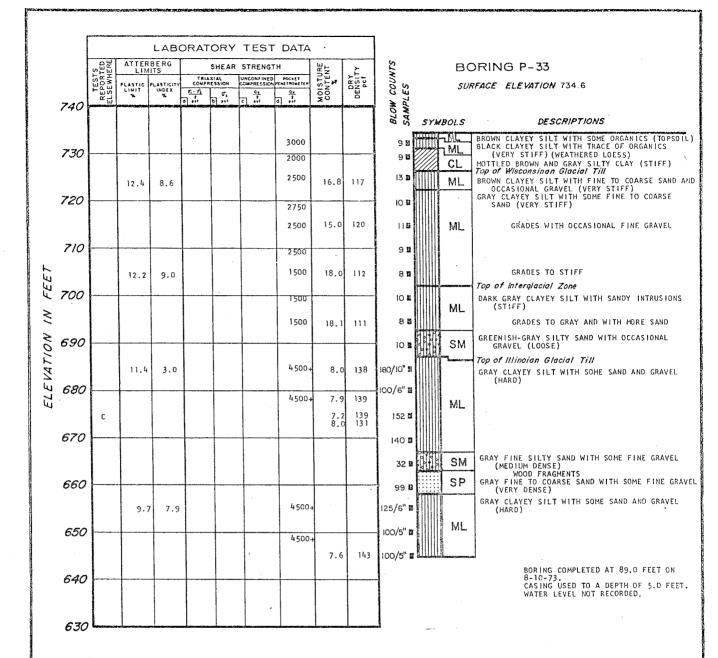


FIGURE 2.5-50

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

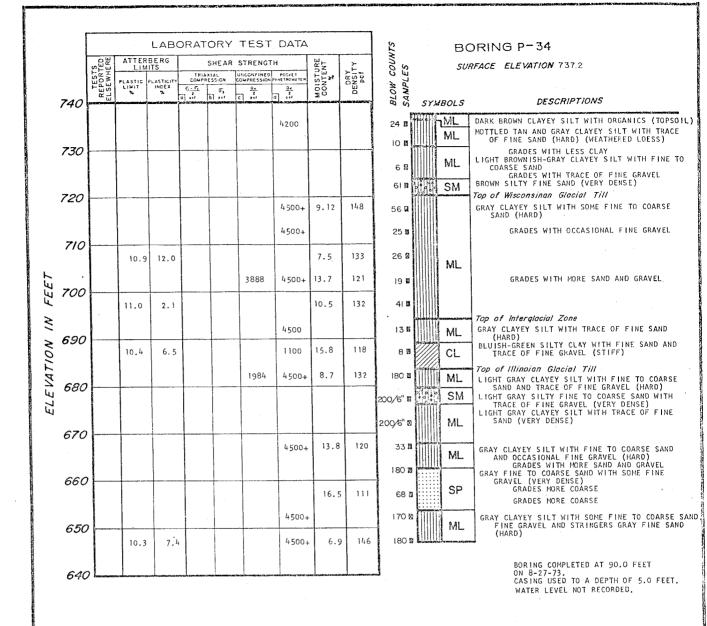


FIGURE 2.5-51

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

				LAB	ORAT	ORY	TEST	DATA			7.5		ВС	DRING P-35
		TESTS REPORTED SEWHERE	ATTER LIM		TAL	SHEAR	STRENG	POCKET	MOISTURE CONTENT	DENSITY Pcf	COUNTS			RFACE ELEVATION 737.8
	740	REPC ELSE	PLASTIC LIMIT	PLASTICITY	5:5	σ, b) ***	G 201	9 hi	CON	DEN	BLOW CO. Samples	SYM	BOLS	VESCRIPTIONS
								250			41		ηML CL	BROWN CLAYEY SILT WITH TRACE OF ORGANICS (TOPSOIL) TAN SILTY CLAY WITH TRACE OF FINE SAND
	730							2750			26 €		CL	(WEATHERED LOESS)  Top of Wisconsinan Glacial Till  BROWN SILTY CLAY WITH SOME FINE SAND AND GRAVEL
	720							4500+			25≌			(VERY STIFF) GRAY CLAYEY SILT WITH SOME FINE TO COARSE SAND AND GRAVEL (HARD)
	, 20			-				4500+			37 ₹		ML	12" LAYER GRAY SANDY SILT AT 14.0 FEET GRADES WITH LESS CLAY GRADES WITH THIN LAYERS OF SAND
	710					-		4500+			241			12" LAYER OF SANDY SILT AT 24.5"
	700		11.6	12.3					9.8	133	42 E		SM	GRAY SILTY SAND WITH LAYERS OF GRAY CLAYEY SILT WITH FINE TO COARSE SAND AND GRAVEL GRADES WITH STRINGERS GRAY CLAY
	700							4500+	13.9	123	l6 <b>s</b>		ML	GRAY CLAYEY SILT WITH SOME FINE TO COARSE SAND AND GRAVEL (HARD) Top of Interglacial Zone
	690							2800			9∎		CL	DARK GRAY SILTY CLAY WITH TRACE OF FINE SAND (STIFF)
			11.7	9.9				2000	15,1	120	7 ∎		CL	GREENISH-BROWN SILTY CLAY WITH FINE SAND AND TRACE OF MEDIUM TO COARSE SAND AND GRAVEL Top of Illinoian Glacial Till
	680	_	<u> </u>	ļ	<del> </del>			4500+		<u> </u>	89 ■			GRAY SANDY SILT WITH SOME CLAY AND GRAVEL (HARD)
FEET	670							4500+	7.37	143	110/6" <b>1</b> 250 <b>1</b>		ML	GRADES WITH LAYERS OF GRAY SILT FINE SAND WITH TRACE OF COARSE SAND AND FINE GRAVEL
	670							4500+	10.0		230 g			
/ //	660			<u> </u>	ļ		ļ	<u> </u>			116 1			12" BOULDER AT 173.0 FEET GRADES WITH TRACE OF ORGANICS GRAY FINE TO COARSE SAND WITH TRACE OF SILT
ELEVATION IN							-				701		sw	(VERY DENSE) GRADES WITH FINE GRAVEL AND LESS FINE SAND
137	650				-	-		<u> </u>	<del>                                     </del>	,,,	152/5" 1			GRAY SANDY SILT WITH TRACE OF CLAY AND GRAVEL (HARD)
7									7.1	141	250 0			GRADES DARK GRAY
	640	<del> </del>	9.7	4.2				4500+	6.4	143	200/5"			
	670							4500+			150/6"	#ill!		
	630							4500+	8.4	138	110/6"			
	620							4500+			200/3"		ML	
								4500+			200/2"			
	610	<u> </u>	10.9	6.4	-			4500+	7.6	140	270/9"			
			11.	6 8.1				4500+	7.4	139	200			GRADES WITH THIN LAYERS OF FINE SAND
	600	-			-			4500+	<del>                                     </del>	-	250/10"			
								4500+ 4500+		140.	230/10" 250			GRADES WITH MORE CLAY
	590							4500+	+-		250			GRADES WITH LESS CLAY
	580	Ŀ						45004	-		270			
	330								-	ſ		CL	INT	ON POWER STATION

FIGURE 2.5-52

LOG OF BORING P-35 .
(SHEET 1 of 2)

NOTE:

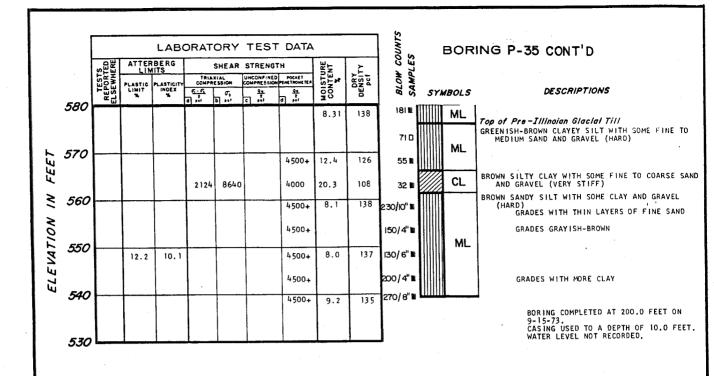


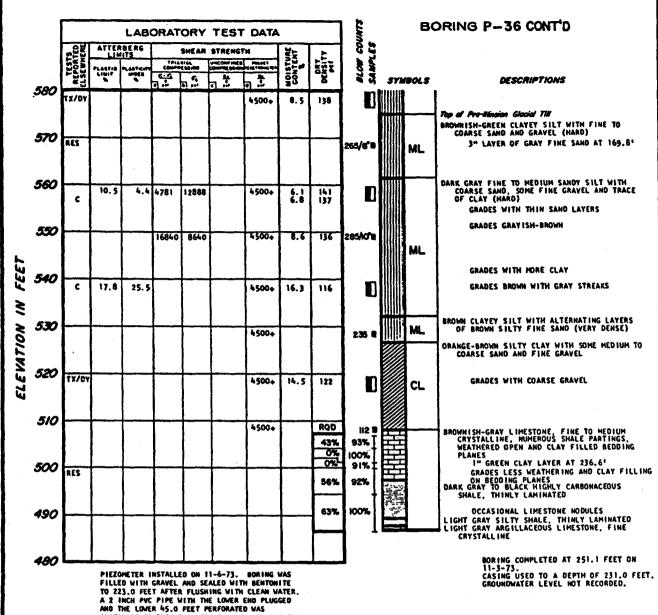
FIGURE 2.5-52 LOG OF BORING P-35 (SHEET 2 of 2)

					ORA"	ΓΟΙ	RY '	TEST	DATA	<u> </u>		.¥.		BO	RING P-36
	TS TED ERE	AT	TERB LIMIT	ERG S				TRENGT	1	URE	λ±.	COUNTS .ES		SUR	FACE ELEVATION 738.2
	TESTS REPORTED ELSEWHERE	PLAS	41T	ASTICITY INDEX	$\sigma_i - \sigma_i$	RESS			POCKET PERETROMETER  Qu 2 d pst	MOISTURE CONTENT	DRY DENSITY pcf	BLOW			
740	_~□		_		0 001	<u> </u>	) C	201	d 60	20			SYME		DESCRIPTIONS
									3000	!		7 LL			BLACK SILT WITH ORGANICS (TOPSOIL) MOTTLED BROWN AND GPAY CLAYEY SILT WITH SOME FINE SAND (VERY STIFF) (WEATHERED LOESS)
									2000 1500			4 🗷		ML	
730						1			1000	***************************************		6 ■		CL	Top of Wisconsinan Glacial Till BROWN CLAYEY SILT WITH SOME FINE TO COARSE SAND AND GRAVEL (MEDIUM STIFF)
									4500+			40 E		SP	GRADES GRAY  BROWN FINE TO MEDIUM SAND WITH TRACE OF SIL
720		L				_						70.8		3F	AND FINE GRAVEL (DENSE) DARK GRAY FINE TO COARSE SANDY SILT WITH TRA
	1								4500+			38 №			OF CLAY AND MEDIUM GRAVEL (HARD)
		ĺ							3750			l7 <b>1</b> ≧			GRADES WITH MORE CLAY AND COARSE GRAVI (VERY STIFF)
710		╁	-+			+			2000			8 1		ML	
									2000			15 ■			GRADES WITH LESS MEDIUM TO COARSE SAN 3" BROWN FINE SAND LAYER AT 34.5"
700									2000			15 10			
700									3000			14 E			4" LAYER OF FINE SILTY SAND AT 49.0" Top of Interglacial Zone
									1 500			9 ■			DARK GRAY SILTY CLAY WITH SOME FINE SAND AN GRAVEL (STIFF)
690	<u> </u>	1			<u> </u>	+			2500		ļ	16 <b>1</b> 2		CL	GRADES BLUE WITH MORE FINE SAND
	1						·		2500			10 2			GRADES WITH SEAMS OF SAND BLUE SILTY CLAY WITH TRACE OF FINE TO MEDIU
									500			8 %		CL	SAND (SOFT) Top of Illinoian Glacial Till
680		+	+		1281	6	4248	<del></del>	4500+	8.2	137	181 ■			GRAY FINE TO MEDIUM SANDY SILT WITH TRACE O CLAY AND FINE GRAVEL (HARD)
	TX/D	V								13.0	132				
670	1	l							2000		"	17		ML.	
									4000			l n			GRADES WITH SEAMS OF FINE GRAVEL
	1														·
660 650	╌	+			+		-		<del> </del> -	-	+	101 🗷		SP	GRAY FINE TO MEDIUM SAND WITH TRACE OF SILT (VERY DENSE)
	1	İ				-								Jr.	DARK GRAY FINE TO MEDIUM SANDY SILT WITH
		1			ì							200/6"			COARSE SAND AND SOME GRAVEL GRADES WITH FINE TO COARSE SAND
650									4500+						LAYERS GRADES WITH LESS SAND
												278/6" ■			
640	·	$\perp$			-	-				-		┨┎		ML	
	С		10.7	6.	5					6.5	140			l	
									4500+			300/6° 1	4111113		
630	<u>'</u>				1			<b> </b>				1 п		SP	GRAY FINE TO COARSE SAND WITH TRACE OF SIL AND OCCASIONAL FINE GRAVEL (VERY DENSE)
	İ				262	80	8236		4500+	6.6	142	200/5"			DARK GRAY FINE TO MEDIUM SANDY SILT, SOME
620	,	$\bot$			4			ļ			<b>↓</b>	274/9"		ML	COARSE SAND AND GRAVEL AND TRACE OF CLA
												2/4/9		SP	GRAY FINE TO MEDIUM SAND WITH TRACE OF SIL
	С		11.9	9.	0				4500+	8.€	135				DARK GRAY FINE TO MEDIUM SANDY SILT WITH COARSE SAND AND SOME GRAVEL
610	<b>'</b>  -	+			140	40	8640		4500+	8.0	137	174			20" SAND LAYER AT 134.5"
										-					
600	, L														
500	С		11.8	7.	0				4500+	7.9	138			ML	GRADES WITH MORE CLAY
•															
590	<b>&gt;</b> ├-	+			+			-	4500+	-	-	300/10"1			
														1	
	_			1											4
580	7 <b>-</b>			1			1						# 1L111	1.	·

FIGURE 2.5-53

LOG OF BORING P-36 (SHEET 1 of 2)

NOTE:



INSTALLED TO ELEVATION 515.2. GRAMULAR BACKFILL WAS PLACED FROM ELEVATION 515.2 TO 560.2; A SENTONITE SEAL FROM ELEVATION 560.2 TO 562.2; AND CEMENT GROUT FROM ELEVATION 562.2 TO 738.2.

### WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET

DATE

11-15-73 12-31-73

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-53

LOG OF BORING P-36

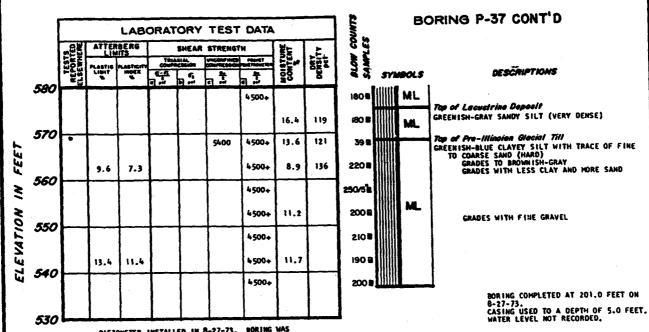
(SHEET 2 of 2)

1			LAB	ORAT	ORY	TEST	DATA			73		вс	ORING P-37
	TS TED HERE	ATTER LIM			SHEAR	STRENGT	'н	ENT		COUNTS ES		SUR	RFACE ELEVATION 739.1
740	. TESTS REPORTED ELSEWHERE	PLASTIC LIMIT	PLASTICITY INDEX %	2-0	TS D POT	UNCONFINED COMPRESSION	GU POI	MOISTURE CONTENT	DRY DENSITY Pcf	BLOW	5YI	MBOLS	DESCRIPTIONS
740							1100			4∎		1 IVIL	DARK BROWN CLAYEY SILT WITH TRACE OF ORGANICS (TOPSOIL) YELLOWISH-BROWN CLAYEY SILT WITH TRACE OF FIN SAND (STIFF) (WEATHERED LOESS)
730				<u> </u>			2250			9 ≌			Top of Wisconsinan Glacial Till Light Gray Clayey Silt with some fine to coar
							4500+			19 🛍			SAND (VERY STIFF) GRADES TO GRAY (HARD)
720							4500+	11.6	128	20 🛭		ML	GRADES WITH FINE GRAVEL
710							4500+			211			
,,,		12.5	2.7				4500+	13.3	117	19 🛍		ML	GRAY CLAYEY SILT WITH FINE SAND (HARD)
700							4500+	12 5	10/	33 ⊠	Ш	<u> </u>	GRADES WITH OCCASIONAL COBBLES GRAY CLAYEY SILT WITH TRACE OF FINE TO MEDIU
							2750	13.5	124	32 <b>■</b> 27 <b>■</b>	Ш	ML	SAND (HARD) GRADES WITH TRACE OF COARSE SAND AND FINE GRAVEL
690	<u> </u>	12.6	16.1	<u> </u>		1	,-	17.0	113	- 13 <b>≥</b>		ML	Top of Interglacial Zone  GRAY CLAYEY SILT WITH TRACE OF FINE SAND AND OCCASIONAL GRAVEL
										20 ∎	D D		GRADES WITH TRACE OF ORGANICS BLUISH-GREEN SILTY FINE TO COARSE SAND WITH
680					1		4500+	8.0	140	147 ■		SM	TRACE OF CLAY (MEDIUM DENSE) GRADES WITH LAYERS OF SILTY SAND Top of Illinoian Giaciai Tili
						12064	4500+	8.2	138	177 <b>/</b> 11" <b>=</b>			GRAY CLAYEY SILT WITH SOME FINE TO COARSE SA AND OCCASIONAL FINE GRAVEL (HARD)
670		9.5	4.8				4500+	8.2	140	127 🗷		ML	GRADES WITH TRACE OF ORGANICS
660			ļ							46 ■			LAYER GRAY FINE TO COARSE SAND AND FIN
660 650							1.500		,,,,	83 ■		SP	GRAY FINE TO COARSE SAND AND FINE GRAVEL GRAY CLAYEY SILT WITH SOME FINE TO COARSE SA
650	-			-	-	-	4500+	8.0	139	175/10"			AND OCCASIONAL FINE GRAVEL (HARD) GRADES WITH OCCASIONAL COARSE GRAVEL
		8.41	5.8				4500+	7.2	144	220 1			
640	$\vdash$	ļ		<del> </del>	-		4500+	-		200			,
							4500+	8.1	137	105 /6"			
630	·	<b>†</b>					4500+			250/4" 🛚			
620		10.4	9.5	5			4500+	10.6		801		ML	
020						13410		7.9	137	I8O/IO" ■			·
610	-				-	-	4500+	8.3	138	200/10"			
		10.7	6.9	,			4500+ 4500+	7.9	140	200			
600	·		"	1	+		4500+	\		200			GRADES WITH MORE GRAVEL
							4500+	7.5	140	100/5"1			
590	<b>'</b>			+			4500+			115/6"1			
580		10.9	6.0	,			4500+	7.3	140	1301			

FIGURE 2.5-54

LOG OF BORING P-37
(SHEET 1 of 2)

NOTE:



PIEZOMETER INSTALLED IN 8-27-73. BORING WAS FILLED WITH GRAWEL AND SEALED WITH BENTONITE TO 41.0 FEET AFTER FLUSHING WITH CLEAN WATER. A 1 1/2 INCH PMC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 24.0 FEET PERFORATED WAS INSTALLED TO ELEVATION 699.1 FEET. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 701.1 TO 726.1: A BENTONITE SEAL FROM ELEVATION 726.1 TO 728.1; AND CENENT GROUT FROM ELEVATION 726.1 TO 728.1; AND CENENT GROUT FROM ELEVATION 726.1

#### WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET	DATE
14.2	10-5-73
16.0	10-29-73
15.9	11-15-73

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-54
LOG OF BORING P-37
(SHEET 2 of 2)

		LABORATORY TEST DATA								BORING P-38			
	TEO TERE	ATTER	BERG	SHEAR STRENGTH				캶	Ł	, co	SURFACE ELEVATION 738.9		
	TEST REPORT ELSEWHE	PLASTIC LIMIT	PLASTICITY INDEX	TRIA COMPR G-G	σ,	UNCONFINEI COMPRESSION	POCKET NPENETROMETER Qu d pol	MOISTURE CONTENT	DENSITY pcf	BLOW COUNTS SAMPLES	SY	4BOLS	DESCRIPTIONS
740				11			2500			9 ₺		ML	BLACK CLAYEY SILT WITH TRACE OF ORGANICS
							1000			6 ⊠		CL	MOTTLED BLACK AND BROWN SILTY CLAY WITH TRAI ORGANICS AND FINE SAND (WEATHERED LOESS)
730		14.8	7.6			<del></del>		18.0	111	5 €		ML	Top of Wisconsinan Glacial Till BROWN CLAYEY SILT WITH FINE TO COARSE SAND
										25		<b> </b>	FINE GRAVEL (SOFT) GRADES HARDER
								12.5	130			SM	GRAY SILTY FINE SAND WITH TRACE OF CLAY ALTERNATING WITH LAYERS OF GRAY FINE SAN (MEDIUM DENSE)
720			<u> </u>					72.7	1,50	36 ▮			GRAY CLAYEY SILT WITH SOME FINE TO COARSE SAND AND OCCASIONAL FINE GRAVEL
										28 🗈		- C14	6" LAYER OF GRAVEL AT 20.0" GRAY SILTY FINE SAND WITH TRACE OF MEDIUM A
710								13.8	124	46		SM	COARSE SAND (MEDIUM DENSE)
							1.500		į	_			GRAY CLAYEY SILT WITH SOME FINE TO COARSE S
							4500+		ŀ	57 9		ML	
700		13.5	10.4	-			3000	16.8	120	26			Top of Interplacial Zone GRAY CLAYEY SILT WITH SOME FINE TO COARSE
										11 10			SAND, TRACE OF ORGANICS AND OCCASIONAL GRAVEL
		11.6	9.2				4500+	13.2	123			ML	-
690							1			21			
										67 🛚		SP	GRAY AND BROWN FINE SAND WITH TRACE OF SILT (VERY DENSE) Top of Illinoian Glacial T///
680		10.3	3.5				4500+	9.4	136	100/3°E		1	LIGHT GRAY CLAYEY SILT WITH SOME FINE TO COARSE SAND AND FINE GRAVEL (HARD)
							1.500					ML	COARSE SAILD AIRD FINE BRAVEL (MARD)
							4500+			57 €			GRAY FINE TO COARSE SANDY SILT WITH
670	SAL					-	+	13.0	132	13 🛢		ML	TRACE GRAVEL (DENSE)
	TX/DY SA									23 🛍		CH	GRAY SILTY FINE TO COARSE SAND WITH
660										105 -	100	SM	TRACE GRAVEL (DENSE) GRADES WITH LESS SILT AND MORE GRAVEL
660 650	С	9.2	6.0	11971	5760		4500+	7.7 7.6	138 146	105			GRAY CLAYEY SILT WITH SOME FINE TO COARSE S AND FINE GRAVEL (HARD)
							4500+			150/11			
<i>650</i>	TX/CU	9.9	7.8	19646	6480		4500+	8.4	140	160/10			•
		3.3		15,510			4500+	0.8	120	100/6			GRADES WITH OCCASIONAL STRINGER OF FI
				8014	6840		4500+	6.0	150			ML	SAND
640	С	9.7	5.8				7300+	6.4	138	100/5			
	С						4500+	6.9	139	150/11			6" COBBLE AT 101.5"
630	TX/DY			<u> </u>			4500+	8.0 7.9	137 143	160/9			
							6.500 ·						
	l						4500+			140		SP	GRAY FINE TO COARSE SAND
620	c			<del> </del>			4500+		<del> </del>	146		1 31	GRAY CLAYEY SILT WITH SOME FINE TO COARSE S
	`						4500+	10.1	. 131	153			AND FINE GRAVEL (HARD)
610							4500+	8.8	137	137			
0,0										1 " n			
							4500+			142			
600	С	9.8	6.6	<b>├</b>	<del> </del>		4500+	8.3	137	I45/II" <b>1</b>		ML	
	ľ	9.0	0.0				4500+	0.3	13/	147/11		1	
<b>5</b> 00							4500+	, ,	120				
590	TX/DY						4500+	7.9 9.2	1	145			
	RES				'		4500+			140			
580	L	9.5	5.4				4500+	8.7	136	135		1	

FIGURE 2.5-55

LOG OF BORING P-38

(SHEET 1 of 3)

NOTE:

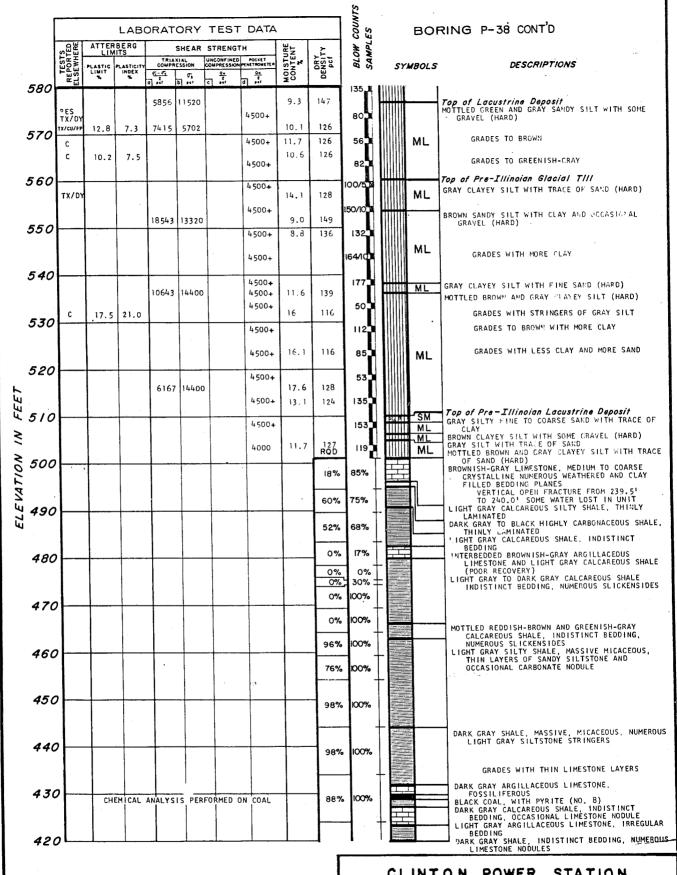


FIGURE 2.5-55

LOG OF BORING P-38
(SHEET 2 of 3)

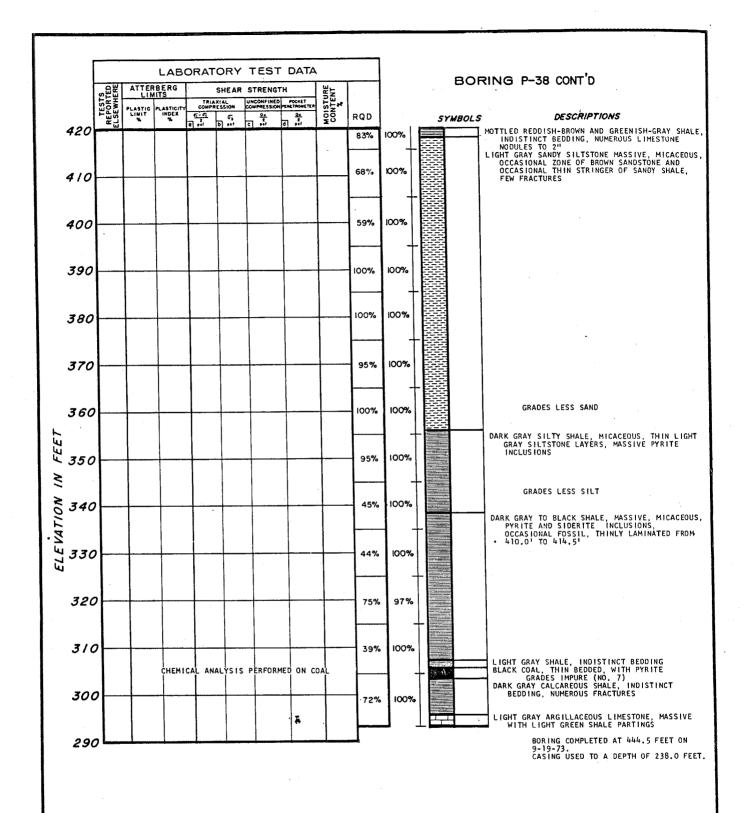
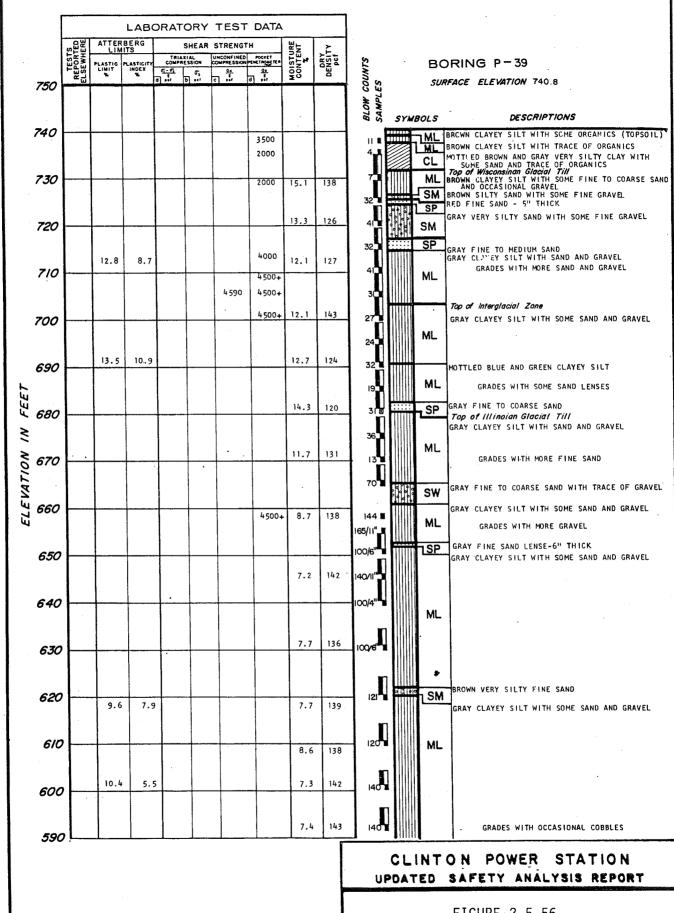


FIGURE 2.5-55

LOG OF BORING P-38

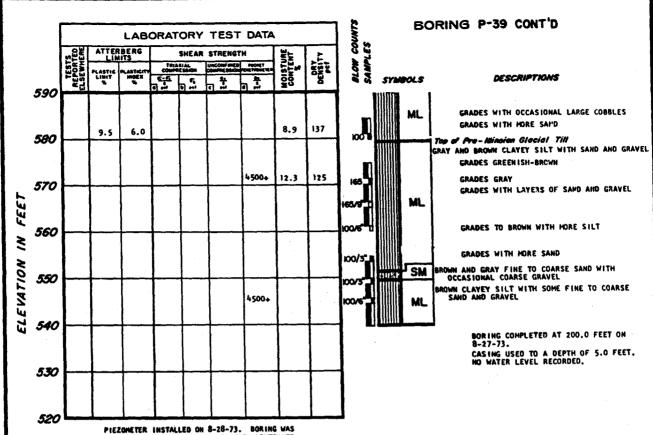
(SHEET 3 of 3)



NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS. FIGURE 2.5-56

LOG OF BORING P-39 (SHEET 1 of 2)



PIEZOMETER INSTALLED ON 8-28-73. BORING WAS FILLED WITH GRAVEL AND SEALED WITH BENTONITE TO 152.0 FEET AFTER FLUSHING WITH CLEAN WATER. A 1 1/2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 90.0 FEET PERPOARTED WAS INSTALLED TO ELEVATION 570.8 FEET. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 590.8 TO 678.8; A BENTONITE SEAL FROM ELEVATION 678.8 TO 680.8; AND CEMENT GROUT FROM ELEVATION 680.8 TO 740.8.

### WATER LEVEL READINGS

SURFACE IN FEET	DATE
32.9	<del>9-</del> 7-73
33.4	10-29-73
11.2	11-15-73

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-56 LOG OF BORING P-39

(SHEET 2 of 2)

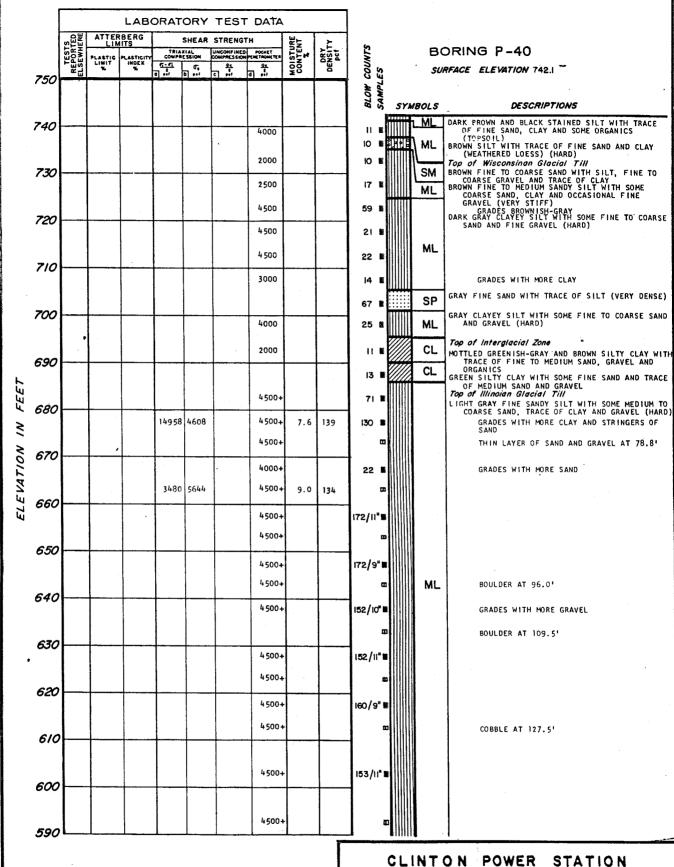
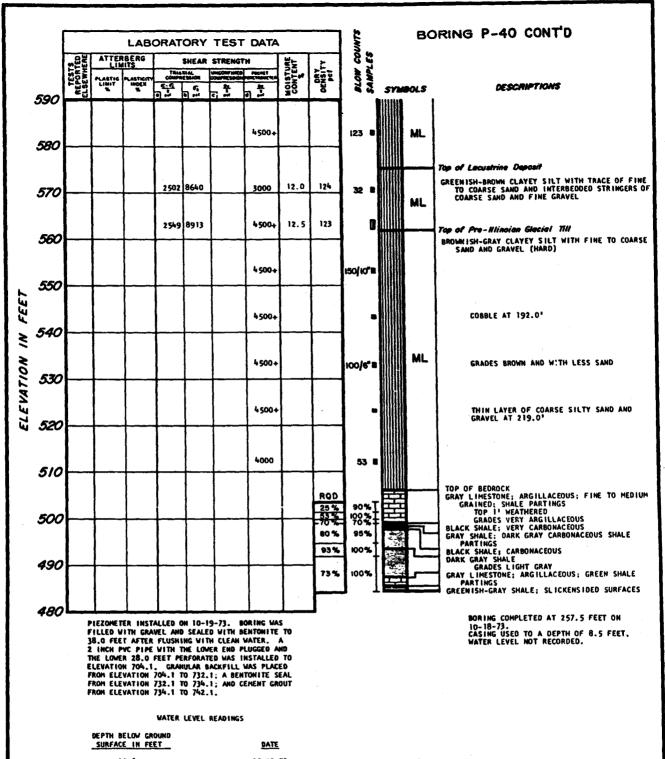


FIGURE 2.5-57

LOG OF BORING P-40 (SHEET 1 of 2)

NOTE:



12.6 12.4 10-29-73 11-15-73

BECAME INOPERATIVE DECEMBER 31, 1973

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-57

LOG OF BORING P-40

(SHEET 2 of 2)

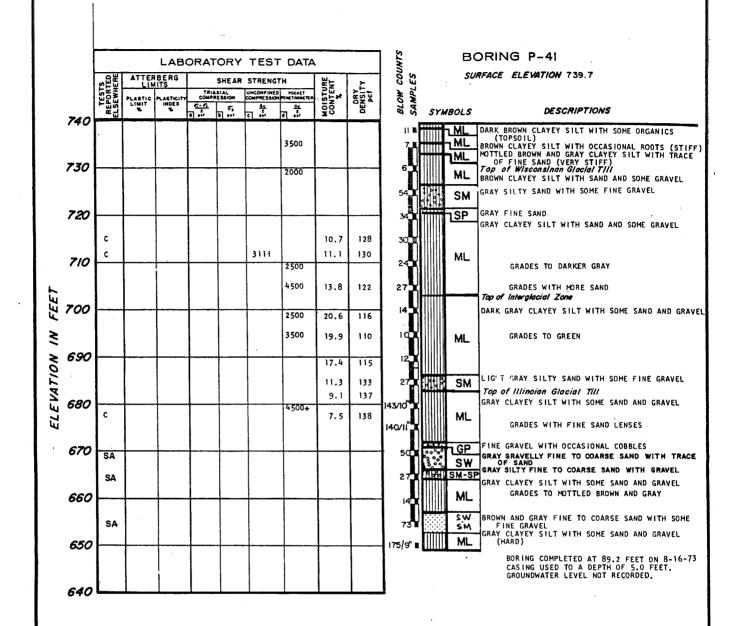


FIGURE 2.5-58

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

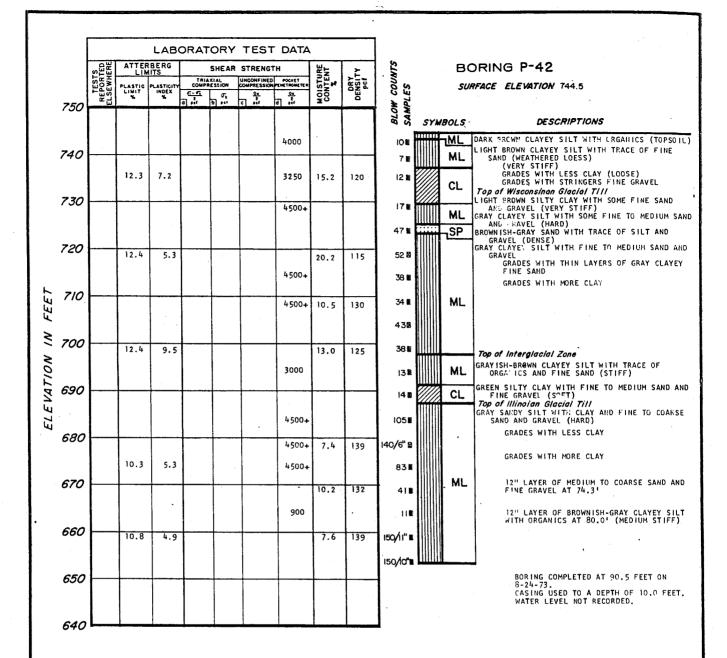


FIGURE 2.5-59

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

Ţ	REPORTED ELSEWHERE	ATTER	RBERG.		SHÉAR	STRENG		- HE	<u>}</u>				
, v	Fer	PLASTIC LIMIT	PLASTICITY INDEX	COMP	AXIAL RESSION		D POCKET IN PENETROMETER	MOISTURE CONTENT	DENSITY pcf	57		во	ORING P-43
٦Ľ	RE ELS	*	**	व कुर इंट्रिक	σ, b •••	0 991	0 per	<b>8</b> 8	8	COUNTS ES			RFACE ELEVATION 740.3
7										. CO		, 30	THOSE ELEVATION 140.5
1	- 1			l	-			-		BLOW			
												BOLS	DESCRIPTIONS
가				<b></b>			3750			11 18		l ML	DARK BROWN SILT WITH CLAY AND SOME ORG
-	- 1		1			Ì	2250	16.2	118	Ä		i	MOTTLED BROWN AND ORANGE WITH BLACK FL
1		12.5	6.7				4400	14.4	121	7 篇		ML	SILT WITH SOME SAND AND TRACE OF CL (VERY STIFF) (WEATHERED LOESS)
가		12.5	1		1	+	1			8 🖺			GRADES MOTTLED ORANGISH-BROWN A Top of Wisconsinan Glacial Till
				}			4500+	13.1	123	15		1	BROWN WITH ORANGE AND BLACK STAINING, SILT WITH SOME FINE TO MEDIUM SAND
١				Ì							ЩЩ	,ML	OCCASIONAL GRAVEL (VERY STIFF) GRADES WITH MORE FINE TO MEDIUM
o -			<del></del>	<del> </del>	+	+		-	1	51 <b>E</b>			GRAVEL (HARD) GRADES WITH MORE CLAY
1							4500+			35 ₺	iiiiiii	SP	BROWN MEDIUM TO COARSE SAND WITH SOME FINE GRAVEL
-								1,2 0		- [			GRADES WITH FINE TO MEDIUM SANG
οL		14.8	9.7	ļ	-		4500+	13.0	123	3/11		ML.	SOME FINE GRAVEL
-							2500		ا ,,,			ML	4" LAYER GRAY MEDIUM SAND AT 2 GRAY AND GRAYISH-BROWN SILT WITH SOME
١			1			1	2500	14.3	118	9 ≝			ORGANICS AND TRACE OF CLAY GRAY CLAYEY SILT WITH FINE TO MEDIUM
<b>)</b>		12.6	7.1				1500	11.4	119	.9 ₪		ML	FINE GRAVEL AND OCCASIONAL SAND SE.  Top of Interglacial Zone
1				İ								ML	DARK GRAY AND BLACK CLAYEY SILT WITH
1							1200	17.9	113	6■		₋ ML	DARK GRAYISH-GREEN CLAYEY SILT WITH F
οŀ		11.1	9.4				800	17.	113	4 8		ML	DARK GREEN FINE TO MEDIUM SANDY SILT CLAY AND FINE GRAVEL (MEDIUM STIFF
7								1					Top of Illinolan Glacial Till
							4500+	9.	135	44 🖥			LIGHT GREEN FINE TO MEDIUM SANDY SILT
		10.4	2.9	,		]	4500+	8.8	3 137	35 ≋		ML	SOME CLAY AND GRAVEL (HARD)
or		<b></b>	<del>                                     </del>	<b>†</b>						30 €		IVIL	GRADES LIGHT GRAY GRADES WITH MORE FINE SAND
1						į.	4500+	7.9	140	95 ₤			
1											min	SW	GRAY FINE TO COARSE SAND WITH GRAVEL
아	·	<del>                                     </del>	+	+			4500+	+		47 ■		311	OF MEDIUM AND FINE SAND
-						1	4500+			27 ■		ML.	LIGHT GRAY SILT WITH SOME FINE SAND A GRAVEL (HARD) 12" LAYER GRAYISH-BROWN SILT W
١		NO.	LASTIC				1.5554	,, ,	, ,,,		للسلا		ORGANICS AT 73.6' GRADES DARK GRAY AND WITH GRAV
0		NUN-P	143116	+				11.9	130	125 🖀	nnin	sw	GRAY FINE TO COARSE SAND WITH SOME SI
- 1							4500+	7.0	142	120 🗈		١	DARK GRAY CLAYEY SILT WITH FINE TO CO AND SOME GRAVEL (HARD)
١												ML	GRADES WITH MORE SAND
0		-	-				4500+	7.8	160	107/6" ■			BORING COMPLETED AT 89.0 FEET
												•	9-25-73. CASING USED TO A DEPTH OF 3.0

FIGURE 2.5-60

LOG OF BORING P-43

NOTE:

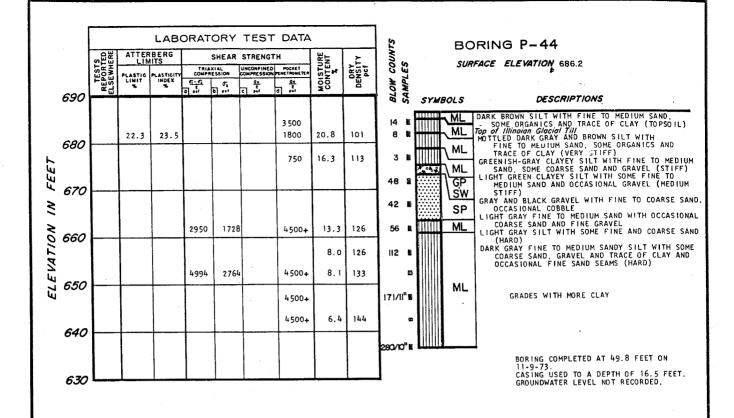


FIGURE 2.5-61

LOG OF BORING P-44

NOTE:

				LAB	ORAT	ORY	TEST	DATA			COUNTS			вс	DRING P-45
		ESTS PORTED EWHERE	ATTER	BERG			STRENGT UNCONFINED COMPRESSION		FURE	<u>}</u>	700	"ES		SUR	RFACE ELEVATION 738.8
	740	REPO ELSEW	PLASTIC LIMIT	PLASTICITY INDEX %	€ <u>-</u> €1		Qu.	Qu Qu d set	MOISTURE CONTENT	DENSIT	MO78	SAMPLI	YM	IBOLS	DESCRIPTIONS
	740		20.6	26.1				4000	14.7	101	13			/ML	BROWN SILT WITH ORGANICS (TOPSOIL) LIGHT BROWN SILT WITH SOME CLAY, FINE SAND AND
	730							3 500			19 13	E		ML	TRACE OF FINE GRAVEL (HARD) (WEATHERED LOESS) GRADES WITH MORE CLAY AND TRACE OF MEDIUM TO COARSE SAND
					1			3 500		1	13	* H	Ш		Top of Wisconsinan Glacial Till
١,								4500			22				GRAY FINE SANDY SILT WITH SOME CLAY, MEDIUM TO COARSE SAND AND FINE GRAVEL (HARD)
FEET	720		12.5	10.9	<del>                                     </del>	<b></b> -		4500	17.7	120	21	E			GRADES WITH COARSE GRAVEL
3								4000			17				GRADES WITH MORE CLAY
1	710		<u> </u>		4248	2088		4000	13.0	124	21	1		ML	•
7/0								4 500			18				
ELEVATION	700		12.5	15.4				4 500	12.7	127	22	E			
£4.								4 500			17				Top of Interglacial Zone
	690	-			2214	3 5 2 8		4 500	13.0	125	20	. L		ML	GRAY CLAYEY SILT WITH SOME FINE SAND AND ORGANICS.
	680														BORING COMPLETED AT 50.0 FEET ON 11-8-73. CASING USED TO A DEPTH OF 10.0 FEET. GROUNDWATER LEVEL NOT RECORDED.

FIGURE 2.5-62

LOG OF BORING P-45

NOTE:

			LAB	ORAT	ORY	TEST	DATA			
,	TESTS REPORTED ELSEWHERE	ATTER LIN PLASTIC LIMIT	PLASTICITY	TRIA COMPR	SHEAR	STRENGT UNCONFINED COMPRESSION		MOISTURE CONTENT	DRY DENSITY pcf	SURFACE ELEVATION 741.1
750										A SYMBOLS DESCRIPTIONS
740 1-33 1-33 1-33		14,1	9.4	2229	432		750	23.4	95 107	7 ML BROWN SILT WITH ORGANICS, FINE TO COARSE SAND AND TRACE OF FINE GRAVEL LIGHT RROWN SILT WITH TRACE OF FINE SAND AND CLAY (LOOSE) (WEATHERED LOESS) TOP OF Wisconsinan Glacial Till BROWN FINE TO MEDIUM SANDY SILT WITH TRACE OF
₹ / 5				3612	1008			13.3	122	CLAY BROWN CLAYEY SILT WITH SOME FINE TO COARSE SAND AND GRAVEL (MEDIUM STIFF) BROWN SILT WITH STRINGERS OF FINE TO COARSE SAND AND FINE GRAVEL (DENSE) GRAY SILTY FINE TO COARSE SAND WITH FINE
ELE VATION 112		13.3	7.4			1	4500	22.0	118	GRAVEL (VERY DENSE) GRAY CLAYEY SILT WITH SOME FINE TO COARSE SAND AND FINE GRAVEL  GRAY FINE TO MEDIUM SAND WITH A TRACE OF FINE GRAVEL AND THIN LAYERS OF GRAY SILT
773				1,662	2808		4000	11.7	126	GRAY CLAYEY SILT WITH SOME FINE TO COARSE SAND AND GRAVEL
70				4002	2308		4000			23 ML GRADES WITH STRINGERS OF FINE TO COARSE
69	o <b>L</b> _	12.6	7.9	1			4500	13.6	125	19 M SAND

BORING COMPLETED AT 50.0 FEET ON 11-7-73. CASING USED TO A DEPTH OF 10.0 FEET. GROUNDWATER LEVEL NOT RECORDED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-63

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

	İ			LAB	ORAT	ORY	TEST	DATA			BLOW COUNTS SAMPLES			PRING P-47
		TS TED HERE	ATTER LIM	BERG ITS	TRIA		STRENGT		rure	<u>,                                    </u>	27. 200 %		SUR	PFACE ELEVATION 739.7
		TESTS REPORTED ELSEWHERE	PLASTIC LIMIT	PLASTICITY INDEX	GOMPR G-Gi	ESSION C.	UNCONFINED COMPRESSION 94.	Og gef	MOISTURE CONTENT	DRY DENSITY pcf	BLON	SYM	IBOLS	DESCRIPTIONS
	740		19.3	45.5				3750	21.0	95	· 9 L		ML	BROWN SILT WITH ORGANICS AND TRACE OF FINE GRAVEL (TOPSOIL) BROWN CLAYEY SILT WITH TRACE OF FINE GRAVEL
	730		13.5	3.4				750	17.0	112	3 L		ML	(WEATHERED LOESS)  Top of Wisconsinan Glacial Till  BROWN FINE SANDY SILT WITH TRACE OF CLAY AND
					1199	1008			12.7	124	19		ML	FINE GRAVEL (LOOSE) GRADES WITH LESS FINE SAND
FEET	720									-	48		SP	BROWN FINE SAND WITH TRACE OF FINE GRAVEL GRADES GRAY AND WITH MEDIUM SAND
<i>N</i> /											83 1			GRAY CLAYEY SILT WITH SOME FINE TO COARSE SAND AND GRAVEL (HARD)
0 ∕	710				3924	2088		4000	12.9	124	18			
471			12.8	7.8				750	15.3	120	8 1		ML	(MEDIUM STIFF)
ELEVATION	700					<del>                                     </del>					9 '			GRADES WITH MORE COARSE GRAVEL
13					4464	3168	3	4000	12.7	118	27 1		ML	(HARD)  Top of Interglacial Zone DARK GRAY SILT WITH ORGANICS AND TRACE OF FINE
	690		14.8	4.6				1000	16.5	113	9 1		INIL	SAND AND CLAY (MEDIUM STIFF)
	680													BORING COMPLETED AT 50.0 FEET ON 11-8-73. CASING USED TO A DEPTH OF 10.0 FEET. GROUNDWATER LEVEL NOT RECORDED.

FIGURE 2.5-64

LOG OF BORING P-47

				LAB	ORAT	ORY	TEST	DATA			
,		TS TED TERE	ATTER	BERG			STRENG		URE	λL	
		TESTS REPORTED ELSEWHERE	PLASTIC LIMIT	PLASTICITY INDEX	<u>e-e</u>	KIAL ESSION	C psf	POCKET "" PENETROMETER  OT.  d psi	MOISTURE CONTENT	DRY DENSITY pcf	
	730					91 J.		91			
	720		13.8	9.1				4500+	11,4	120	3
FEET	, 20				3317	720		4500+	13.3	123	3
FE	710	<u> </u>			<b></b>		<del> </del>	4500+	ļ		
ELEVATION IN			12.4	13.0				4500+	12.4	126	2
0.0	700.	ļ		-		-	<del>                                     </del>	-			1
471					1962	2088			14.0	121	
EV	690		ļ	<u> </u>	ļ		ļ	4500+	14.4	124	2
7.3	,				3942	2808		4500+	13.7	123	2
	<b>6</b> 80					<u> </u>			<del>                                     </del>	<del>                                     </del>	1 ;
			11.2	3.8					9.2	135	
	670	<u> </u>	<u> </u>	<u></u>				<u> </u>	<u> </u>		1

BLOW COUNTS	ĘŞ	BORING P-48  SURFACE ELEVATION 724.6										
MOTE	SAMPLI	SYA	IBOLS	DESCRIPTIONS								
18	BIE		ML	Top of Wisconsinan Glacial Till MOTTLED BROWN AND GRAY ORGANIC FINE TO								
36	E		SP	MEDIUM SANDY SILT WITH SOME COARSE SAND AND FINE GRAVEL AND TRACE OF CLAY								
34				ORANGE FINE SAND WITH TRACE OF MEDIUM SAND DARK GRAY SILT WITH SOME CLAY AND FINE TO MEDIUM SAND AND GRAVEL (HARD)								
21	E			•								
24	E		ML	·								
16 20												
18			ML	Top of Interglacial Zone DARK GREENISH CLAYEY SILT WITH SOME FINE SAND								
27	E		₩.	AND GRAVEL AND OCCASIONAL ORGANICS (HARD) 3" COBBLE AT 30,7"								
24	E		ML	Top of Illinoian Glacial Till GRAY SILT WITH FINE SAND, OCCASIONAL FINE GRAVEL AND OCCASIONAL FINE TO MEDIUM SAND SEAMS (HARD)								
30	E		SP	BLACK FINE TO COARSE SAND WITH SOME FINE GRAVEL								
54			ML	LIGHT GRAY FINE TO MEDIUM SANDY SILT WITH SOME COARSE SAND, FINE GRAVEL AND TRACE OF CLAY (HARD)								

BORING COMPLETED AT 50.0 FEET ON 11-6-73. CASING USED TO A DEPTH OF 3.5 FEET. GROUNDWATER LEVEL NOT RECORDED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-65

NOTE:

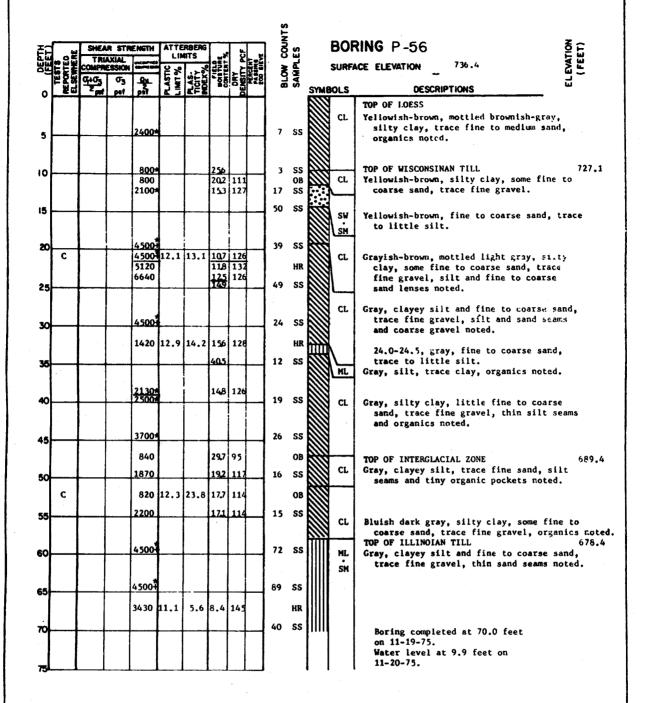
SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

្ឋា	, l	SHEA	R STR	ENGTH		RBERG		بيا		Š	SAMPLES		E	30F	RING	P-4	9		ELEVATION (FEET)
4.	PORTED SEWHERE	TRI/	XIAL	Comment and by	FIW		ENT.	₽	SEN	. ₹	Ē		S	SURFA	ACE ELEV	ATION	740.2		₩.E
7	SEG	GIO3	σ ₃	Qu.	PLAST!	PLAS- TICITY INDEX%	MOISTUR CONTENT	Š	200 S	0.0	SAR	~~	~	o. c		ne e	SCRIPTIONS		긥
-	F F	2 perf	pst	pst-	4 5	4€∑			$\vdash$			SYM	***	MI.	TOP OF		SCRIPTIONS		
				1									1	m.	101 01	1.(/1)			
l	1			800*						8	22		$\ $	GI.	Brown, m	ottled	gray, silty clay,	trace	
H											00	Щ			organ	ic medi	ium sand, pockets o erial noted. SINAN TILL	or orack	
l	i											M	3	$\overline{}$	TOP OF	WISCONS	SINAN TILL		734.
L			Ĺ	1500						13	SS		B	CL	Brown, s	silty c	lay, some fine to	ŀ	
l													3				, trace fine grave! erial noted.	٠,	
١				1			]						1						
ŀ							-1			39	55		1	ML	Light g	rayish-	-brown, fine sand a	nd silt,	
١		i					)					Ш	II.		trace	clay,	thin sand seams no	oted.	
L			l	2100*			L			13	SS		1	CI.	Graty, s	ilty c	tay, some fine to	coarse	
Γ													E		sand,	little	e fine gravei.		
l	C			2000*	11,	16.7	1	118	1	1	HK		3	į			o coarse gravel, se terial noted.	cartered	
ŀ		<u> </u>		38004	ļ	<del> </del>	140		-	21	SS		Ï						
l				( )									3						
l				45004					·	35	SS		3		-little	fine	gravel.		
T										1	HR		3				-115 fine to		
l	С	l		45001	11.5	14.0	10.5	128				77	ß	CL	sand,	trace	silt, some fine to fine gravel, tiny	pockets	
L		<u> </u>		2600	<b> </b>		-		├	14	SS		1	ML	of sa	and, th	nin silt seams, and	coarse	
١		i											3	<u> </u>		el note	clay, some fine to	coarse	
١			1	4070 2500			148	12	4	22	SS			CL	sand,	, trace	fine gravel, thin	clay	
十	С	<b></b> -	-	2300	NP		256	99		1	OB	11111	W	\	seams	s noted			
l										1			Ш		1		LACIAL ZONE		700.
Ļ			<u> </u>	1500	L	ļ	<b> </b>	ļ.,	<del> </del>	8	SS		1	ME			y, silt, trace fine	to	, ,,,,
	C.		İ	3500	10.0	24.4	174	11	d		08			<u> </u>	media	um sand	d, trace clay.	ina ta	
l	••			. ,			186	ĺ		14	<b>5</b> S	1	$\frac{3}{2}$	CI.	medi	sn-gray um sand	y, silty clay and f d, trace fine grave	el.	
ት		<b>-</b>		1	<del>                                     </del>	<del>                                     </del>	<del>                                     </del>		T	1		33	Ŋ	SC	4		y, fine to medium s		e
١		1										13	Š	CL	silt	y clay,	, trace fine gravel	•	688.
Ļ				4500			9.2		<u> </u>	$\frac{100}{40}$	SS			ML			01AN THA. silt and fin <b>e</b> to m	ied Lum	500.
1					1								Ш		sand	, trace	e fine gravel.		
1				4500	4		8.0			100			Ш	SM					
ŀ			<del> </del>	1	•	+	1		T		SS	-	Ш	1			•		
I	,C			4500 <del>1</del>	10.3	8.0	7.1	13	1		HR						o coarse sand.		
L						ļ	<u></u>	<u> </u>	<u> </u>	145	SS	#	₩	<del> </del> -	-and f	ine to	coarse sand.		
<b>i</b>	С	[		4500	10.8	7.4	8.4	13	1		HR		∭	ML	Gray,	silt,	trace fine to coar.	se sand,	
	-				1			1		1,20	S\$	H.	Щ	SM	trac	e clay.	o coarse sand, lit	tle fine	
╁		-	<del> </del>	+	$\vdash$	+	10.7	+-	+	1 1 28	35	Hit	۱۴.	3M	grav	el, li	ttle silt, trace c	lay.	
1		1	Ì		1	1							1		-				
١					]	L		L		]				ļ			Boring complete	d at 70.0	)
		~															feet on 4-7-75. Water level at		

Logged by: Sargent & Lundy Engineers
Drilled by: Raymond International
Tested by: Soil Testing Services Inc.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-66



Logged by: Sargent & Lundy Engineers
Drilled by: Raymond International
Tested by: Westenhoff & Novick, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-67

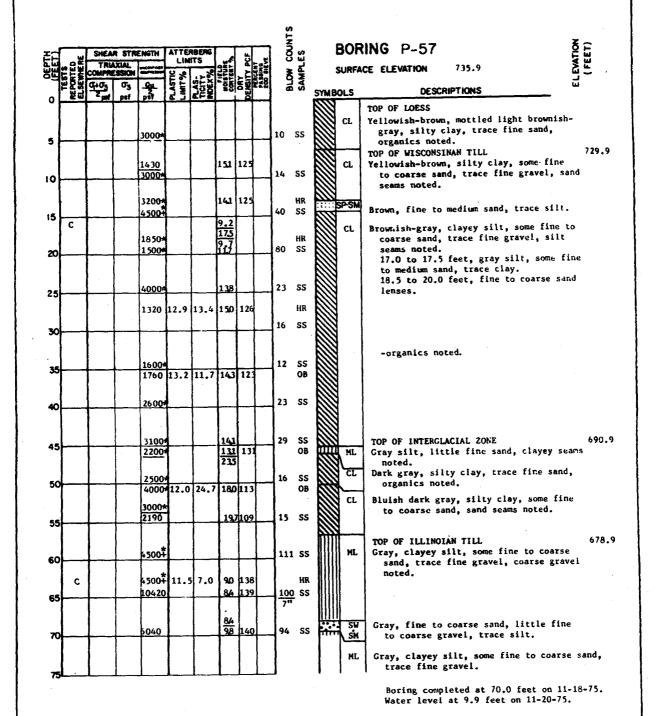
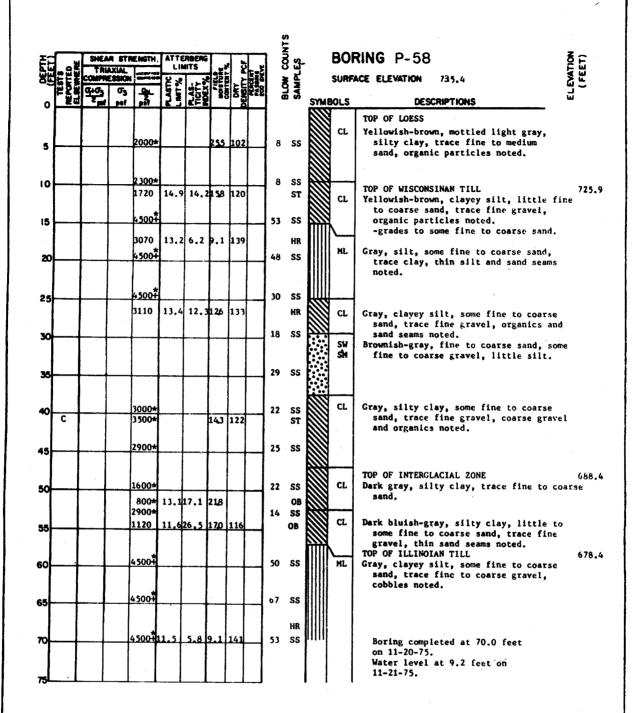


FIGURE 2.5-68

LOG OF BORING P-57

Logged by: Sargent & Lundy Engineers
Drilled by: Raymond International

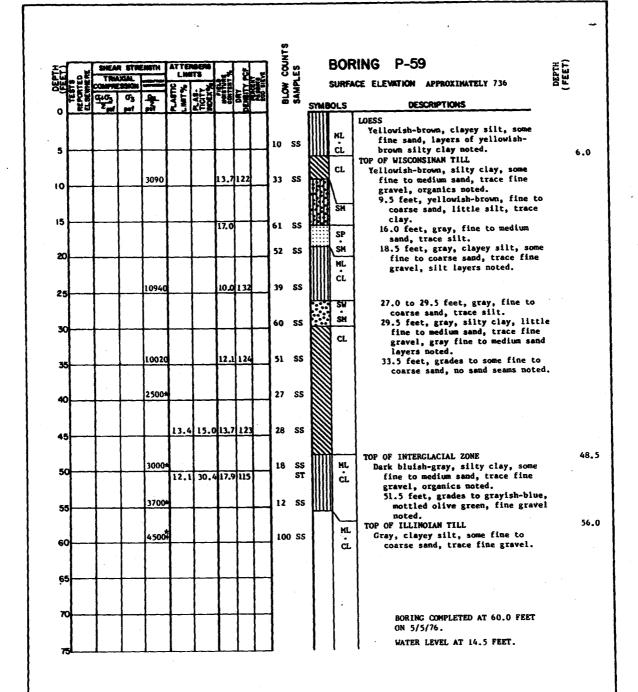
Tested by: Westenhoff & Novick, Inc.



Logged by: Sargent & Lundy Engineers
Drilled by: Raymond International
Tested by: Westenhoff & Novick, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

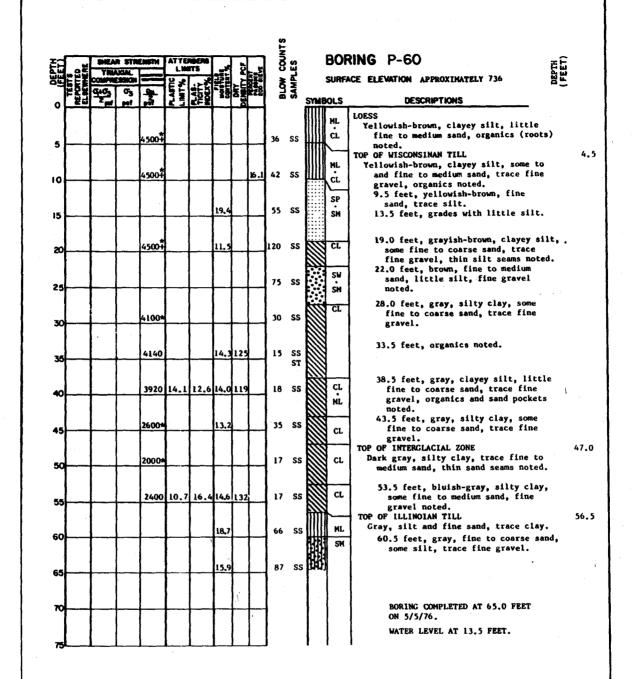
FIGURE 2.5-69



- 1. LOGGED BY: SARGENT & LUNDY.
  2. DRILLED BY: RAYMOND INTERNATIONAL.
  3. TESTED BY: WESTENHOFF & NOVICK.

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

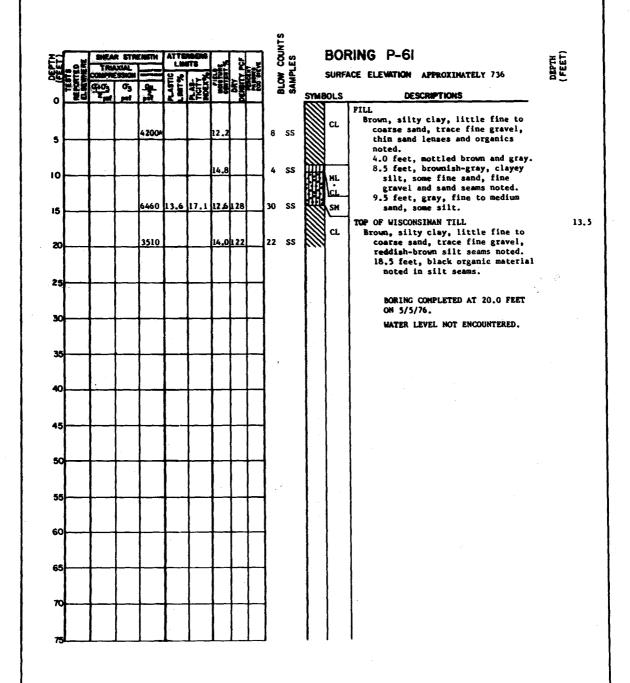
FIGURE 2.5-70



- 1. LOGGED BY: SARGENT & LUNDY.
  2. DRILLED BY: RAYMOND INTERNATIONAL.
  3. TESTED BY: WESTENHOFF & NOVICK.

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

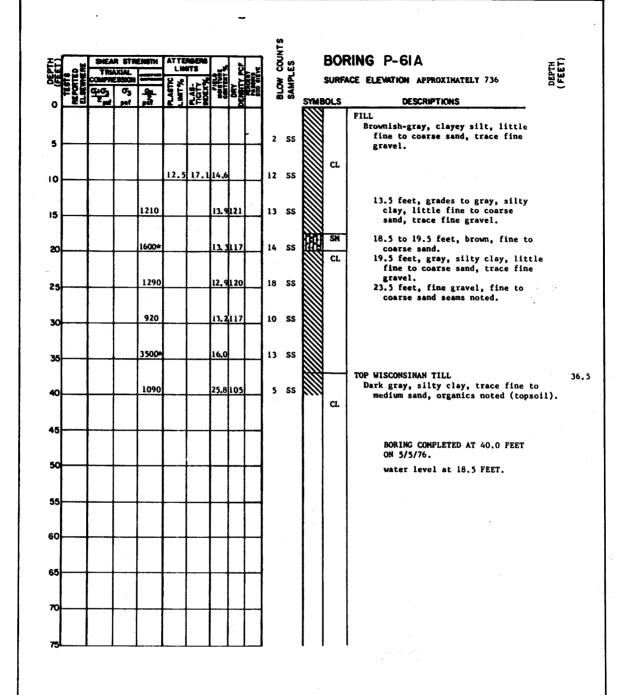
FIGURE 2.5-71



- 1. LOGGED BY: SARGENT & LUNDY.
  2. DRILLED BY: RAYMOND INTERNATIONAL.
  3. TESTED BY: WESTENHOFF & MOVICK.

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-72



- 1. LOGGED BY: SARGENT & LUNDY.
  2. DRILLED BY: RAYMOND INTERNATIONAL.
  3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-73

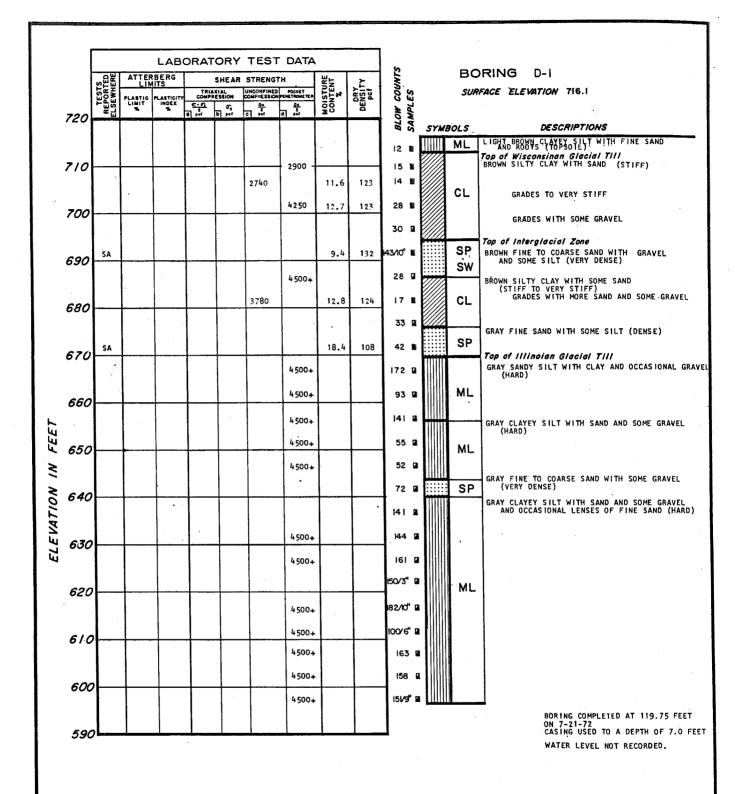


FIGURE 2.5-74

LOG OF BORING D-1

NOTE:

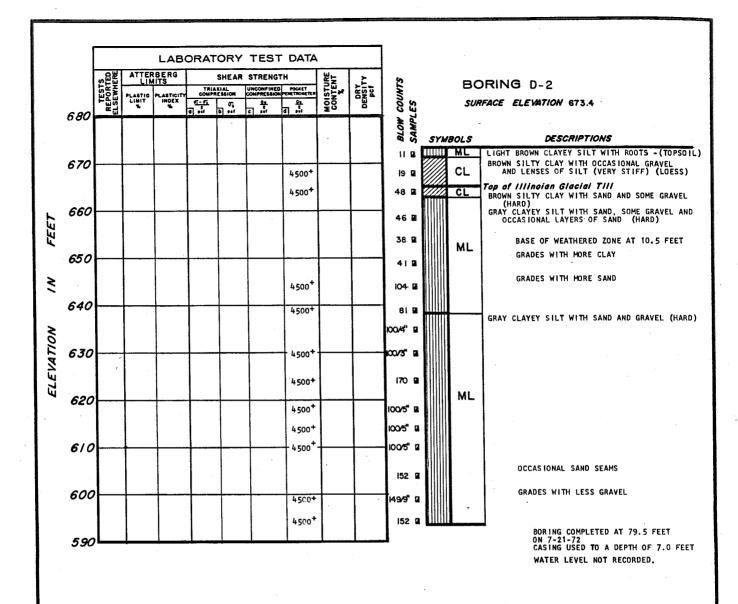


FIGURE 2.5-75

LOG OF BORING D-2

NOTE:

				LAB	ORAT	ORY	TEST	DATA	,		
		TS TED HERE	ATTE	RERG	TRIA		STRENGT		URE	<u>,                                    </u>	
	670	TES REPO! ELSEW	LIBIT	MASTICITY MOCK	COMPR	( SSION	CLUPRE SOUN	Sa.	MOISTURE CONTENT	E S S	BORING D-3
	670										्रिव
1	660										S SYMBOLS DESCRIPTIONS
FEE	860										TOP OF SELL CIEB ALLINIUM  SP BROWN CLAYEY SILT WITH ORGANIC MATTER - (TOPSOIL BROWN FINE TO HEDIUM SAND (VERY LOOSE)
•	CEO	SA	•				2175		20.1	104	7 B CL BROWN SILTY CLAY WITH TRACE OF VERY FINE SAND (MEDIUM STIFF)
\$	650	CHEH+							•		AND FINE TO COARSE SAND WITH SOME GRAVEL (VERY LODGE AND VERY SOFT)  SP OCCASIONAL LAYERS OF CLAYEY SILT WITH
*	640			ļ				ļ			SAND SAND BASE OF WEATHERED ZONE AT 17.0 FEET GRAY FINE TO CUARSE SAND WITH SOME GRAVEL
ELEVATION		·			10,200	3000			9.8	135	(DENSE)    12/KT
797	630	<u> </u>	ļ	ļ	ļ	ļ		4500 ⁺		<u> </u>	85/6 B ML
¥		PERM SA						4500+	7.5	144	85 B
	620			<u> </u>			<u> </u>	<u> </u>			1004 2
		•	P	IEZOMET 3/4 IN	ER INST	PIPE 1	ITH THE	0-7-72   ON 7-13   LOWER	NO.		BORING COMPLETED AT 40.0 FEET ON 7-13-72 CASING USED TO A DEPTH OF 20.0 FEET

WATER SAMPLE OBTAINED ON 10-7-72

PIEZOMETER INSTALLED IN D-3A ON 7-13-72

A 3/4 INCH PVC PIPE WITH THE LOWER END
PLUGGED AND THE LOWER 5 FEET PERFORATED WAS
PLACED AT ELEVATION 620.0 TO 630.0;

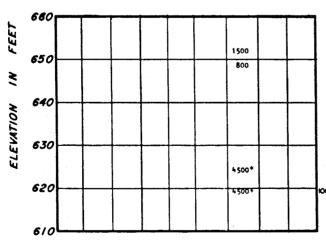
A BENTONITE SEAL FROM ELEVATION 630.0 TO
632.0;AND PEA GRAVEL AND CEMENT GROUT
FROM ELEVATION 632.0 TO 660.0.

PIEZOMETER INSTALLED IN 0-38 ON 7-13-72
BORING 0-38 WAS LOCATED 6 FEET FROM D-3A
AND WAS DRILLED TO A DEPTH OF 20.5 FEET.
A 3/4 INCH PVC PIPE WITH THE LOWER END
PLUGGED AND THE LOWER 5 FEET PERFORATED WAS
PLACED AT ELEVATION 639.5. 'PEA GRAVEL
WAS PLACED FROM ELEVATION 639.5 TO 649.5;
A BENTONITE STAL FROM ELEVATION 649.5
TO 651.5; AND PEA GRAVEL AND CEMENT GROUT
FROM ELEVATION 651.5 TO 660.0.
WATER LEVETS BEADINGS WATER LEVEL READINGS

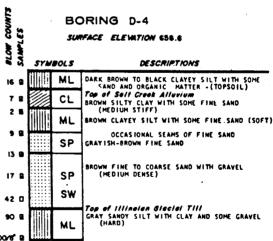
DEPTH BELOW GROUND SURFACE IN FEET

TIP ELEVATION 670.0 TIP ELEVATION 639.5 DATE 8-3-72 8-15-72 9-6-72

REFER TO FIGURE 2.4-37 FOR WATER LEVEL OBSERVATIONS.



BORING D-4



BORING COMPLETED AT 39.5 FEET ON 7-14-77 CASING USED TO A DEPTH OF 2C.0 FEET WATER LEVEL NOT RECORDED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-76

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS. LOG OF BORINGS D-3 AND D-4

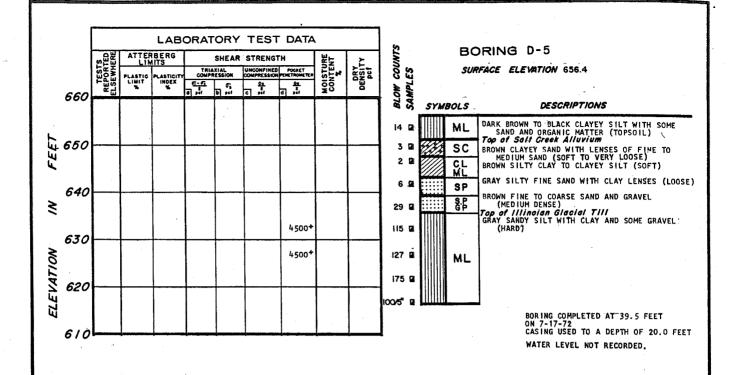


FIGURE 2.5-77

LOG OF BORING D-5

NOTE:

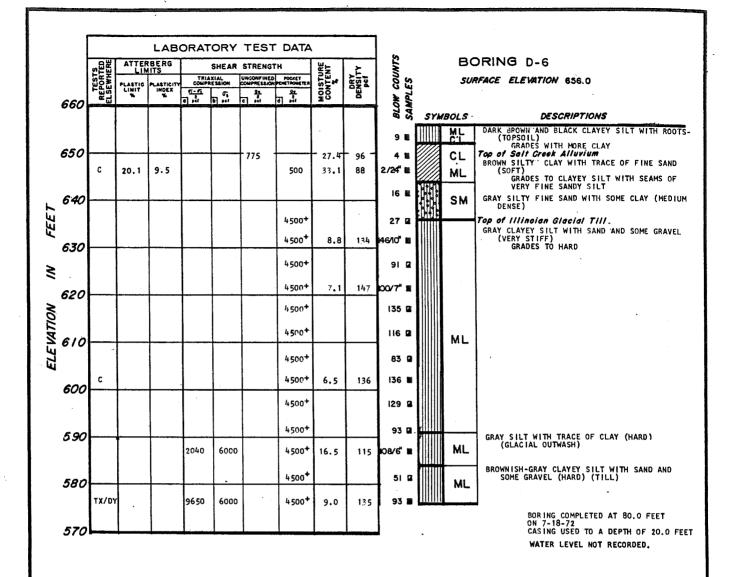


FIGURE 2.5-78

LOG OF BORING D-6

NOTE:

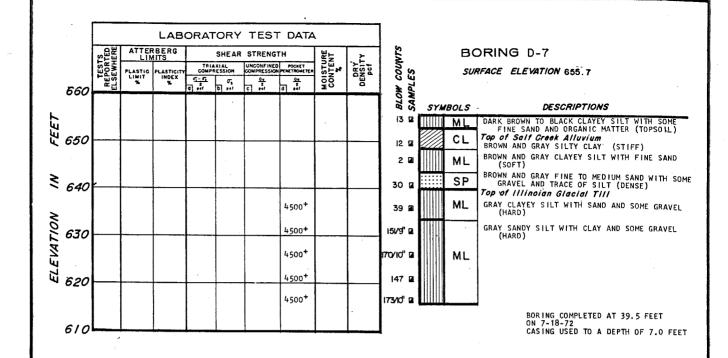
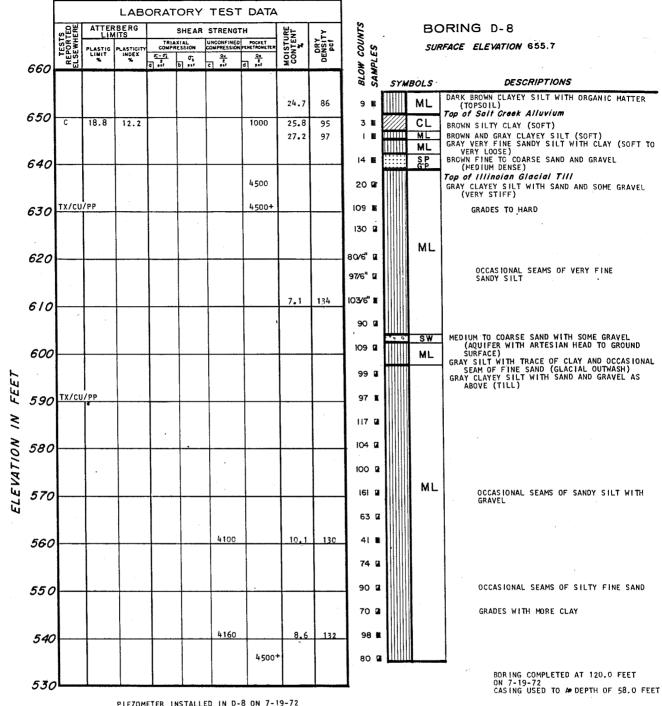


FIGURE 2.5-79

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.



PIEZOMETER INSTALLED IN D-8 ON 7-19-72
BORING D-8B WAS LOCATED ADJACENT TO
8A AND WAS DRILLED TO A DEPTH OF 16.0 FEET
A 3/4 INSC HOLD PIPE WITH THE LOWER END
PLUGGED AND THE LOWER 5 FEET PERFORATED
WAS PLACE AT ELEVATION 639.7. PEA
GRAVEL WAS PLACED FROM ELEVATION 639.7
TO 654.2;AND CEMENT GROUT FROM ELEVATION
654.2 TO 655.7.

#### WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET

9

6.2

B-3-72 8-15-72 9-6-72

REFER TO FIGURE 2.4-37 FOR WATER LEVEL OBSERVATIONS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-80

LOG OF BORING D-8

NOTE:

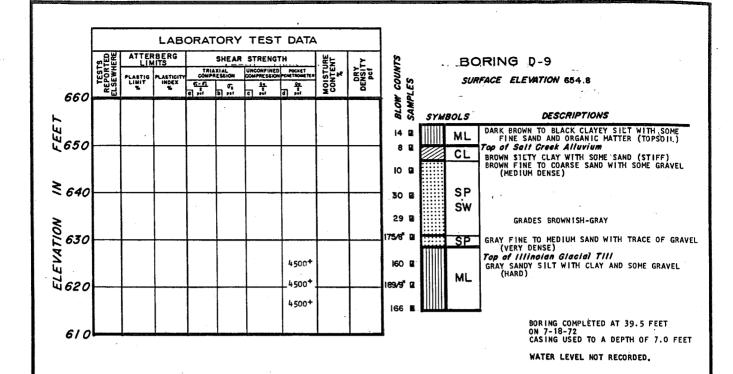


FIGURE 2.5-81

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

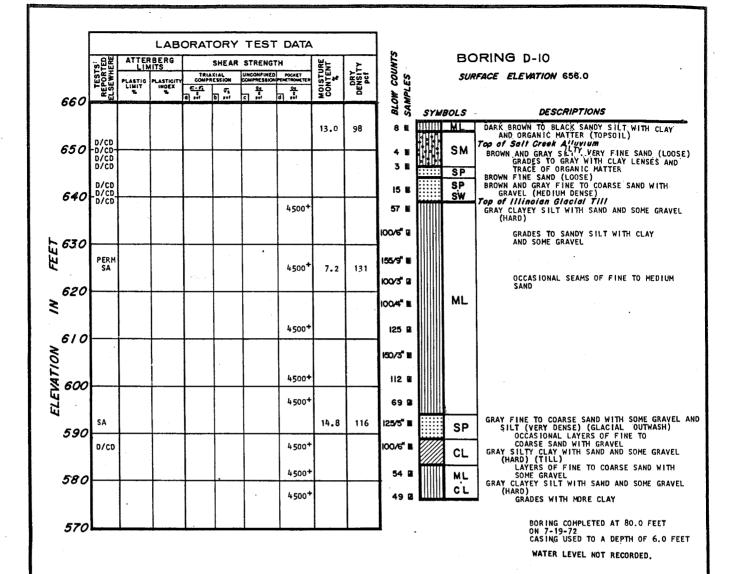


FIGURE 2.5-82

LOG OF BORING D-10

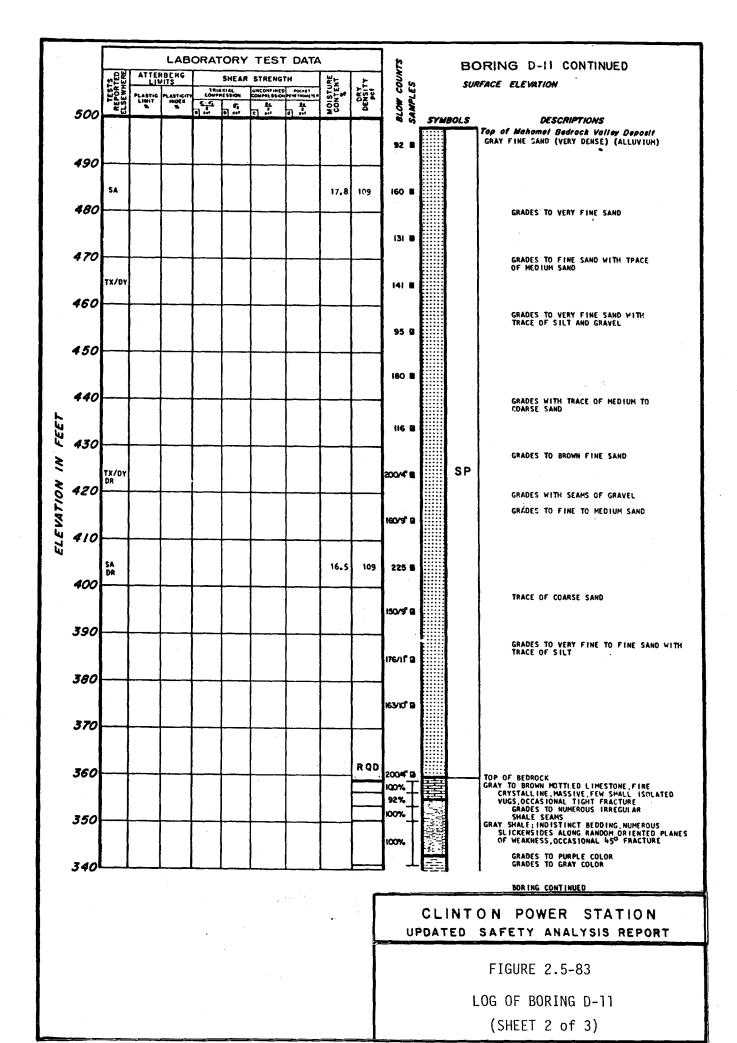
NOTE:

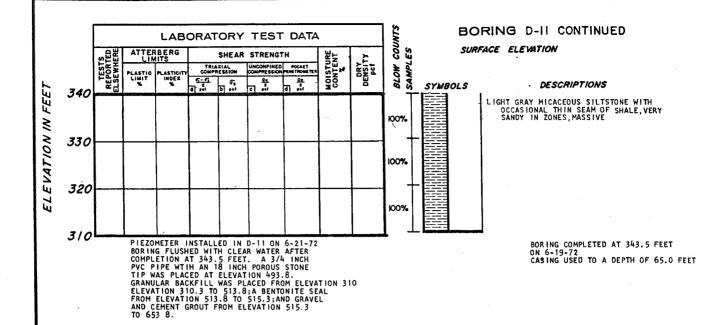
	~B#	ΑT	TER	BERG	-	SHEAR	STRENG	тн	H-	<u>-</u>	50	BORING D-II				
	TESTS REPORTED FI SFWHERE	PLA:	TIG	PLASTICITY INDEX	TRIA: COMPR			D POCKET NPENETROMETER	MOISTURE CONTENT	DENSITY pcf	BLOW COUNTS SAMPLES			RFACE ELEVATION 653.8		
660	- E	₩.		*	d per	<b>Б</b> ]	2 c 991	d 901	ΣO	٠	BLOW CO					
									-			_	BOLS	DESCRIPTIONS  DARK BROWN CLAYEY SILT WITH TRACE OF ORGA		
650		1_	_					1700	23.2		[		CL.	MATTER (TOPSOIL)		
								1500	25.2			191510	SM	Top of Sali Creek Alluvium  DARK GRAY AND BROWN SILTY CLAY WITH SOME FINE SAND (STIFF)		
											P■		SP SW	OCCASIONAL SAND SEAM GRADES TO MOTTLED DARK GRAY AND BROGRAY SILTY FINE SAND WITH POCKETS OF CLAY		
640		T						4500+	10.7	135	20 🗉		3 W	AND ORGANIC MATTER GRAY FINE TO COARSE SAND WITH GRAVEL (DEN		
								4500+			27 🕰			Top of Illinoian Giacial TIII GRAY CLAYEY SILT WITH SAND AND SOME GRAVE (HARD)		
630	D/CI	-						4500+	7.5	142	166 <b>E</b>			SEAM OF SAND		
									'''	142				6 INCH SEAM COARSE GRAVEL		
								4500*			107 🖬					
620		1			İ			4500**			170/10" E			·		
	1	1.						4500 <b>+</b>			100/6" 🛭			LAYER OF SAND AND GRAVEL		
610	TX/D	_			ļ	<u> </u>	ļ	4500+			144 ■			EATEN OF SAME AND GRAVES		
		'														
	_	1		-		ļ		4500+			82 12					
600	TX/C	Υ			16,900	400C		4500+	7.9	139	100 ■			LAYERS OF FINE TO COARSE SAND WITH GRAVEL		
								4500+			152/10" 🖬					
590	<b>/</b>	+		ļ	15,300	4000	ļ	4500+	8.4	137	115 W		S; P	18 INCH LAYER OF FINE TO MEDIUM SA (AQUIFER WITH ARTESIAN HEAD		
	1	1			13,300	1.4000			"."	13/			ML	TO GROUND SURFACE) 20 INCH LAYER OF GRAVEL		
								4500+			85 🖫					
580	十一							4500+			112 🖫			•		
	DR	İ						4500+			78 <b>D</b>					
570	2 0/9	-				-	+	4500+	<del> </del>	<u> </u>	94			6 INCH LAYER OF SAND		
	1									1				PIECES OF ORGANIC MATTER		
<i>E</i>								4500+			65 2			FIELES OF ORGANIC PATIEN		
560	厂							4500+			66 ■					
		ļ						4500+			_67 P		Ì			
550	o	+		<del> </del>	-	<del> </del>	-	4500+		<del> </del> -	69			3 INCH SEAM OF GRAVEL		
				Ì				4500+	1		96 🖫					
540	ا		-							<u></u>	1					
340								4500+			75 №					
		1						4500 ⁺		1	91 🗷					
53	0 -	+		1	+	+				+	-			3 INCH SEAM OF GRAVEL		
								l, coot	.		30 ■			GRADES WITH MORE CLAY		
<b>5</b> 2	0/0	D						4500+						GRADES WITH LESS CLAY		
52		T								1			1	Top Of Pre-Illinoian Glacial Till (ESTIMAT		
											62 🛚			GRADES WITH SEAMS OF CLAY GRADES TO MOTTLED GREEN AND GRAY		
51	0	+		+	-	-	+-	-	-	+	1					
								4500+	9.6	128	52			TRACE OF ORGANIC MATERIAL		
50	$_{o}$ L			<u> </u>								اللذارا	<u>.</u>	•		
-	-											CL	.INT	ON POWER STATION		

BORING CONTINUED

FIGURE 2.5-83

LOG OF BORING D-11 (SHEET 1 of 3)





WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET

DATE

6-29-72

33.5 PROBE LODGED AT 34.5 - NO WATER

REFER TO FIGURE 2.4-37 FOR WATER LEVEL OBSERVATIONS.

> CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

> > FIGURE 2.5-83

LOG OF BORING D-11

(SHEET 3 of 3)

NOTE:

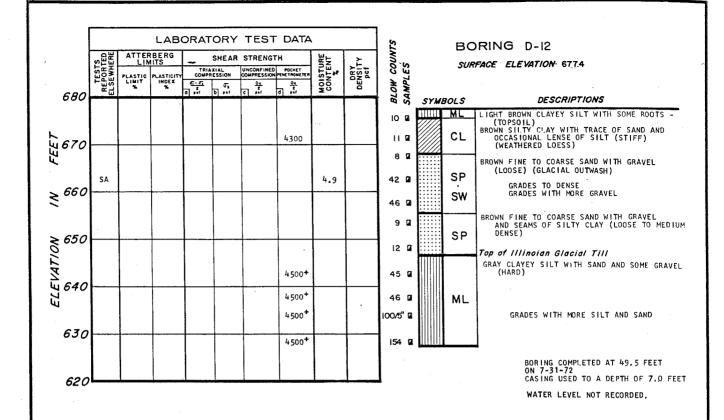


FIGURE 2.5-84

LOG OF BORING D-12

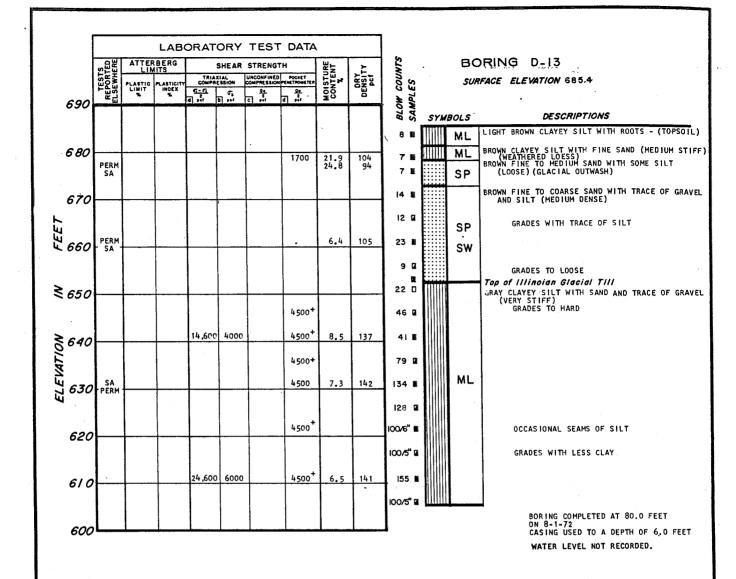
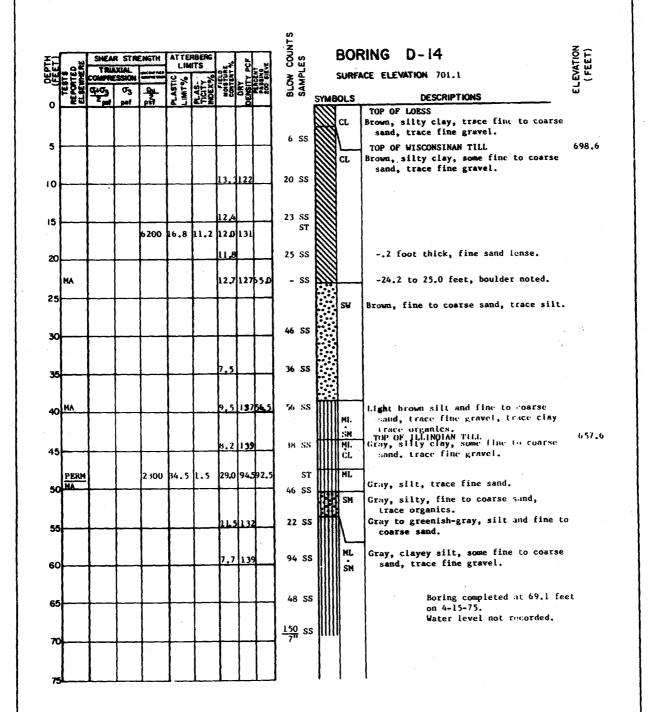


FIGURE 2.5-85

LOG OF BORING D-13



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-86

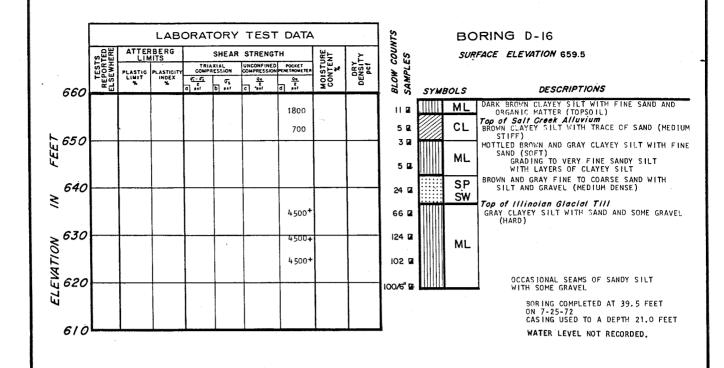
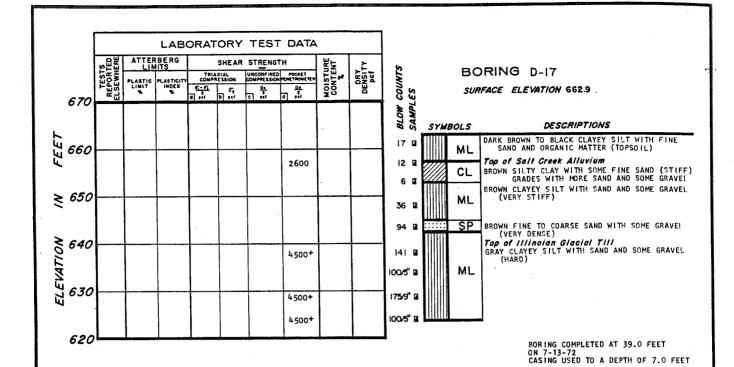


FIGURE 2.5-87

LOG OF BORING D-16

NOTE:



WATER LEVEL NOT RECORDED.

FIGURE 2.5-88

LOG OF BORING D-17

NOTE:

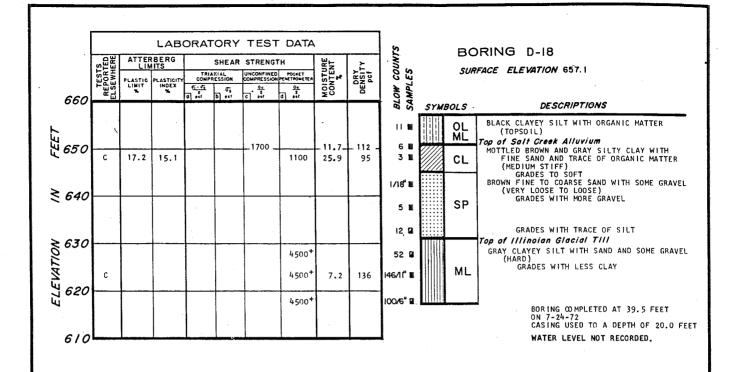
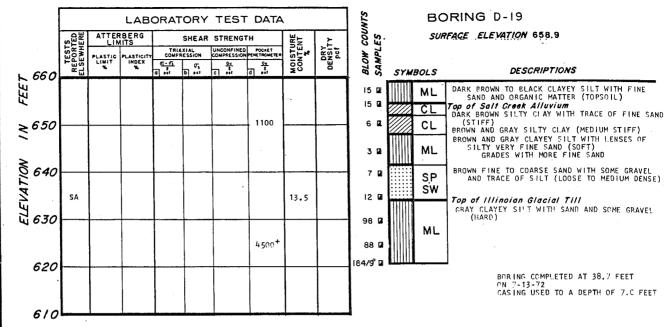


FIGURE 2.5-89

LOG OF BORING D-18

NOTE:



PIEZOMETERS INSTALLED IN D-19B ON 7-13-72
BORING D-19B WAS DRILLED TO 38.0 FEET
ADJACENT TO D-19A AND WAS FLUSHED WITH
CLEAN WATER AFTER COMPLETION. A 3/4
INCH PVC P1PE WITH A POROUS STONE TIP WAS
PLACED AT ELEVATION 620.9. PEA GRAVEL
WAS PLACED FROM ELEVATON 620.9 TO 625.9.
A BENTONITE SEAL FROM 625.9 TO 628.9,
AND PEA GRAVEL FROM 625.9 TO 630.9.
A 3/4 INCH PVC P1PE WITH A POROUS STONE TIP
WAS PLACE AT ELEVATION 630.9. PEA
GRAVEL WAS PLACED FROM ELEVATION 630.9
TO 635.9;A BENTONITE SEAL FROM ELEVATION
635.9 TO 637.1;AND GRAVEL FROM ELEVATION
637.1 TO 658.9.

WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET

TIP ELEVATION 620.9	TIP ELEVATION 630.9	DATE
8.9	8.8	8-8-72
9.7	10.0	8-22-72
10.5	10.5	9-6-72

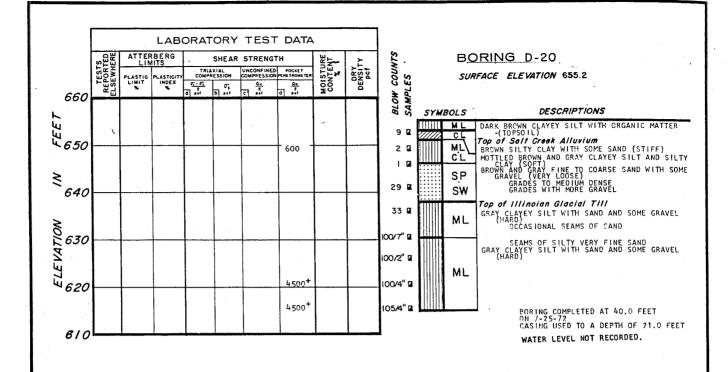
REFER TO FIGURE 2.4-37 FOR WATER LEVEL OBSERVATIONS.

> STATION CLINTON POWER UPDATED SAFETY ANALYSIS REPORT

> > FIGURE 2.5-90

LOG OF BORING D-19

NOTE:



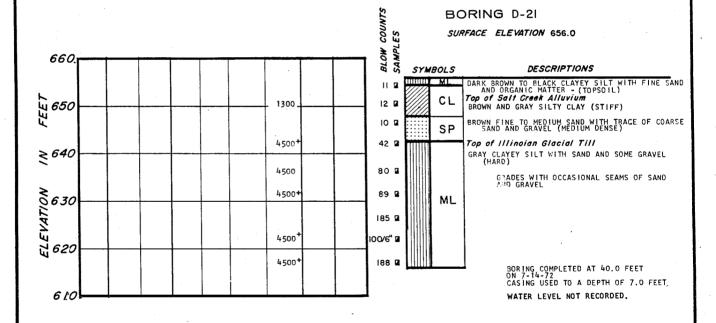
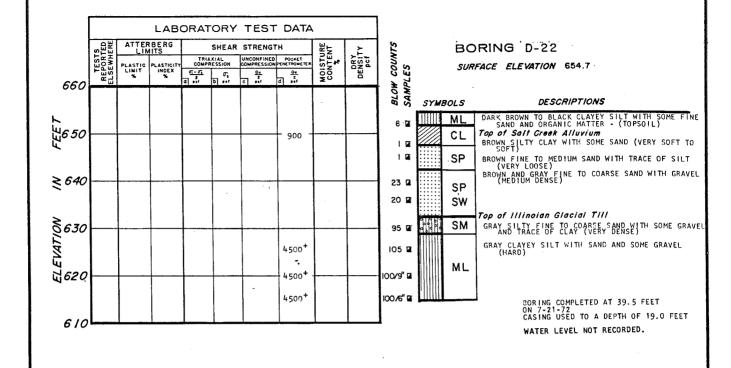
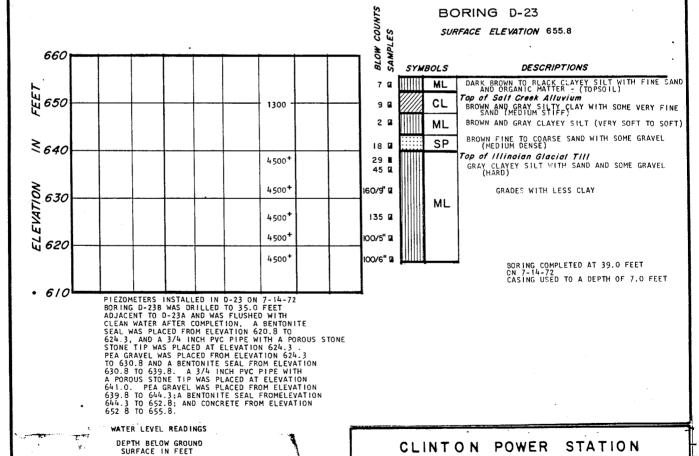


FIGURE 2.5-91

LOG OF BORINGS D-20 AND D-21

NOTE:





DATE

8-8-72 8-22-72 9-6-72

T-P ELEVATION 641.0

6.9 8.4 REFER TO FIGURE 2.4-37 FOR

WATER LEVEL OBSERVATIONS.

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

TIP ELEVATION 624.3

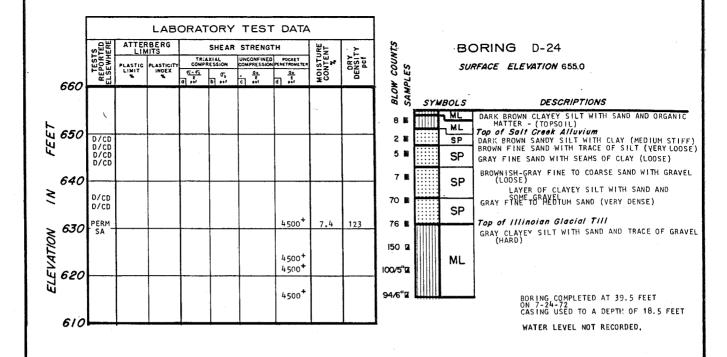
4.9 5.3 6.0

NOTE:

UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-92

LOG OF BORINGS D-22 AND D-23



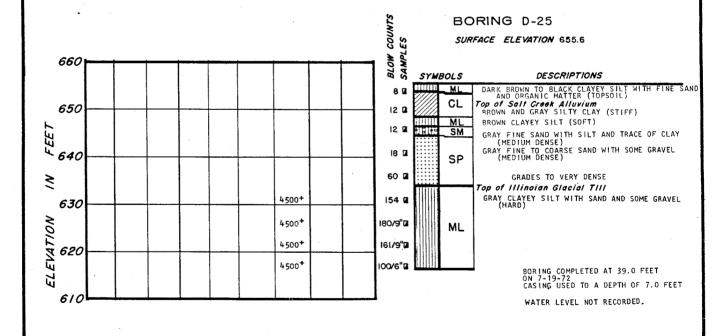


FIGURE 2.5-93

LOG OF BORINGS D-24 AND D-25

NOTE:

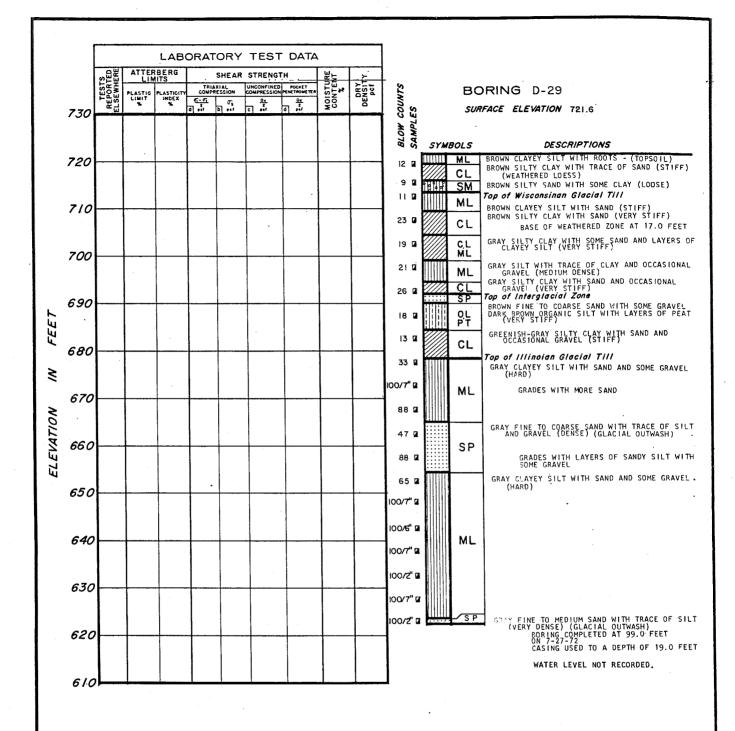
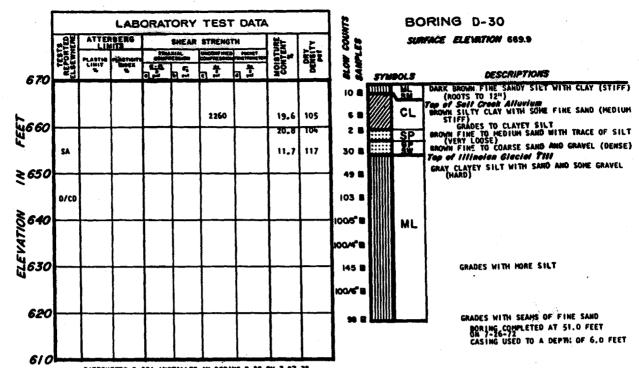


FIGURE 2.5-94

LOG OF BORING D-29

NOTE:



PIEZOMETER B-JOA INSTALLED IN BORING B-30 ON 7-27-72. A 3/4 INCH PVC PIPÉ VITH A POROUS STONE TIP WAS PLACED AT ELEVATION 620. PEA GRAVEL WAS PLACED FROM ELEVATION 620 TO 625. BENTONITE PLACED FROM 625 TO 627 AND CEMENT GROUT FROM ELEVATION 627 TO 669.9.

PIEZOMETER D-308 INSTALLED IN BORING D-308 (DRILLED ADJACENT TO D-30A) ON 7-27-72. A 3/4-INCH PVC PIPE WITH A POROUS STONE TIP MAS PLACED AT ELEVATION 658. PEA GRAVEL MAS PLACED FROM ELEVATION 658 TO 666.4 AND CEMENT GROUT FROM ELEVATION 666.4 TO 669.9.

PIEZOMETER D-30C INSTALLED IN BORING D-30C (ORILLED ADJACENT TO 8-50A) ON 8-3-72. 15 FEET OF 4-INCH CASING MAS USED TO SEAL OFF THE SALT CREEK ALLUVIUM. A 3/4-INCH PVC PIPE WITH A PORGUS STONE TIP WAS PLACED FROM ELEVATION 620, PEA GRAVEL WAS PLACED PROM ELEVATION 620 TO 625, A BENTOMITE SEAL FROM ELEVATION 625 TO 627 AND CEPENT GROUT FROM ELEVATION 627 TO 669.9.

#### WATER LEVEL READINGS

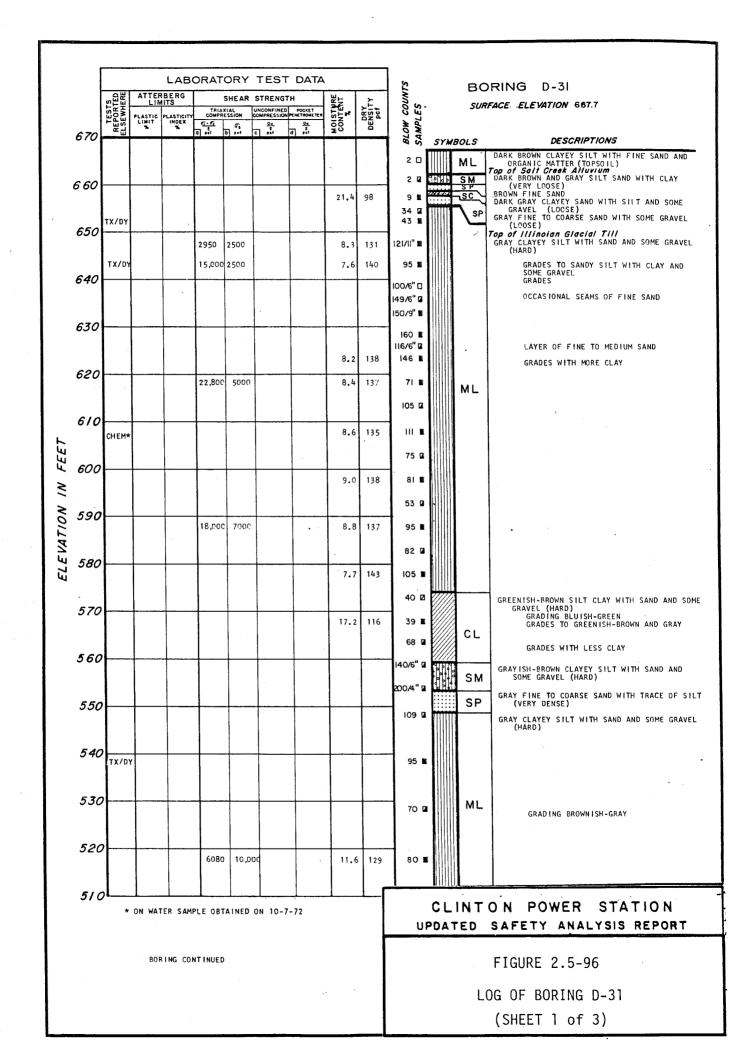
(DEPTH BELOW GROUND SURFACE IN FEET)

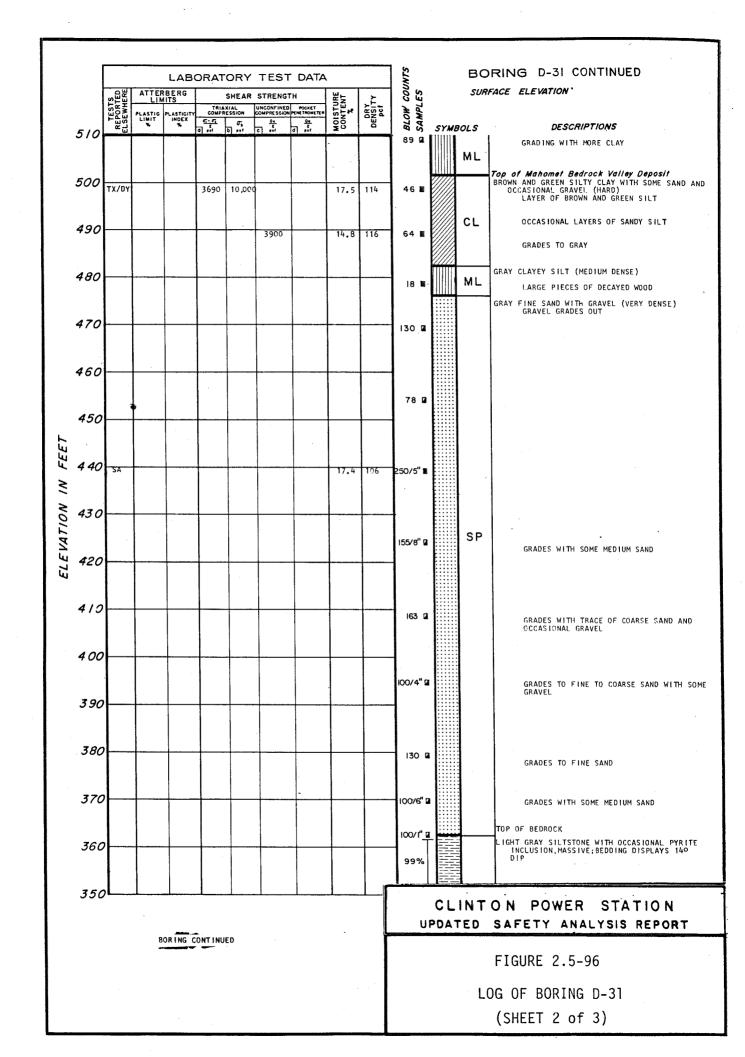
0-30A	<u>e-100</u>	D-30C	DATE
9.7	3.6	42.3	8-8-72
10.0	10.0	41.9	8-22-72
10.0	10.0	41.8	9-6-72

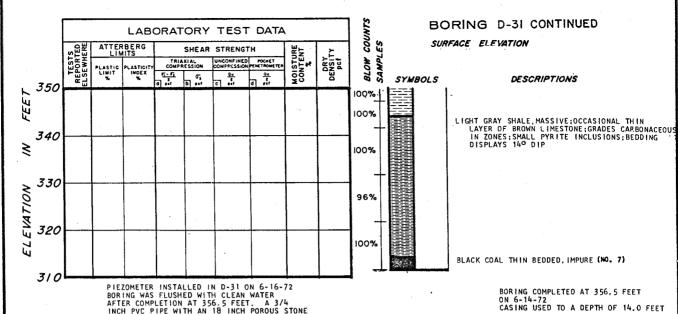
REFER TO FIGURE 2.4-37 FOR WATER LEVEL OBSERVATIONS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-95







PIEZOMETER INSTALLED IN D-31 ON 6-16-72
BORING WAS FLUSHED WITH CLEAN WATER
AFTER COMPLETION AT 356.5 FEET. A 3/4
INCH PVC PIPE WITH AN 18 INCH POROUS STONE
TIP WAS PLACED AT ELEVATION 461.7. GRANULAR
BACKFILL WAS PLACED FROM ELEVATION 311.2
TO 509.7 TO 511.2; AND CEMENT GROUT AND GRAVEL
FROM 511.2 TO 667.7.

WATER LEVEL READINGS

REFER TO FIGURE 2.4-37 FOR WATER LEVEL OBSERVATIONS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-96

LOG OF BORING D-31 (SHEET 3 of 3)

NOTE:

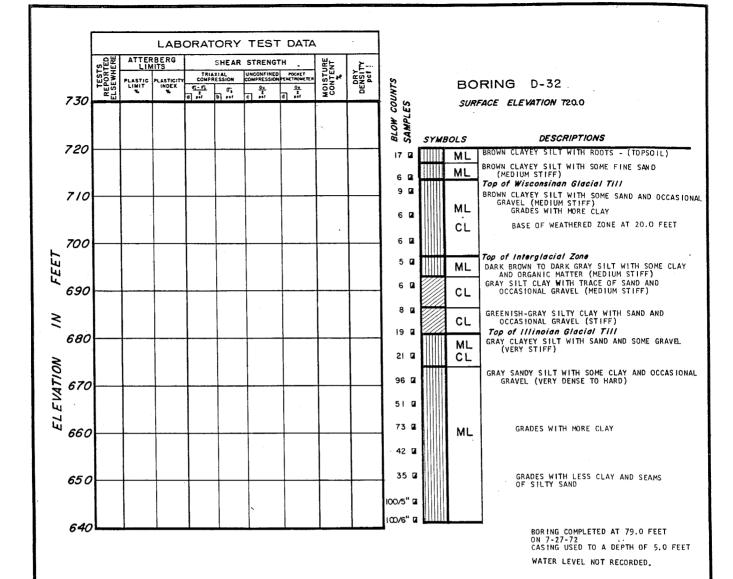


FIGURE 2.5-97

LOG OF BORING D-32

NOTE:

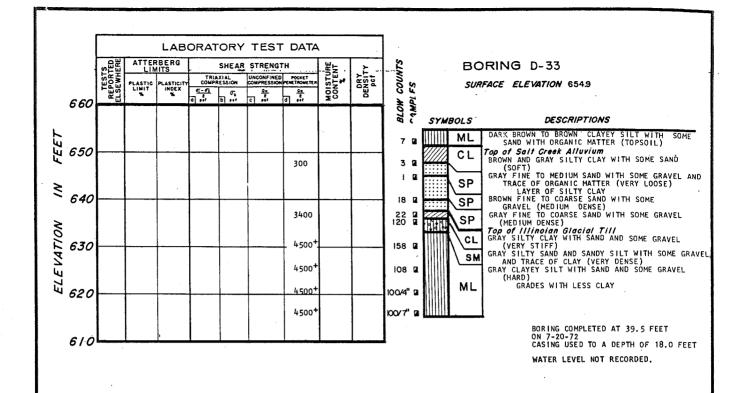


FIGURE 2.5-98

LOG OF BORING D-33

NOTE:

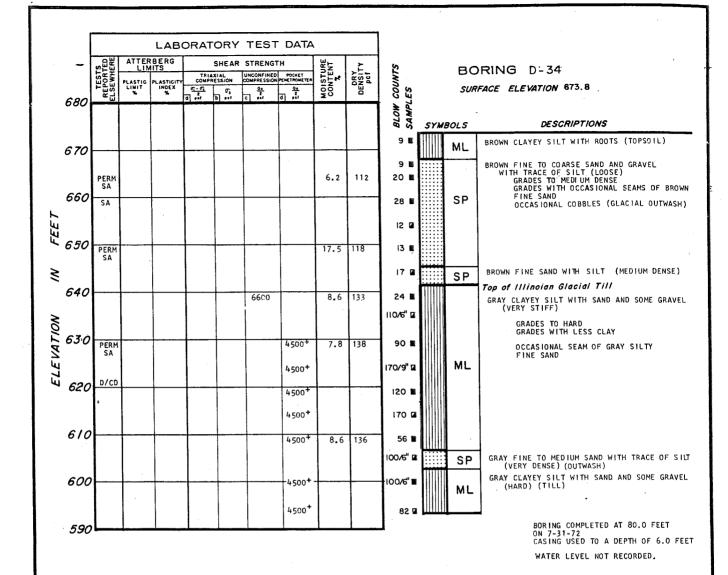


FIGURE 2.5-99

LOG OF BORING D-34

NOTE:

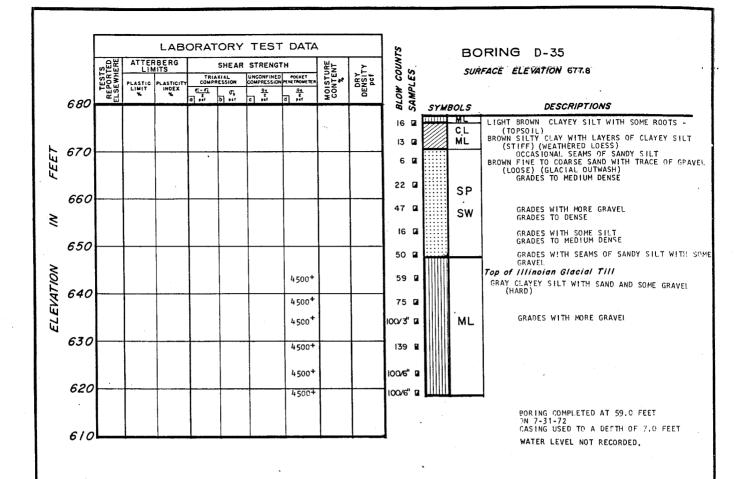


FIGURE 2.5-100

LOG OF BORING D-35

NOTE:

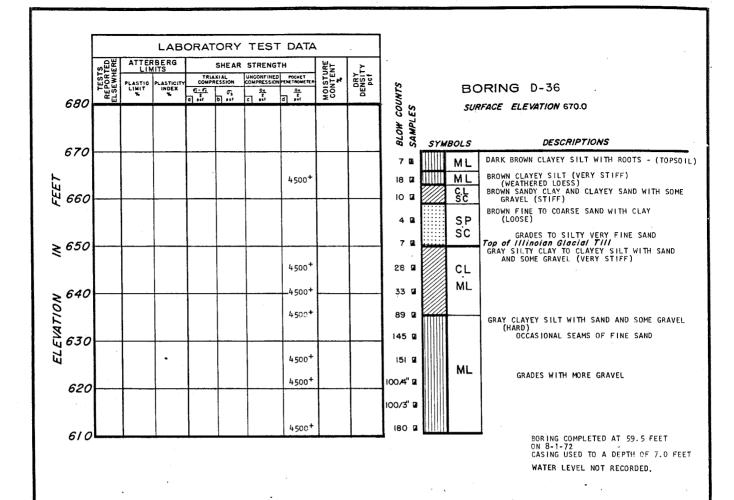


FIGURE 2.5-101

LOG OF BORING D-36

NOTE:

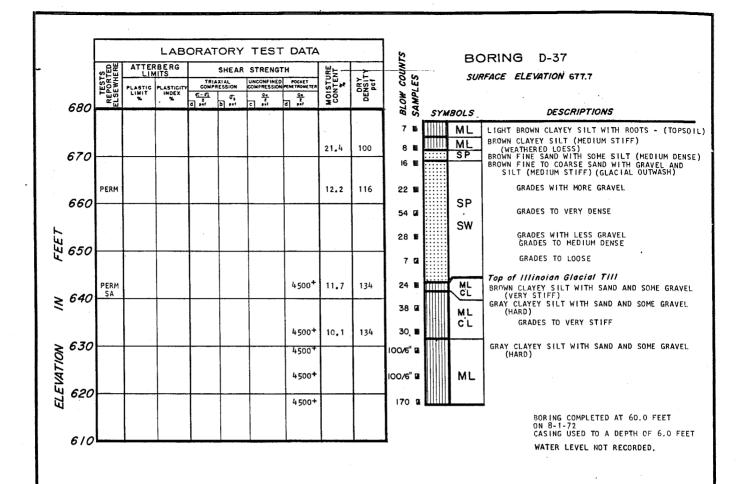


FIGURE 2.5-102

LOG OF BORING D-37

NOTE:

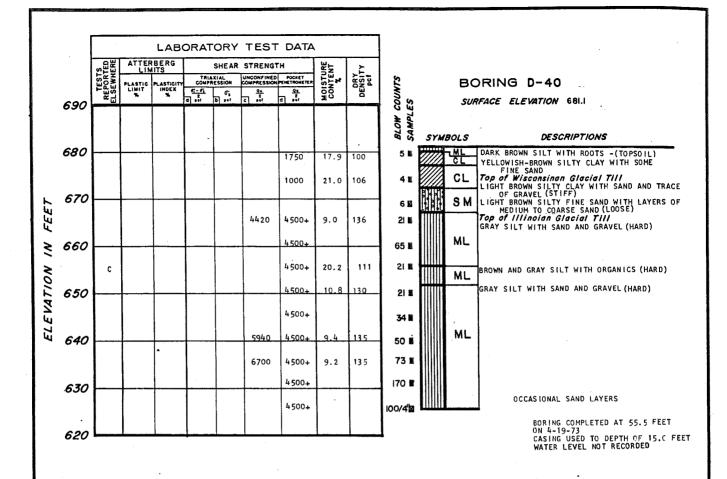


FIGURE 2.5-103

LOG OF BORING D-40

NOTE:

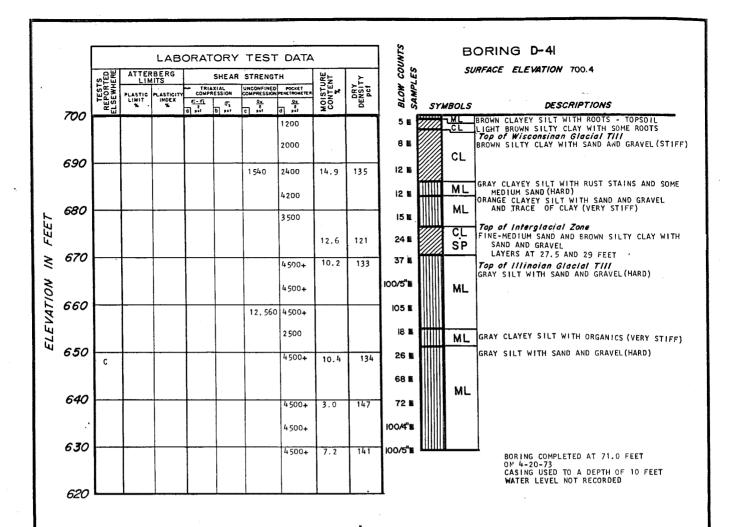


FIGURE 2.5-104

LOG OF BORING D-41

NOTE:

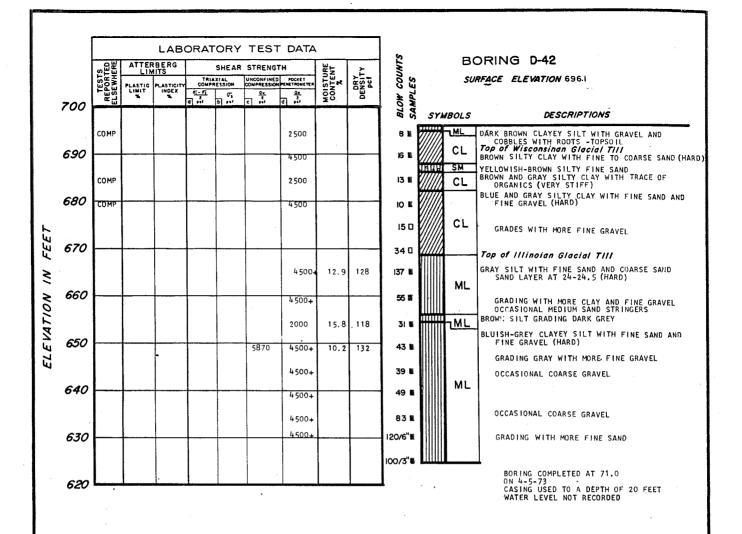


FIGURE 2.5-105

LOG OF BORING D-42

NOTE:

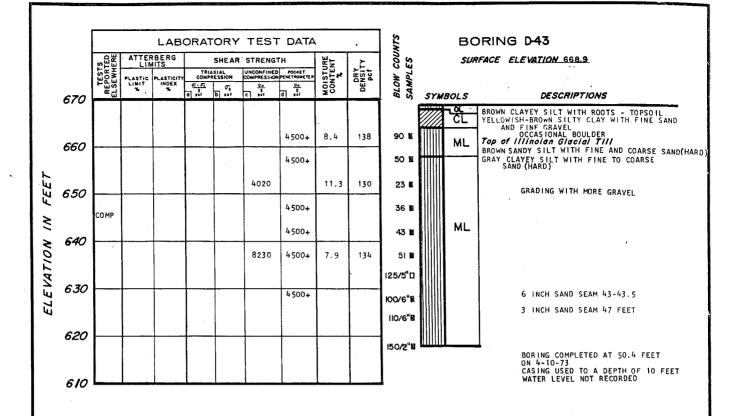


FIGURE 2.5-106

LOG OF BORING D-43

NOTE:

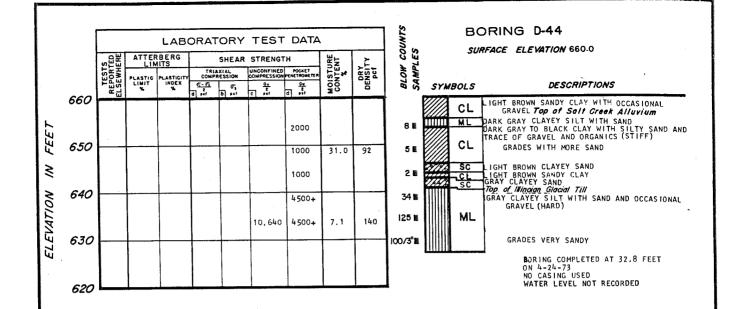
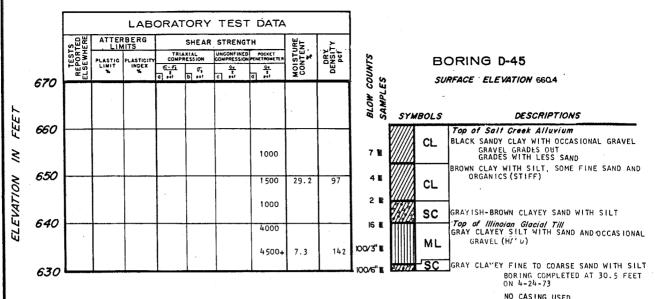


FIGURE 2.5-107

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.



NO CASING USED
WATER LEVEL NOT RECORDED

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-108

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

				LAB	ORAT	ORY	TES	T DATA	`					
		TESTS EPORTED SEWHERE	ATTER LIN	RBERG	791		STRENG		URE	,È,				
		REPOI ELSEW	PLASTIC LIMIT	PLASTICITY INDEX	G P	RESSION	COMPRESSIO	D POCKET NPENETROMETER Qu d per	MOISTURE CONTENT	DENSITY pcf	VTS		В	ORING D-46
72	20	— iu			0   per	D per	C 941	d pår		-	cou!		SU	RFACE ELEVATION 710.5
	i								,		BLOW COUNTS SAMPLES			
7.	10				<u> </u>				-		28 88	SYA	IBOLS	DESCRIPTIONS
								4000	20.6	106	7 • (			Top of Wisconsinan Glacial TIII BROWN SILTY CLAY WITH FINE TO COARSE SAND (VERY STIFF)
	١							3000			3 ■			OCCASIONAL GRAVEL, SOME STRINGERS
70	70					<del>                                     </del>		3000	14.6	119	ь.		CL	OF MEDIUM TO FINE SAND
					-		5580	1.500		101	l			GRADES TO HARD
69	0		<u> </u>			ļ	5580	4500+	12.5	124	24 🗉			Top of Interglacial Zone
	•	SA									150/6"		GW	YELLOWISH-BROWN FINE GRAVEL WITH SAND AND CLAY INCLUSIONS (VERY DENSE)
68	1								<u> </u>		24 🛭	,,,,,	SP	BROWN MEDIUM TO COARSE SAND WITH FINE GRAVEL DARK GRAY SILTY CLAY WITH OCCASIONAL
68	30				<u> </u>	1		2000	20.2	109	1		CL	BLACK ORGANIC SPECKS (VERY STIFF)
66	- 1				ł						13 8			PLUS AND COSSULA VALUE
67	20									ļ	] ~ ]		ML	BLUE AND GREEN CLAYEY SILT WITH SAND (STIFF)
٠.			.					4500+	11.0	132	23 🛮			GRAY SILT WITH SAND AND CRAYEL (HARD)
								4500+	8.1	136	63 ∎		ML	SOME SAND LAYERS
66	50				<del> </del>	<del></del>	<del> </del>	4500+		<u> </u>	100/3"■			OCCASIONAL GRAYISH-BROWN SILT LAYER
								4500+			100/4"#		ML	BROWN AND GRAY SILT WITH BLACK ORGANICS
65	50							7,5004			100/4			AND TRACE OF PIECES OF WOOD BROWN AND GRAY SILT WITH SAND AND GRAVEL (HARD
-								4500+	8.6	135	100/5"1			
	ı							4500+			50/1"1		ML	OCCASIONAL GRAVEL LAYER
64	10			1	1		+	4500+	7.6	139	100/5"■		İ	•
					ŀ									·
63	₂₀ L			]				4500+			100/5" 🗷			
0.	~ [							4500+			100/5"	41011		BORING COMPLETED AT 80.3 FEET ON 4-18-73
											1			CASING USED TO A DEPTH OF 10 FEET
62	20 L		<u> </u>	<u> </u>					<u></u>	<u> </u>	1			

A PIEZOMETER WAS INSTALLED IN 0-46A ON 4-24-73.
BORING D-46A WAS AUGERED TO 29.0 FEET ADJACENT
TO D-46. A 3/4 INCH CPVC PIPE WITH A CAPPED
TIP WAS PLACED AT ELEVATION 681.5. THE PIPE WAS
SLOTTED FROM ELEVATION 681.5 TO 701.5. PEA GRAVEL
WAS PLACED FROM ELEVATION 681.5 TO 708.5. A CONCRETE
SEAL WAS PLACED FROM ELEVATION 708.5 TO 710.5.

### WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET	DATE
18.6 18.2 18.8	4-25-73 4-27-73 4-30-73
20.0	6-12-73 7-3-73

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-109

LOG OF BORING D-46

NOTE:

			LAB	ORA	TORY	TEST	T DATA	•					
	TS TEO ERE	ATTER	RBERG			STRENG		URE	<u>,</u>	N/TS			ORING D-47
	TESTS REPORTED ELSEWHERE	PLASTIC LIMIT	PLASTICITY INDEX	σ <u>ι-σι</u> σ <u>ι-σι</u>	RESSION		POCKET PENETROMETER Ou 2 d par	MOISTURE CONTENT	BLOW COUNTS SAMPLES		SUR	RFACE ELEVATION 715.7	
720	<del>-</del> "			101 141	101 111		0, 11			BLO	SYM	BOLS.	DESCRIPTIONS
. 740										r		TML CL	BROWN CLAYEY SILT WITH ROOTS - TOPSOIL  Top of Wisconsinan Glacial Till  REDDISH-BROWN SILTY CLAY WITH FINE SAND
710										11 1		SP·SM	REDDISH-BROWN SILTY FINE TO MEDIUM SAND WITH SOME GRAVEL
						3150	4500+	14.1	121	25 ■		ML	(MOTTLED) BROWN CLAYEY SILT WITH SAND AND GRAVEL (HARD)
700							4500+			55 ■		ML	BROWN SILT WITH SAND AND GRAVEL AND SOME CLAY (VERY DENSE) Top of Interglacial Zone
	-									22 🛭		SP	REDDISH-BROWN FINE TO COARSE SAND WITH GRAVEL AND REDDISH-BROWN CLAY (MEDIUM DENSE) GRADING MORE SILTY CLAY
690	S:A					<del> </del>	3 500	14.0	122	33 ■		CL	GRAY CLAY WITH SAND AND GRAVEL (VERY STIFF)
E7										27 🛭		GP·SP	FINE GRAVEL WITH COARSE SAND
680 680	$\vdash$			T			3 500		<u> </u>	4 ⊠		CL	GREENISH-BLACK SILTY CLAY WITH TRACE OF FINE SAND (VERY STIFF)
<b>≷</b>							4500+			17 🛭		ML	BLUE AND GREEN CLAYEY SILT WITH SAND AND GRAVE (HARD)
≥ <i>670</i>			1	<del> </del>	1		4500+	7.3	131	100/4"			Top of Illinoian Glacial TIII GRAY SILT WITH SAND AND GRAVEL
NO11 660							4500+			100/5" <b>3</b>			(LAYERS OF SAND AND GRAVEL) (HARD) SAND AND GRAVEL LAYERS GRADE OUT
¥ 660	<u> </u>			+	-	+	4500+		<del> </del>	200/3"			SAND AND GRAVEL CATERS GRADE OUT
EL		ļ					4500+	8.3	133	104 ■		ML	LAYER OF BROWN SILT AT 60 FEET
650		<u> </u>		-	<del>                                     </del>	+	4500+			95.∎			
							4500+	5.2	144	100/5"			
640		-		+	-		4500+	7.7	139	45 ■			
							4500+	7.7	143	100/5"≣			
630			-	+	+		-		+	100/5"■			
620								7.3	143	100/6'≡		<u> </u>	BORING COMPLETED AT 91.0 FEET ON 4-11-73

A PIEZOMETER WAS INSTALLED IN D-47A ON 4-24-73.
BORING D-47A WAS AUGERED TO 38.0 FEET ADJACENT TO D-47. A 3/4 INCH CPVC PIPE WITH A CAPPED TIP WAS PLACED AT ELEVATION 677.7. THE PIPE WAS SLOTTED FROM ELEVATION 677.7 TO 707.7. PEA GRAVEL WAS PLACED FROM ELEVATION 677.7 TO 713.7. A CONCRETE SEAL WAS PLACED FROM ELEVATION 713.7 TO 715.7.

### WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET	DATE
13.3	4-25-73
14.3	4-27-73
14.4	4-30-73
18.1	6-12-73
18.5	7-3-73

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-110

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

	<u>~₩1</u>	ATTE			TORY			10.		BORING D-48				
	TESTS REPORTED ELSEWHERE		RBERG	TRI	SHEAR AXIAL PRESSION	STRENGT UNCONFINED COMPRESSION		MOISTURE CONTENT	DRY DENSITY Pcf	BLOW COUNTS SAMPLES			PRFACE ELEVATION 715.3	
	EPO SEW	PLASTIC LIMIT	PLASTICITY INDEX	<u> </u>	J Gi	<u>04</u>	04	SION I	DEN	BLOW CO.	•	30	The result of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of th	
20	~ 1			0 ,21	D	<b>ਹ •ਾਂ</b>	8) sir	30		LON				
				ļ						8 3	SY	MBOLS	DESCRIPTIONS	
							2200			6			BROWN CLAYEY SILT WITH ROOTS - TOPSOIL REDDISH-BROWN CLAY WITH SOME ROOTS	
710	TX/CU		<del> </del>	+-	+	<del> </del>	2000			10 ■			<i>Top of Wisconsinan Glacial Till</i> Brown Silty Clay with Sand Occasional Gravi	
	PP		Ì							a		CL	AND ORGANICS (HARD)	
	TX/CU	Ì					4500			26 ■		CL.	GRADING WITH MORE GRAVEL	
200				1	-		4500	12.6	124	23 🛢		1		
		l				4190	4000	12.4	124	21		<del> </del>	GRAY SILTY CLAY WITH SAND AND GRAVEL (HARD	
590						1 4150	1000	,		l 1		CL	STATE SEAT WITH SAME AND GRAVES (MAKE)	
90	TX/CU	1					4000			20 🖪		1	·	
	٦						4000			11 🖦	/////	ML	Top of Interglacial Zone	
80												1	DARK BROWN SILT WITH SOME ORGANICS, TRACE OF CLAY	
,,,,	l					1420	2250	22.6	104	9 🖪		CL	DARK GRAY SILTY CLAY, SOME SAND, GRAVEL, A ORGANICS (VERY STIFF)	
				-			2500	16.3	117	14 🗷		1	BLUE AND GREEN CLAYEY SILT WITH SAND AND C	
670		ļ		↓	<u> </u>		1.500		<u> </u>	89 🏻		ML	(VERY STIFF)	
							4500+		1			i	Top of Illinoian Glacial Till GRAY SILT WITH SAND AND GRAVEL (HARD)	
			1				4500+	7.7	143	100/5" 🛚		ML	,	
660	<u> </u>	<del> </del>	-	<del>.  </del>	-	<del> </del>	<del> </del>			100/5"12				
,						2 5 6 0	2750	17.0	117	47		ML	GREEN AND BROWN SILT, SOME CLAY AND BLACK	
	1									"" -			ORGANICS (VERY STIFF) GRADING TO GRAY WITH SOME FINE SAND	
<i>650</i>	<b> </b>	<del>                                     </del>	†	T	+	1	4500+	9.3	135	86 🖩			GRAY AND BROWN SILT WITH SAND AND GRAVEL, CLAY (HARD)	
							4500+			145 ■			GRADING TO GRAY	
640	L						7,000			l		ML		
U4U			1					7.3	143	123 🛭			t in the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second of the second	
							4500+	8.4	140	137 🖸				
630		<u> </u>		1_			<u> </u>	ļ	ļ					
-00			1				4500+			125/6"				
	1			1			4500+			80/2" 🛘	Ш		PORTING COMPLETED AT 00 4 FEET	
620		<u> </u>					1	<u> </u>	1	J			BORING COMPLETED AT 90.6 FEET ON 4-13-73 CASING USED TO A DEPTH OF 15 FEET	

A PIEZOMETER WAS INSTALLED IN D-48A ON 4-24-73.
BORING D-48A WAS AUGERED TO 39.0 FEET ADJACENT
TO D-48. A 3/4 INCH CPVC PIPE WITH A CAPPED
TIP WAS PLACED AT ELEVATION 676.3. THE PIPE WAS
SLOTTED FROM ELEVATION 676.3 TO 706.3. PEA GRAVEL
WAS PLACED FROM ELEVATION 676.3 TO 713.3. A CONCRETE
SEAL WAS PLACED FROM ELEVATION 713.3 TO 715.3.

#### WATER LEVEL READINGS

DEPTH BELOW GROUND	DATE
SURFACE IN FEET	
25.5	4-25-7
25.6	4-27-7
28.7	6-12-7
•••	7 2 72

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-111

LOG OF BORING D-48

NOTE:

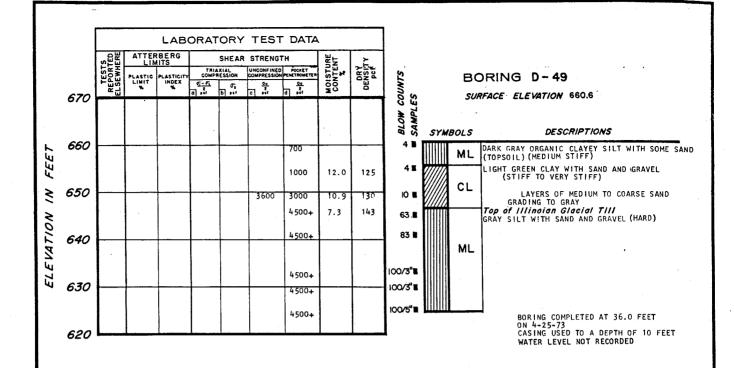


FIGURE 2.5-112

LOG OF BORING D-49

NOTE:

			LAB	ORAT	FORY	TES	T DATA			
	TESTS REPORTED SEWHERE	ATTEI LI	RBERG			STRENG		URE	<u>}</u>	
	TES EPOF	PLASTIC LIMIT	PLASTICITY INDEX	G-G	RESSION		D POCKET H PENETROMETER G gat	MOISTURE CONTENT	DENSITY pef	BORING D-50
730	<u> </u>			o ,ii	b	C 981	d ,	20	<u> </u>	BORING D-50 SURFACE ELEVATION 720.2
								,		SURFACE ELEVATION 720.2  NOTE: The surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the surface of the s
700										ซี รี SYMBOLS DESCRIPTIONS
720			-				2000			5 L CL BROWN SILTY CLAY, SOME FINE ROOTS (TOPSO
	Ī						1.500			90 SW OCCASIONAL SILTY CLAY Top of Wisconsinan Glacial Till
710		<u> </u>		<u> </u>			4500+			CL BROWN SILTY CLAY WITH SAND AND GRAVEL (H.
,,,										SW-SP SAND AND GRAVEL BROWN MEDIUM SAND
							4500+			BROWN CLAY, SOME SILTY SAND AND GRAVEL (
700	L			<u> </u>	ļ	ļ	1,5007		ļ	
							4000			28 GRADING TO GRAY
	1		1				4500+			40 SOME SAND AND GRAVEL LAYERS
690	<u> </u>		ļ	├	-		<del> </del>	<u> </u>	ļ	FINE TO COARSE SAND WITH SOME GRAVEL AND CLAY INCLUSIONS (DENSE)
	ŞA									39 1 000
	1						4000			GRAY CLAY WITH SAND AND GRAVEL (HARD)
680	-			<del>                                     </del>	+	-			ļ	
	1	١.		l			4500			29 T CL DARK GRAY CLAY, SOME SILT, TRACE OF SAN AND GRAVEL (HARD)
							3700			GRADING WITH MORE SAND AND GRAVEL  BLUE AND GREEN CLAY, SOME SILT, TRACE O
670		<b></b>	<del> </del>	<u> </u>	+-	<del> </del>	4500+	-	-	CL BLUE AND GREEN CLAY, SOME SILT, TRACE O AND GRAVEL (VERY STIFF) Top of Illinoian Glacial Till
*	1.								1	GRAY SILT WITH SAND AND GRAVEL, SOME CL
							4 500 4	-		140 ■       M L
660				1	1	1	45004			120/4" ■
							4000			IOO/5" ML BROWN SILT SOME GRAY CLAY LAYERS
650						1				SM GRAY SILTY FINE SAND GRAY SILT
0.00							2000			91 ML GRAY SILT WITH SAND AND GRAVEL
										BORING COMPLETED AT 71.5 FEET ON 4-30-73
640						1				CASING USED TO A DEPTH OF 20 FEE

A PIEZOMETER WAS INSTALLED IN D-50A ON 4-30-73.
BORING D-50A WAS AUGERED TO 37.0 FEET ADJACENT
TO D-50. A 3/4 INCH CPVC PIPE WITH A CAPPED TIP
WAS PLACED AT ELEVATION 683.2. THE PIPE WAS
SLOTTED FROM ELEVATION 683.2 TO 713.2. PEA GRAVEL
WAS PLACED FROM ELEVATION 783.2 TO 718.2. A CONCRETE
SEAL WAS PLACED FROM ELEVATION 718.2 TO 720.2.

WATER LEVEL READINGS

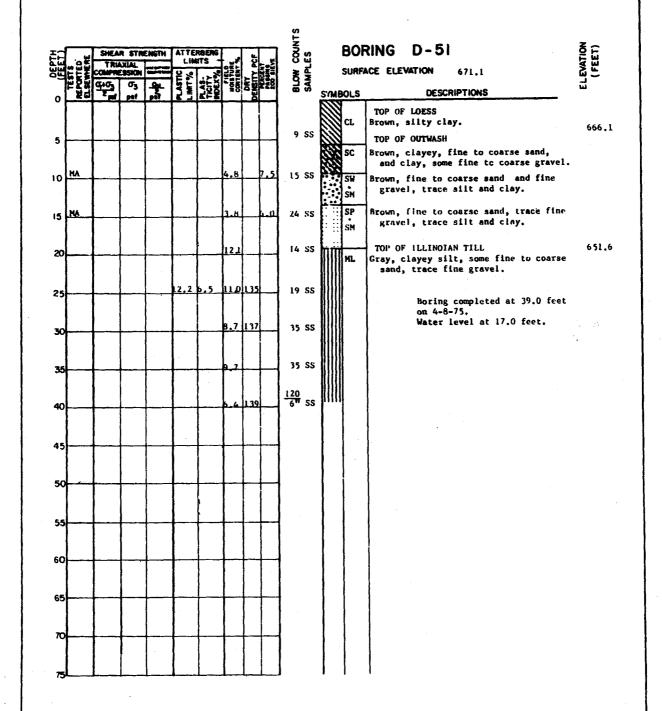
SURFACE IN FEET	DATE
20.0	4-30-73
20.3	6-12-73
20.8	7-3-73

CLINTON POWER STATION ... UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-113

LOG OF BORING D-50

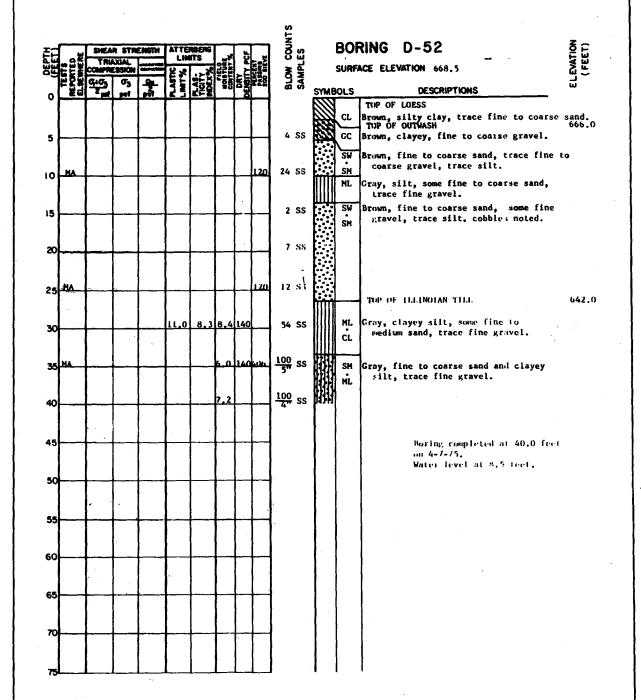
NOTE:



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

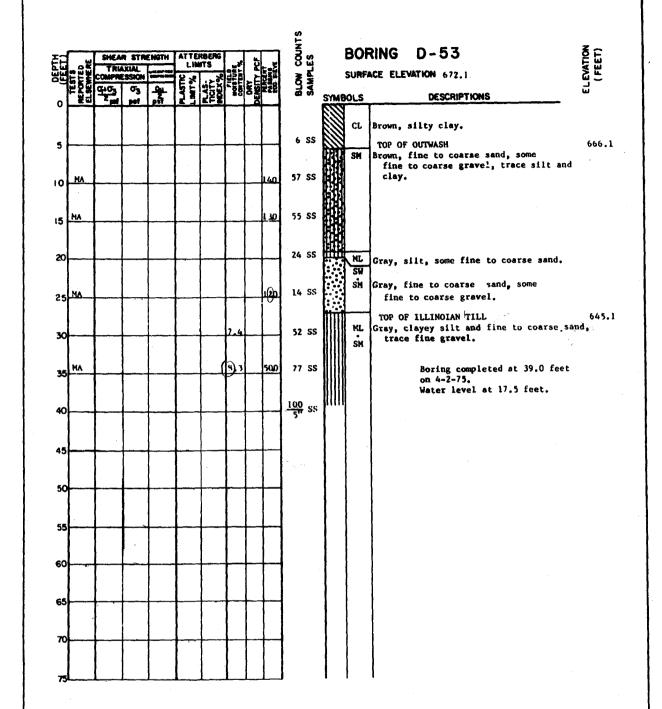
FIGURE 2.5-114



- 1. Lagged by: Surgent & Landy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

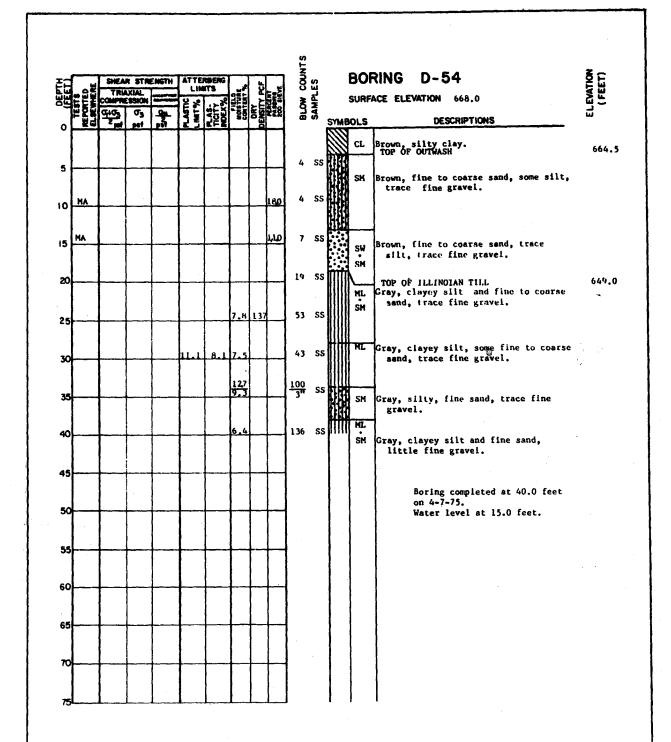
FIGURE 2.5-115



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

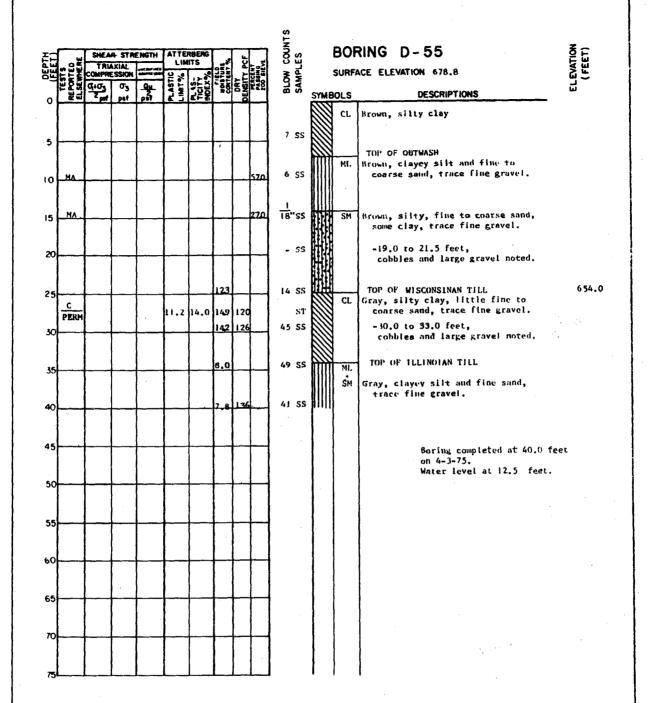
FIGURE 2.5-116



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

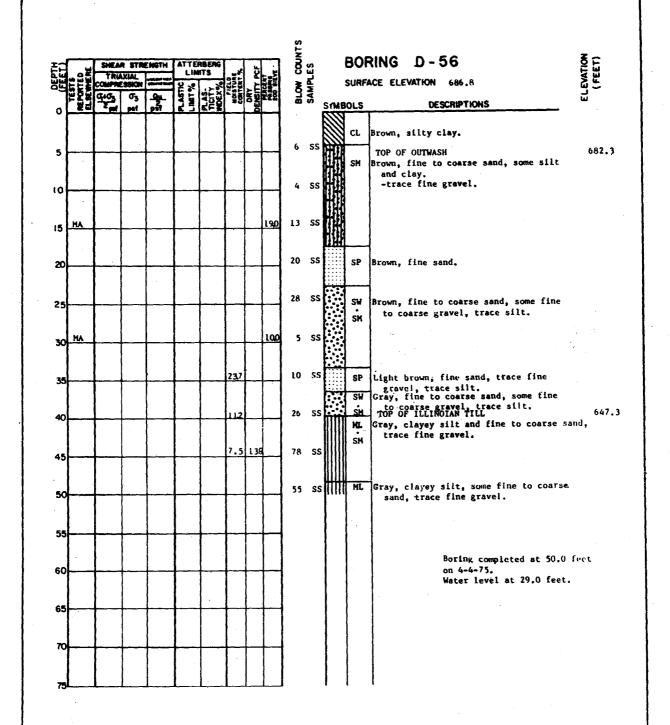
FIGURE 2.5-117



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tosted by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

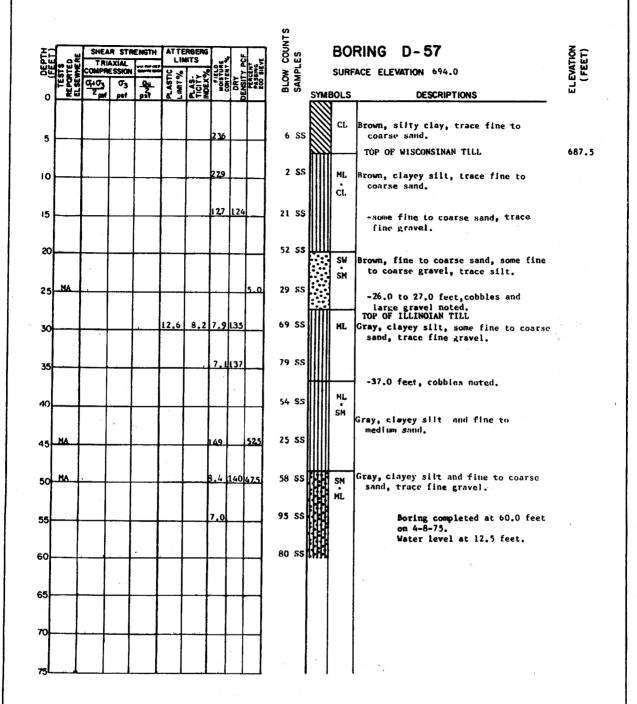
FIGURE 2.5-118



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-119



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

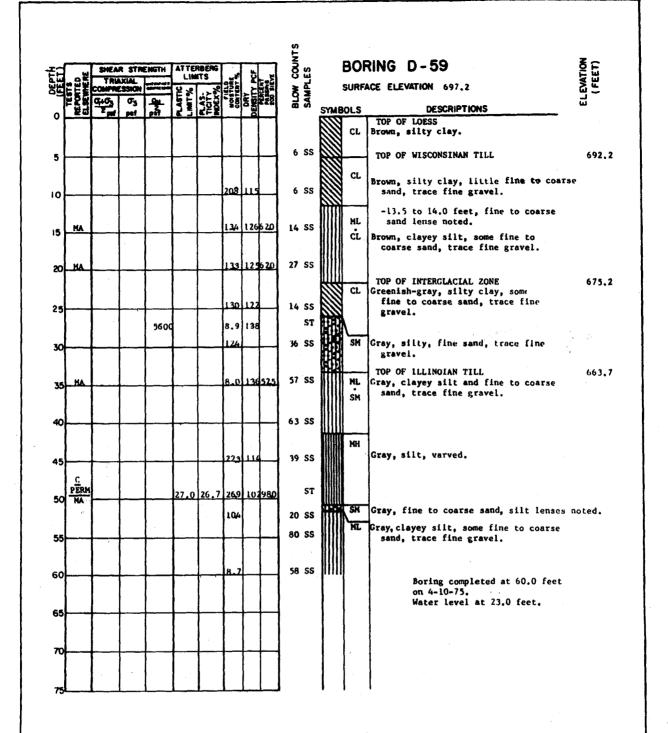
FIGURE 2.5-120

٦٧		R STR	ENGTH		RBERG UTS	.,	ÇF		중	ES	BORING D-58					
	COMPIL	SSION		Ë,	15.8	MOISTURE CONTENT	DRY ENSITY P		BLOW COUNTS	털	BORING D-58  SURFACE ELEVATION 695.7					
52	कुट	O'3	917 917	PLASTIC LIBIT %	POCTA S	28	0 S	125	. 6	S	SIMBOLS	DESCRIPTIONS				
											(M) c	TOP OF LOESS Brown, silty clay, trace fine sand.				
			1						6	SS		Stown, Strey Clay, Class Company				
												TOP OF OUTWASH 688.				
МА			<u> </u>	<u> </u>		207		255	4	SS	SI	Brown, silty, fine to coarse sand,				
												trace fine gravel. TOP OF WISCONSINAN TILL 682.				
,				<u> </u>	<u> </u>	227	115		20	SS		Brown, silty clay, trace fine to coarse				
									ı		<b>1</b> c	sand, trace fine gravel.				
			J	<u> </u>		218			9	SS		- 18.5 feet color grades to gray.				
				13.1 NP	14.2	148 108	124			ST		o 10.5 reet coror grases to gray.				
MA								140	15	ss	s	Reddish brown, fine to coarse sand, some				
												fine gravel, some silt and clay.  TOP OF ILLINOIAN TILL 669				
				11.9	7.1	11.7	143		33	SS	IIII M	Grav. clayey silt, trace fine to coarse				
			Π	Γ								sand, trace fine gravel.				
						103	136		79	SS						
						103			26	SS	н					
											s	trace fine gravel.				
MA						132		46.5	37	SS	S S	Gray, clayey silt and fine to medium				
											MI M	sand.				
MA						112	135	510	23	SS	M					
											s s	Greenish gray to gray, clayey silt				
						9.1	141		68	SS		and fine to coarse sand, trace fine gravel.				
												-53.3 feet color grades to gray.				
		<u> </u>		L		ιω			42	SS	MI	Gray, clayey silt, some fine to coarse				
												sand, trace fine gravel.  Boring completed at 60.0 feet				
						L	L					on 4-7-75. Water level at 6.5 feet.				
												water least at o. ) teat.				
				L												
						1										

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

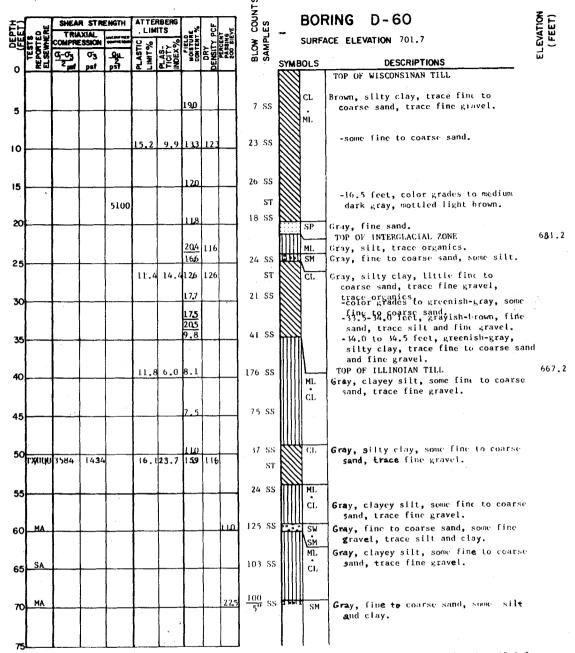
FIGURE 2.5-121



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-122



Boring completed at 69.0 feet on 4-9-75. Water level at 23.5 feet.

#### NOTES

- I. Logged by: Sargent & Lundy Englneers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

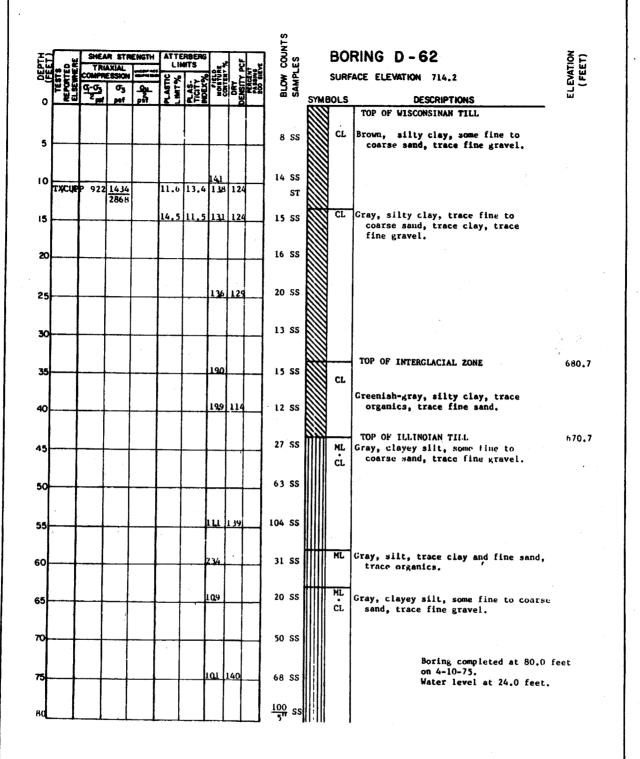
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-123

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

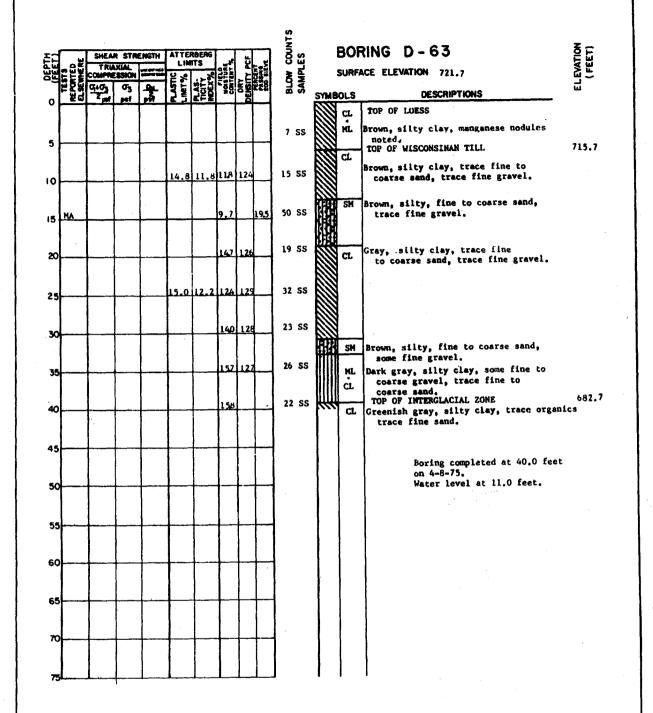
FIGURE 2.5-124



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

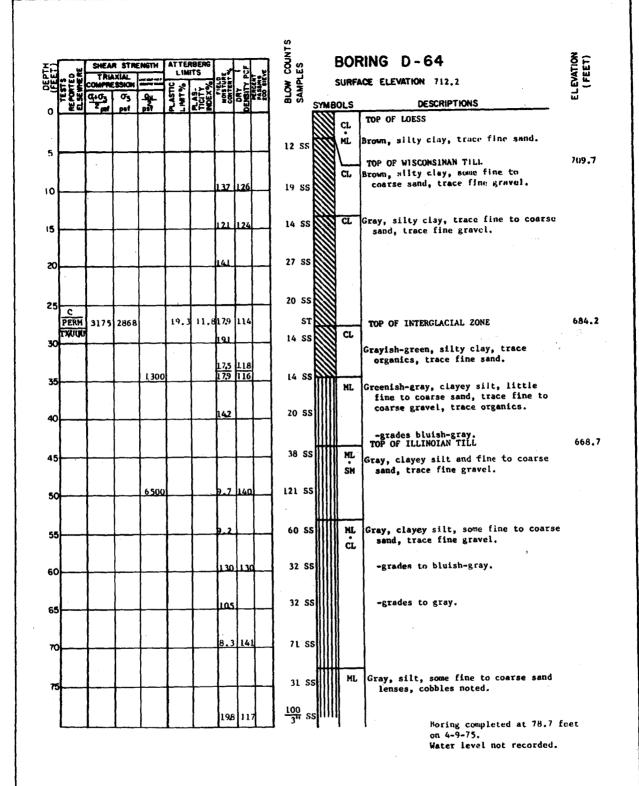
FIGURE 2.5-125



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## GLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

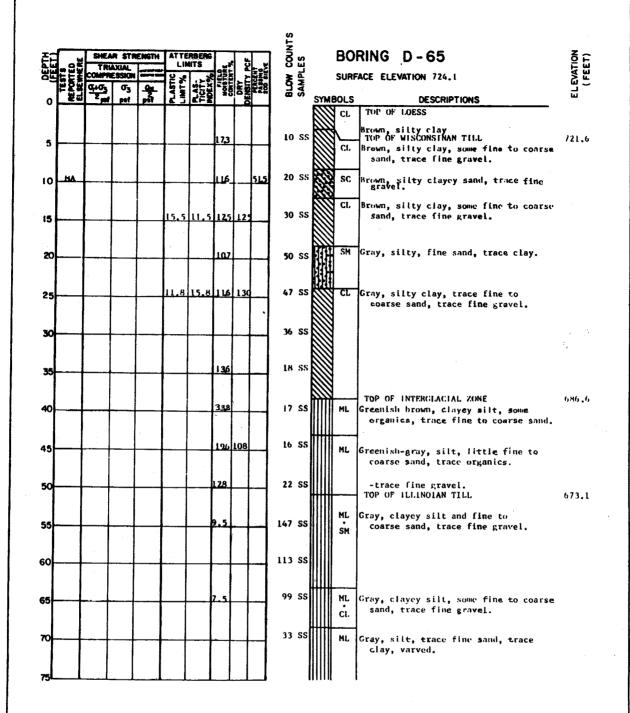
FIGURE 2.5-126



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-127



BORING CONTINUED

#### NOTES

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-128

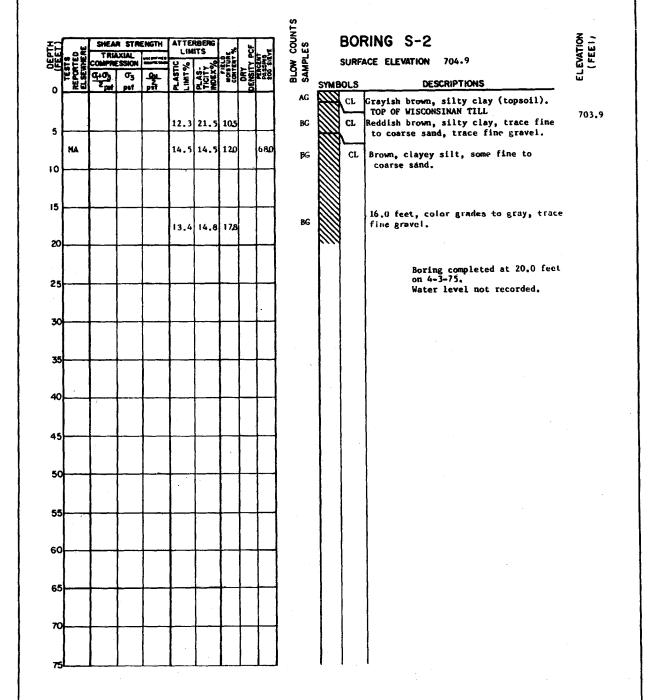
LOG OF BORING D-65

(SHEET 1 of 2)

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

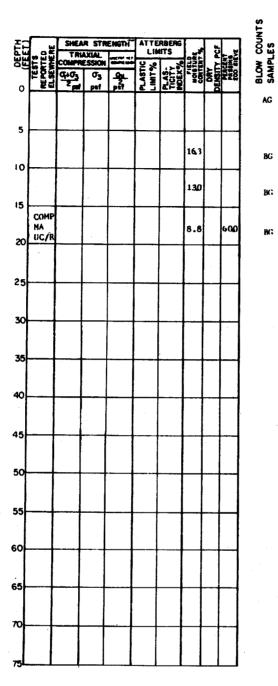
FIGURE 2.5-128
LOG OF BORING D-65
(SHEET 2 of 2)



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-129



## BORING S-3

SURFACE ELEVATION 725.0

ELEVATION

SYMBOLS	DESCRIPTIONS	
ML	TOP OF LOESS  Brown, clayey silt, manganese nodules	
ШШ	noted.	
ML	Yellow, silt, trace fine sand and clay.	
	TOP OF WISCONSINAN TILL	714.0
Cr Cr	Brown, silty clay, little fine to coarse sand.	714.0
G.	Gray, clayey silt, some fine to coarse sand, trace fine gravel.	

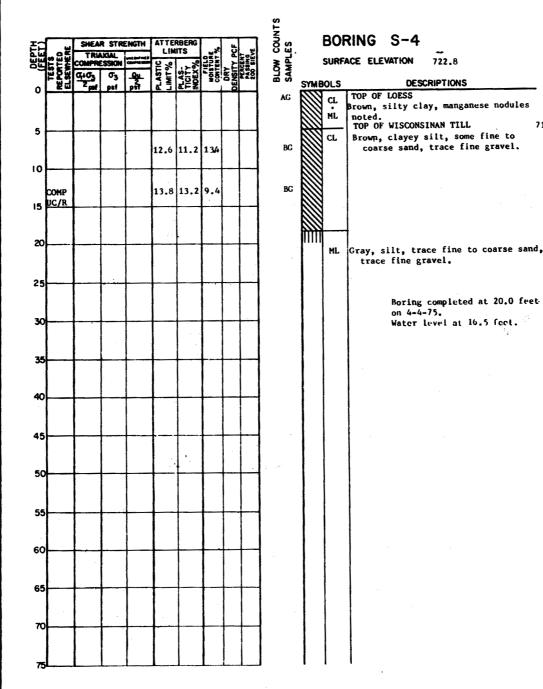
Boring completed at 20.0 feet on 4-3-75. Water level at 11.0 feet.

#### NOTES

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-130



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

718.3

FIGURE 2.5-131

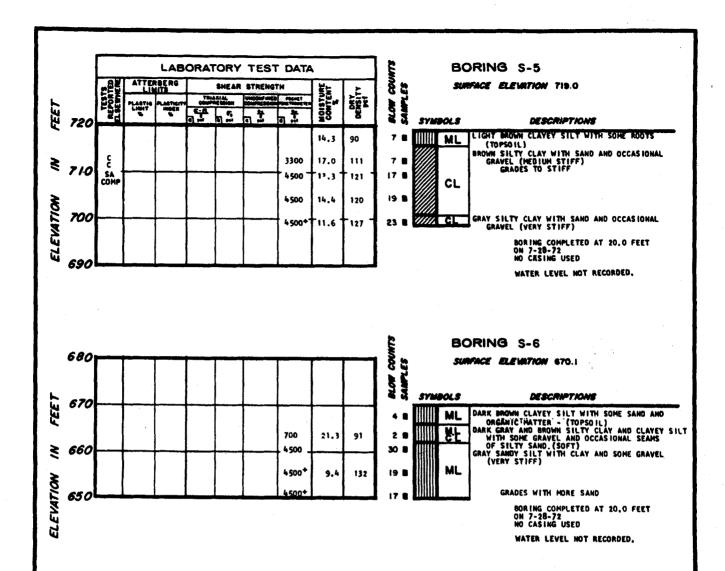
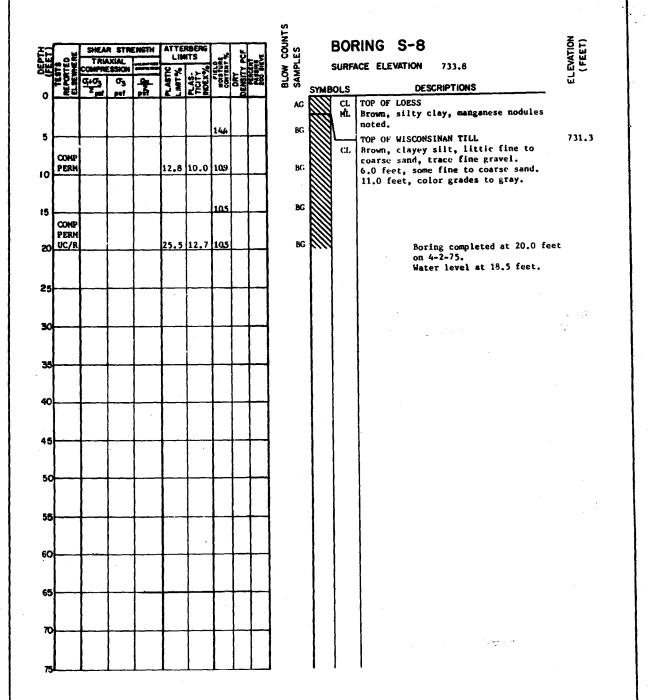


FIGURE 2.5-132

LOG OF BORINGS S-5 AND S-6

### HOTES

- 1. LOGGED BY: DAMES & MOORE
- 2. DRILLED BY: RAYMOND INTERNATIONAL
- 3. TESTED BY: DAMES & HOORE



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

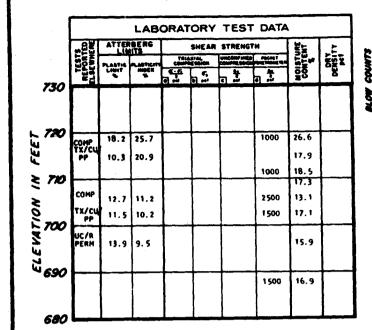
FIGURE 2.5-133

	_0E	ATTE	RERG			STREM	<b>GTH</b>	불눈	E	BORING S-9 Summace Elevation 717.2
	TEST REPORTELSEWN	PLASTIC LIMIT	PLASTICITY		OF DOLON	- T	O PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY OF THE PERSONAL PROPERTY O	MOISTURE CONTENT	DESE	
720										S SYMBOLS DESCRIPTIONS
								22.2		Top of Wisconsinen Glacial Till BROWN SILTY CLAY (HARD)
710	COMP UC/R	13.1	9.3				4500+	10.8		GRADES WITH GRAVEL GRADES TO VERY STIFF
							3000	13.6		GRAVEL GRADES OUT
700							3500	13.4		CL GRADES WITH SAND
				•	1			l		GRADES TO HARD
690	COMP	13.5	10.7	↓	ļ	<b>↓</b>		11.1	╄	GRADES TO GRAY AND WITH GRAVEL
							4500+	11.4		•
680	<u> </u>			_	_	-		11.1	<u> </u>	•
								12.1		BORING COMPLETED AT 40.0 FEE ON 3-19-73 NO CASING USED

FIGURE 2.5-134

LOG OF BORING S-9

- 1. LOGGED BY: DAMES & HOORE
- 2. DRILLED BY: RAYMOND INTERNATIONAL
- 3. TESTED BY: DAMES & MOORE



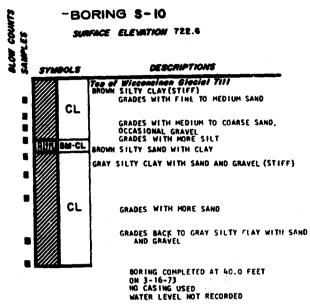


FIGURE 2.5-135

LOG OF BORING S-10

- 1. LOGGED BY: DAMES & MOORE
- 2. DRILLED BY: RAYMOND INTERNATIONAL
- 3. TESTED BY: DAMES & MOORE

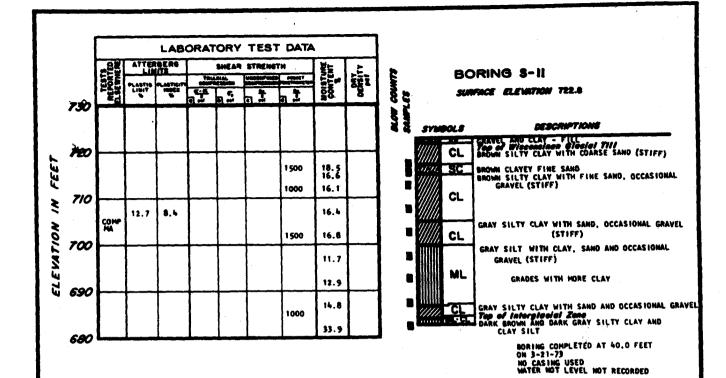


FIGURE 2.5-136

LOG OF BORING S-11

- 1. LOGGED BY: DAMES & HOORE
- 2. DRILLED BY: RAYMOND INTERNATIONAL
- 3. TESTED BY: DAMES & MOORE

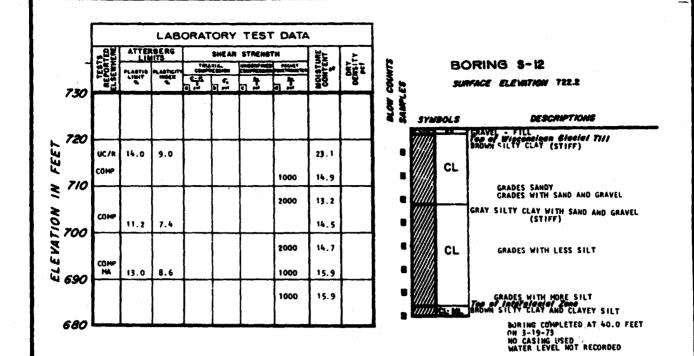


FIGURE 2.5-137

LOG OF BORING S-12

- 1. LOGGED BY: DAMES & MOORE
- 2. DRILLED BY: RAYMOND INTERNATIONAL
- 3. TESTED BY: DAMES & MOORE

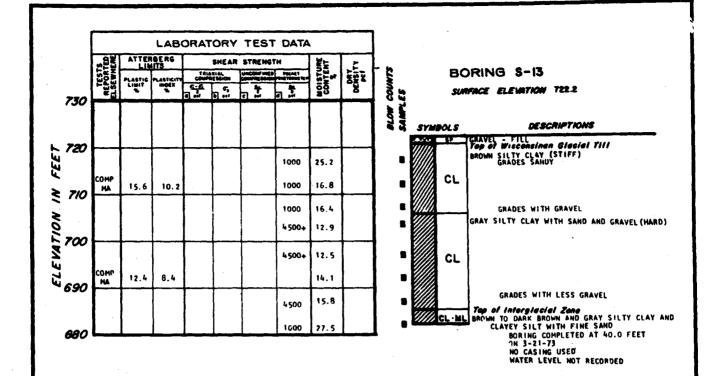


FIGURE 2.5-138

LOG OF BORING S-13

- 1. LOGGED BY: DAMES & MOORE
- 2. DRILLED BY: RAYHOND INTERNATIONAL
- 3. TESTED BY: DAMES & MOORE

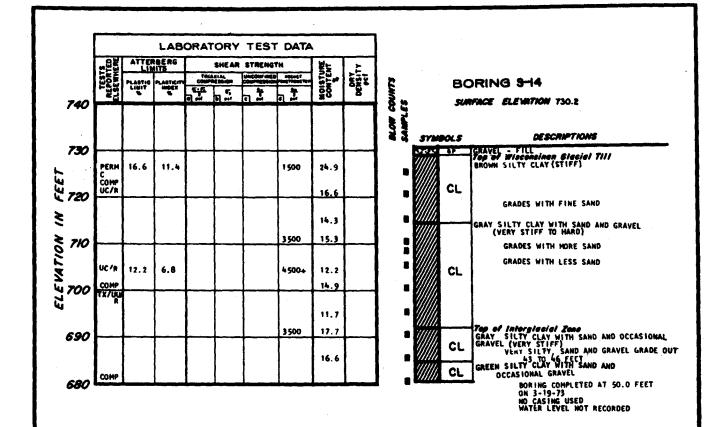
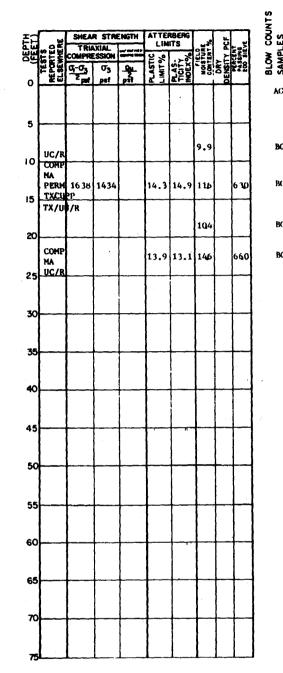


FIGURE 2.5-139

LOG OF BORING S-14

- 1. LOGGED BY: DAMES & MOORE
- 2. DRILLED BY: RAYHOND INTERNATIONAL
- 3. TESTED BY: DAMES & MOORE

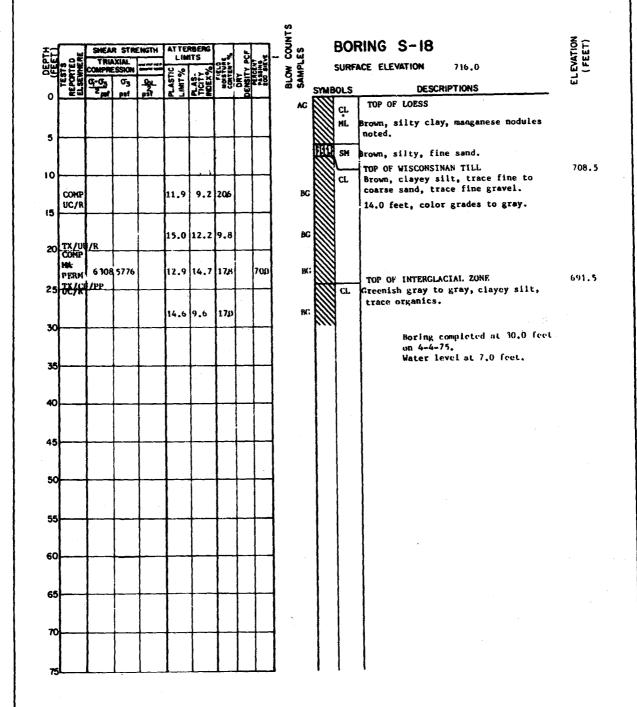


во	RING S-17	ELEVATION (FEET)
SURF	ACE ELEVATION 715.4	
SYMBOLS	DESCRIPTIONS	w
Cr Cr	TOP OF LUESS	
MÎ.	Brown, silty clay, mangamese nodules noted.	
cr T	TOP OF WISCONSINAN TILL Brown, clayey silt, some fine to coarse sand, trace fine gravel.	711.9
	16.5 feet, color grades to gray.	
	TOP OF INTERGLACIAL ZONE	689.9
ML	Greenish gray, silt, some organics.  Boring completed at 30.0 feet on 4-4-75.	
	Water level at 16.5 feet.	

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

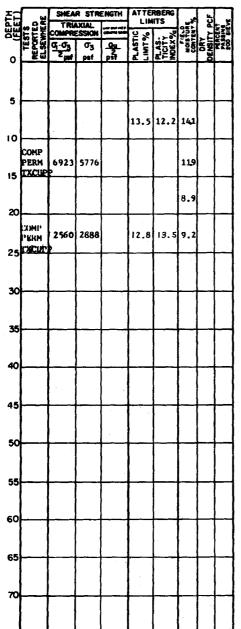
FIGURE 2.5-140



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-141

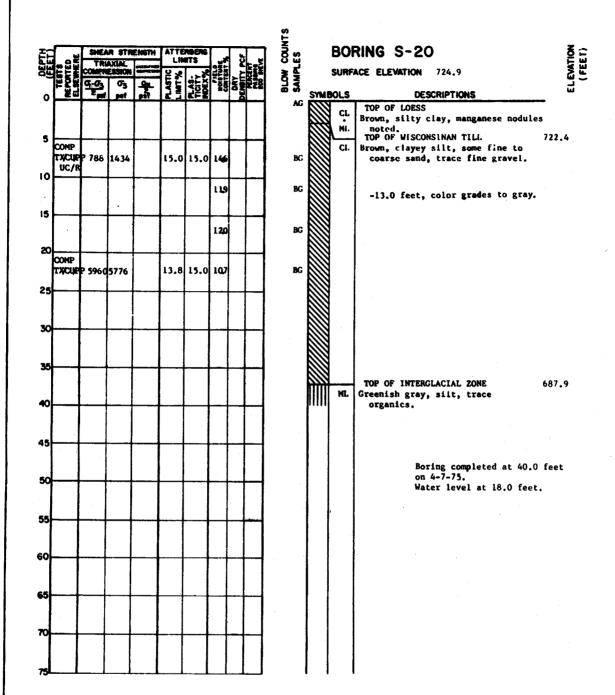


BLOW COUNTS SAMPLES			RING S-19 ACE ELEVATION 725.5	VATION FEET)
SAM				ELE (
AG	SYMB	OLS CL	DESCRIPTIONS TOP OF LOESS	
AU			Grayish brown to brown, silty clay, manganese nodules noted. TOP OF WISCONSINAN TILL	721.5
ВG		CL.	Brown, clayey silt, little fine to coarse sand, trace fine gravel.  Brown silty, fine to coarse sand,	,21,3
₿G		CL	trace fine gravel. Brown, clayey silt, some fine to coarse sand, trace fine gravel.	
BC			15.0 feet, color grades to gray.	
ja:				
			TOP OF INTERGLACIAL ZONE	695.5
		ML	Dark gray, mottled light gray, silt, trace organics.	
		CL	Gray, silty clay, trace organics.	
			Boring completed at 40.0 feet on 4-2-75.	
			Water level at 9.0 feet.	
1				
	1 1			

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

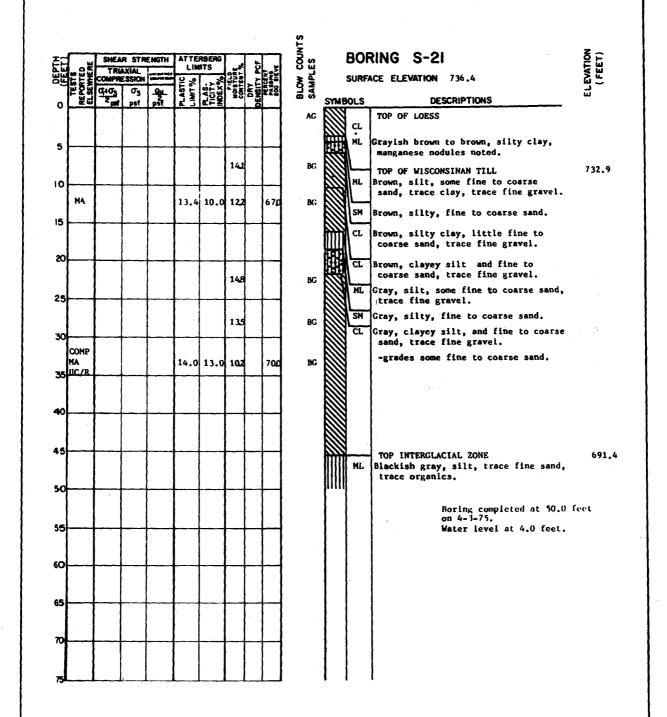
FIGURE 2.5-142



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Soil Testing Services, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

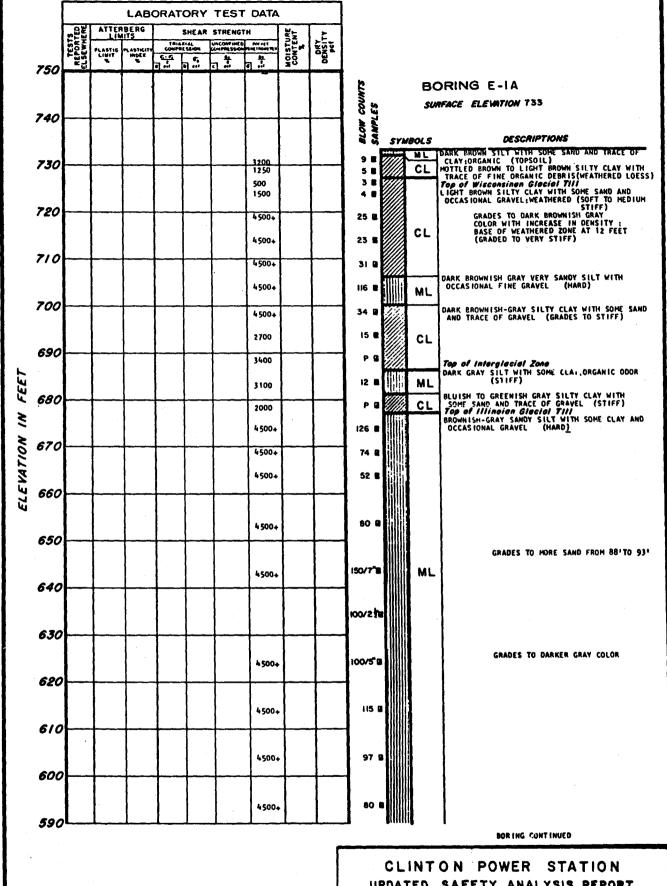
FIGURE 2.5-143



- Logged by: Sargent & Lundy Engineers
   Drilled by: Raymond International
   Tested by: Soil Testing Services, Inc.

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-144

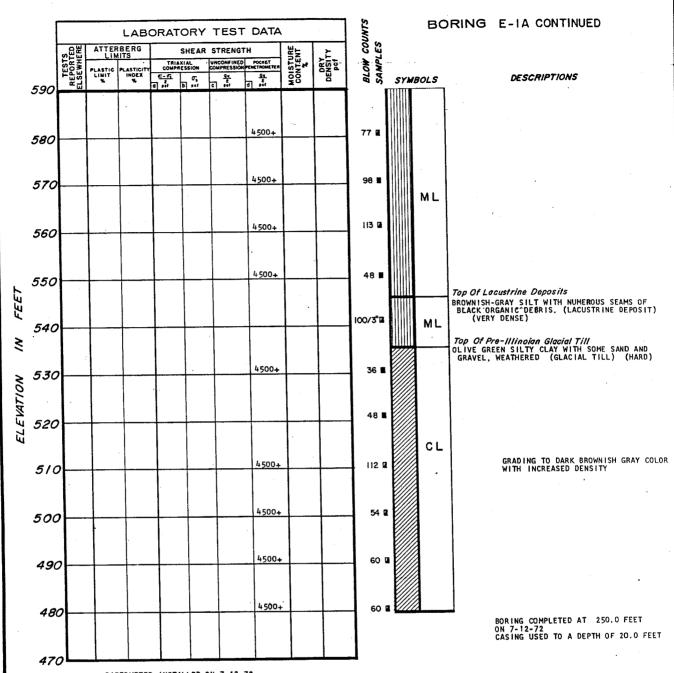


UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-145

LOG OF BORING E-1A

(SHEET 1 of 2)



PIEZOMETER INSTALLED ON 7-13-72
BORING E-1B, LOCATED 10 FEET FROM E-1A,
WAS DRILLED TO A DEPTH OF 40 FEET.
A 3/4 INCH PVC. PIPE WITH THE LOWER END
PLUGGED AND THE LOWER 5 FEET PERFORATED
WAS PLACED TO ELEVATION 693. GRANULAR
BOCKFILL WAS PLACED FROM ELEVATION 693 TO 703;
BENTONITE SEAL FROM ELEVATION 703 TO 705;
AND CEMENT GROUT FROM ELEVATION 705 TO 733.

WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET

DATE

10.6

8-3-/2 8-22-72 9-6-72

REFER TO FIGURE 2.4-38 FOR WATER LEVEL OBSERVATIONS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-145

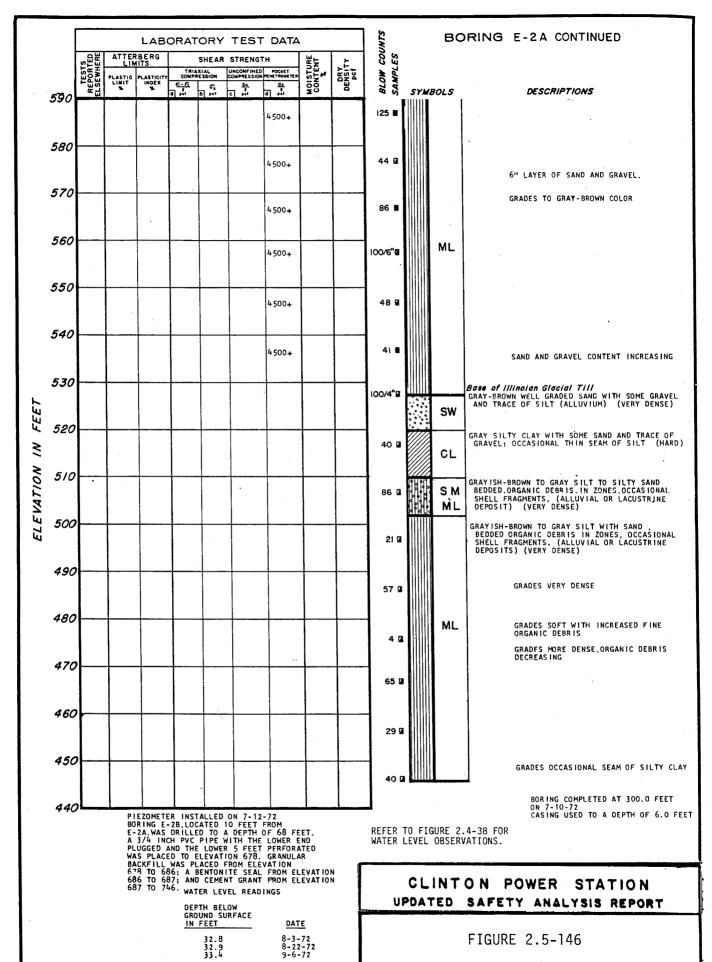
LOG OF BORING E-1A

(SHEET 2 of 2)

NOTE:

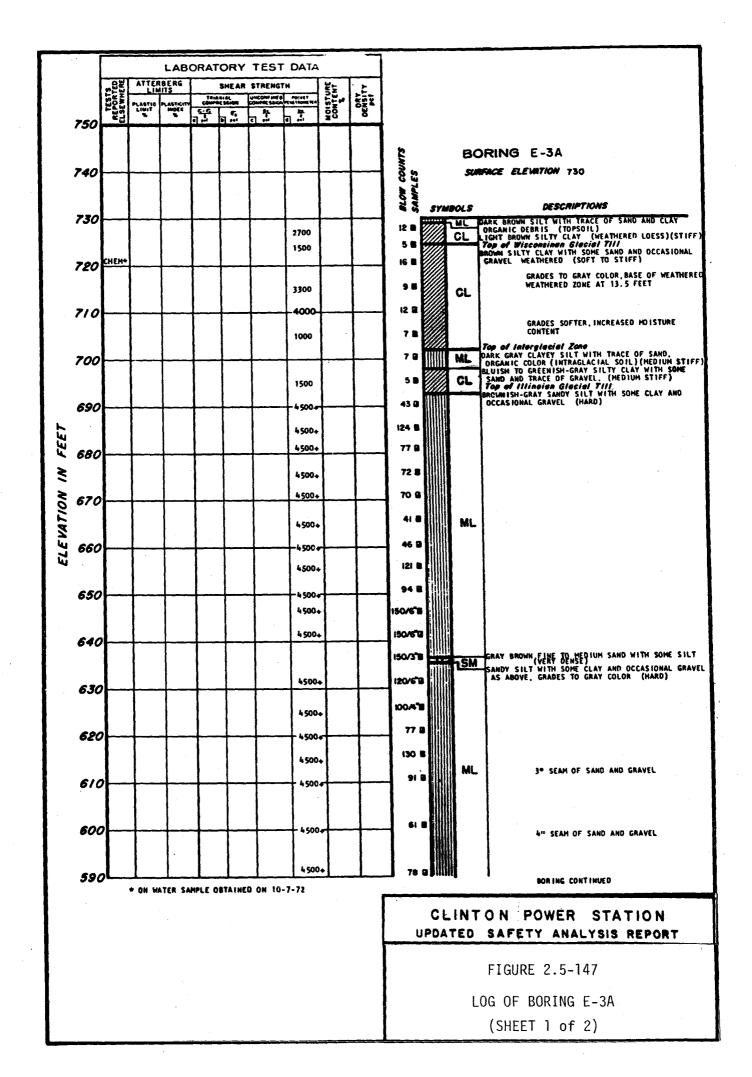
SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

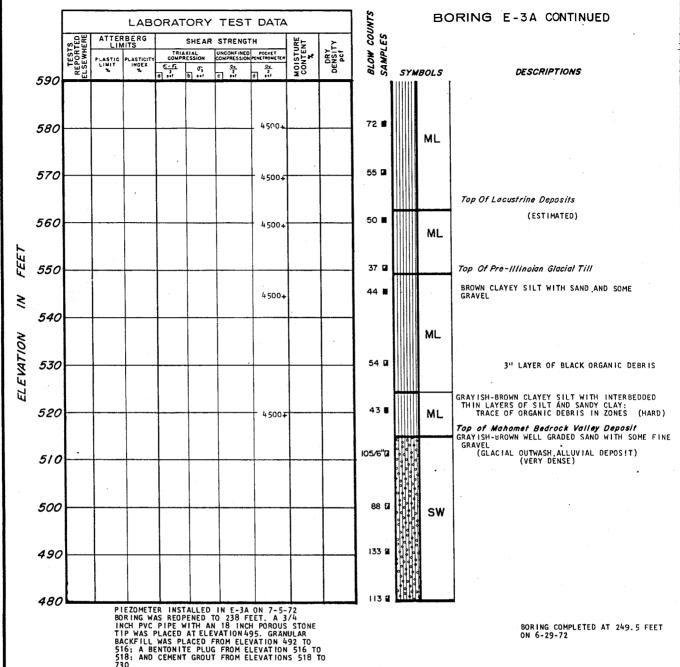
ļ	5111	LABORATORY TEST DATA							T	ي		<b>P</b> (	DRING E-2A
	RTEC	LIN	RBERG	781		AR STRENG		ENE	> <u>+</u> +	<i>₩</i>			
	TESTS REPORTED ELSEWHERE	PLASTIC LIMIT	PLASTICITY	5-S	RESSION	COMPRESSION	POCKET N PENETROMETER	MOISTURE CONTENT	DENSITY Pef	, co		301	RFACE ELEVATION 746
750	윤급	<del></del>		a 👬	D 001	c ,ii	d sit	30	-	BLOW COUNTS SAMPLES			OFFICE HEAD
											57A	ML ML	DESCRIPTIONS  DARK BROWN SILT WITH SOME SAND AND TRACE OF (
,					ĺ	İ	4500+			12 🕷		ML	LIGHT BROWN SILT WITH SOME CLAY AND TRACE OF SAND AND FINE GRAVEL. (WEATHERED LOESS) (STIL
740				<del> </del>	1	<b> </b>	2000			9 ■			Top of Wisconsinan Glacial TIII BROWN SILTY CLAY WITH SOME WELL GRADED SAND AND OCCASIONAL GRAVEL. (WEATHERED) (STIFF
							1000			12 🗷		CL	I
730					1		3700			. 17 🗷		1	GRADES TO GRAYISH-BROWN COLOR BASE OF WEATHERED ZONE AT 18.0 FEET
750										82 1		<u> </u>	GRAY CLAYEY AND SAMDY SILT WITH INTERBEDDED
				l									THIN LAYERS OF CLEAN FINE TO MEDIUM SAND (HAI
720				ļ			4500+		ļ	45 ■		ML	INTERBEDDED SAND LAYERS GRADE OUT
										80 2			GRADES TO GRAY SILT WITH SOME CLAY AND TRACE OF SAND AND FINE GR.
				İ						145	ЩЩ	-	GRAY FINE TO MEDIUM SAND (VERY DENSE)
710					<del> </del>		<del> </del>	<del>                                     </del>	+-	1	,,,,,	SP	DARK GRAY SILTY CLAY WITH SOME SAND AND TRACE
							4000			37 🛭		1	OF GRAVEL. (VERY STIFF)
700							4400			27 🗷		CL	
700				1						1 [			
							4500+			31 0		1	Top of Interglacial Zone
690				<u> </u>	<u> </u>	<u> </u>		<u> </u>	<u> </u>	21 🔳		ML	GRAY SILT WITH SOME CLAY AND OCCASIONAL THIN SEAM OF FINE SAND (MEDIUM STIFF)
							3000			14 0		CL	GRAY SILTY CLAY WITH SOME SAND AND OCCASIONA: GRAVEL. (STIFF)
							1	1				I UL	
680	<b> </b> -			+	+		-	<u> </u>	+	28 ■ [		SW	GRAY FINE TO COARSE SAND WITH SOME FINE GRAV AND TRACE OF SILT (MEDIUM DENSE) Top of Illinoian Glacial Till
	Ī						4500+		1	33 🗷			GRAY-PROWN SANDY SILT WITH SOME CLAY AND OCCASIONAL GRAVEL. (HARD)
							1,500			127/9"		ML	
670			<b>T</b>	1	+		4500+	<b>†</b>	1	1	Щ		1
		l								1000	ie ie	SM	GRAY-BROWN FINE TO MEDIUM SAND WITH SOME SIL GRAY-BROWN SANDY SILT WITH SOME CLAY AND
660							450:3+			50 ■			OCCASIONAL GRAVEL. (HARD)
500							4500+			85 12			
							-			Jec 2."-			
650	<b> </b> -	<del> </del>	-	+	<del> </del>	-	4500+		-	150/11"■		1	4" SEAM OF SAND AND GRAVEL
										100/3"[2]			
							4500+			100/5"2			
640		<del> </del>	1	+-	1		4500+	<del> </del>	<del> </del>	100/6"12			GRADES TO GRAY COLOR
		1					4500+			100/612		1	
630			<u> </u>				4500+			98 ■			1
030							4500+			 54 🗷		ML	
							+300+						
620	<u> </u>	ļ	-					<u> </u>		4			
							4500+			76 🗷			
	1	1											
610	<b> </b>	<del> </del>	+	+-	+	+		+	+-	┨ . │		<b> </b>  -	
							4500+			ıi2 👪			
600													
600		[ ·			1 -		15			103 🛭		1	
	1	1					4500+		'	103 1		1	
590	L	1	1					1	<u>r</u>		ШШ	1/8	
													ON POWER STATION
										UP	DA.	TED	SAFETY ANALYSIS REPORT
					_				Ĭ				FIGURE 2.5-146
BORING CONTINUED									l				FIGURE 2.3-140
									- 1			L	OG OF BORING E-2A
												_	



SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

LOG OF BORING E-2A (SHEET 2 of 2)





730
PIEZOMETER INSTALLED ON 7-12-72
BORING E-3B LOCATED 10 FEET
FROM E-3A WAS DRILLED TO A
DEPTH OF 75 FEET. A 3/4 INCH PVC
PIPE WITH THE LOWER END PLUGGED AND
THE LOWER 5 FEET PERFORATED WAS PLACED
AT ELEVATION555, GRANULAR BACKFILL WAS
PLACED FROM ELEVATIONS 655 TC.662: A BENTONITE
SEAL FROM ELEVATIONS 622 TO633: AND PEA GRAVEL
AND CEMENT GROUT FROM ELEVATIONS 663 TO730.

WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET

TIP ELEVATION 492.0 TIP ELEVATION 657.5

DATE

8-15-72 9-6-72

REFER TO FIGURE 2.4-38 FOR WATER LEVEL OBSERVATIONS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-147

LOG OF BORING E-3A (SHEET 2 of 2)

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

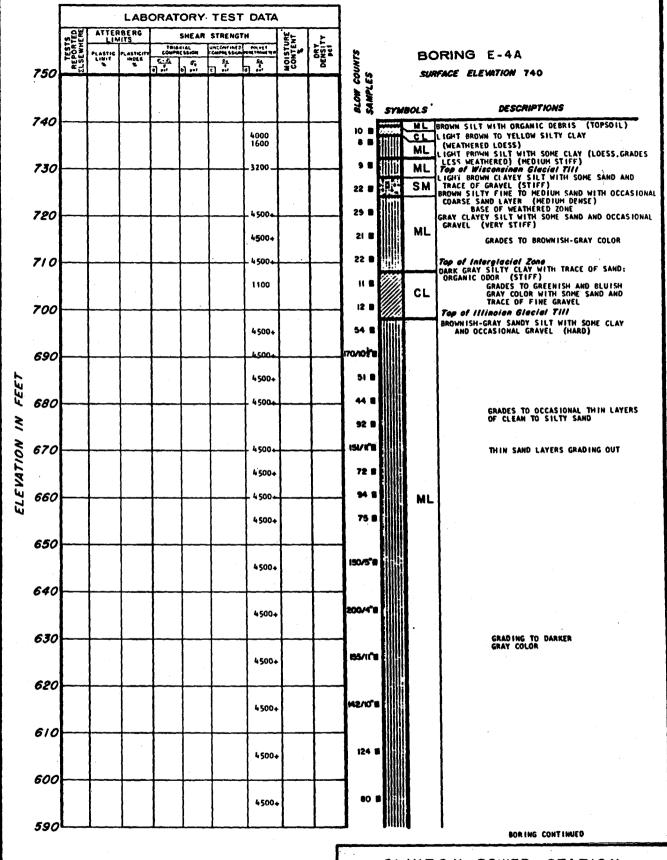
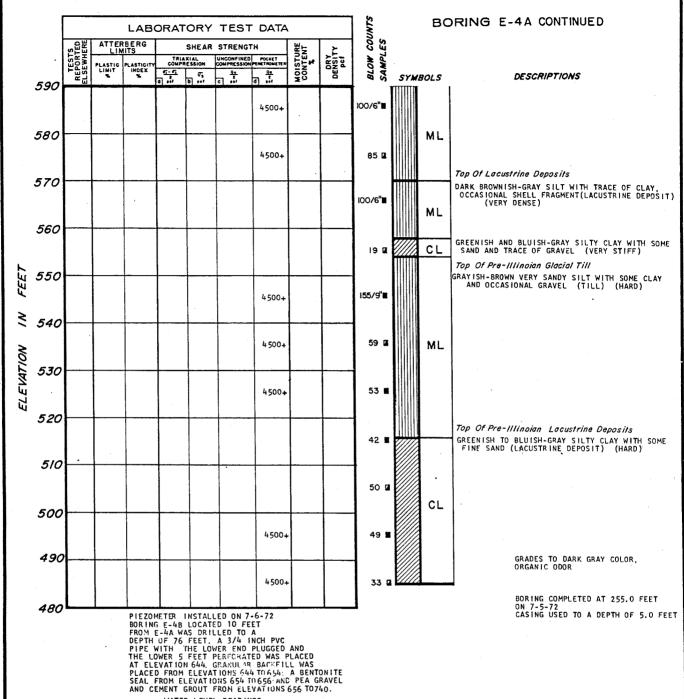


FIGURE 2.5-148
LOG OF BORING E-4A
(SHEET 1 of 2)



WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET

REFER TO FIGURE 2.4-38 FOR WATER LEVEL OBSERVATIONS.

> CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

> > FIGURE 2.5-148

LOG OF BORING E-4A (SHEET 2 of 2)

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

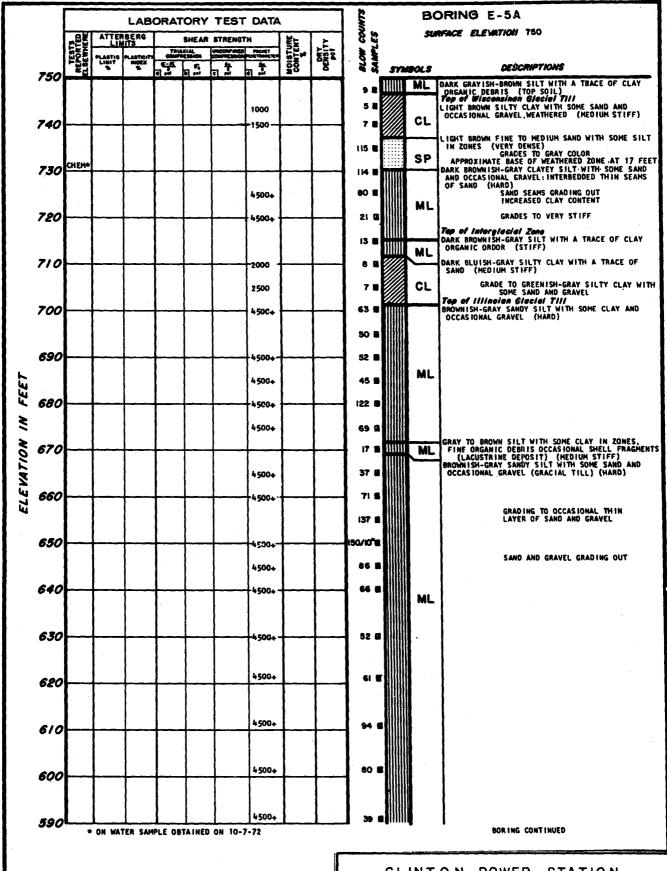
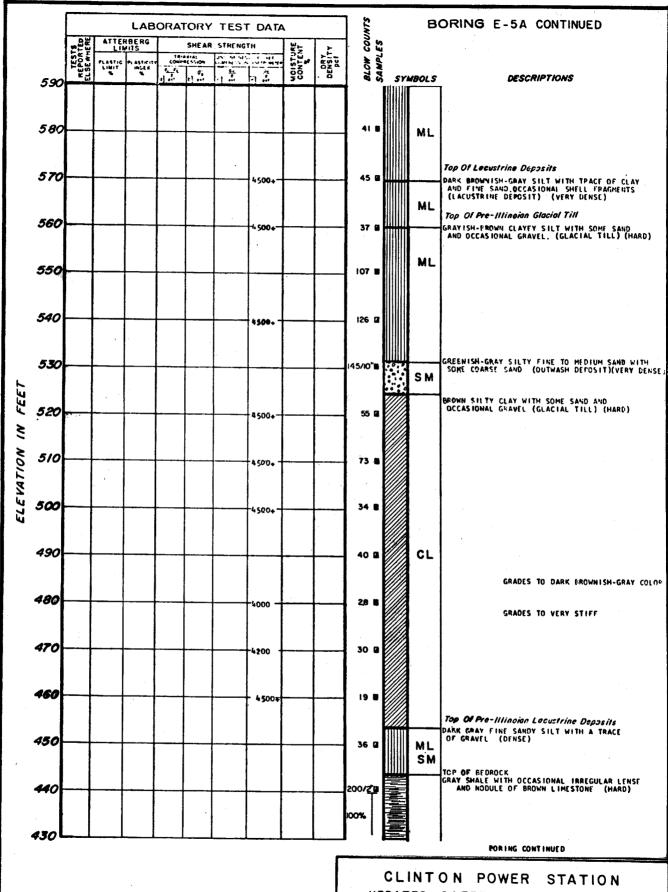


FIGURE 2.5-149

LOG OF BORING E-5A

(SHEET 1 of 3)

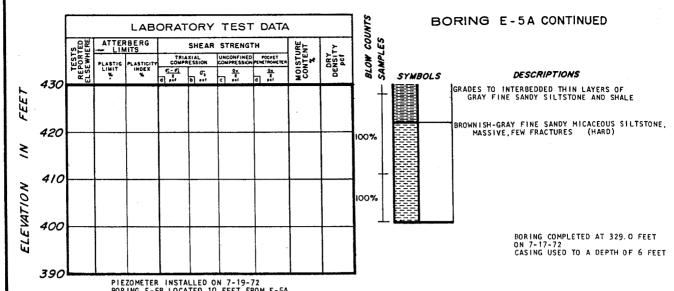


UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-149

LOG OF BORING E-5A

(SHEET 2 of 3)



PIEZOMETER INSTALLED ON 7-19-72
BORING E-58, LOCATED 10 FEET FROM E-5A,
WAS DRILLED TO A DEPTH OF 76 FEET.
A 3/4 INCH PVC PIPE WITH AN 18 INCH
POROUS STONE TIP WAS PLACED TO ELEVATION 675.
GRANULAR BACKFILL WAS PLACED TO ELEVATION 676,
GRANULAR BACKFILL WAS PLACED BETWEEN ELEVATIONS
680 AND 683; AND CEMENT GROUT FROM ELEVATION
683 TO 750.

WATER LEVEL PEARLINGS

WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET

REFER TO FIGURE 2.4-38 FOR WATER LEVEL OBSERVATIONS.

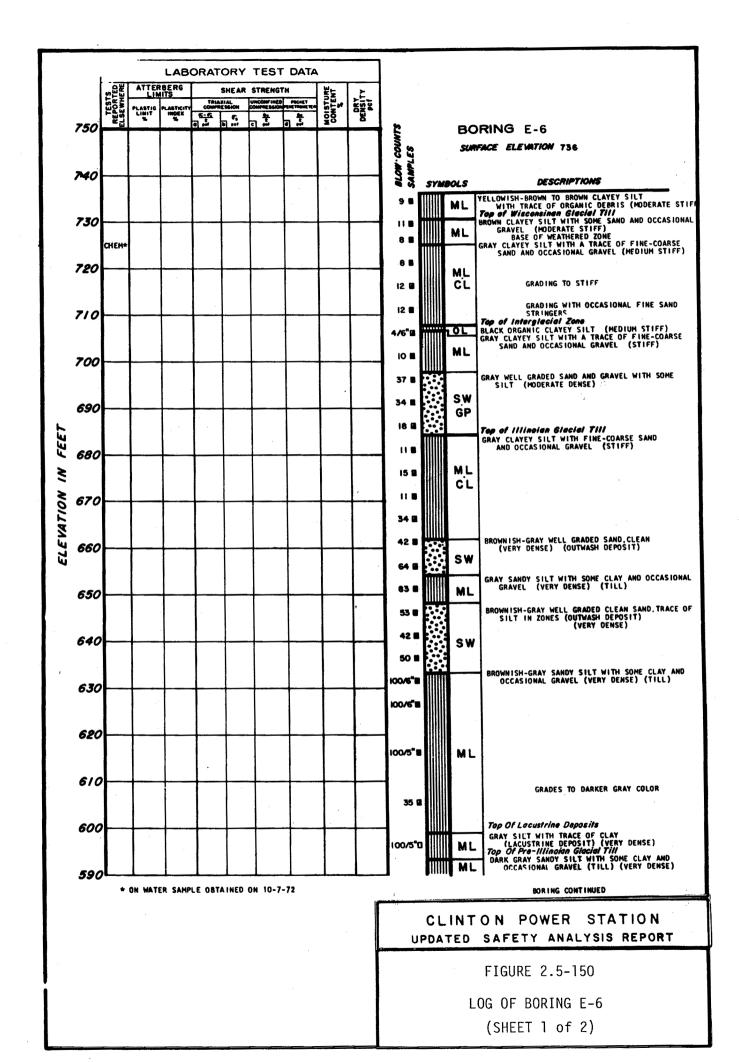
> CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

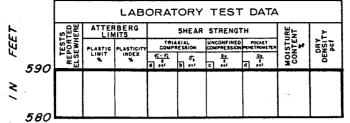
> > FIGURE 2.5-149

LOG OF BORING E-5A (SHEET 3 of 3)

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.





SAMPLES SAMPLES SAMPLES

BORING E-6 CONTINUED

DESCRIPTIONS

ELEVATION

PIEZOMETER INSTALLED ON 7-25-72
BORING WAS FLUSHED WITH CLEAN
WATER AND A 2 INCH ID PVC PIPE
WITH THE LOWER END PLUGGED AND
PERFORATIONS AT 1 FOOT INTERVALS
FOR THE ENTIRE LENGTH WAS PLACED
TO ELEVATION 586. GRANULAR BACKFILL WAS PLACED FROM ELEVATIONS
585 TO 736.

BORING COMPLETED AT 151.0 FEET ON 7-25-72 CASING USED TO A DEPTH OF 6 FEET

#### WATER LEVEL READINGS

DEPTH BELOW GROUND SURFACE IN FEET DATE

11.8 9-19-72
11.4 9-26-72
11.3 10-10-72

REFER TO FIGURE 2.4-38 FOR WATER LEVEL OBSERVATIONS.

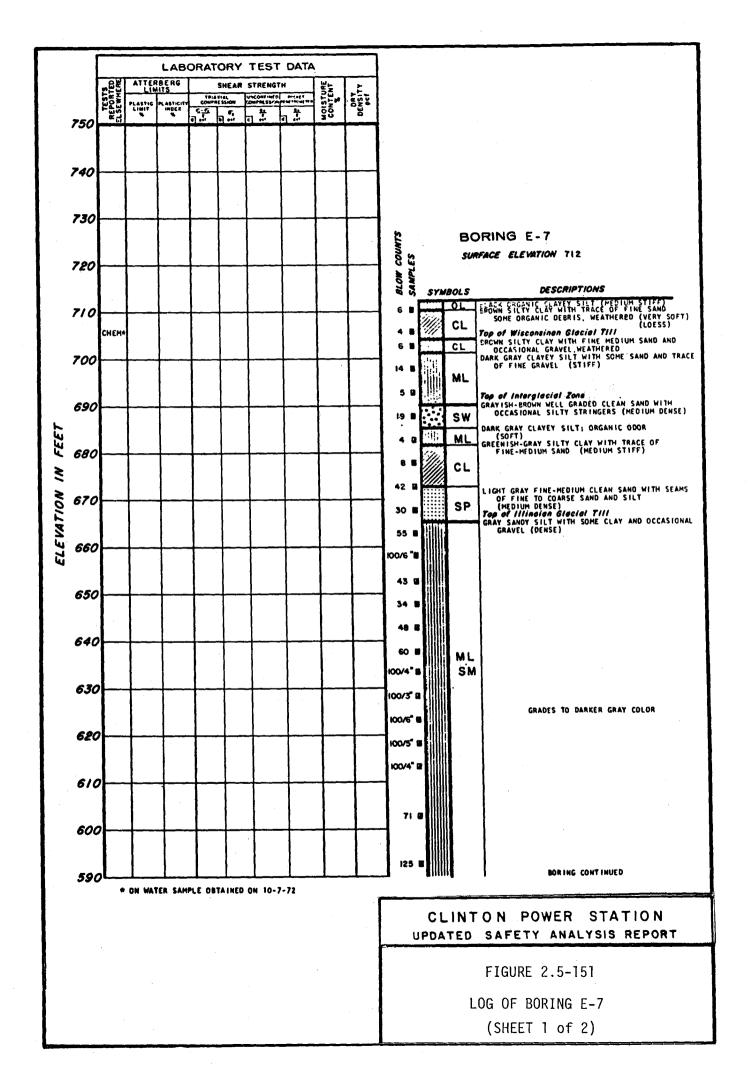
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

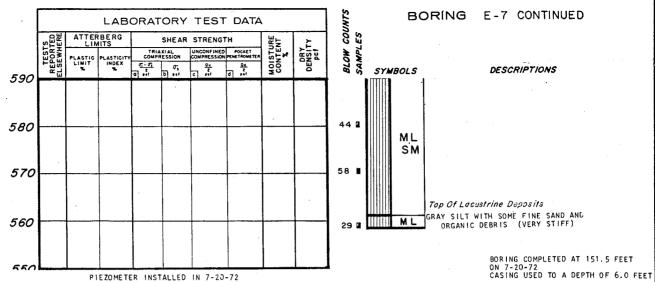
FIGURE 2.5-150

LOG OF BORING E-6
(SHEET 2 of 2)

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.





PIEZOMETER INSTALLED IN 7-20-72
BORING WAS FLUSHED WITH CLEAN WATER
AND A 3 INCH ID PIPE WITH THE LOWER
END PLUGGED AND PERFORATION AT 1 FOOT
INTERVALS FOR THE ENTIRE LENGTH WAS
PLACED TO ELEVATION 562. GRANULAR
BACKFILL WAS PLACED FROM ELEVATION
560.5 TO 712.

SURFACE IN FEET	DATE
5.5	9-19-72
2.4	9-26-72
3.7	10-10-72

REFER TO FIGURE 2.4-38 FOR WATER LEVEL OBSERVATIONS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-151

LOG OF BORING E-7
(SHEET 2 of 2)

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

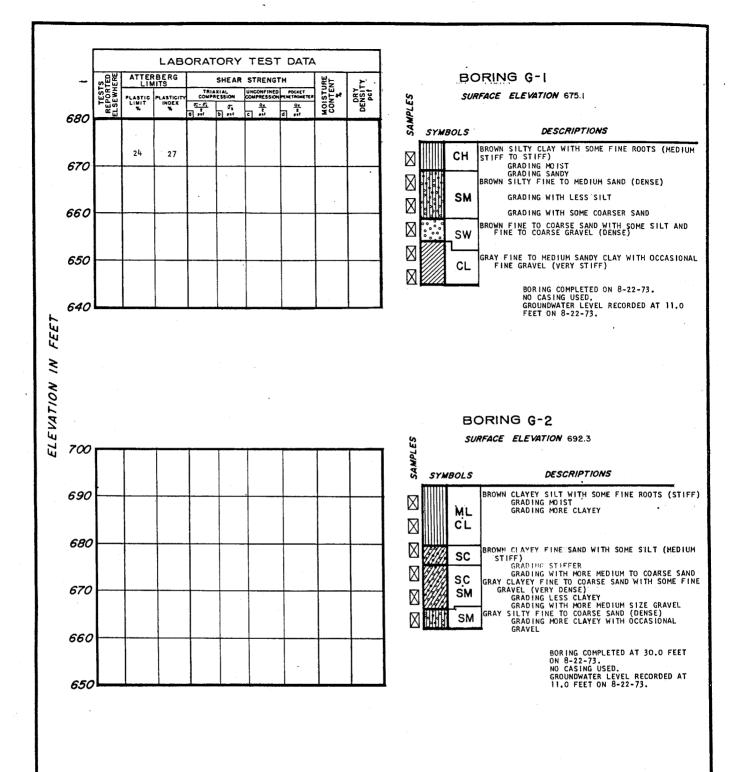


FIGURE 2.5-152

LOG OF BORINGS G-1 AND G-2

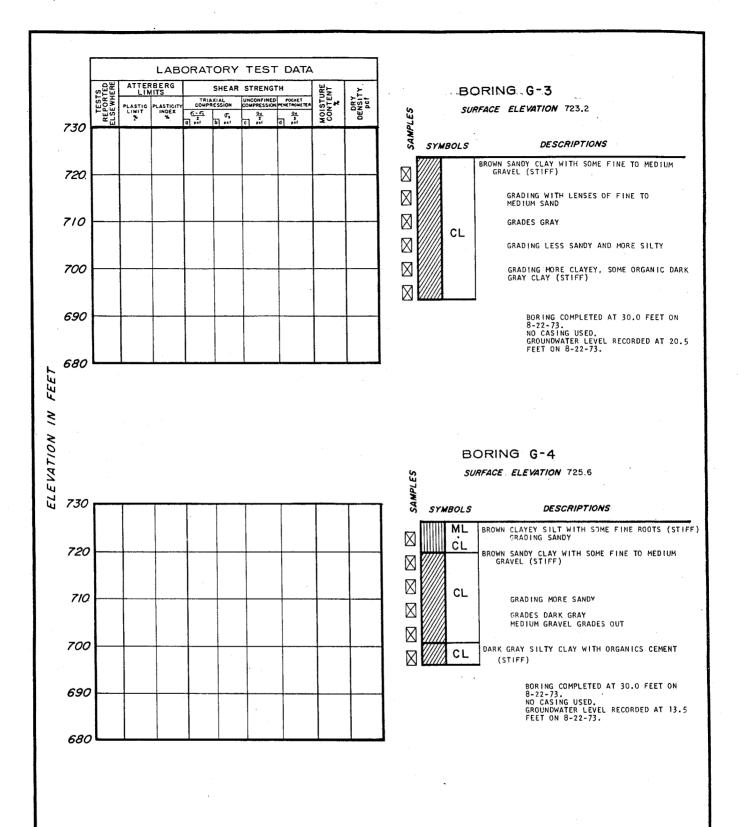
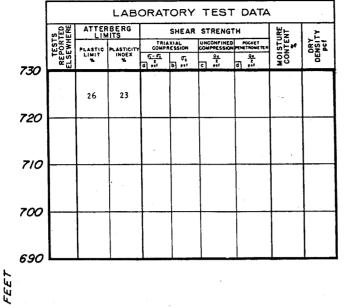


FIGURE 2.5-153

LOG OF BORINGS G-3 AND G-4



# BORING G-5

SURFACE ELEVATION 729.3

SAMPL	SYMBOLS		DESCRIPTIONS		
⊠ ⊠		CL	DARK BROWN SILTY CLAY WITH SOME FINE ROOTS (MEDIUM STIFF) GRADING MORE CLAYEY  GRADING WITH OCCASIONAL MEDIUM GRAVEL		
		GC	LIGHT BROWN AND GRAY CLAYEY FINE TO MEDIUM SAND AND GRAVEL (MEDIUM DENSE) GRADING WITH LESS FINE SAND GRADING MORE CLAYEY		
		CL BROWN TO	BROWN AND GRAY FINE SANDY CLAY (MEDIUM STIFF TO STIFF) GRADING GRAY AND STIFFER GRADING DARKER GRAY WITH OCCASIONAL FINE GRAVEL (STIFF TO VERY STIFF) GRADING MORE SANDY		

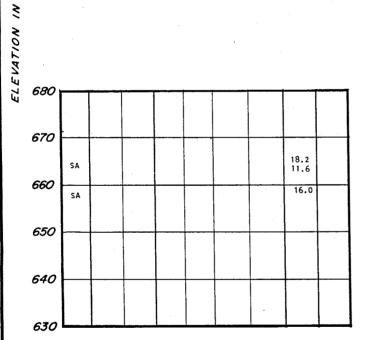
BORING COMPLETED AT 30.0 FEET ON 8-17-73. NO CASING USED. GROUNDWATER LEVEL RECORDED AT 9.5 FEET ON 8-17-73.

# BORING G-6

SURFACE ELEVATION 677.8

SAMPLES DESCRIPTIONS SYMBOLS BROWN CLAYEY SILT WITH SOME FIME ROOTS (STIFF)
GRADING SANDY
BROWN FINE MEDIUM SAND WITH SOME CLAY
(MEDIUM DENSE TO DENSE) IIIII ML  $\boxtimes$ GRADING WITH SOME FINE TO MEDIUM GRAVEL GRADING WITH LESS GRAVEL AND MORE SILT SC GRADING WITH MORE COARSE GRAVEL (DENSE)  $\boxtimes$ GRAY CLAYEY SILT WITH SOME FINE TO MEDIUM SAND AND GRAVEL (VERY STIFF) ML

BORING COMPLETED AT 30.0 FEET ON 8-21-73, NO CASING USED. GROUNDWATER LEVEL RECORDED AT 13.5 FEET ON 8-21-73.



# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-154

LOG OF BORINGS G-5 AND G-6

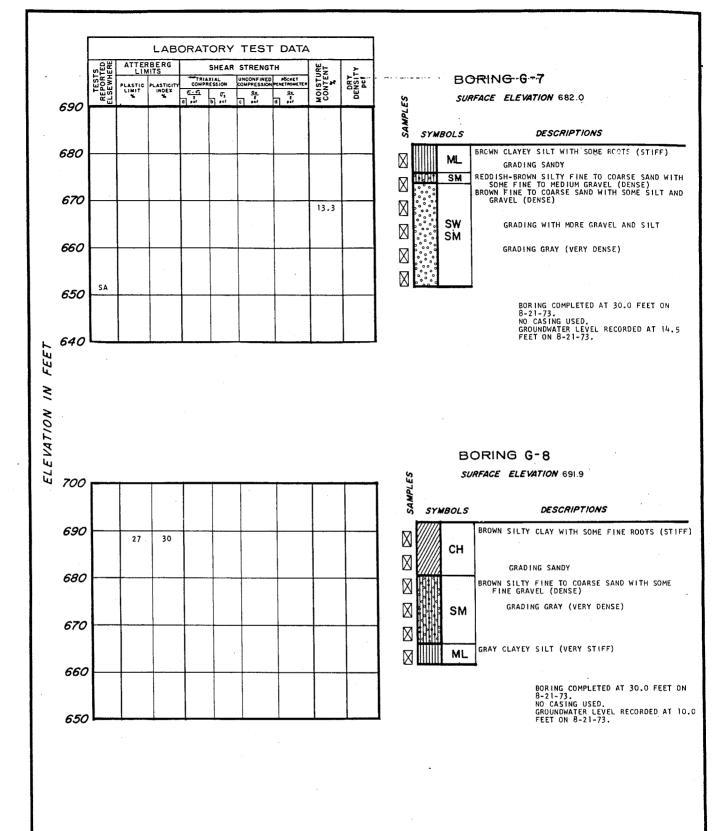
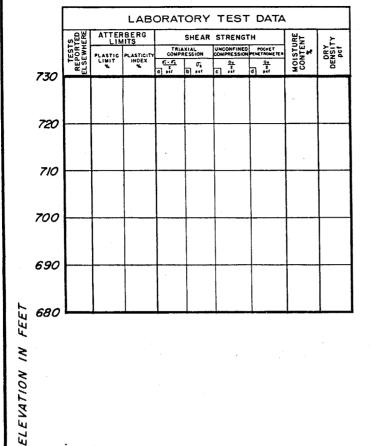


FIGURE 2.5-155

LOG OF BORINGS G-7 AND G-8



730

710

700

690

## BORING G-9

SURFACE ELEVATION 724.9

SAMPLES DESCRIPTIONS SYMBOLS BROWN SILTY CLAY WITH SOME ROOTS (STIFF)  $\boxtimes$ CL GRADING MORE SILTY ML REDDISH-BROWN SILTY FINE TO COARSE SAND WITH FINE TO MEDIUM GRAVEL (VERY DENSE) GRADING LESS SANDY SM  $\boxtimes$ GRADING MORE SANDY GRADING DARK GRAYISH-BROWN GRADING GRAY AND MORE SILTY DARK GRAY CLAYEY SILT WITH FINE TO COARSE SAND AND SOME FINE GRAVEL (STIFF TC VERY STIFF) GRADING WITH SOME PEAT ML

BORING COMPLETED AT 30.0 FEET ON 8-20-73. NO CASING USED. GROUNDWATER LEVEL RECORDED AT 14.5 FEET ON 8-20-73.

## BORING G-10

SURFACE ELEVATION 727.2 SAMPLES SYMBOLS DESCRIPTIONS MOTTLED BROWN AND GRAY SILTY CLAY WITH SOME FINE SAND (STIFF) ORANGE-BROWN SILTY SAND WITH CCCASIONAL FINE TO MEDIUM GRAVEL AND SOME CLAY (MEDIUM DENSE) CL SM SP
REDDISH-BROWN SAND WITH SOME GRAVEL AND SILT
(MEDIUM DENSE TO DENSE)

GRAY CLAYEY SILT WITH OCCASIONAL FINE GRAVEL
(STIFF TO VERY STIFF)
GRAY CLAYEY GRAVEL WITH SOME SAND (DENSE)
GRADING DENSER WITH MORE FINE TO MEDIUM
GRAVEL
GRAVEL (STIFF)
DARK GRAY PEAT
GRAY CLAYEY SILT WITH SOME SAND AND OCCASIONAL
FINE GRAVEL (STIFF)
DARK GRAY PEAT
GRAY CLAYEY SILT WITH SOME SAND AND GRAVEL X

BORING COMPLETED AT 30.0 FEET ON 8-17-73. NO CASING USED. GROUNDWATER LEVEL RECORDED AT 7.0 FEET ON 8-17-73.

#### STATION CLINTON POWER SAFETY ANALYSIS REPORT UPDATED

FIGURE 2.5-156

LOG OF BORINGS G-9 AND G-10

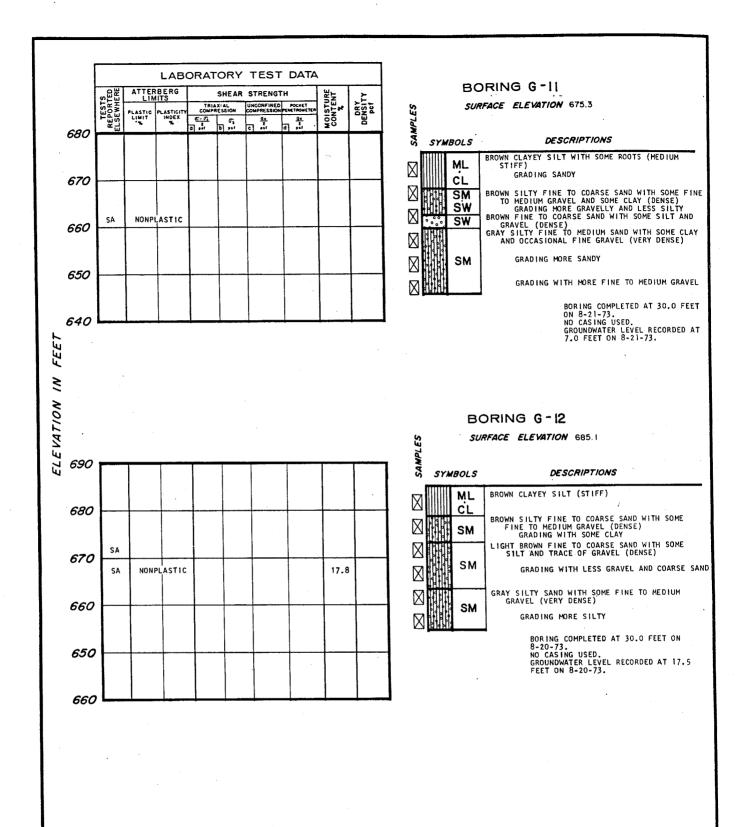


FIGURE 2.5-157

LOG OF BORINGS G-11 AND G-12

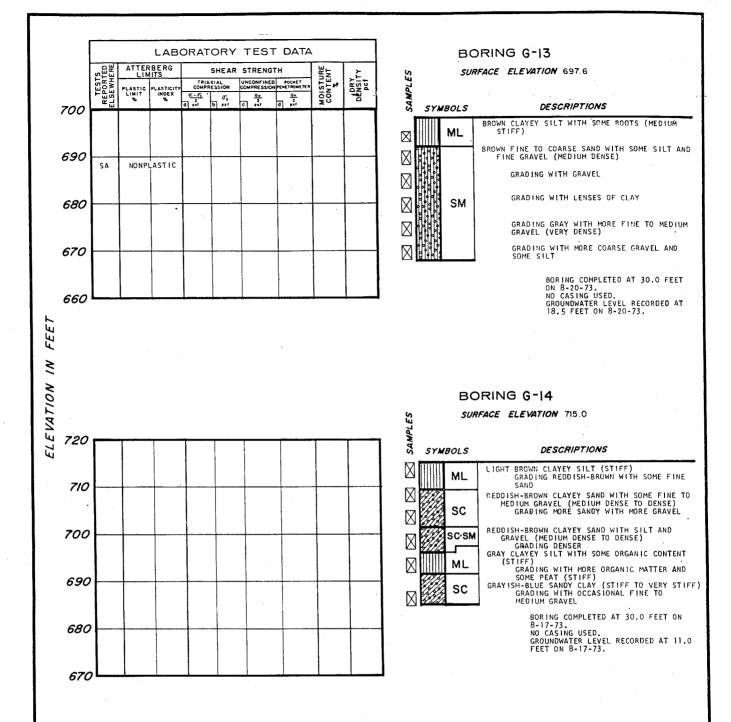
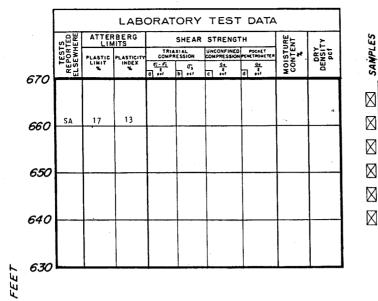


FIGURE 2.5-158

LOG OF BORINGS G-13 AND G-14



# BORING G-15

SURFACE ELEVATION 669.6

SYMBOLS DARK GRAY SILTY CLAY (MEDIUM STIFF) CL 

SC

SM

#### DESCRIPTIONS

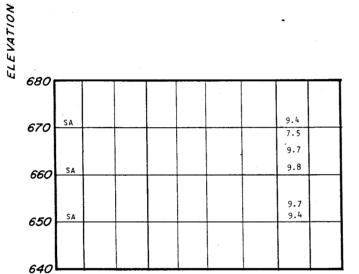
BROWN SILTY FINE TO COARSE SAND WITH SOME GRAVEL (MEDIUM DENSE) GRADING WITH LENSES OF GRAY SILTY CLAY

GRADING WITH LESS GRAVEL GRADING WITH COARSER SAND

GRADING WITH CLAY

GRAY SILTY FINE TO COARSE SAND WITH SOME FINE TO MEDIUM SIZE GRAVEL (VERY DENSE)

BORING COMPLETED AT 30.0 FEET ON 8-21-73. NO CASING USED. GROUNDWATER LEVEL RECORDED AT 9.5 FEET ON 8-21-73.



≥

## BORING G-16

SURFACE ELEVATION 678.1

SAMPLES SYMBOLS ML XML SM SP SM ML

### DESCRIPTIONS

BROWN SILT WITH SOME ORGANICS, TRACE OF FINE SAND AND CLAY (TOPSOIL)
BROWN SILT WITH CLAY AND TRACE OF FINE SAND (STIFF)
BROWN FINE TO MEDIUM SAND WITH SOME SILT AND GRAVEL
BROWN FINE TO MEDIUM SAND WITH SOME GRAVEL AND
TRACE OF SILT

BROWN FINE TO COARSE SAND WITH SOME SILT AND GRAVEL
GRADES WITH LESS GRAVEL
GRADES WITH MORE FINE SAND AND SILT
GRADES WITH LESS SILT

GRADES WITH LENSES OF FINE AND MEDIUM

GRAYISH-BROWN SILT WITH SOME CLAY, FINE SAND AND GRAVEL

BORING COMPLETED AT 30.0 FEET CN 9-19-73. NO CASING USED. GROUNDWATER LEVEL RECORDED AT 11.5 FEET ON 9-19-73.

#### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-159

LOG OF BORINGS G-15 AND G-16

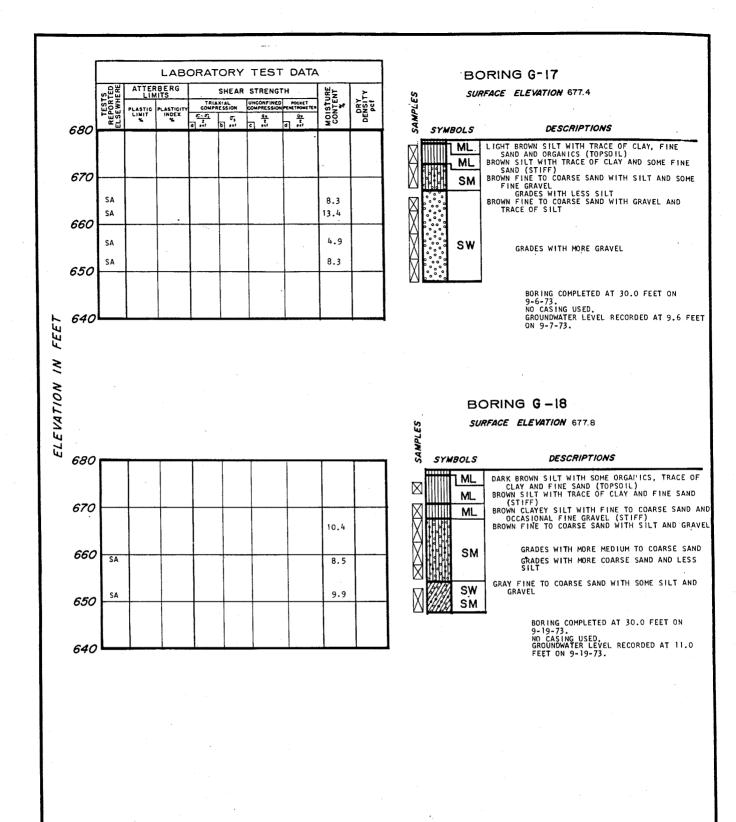
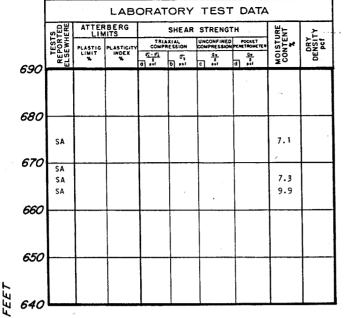


FIGURE 2.5-160

LOG OF BORINGS G-17 AND G-18



₹

ELEVATION

670

660

650

640

630

SA

## BORING G-19

SURFACE ELEVATION 682.6

SAMPLES SYMBOLS SM SM SM ML

#### DESCRIPTIONS

BROWN SILT WITH SOME ORGANICS AND TRACE OF CLAY AND FINE SAND (TOPSOIL)
BROWN SILT WITH SOME CLAY AND TRACE OF FINE SAND (STIFF)
GRADES WITH MORE FINE SAND
DARK BROWN SILTY SAND WITH SOME CLAY
BROWN FINE TO COARSE SAND WITH SOME GRAVEL AND SILT

GRADES WITH MORE FINE SAND

GRADES WITH LESS FINE SAND
COARSE GRAVEL GRADES OUT
GRAY FINE TO COARSE SAND WITH SILT AND SOME
FINE GRAVEL
GRAY AND BROWN LAYERED SILT WITH SEAMS OF
SAND AND FINE GRAVEL AND TRACE OF ORGANICS
(MEDIUM STIFF)
LIGHT GRAY SANDY SILT WITH SOME GRAVEL.
(MEDIUM STIFF)

BORING COMPLETED AT 30.0 FEET ON 9-19-73. NO CASING USED. GROUNDWATER LEVEL RECORDED AT 16.0 FEET ON 9-19-73.

## BORING G-20

SURFACE ELEVATION 668.9

1BOLS	S SYM	_
CL	M	
CL		
SM		9
SM	Ž III	
ML		

11. 9.

## DESCRIPTIONS

BLACK CLAY WITH SILT AND TRACE OF FINE SAND
AND ORGANICS (TOPSOIL)
DARK GRAY TO BLACK CLAY WITH SILT, TRACE OF
FINE SAND AND SOME ORGANICS (VERY STIFF)
GRADES LESS STIFF
BROWN FINE TO COARSE SAND WITH SOME SILT,
GRAVEL AND TRACE OF CLAY
GRADES WITH SOME SEAMS OF CLAYEY SILT
AND CLAYEY SAND
GRADES WITH LESS SILT GRAY SILTY FINE TO COARSE SAND WITH SOME GRAVEL, TRACE OF CLAY AND SOME SEAMS AND AND LAYERS OF SILT GRAY SANDY SILT WITH SOME FINE GRAVEL AND TRACE OF CLAY GRADES WITH SEAMS OF FINE SAND

BORING COMPLETED AT 30.0 FEET ON 9-20-73.
NO CASING USED.
GROUNDWATER LEVEL RECORDED AT 3.2 FEET ON 9-20-73.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-161

LOG OF BORINGS G-19 AND G-20

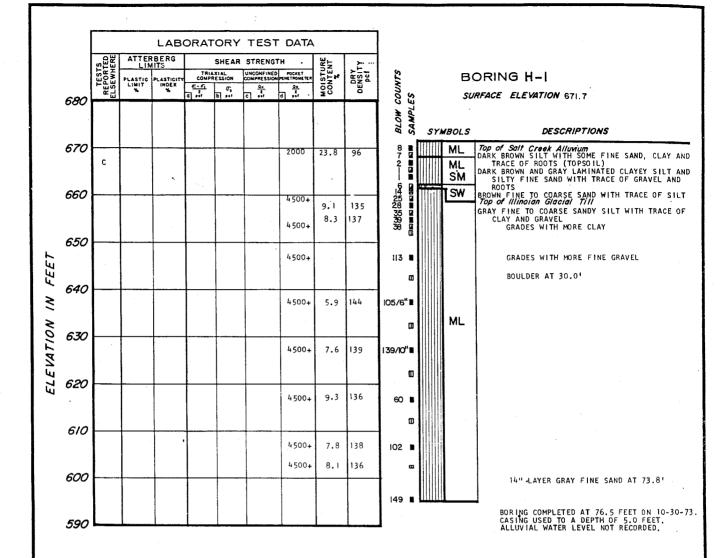


FIGURE 2.5-162

LOG OF BORING H-1

NOTE:

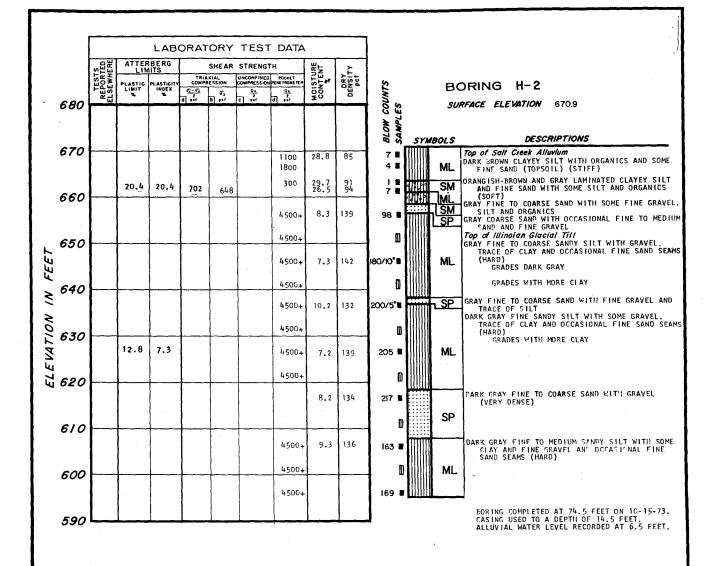


FIGURE 2.5-163

LOG OF BORING H-2

NOTE

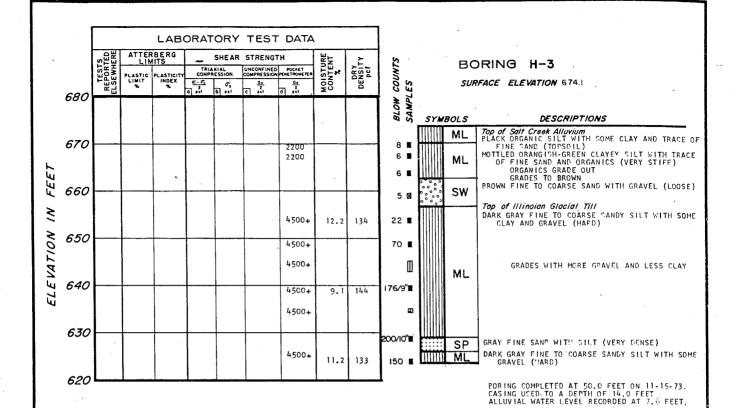


FIGURE 2.5-164

LOG OF BORING H-3

NOTE:

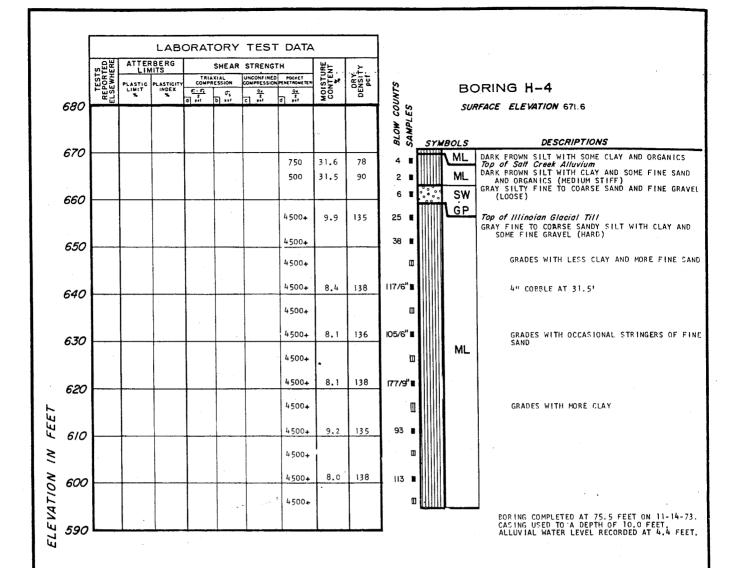


FIGURE 2.5-165

LOG OF BORING H-4

NOTE:

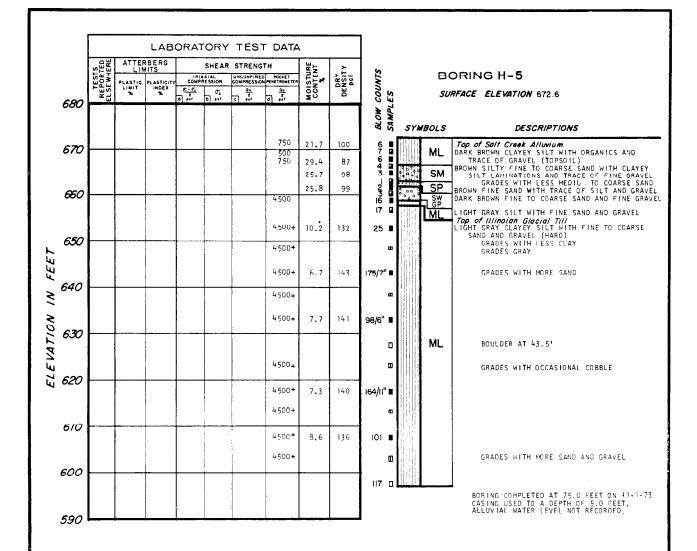
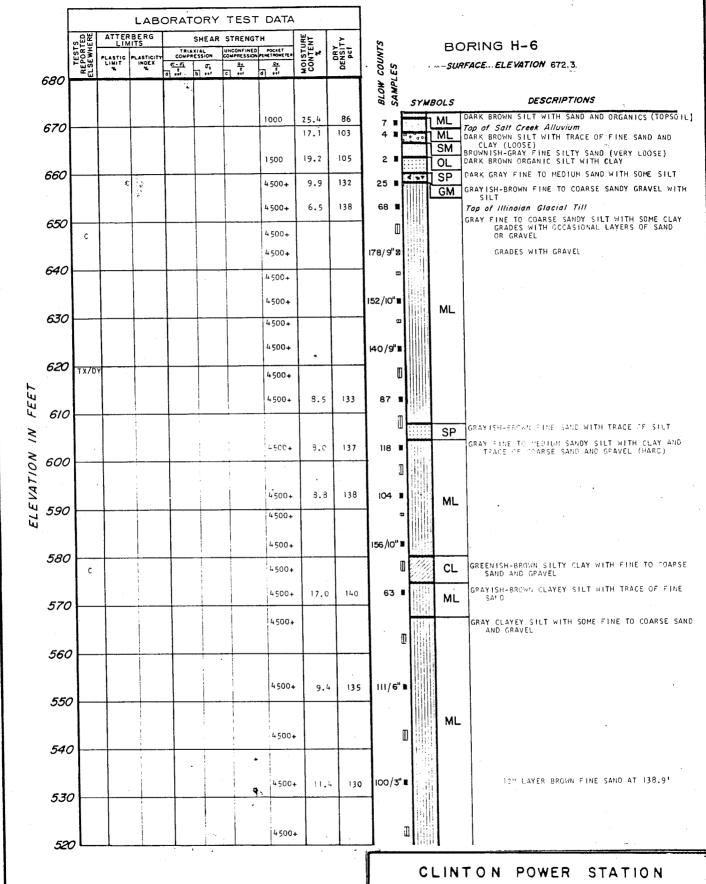


FIGURE 2.5-166

LOG OF BORING H-5

NOTE:



UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-167

LOG OF BORING H-6

(SHEET 1 of 3)

NOTE:

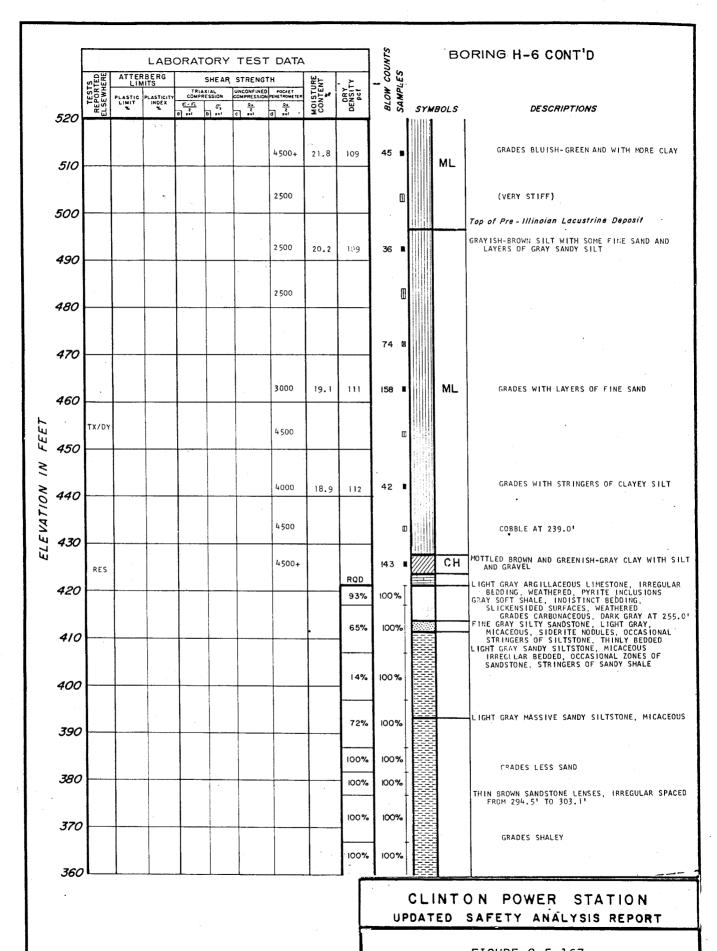


FIGURE 2.5-167

LOG OF BORING H-6

(SHEET 2 of 3)

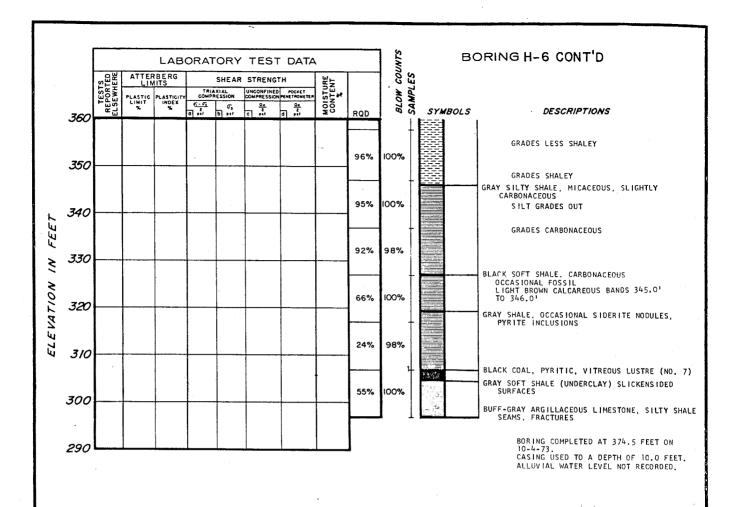


FIGURE 2.5-167

LOG OF BORING H-6

(SHEET 3 of 3)

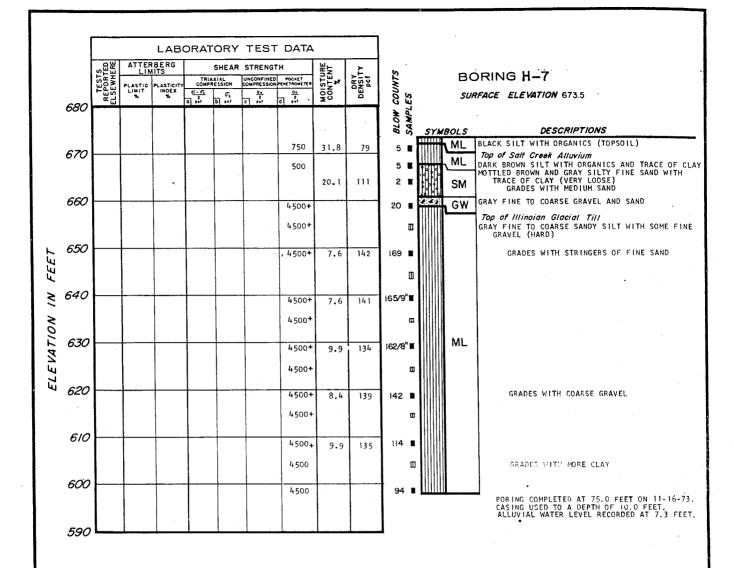


FIGURE 2.5-168

LOG OF BORING H-7

NOTE:

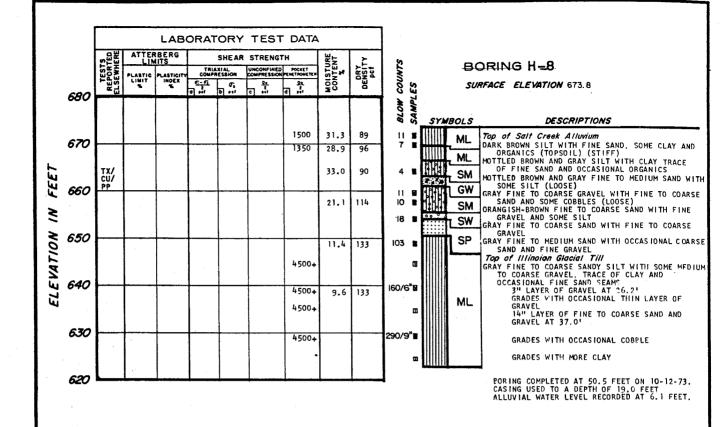
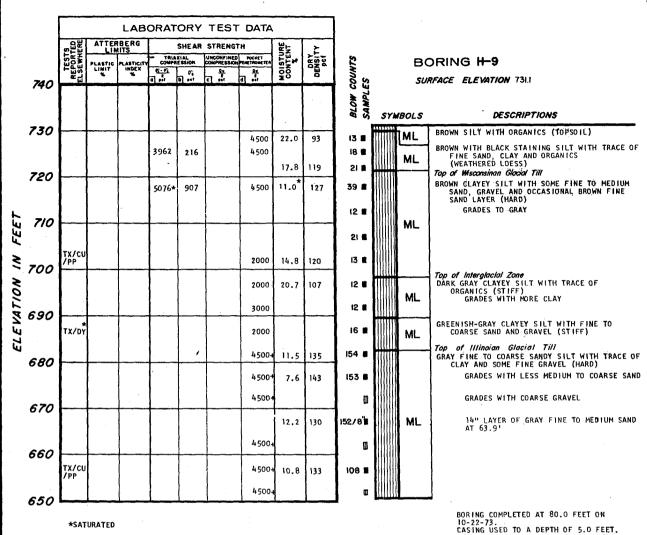


FIGURE 2.5-169

LOG OF BORING H-8

NOTE:



BORING COMPLETED AT 80.0 FEET ON 10-22-73. CASING USED TO A DEPTH OF 5.0 FEET. ALLUVIAL WATER LEVEL NGT RECORDED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-170

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

LOG OF BORING H-9

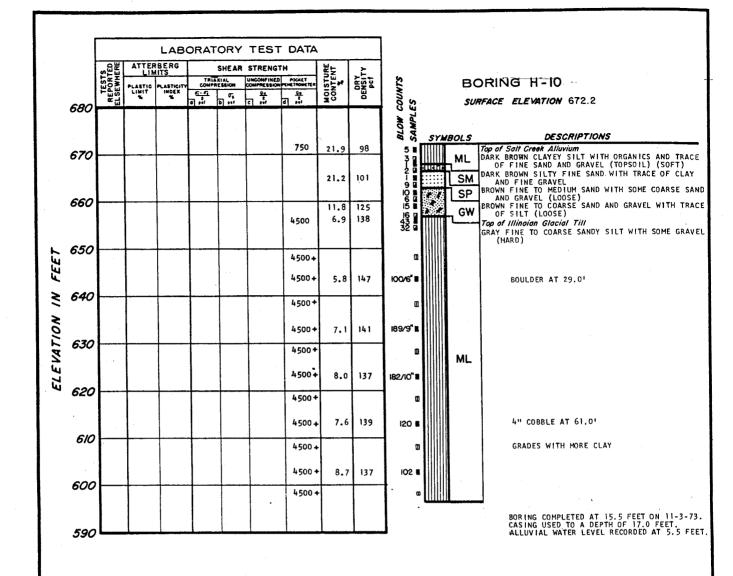


FIGURE 2.5-171

LOG OF BORING H-10

NOTÉ:

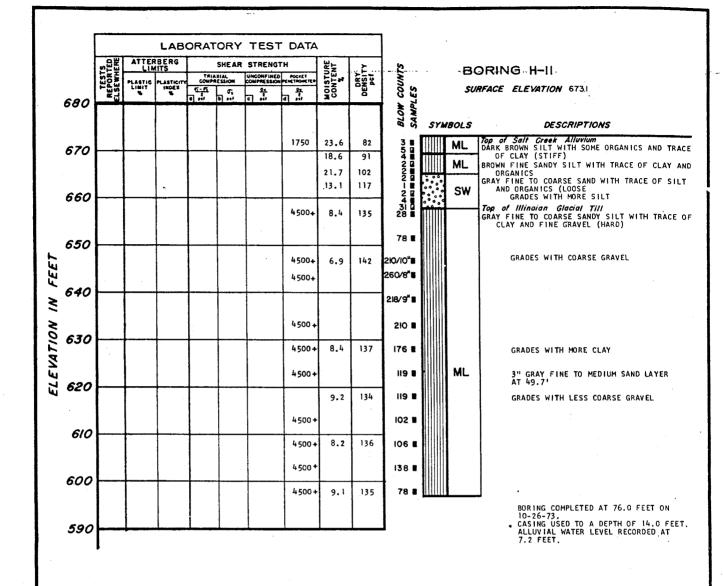


FIGURE 2.5-172

LOG OF BORING H-11

FIGURE 2.5-173

LOG OF BORING H-12

NOTE:

ELEVATION IN

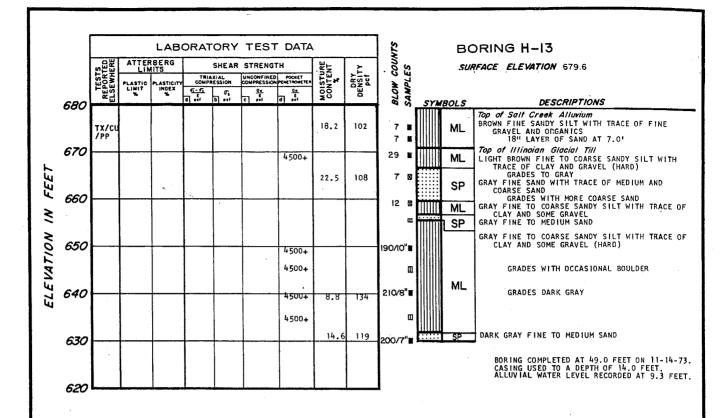


FIGURE 2.5-174

LOG OF BORING H-13

NOTE:

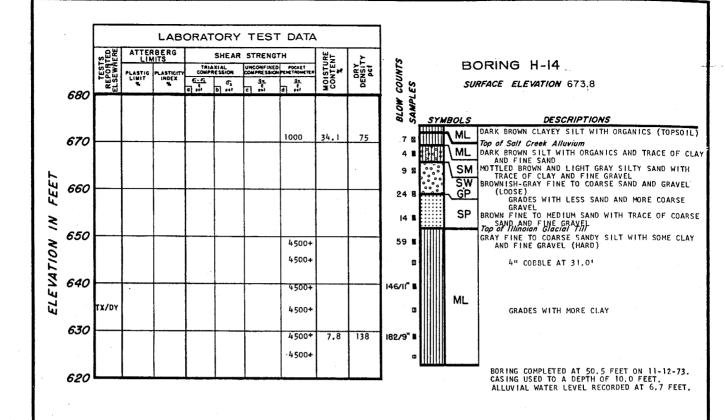


FIGURE 2.5-175

LOG OF BORING H-14

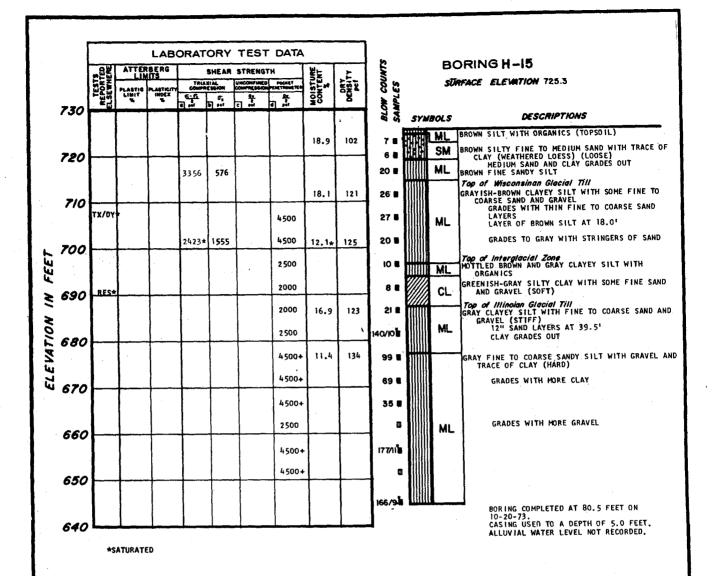


FIGURE 2.5-176

LOG OF BORING H-15

NOTE:

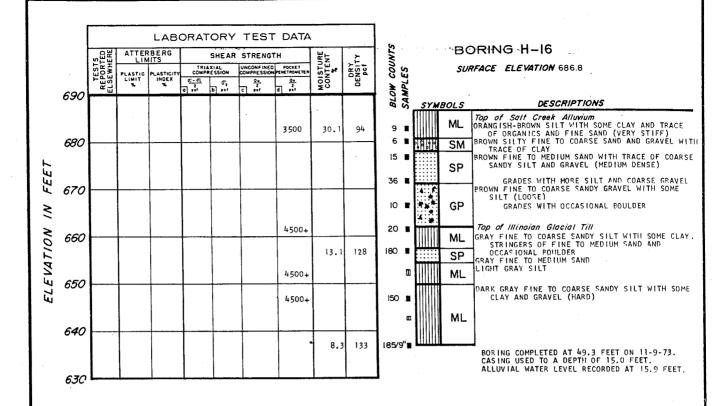


FIGURE 2.5-177

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

LOG OF BORING H-16

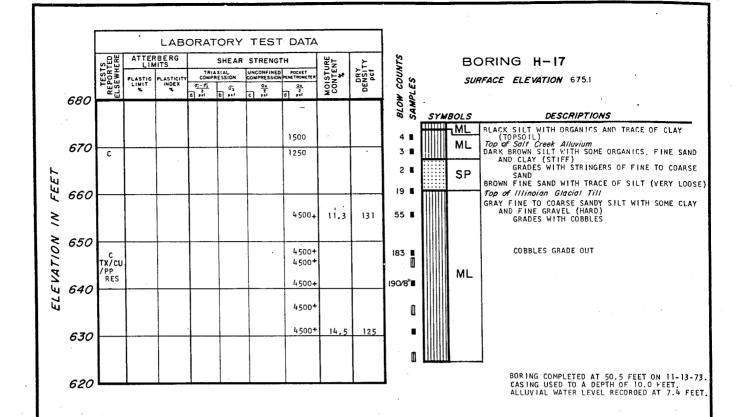


FIGURE 2.5-178

LOG OF BORING H-17

NOTE:

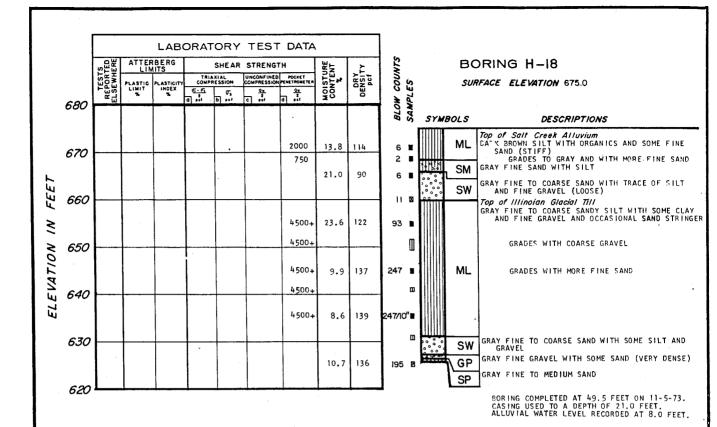


FIGURE 2.5-179

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

LOG OF BORING H-18

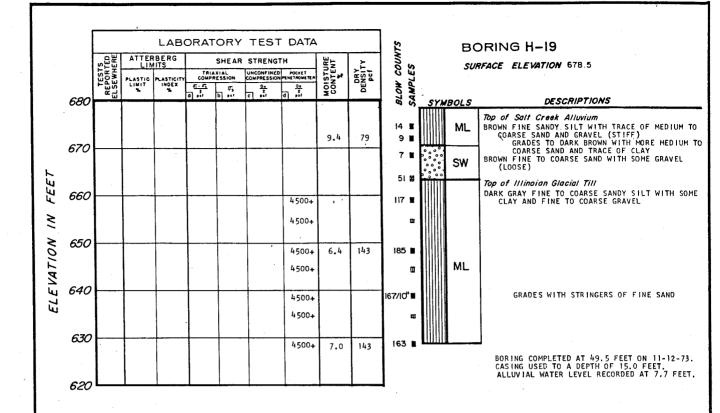
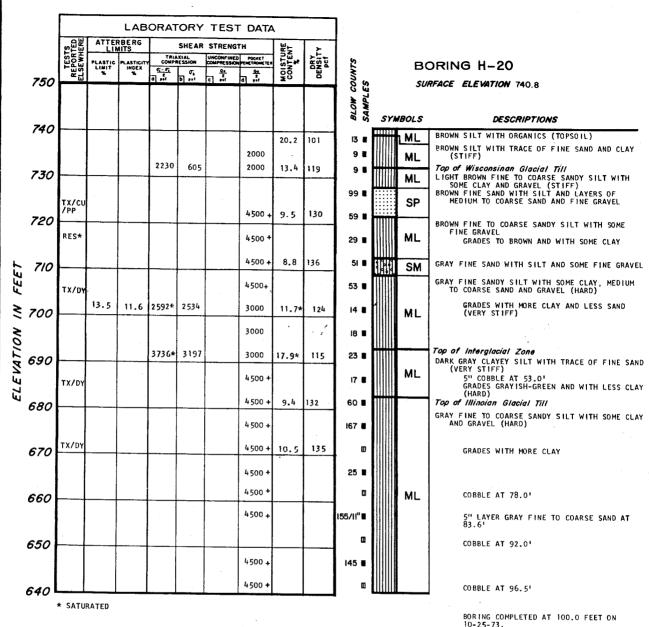


FIGURE 2.5-180

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

LOG OF BORING H-19



BORING COMPLETED AT 100.0 FEET ON 10-25-73.
CASING USED TO A DEPTH OF 5.0 FEET. GROUNDWATER LEVEL NOT RECORDEJ.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-181

LOG OF BORING H-20

NOTE:

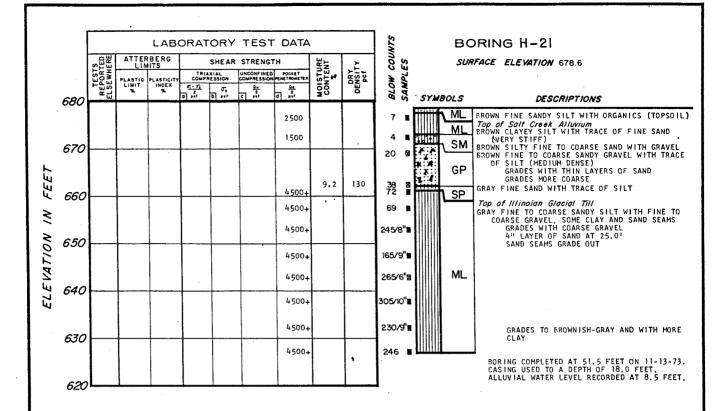
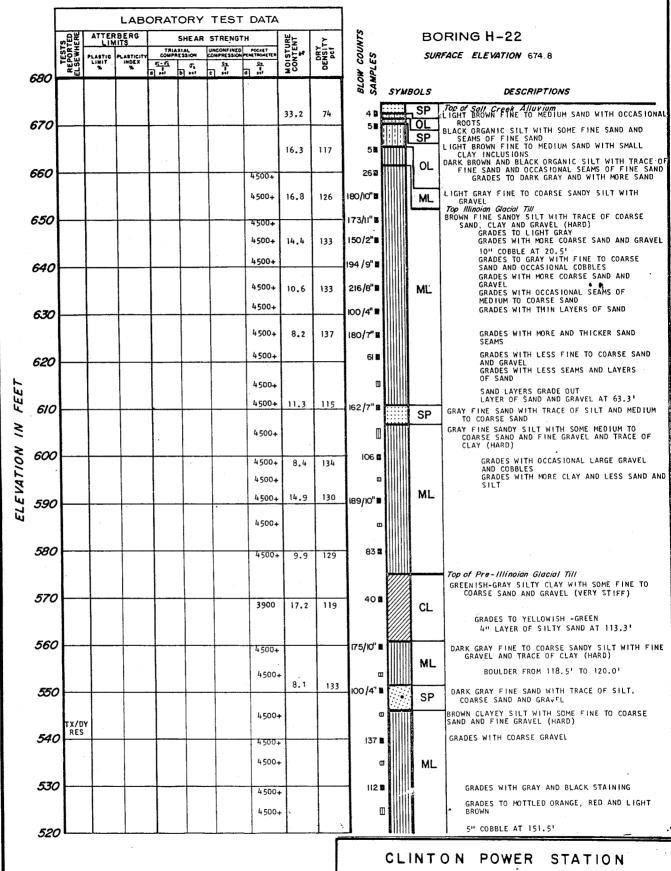


FIGURE 2.5-182

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

LOG OF BORING H-21



UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-183

LOG OF BORING H-22 (SHEET 1 of 2)

NOTE:

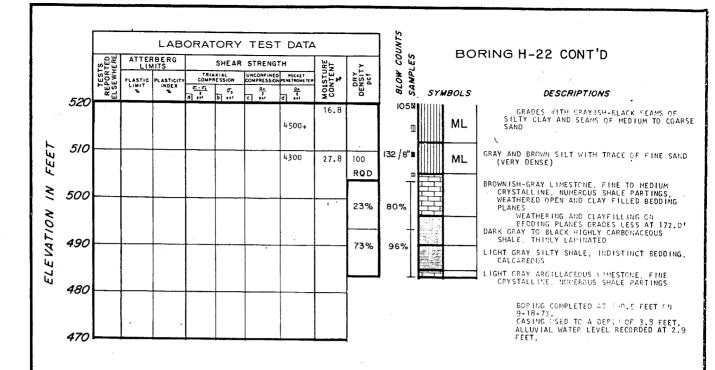


FIGURE 2.5-183

LOG OF BORING H-22 (SHEET 2 of 2)

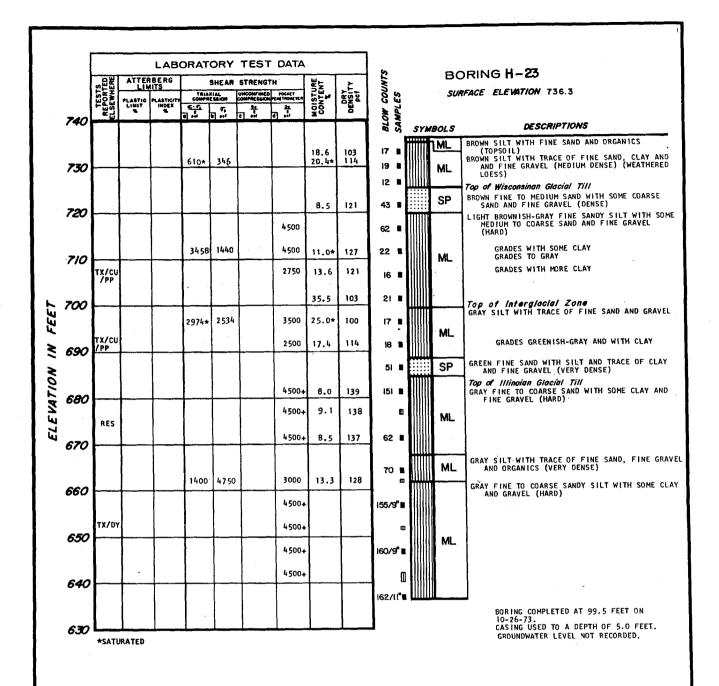


FIGURE 2.5-184

NOTE

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

LOG OF BORING H-23

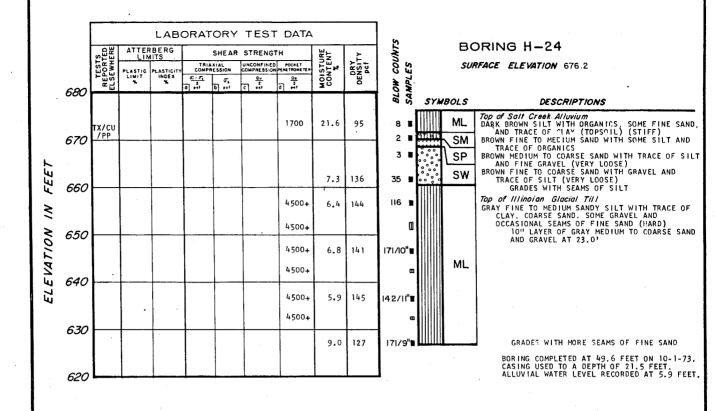


FIGURE 2.5-185

LOG OF BORING H-24

NOTE:

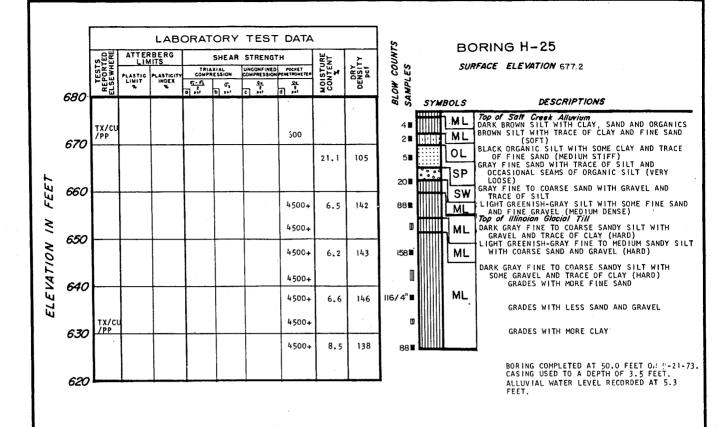


FIGURE 2.5-186

LOG OF BORING H-25

NOTE:

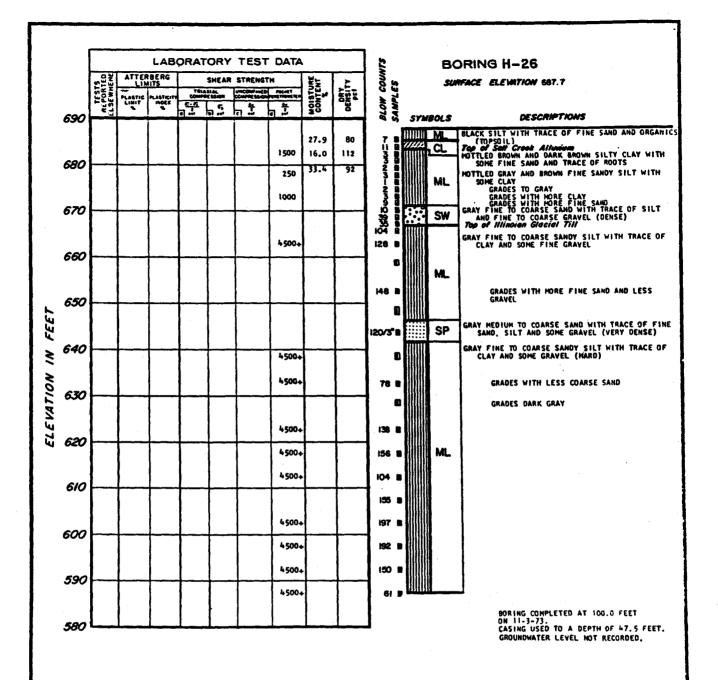


FIGURE 2.5-187

LOG OF BORING H-26

NOTE:

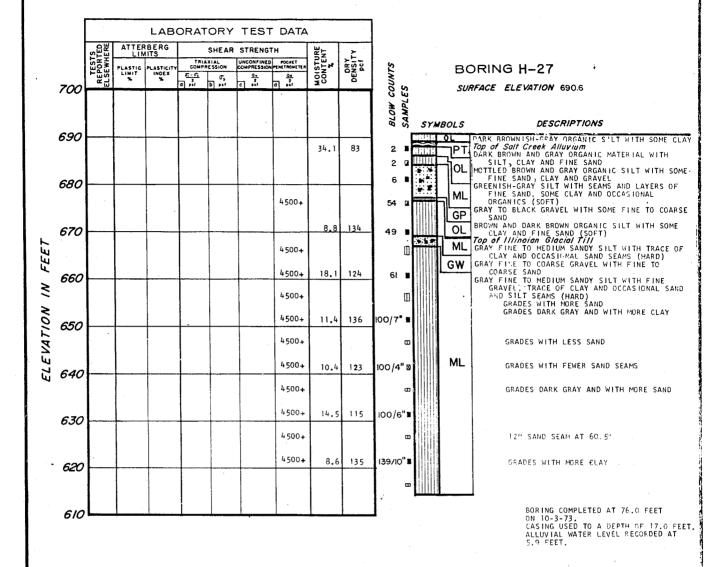


FIGURE 2.5-188

LOG OF BORING H-27

NOTE:

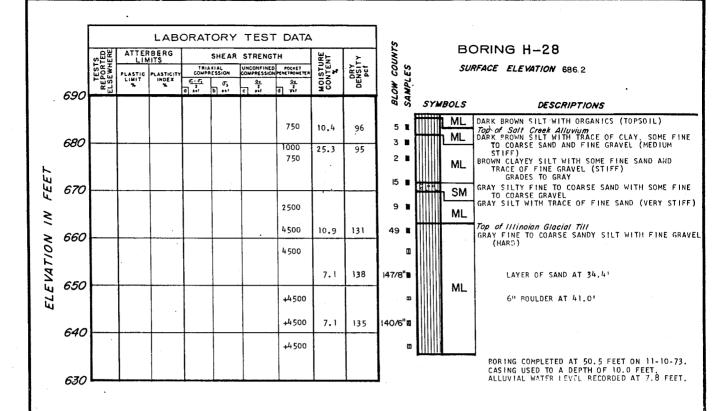


FIGURE 2.5-189

LOG OF BORING H-28

NOTE:

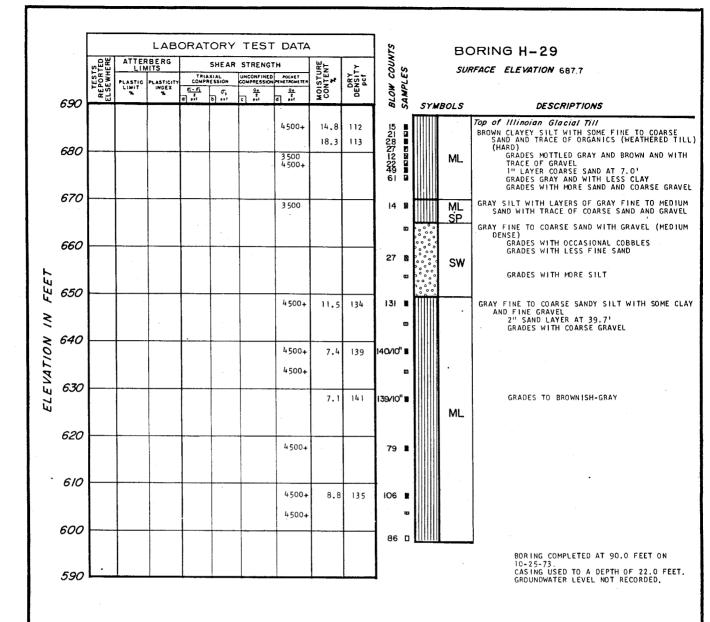
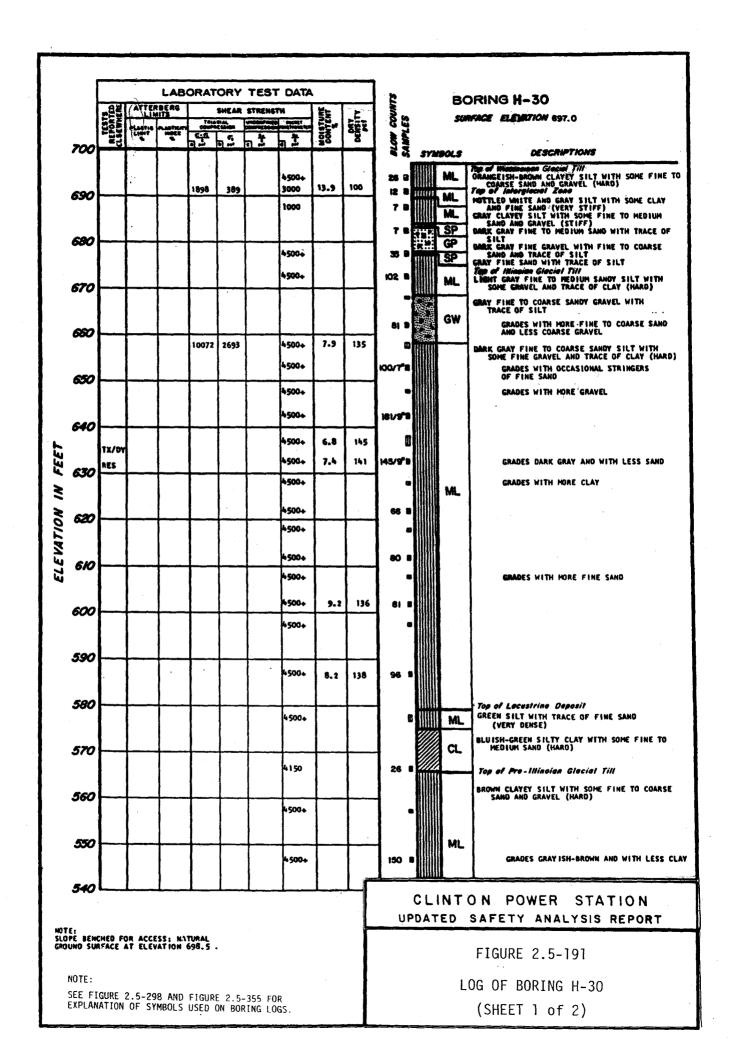


FIGURE 2.5-190

LOG OF BORING H-29

#### NOTES:

SLOPE BENCHED FOR ACCESS; NATURAL GROUND SURFACE AT ELEVATION 689.7.



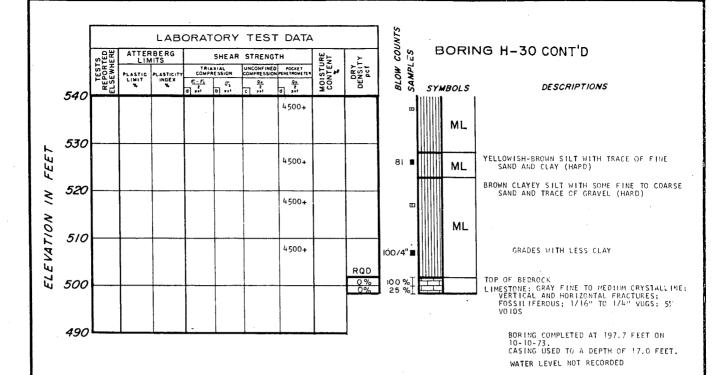
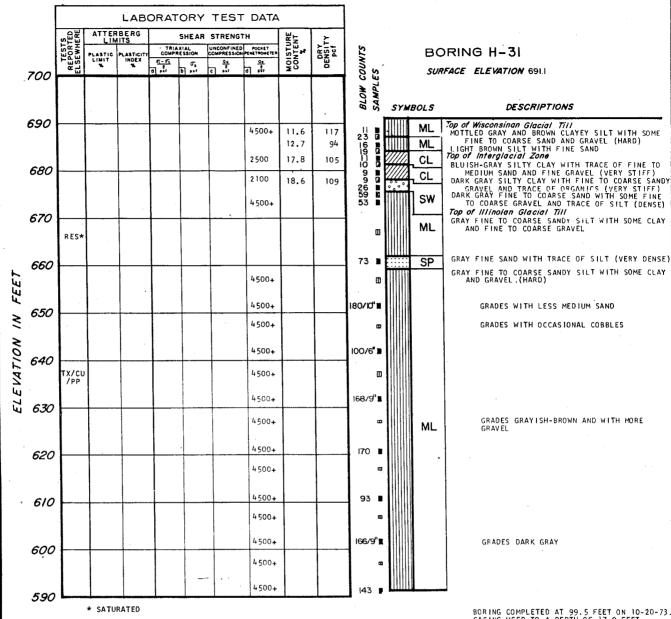


FIGURE 2.5-191

LOG OF BORING H-30 (SHEET 2 of 2)



BORING COMPLETED AT 99.5 FEET ON 10-20-73. CASING USED TO A DEPTH OF 17.0 FEET. GROUNDWATER LEVEL NOT RECORDED.

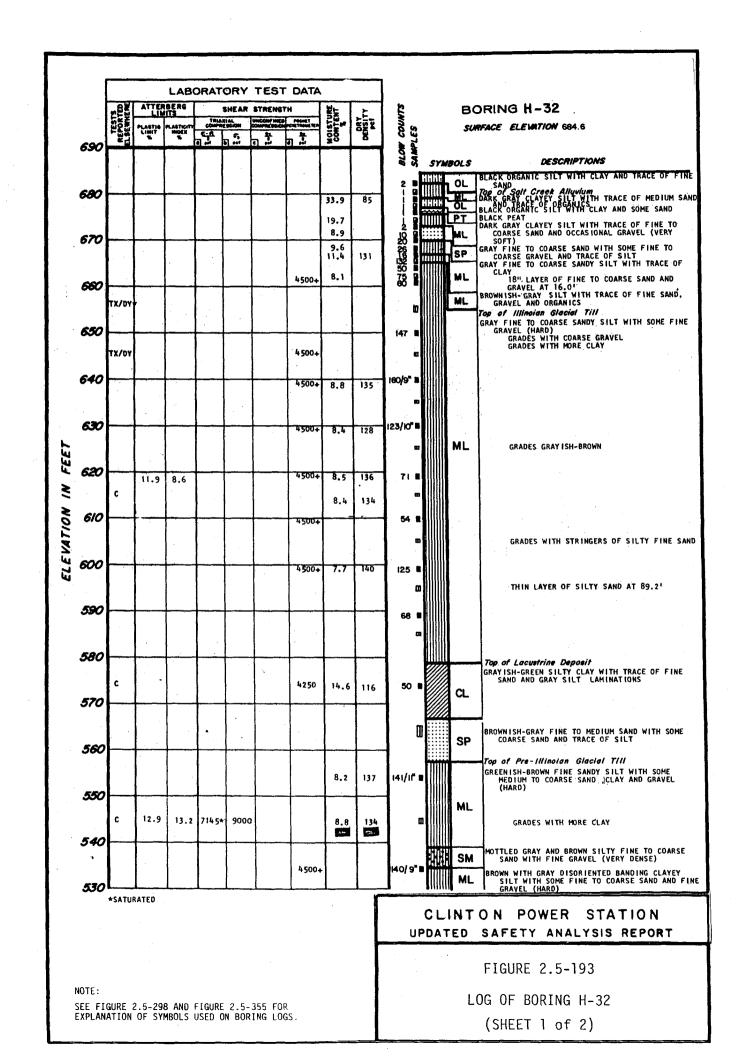
### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-192

LOG OF BORING H-31

#### NOTES:

SLOPE BENCHED FOR ACCESS; NATURAL GROUND SURFACE AT ELEVATION 692.6.



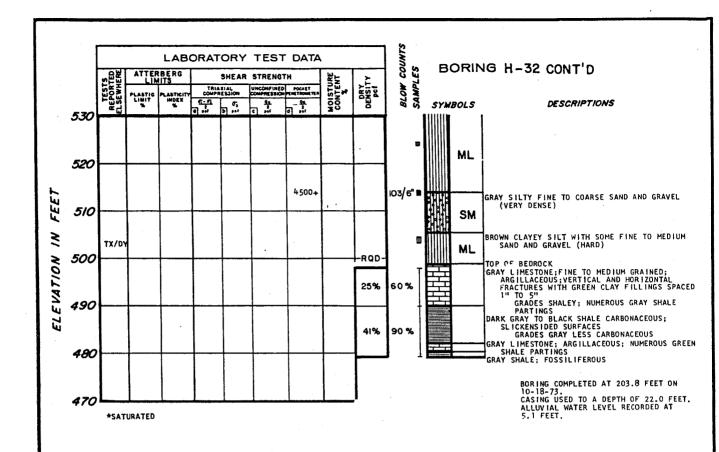


FIGURE 2.5-193

LOG OF BORING H-32

(SHEET 2 of 2)

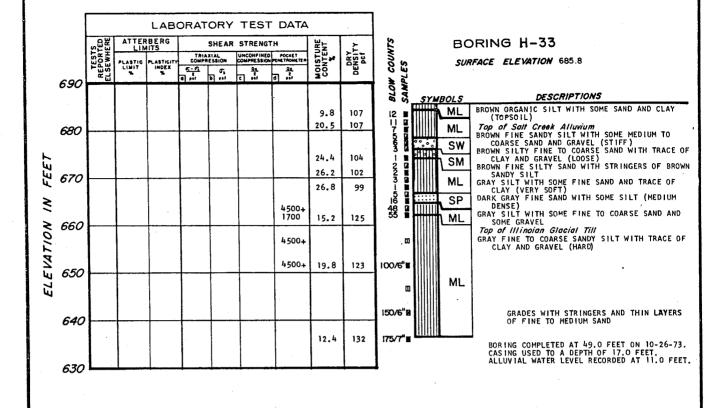


FIGURE 2.5-194

LOG OF BORING H-33

NOTE:

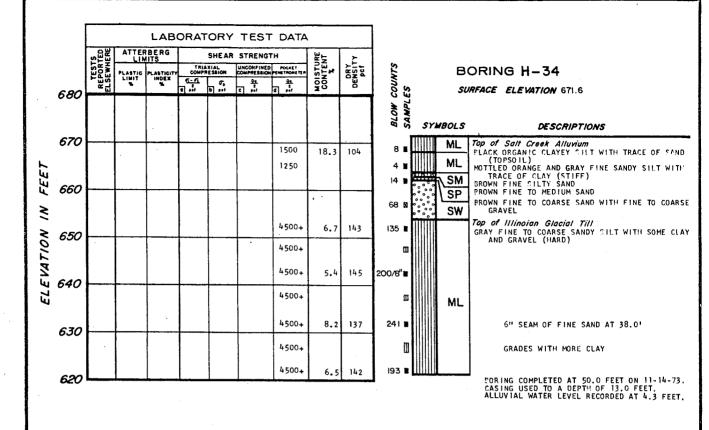
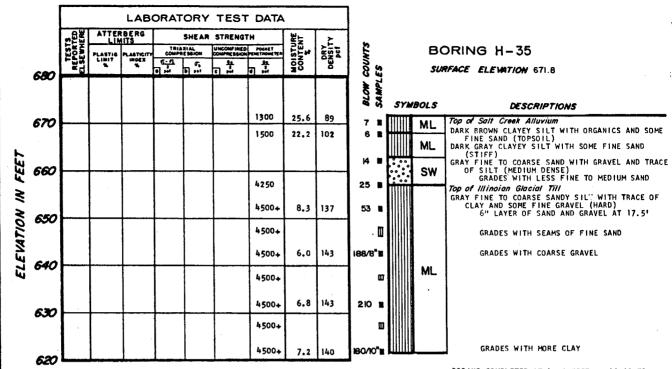


FIGURE 2.5-195

LOG OF BORING H-34

NOTE:



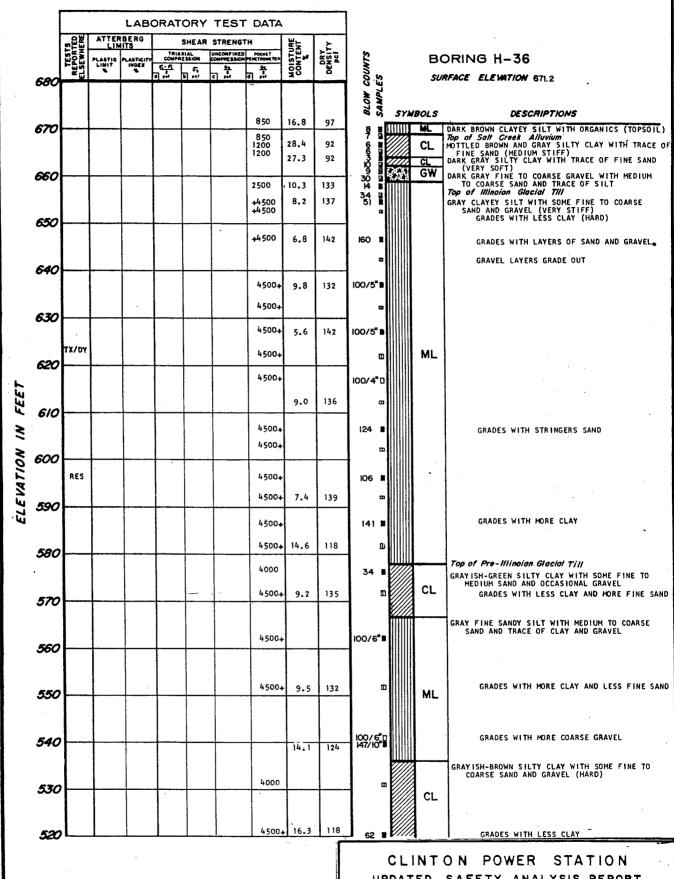
BORING COMPLETED AT 49.9 FEET ON 11-13-73. CASING USED TO A DEPTH OF 14.5 FEET. ALLUVIAL WATER LEVEL RECORDED AT 3.4 FEET.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-196

LOG OF BORING H-35

NOTE:

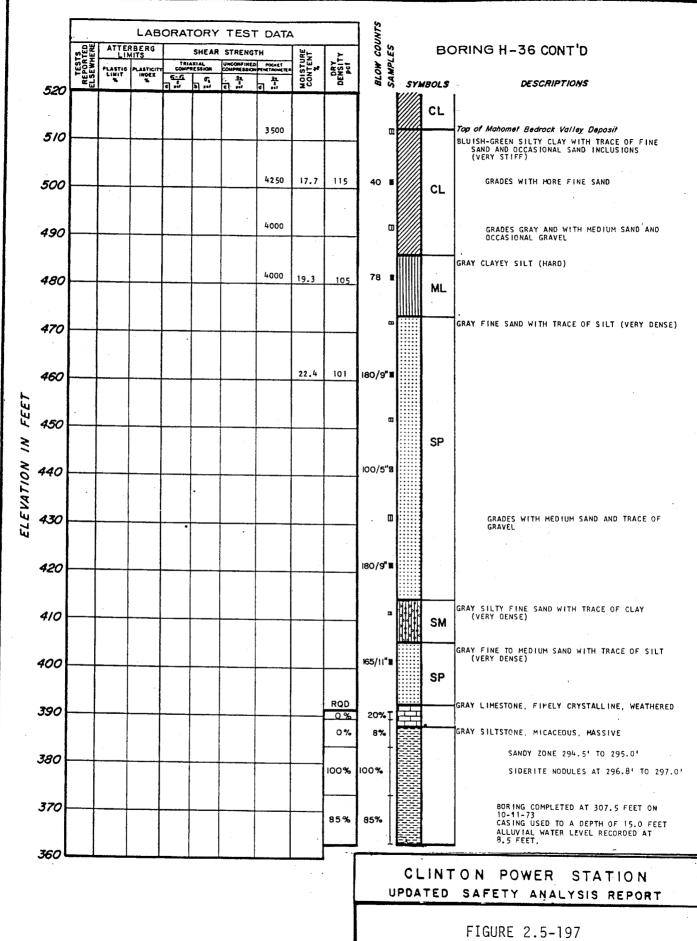


SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

SAFETY ANALYSIS REPORT UPDATED

FIGURE 2.5-197

LOG OF BORING H-36 (SHEET 1 of 2)



LQG OF BORING H-36

(SHEET 2 of 2)

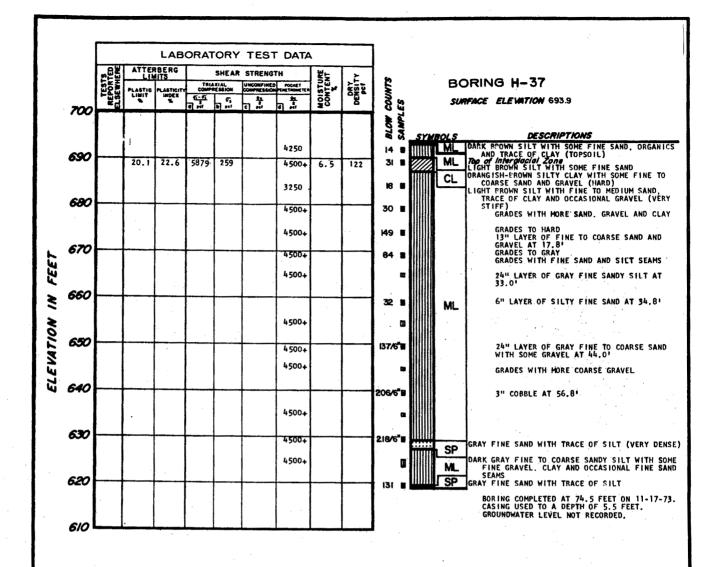


FIGURE 2.5-198

LOG OF BORING H-37

NOTE:

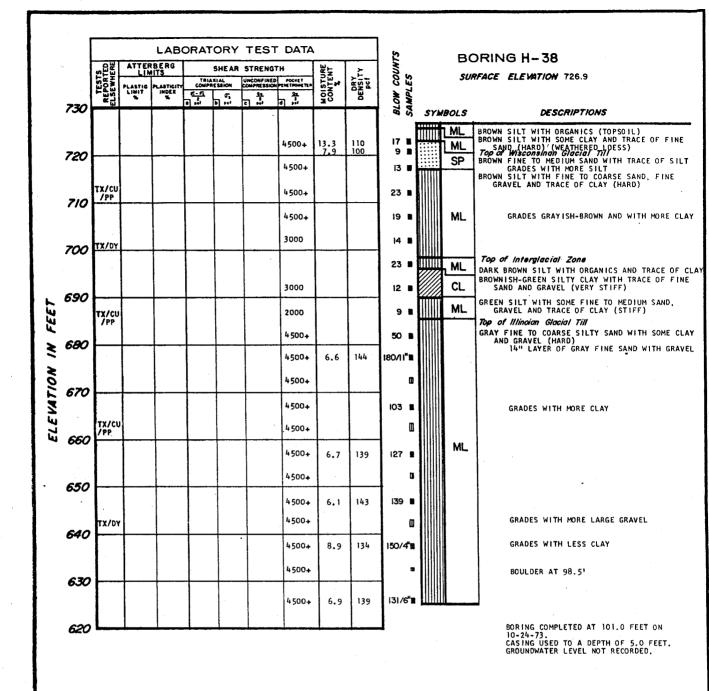


FIGURE 2.5-199

NOTE

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

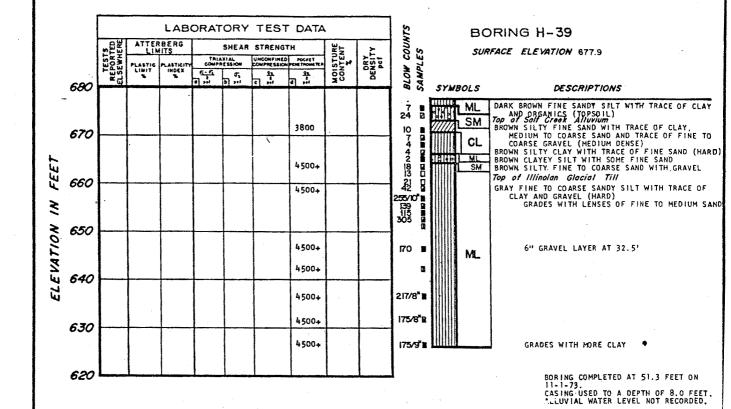


FIGURE 2.5-200

LOG OF BORING H-39

NOTE:

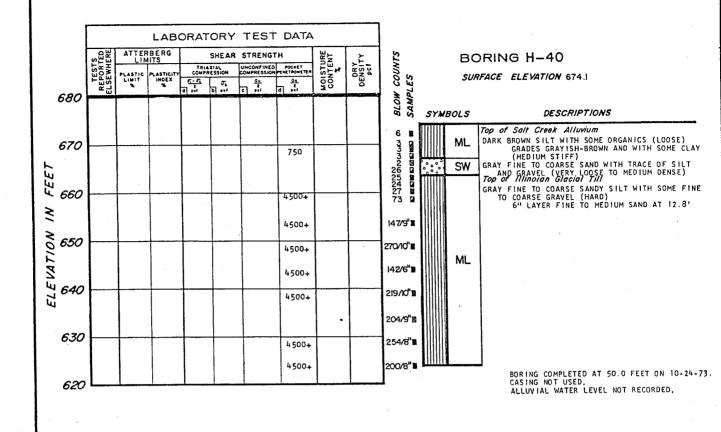


FIGURE 2.5-201

LOG OF BORING H-40

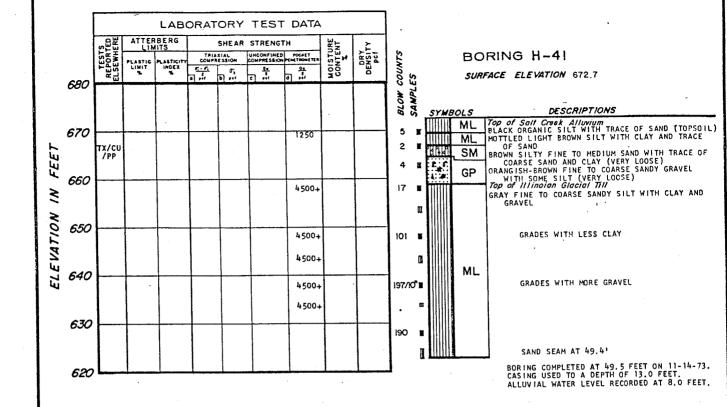


FIGURE 2.5-202

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

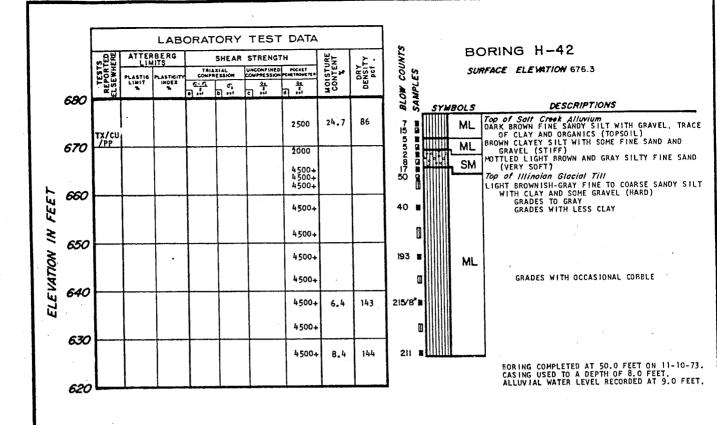
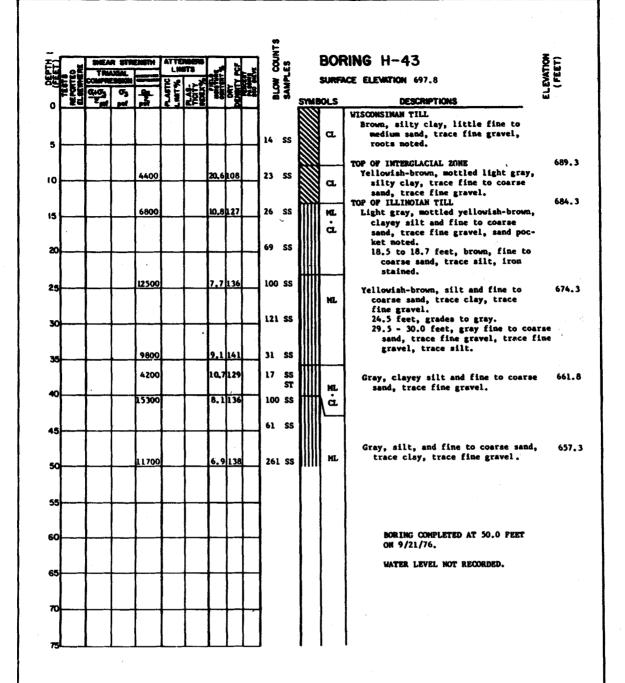


FIGURE 2.5-203

NOTE:

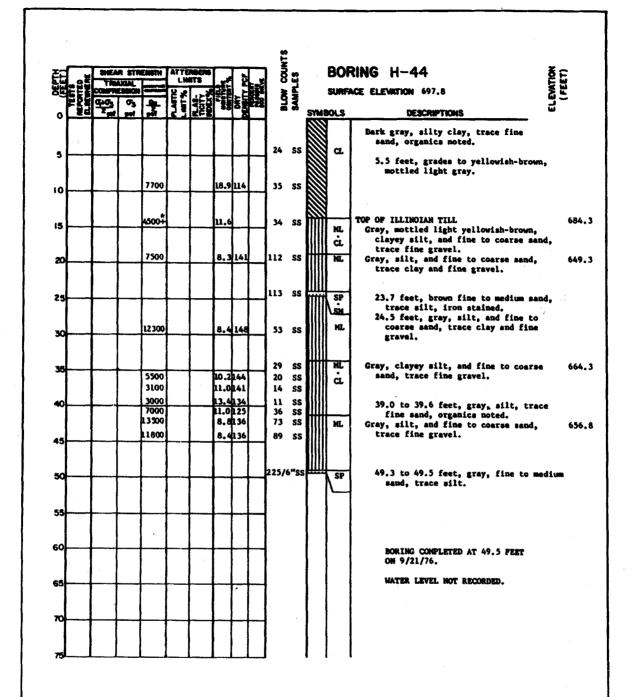
SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.



- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-204



- 1. LOGGED BY: SARGENT & LANDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 1. TESTED BY: WESTENHOFF & NOVICK.

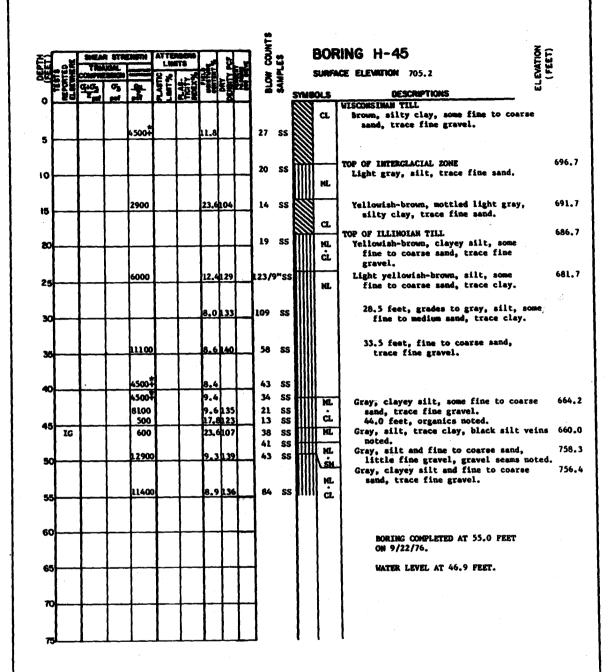
# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-205

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & MOVICK.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-206

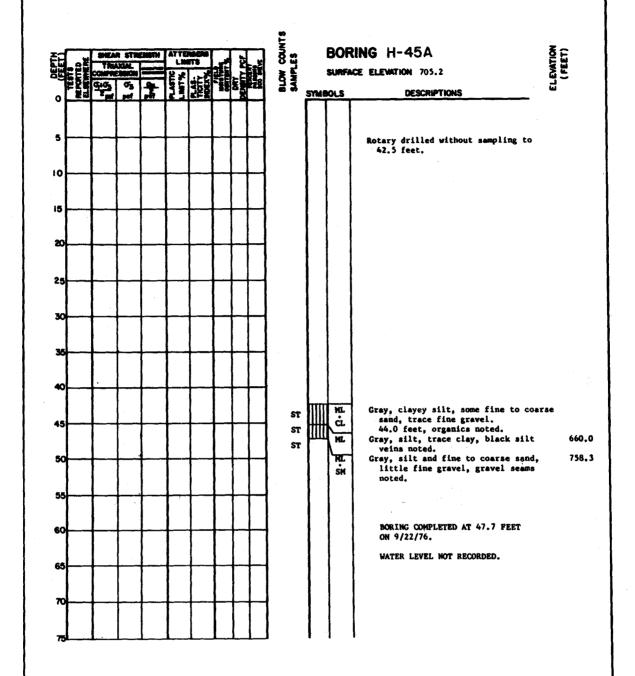


#### MOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

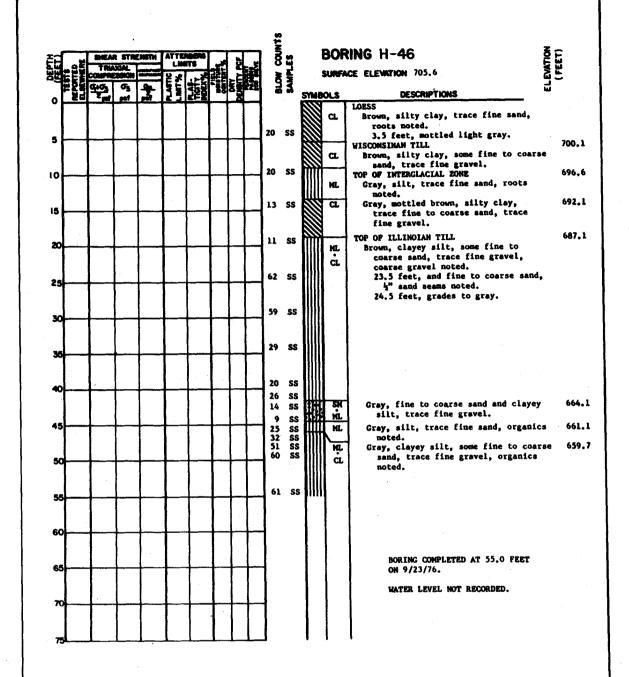
FIGURE 2.5-207



- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-208



- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-209

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

WATER LEVEL NOT RECORDED.

697.2

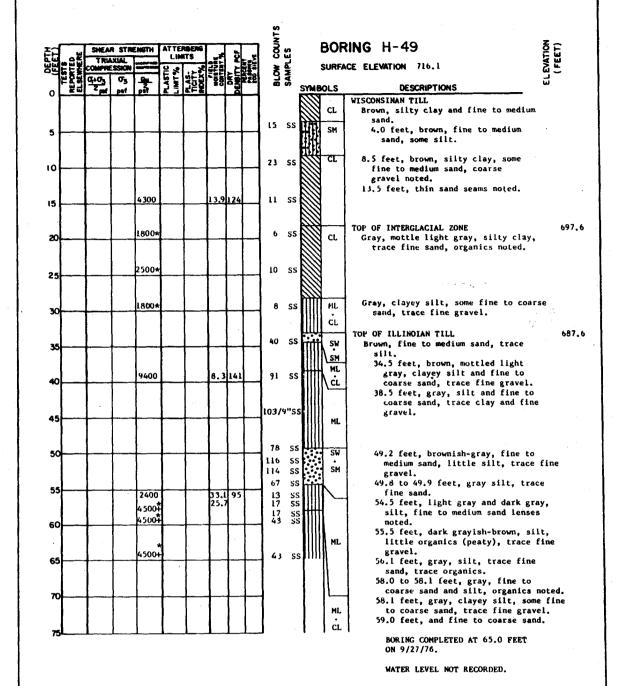
685.2

FIGURE 2.5-210

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

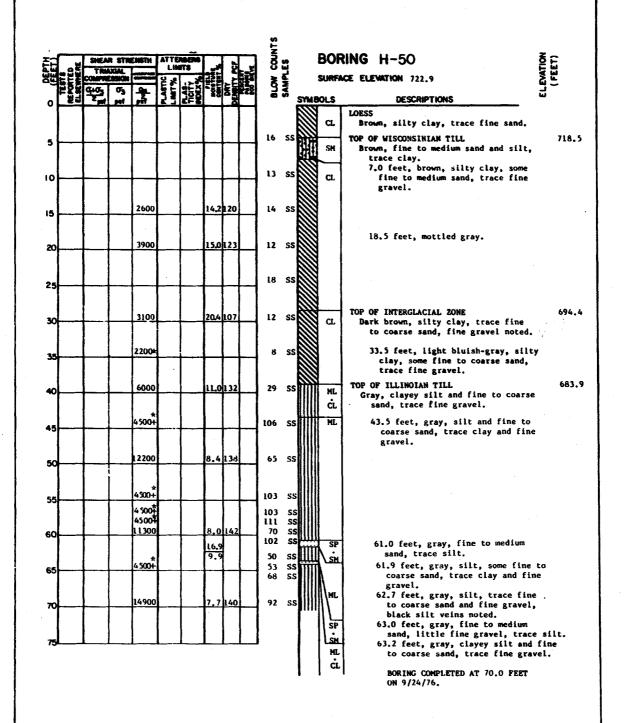
FIGURE 2.5-211



- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

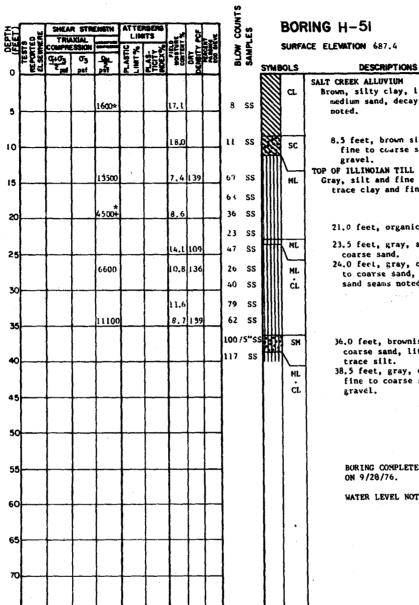
FIGURE 2.5-212



- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INCERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-213



SURFACE ELEVATION 687.4

SALT CREEK ALLUVIUM

Brown, silty clay, little fine to medium sand, decaying organics

8.5 feet, brown silty clay and fine to coarse sand, little fine gravel.

TOP OF ILLIMOIAN TILL
Gray, silt and fine to coarse sand,
trace clay and fine gravel.

676.4

21.0 feet, organics noted.

23.5 feet, gray, silt, trace fine to coarse sand.

24.0 feet, gray, clayey silt and fine to coarse sand, trace fine gravel, sand seams noted.

36.0 feet, brownish-gray, fine to coarse sand, little fine gravel, trace silt.

38.5 feet, gray, clayey silt and fine to coarse sand, trace fine gravel.

BURING COMPLETED AT 40.0 FEET ON 9/28/76.

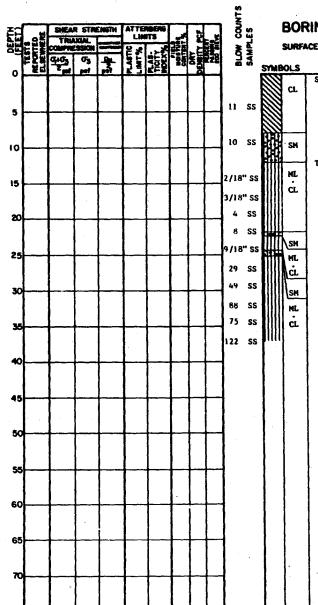
WATER LEVEL NOT RECORDED.

### NOTES

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-214



BORING H-52

SURFACE ELEVATION 686.5

DESCRIPTIONS SALT CREEK ALLUVIUM Black, silty clay, trace fine to medium sand, organics noted.

8.5 feet, brown, fine to coarse sand, some silt, little fine to coarse gravel.
TOP OF ILLINOIAN TILL

Gray, clayey silt and fine to medium sand, sand lenses noted.
16.0 feet, grades to gray.

18.5 feet, trace fine gravel.

21.5 feet, gray, fine to medium sand, some silt.

22.4 feet, gray, clayey silt and fine to medium sand, trace fine gravel.

24.5 feet, gray, fine to medium sand, some silt.
25.0 feet, gray, clayey silt and fine to coarse sand, trace fine gravel.

BORING COMPLETED AT 37.5 FEET

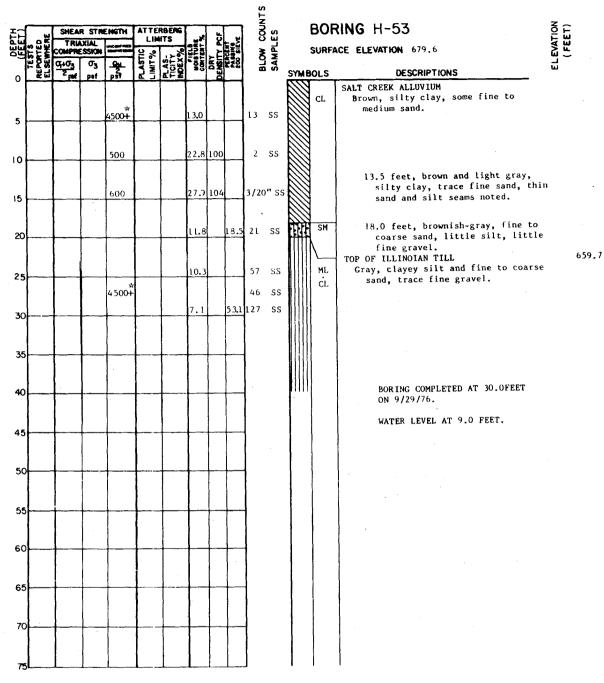
WATER LEVEL AT 8.5 FEET.

#### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

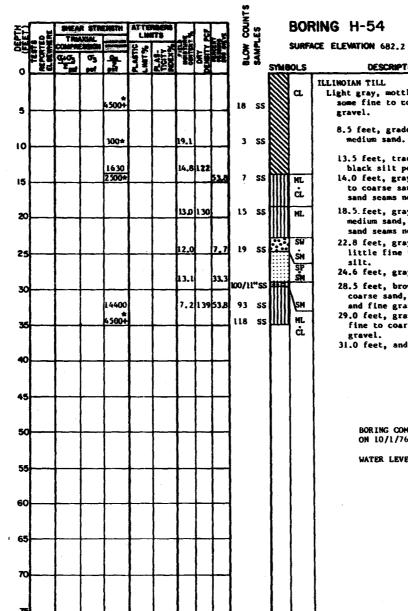
FIGURE 2.5-215



- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-216



**BORING H-54** 

ILLINOIAN TILL Light gray, mottled brown, silty clay, some fine to coarse sand, trace fine gravel.

8.5 feet, grades to gray, and fine to medium sand.

13.5 feet, trace fine to coarse sand, black silt pockets noted.14.0 feet, gray, clayey silt and fine to coarse sand, trace fine gravel, thin sand seams noted.

18.5 feet, gray, silt and fine to medium sand, trace fine gravel, thin sand seams noted.

22.8 feet, gray, fine to coarse sand, little fine to coarse gravel, trace silt.

24.6 feet, gray, fine sand, trace silt.

28.5 feet, brownish gray, fine to coarse sand, some silt, trace clay and fine gravel.

29.0 feet, gray, clayey silt, some fine to coarse sand, trace fine

31.0 feet, and fine to coarse sand.

BORING COMPLETED AT 35.0 FEET ON 10/1/76.

WATER LEVEL AT 8.5 FEET.

#### NOTES:

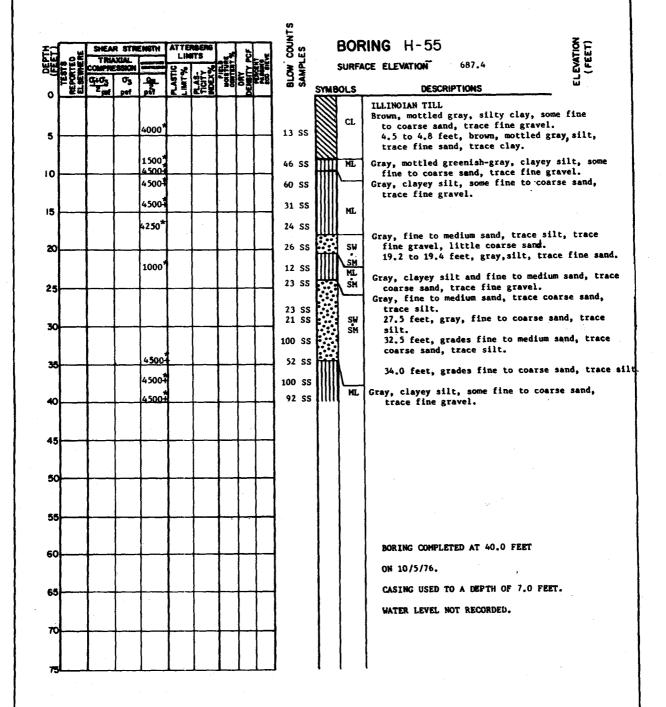
1. LOGGED BY: SARGENT & LUNDY.

2. DRILLED BY: RAYMOND INTERNATIONAL.

3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-217



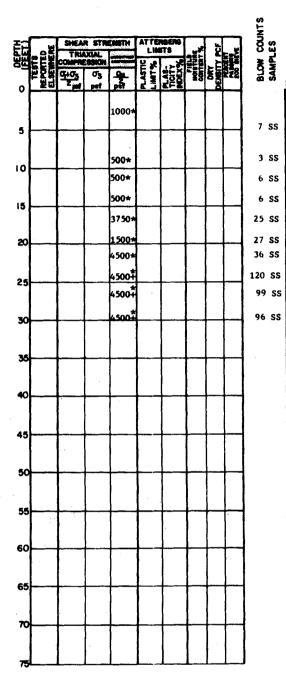
1. LOGGED BY: SARGENT & LUNDY.

2. DRILLED BY: RAYMOND INTERNATIONAL.

3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-218



## BORING H-56

SYMBOLS

SM

SP ML

SM

SURFACE ELEVATION 680.7

ELEVATION

SM	CALT CREEK ALLUVIUM  Brown, silty, fine to coarse sand, some clay, trace fine gravel.
SM	TOP OF ILLINOIAN TILL 676.2  Brown, clayey silt and fine to coarse sand, trace fine gravel.
ML SM	Gray, clayey silt and fine to medium sand, silt and sand lenses noted.

DESCRIPTIONS

- 9.5 to 9.7 feet, gray, fine to medium sand, some silt.
  9.7 to 12.0 feet, gray, silt, some clay, trace fine sand, trace fine gravel.
  12.0 feet, gray, clayey silt and fine to coarse sand, trace fine gravel.
- 17.2 feet, gray, silt, trace fine sand, fine gravel and organics noted.
- Gray, clayey silt and fine to coarse sand, trace fine gravel, coarse gravel noted.
- 26.0 to 26.6 feet, gray, fine to medium sand, trace silt.
  Gray, clayey silt and fine to coarse sand, trace fine gravel.

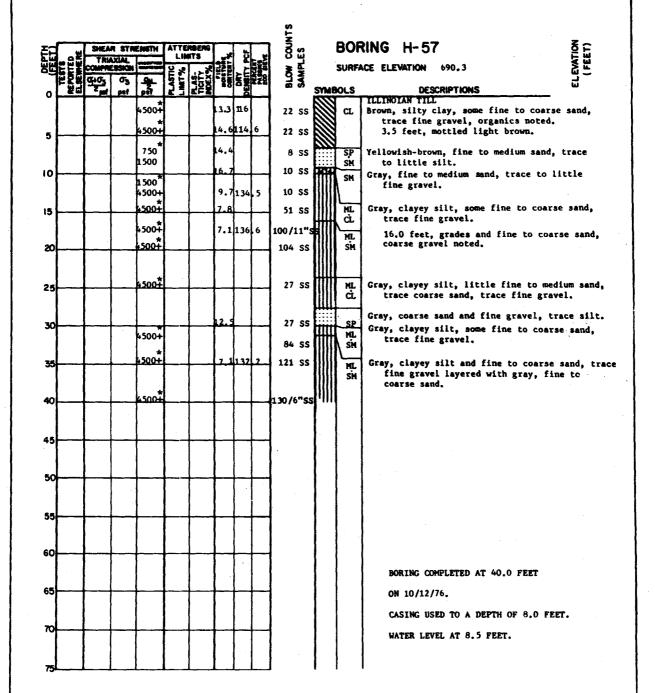
BORING COMPLETED AT 30.0 FEET
ON 10/5/76.
CASING USED TO A DEPTH OF 10.0 FEET.
WATER LEVEL AT 7.0 FEET.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-219



- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

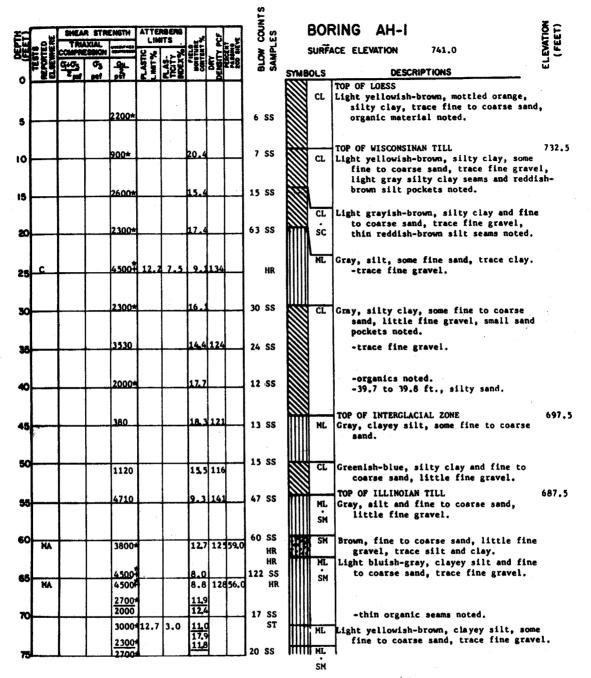
# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-220

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-221



Boring continued.....

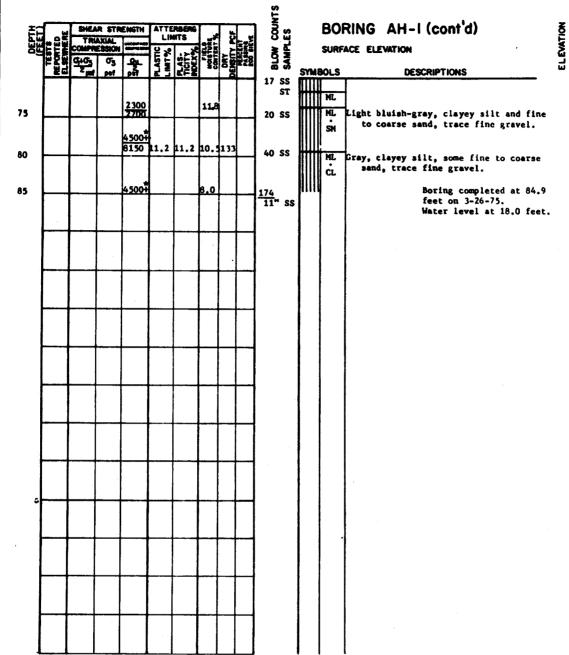
#### NOTES

Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-222

LOG OF BORING AH-1
(SHEET 1 of 2)



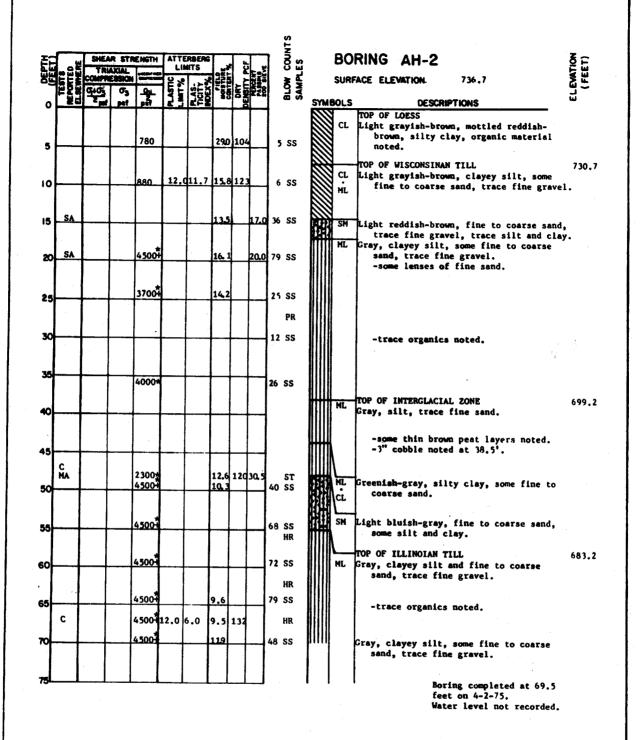
Logged by: Sargent & Lundy Drilled by: Raymond International Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-222

LOG OF BORING AH-1

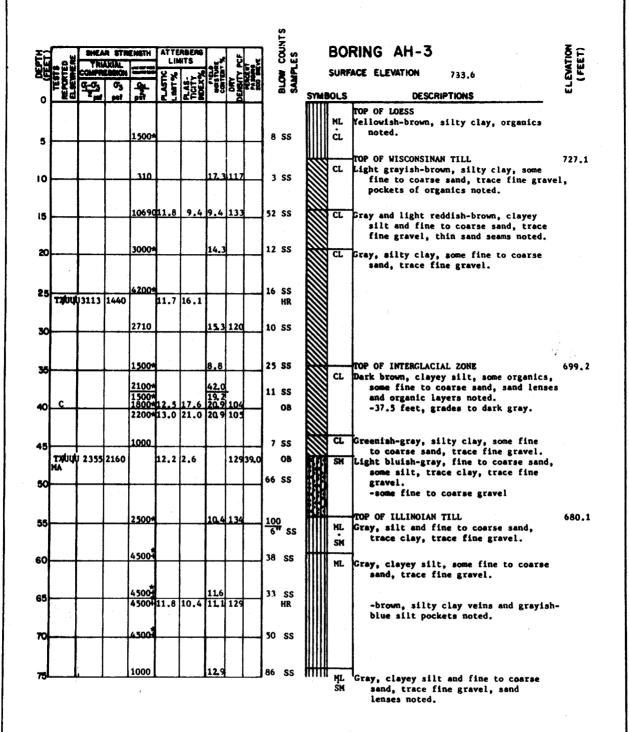
(SHEET 2 of 2)



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-223



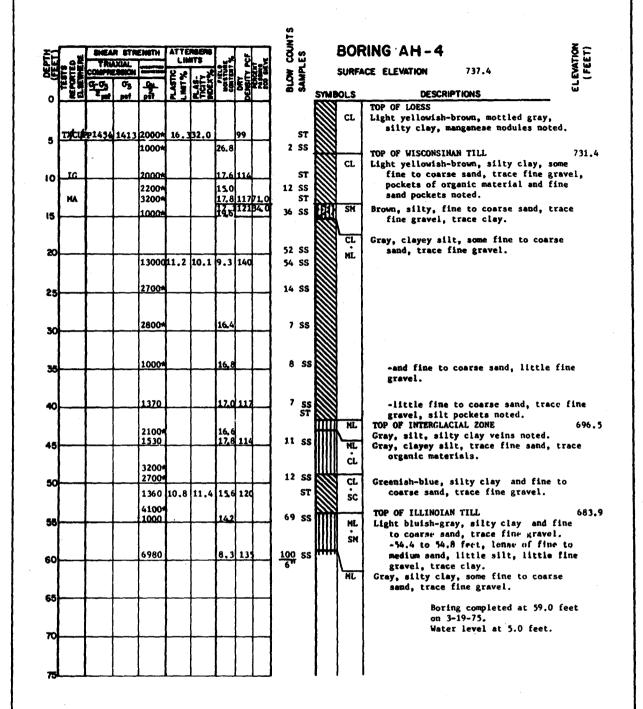
Boring completed at 75.0 feet on 4-4-75. Water level at 3.0 feet.

#### NOTES

Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

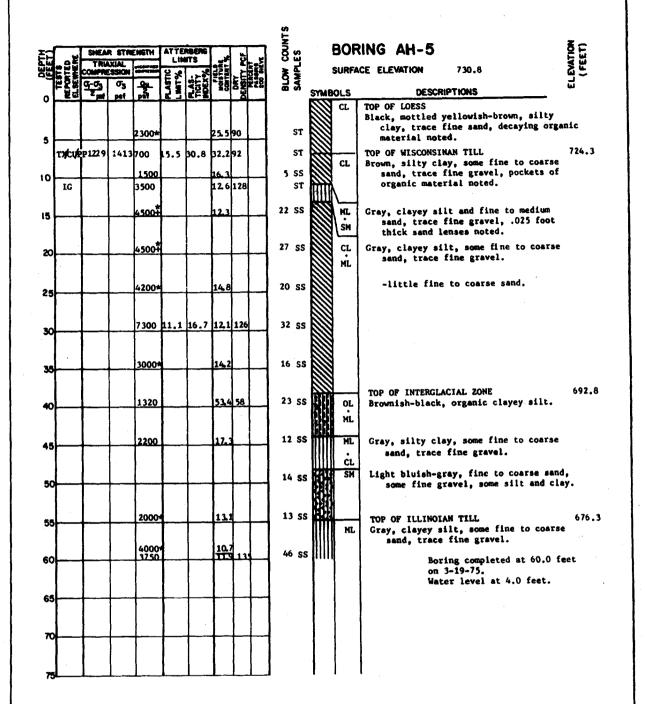
FIGURE 2.5-224



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

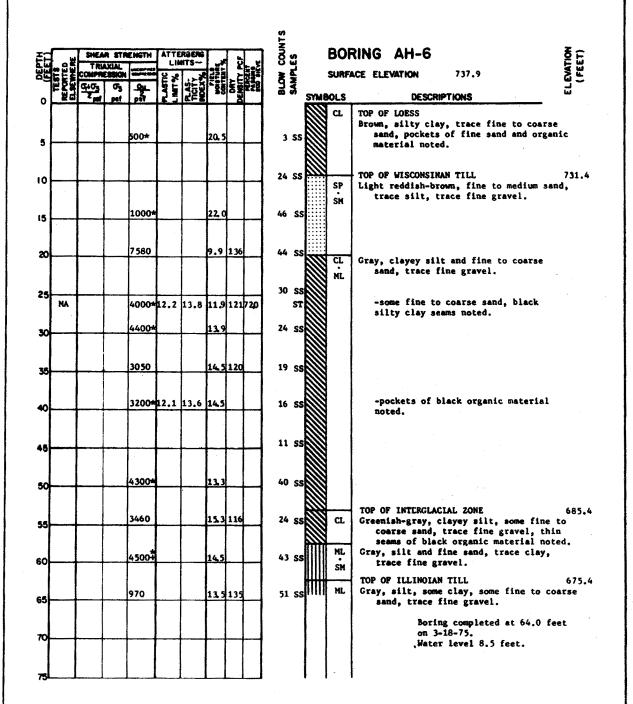
FIGURE 2.5-225



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

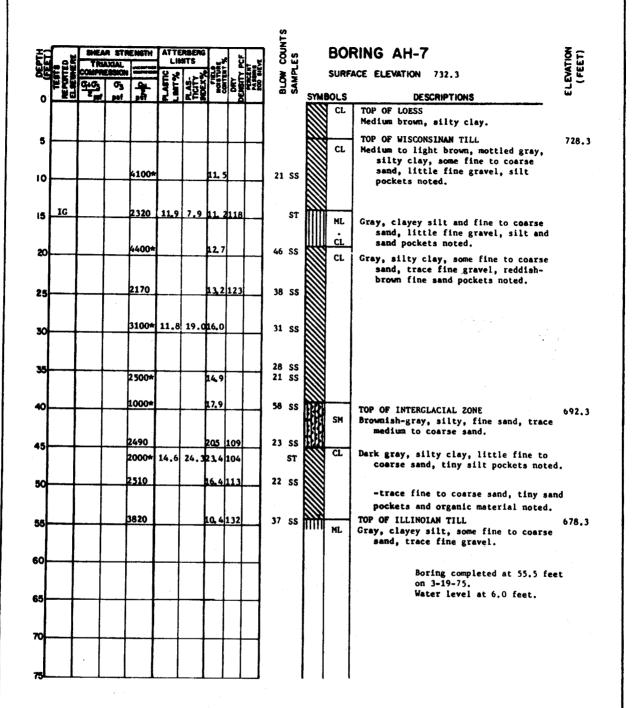
FIGURE 2.5-226



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

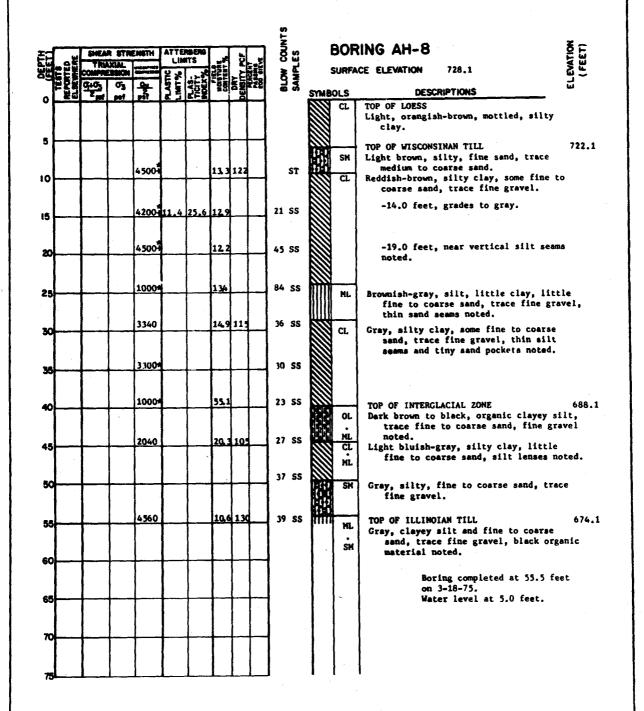
FIGURE 2.5-227



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-228



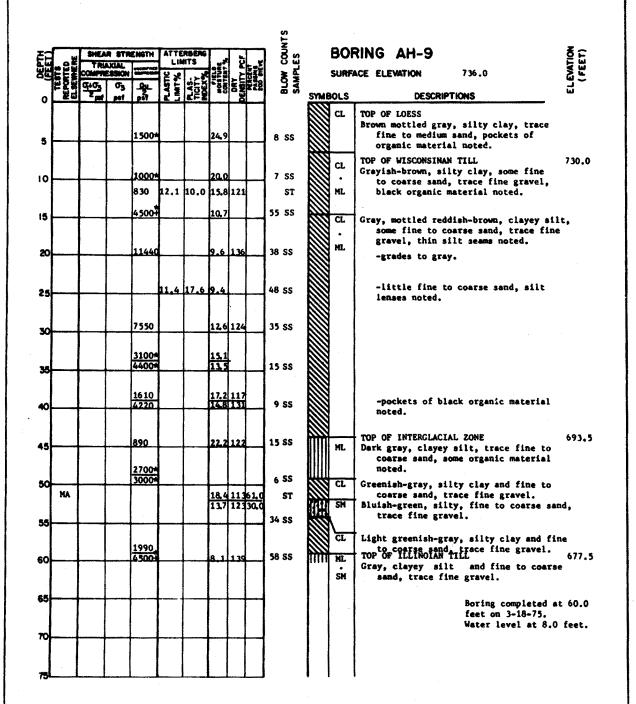
Logged by: Sargent & Lundy

Drilled by: Raymond International

Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

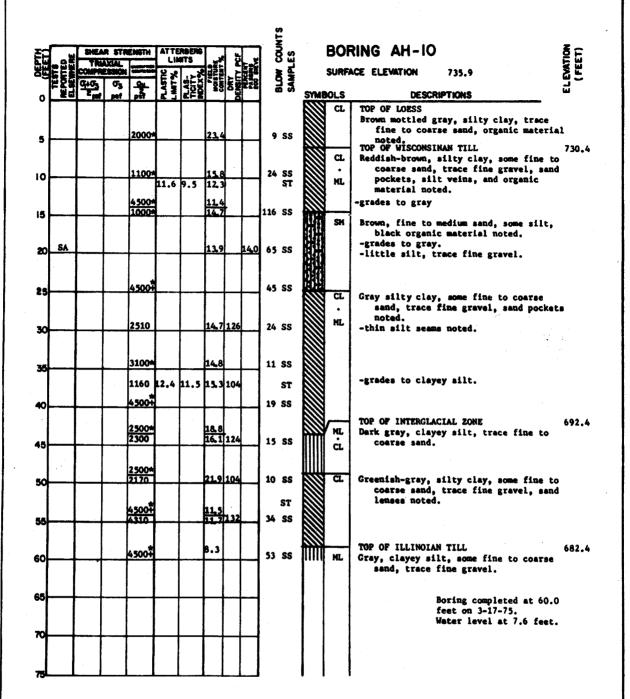
FIGURE 2.5-229



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

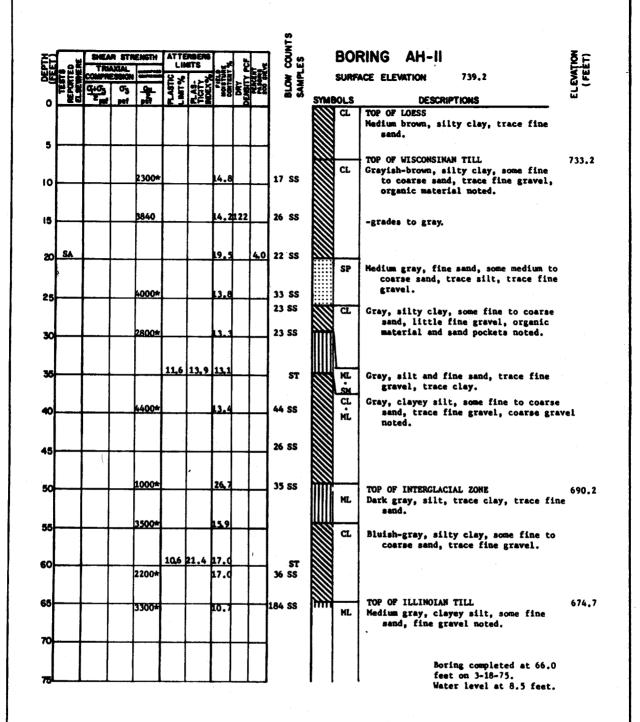
FIGURE 2.5-230



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

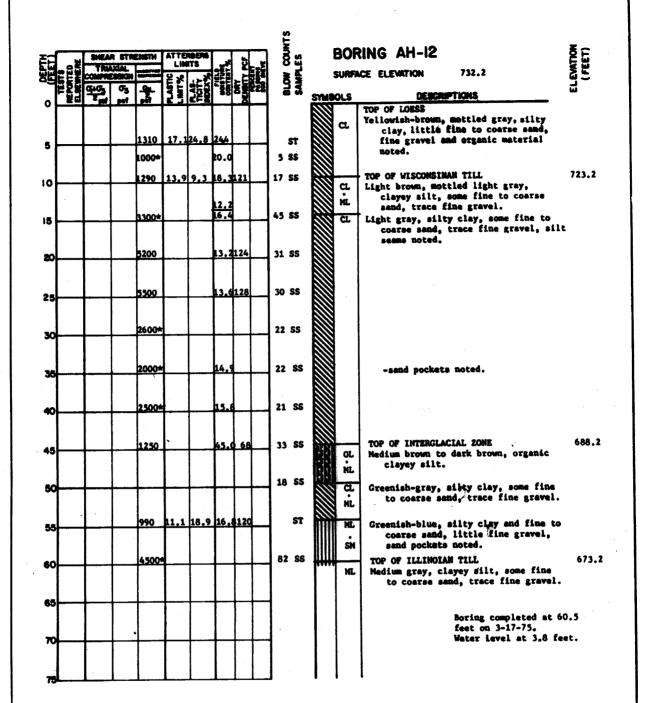
FIGURE 2.5-231



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

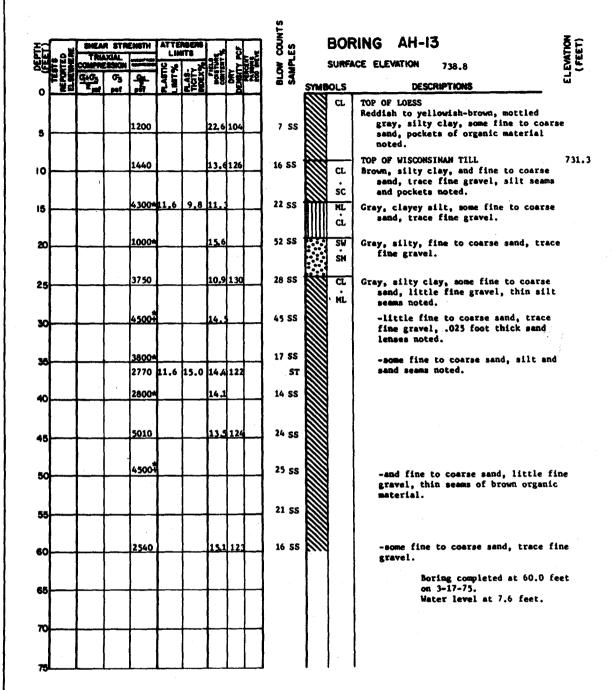
FIGURE 2.5-232



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

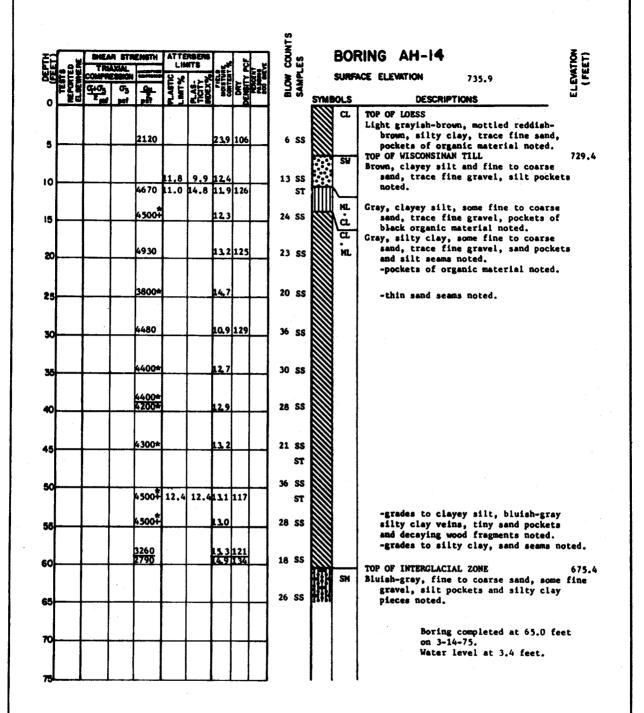
FIGURE 2.5-233



Logged by: Sargent & Lundy
Drilled by: Raymond International
Teated by: Westenhoff & Novick

CLINTON POWER STATION
UPDATED SAFETY ANALYSIS REPORT:

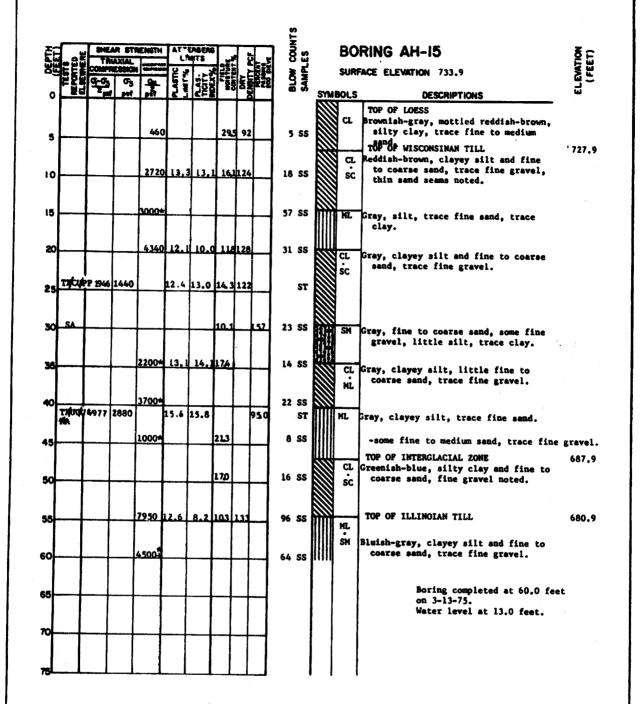
FIGURE 2.5-234



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

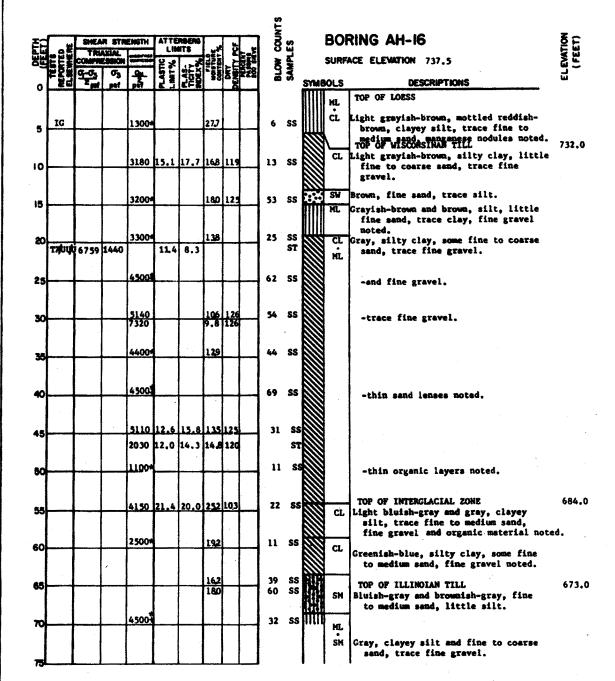
FIGURE 2.5-235



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-236



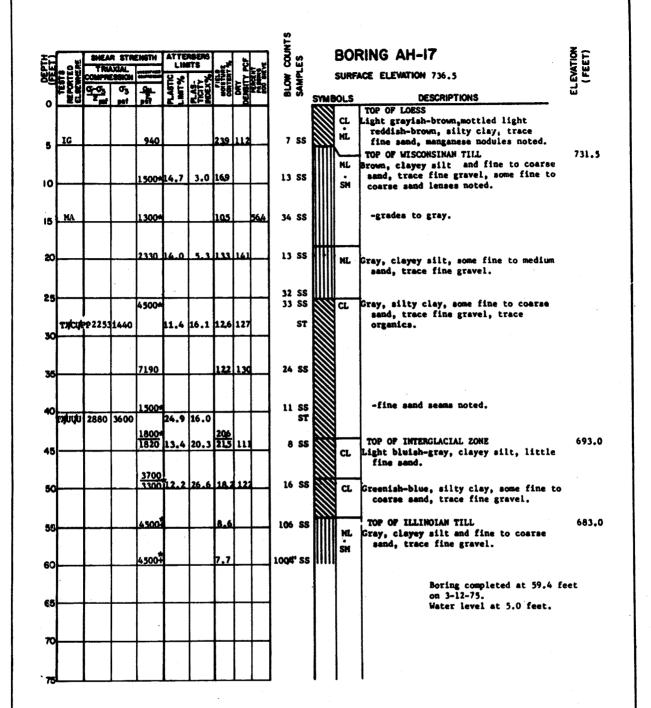
Boring completed at 70.0 feet on 3-17-75. Water level at 12.5 feet.

#### NOTES

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

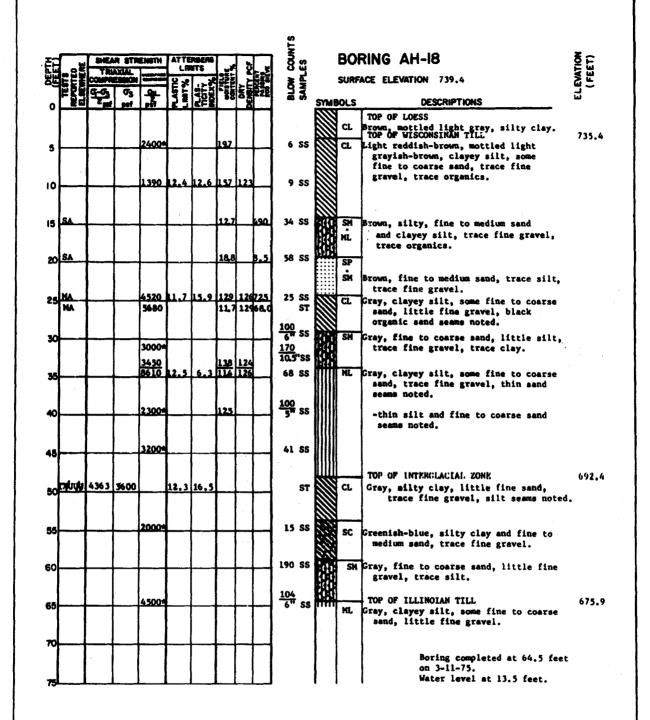
FIGURE 2.5-237



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-238



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

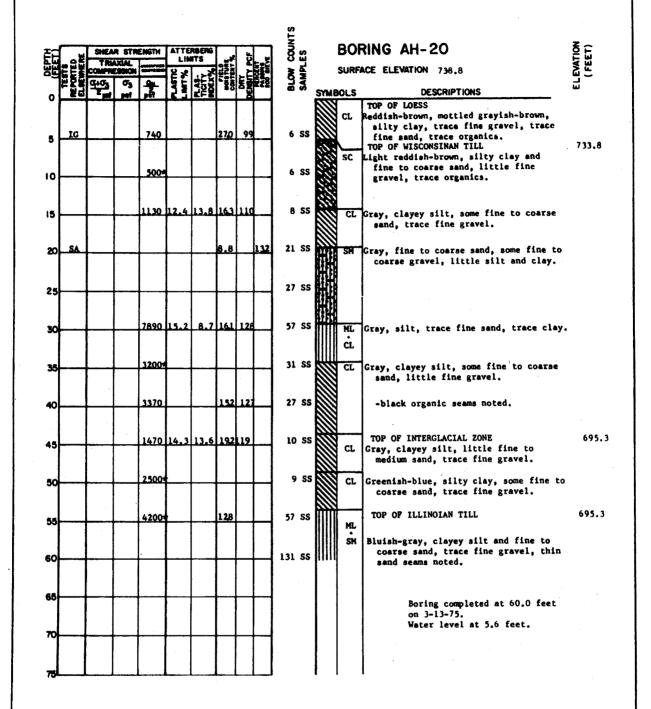
FIGURE 2.5-239

EE	POTTED POTTED REPERENE	A STR AXIAL EDUCH	EMBTH	LI	STS	2ži	A PCF	T SE	BLOW COUNTS	SAMPLES			RING AH-19 ACE ELEVATION 737.0	ELEVATION (FEET)
		O's	3	A TIME	1245- 2005-	2000	SERETY !	118	2	Ş	SYM	BOLS	DESCRIPTIONS	<u> </u>
, 			6004			253			7	ss		ML ML	TOP OF LOESS Yellow brown, mottled gray, clayey silt, trace fine send, trace organics. TOP OF WISCONSINAN TILL Yellow brown, clayey silt, some fine to	731.6
$\Big\}$			2650	12.5	9.5	141	123		15	SS		CL	coarse sand, trace fine gravel, trace organics.	
,	SA					16, 1		120	22	SS		SP SM	Brown, fine to coarse send, trace fine gravel, trace silt.	
0						12.1						ML	Gray silt, little fine to medium sand, trace fine gravel, trace clay.	
\$			7110	13.0	15.0	121	128		34	SS		Cr	Gray, clayey silt, some fine to coarse sand, trace fine gravel, trace organics.	
╬	МА		1850			156	124	701	37	SŚ		HL	Gray, silt, some fine to medium sand, trace fine gravel, thin sand seams noted.	
5			2100	13.2	11.0	42.5			20	SS		ML CL	Gray, clayey silt and silty clay, layere some fine to coarse sand, trace fine to coarse gravel, thin sand seams noted.	
╬			22004			175			14	88 81		CL	TOP OF INTENGLACIAL ZONE Gray, clayer silt, little fine to medium sand, trace fine gravel47.0 to 48.0 feet, boulder.	693.
<u>ተ</u>				12.1	30.3	201	L09			SS		CL	Bluish-gray, silty clay, some fine to medium sand, fine gravel noted.	683.
\$			4400						100			HL	TOP OF ILLIMOIAN TILL Light bluish-gray, clayey silt, some fine to coarse sand, little fine gravel, small sand pockets noted.	663.
<b>1</b>									100 3"	5\$		SH	Brown and gray, fine to coarse sand, little fine gravel, little silt, trace clay.	
*													Boring completed at 59.5 feet on 3-11-75.	

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

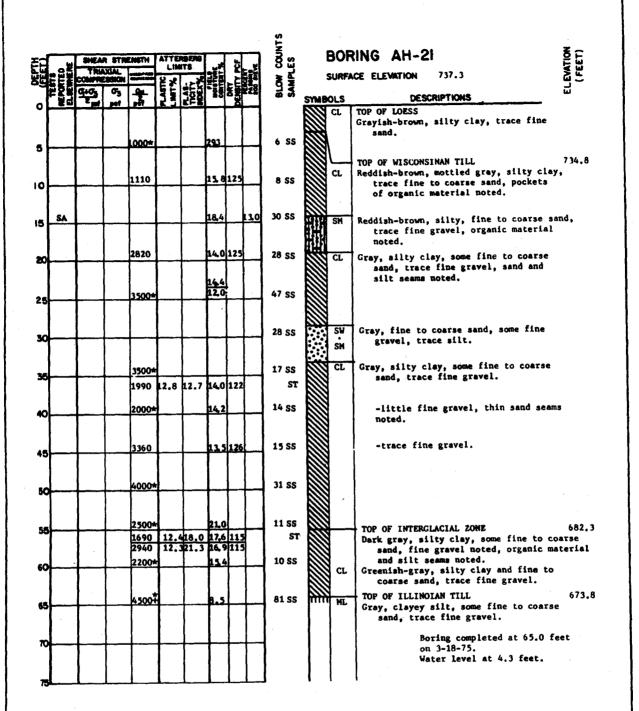
FIGURE 2.5-240



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

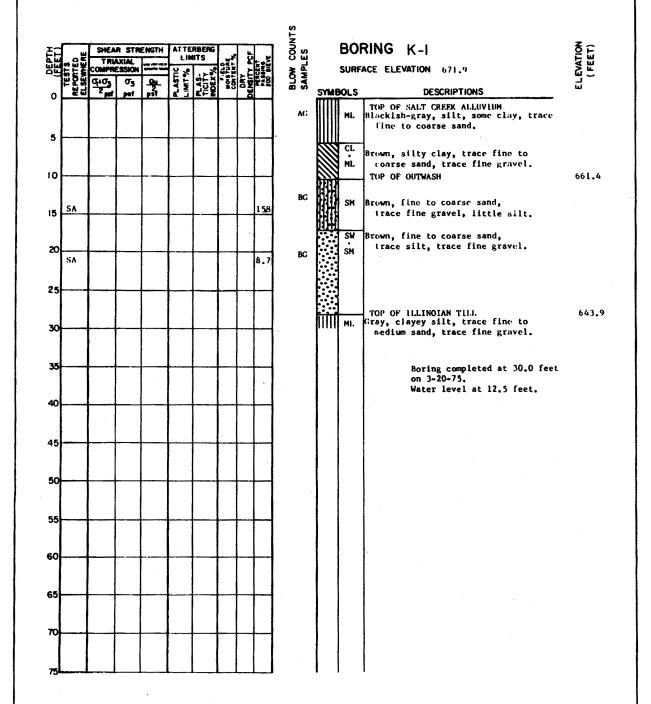
FIGURE 2.5-241



Logged by: Sargent & Lundy
Drilled by: Raymond International
Tested by: Westenhoff & Novick

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-242

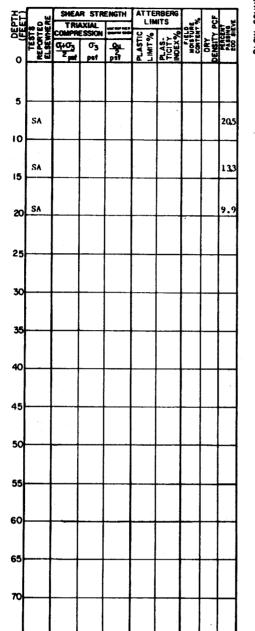


- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- Compaction and relative density tests
   performed on bulk samples from 10.5 to

   25.5 feet depth.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-243

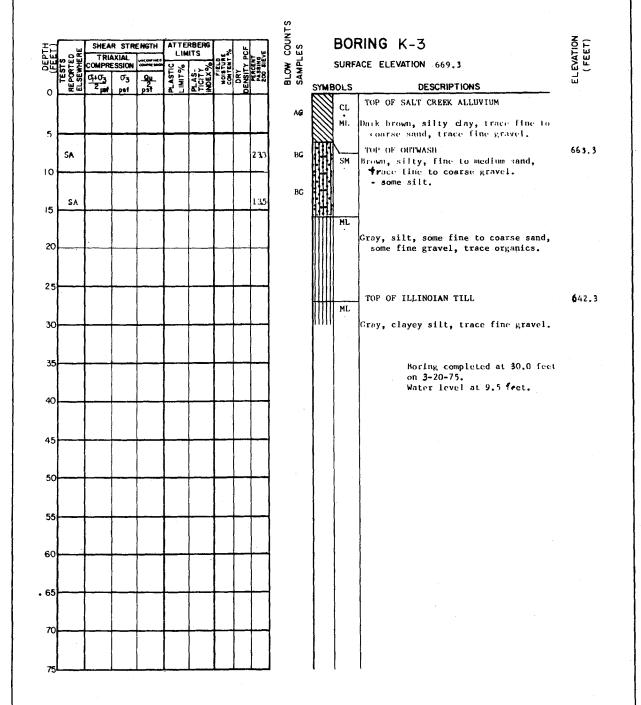


BLOW COUNTS SAMPLES	BORING K-2									
SAME			ACE ELEVATION 670.5							
	SYMI	30LS	DESCRIPTIONS	_						
AG		CL	TOP OF SALT CREEK ALLUVIUM							
		ML	Brown, silty clay, trace fine sand.							
		<del> </del>	TOP OF OUTWASH	665.0						
BG	H	SM	Brown, fine to coarse sand, some							
	Ш	<u> </u>	silt, some fine to coarse gravel.							
		GM								
BG		ĺ	Brown, fine to coarse gravel, some fine to coarse sand, some silt.							
	HOT		, co costo, come pare.							
		SW	Brown, fine to coarse sand, some fine to Coarse gravel, trace silt.							
ВG		SM	to coatse graver, trace sitt.							
			TOP OF ILLINOIAN TILE.	642.5						
		Mī.	Grey, clayey silt, some fine to							
	1		coarse sand, trace fine gravel.							
			Boring completed at 30.0 feet							
	1		on 3-19-75.							
			Water level at 11.0 feet.							
	1									
			·							
		3								
			·							
			•							

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenholf and Novick, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

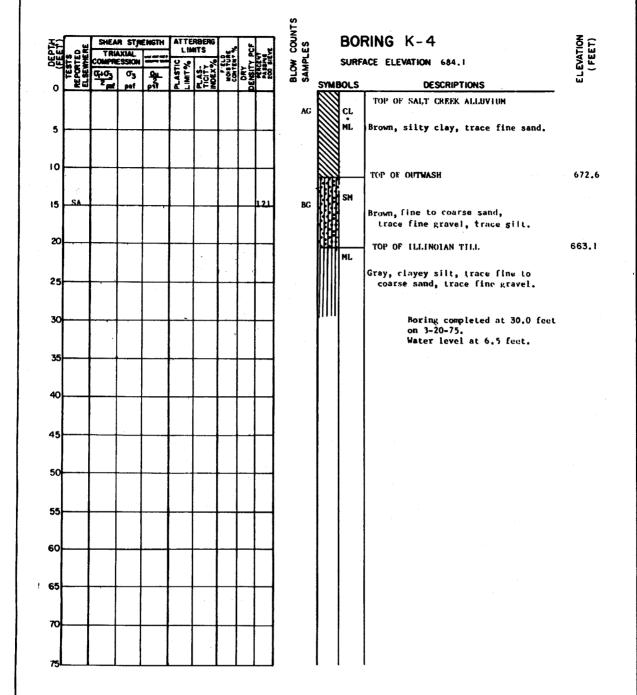
FIGURE 2.5-244



- I. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- 4. Compaction and relative density tests performed on mixed bulk samples from 10.0 to 15.0 feet depth in K-3 and 15.0 to 25.0 feet in K-7.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

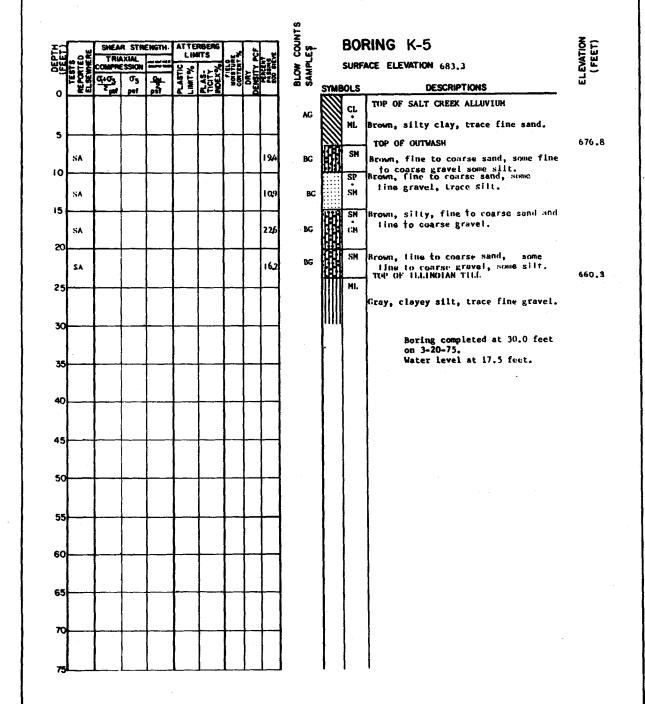
FIGURE 2.5-245



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- 4. Compaction and relative density tests performed on mixed bulk samples from 12.0 to 21.0 feet depth in K-4 and 6.5 to 15.0 feet depth in K-5.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

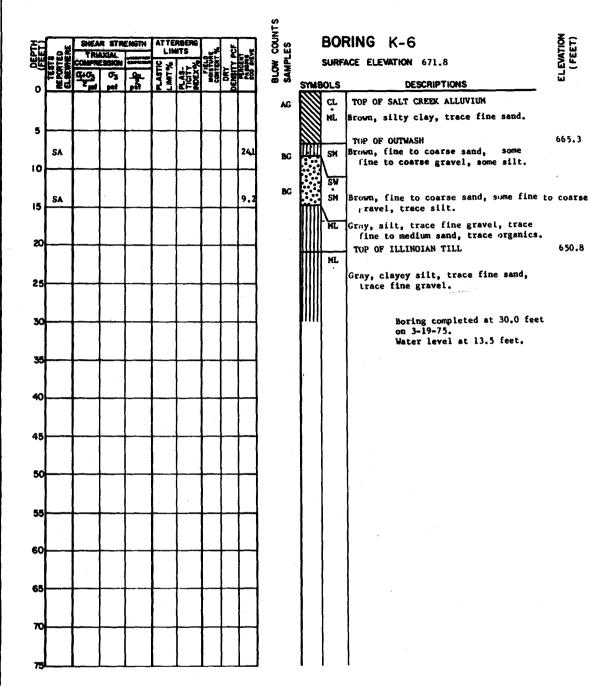
FIGURE 2.5-246



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- 4. Compaction and relative density tosts performed on bulk samples from 6.5 to 20.0 feet depth and on mixed bulk samples from 6.5 to 15.0 feet depth in K-5 and 12.0 to 21.0 feet depth in K-4.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

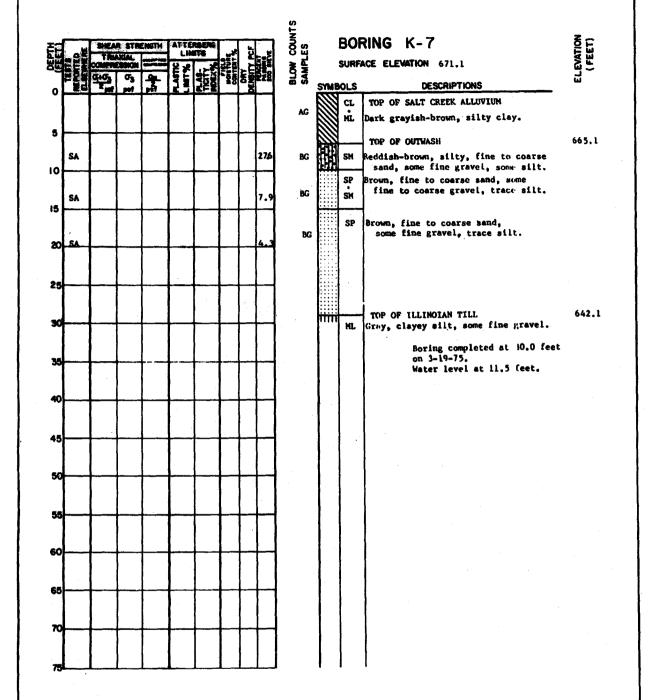
FIGURE 2.5-247



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- Compaction and relative density tests perfromed on bulk samples from 6.5 to 15.0 feet depth.

# CLINTON POWER STATION UPDATED SAFETY, ANALYSIS REPORT

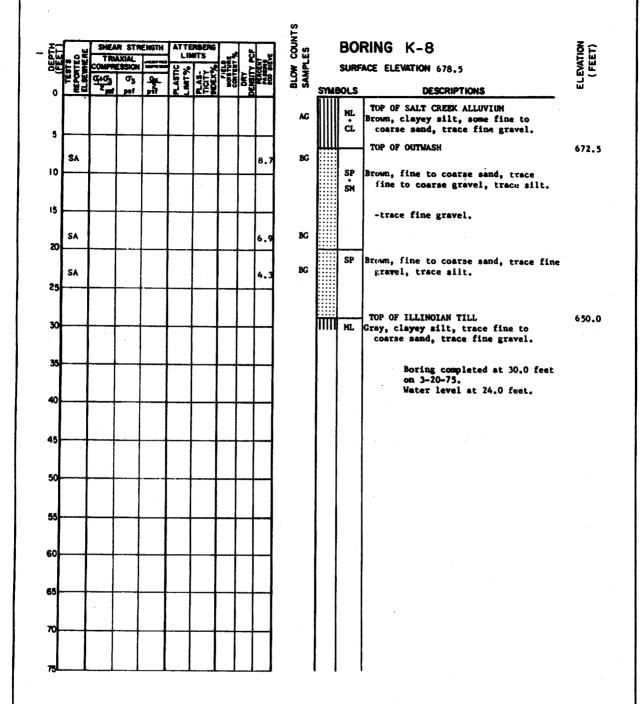
FIGURE 2.5-248



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- 4. Compaction and relative density tests performed on mixed bulk samples from 15.0 to 25.0 feet depth in K-7 and 10.0 to 15.0 feet depth in K-3.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

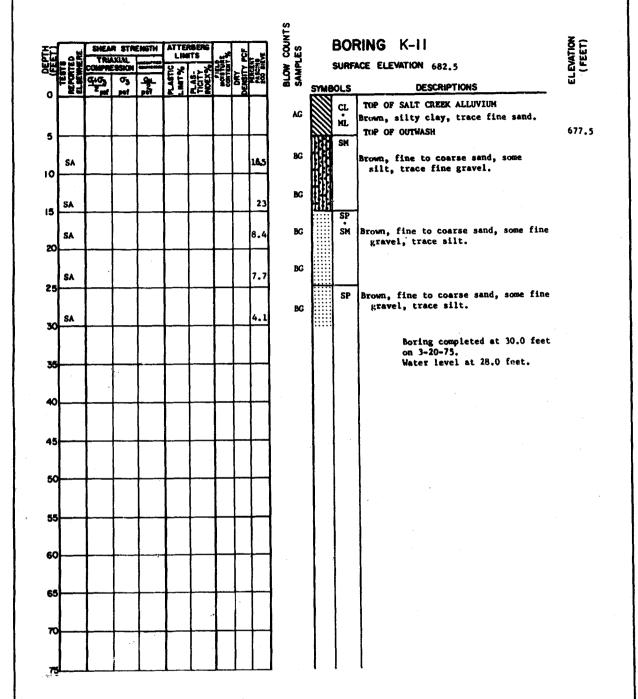
FIGURE 2.5-249



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- Compaction and relative density tests performed on bulk samples from 6.0 to 20.0 feet depth.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-250



- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- Compaction and relative density tests performed on bulk samples from 5.0 to 25.0 feet depth.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-251

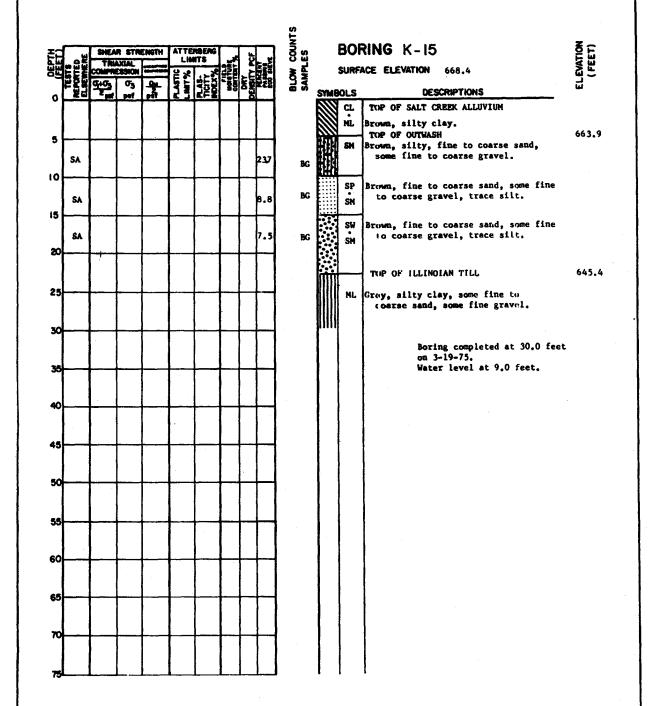
5	<u>لا</u> ک	SHEA		ENGTH	ATTE	RBENG ITS	بون	, P	۲
	E E E	Triaxial Compression					35 5	. 2	
0	TESTS MEPONTED ELSEWNERE	COMPRISON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE PERSON OF THE	σ ₃ psf	ON PST	PLASTIC LIMIT%	MAS- MOEX%	BOSTURC CONTENT %	DENST	228
5	MA								474
3	SA								151
0	SA								7.3
5	34								
20	SA								3.8
25			-						-
30			-					-	-
35									_
40									
45		-		-	-		-	-	-
<b>5</b> 0	<b> </b>	<u> </u>	_		<u> </u>		<u> </u>	_	L
55									
6 <b>0</b>	-	-		-			一	$\vdash$	
65		-	-		_	_	-	-	$\vdash$
70								_	L
-									

BLOW COUNTS SAMPLES	SYMB	ELEVATION (FEET) t		
AG			TOP OF SALT CREEK ALLUVIUM	
	圌	CL .	nu allau alau basas fina ta	
BG		ML	Brown, silty clay, trace fine to coarse sand.	
BG		SM	TOP OF OUTWASH Brown, fine to coarse sand and clayey	671.8
		HL	silt.	
BG		SM	Brown, fine to coarse sand, some fine	
		<u> </u>	gravel, some silt.	
		SP	Brown, fine to coarse sand, some fine	
₿G		)an	gravel, trace silt.	
		SP	Brown, fine to coarse sand, trace fine	
	Щ	$ldsymbol{ldsymbol{ldsymbol{ldsymbol{L}}}$	gravel, trace silt. TOP OF ILLINOIAN TILL	652.8
		ML		
		CL	Greenish-gray, clayey silt, trace fine gravel, trace organics.	
	111111	L	line Kraser's grace organizes.	
		ML	Gray, clayey silt, trace fine to coarse sand, trace fine gravel.	
			Boring completed at 30.0 feet on 3-20-75. Water level at 21.5 feet.	
		1		
	1			
			1	

- L. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.
- Compaction and relative density tests perfromed on bulk samples from 5.0 to 20.0 feet depth.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-252

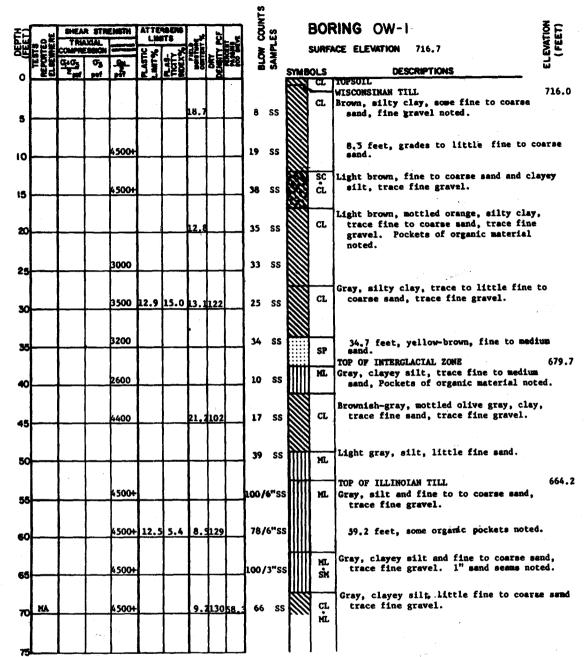


#### NOTES

- 1. Logged by: Sargent & Lundy Engineers
- 2. Drilled by: Raymond International
- 3. Tested by: Westenhoff and Novick, Inc.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-253



PIEZOMETER INSTALLED ON 5/12/76. A 2
INCH PVC PIPE WITH THE LOWER END PLUGGED
AND THE LOWER 3 FEET PERFORATED WAS PLACED
TO ELEVATION 646.7. GRANULAR BACKFILL WAS
PLACED FROM ELEVATION 646.7 TO 656.7; BENTONITE SEAL FROM ELEVATION 656.7 TO 658.7;
AND CEMENT GROUT FROM ELEVATION 658.7 TO 716.7.

BORING COMPLETED AT 70.0 FEET.

ON 5/12/76.

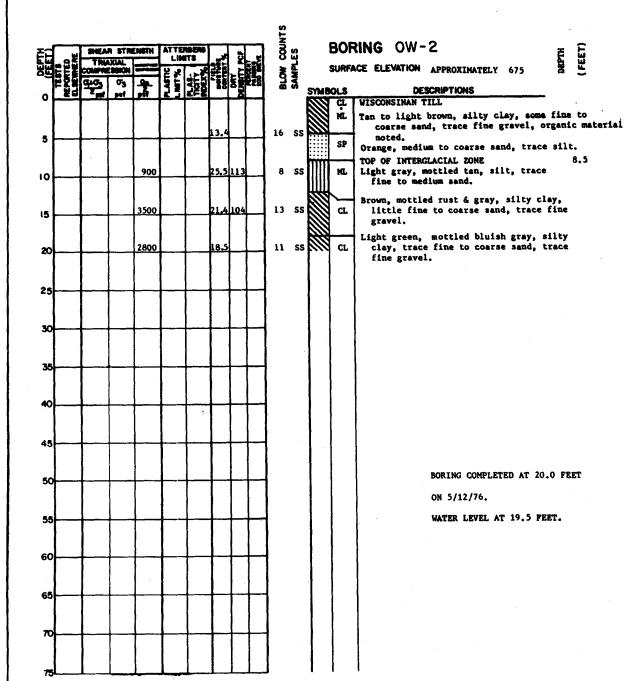
CASING USED TO A DEPTH OF 10.0 FEET.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-254



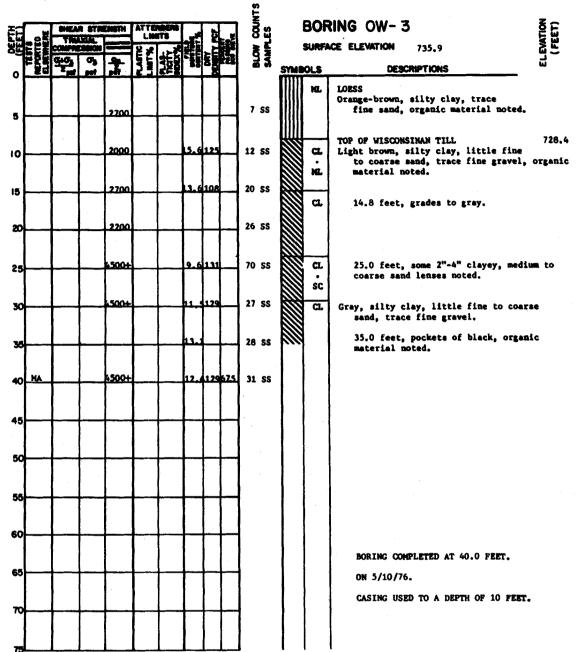
PIEZOMETER INSTALLED ON 5/12/76. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED TO ELEVATION 675. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 695 TO 690; BENTONITE SEAL FROM ELEVATION 690 TO 692; AND CEMENT GROUT FROM ELEVATION 692 TO 695.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-255



PIEZMETER INSTALLED IN OW-3 DEEP ON 5/10/76. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED TO ELEVATION 695.9. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 705.9 TO 707.9, AND CEMENT GROUT FROM ELEVATION 707.9 TO 735.9.

PIEZMETER INSTALLED ON 5/10/76. BORING OW-3 SHALLOW LOCATED 2 FEET NORTH OF OW-3 DEEP WAS DRILLED TO A DEPTH OF 10 FEET. A 2 INCH PVC 2IPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET FERFORATED WAS PLACED TO ELEVATION 725.9. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 725.9 TO 730.9, BENTONITE SEAL FROM ELEVATION 730.9

TO 732.9, AND CEMENT GROUT FROM ELEVATION 730.9

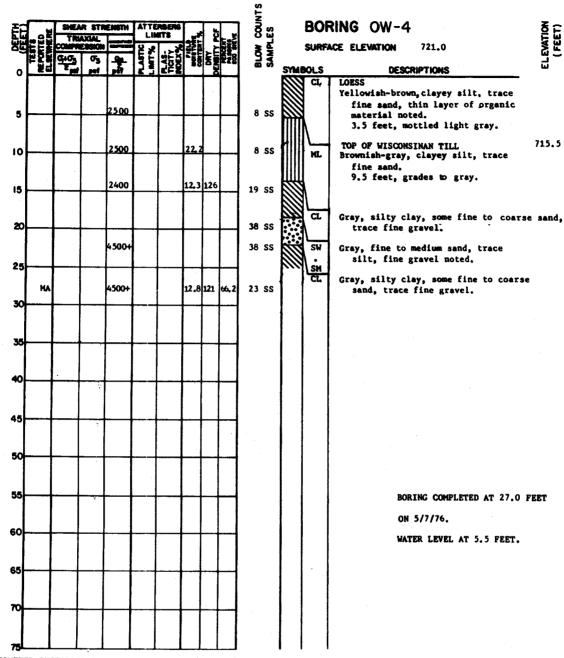
732.9 TO 735.9.

#### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-256



PIEZOMETER INSTALLED IN OW-4 DEEP ON 5-7-76. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED TO ELEVATION 697.5 GRANJLAR BACKFILL WAS PLACED FROM ELEVATION 697.5 TO 711; BENTONITE SEAL FROM ELEVATION 711 TO 713; AND CEMENT GROUT FROM ELEVATION 713 TO 721.

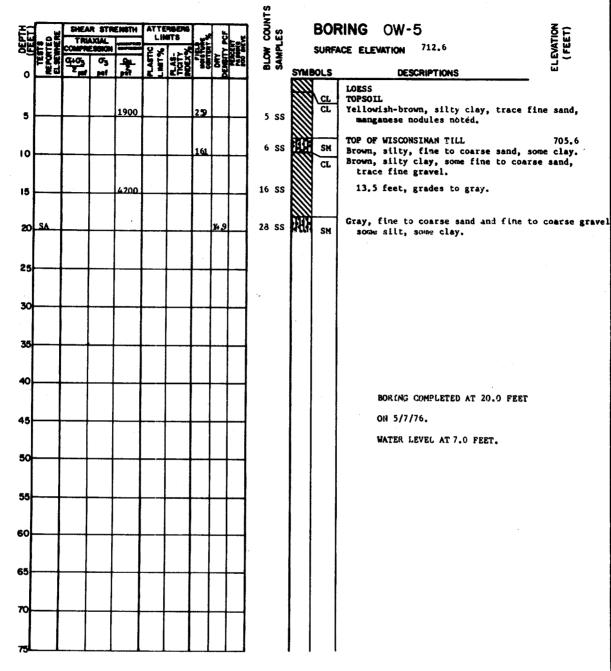
PIEZOMETER INSTALLED ON 5-7-76. BORING OW-4 SHALLOW LOCATED 3.5 FEET MORTH OF GW-4 DEEP WAS DRILLED TO A DEPTH OF 6.9 FEET. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED TO ELEVATION 714.1 GRANULAR BACKFILL WAS PLACED FROM ELEVATION 714.1 TO 718.1; BENTONITE SEAL FROM ELEVATION CEMENT GROUT FROM ELEVATION 720.1 TO 721.0.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY,
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTERNHOFF & NOVICK.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-257



PLEZOMETER INSTALLED IN OW-5 DEEP ON 5/7/76. A 2 INCH INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED TO ELEVATION 694.4. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 704.6; BENTONITE SEAL FROM ELEVATION 702.6 TO 704.6; AND CEMENT GROUT FROM ELEVATION 704.6 TO 712.6.

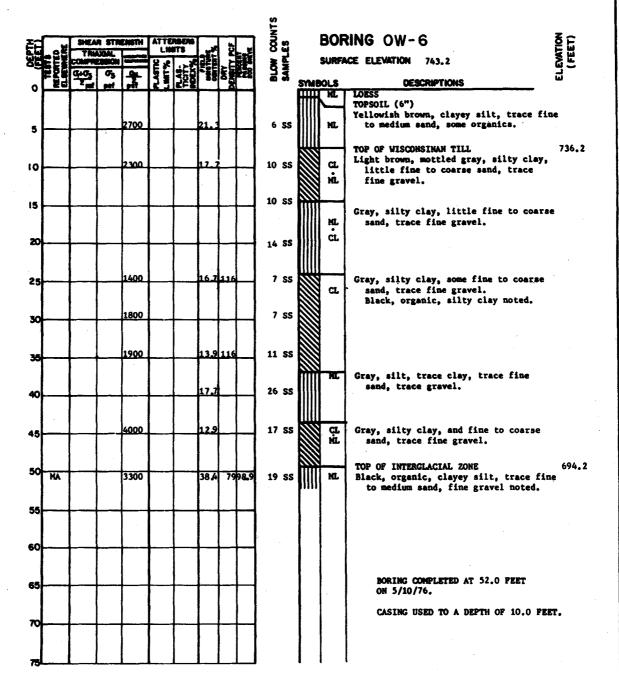
PIEZOMETER INSTALLED ON 5/7/76. BORING OW-5 SHALLOW LOCATED 2 FEET SOUTH OF OW-5 DEEP WAS DRILLED TO A DEPTH OF 8 FEET. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED TO ELEVATION 704.6 TO 708.6: BENTONITE SEAL FROM ELEVATION 703.6 TO 710. AND CEMENT GROUT FROM ELEVATION 710.6 TO 712.6.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-258



PIEZOMETER INSTALLED IN OW-6 DEEP ON 5/10/76. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED AT ELEVATION 691.2. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 691.2 TO 733.2; BENTONITE SEAL FROM ELEVATION 733.2 TO 735.2; AND CEMENT GROUT FROM ELEVATION 735.2 TO 743.2.

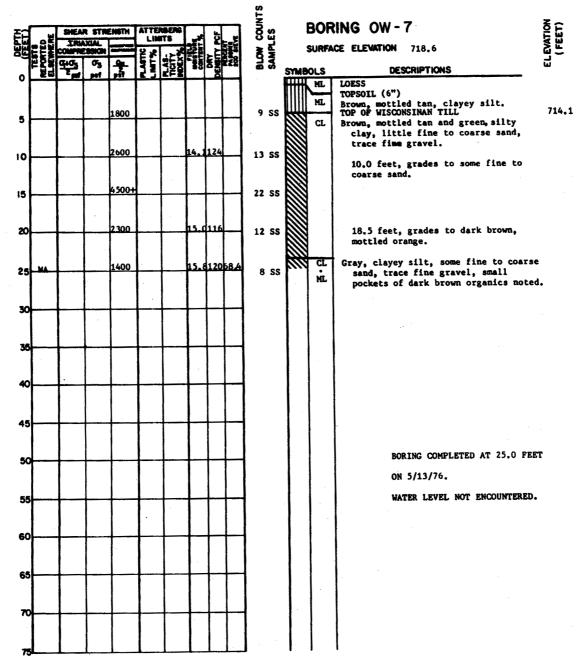
PIEZOMETER INSTALLED ON 5/10/76. BORING OW-6 SHALLOW LOCATED 2 FEET NORTH OF OW-6 DEEP WAS DRILLED TO A DEPTH OF 7.5 FEET. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED AT ELEVATION 735.8. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 735.8 TO 740.8; BENTONITE SEAL FROM ELEVATION 740.8 TO 741.8; AND CEMENT GROUT FROM 741.8 TO 743.3.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-259



PIEZOMETER INSTALLED IN OW-7 DEEP ON 5/13/76. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED AT ELEVATION 693.6. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 693.6 TO 708.6; BENTONITE SEAL FROM ELEVATION 708.6 TO 710.6; AND CEMENT GROUT FROM 710.6 TO 718.6.

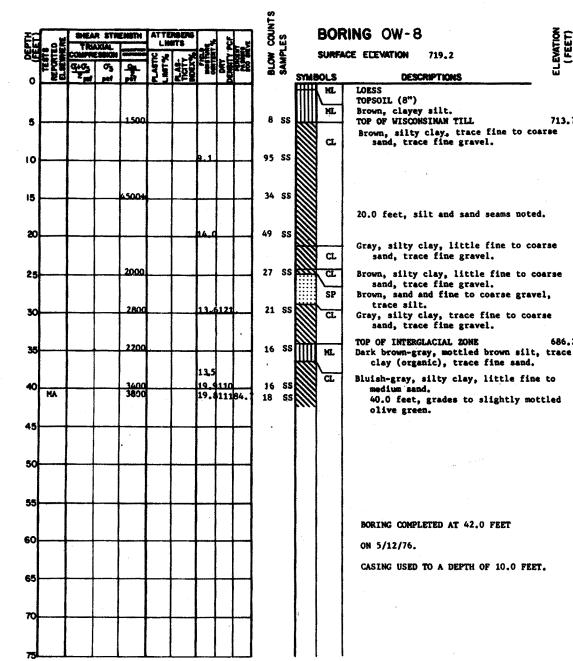
PIEZOMETER INSTALLED ON 5/13/76, BORING OW-7 SHALLOW LOCATED 2.5 FEET WEST OF OW-7 DEEP WAS DRILLED TO A DEPTH OF 6.0 FRET. A 2 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED AT ELEVATION 712.6. GRANULAR. BACKFILL WAS PLACED FROM ELEVATION 712.6 TO 716.6; BENTONITE SEAL FROM ELEVATION 716.6 TO 717.6; AND CEMENT GROUT FROM ELEVATION 717.6 TO 718.6.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-260



PIEZOMETER INSTALLED ON 5/12/76. PVC PIPE WITH THE LOWER BND PLUGGED AND THE LOWER 3 FEET PERFORATED WAS PLACED TO ELEVATION 677.2. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 677.2 TO 701.2; BENTONITE SEAL FROM ELEVATION 701.2 TO 703.2; AND CEMENT GROUT FROM ELEVATION 703.2 TO 719.2.

### NOTES:

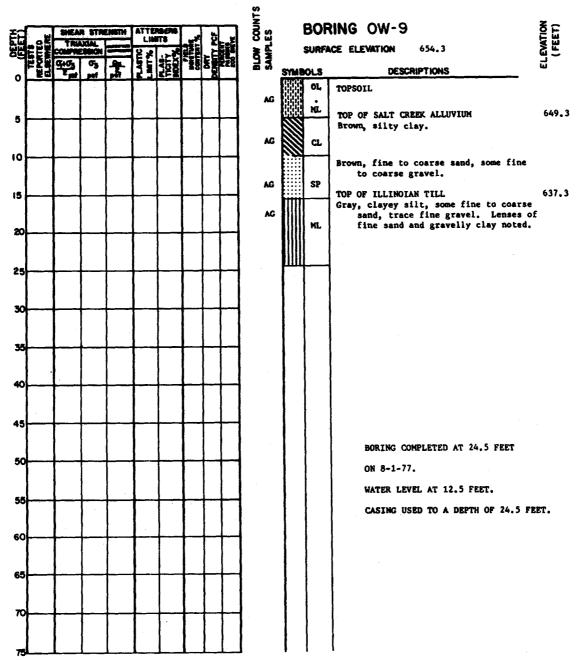
- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: RAYMOND INTERNATIONAL.
- 3. TESTED BY: WESTENHOFF & NOVICK.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

713.7

686.2

FIGURE 2.5-261



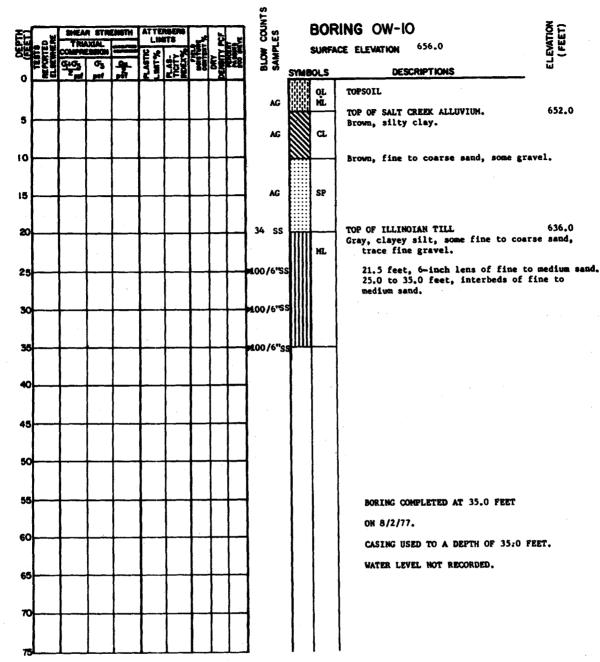
PIEZOMETER INSTALLED ON 8-1-77. A 1½ INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 5 FEET SLOTTED WAS PLACED TO ELEVATION 629.8. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 629.8 TO 637.8; BENTONITE SEAL FROM ELEVATION 637.8 TO 639.8; AND GRANULAR BACKFILL FROM ELEVATION 639.8 TO 654.3.

### NOTES:

- 1. LOGGED BY: SOIL TESTING SERVICES.
- 2. DRILLED BY: SOIL TESTING SERVICES

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-262



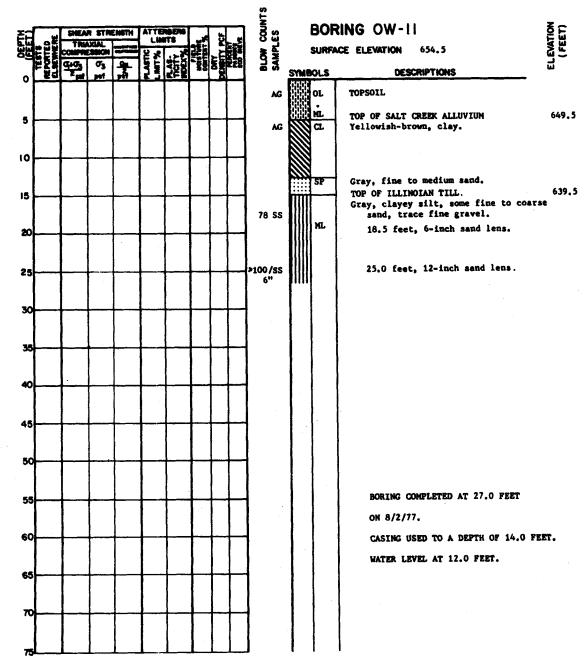
PIEZONETER INSTALLED ON 8/2/77. A 1½ INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 5 FEET SLOTTED WAS PLACED TO ELEVATION 621.0. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 621.0 TO 629.0; BENTONITE SEAL FROM ELEVATION 629.0 TO 631.0; AND GRANULAR BACKFILL FROM ELEVATION 631.0 TO 656.0.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: SOIL TESTING SERVICES.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-263



PIEZOMETER INSTALLED ON 8/2/77. A 1½ INCH
PVC PIPE WITH THE LOWER END PLUGGED AND THE
LOWER 5 FEET SLOTTED WAS PLACED TO ELEVATION
627.5. GRANULAR BACKFILL WAS PLACED FROM
ELEVATION 627.5 TO 635.5; BENTONITE SEAL FROM
ELEVATION 635.5 TO 637.5; AND GRANULAR BACKFILL
FROM ELEVATION 637.5 TO 654.5.

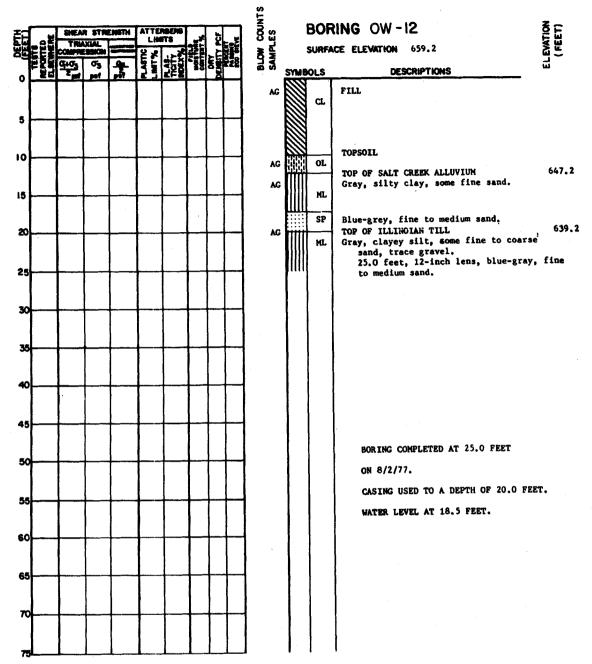
## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-264

LOG OF BORING OW-11

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: SOIL TESTING SERVICES.



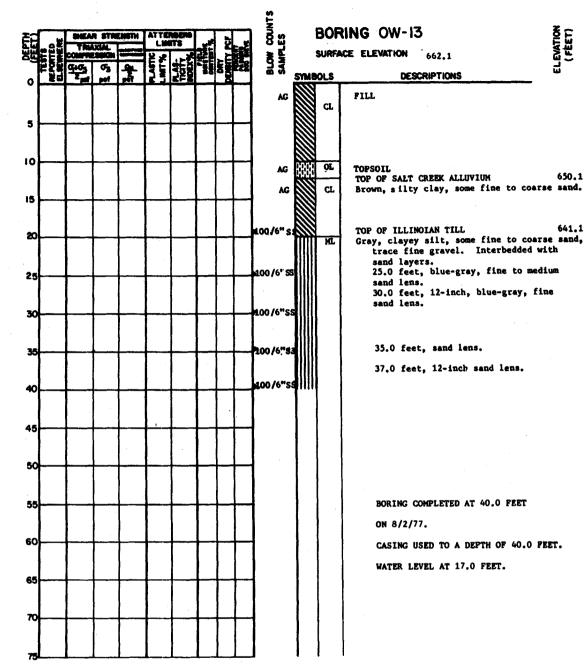
PIEZOMETER INSTALLED ON 8/2/77. A 1½ INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 5 FEET SLOTTED WAS PLACED TO ELEVATION 634.2. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 634.2 TO 642.2; BENTONITE SEAL FROM ELEVATION 642.2 TO 644.2; AND GRANULAR BACKFILL FROM ELEVATION 644.2 TO 659.2.

### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: SOIL TESTING SERVICES.

### C'LINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-265



PIEZOMETER INSTALLED ON 8/2/77. A 11 INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 5 FEET SLOTTED WAS PLACED AT ELEVATION 622.1. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 622.1 TO 630.1; BENTONITE SEAL FROM ELEVATION 630.1 TO 632.1; AND GRANULAR BACKFILL FROM ELEVATION 632.1 TO 662.1.

#### NOTES:

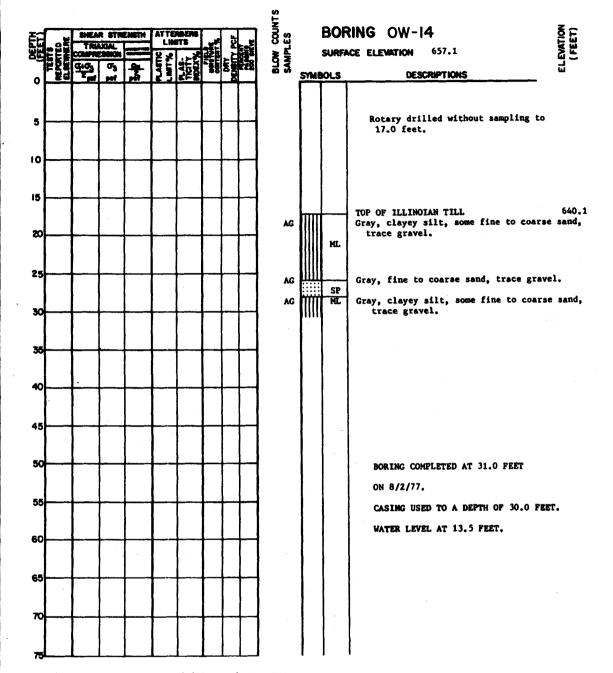
- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: SOIL TESTING SERVICES.

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

650.1

641.1

FIGURE 2.5-266



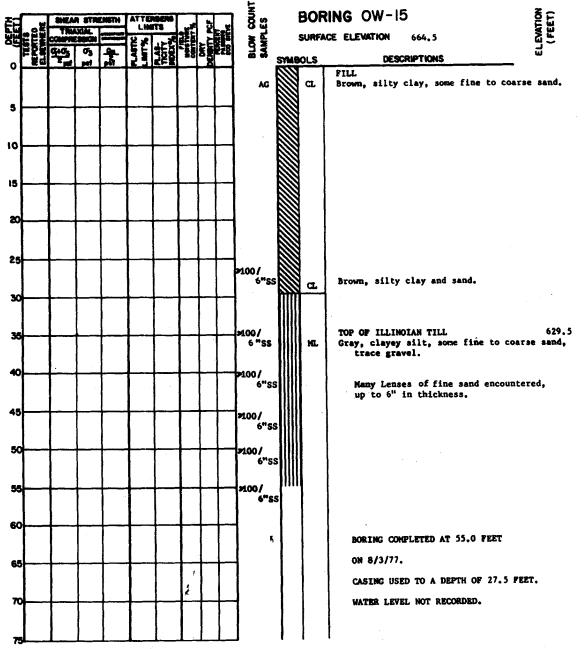
PIEZONETER INSTALLED ON 8/2/77. A 1½ INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 5 PEET SLOTTED WAS PLACED FROM ELEVATION 626.1. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 626.1 TO 634.1; BENTONITE SEAL FROM ELEVATION 634.1 TO 636.1; AND GRANULAR BACKFILL FROM ELEVATION 636.1 TO 657.1.

#### NOTES:

- 1. LOGGED BY SARGENT & LUNDY.
- 2. DRILLED BY: SOIL TESTING SERVICES.

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-267



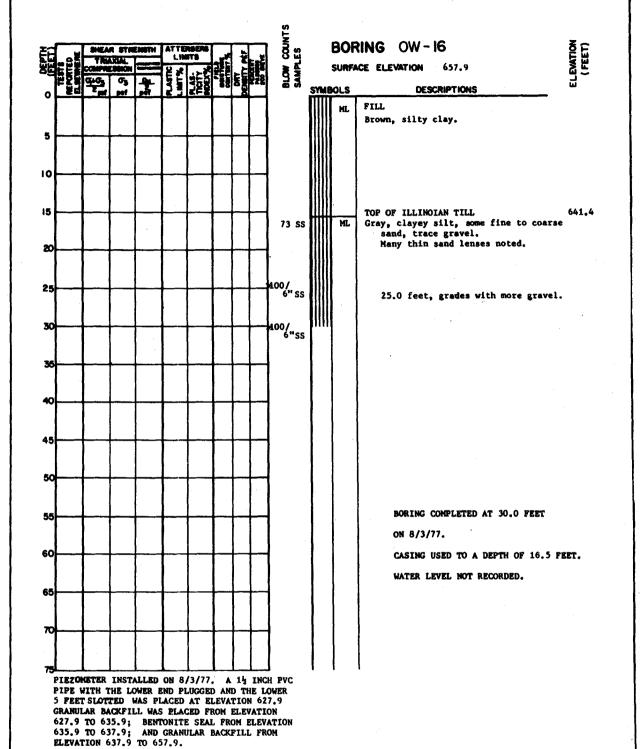
PIEZOMETER INSTALLED ON 8/3/77. A 1½ INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 5 FEET SLOTTED WAS PLACED TO ELEVATION 609.5. GRANULAR BACKFILL WAS PLACED FROM ELEVATION 609.5 TO 617.5; BENTONITE SEAL FROM ELEVATION 617.5 TO 619.5; AND GRANULAR BACKFILL FROM ELEVATION 619.5 TO 664.5.

#### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: SOIL TESTING SERVICES

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-268

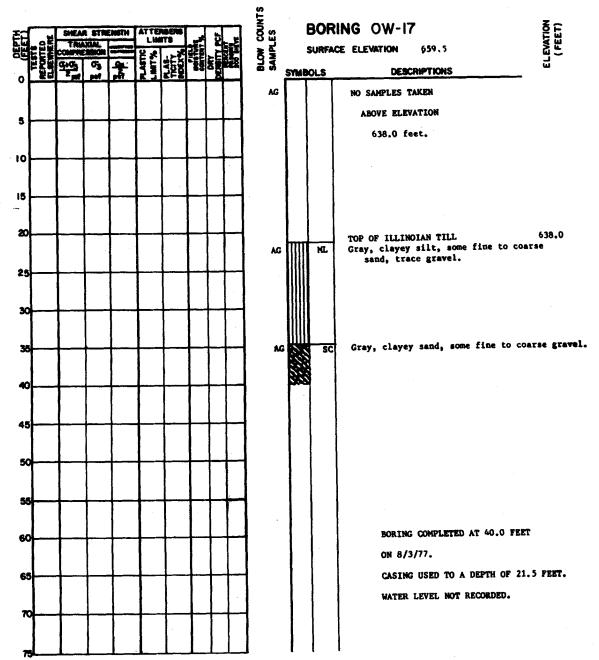


### NOTES:

- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: SOIL TESTING SERVICES

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-269



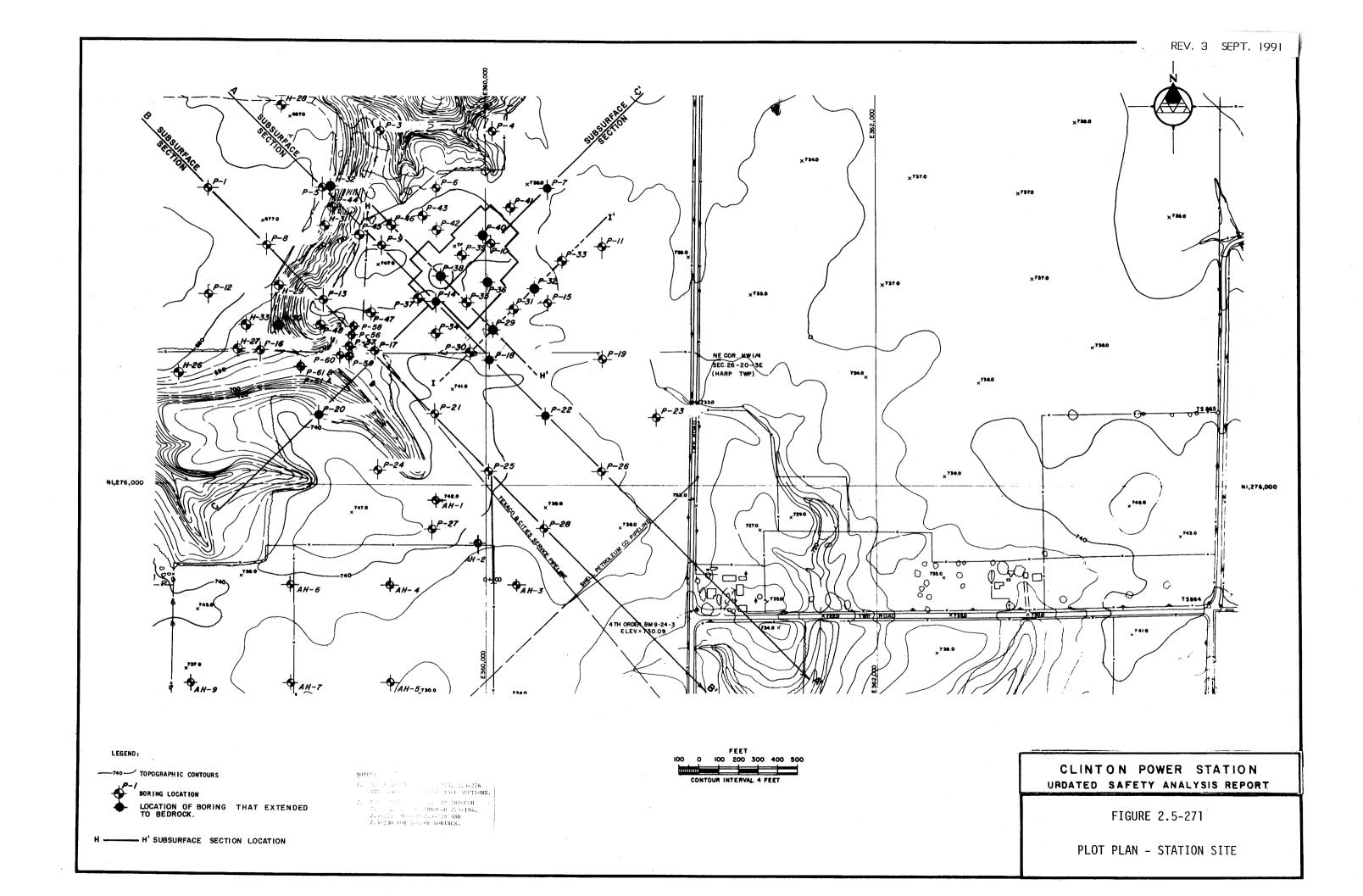
PIEZOMETER INSTALLED ON 8/3/77. A 1½ INCH PVC PIPE WITH THE LOWER END PLUGGED AND THE LOWER 5 FEET SLOTTED WAS PLACED TO ELEVATION 619.5. GRANULAR BACKFILL WAS PLACED FROM ELE-VATION 619.5 TO 627.5; BENTONITE SEAL FROM ELEVATION 627.5 TO 629.5; AND GRANULAR BACKFILL FROM ELEVATION 629.5 TO 659.5.

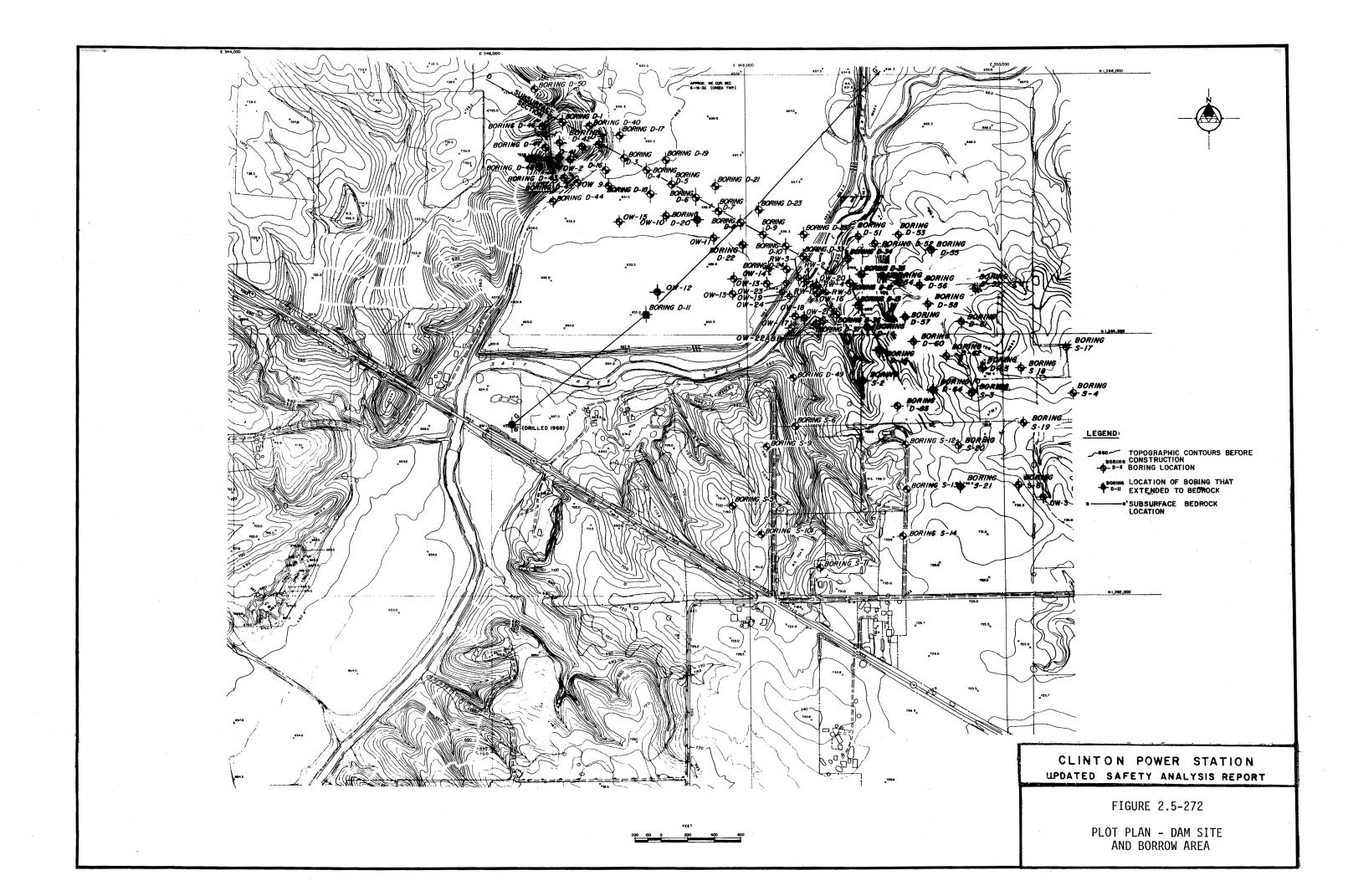
### NOTES:

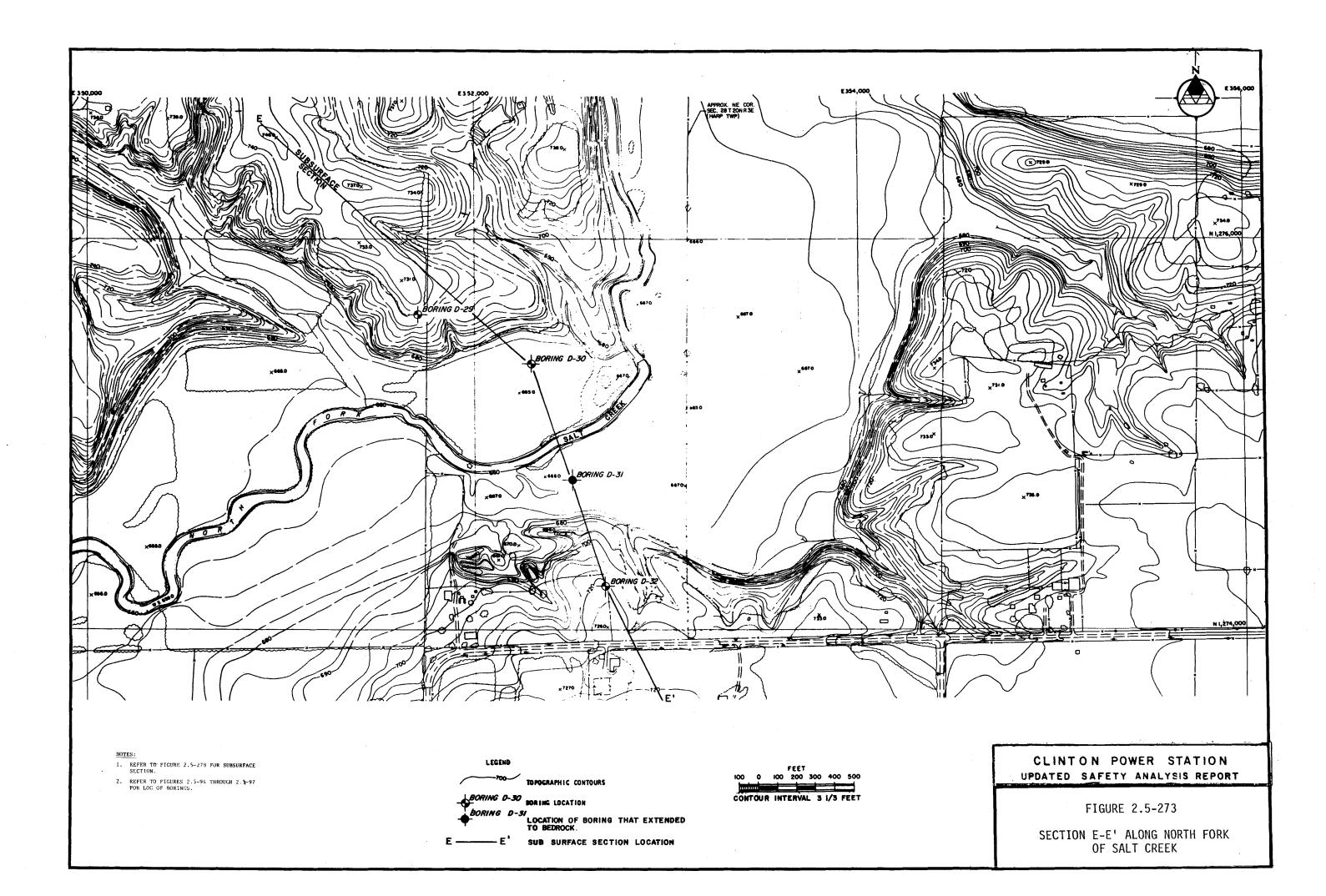
- 1. LOGGED BY: SARGENT & LUNDY.
- 2. DRILLED BY: SOIL TESTING SERVICES.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-270







TIME STRATIGRAPHY				STRATIGRAPHIC UNITS			
				FSAR		PSAR	BORING LOGS
		Holocene Stage		Cahokia Alluvium	Peyton Colluvium	Salt Creek Alluvium or Flood Plain Alluvium and Recent Channel Deposits	Salt Creek Alluvium
		Wisconsinan Stage	Valderan Substage Twocreekan Substage	Richland Loess	Henry Formation	Loess	Loess
			Woodfordian Substage	Wedron Formation		Wisconsinan Till or Wisconsinan Glacial Till	Wisconsinan Glacial Till
			Farmdalian Substage	Robein Silt		Interglacial Zone or Sangamon Interglacial Zone	Interglacial Zone
			Altonian Substage -				
stem	eries	Sangamonian Stage				or Sang <b>a</b> mon Soil Interval	
Sy	S	Illinoian Stage		weathered Glasford Formation			
a t	Pleistocene			unaltered Glasford Formation		Illinoian Till or Illinoian Glacial T <b>i</b> ll	Illinoian Glacial Till
		Yarmouthian Stage				Lacustrine Deposit	Lacustrine Deposit
		Kansan Stage		B <b>a</b> nner Formation		Pre-Illinoian Glacial Till or Kansan Till	Pre-Illinoian Glacial Till
						Pre-Illinoian Alluvial and Lacustrine Deposit or Kansan Alluvial or Lacustrine Soils	Pre-Illinoian Lacustrine Deposit
			Jnconformity —			Bedrock Valley Outwash <b>Dep</b> osit or Mahomet Valley Deposit	Mahomet Bedrock Valley Deposit
Pe	enn		anian System	Bedrock		Bedrock	Bedrock

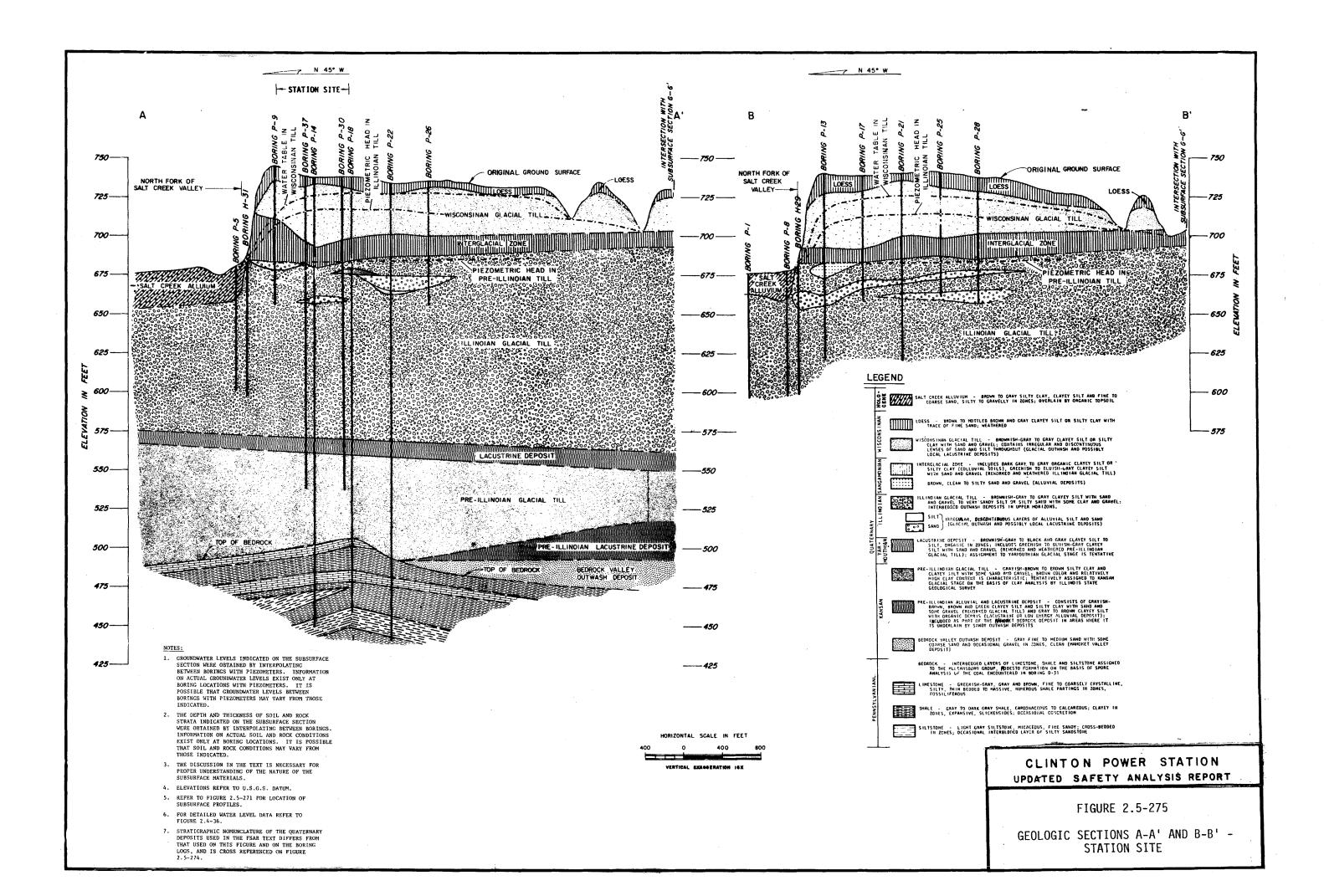
### NOTES:

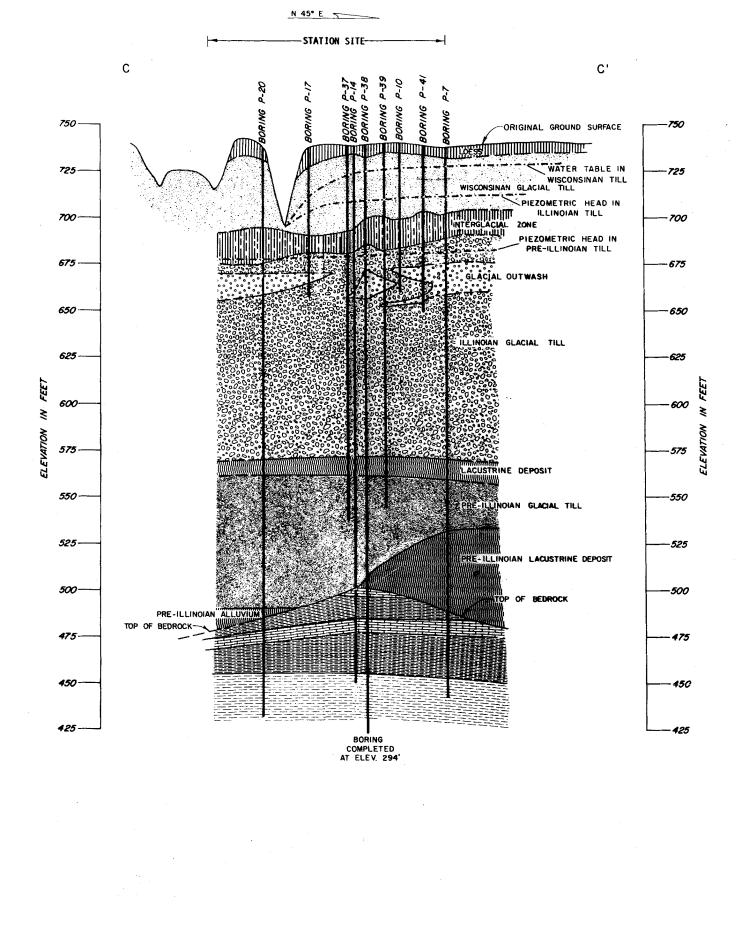
- 1. EXCAVATIONS FOR THE CLINTON POWER STATION DID NOT EXTEND BELOW THE UNALTERED GLASFORD FORMATION.
- 2. BORINGS FOR THE CLINTON POWER STATION DID NOT EXTEND INTO ROCKS OLDER THAN THOSE OF THE PENNSYLVANIAN SYSTEM.
- 3. ILLINOIAN-AGE TILL OF THE GLASFORD FORMATION WAS SUBJECTED TO A SIGNIFICANT PERIOD OF WEATHERING DURING THE SANGAMONIAN STAGE AND ALTONIAN SUBSTAGE.
- 4. DEPOSITS OF CAHOKIA ALLUVIUM AND HENRY FORMATION WERE NOT DIFFERENTIATED.
- 5. THE HOLOCENE STAGE IS REPRESENTED BY A SIGNIFICANT PERIOD OF WEATHERING AND DEVELOPMENT OF AGRICULTURAL SOIL PROFILES (MODERN SOIL).
- 6. VERTICAL SCALE DOES NOT REPRESENT EITHER RELATIVE THICKNESS OF STRATIGRAPHIC UNITS OR RELATIVE DURATION OF TIME INTERVAL.

# CLINTON POWER STATION FINAL SAFETY ANALYSIS REPORT

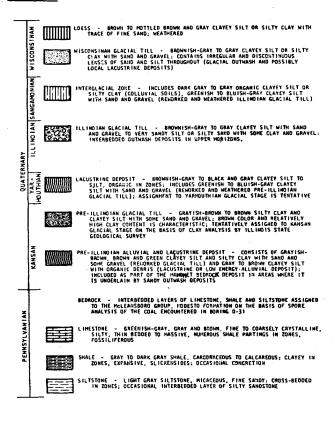
FIGURE 2.5-274

COMPARISON OF TERMINOLOGY USED FOR THE FSAR, PSAR AND BORING LOGS





### LEGEND



#### NOTES:

- 1. GROUNDWATER LEVELS INDICATED ON THE SUBSURFACE SECTION WERE OBTAINED BY INTERPOLATING BETWEEN BORINGS WITH PIEZOMETERS. INFORMATION ON ACTUAL GROUNDWATER LEVELS EXIST ONLY AT BORING LOCATIONS WITH PIEZOMETERS. IT IS POSSIBLE THAT GROUNDWATER LEVELS BETWEEN BORINGS WITH PIEZOMETERS MAY VARY FROM THOSE IMPICATED.
- 2. THE DEPTH AND THICKNESS OF SOIL AND ROCK STRATA INDICATED ON THE SUBSURFACE SECTION WERE OBTAINED BY INTERPOLATING BETWEEN BORINGS. IMPORMATION ON ACTUAL SOIL AND ROCK CONDITIONS EXIST ONLY AT BORING LOCATIONS. IT IS POSSIBLE THAT SOIL AND ROCK CONDITIONS MAY VARY FROM THOSE INDICATED.
- 3. THE DISCUSSION IN THE TEXT IS NECESSARY FOR PROPER UNDERSTANDING OF THE NATURE OF THE SUBSURFACE MATERIALS.
- 4. ELEVATIONS REFER TO U.S.G.S. DATUM.
- REFER TO FIGURE 2.5-271 FOR LOCATION OF SUBSURFACE PROFILES.
- 6. FOR DETAILED WATER LEVEL DATA REFER TO FIGURE 2.4-36.
- 7. STRATIGRAPHIC NOMENCLATURE OF THE QUATERNA'
  DEPOSITS USED IN THE FSAR TEXT DIFFERS FR'
  THAT USED ON THIS FIGURE AND ON THE BORINLOCS, AND IS CROSS REFERENCED ON FIGURE
  2.5-274.

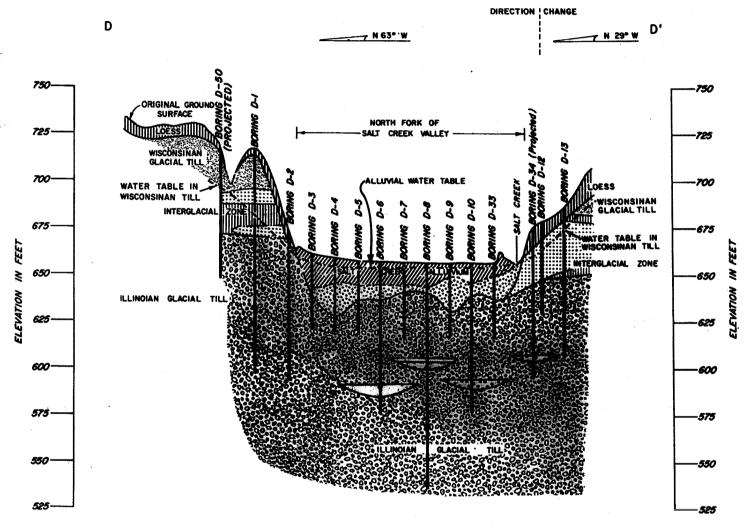
HORIZONTAL SCALE IN FEET



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-276

GEOLOGIC SECTION C-C' - STATION SITE

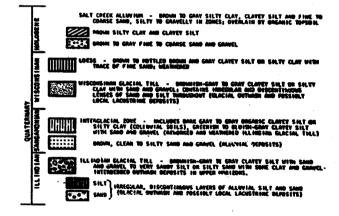


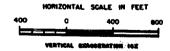
#### NOTES:

- NOISE:

  1. GROUNDWATER LEVELS INDICATED ON THE SUBSURFACE SECTION WERE OBTAINED BY INTERPOLATING BETWEEN BORINGS WITH PIEZOMETERS. INFORMATION ON ACTUAL GROUNDWATER LEVELS EXIST ONLY AT BORING LOCATIONS WITH PIEZOMETERS. IT IS POSSIBLE THAT GROUNDWATER LEVELS BETWEEN BORINGS WITH PIEZOMETERS MAY VARY FROM THOSE INDICATED.
- 2. THE DEPTH AND THICKNESS OF SOIL AND ROCK
  STRATA INDICATED ON THE SUBSURFACE SECTION
  WERE OBTAINED BY INTERPOLATING BETWEEN BORINGS.
  INFORMATION ON ACTUAL SOIL AND ROCK CONDITIONS
  EXIST ONLY AT BORING LOCATIONS. IT IS POSSIBLE
  THAT SOIL AND ROCK CONDITIONS MAY VARY FROM
  THOSE INDICATED.
- THE DISCUSSION IN THE TEXT IS NECESSARY FOR PROPER UNDERSTANDING OF THE NATURE OF THE SUBSURFACE MATERIALS.
- 4. ELEVATIONS REFER TO U.S.G.S. DATUM.
- REFER TO FIGURE 2.5-272 FOR LOCATION OF SUBSURFACE PROFILES.
- FOR DETAILED WATER LEVEL DATA REFER TO FIGURES 2.4-37 AND 2.4-41.
- 7. STRATIGRAPHIC NOMENCLATURE OF THE QUATERNARY DEPOSITS USED IN THE FSAR TEXT DIFFERS FROM THAT USED ON THIS FIGURE AND ON THE BORING LOGS, AND IS CROSS REFERENCED ON FIGURE 2.5-274.

### LEGEND

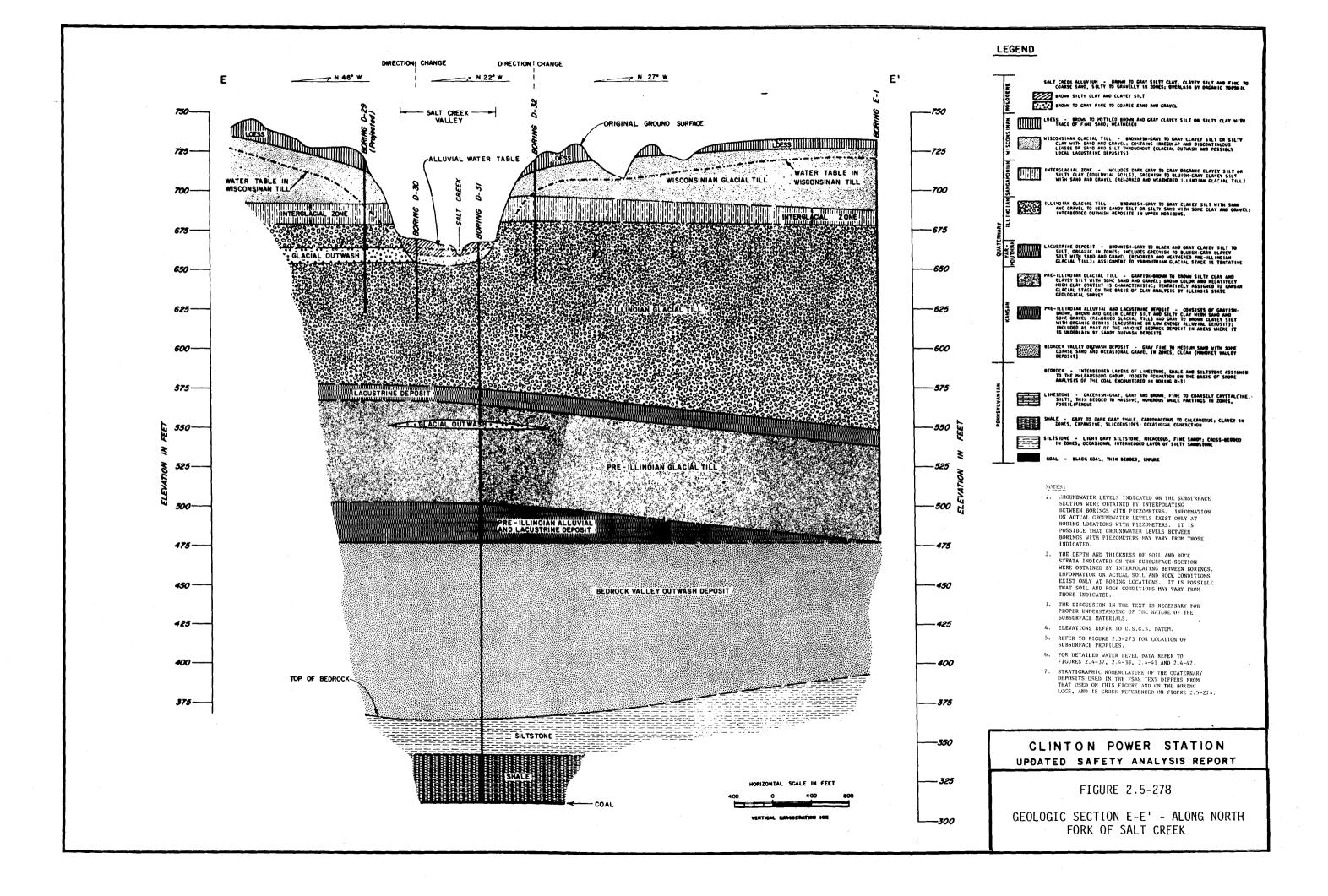


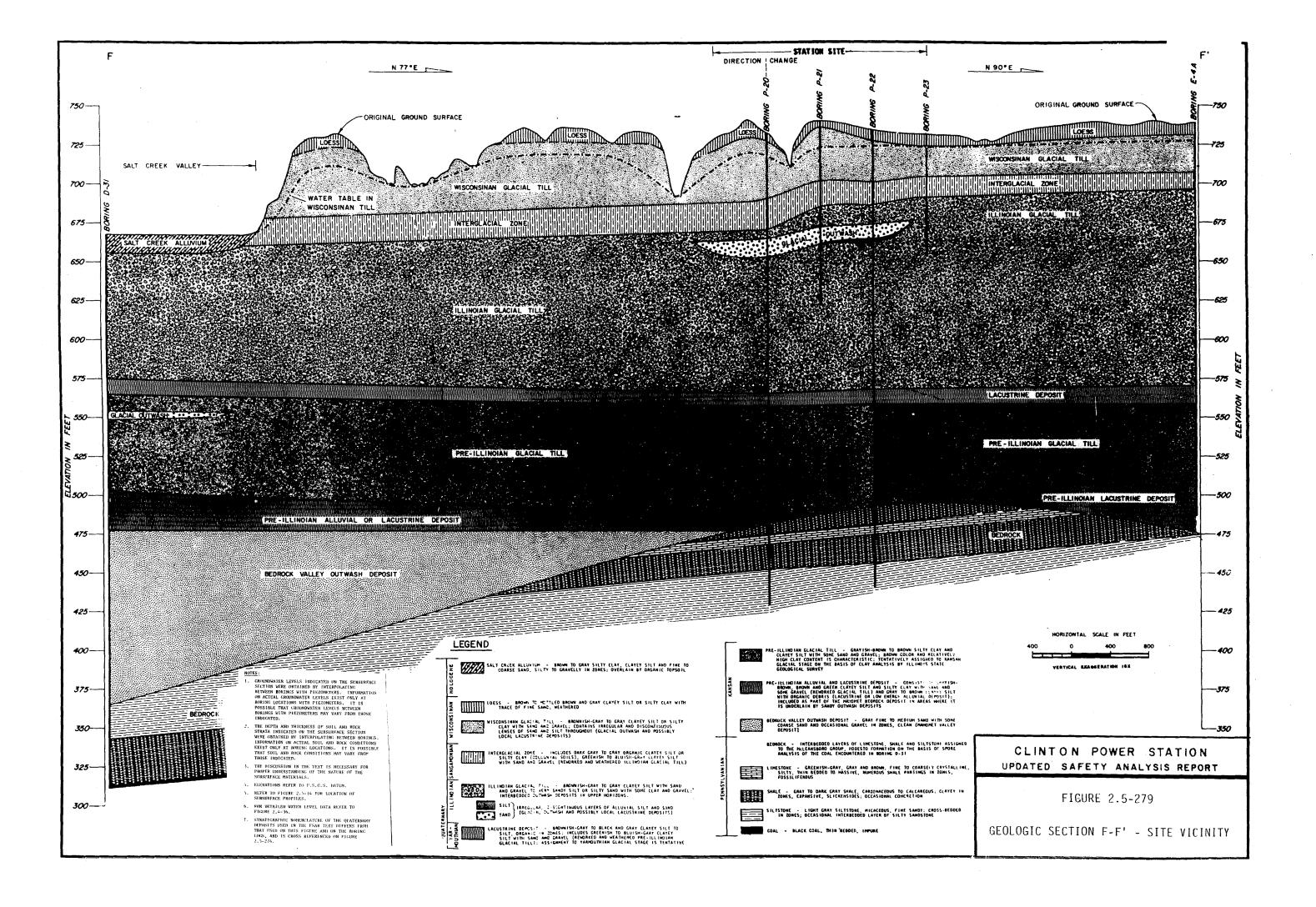


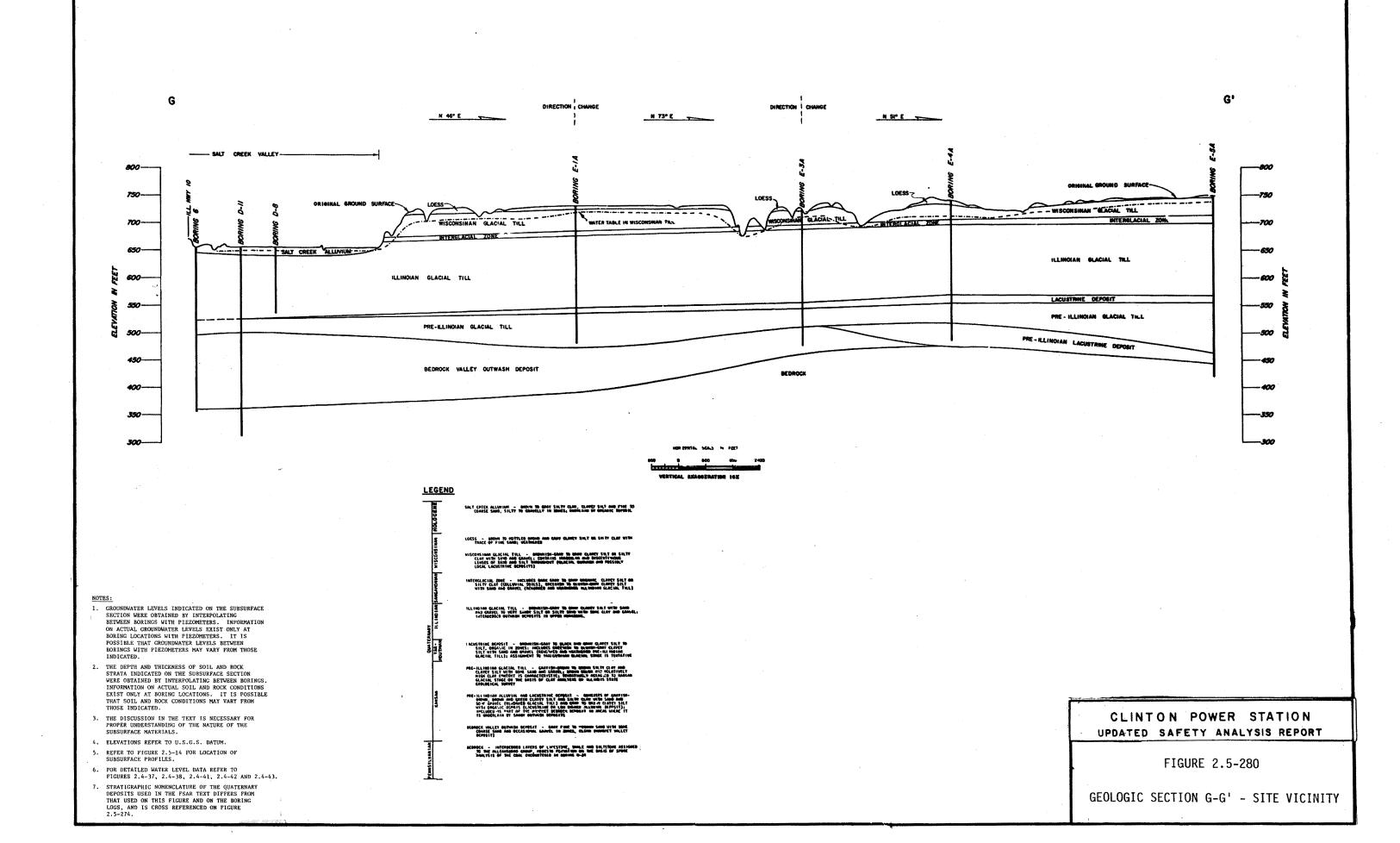
# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

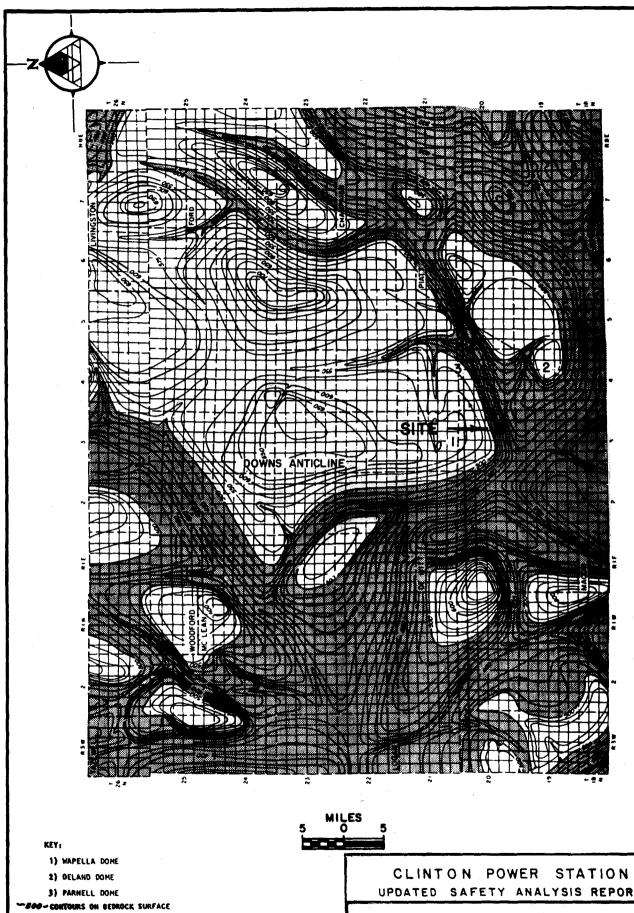
FIGURE 2.5-277

GEOLOGIC SECTION D-D' - DAM SITE









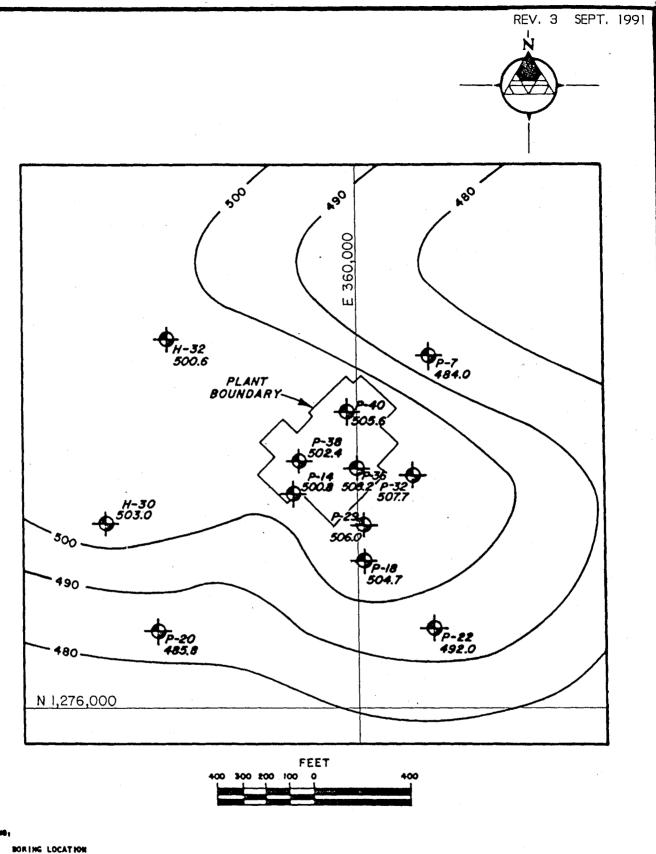
APPROXIMATE POSITION OF HANDHET VALLEY FILL DEPOSITS

MODIFIED FROM: GEOLOGICAL SIGNIFICANCE OF THE GRAVITY FIELDS IN THE DEWITT - MCLEAN COUNTY AREA, ILLINGIS BY P.C. HETGOLD, L.D. MCGINNIS AND R.H. HOWARD: ILLINOIS STATE GEOLOGICAL SURVEY CIRCULAR 369, 1964.

UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-281

RELIEF OF BEDROCK SURFACE



ELEVATION OF TOP OF BEDROCK

CONTOUR ON TOP OF BEDROCK SURFACE CONTOUR INTERVAL 10 FEET

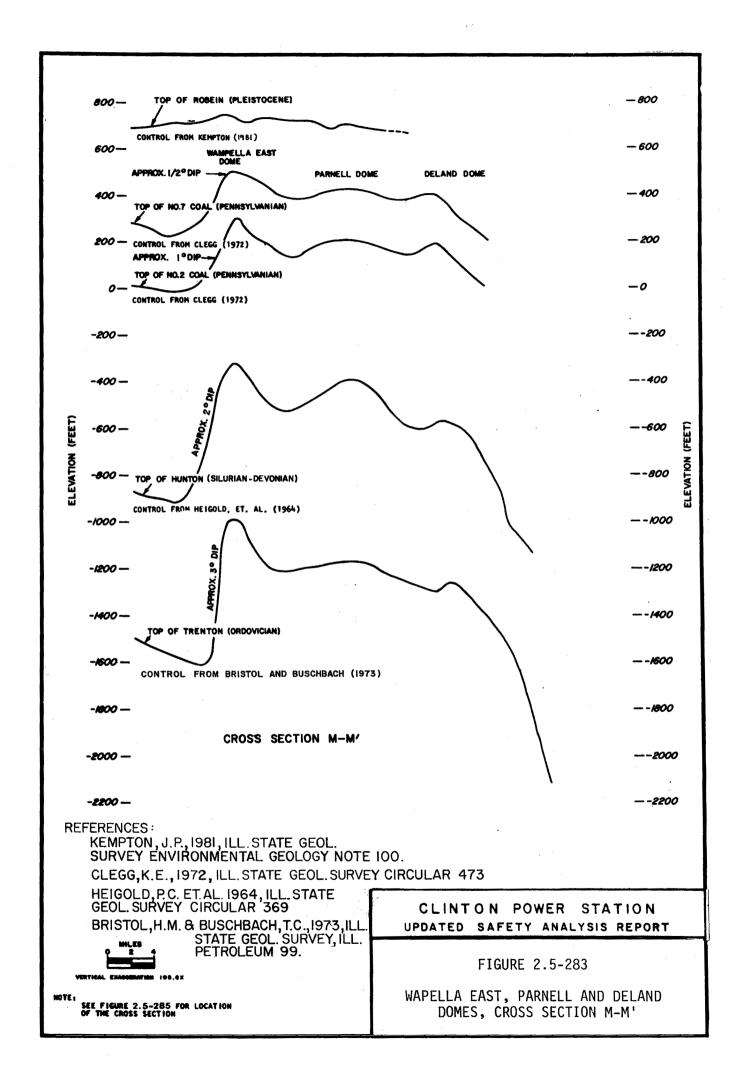
### HOTES:

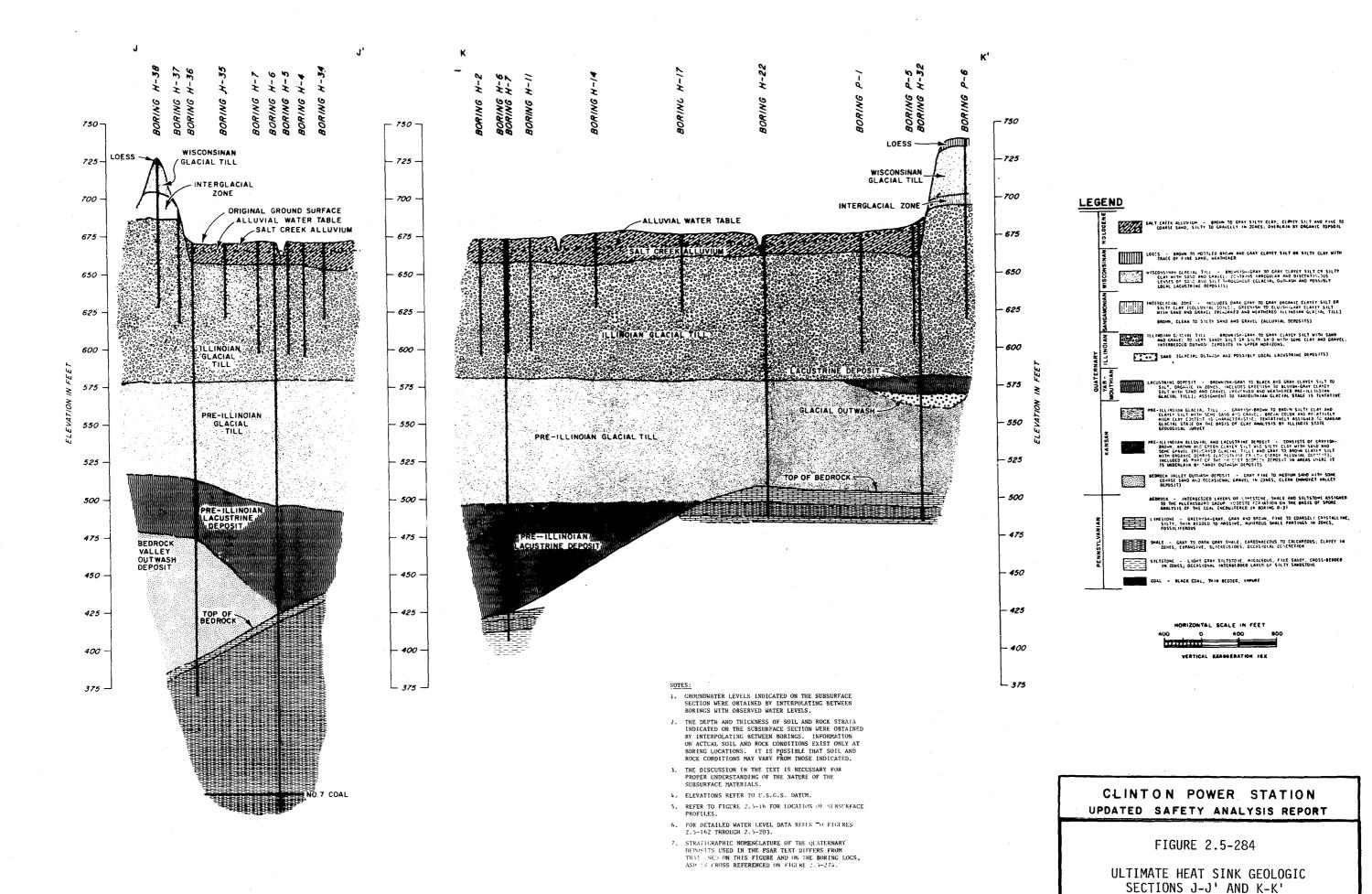
- ENLARGED VIEW OF BORINGS IN PLANT AREA FROM FIGURE 2.5-17.
- BEDROCK CONTOURS ARE BASED ON GEDLOGICAL SIGNIFICANCE OF THE GRAVITY FIELDS IN THE DEVITT-HCLEAN COUNTY AREA, ILLINOIS BY P.C. HEIGOLD, L.D. HCGINHIS AND R.H. HOMARDS: ILLINOIS STATE GEOLOGICAL SURVEY CIRCULAR 369, 1964, WITH MODIFICATION FROM BOREHOLE DATA.

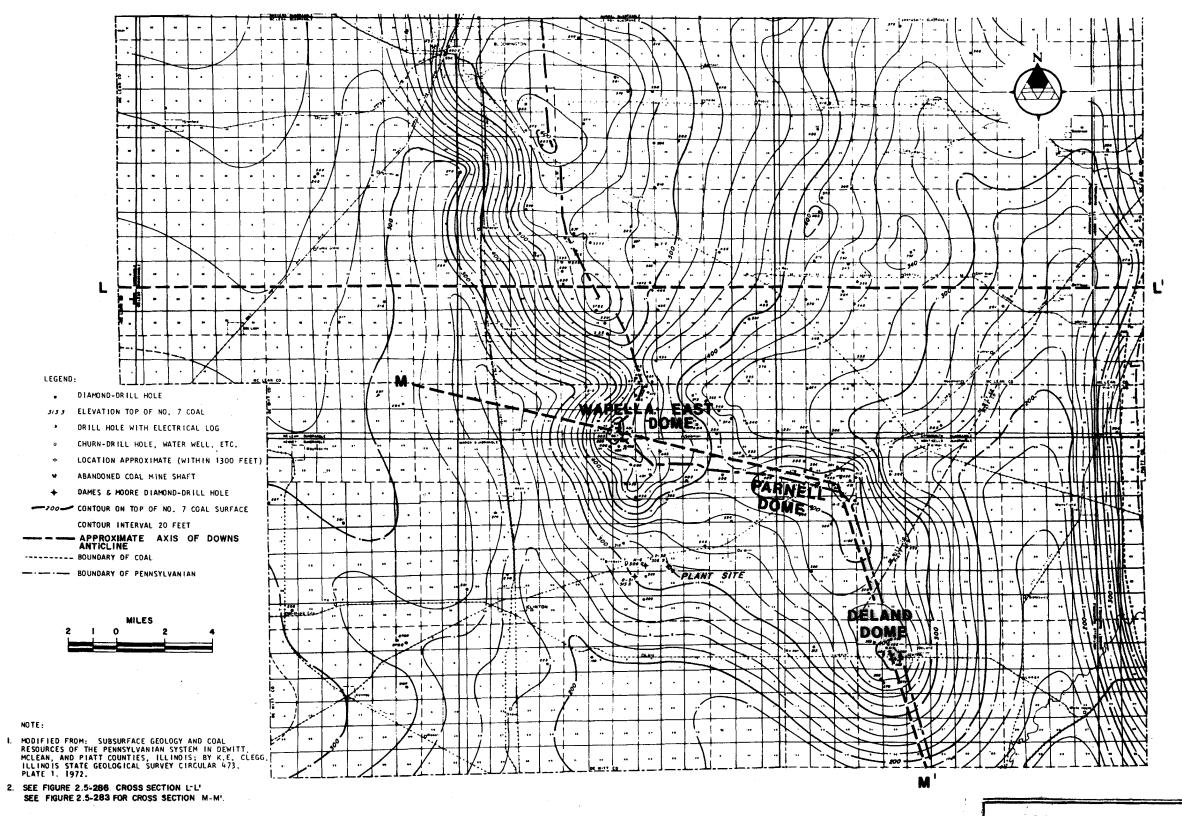
### CLINTON POWER STATION FINAL SAFETY ANALYSIS REPORT

FIGURE 2.5-282

CONTOURS OF BEDROCK SURFACE STATION SITE



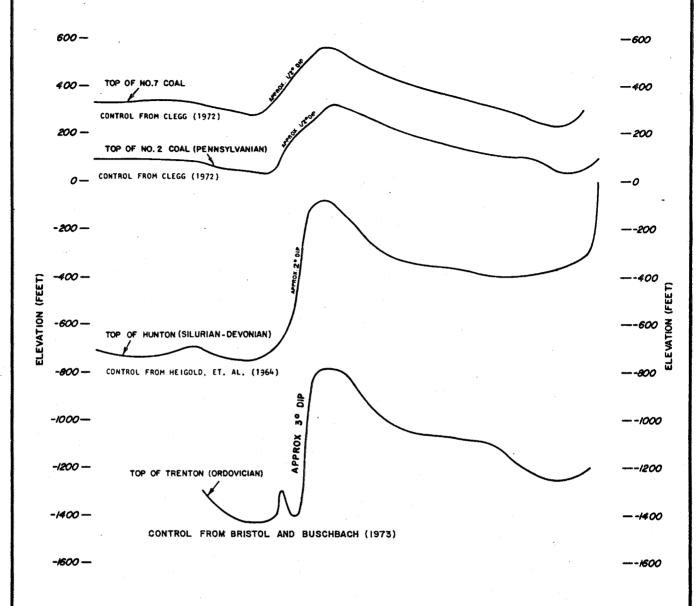




· CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-285

STRUCTURAL CONTOUR MAP OF THE TOP OF THE NUMBER 7 COAL MEMBER



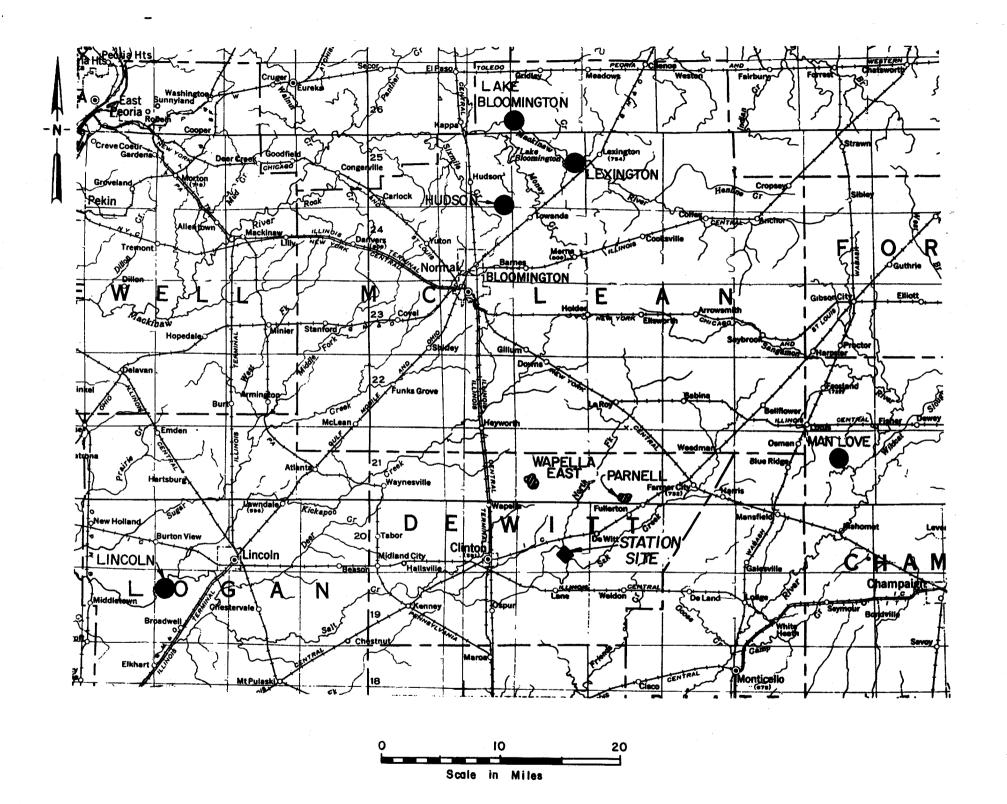
REFERENCES: CROSS SECTION L-L'
KEMPTON, J.P., 1981, ILL. STATE GEOL. SURVEY
ENVIRONMENTAL GEOLOGY NOTE 100.
CLEGG, K.E., 1972, ILL. STATE GEOL. SURVEY CIRCULAR 473.
HEIGOLD, P.C. ET. AL. 1964, ILL. STATE GEOL.
SURVEY CIRCULAR 369
BRISTOL, H.M. & BUSCHBACH, T.C., 1973, ILL. STATE GEOL.
SURVEY, ILL. PETROLEUM 99.



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FIGURE 2.5-286

NOTE: SEE FIGURE 2.5-285 FOR LOCATION OF THE CROSS SECTION DOWNS ANTICLINE - CROSS SECTION L-L'



### **LEGEND**

Gas Storage Project

Oil field

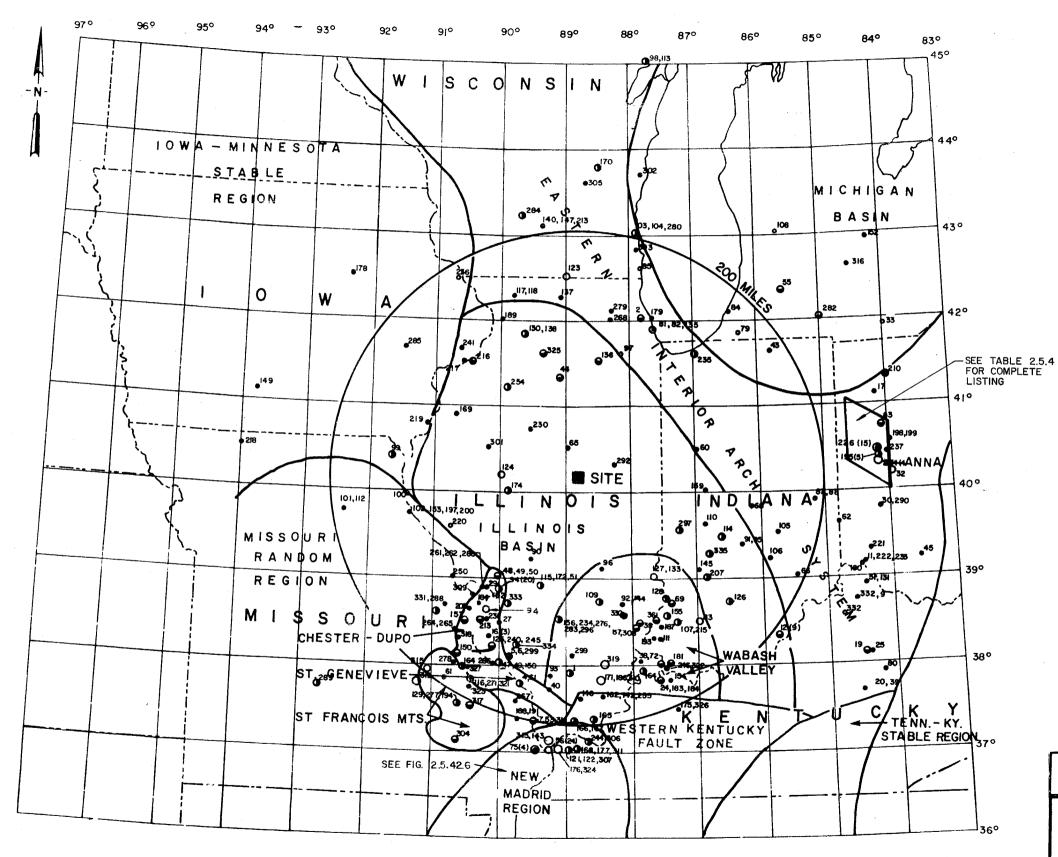
### NOTES

 Adapted from W. F. Meents, Oil and Gas Industry in Illinois, 1977, Illinois State Geological Survey, Urbana, 1977

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FIGURE 2.5-287

LOCATION OF GAS STORAGE PROJECTS AND OIL FIELDS IN THE SITE VICINITY



### LEGEND

LOCATION OF MAXIMUM INTENSITY

- . INTENSITY NOT RECORDED
- IV OR LESS
- O IV-V TO V
- V-VI TO VI
- O VI-VII TO VII
- VN-VN TO VIII

### NOTES

I. BASIS FOR SEISMOTECTONIC
BOUNDARIES DISCUSSED IN TEXT.

2.ONLY THE LARGEST EVENT IS PRESENTED ON MAP.

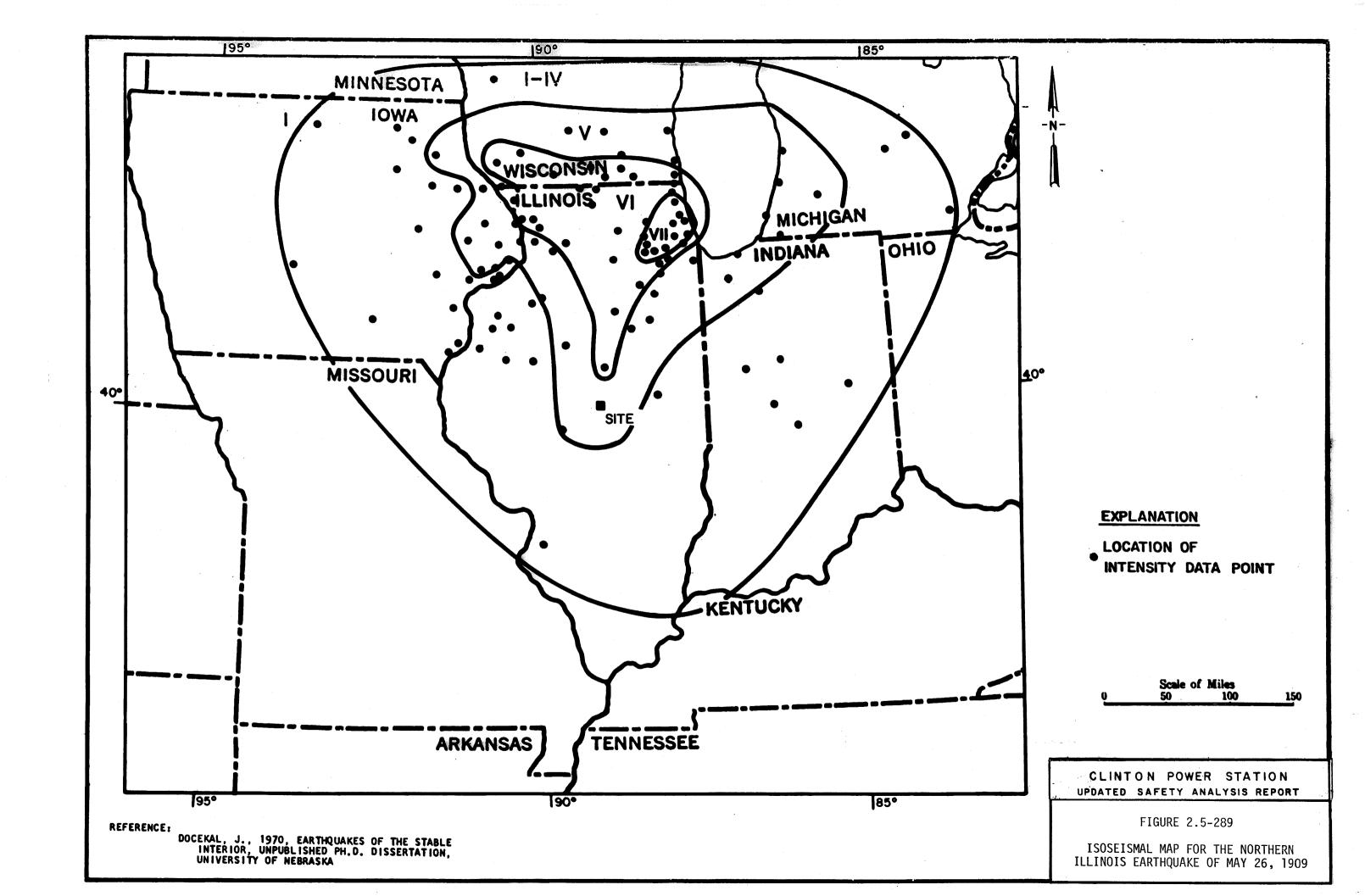
3. EARTHQUAKES LISTED IN TABLE 2.5.4 NOS. IN PARENTHESES INDICATE NO. OF EVENTS AT ONE LOCATION.

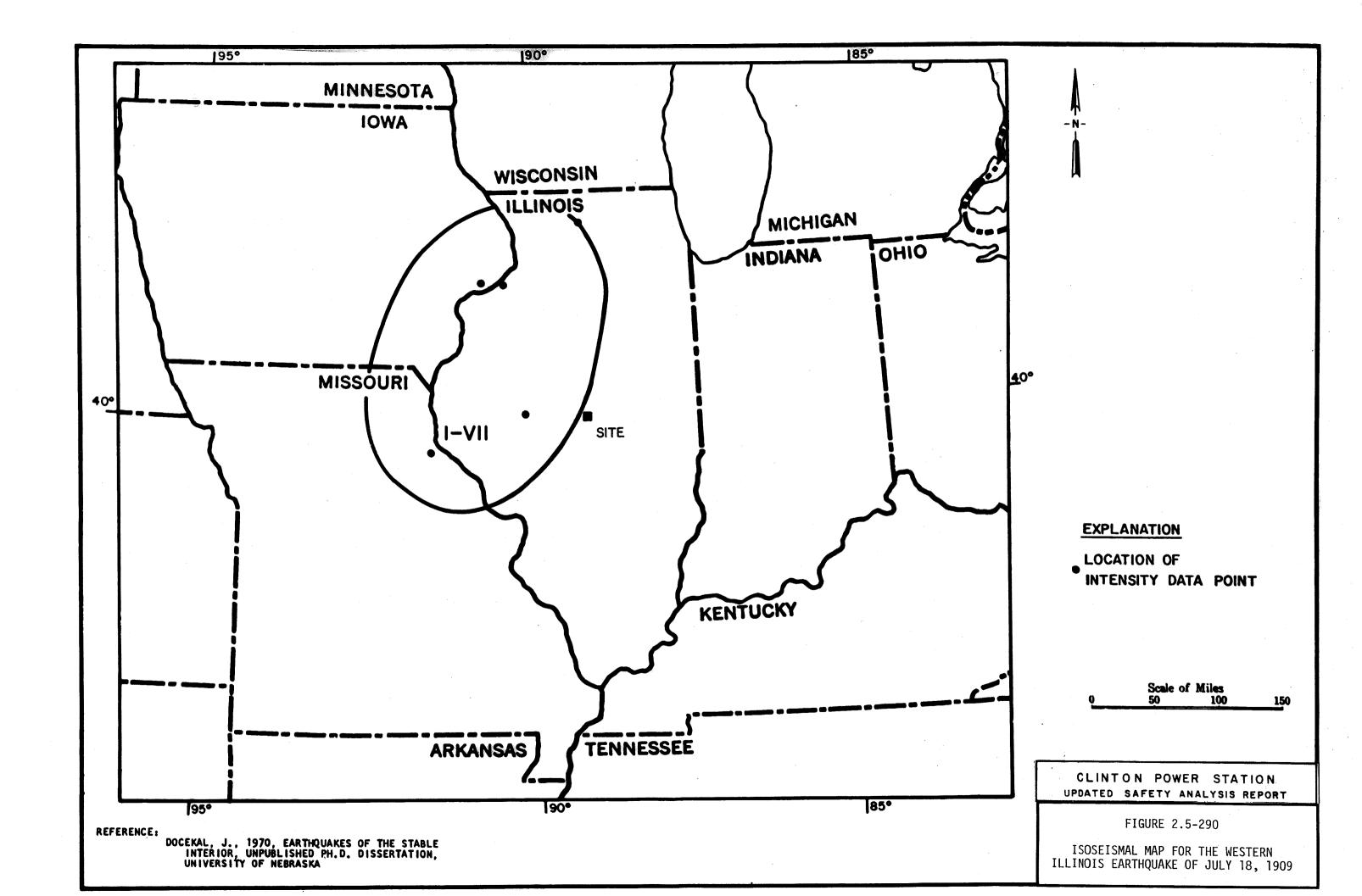


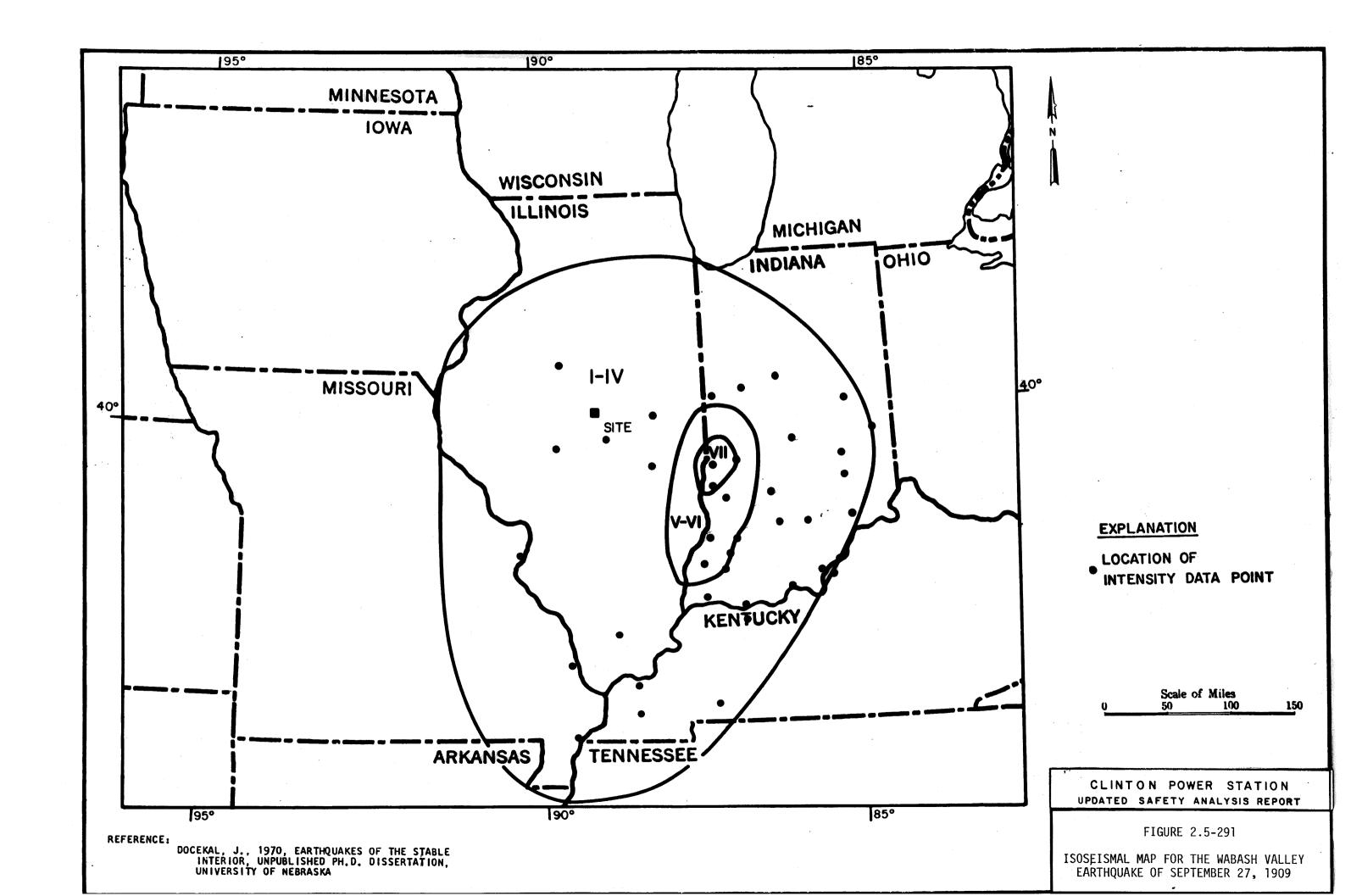
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

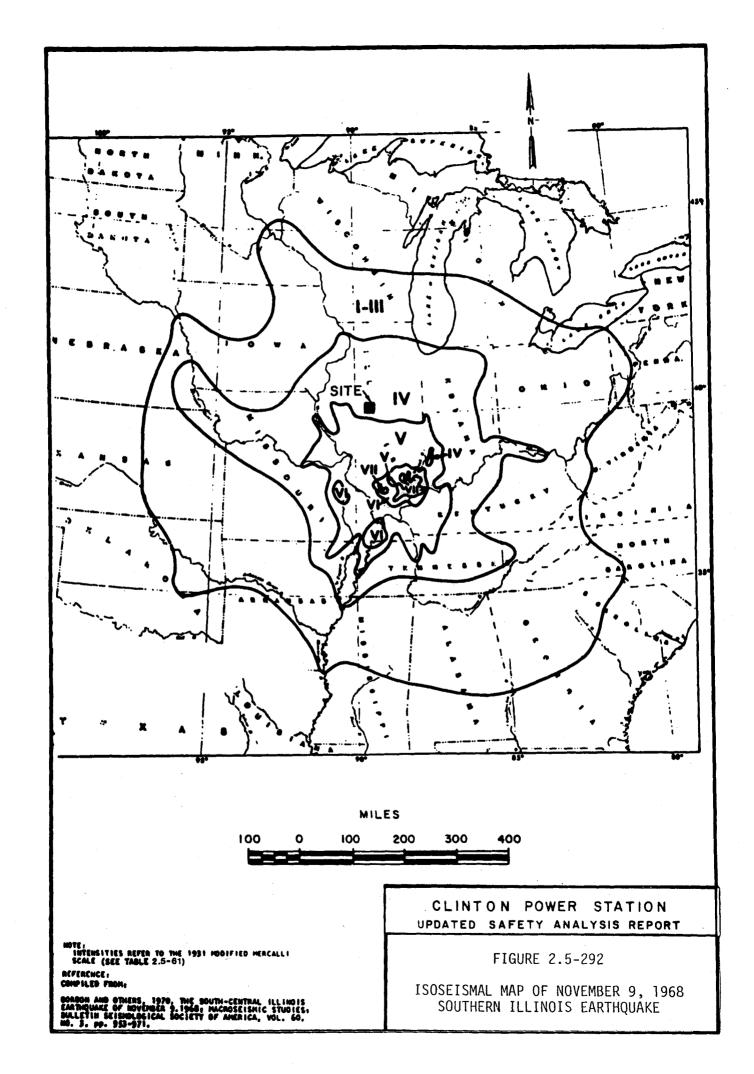
FIGURE 2.5-288

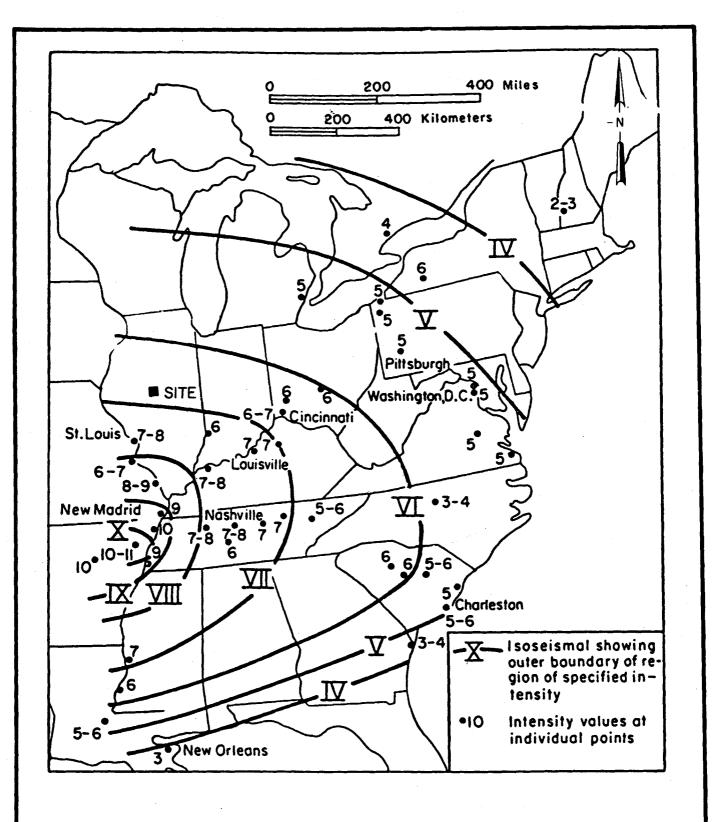
EARTHQUAKE EPICENTERS AND RELATIONSHIP TO SEISMOTECTONIC REGIONS











#### NOTES:

- 1. INTENSITIES REFER TO THE 1931 MODIFIED MERCALLI SCALE.
- 2. ISOSEISMAL LINES INDICATE THE APPROXIMATE OUTER BOUNDARY OF THE REGION OF SPECIFIED INTENSITY.

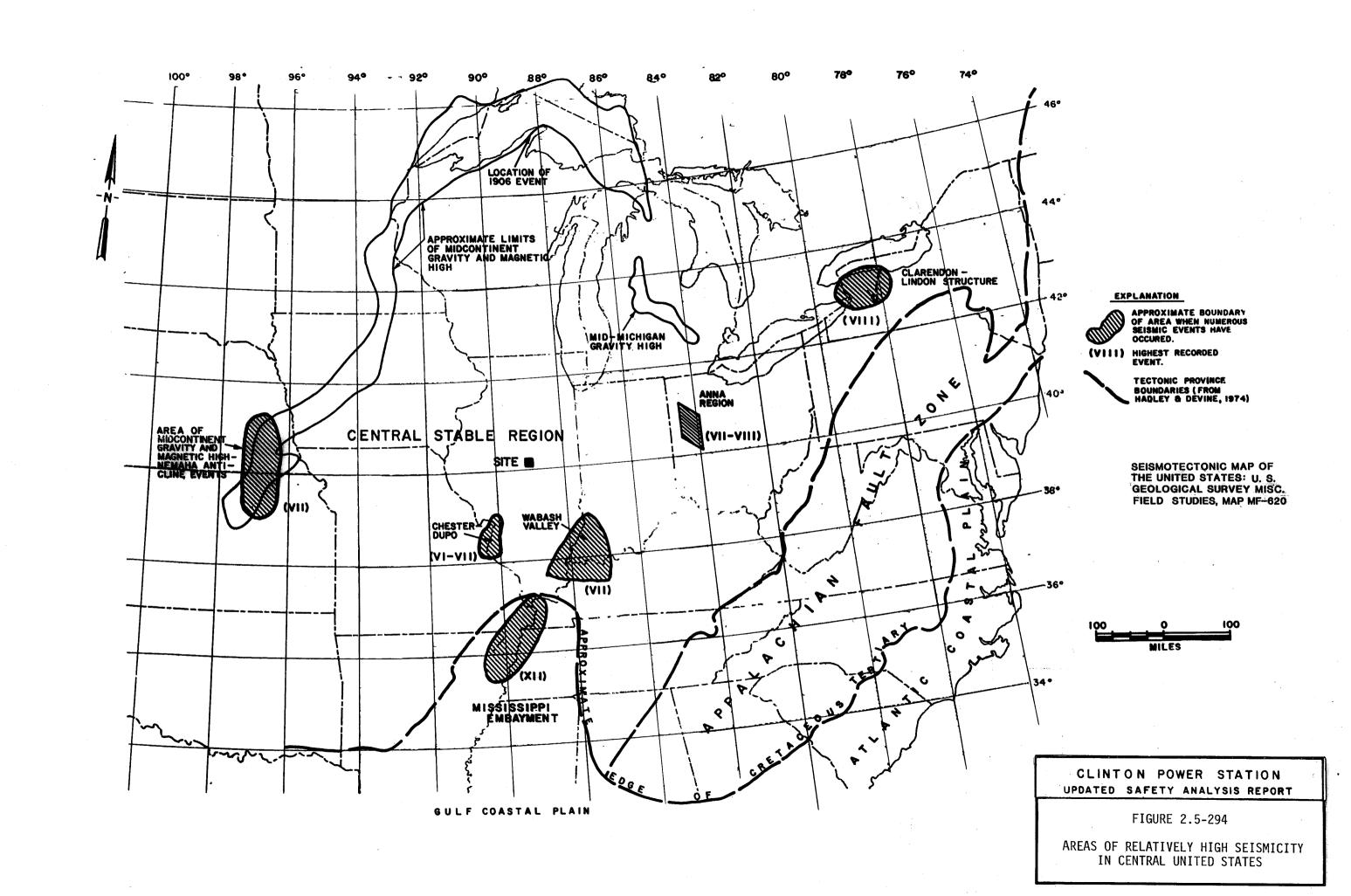
#### REFERENCE:

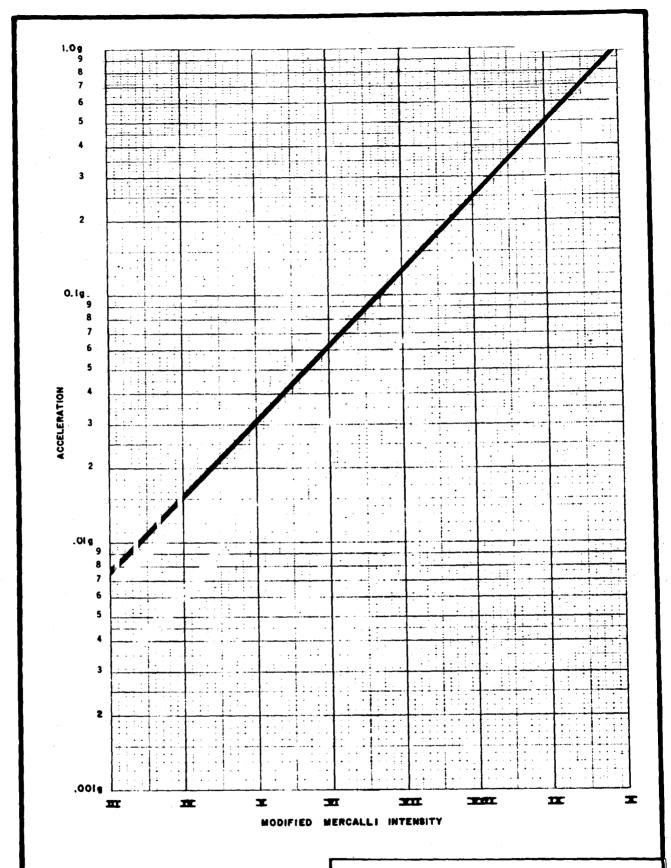
NUTTLI, O.W., 1973, THE MISSISSIPPI VALLEY EARTH-QUAKE OF 1811 AND 1812, INTENSITIES, GROUNDMOTION, AND MAGNITUDES; SEISMOLOGICAL SOCIETY OF AMERICA, BULL. 63, NO. 1 P. 227-248.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-293

ISOSEISMAL MAP FOR NEW MADRID EARTH-QUAKE OF DECEMBER 16, 1811

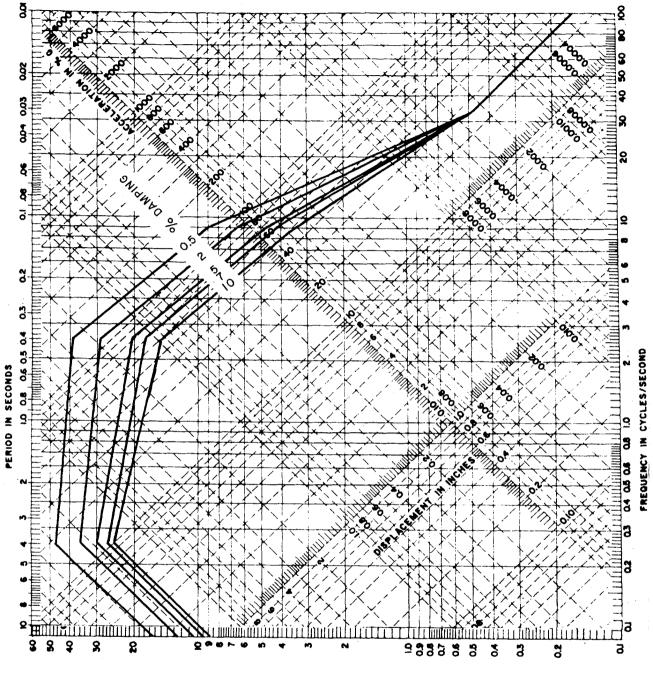




REFERENCE:

TRIFUMMC, M.B., AND BRADY, A.G. 1975, ON THE CORRELATION OF SEISHIC INTENSITY SCALES WITH THE PEAKS OF RECORDED STRONG GROUND HOTION; SEISHOL. SOC. AMERICA BULL., VOL. 65, NO. 1, pp.139-162. FIGURE 2.5-295

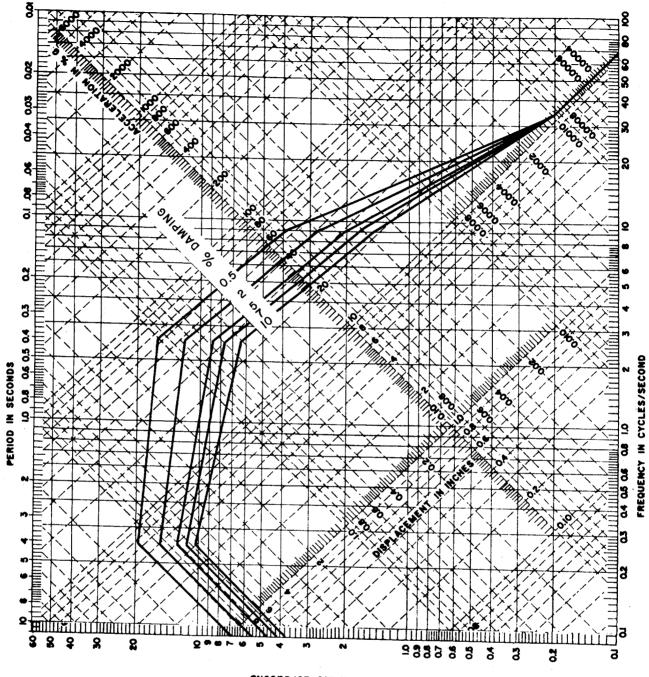
COMPARISON OF EARTHQUAKE INTENSITY AND AVERAGE HORIZONTAL ACCELERATION



AEFOCILL IN INCHES/SECOND

FIGURE 2.5-296

HORIZONTAL SPECTRA FOR A MAXIMUM HORIZONTAL GROUND ACCELERATION OF 26% OF GRAVITY (SAFE SHUTDOWN EARTHQUAKE)



AEFOCILA IN INCHES\SECOND

FIGURE 2.5-297

HORIZONTAL SPECTRA FOR A MAXIMUM HORIZONTAL GROUND ACCELERATION OF 11% OF GRAVITY (OPERATING-BASIS EARTHQUAKE)

### KEY TO LOG OF BORINGS

KEY TO SAMPLES			KEY TO TEST DATA				
BORINGS LOGGED BY:			-		SHEAR STRENGTH DATA:		
WOODWARD- CLYDE CONSULTANTS	DAMES & MOORE	SARGENT & LUNDY ENGINEERS	SAMPLE DESCRIPTION	DAMES & MOORE	SARGENT & LUNDY ENGINEERS	TEST DESCRIPTION	
	15 <b>1</b>		INDICATES THE NUMBER OF BLOWS REQUIRED TO DRIVE A DAMES & MOORE TYPE U SAMPLER, ONE FOOT WITH A 340 POUND WEIGHT FALLING 24 INCHES.  INDICATES DEPTH OF RELATIVELY UNDISTURBED SAMPLE OBTAINED WITH A DAMES & MOORE TYPE U SAMPLER.  INDICATES DAMES & MOORE TYPE U SAMPLER WAS HYDRAULICALLY PUSHED TO OBTAIN SAMPLE.	a. b.	<u> </u>	Triaxial Compression  SHEAR STRENGTH DEFINED AS ONE-HALF THE PEAK AXIAL COMPRESSIVE STRESS IN PSF OR ONE-HALF THE AXIAL COMPRESSIVE STRESS AT 10 PERCENT AXIAL STRAIN, WHICHEVER OCCUPS FIRST.  CELL PRESSURE IN PSF FOR UNCONSOLIDATED COMPRESSION TESTS.	
	0		INDICATES DEPTH OF DISTURBED SAMPLE OBTAINED WITH A DAMES & MOORE TYPE U SAMPLER.  INDICATES DEPTH OF SAMPLING ATTEMPT WITH NO RECOVERY USING A DAMES & MOORE TYPE U SAMPLER.  — INDICATES THE NUMBER OF BLOWS REQUIRED TO DRIVE A SPLIT SPOON SAMPLER, WITH AN OUTSIDE DIAMETER OF	c.	Qu/2 Qu/2*	Unconfined Compression  SHEAR STRENGTH DEFINED AS ONE-HALF THE PEAK AXIAL COMPRESSIVE STRESS IN PSF.  SHEAR STRENGTH DEFINED AS COHESION IN PSF AS DETERMIXED BY A POCKET PENETROMETER. VALUES IN EXCESS OF 4500 PSF ARE INDICATED BY 4500+.	
21 🖬	20 7	23 SS	2.0 INCHES, ONE FOOT WITH A 140 POUND WEIGHT FALLING 30 INCHES (ASTM Test Designation D1586-67).  INDICATES DEPTH OF SAMPLE OBTAINED USING A SPLIT SPOON SAMPLER WITH AN OUTSIDE DIAMETER OF 2.0 INCHES.  INDICATES DEPTH OF SPLIT SPOON SAMPLE WITH NO RECOVERY.	l –	TESTS REPORTED ELSEWHERE:  DAMES & MOORE/SARGENT & LUNDY ENGINEERS  C CONSOLIDATION TEST CHEM CHEMICAL TEST ON GROUNDWATER SAMPLES COMP BULK COMPACTION TEST		
Δ	Ø	AG BG ST	INDICATES AUGER BORING.  INDICATES DEPTH OF DISTURBED SAMPLE OBTAINED WITH CONTINUOUS FLIGHT AUGERS.  INDICATES DEPTH OF UNDISTURBED SAMPLE OBTAINED		D/CD IG MA PERM DR RES	PORE PRESSURE MEASUREMENTS DYNAMIC TRIAXIAL COMPRESSION TEST	
	0	PR	USING A SHELBY TUBE WITH AN GUTSIDE DIAMETER OF 3.0 INCHES AND AN INSIDE DIAMETER OF 2.9 INCHES.  INDICATES DEPTH OF RELATIVELY UNDISTURBED SAMPLE OBTAINED WITH A PITCHER SAMPLER WITH AN OUTSIDE DIAMETER OF 3.0 INCHES AND AN INSIDE DIAMETER OF 2.9 INCHES.	SA SHOCK TX/CD TX/CU/P TX/DY TX/UU/U TX/UU/R	SA SHOCK TX/CD TX/CU/P TX/DY		
		ОВ	INDICATES DEPTH OF RELATIVELY UNDISTURBED SAMPLE OBTAINED USING AN OSTERBERG SAMPLER WITH AN OUTSIDE DIAMETER OF 3.0 INCHES AND AN INSIDE DIAMETER OF 2.9 INCHES.		ON UNDISTURBED SAMPLE.		
	O T	HR	INDICATES DEPTH OF RELATIVELY UNDISTURBED SAMPLE OBTAINED WITH A DOUBLE TUBE CORE BARREL WITH AN INSIDE DIAMETER OF 4.0 INCHES. (HIGH RECOVERY CORE BARREL)				
	95% <u> </u>		INDICATES DEPTH, LENGTH AND PERCENT OF CORE RUN RECOVERED FOR NX DIAMOND DRILL ROCK CORING.  INDICATES PERCENT OF ROCK QUALITY DESIGNATION FOR NX DIAMOND DRILL ROCK CORING.				

ELEVATION REFERENCE

DRILLING REFERENCE

ELEVATIONS REFER TO MEAN SEA LEVEL DATUM.

BORINGS WERE DRILLED USING TRUCK-MOUNTED AUGER/ROTARY WASH TYPE DRILLING EQUIPMENT.

#### PIEZOMETER REFERENCE

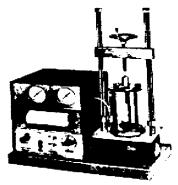
PIEZOMETERS WERE INSTALLED IN BORINGS TO RECORD GROUND WATER CONDITIONS. DETAILS OF EACH INSTALLATION ARE DESCRIBED ON THE BORING LOGS.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-298

KEY TO LOG OF BORINGS

# Triaxial Compression Test Unit



TRIAXIAL COMPRESSION TEST UNIT

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Figure 2.5-299
Sheet 1 of 2
UNCONFINED COMPRESSION
AND TRIAXIAL COMPRESSION
TESTS (METHOD)

#### NOTES FOR FIGURE 2.5-299

#### Methods of Performing Unconfined Compression and Triaxial Compression Tests

The shearing strengths of soils are determined from the results of unconfined compression and triaxial compression tests. In triaxial compression tests the test method and the magnitude of the confining pressure are chosen to simulate anticipated field conditions.

Unconfined compression and triaxial compression tests are performed on undisturbed, or remolded samples of soil, approximately six inches in length and two and one-half inches in diameter. The tests are run either strain-controlled or stress-controlled. In a strain-controlled test the sample is subjected to a constant rate of deflection and the resulting stresses are recorded. In a stress-controlled test the sample is subjected to equal increments of load with each increment being maintained until an equilibrium condition with respect to strain is achieved.

Yield, peak, or ultimate stresses are determined from the stress-strain plot for each sample and the principal stresses are evaluated. The principal stresses are plotted on a Mohr's circle diagram to determine the shearing strength of the soil type being tested.

Unconfined compression tests can be performed only on samples with sufficient cohesion so that the soil will stand as an unsupported cylinder. These tests may be run at natural moisture content or on artificially saturated soils.

In a triaxial compression test the sample is encased in a rubber membrane, placed in a test chamber, and subjected to a confining pressure throughout the duration of the test. Normally, this confining pressure is maintained at a constant level, although for special tests it may be varied in relation to the measured stresses. Triaxial compression tests may be run on soils at field moisture content or on artificially saturated samples.

The tests are performed in one of the following ways:

<u>Unconsolidated-undrained:</u> The confining pressure is imposed on the sample at the start of the test. No drainage is permitted and the stresses which are measured represent the sum of the intergranular stresses and pore water pressures.

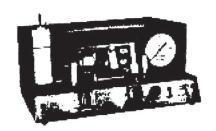
<u>Consolidated-undrained:</u> The sample is allowed to consolidate fully under the applied confining pressure prior to the start of the test. The volume change is determined by measuring the water and/or air expelled during consolidation. No drainage is permitted during the test and the stresses which are measured are the same as for the unconsolidated-undrained test.

<u>Drained:</u> The intergranular stresses in a sample may be measured by performing a drained, or slow, test. In this test, the sample is fully saturated and consolidated prior to the start of the test. During the test, drainage is permitted and the test is performed at a slow enough rate to prevent the buildup of pore water pressures. The resulting stresses which are measured represent only the intergranular stresses. These tests are usually performed on samples of generally non-cohesive soils, although the test procedure is applicable to cohesive soils if a sufficiently slow test rate is used.

An alternate means of obtaining the data resulting from the drained test is to perform an undrained test in which special equipment is used to measure the pore water pressures. The differences between the total stresses and the pore water pressures measured are the intergranular stesses.

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Figure 2.5-299 Sheet 2 of 2 UNCONFINED COMPRESSION AND TRIAXIAL COMPRESSION TESTS (METHOD)



DIRECT SHEAR TESTING & RECORDING APPARATUS

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure 2.5-300 Sheet 1 of 2

DIRECT SHEAR AND FRICTION TESTS (METHOD)

#### **NOTES FOR FIGURE 2.5-300**

#### Method of Performing Direct Shear and Friction Tests

Direct shear tests are performed to determine the shearing strengths of soils. Friction tests are performed to determine the frictional resistances between soils and various other materials such as wood, steel, or concrete. The tests are performed in the laboratory to simulate anticipated field conditions.

Each sample is tested within three brass rings, two and one-half inches in diameter and one inch in length. Undisturbed samples of in-place soils are tested in rings taken from the sampling device in which the samples were obtained. Loose samples of soils to be used in constructing earth fills are compacted in rings to predetermined conditions and tested.

#### **Direct Shear Tests**

A three-inch length of the sample is tested in direct double shear. A constant pressure, appropriate to the conditions of the problem for which the test is being performed, is applied normal to the ends of the sample through porous stones. A shearing failure of the sample is caused by moving the center ring in a direction perpendicular to the axis of the sample. Transverse movement of the outer rings is prevented.

The shearing failure may be accomplished by applying to the center ring either a constant rate of load, a constant rate of deflection, or increments of load or deflection. In each case, the shearing load and the deflections in both the axial and transverse directions are recorded and plotted. The shearing strength of the soil is determined from the resulting load-deflection curves.

#### Friction Tests

In order to determine the frictional resistance between soil and the surfaces of various materials, the center ring of soil in the direct shear test is replaced by a disk of the material to be tested. The test is then performed in the same manner as the direct shear test by forcing the disk of material from the soil surfaces.

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Figure 2.5-300 Sheet 2 of 2 DIRECT SHEAR AND FRICTION TESTS (METHOD)

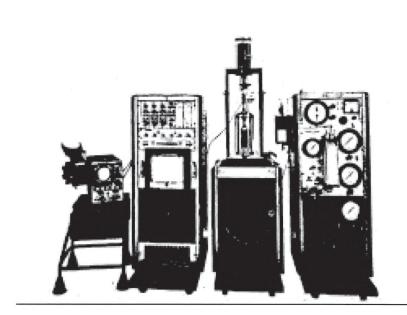


Figure 2.5-301 Sheet 1 of 2

PULSATING LOAD TRIAXIAL TEST (METHOD)

#### NOTES FOR FIGURE 2.5-301

#### Methods of Performing Pulsating Load Triaxial Tests

Pulsating axial load tests are performed to evaluate the dynamic properties and the liquefaction potential of the soils under simulated anticipated field loading conditions.

Pulsating load tests are stress controlled and are performed on undisturbed or reconstituted samples of soil approximately six inches in length and two and one-half inches in diameter. The samples are encased in a rubber membrane, placed in a test chamber, and subjected to confining pressure throughout the duration of the test. The tests may be run on soils at field moisture content or on artificially saturated samples. The triaxial equipment acting through a Bellofram system applies a pulsating axial load. The cycling speed of the load can be varied between one-half to five cycles per second to simulate the field loading frequency.

#### Dynamic Properties Determination

To evaluate the dynamic parameters, the soil sample is loaded in cyclic compression. The load and deflection are recorded on two channels of a recording oscillograph. By tapping the output of the load and deflection transducers and applying these to vertical and horizontal plates, respectively, of a cathode ray oscilloscope, a hysteresis loop is produced. This loop is photographed, and the photograph is used to evaluate the damping value present. The procedure is repeated at various strain amplitudes to evaluate the dynamic properties in the range of interest on a particular sample. The load and deflection values obtained from the oscillograph are used to evaluate the dynamic moduli of elasticity.

#### Liquefaction Potential

To evaluate the liquefaction potential, the soil sample is subjected to axial cyclic loading, the magnitude, frequency, duration and sequence of loading is determined on the basis of past earthquake records. The load deflection, and pore pressure are recorded on three channels of a recording oscillograph. These records are used to evaluate the liquefaction potential for that particular soil type under the test conditions.

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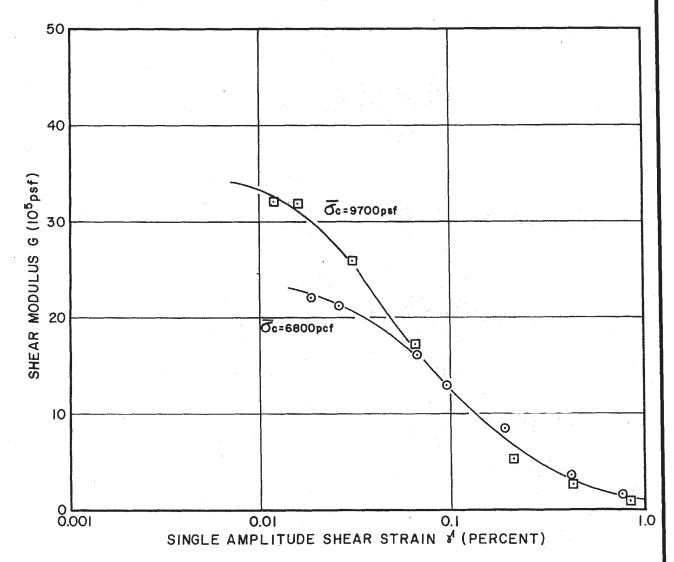
Figure 2.5-301 Sheet 2 of 2

PULSATING LOAD TRIAXIAL TEST (METHOD)

COMBINED BULK SAMPLE:

G-18 (ELEV. 663-654) G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

FOR 80% RELATIVE DENSITY (123 PCF), AT THE 10TH CYCLE



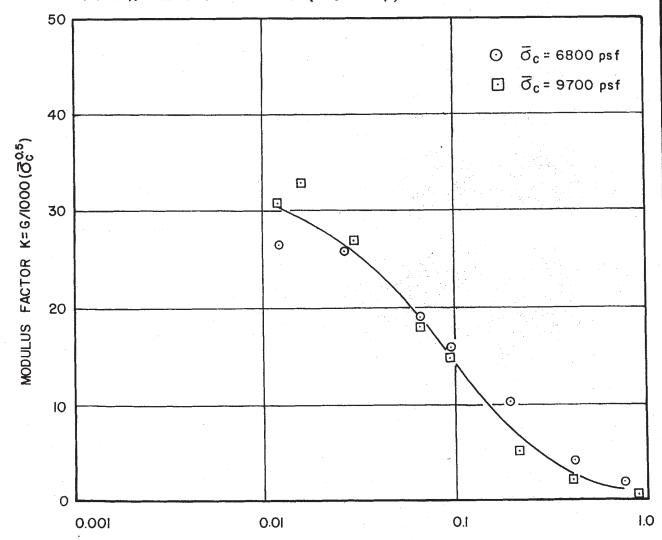
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-302

COMBINED BULK SAMPLE:

G-18 (ELEV. 663-654) G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

FOR 80% RELATIVE DENSITY (123 PCF), AT THE 10TH CYCLE



SINGLE AMPLITUDE SHEER STRAIN & (PERCENT)

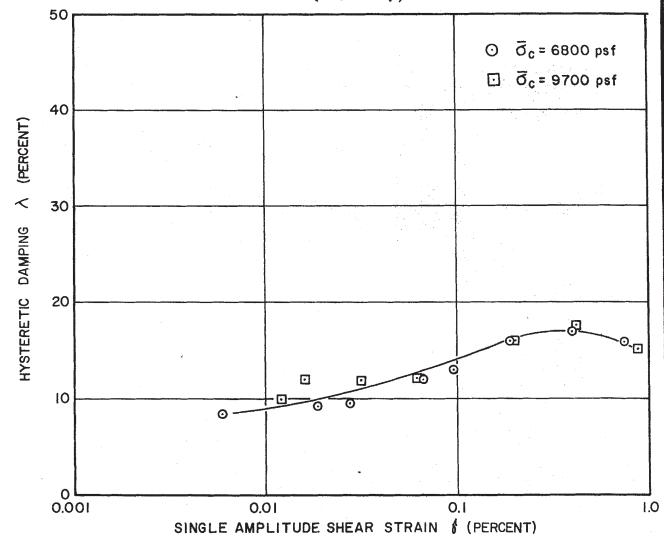
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-303

COMBINED BULK SAMPLE:

G-18 (ELEV. 663-654) G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

FOR 80% RELATIVE DENSITY (123 PCF), AT THE 10TH CYCLE



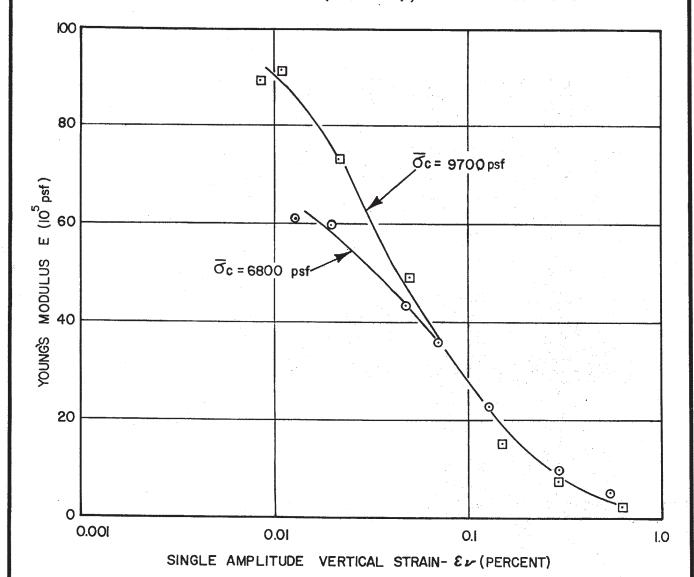
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-304

COMBINED BULK SAMPLE:

G-18 (ELEV. 663-654) G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

FOR 80% RELATIVE DENSITY (123 PCF), AT THE 10TH CYCLE



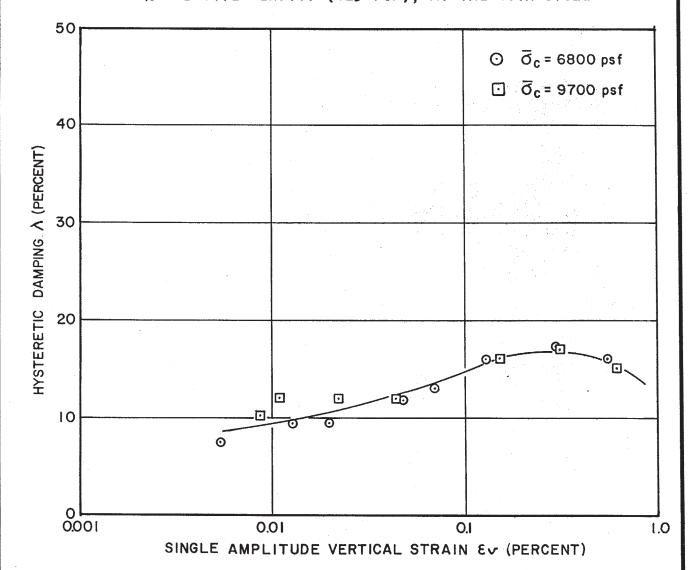
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-305

COMBINED BULK SAMPLE:

G-18 (ELEV. 663-654) G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

FOR 80% RELATIVE DENSITY (123 PCF), AT THE 10TH CYCLE



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

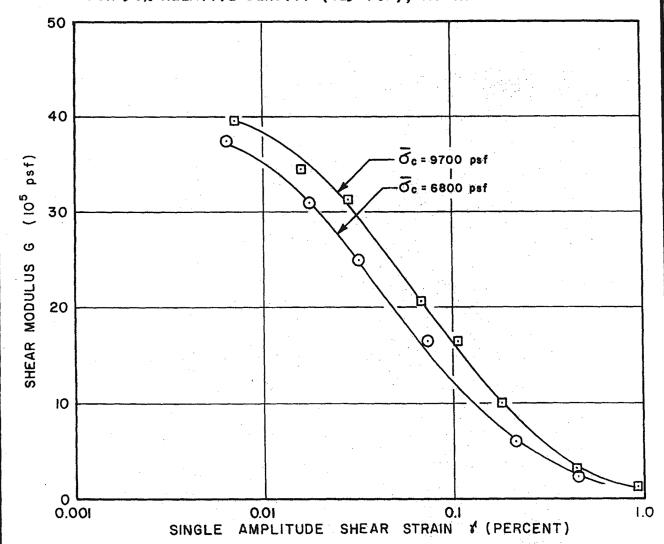
FIGURE 2.5-306

COMBINED BORROW SAMPLE:

G-18 (ELEV. 663-654)

G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

FOR 90% RELATIVE DENSITY (129 PCF), AT THE 10TH CYCLE



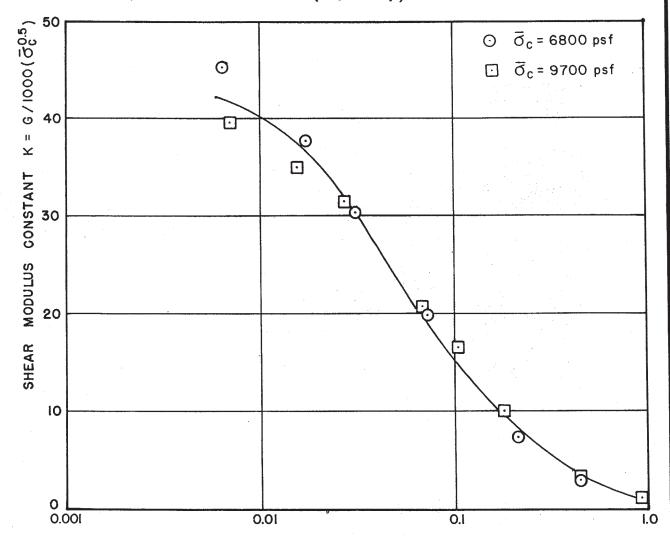
CLINTON POWER: STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-307

COMBINED BORROW SAMPLE:

G-18 (ELEV. 663-654) G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

FOR 90% RELATIVE DENSITY (129 PCF), AT THE 10TH CYCLE



SINGLE AMPLITUDE SHEAR STRAIN & (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

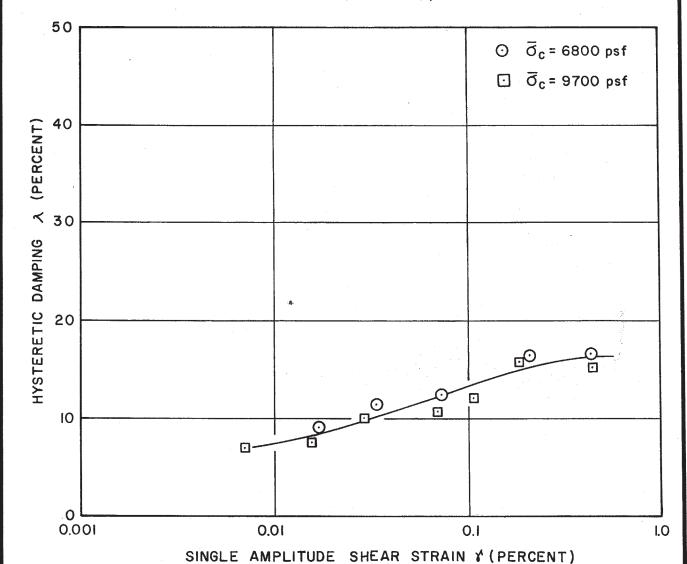
FIGURE 2.5-308

COMBINED BORROW SAMPLE:

G-18 (ELEV. 663-654)

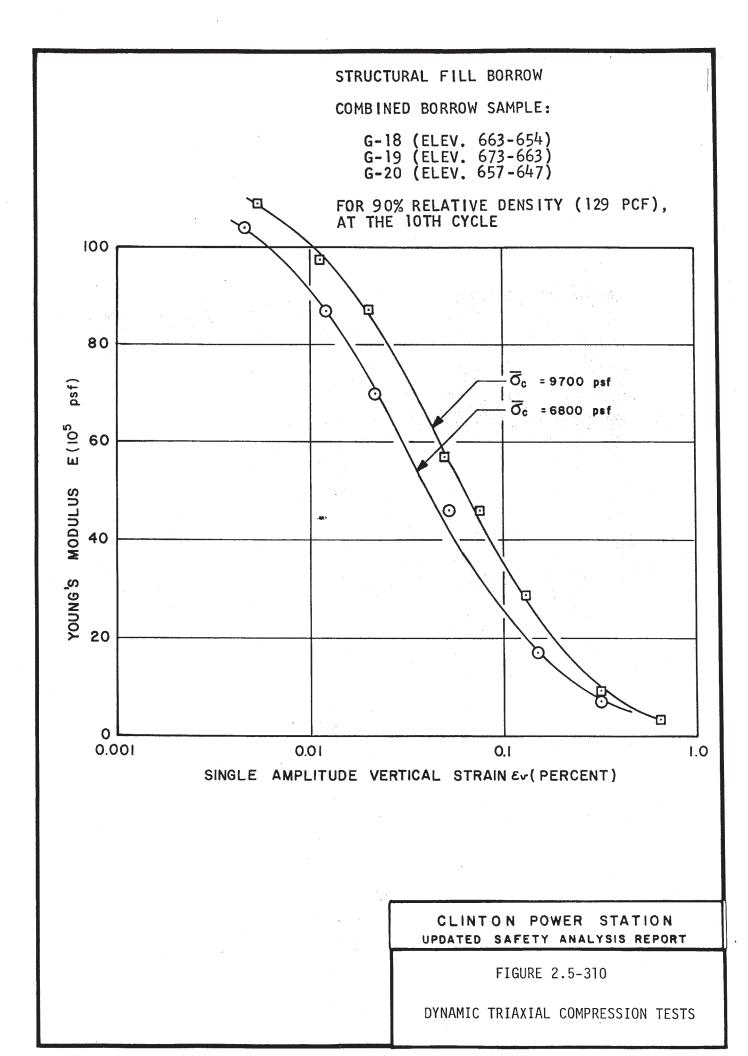
G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

FOR 90% RELATIVE DENSITY (129 PCF), AT THE 10TH CYCLE



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-309

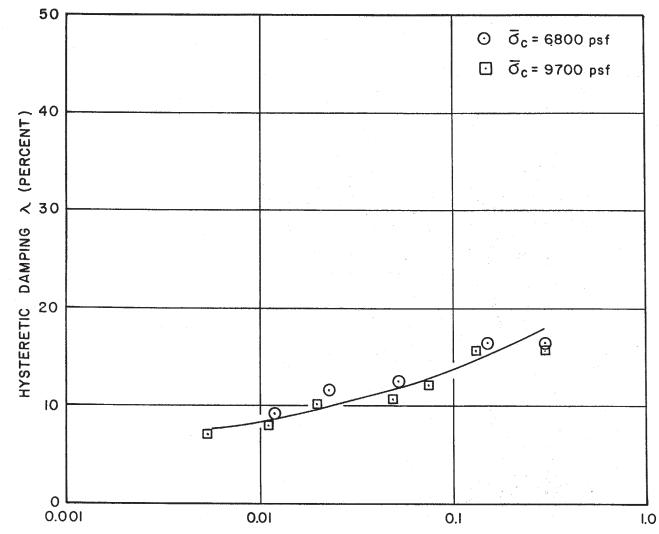


COMBINED BORROW SAMPLE:

G-18 (ELEV. 663-654)

G-19 (ELEV. 673-663) G-20 (ELEV. 657-647)

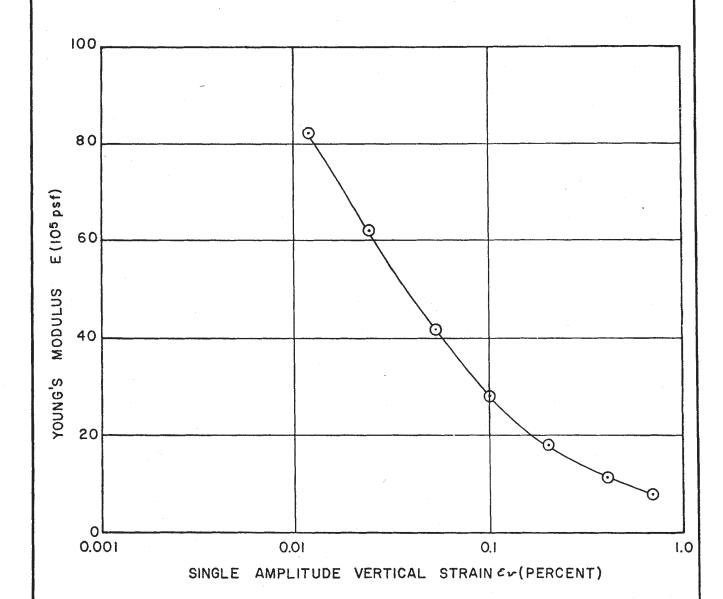
FOR 90% RELATIVE DENSITY (129 PCF), AT THE 10TH CYCLE



SINGLE AMPLITUDE VERTICAL STRAIN Ex (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-311

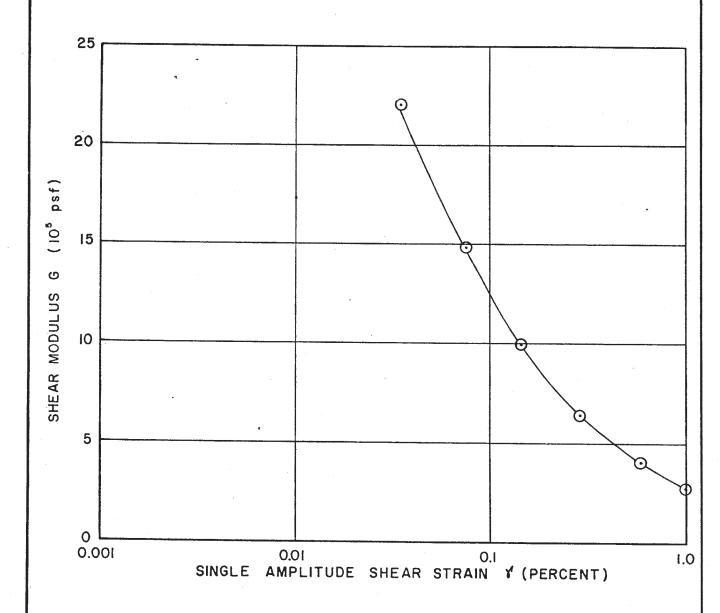


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-312

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-6)

(SHEET 1 of 4)

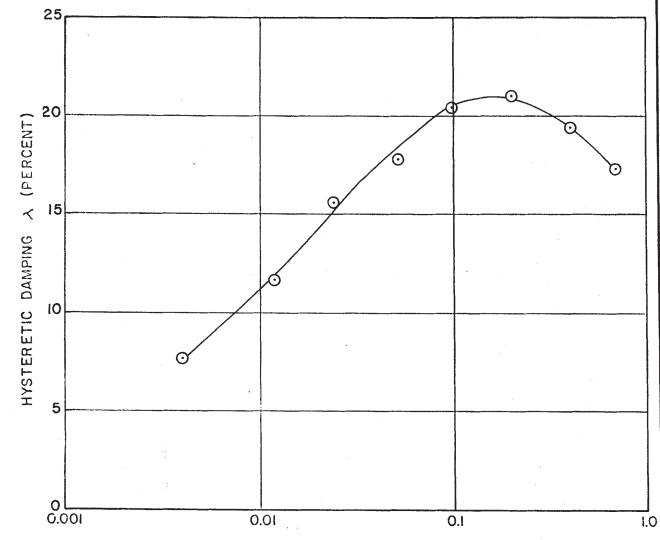


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-312

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-6)

(SHEET 2 of 4)



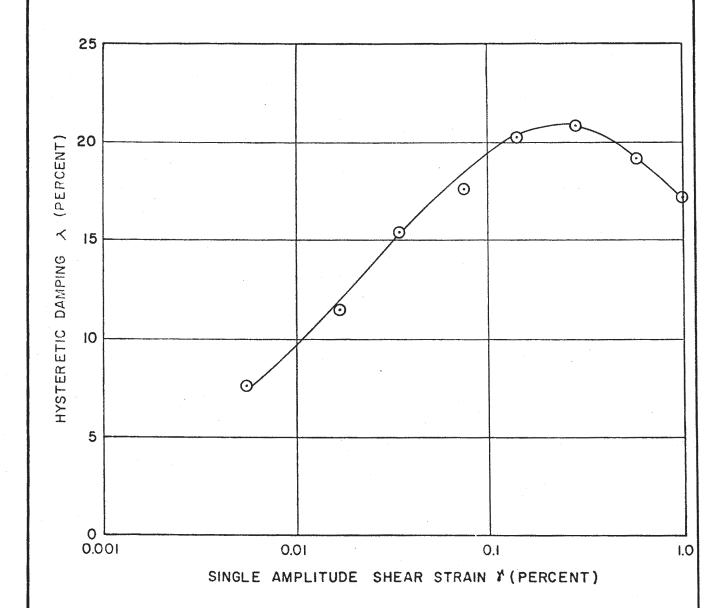
SINGLE AMPLITUDE VERTICAL STRAIN &> (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-312

DYNAMIC TRIAXIAL COMPRESSION TESTS
(BORING H-6)

(SHEET 3 of 4)

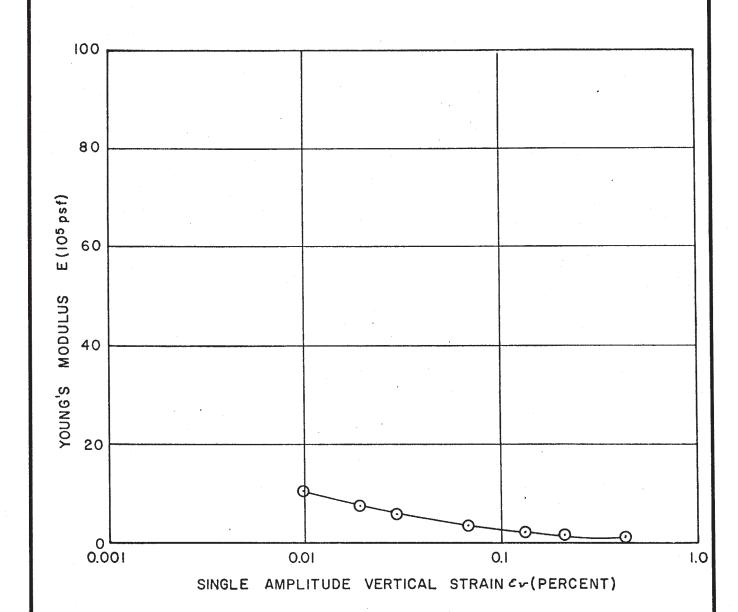


CLINTON POWER STATION
UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-312

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-6)

(SHEET 4 of 4)

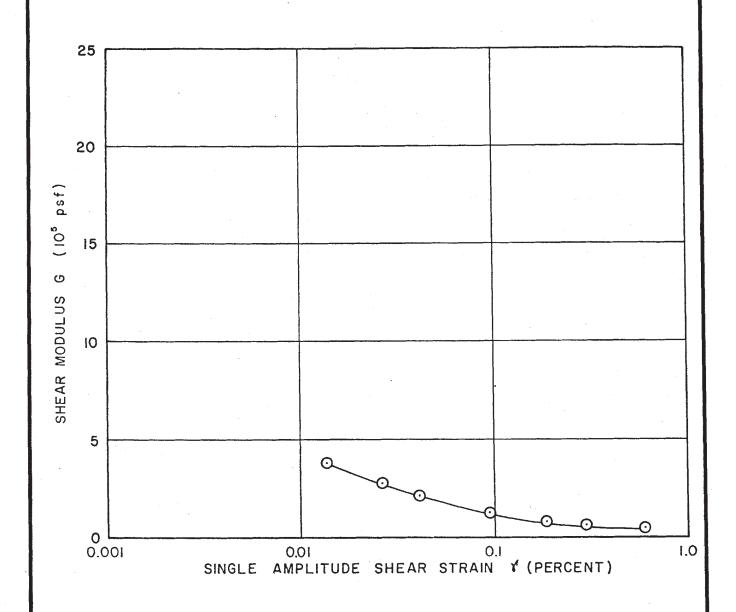


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-313

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-14)

(SHEET 1 of 4)

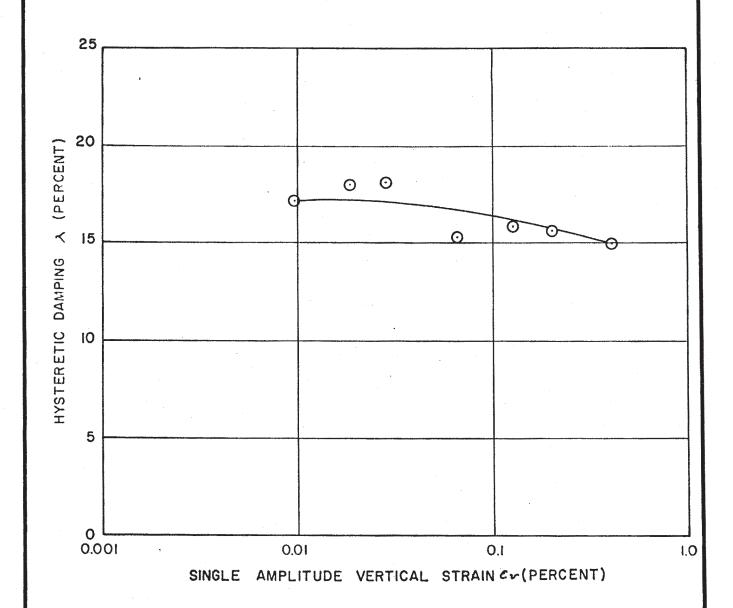


CLINTON POWER STATION
UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-313

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-14)

(SHEET 2 of 4)

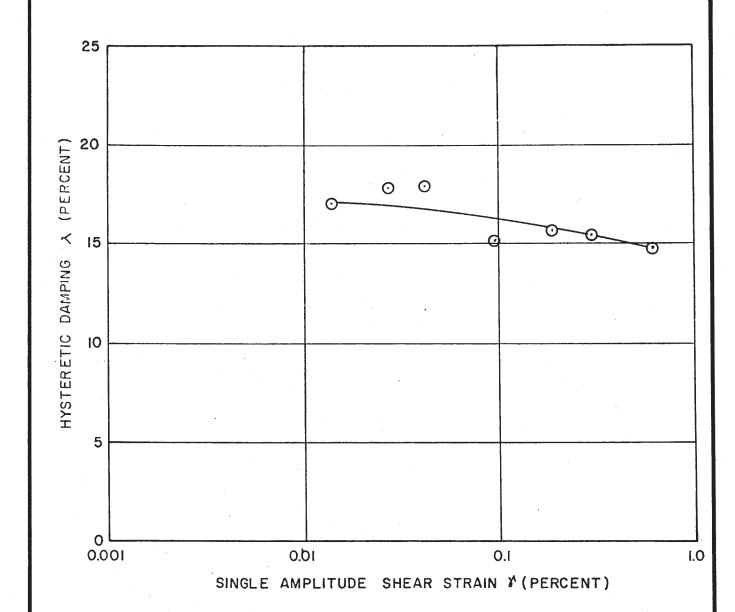


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-313

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-14)

(SHEET 3 of 4)



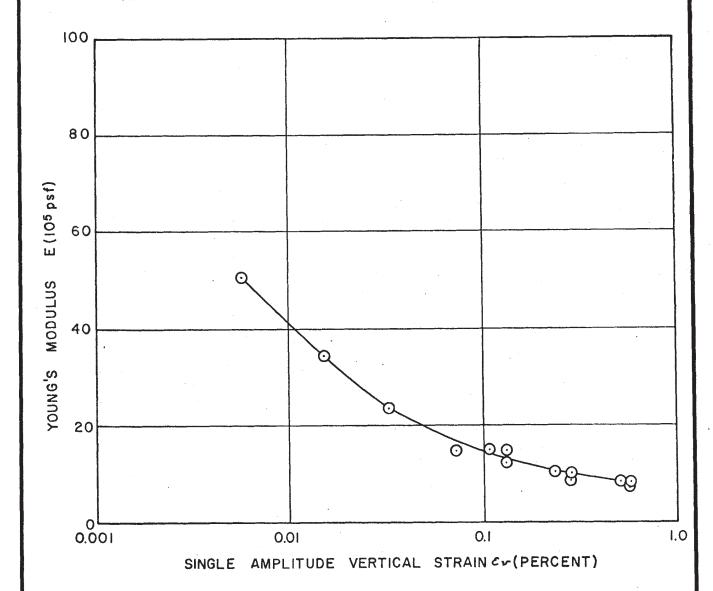
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-313

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-14)

(SHEET 4 of 4)

BORING H-20 @ ELEVATION 706.8 FEET GRAY FINE SANDY SILT WITH SOME CLAY AND MEDIUM TO COARSE SAND AND GRAVEL (WISCONSINAN GLACIAL TILL) FIELD MOISTURE CONTENT: 8.6% FIELD DRY DENSITY: 136 LBS./CU.FT. TEST DATA OBTAINED FROM PITCHER SAMPLE



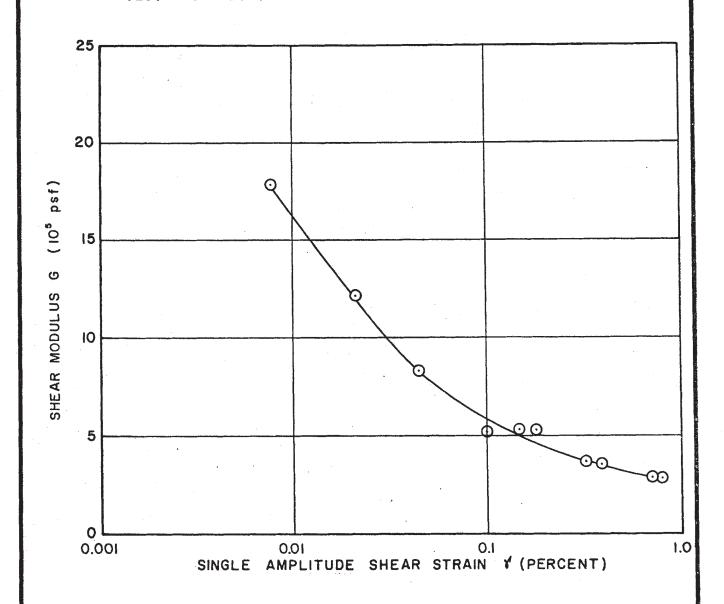
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 1 of 12)

BORING H-20 @ ELEVATION 706.8 FEET GRAY FINE SANDY SILT WITH SOME CLAY AND MEDIUM TO COARSE SAND AND GRAVEL (WISCONSINAN GLACIAL TILL) FIELD MOISTURE CONTENT: 8.6% FIELD DRY DENSITY: 136 LBS./CU.FT. TEST DATA OBTAINED FROM PITCHER SAMPLE



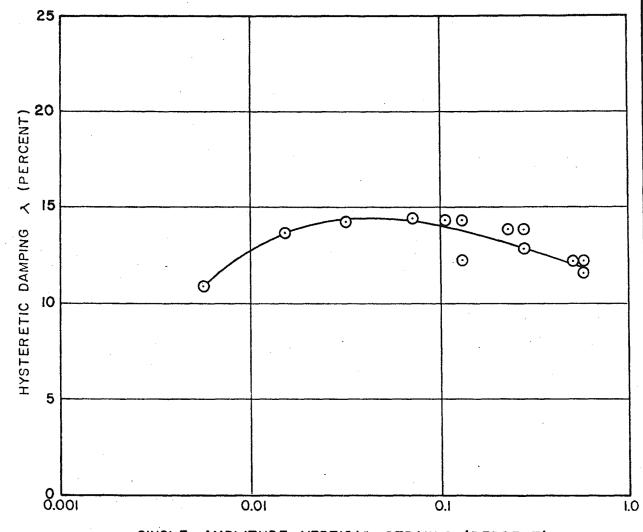
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 2 of 12)

BORING H-20 @ ELEVATION 706.8 FEET GRAY FINE SANDY SILT WITH SOME CLAY AND MEDIUM TO COARSE SAND AND GRAVEL (WISCONSINAN GLACIAL TILL) FIELD MOISTURE CONTENT: 8.6% FIELD DRY DENSITY: 136 LBS./CU.FT. TEST DATA OBTAINED FROM PITCHER SAMPLE



SINGLE AMPLITUDE VERTICAL STRAIN Cr (PERCENT)

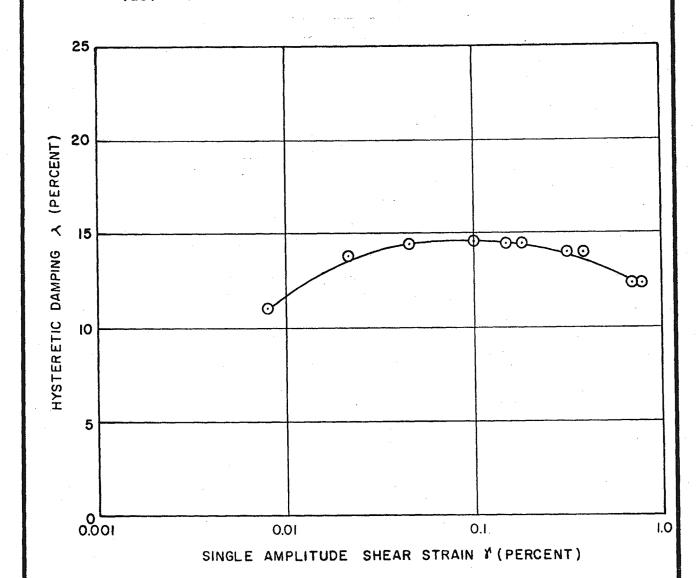
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 3 of 12)

BORING H-20 @ ELEVATION 706.8 FEET GRAY FINE SANDY SILT WITH SOME CLAY AND MEDIUM TO COARSE SAND AND GRAVEL (WISCONSINAN GLACIAL TILL) FIELD MOISTURE CONTENT: 8.6% FIELD DRY DENSITY: 136 LBS./CU.FT. TEST DATA OBTAINED FROM PITCHER SAMPLE

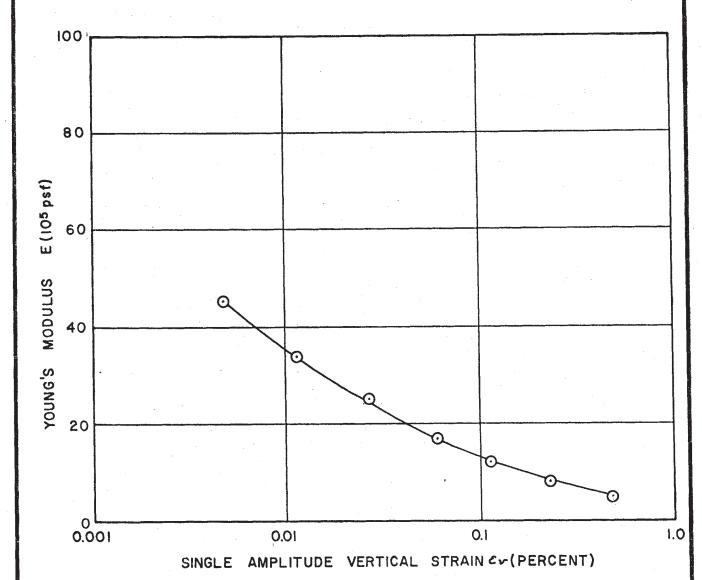


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 4 of 12)

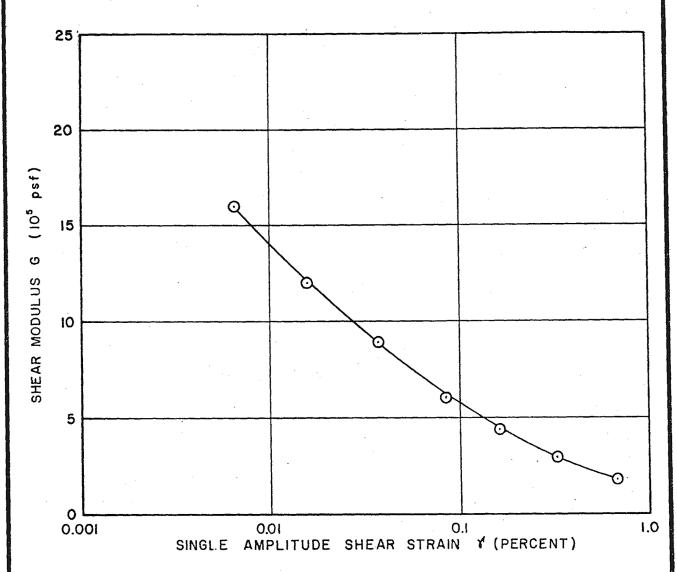


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 5 of 12)

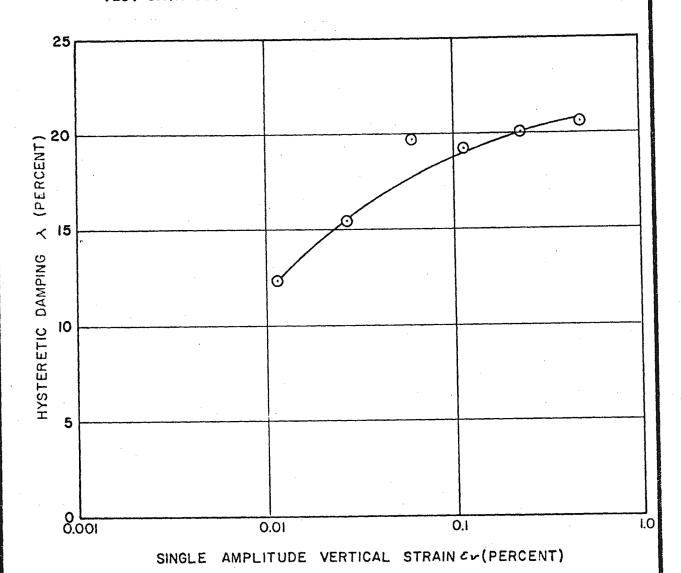


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 6 of 12)

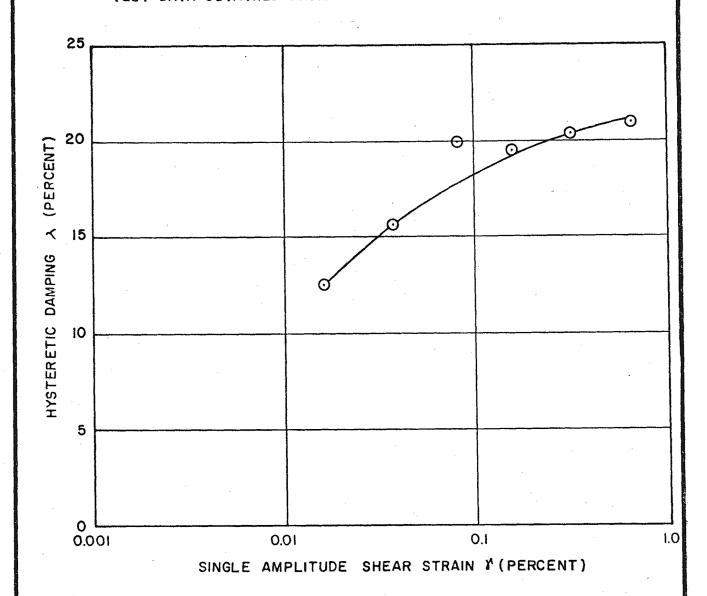


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 7 of 12)

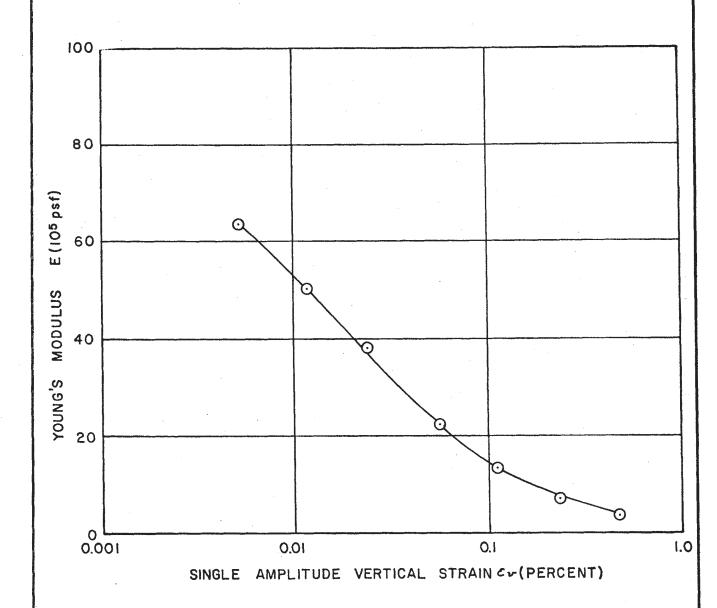


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 8 of 12)

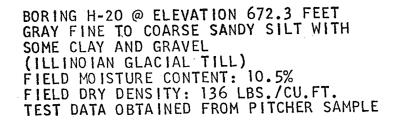


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 9 of 12)



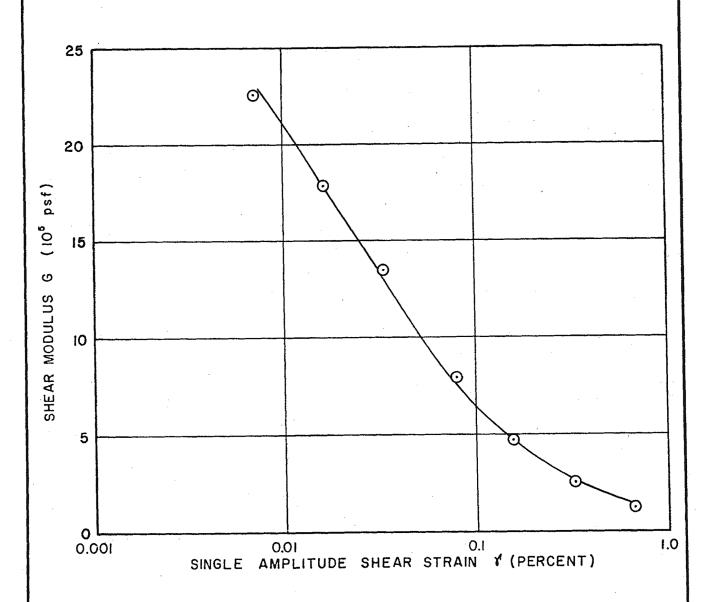
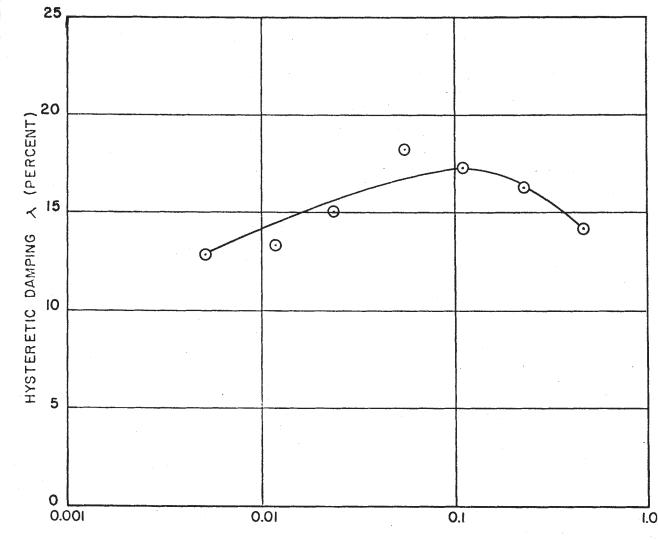


FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 10 of 12)



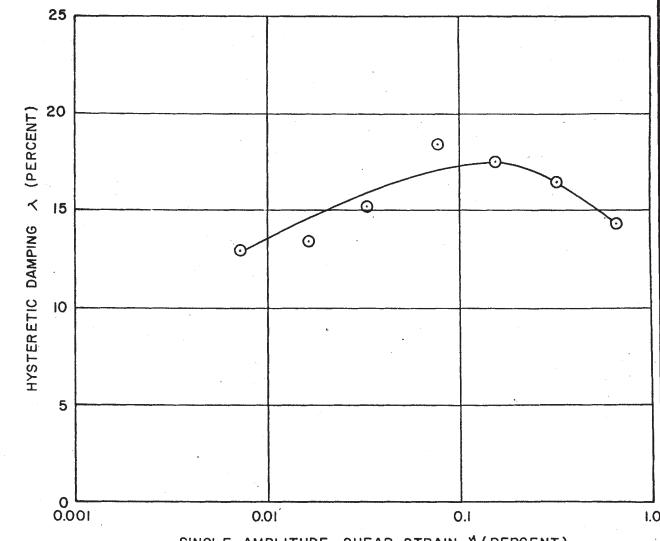
SINGLE AMPLITUDE VERTICAL STRAIN Cr (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 11 of 12)



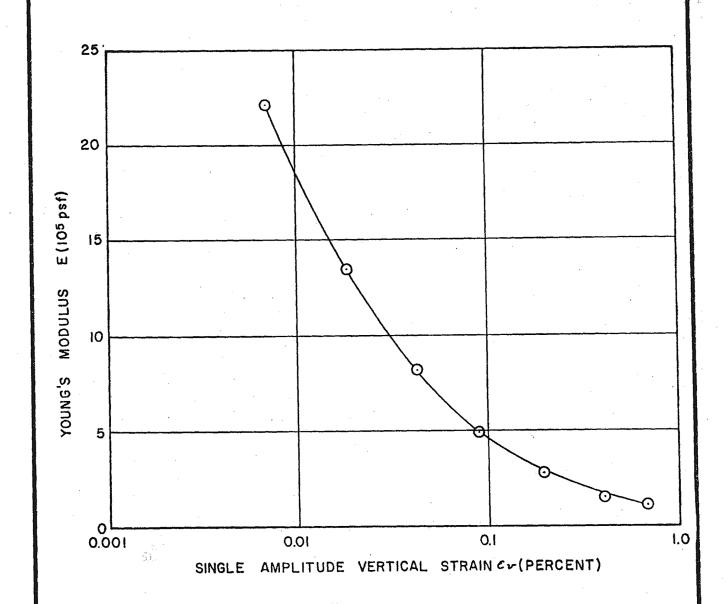
SINGLE AMPLITUDE SHEAR STRAIN & (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-314

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-20)

(SHEET 12 of 12)

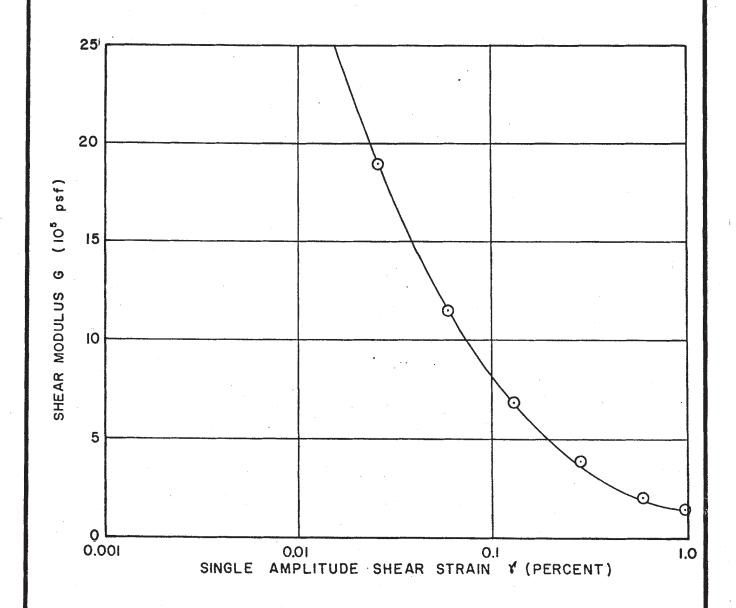


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-315

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-23)

(SHEET 1 of 4)

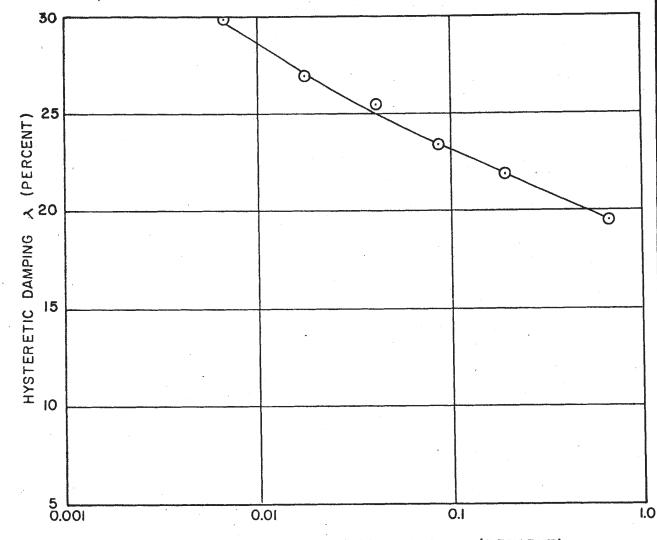


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-315

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-23)

(SHEET 2 of 4)



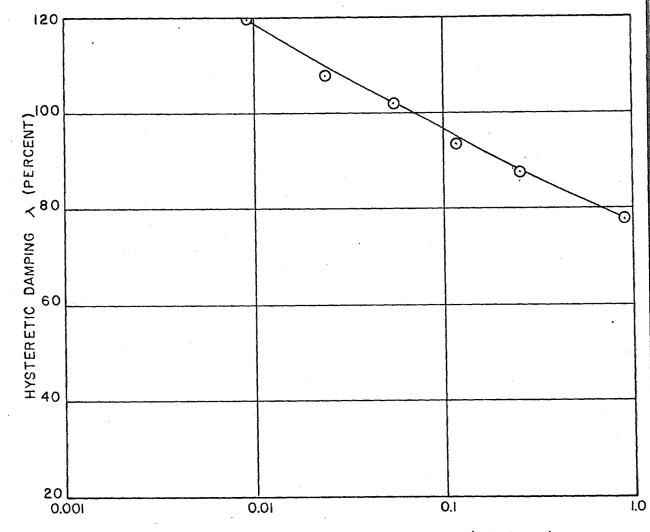
SINGLE AMPLITUDE VERTICAL STRAIN & (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-315

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-23)

(SHEET 3 of 4)



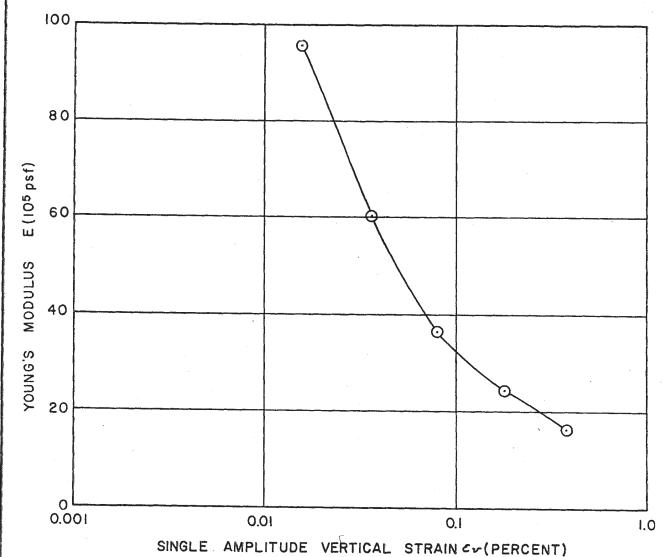
SINGLE AMPLITUDE VERTICAL STRAIN & (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-315

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-23)

(SHEET 4 of 4)



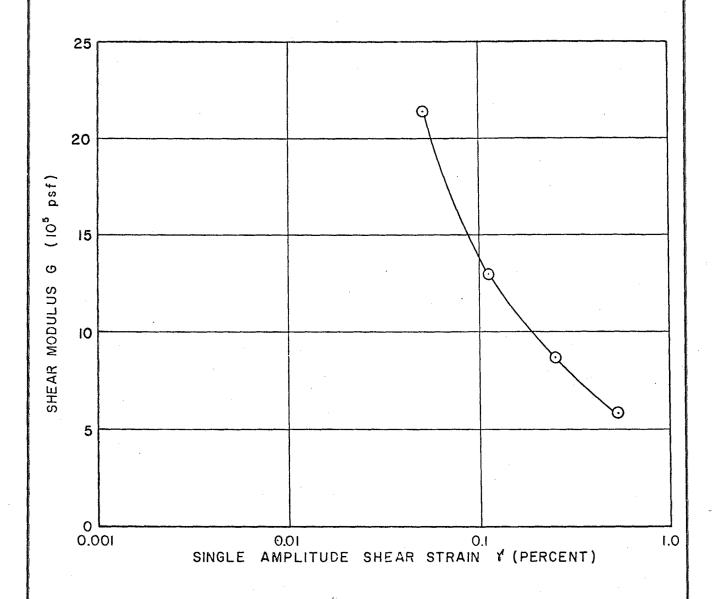
MINDEL VERTICAL STRAIR CO (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-316

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-30)

(SHEET 1 of 4)

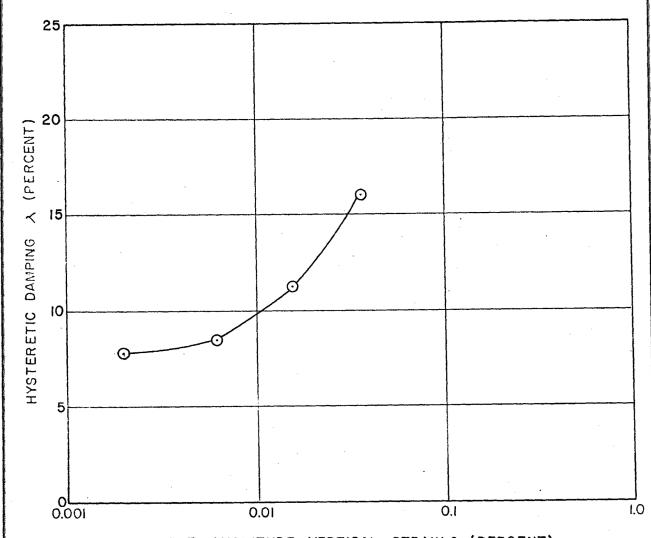


## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-316

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-30)

(SHEET 2 of 4)



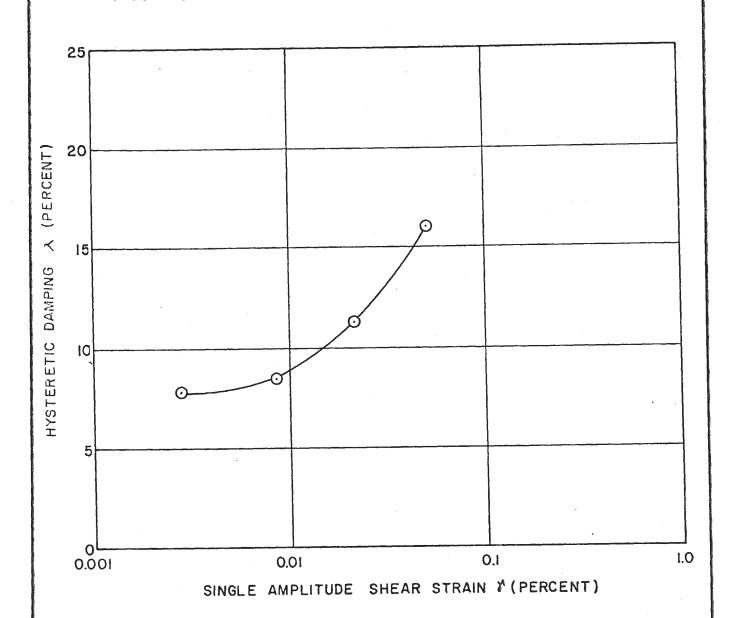
SINGLE AMPLITUDE VERTICAL STRAIN (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-316

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-30)

(SHEET 3 of 4)

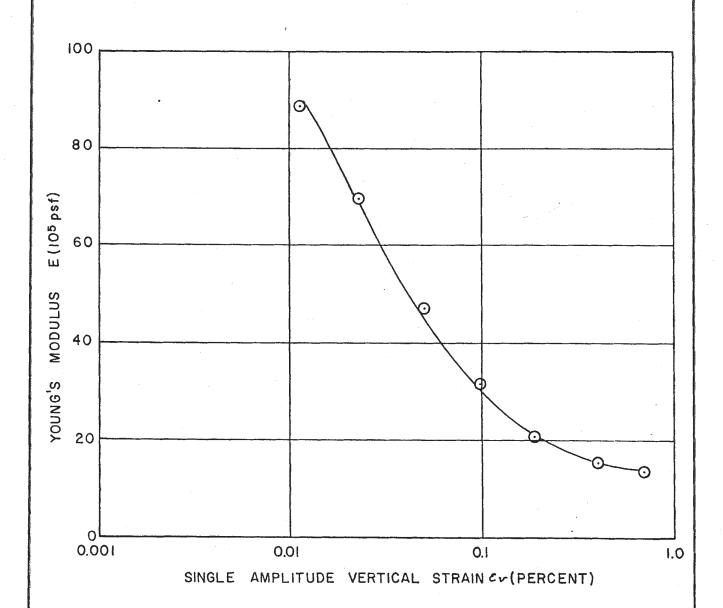


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-316

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-30)

(SHEET 4 of 4)

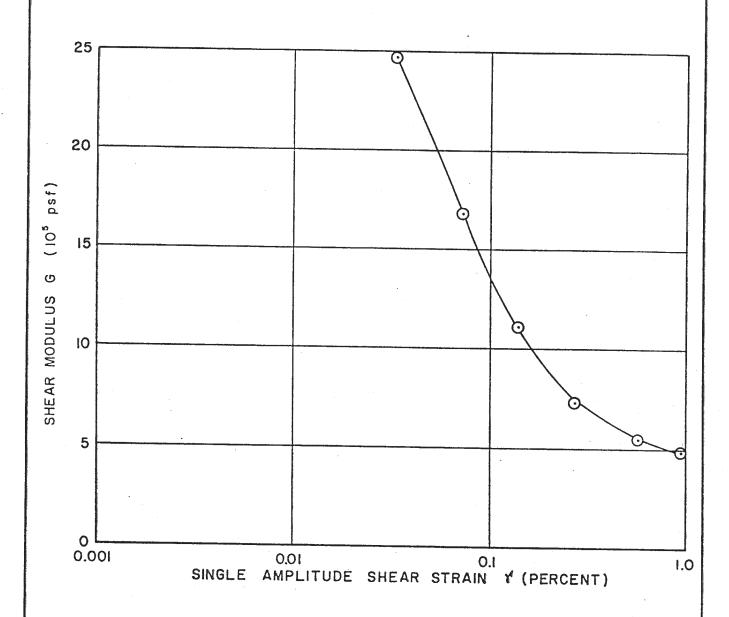


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-317

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-36)

(SHEET 1 of 4)

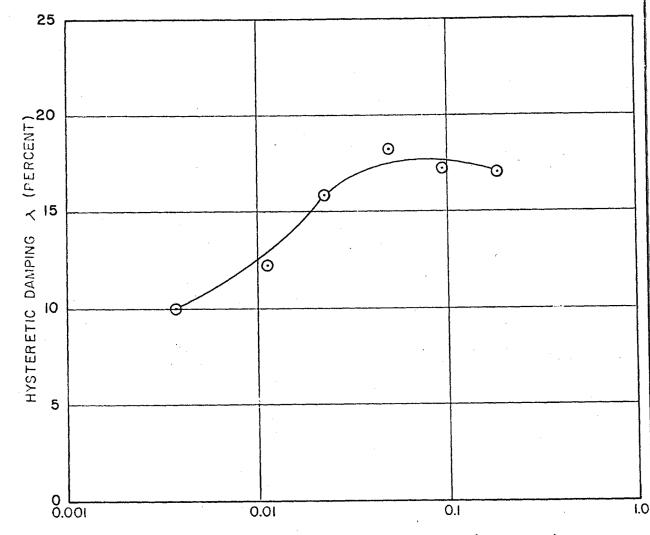


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-317

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-36)

(SHEET 2 of 4)



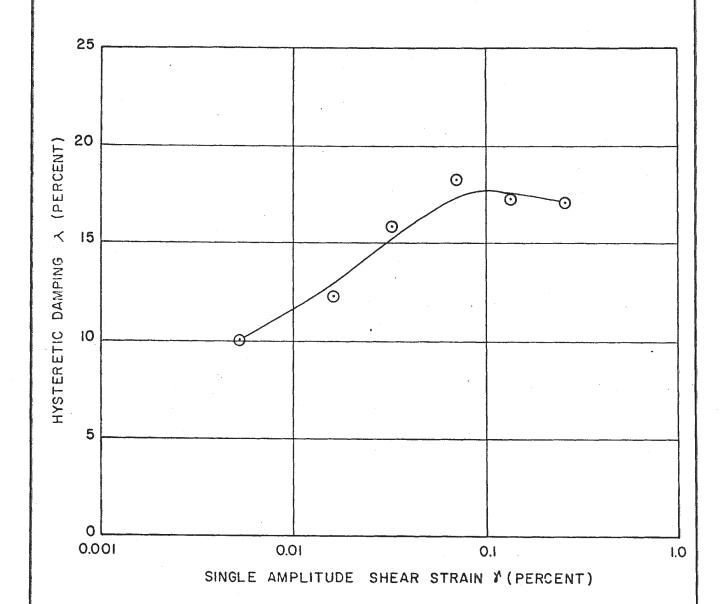
SINGLE AMPLITUDE VERTICAL STRAIN & (PERCENT)

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-317

DYNAMIC TRIAXIAL COMPRESSION TESTS (BORING H-36)

(SHEET 3 of 4)



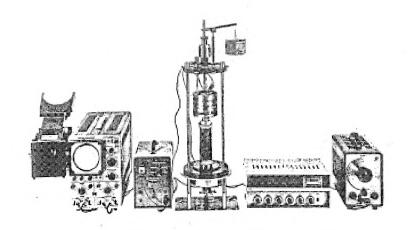
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-317

DYNAMIC TRIAXIAL COMPRESSION TESTS
(BORING H-36)

(SHEET 4 of 4)

## METHOD OF PERFORMING RESONANT COLUMN TESTS



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure 2.5-318 Sheet 1 of 2

RESONANT COLUMN TESTS (METHOD)

## Method of Performing Resonant Column Tests

Resonant column tests are performed to determine the dynamic properties of soils under high frequency, small amplitude cyclic strains. The test is based on the fact that analytical solutions can relate the stiffness of the soil column to its resonant frequency. In the test the sample is excited by an oscillating device and the frequency is varied until the maximum response, or resonant frequency, is found.

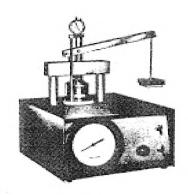
The Dames and Moore resonant column apparatus subjects solid cylindrical samples to torsional oscillations. The sample base is fixed and the top of the sample is excited by a Hardin oscillator which is driven by a variable frequency sine wave generator. The response of the sample is measured by an accelerometer mounted in the oscillator and the output is displayed on an oscilloscope.

The equivalent linear shear modulus of the soil is obtained from the resonant frequency of the system after the manner suggested by Drnevich and Hardin ("Proposed Standard for Modulus and Damping of Soils by the Resonant Column Method", ASTM Committee D18.09, May 1974). The shear modulus of soils varies with the shear strain amplitude and thus actually varies along the radius of the sample but in calculating the shear modulus the average shear strain is taken to correspond to the cyclic shear strain developed two-thirds of the distance along the radius. The damping ratio at small strains may be computed from measurements of the logarithmic decrement which are obtained by subjecting the sample to a steady state oscillation and then shutting off the input voltage. The decay curve is retained on a recording oscilloscope and may be photographed to make a permanent record.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure 2.5-318 Sheet 2 of 2

RESONANT COLUMN TESTS (METHOD)



DEAD LOAD-PHEUMATIC CONSOLIDOMETER

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure 2.5-319 Sheet 1 of 2

CONSOLIDATION TESTS (METHOD)

## Method of Performing Consolidation Tests

Consolidation tests are performed to evaluate the volume changes of soils subjected to increased loads. Time-consolidation and pressure-consolidation curves may be plotted from the data obtained in the tests. Engineering analyses based on these curves permit estimates to be made of the probable magnitude and rate of settlement of the tested soils under applied loads.

Each sample is tested within brass rings two and one-half inches in diameter and one inch is length. Undisturbed samples of in-place soils are tested in rings taken from the sampling device in which the samples were obtained. Loose samples of soils to be used in constructing earth fills are compacted in rings to predetermined conditions and tested.

In testing, the sample is rigidly confined laterally by the brass ring. Axial loads are transmitted to the ends of the sample by porous disks. The disks allow drainage of the loaded sample. The axial compression or expansion of the sample is measured by a micrometer dial indicator at appropriate time intervals after each load increment is applied. Each load is ordinarily twice the preceding load. The increments are selected to obtain consolidation data representing the field loading conditions for which the test is being performed. Each load increment is allowed to act over an interval of time dependent on the type and extent of the soil in the field.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure 2.5-319 Sheet 2 of 2

CONSOLIDATION TESTS (METHOD)

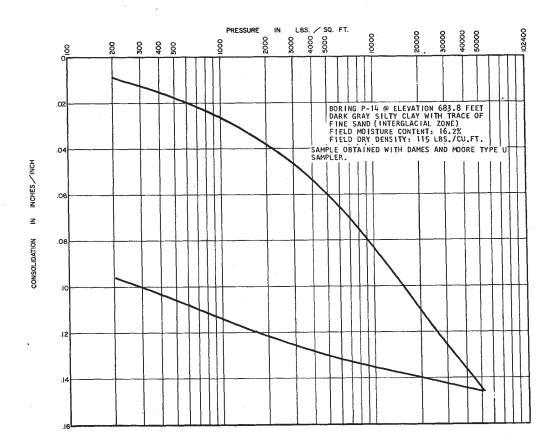
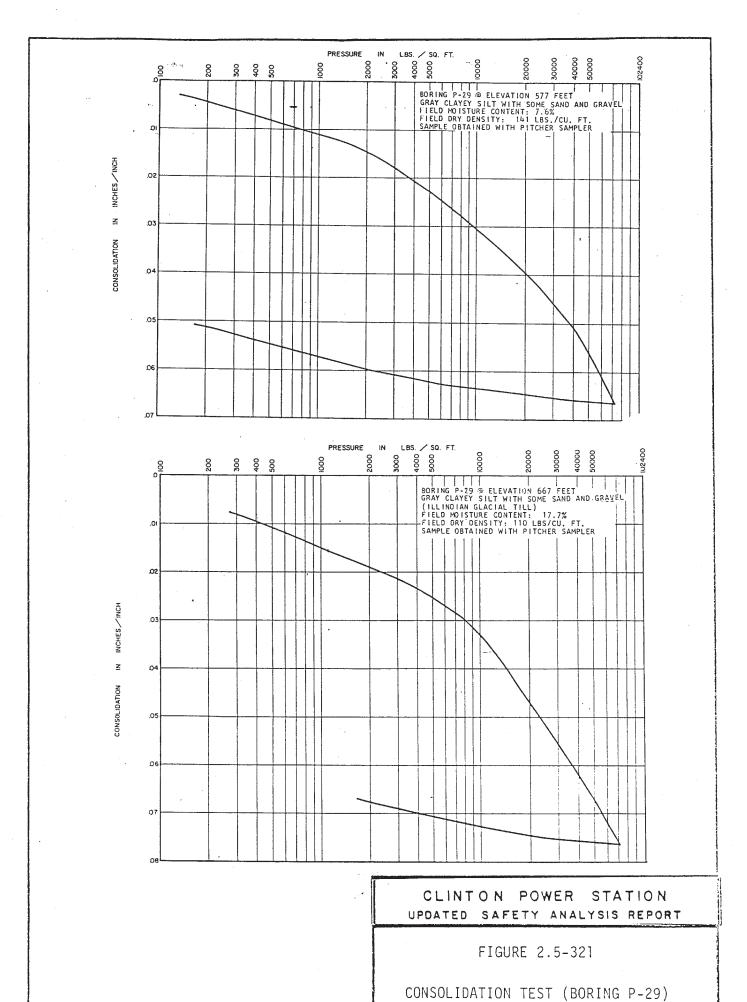


FIGURE 2.5-320

CONSOLIDATION TEST (BORING P-14)



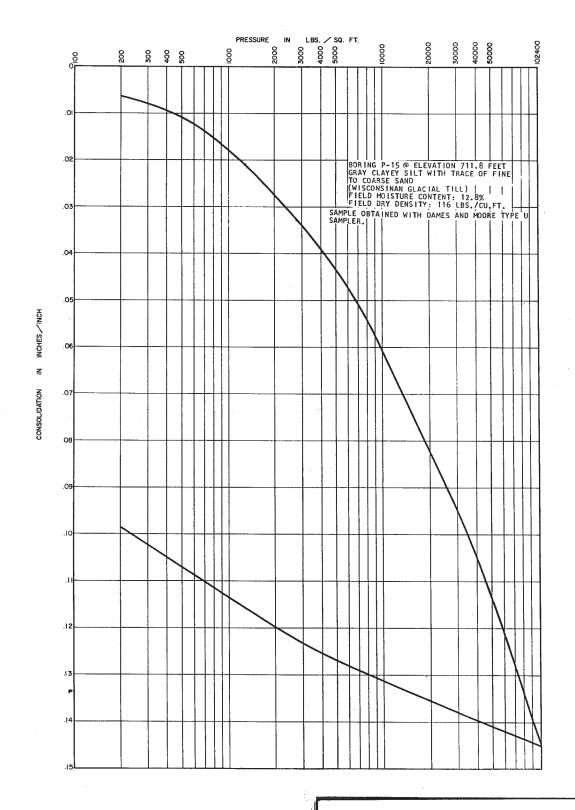


FIGURE 2.5-322

CONSOLIDATION TEST (BORING P-15)

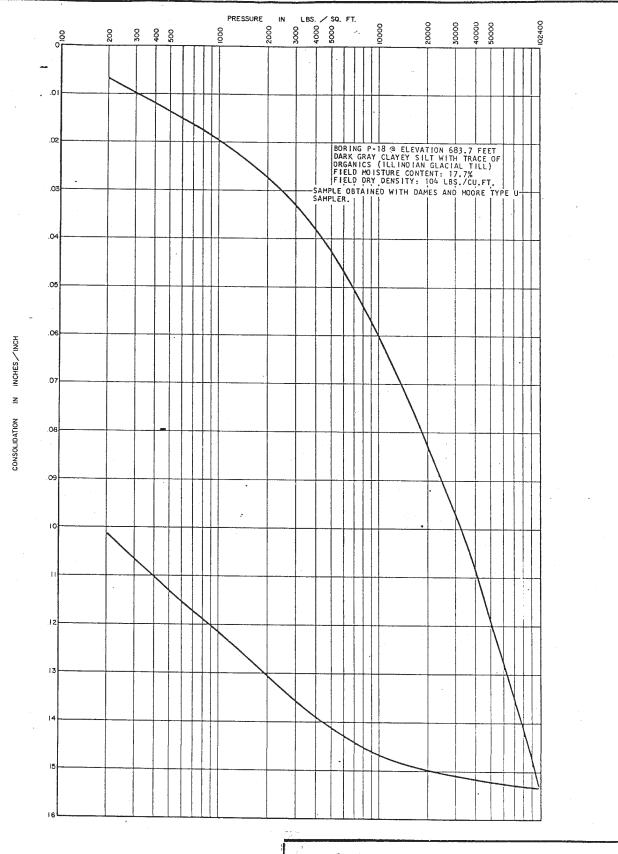


FIGURE 2.5-323

CONSOLIDATION TEST (BORING P-18)

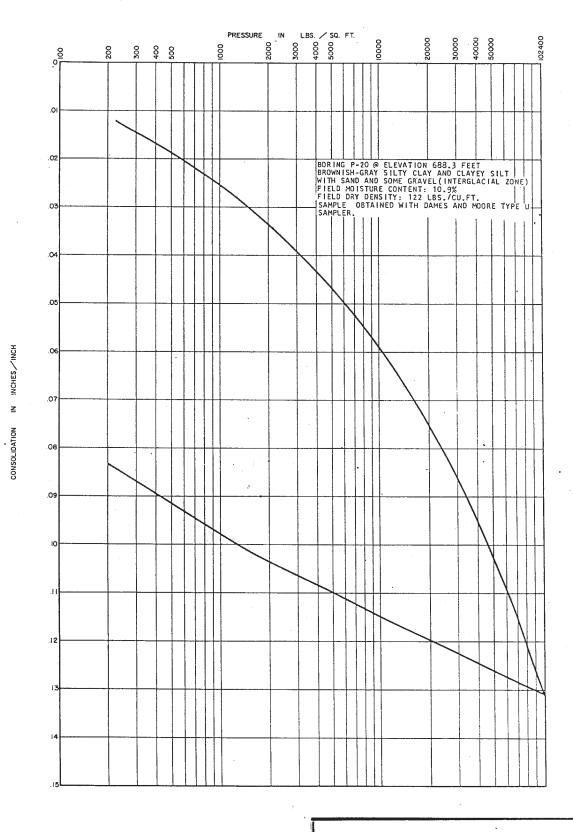


FIGURE 2.5-324

CONSOLIDATION TEST (BORING P-20)

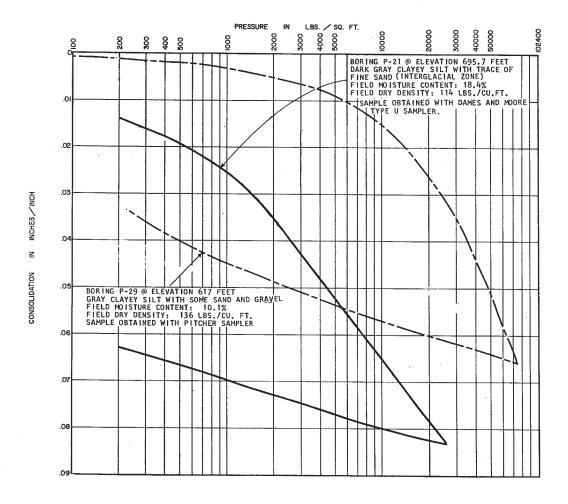


FIGURE 2.5-325

CONSOLIDATION TEST (BORING P-21)

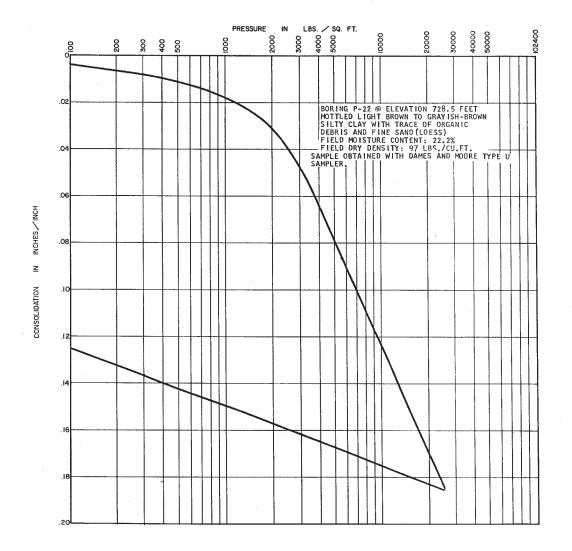
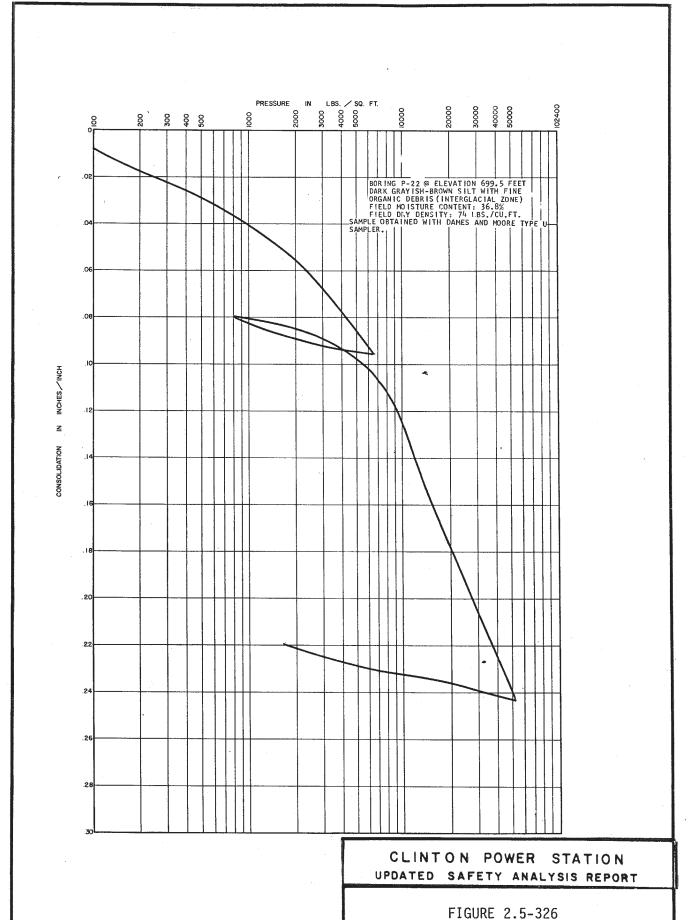


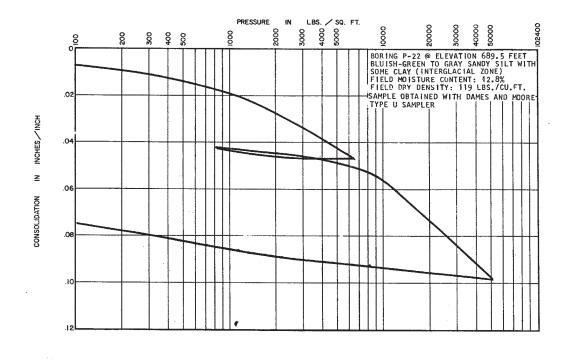
FIGURE 2.5-326

CONSOLIDATION TEST (BORING P-22)

(SHEET 1 of 2)



CONSOLIDATION TEST (BORING P-22)
(SHEET 2 of 2)



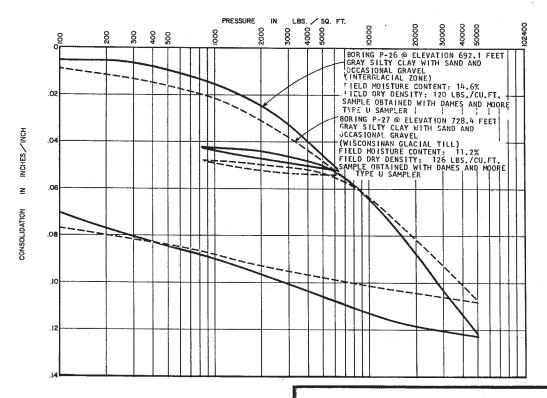
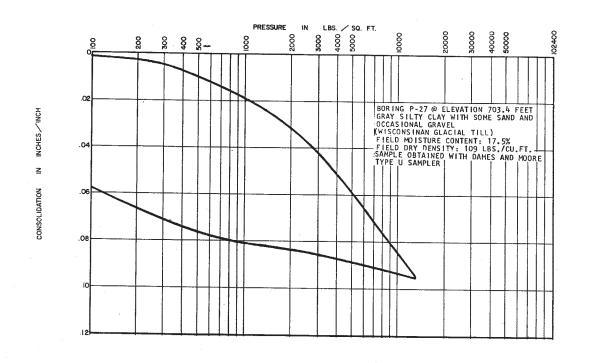


FIGURE 2.5-327

CONSOLIDATION TEST (BORINGS P-22, P-26 AND P-27)



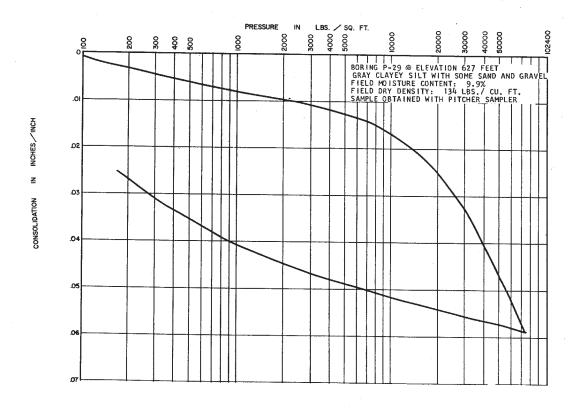


FIGURE 2.5-328

CONSOLIDATION TEST (BORINGS P-27 AND P-29)

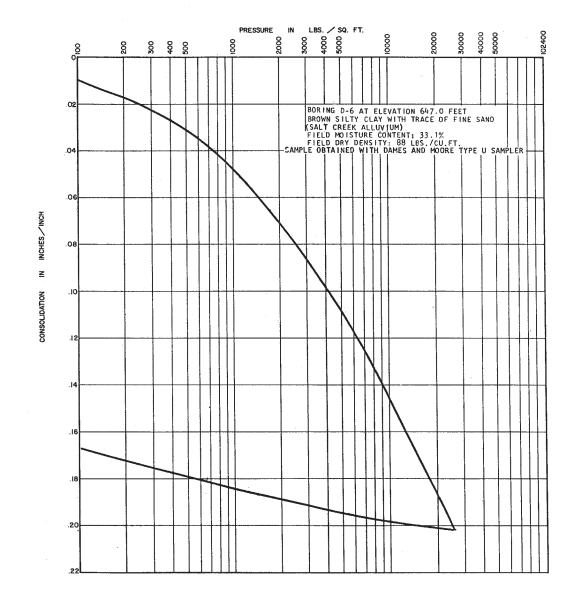


FIGURE 2.5-329

CONSOLIDATION TEST (BORING D-6)

(SHEET 1 of 2)

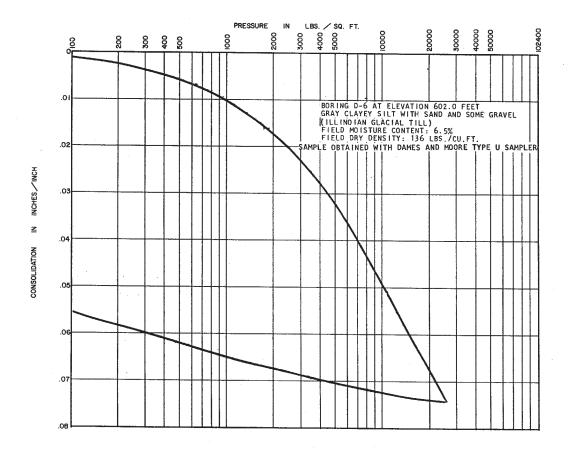


FIGURE 2.5-329

CONSOLIDATION TEST (BORING D-6)
(SHEET 2 of 2)

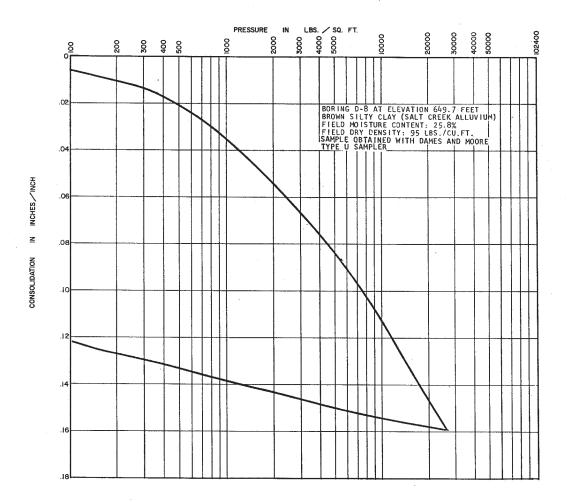


FIGURE 2.5-330

CONSOLIDATION TEST (BORING D-8)

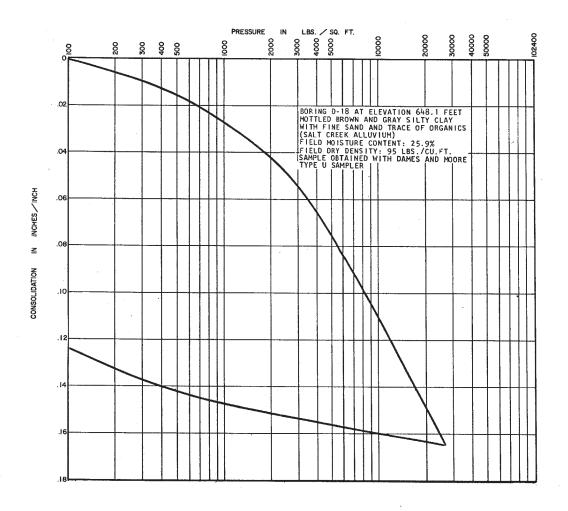


FIGURE 2.5-331

CONSOLIDATION TEST (BORING D-18)
(SHEET 1 of 2)

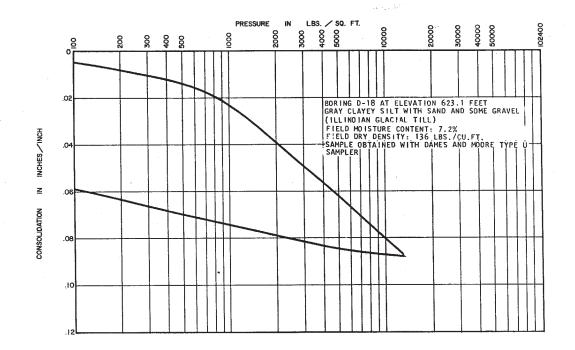


FIGURE 2.5-331

CONSOLIDATION TEST (BORING D-18)

(SHEET 2 of 2)

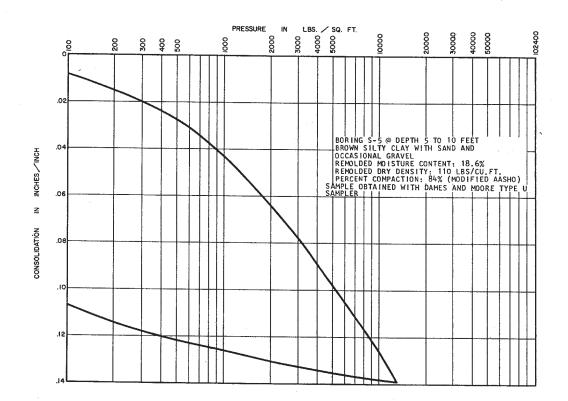


FIGURE 2.5-332

CONSOLIDATION TEST (BORING S-5)
(SHEET 1 of 2)

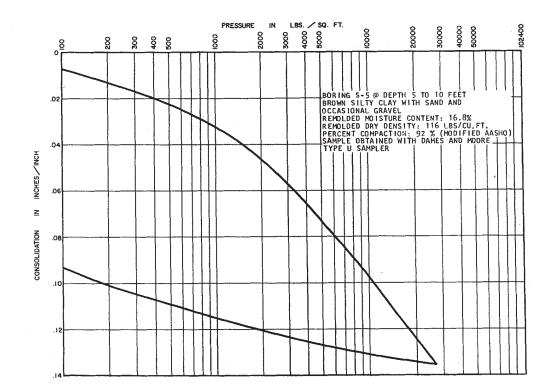


FIGURE 2.5-332

 ${\tt CONSOLIDATION\ TEST\ (BORING\ S-5)}$ 

(SHEET 2 of 2)

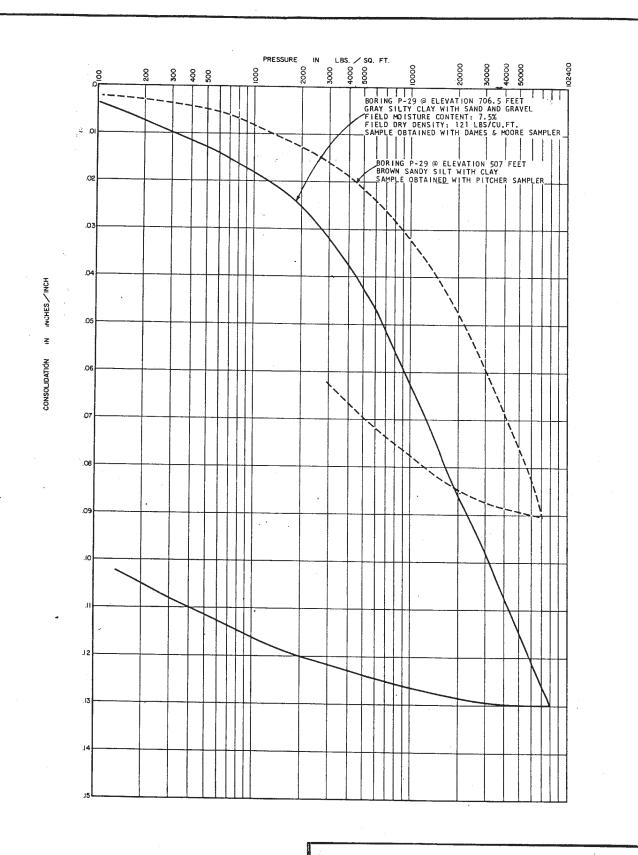


FIGURE 2.5-333

CONSOLIDATION TEST (BORING P-29)
(SHEET 1 of 3)

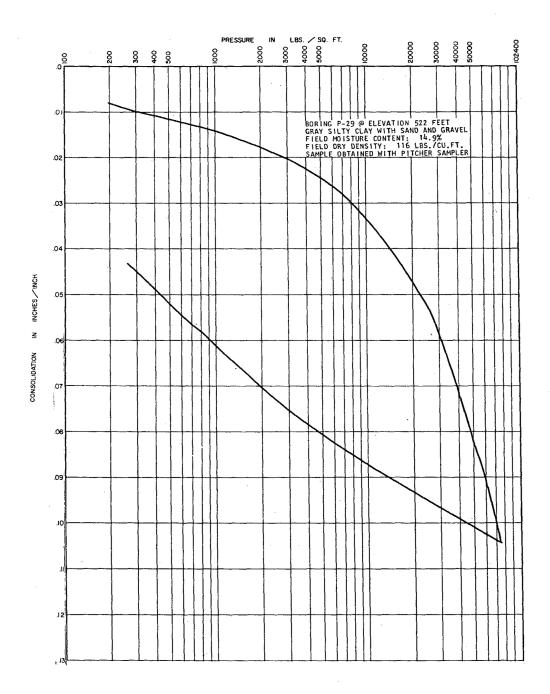


FIGURE 2.5-333

CONSOLIDATION TEST (BORING P-29)

(SHEET 2 of 3)

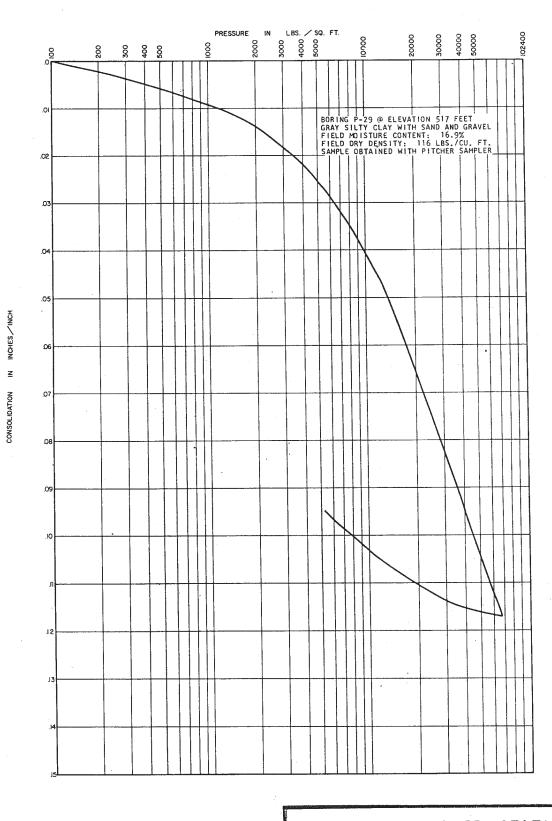
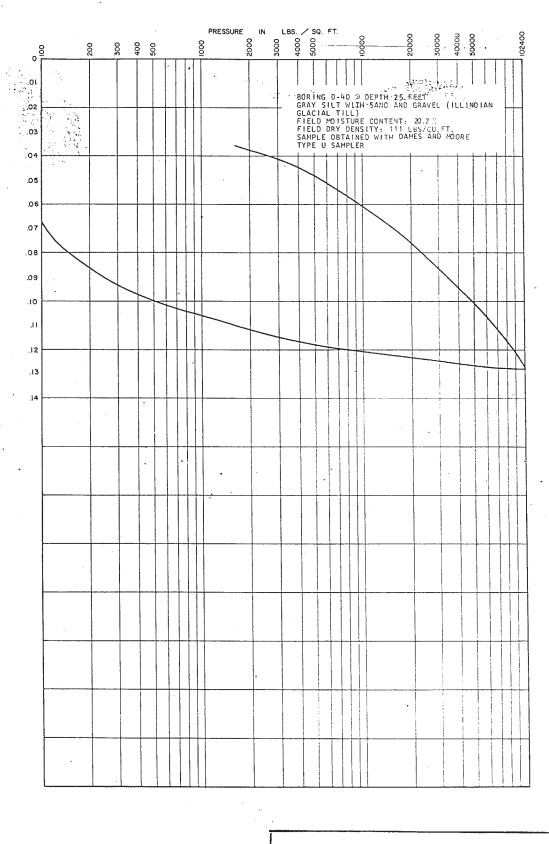


FIGURE 2.5-333

CONSOLIDATION TEST (BORING P-29)

(SHEET 3 of 3)



Z

CONSOLIDATION

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-334

CONSOLIDATION TEST (BORING D-40)

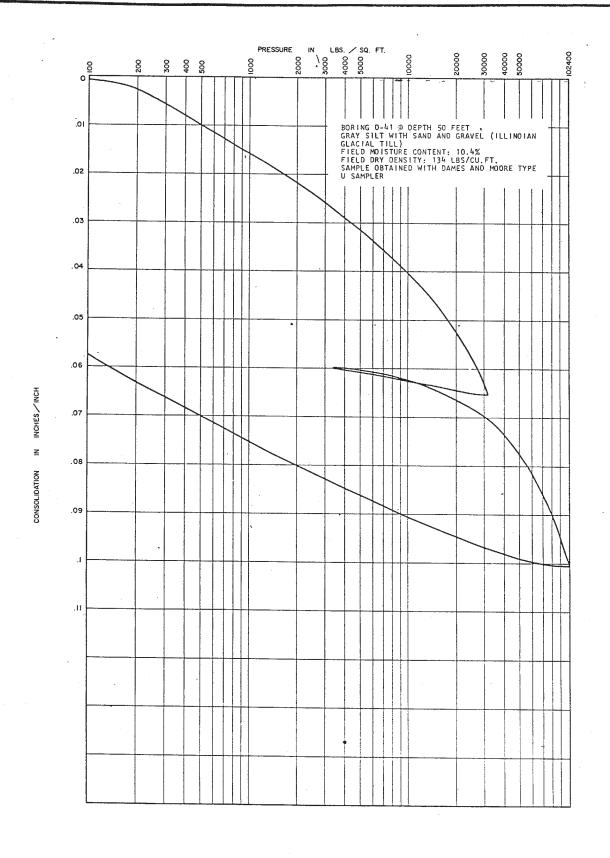
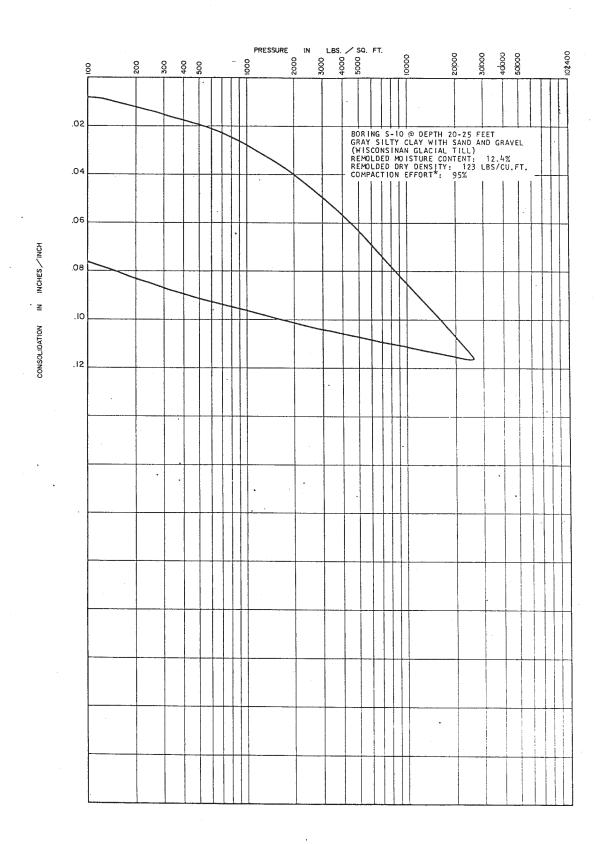


FIGURE 2.5-335

CONSOLIDATION TEST (BORING D-41)

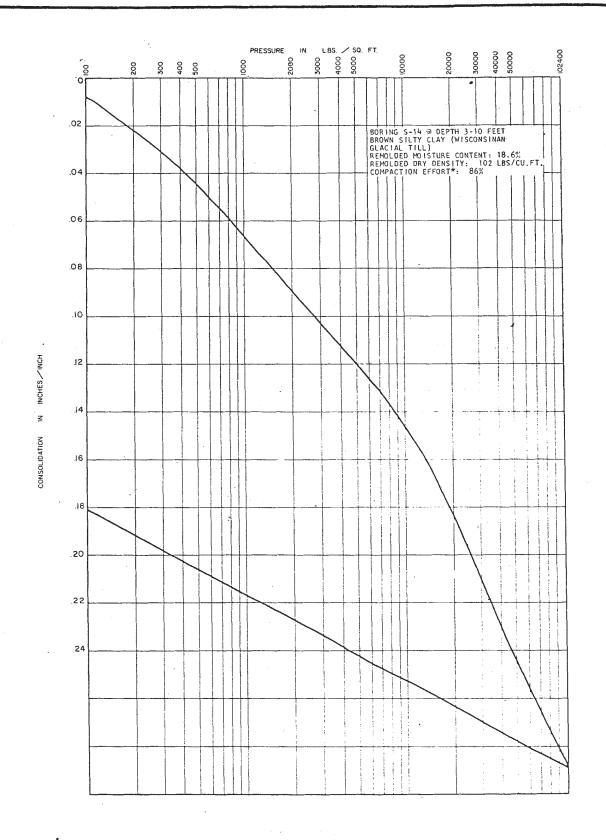


* A.A.S.H.O. TEST DESIGNATION T-180

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-336

CONSOLIDATION TEST (BORING S-10)



* A.A.S.H.O. TEST DESIGNATION T-180

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-337

CONSOLIDATION TEST (BORING S-14)

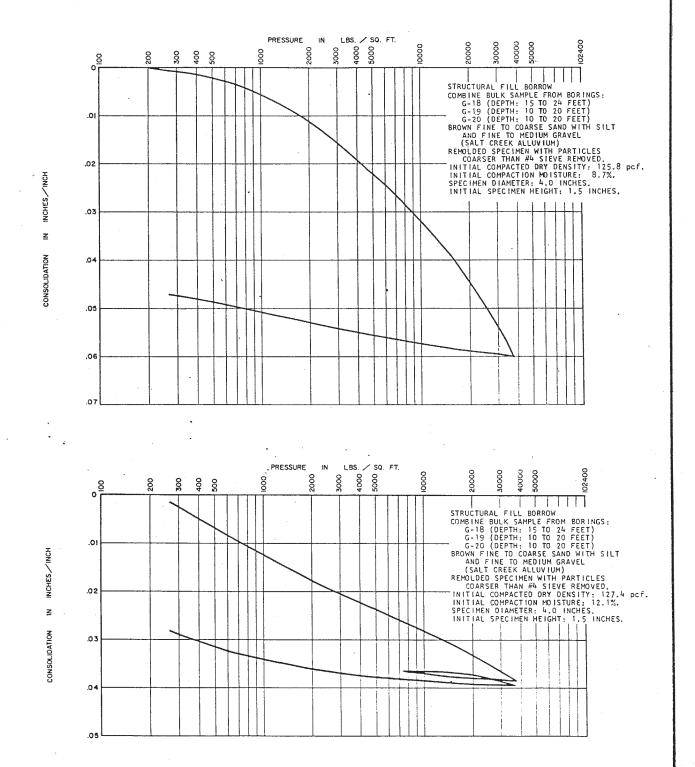
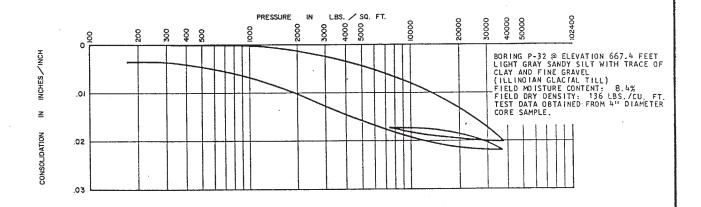


FIGURE 2.5-338

CONSOLIDATION TEST (BORINGS G-18, G-19 AND G-20)



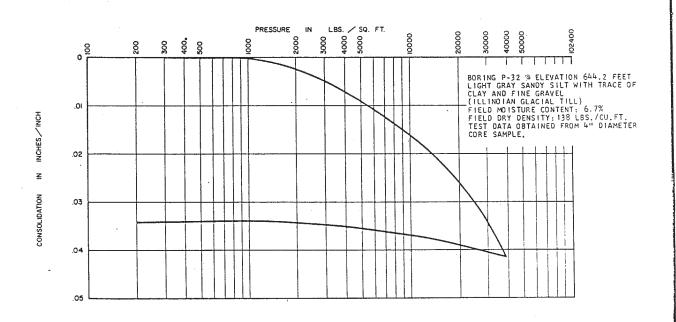
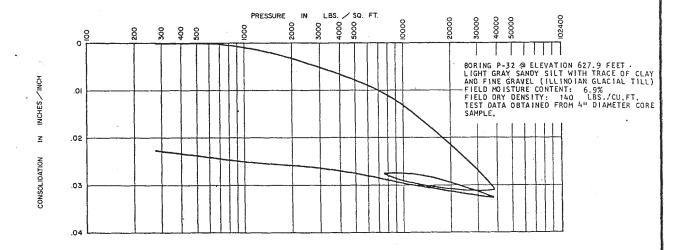


FIGURE 2.5-339

CONSOLIDATION TEST (BORING P-32)
(SHEET 1 of 2)



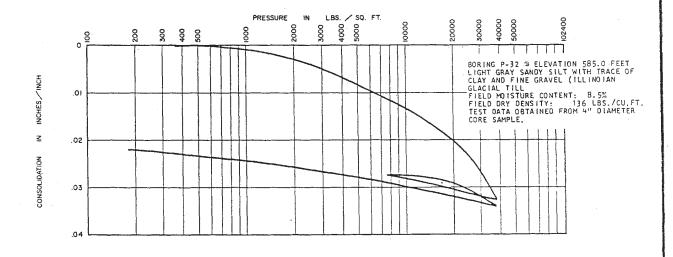


FIGURE 2.5-339

CONSOLIDATION TEST (BORING P-32)

(SHEET 2 of 2)

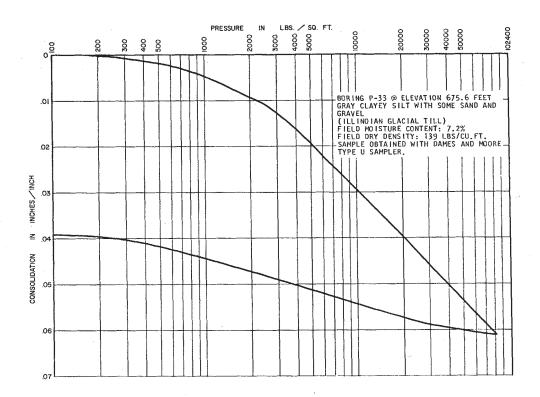
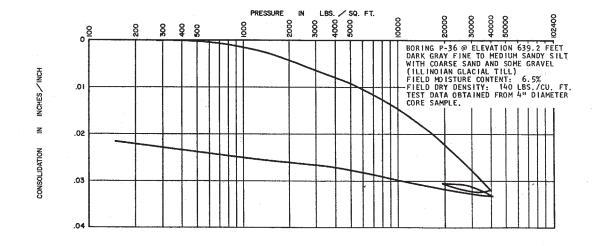


FIGURE 2.5-340

CONSOLIDATION TEST (BORING P-33)



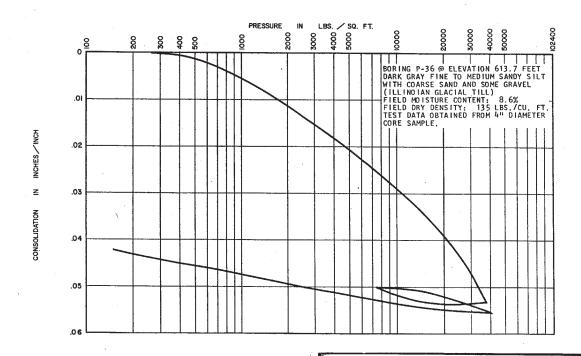


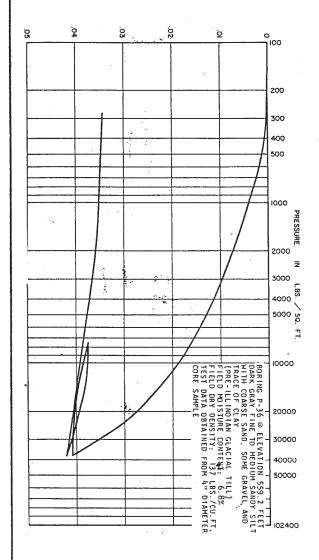
FIGURE 2.5-341

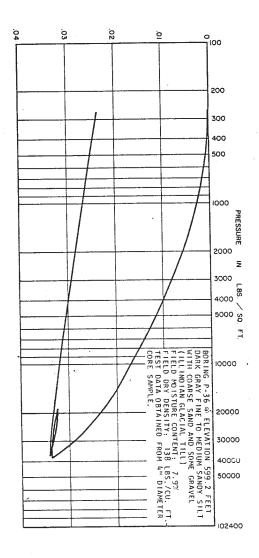
CONSOLIDATION TEST (BORING P-36)

(SHEET 1 of 3)

CONSOLIDATION IN INCHES / INCH

CONSOLIDATION IN INCHES / INCH





## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-341

CONSOLIDATION TEST (BORING P-36)

(SHEET 2 of 3)

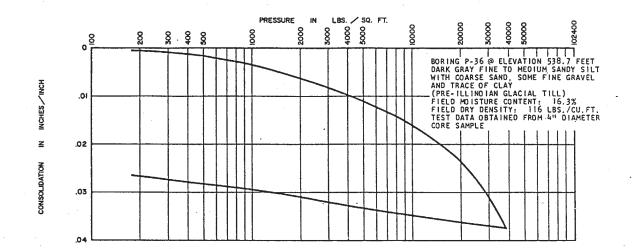
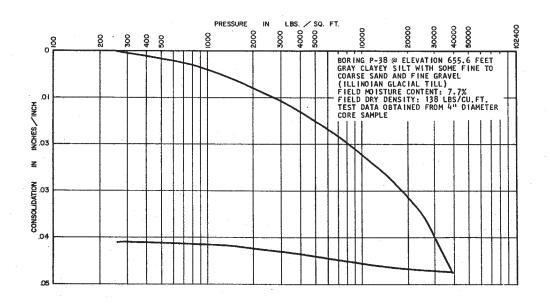


FIGURE 2.5-34]

CONSOLIDATION TEST (BORING P-36)

(SHEET 3 of 3)



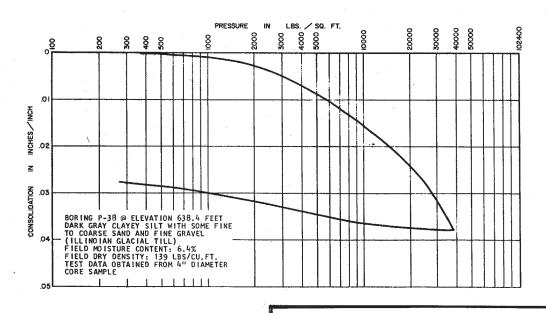
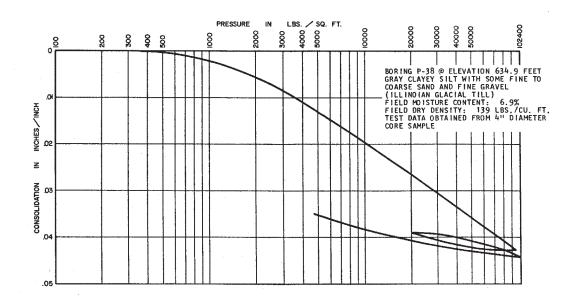


FIGURE 2.5-342

CONSOLIDATION TEST (BORING P-38)

(SHEET 1 of 5)



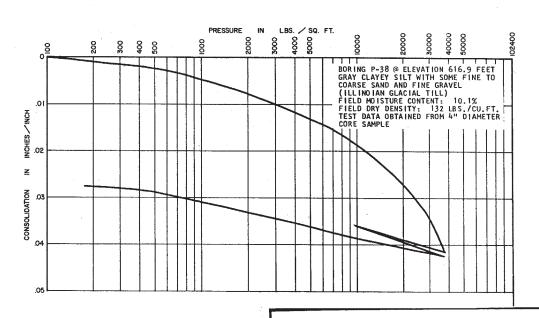
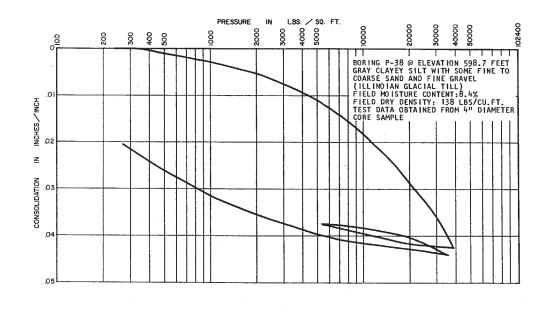


FIGURE 2.5-342

CONSOLIDATION TEST (BORING P-38)

(SHEET 2 of 5)



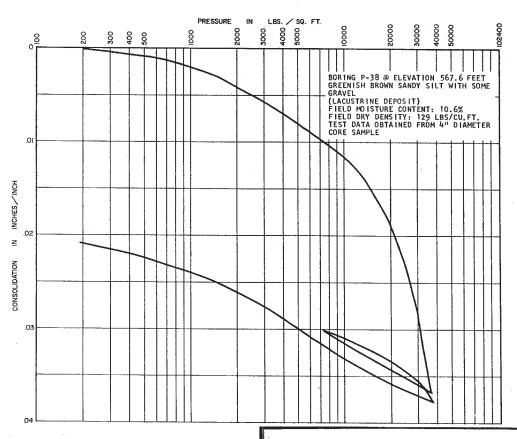


FIGURE 2.5-342

CONSOLIDATION TEST (BORING P-38)

(SHEET 3 of 5)

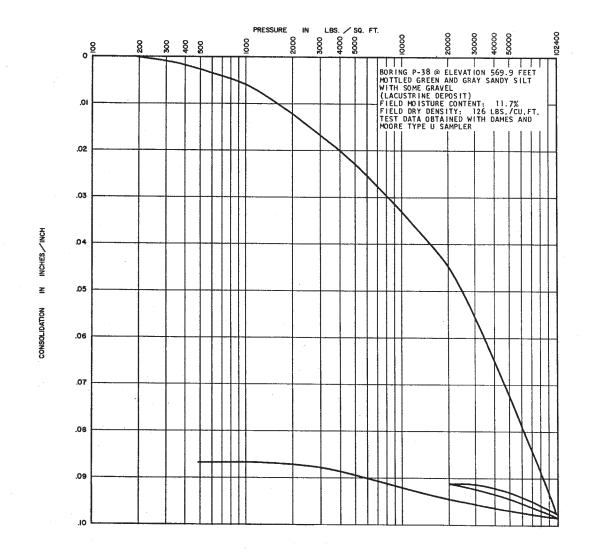


FIGURE 2.5-342

CONSOLIDATION TEST (BORING P-38)
(SHEET 4 of 5)

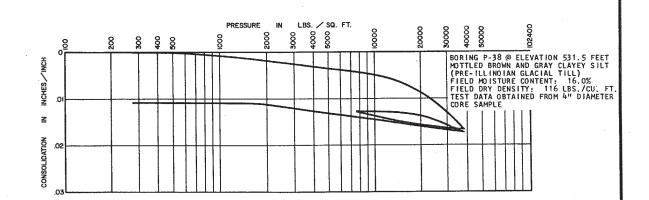


FIGURE 2.5-342

CONSOLIDATION TEST (BORING P-38)

(SHEET 5 of 5)

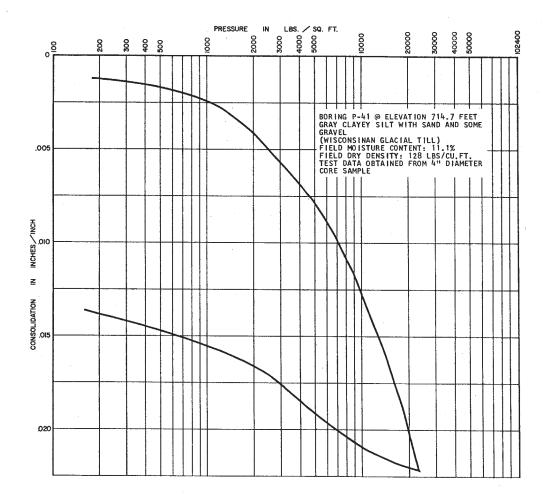


FIGURE 2.5-343

CONSOLIDATION TEST (BORING P-41)

(SHEET 1 of 2)

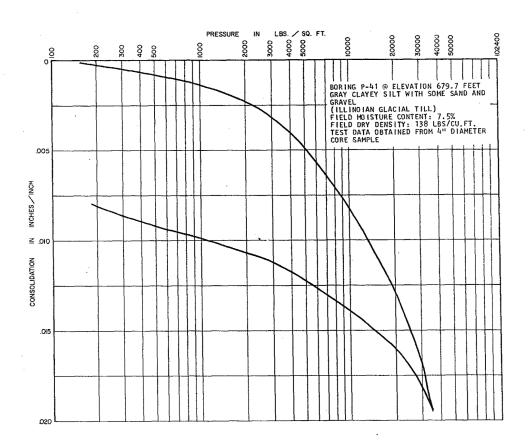
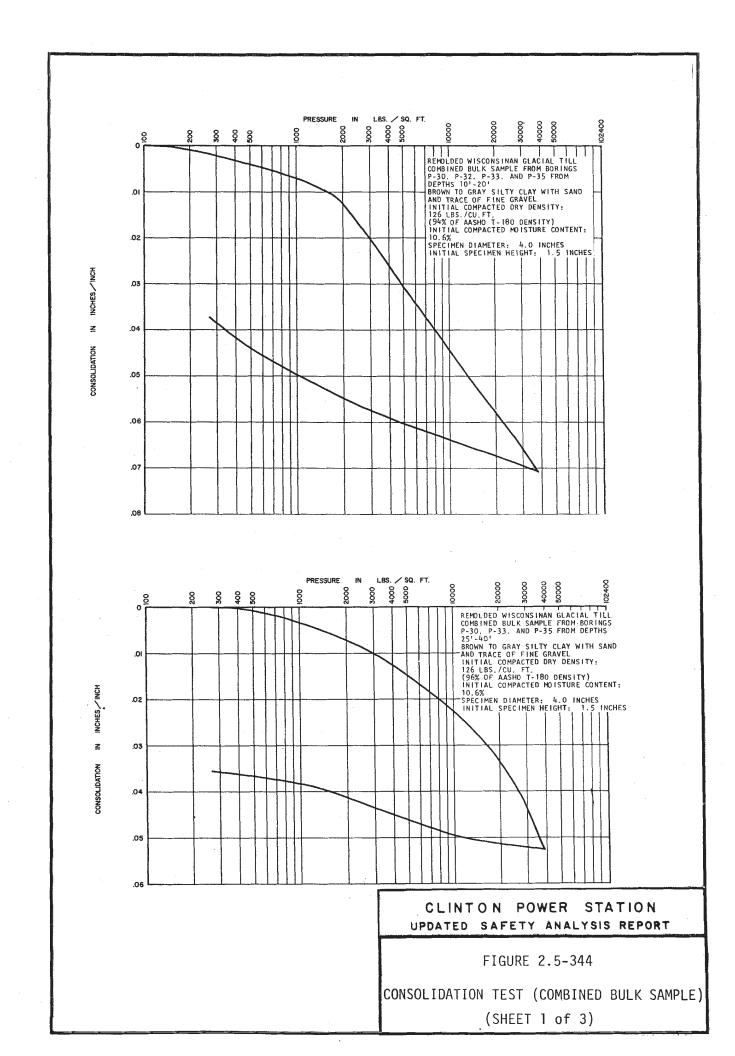


FIGURE 2.5-343

CONSOLIDATION TEST (BORING P-41)

(SHEET 2 of 2)



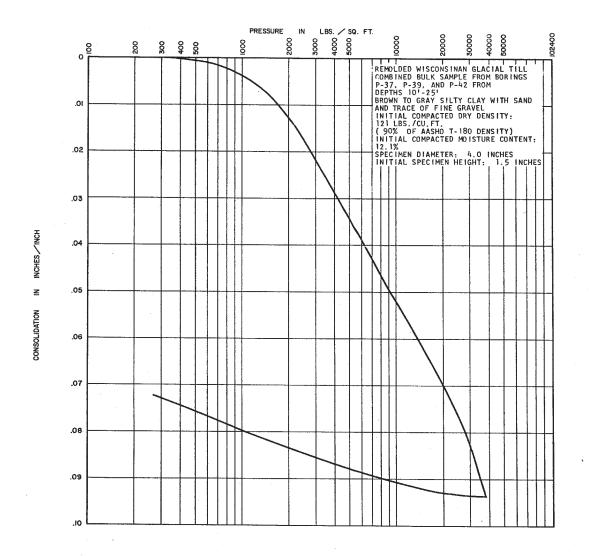


FIGURE 2.5-344

CONSOLIDATION TEST (COMBINED BULK SAMPLE)
(SHEET 2 of 3)

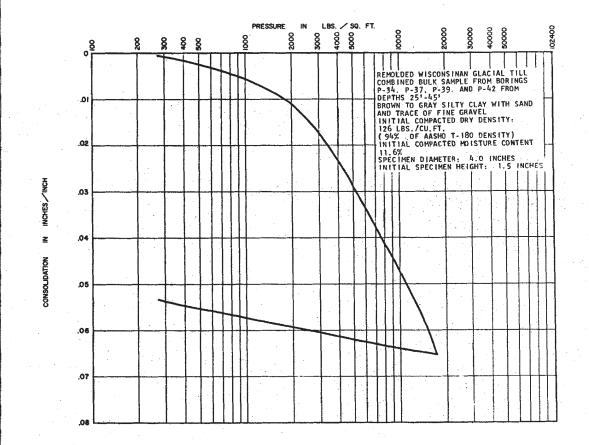


FIGURE 2.5-344

CONSOLIDATION TEST (COMBINED BULK SAMPLE)
(SHEET 3 of 3)

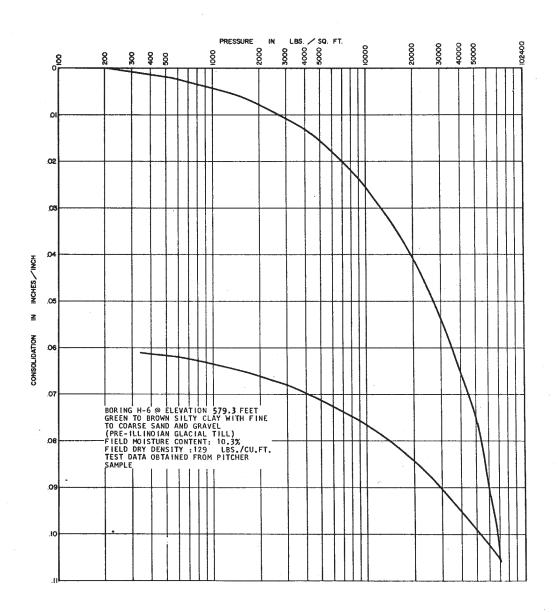


FIGURE 2.5-345

CONSOLIDATION TEST (BORING H-6).

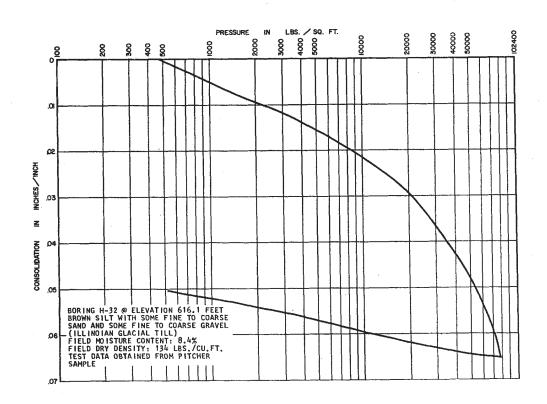


FIGURE 2.5-346

CONSOLIDATION TEST (BORING H-32)

(SHEET 1 of 2)

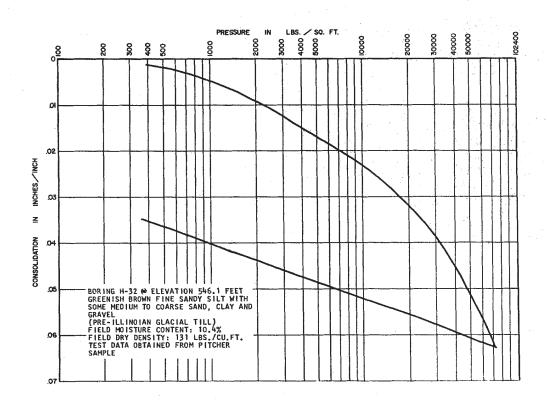


FIGURE 2.5-346

CONSOLIDATION TEST (BORING H-32)

(SHEET 2 of 2)

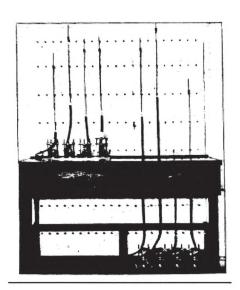


Figure 2.5-347 Sheet 1 of 2

PERCOLATION TEST (METHOD)

#### **NOTES FOR FIGURE 2.5-347**

### Methods of Performing Percolation Tests

The quantity and the velocity of flow of water which will escape through an earth structure or percolate through soil are dependent upon the permeability of the earth structure or soil. The permeability of soil has often been calculated by empirical formulas but is best determined by laboratory tests, especially in the case of compacted soils.

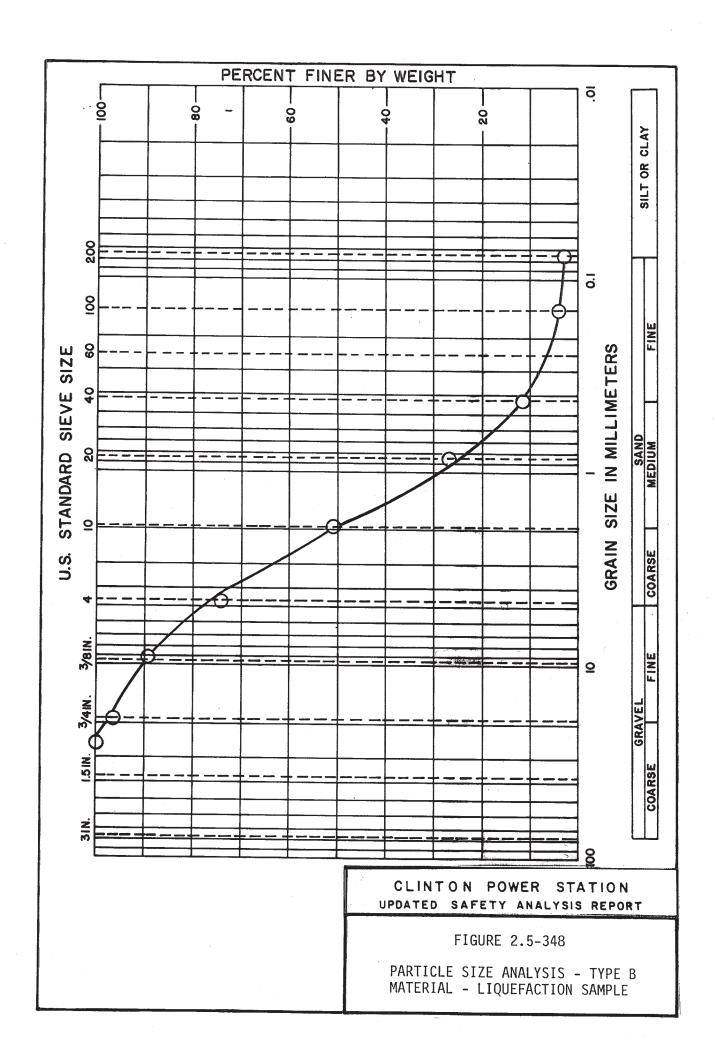
A one inch length of the core sample is sealed in the percolation apparatus, placed under a confining load, or surcharge pressure, and subjected to the pressure of a known head of water. The percolation rate is computed from the measurements of the volume of water which flows through the sample in a series of time intervals. These rates are usually expressed as the velocity of flow in feet per year under a hydraulic gradient of one and at a temperature of 20 degrees Centigrade. The rate so expressed may be adjusted for any set of conditions involving the same soil by employing established physical laws. Generally, the percolation rate varies over a wide range at the beginning of the test and gradually approaches equilibrium as the test progresses.

During the performance of the test, continuous readings of the deflection of the sample are taken by means of micrometer dial gauges. The amount of compression or expansion, expressed as a percentage of the original length of the sample, is a valuable indication of the compression of the soil which will occur under the action of load or the expansion of the soil as saturation takes place.

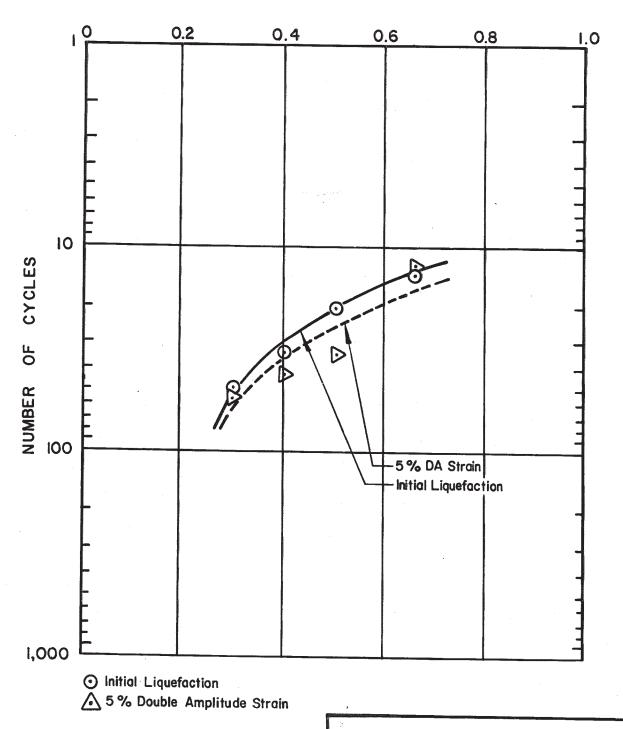
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Figure 2.5-347 Sheet 2 of 2

PERCOLATION TEST (METHOD)



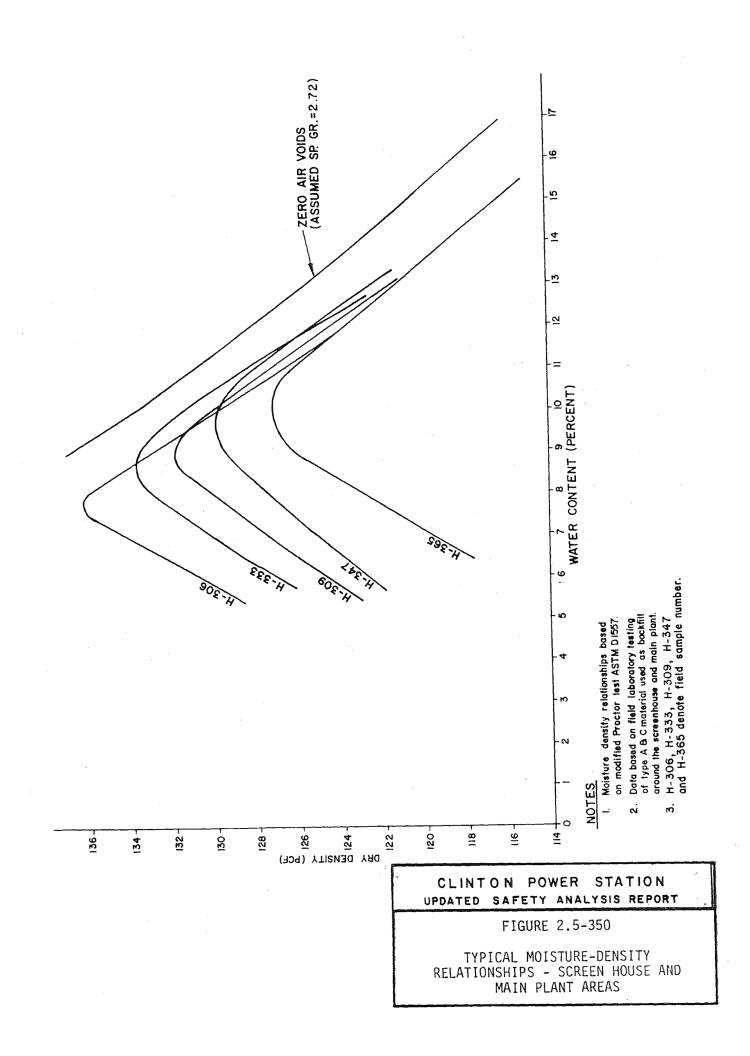
# STRESS RATIO, R = $\frac{\Delta \sigma_v}{2 \, \bar{\sigma}_c}$

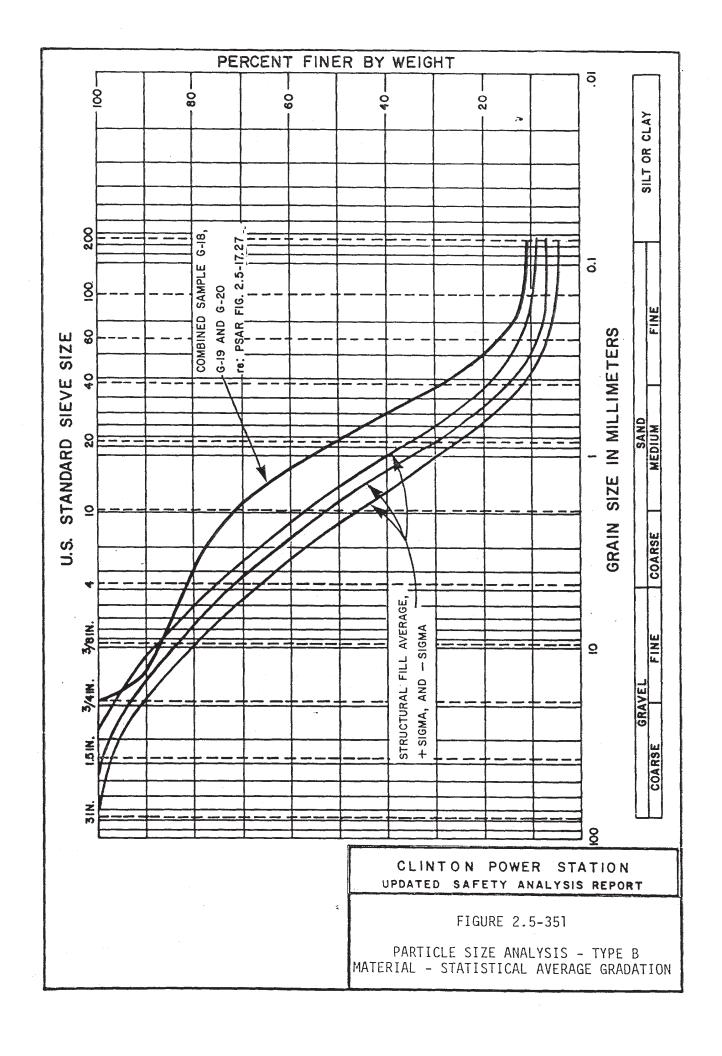


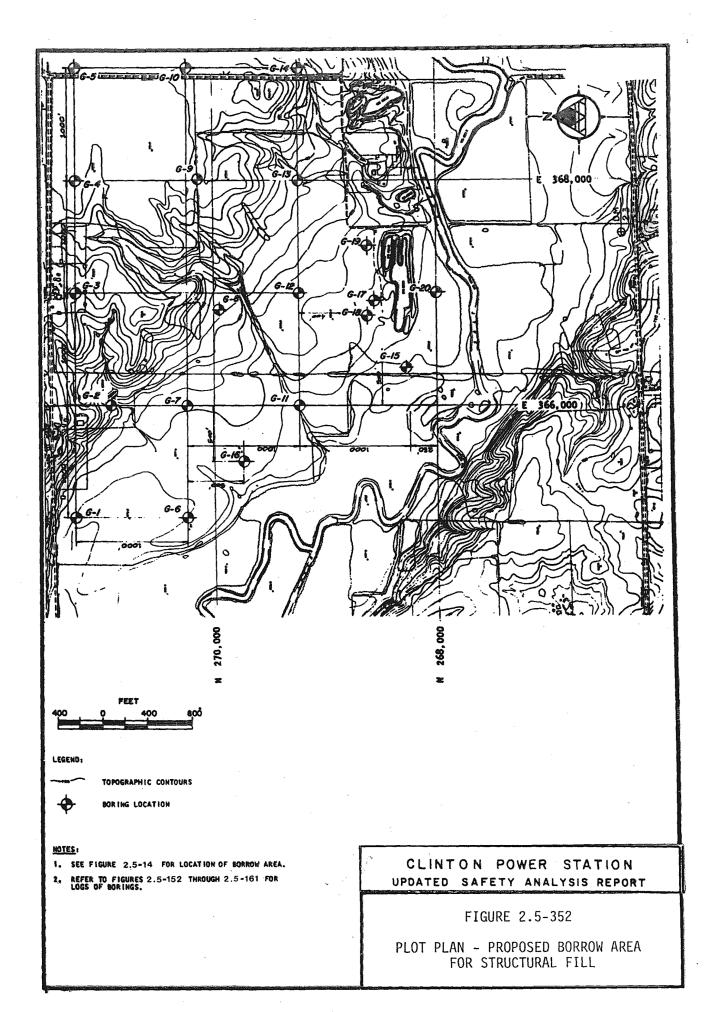
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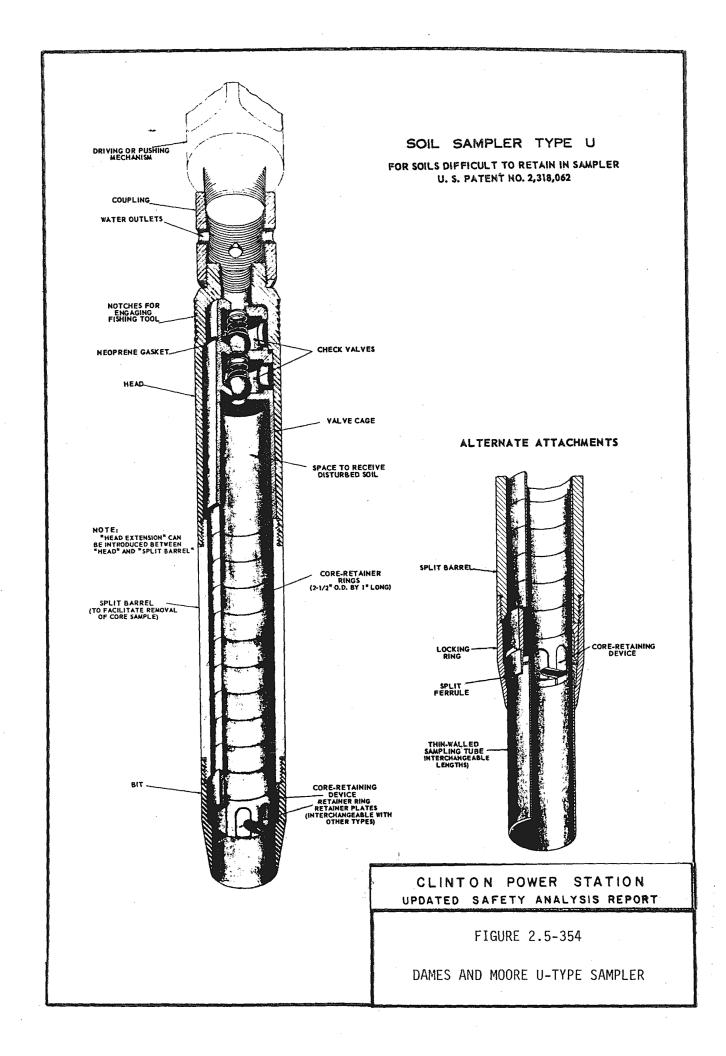
FIGURE 2.5-349

LIQUEFACTION TESTS OF TYPE B MATERIAL









SOIL	CLASSIFICATION	N CHART

	MAJOR DIVISIONS			WOOD	MARD-CLYDE CONSULTANTS			DAMES AND MOORE		EAR	SENT & LUNDY ENGINEERS	
	,		GRAPH SYMBOL	LETTER SYNBOL	TYPICAL DESCRIPTION	CRAPH SYMBOL	LETTER SYMBOL	TYPICAL DESCRIPTION	CRAPH SYMBOL	LETTER	TYPICAL DESCRIPTION	
	GRAVEL AND GRAVELLY SOILS	CLEAN GRAVELS (little or no fines)					GW	Well-graded gravels, gravel - sand wixtures, little or no fines.		GN	Well-graded gravels, gravel - sand mixtures, little or no fines.	
CDARSE CRAINED SOILS				SP-GP GP-SP GP	Gravelly sand, sandy gravel, poorly graded gravel.		GP	Poorly-graded gravels, gravel - sand mixtures, little or mo fines.		GP GP	Poorly-graded gravels, gravel - sand mixtures, little or no fines.	
	More than 50% of coarse frac- tion RETAINED	GRAVELS WITH FINES (appreciable amount of fines)		GN GP-GN	Sandy GRAVEL, with trace or some silt, silty GRAVEL.		СН	Silty gravels, gravel-sand- silt mixtures.		CH	Silty gravels, gravel-send- silt mixtures.	
	on No. 4 sieve						GC	Clayey gravels, gravel-sand- clay mixtures.		GC	Clayey gravels, gravel-sand- clay mixtures.	
	SAND AND SANDY SOILS	CLEAN SAND (little or no fines)		SP SP-SM SW	Sand with trace silt,		SW	Well-graded sands, gravelly sands, little or no fines.		SW -	Well-graded sands, gravelly sands, little or no fines.	
More than 50% of material is LANGER than No. 200 sieve size							SP	Poorly-graded sands, gravelly sands, little or no fines.		SP^	Pourly-graded sands, gravelly sands, little or no fines.	
200 21545 2135	More than 50% of coarse frac- tiom <u>PASSING</u> No. 4 sieve	SANDS WITH FINES (appreciable amount of fines)	(appreciable amount		SM ML	Sand with some silt, silty sand, sandy silt.		SH	Silty sands, sand-silt mixtures.		: 3 SM	Stity sends, sand-silt mixtures,
				sc	Clayey SAND.		sc	Clayey sands, send-clay mixtures.		sc	Cinyey sands, sand-clay mixtures.	
PINE GRAINED	SILTS AMD	Liquid limit LESS tham 30		ML	SILT, Clayey SILT.		le.	Imorganic silts and very fine sands, rock flour, silty or clayey fine sands or clayey silts with slight plasticity.		ML,	<pre>lnor_anic sitts end very fine sands, rock flowr, sitty or clayey fine sands or clayey sitts with slight plasticity.</pre>	
SOILS	CLAYS	<u></u>		CT-XI	Silty clay.		Ct.	Notganic clays of low to medium planticity, gravelly clays, sundy clays, clays, leam clays,		CL	Inorganic clays of low to medium plasticity gravelly clays, sandy clays, silty clays, lean clays,	
							OL	Organic silts and organic silty clays of low plasticity.		OL.	Organic silts and organic silty clays of low plasticity.	
More than 50% of material is SMALLER than No.	SILTS AND CLAYS	Liquid limit GREATER then 50	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				***	Immremic silts, microcoms or distanceous fine sand or silty soils.		104	Inorganic ailto, micacrous or distansceous fine sand or sifty soils.	
200 sieve size				ÇH.	CLAY (High plasticity)		Cal	Imorganic clays of high plasticity, tar- clays.		ĆH	Inorganic aloys of high plasticity, fat clays.	
							CH	Organic clays of medium to high plasticity, organic silts.		OH	Organic clays of medium to high plasticity, organic silts,	
	HIGHLY ORGANIC SOILS						PT	Peat, humus swamp soils with high organic contents.		PT	Peat, humus, suamp soils with high organic contents.	
			2222		Topsotl					-		

	PARTICLE SIZE							
MATERIAL SIZE	LOWER LI	-RIT	UPPEP LIMIT					
	MILLIMETERS	SIEVE SIZE	MILLIMETERS	SIEVE SIZE				
SAND								
PINE	.074	# 200 <b>*</b>	0.42	# 40*				
HEDLUM	0.42	<b>#</b> 40 <b>米</b>	2.00	₩10*				
COARSE	2.00	<b>∔</b> ιο*	4,76	# 4*				
GRAVEL								
FINE	4.76	# 4*	19,1	3/4"#				
COARSE	19, 1	3/4"#	76.2	5" 4				
COBBLES	76.2	3"#	304.8	12"4				
BOULDERS	304.8	12**	914.4	36"4				

* U.S. STANDARD

. CLEAR SQUARE OPENINGS

### GRADATION CHART

<b>6</b> 0 f	)	10	20 3	10 4	O 2	IMIT IO 6	0 7	o 8	0 9	0 10
50 ×			4				СН			
A index					2		A-LINE.			
Zencity 80				a.						
80							ant é	OH		
10		CL-1		MLE	OL.					
o		<u> </u>	P	LASTIC	ITY	CHART				<u> </u>

- Woodward-Clyde Consultants presents only those materials encountered by their field study.
- Dual symbols are used to indicate borderline classifica-tions.
- When shown on the boring logs, the following terms are used to describe the consistency of cohesive soils and the relative compactness of cohesionless soils.

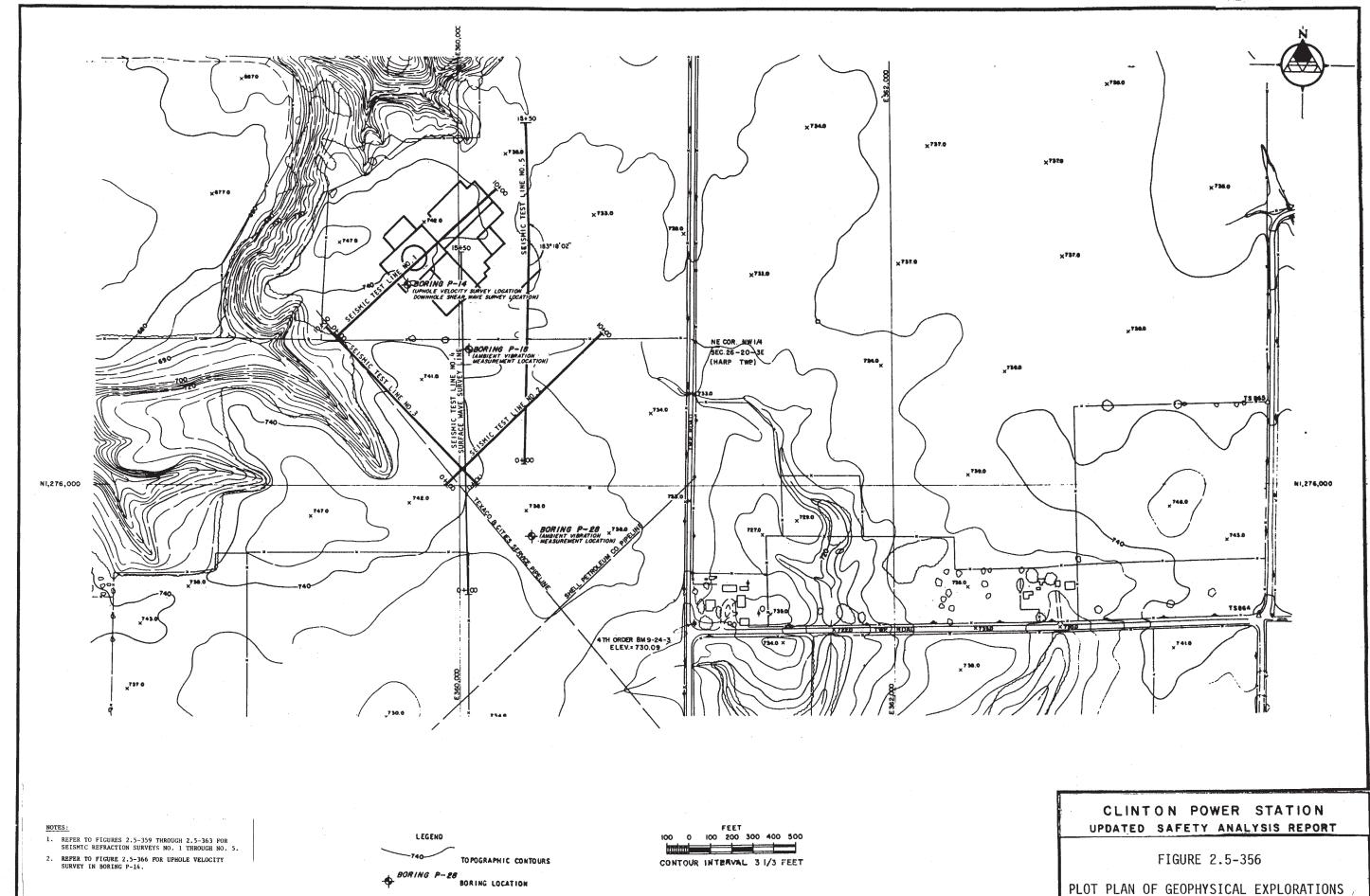
	CONSISTENCY OF CONESIVE SOIL	RELATIVE DENSITY OF GRANULAR SOILS			
NO. BLOWS/PT	UNCONFINED COMPRESSIVE STRENGTH (tsf)	COMSISTENCY	NO. OF BLOWS/FT	RELATIVE DENSITY	
< 2 2-3 4-7 8-14 15-30 >30	<0.25 0.25-0.49 0.50-0.99 1.00-1.99 2.00-4.00 >4.00	Very Soft Soft Medium Stiff Very stiff Hard	0-3 4-9 10-29 30-49 50-80 > 80	Very Loose Loose Hedium Dense Very Dense Extremely dense	

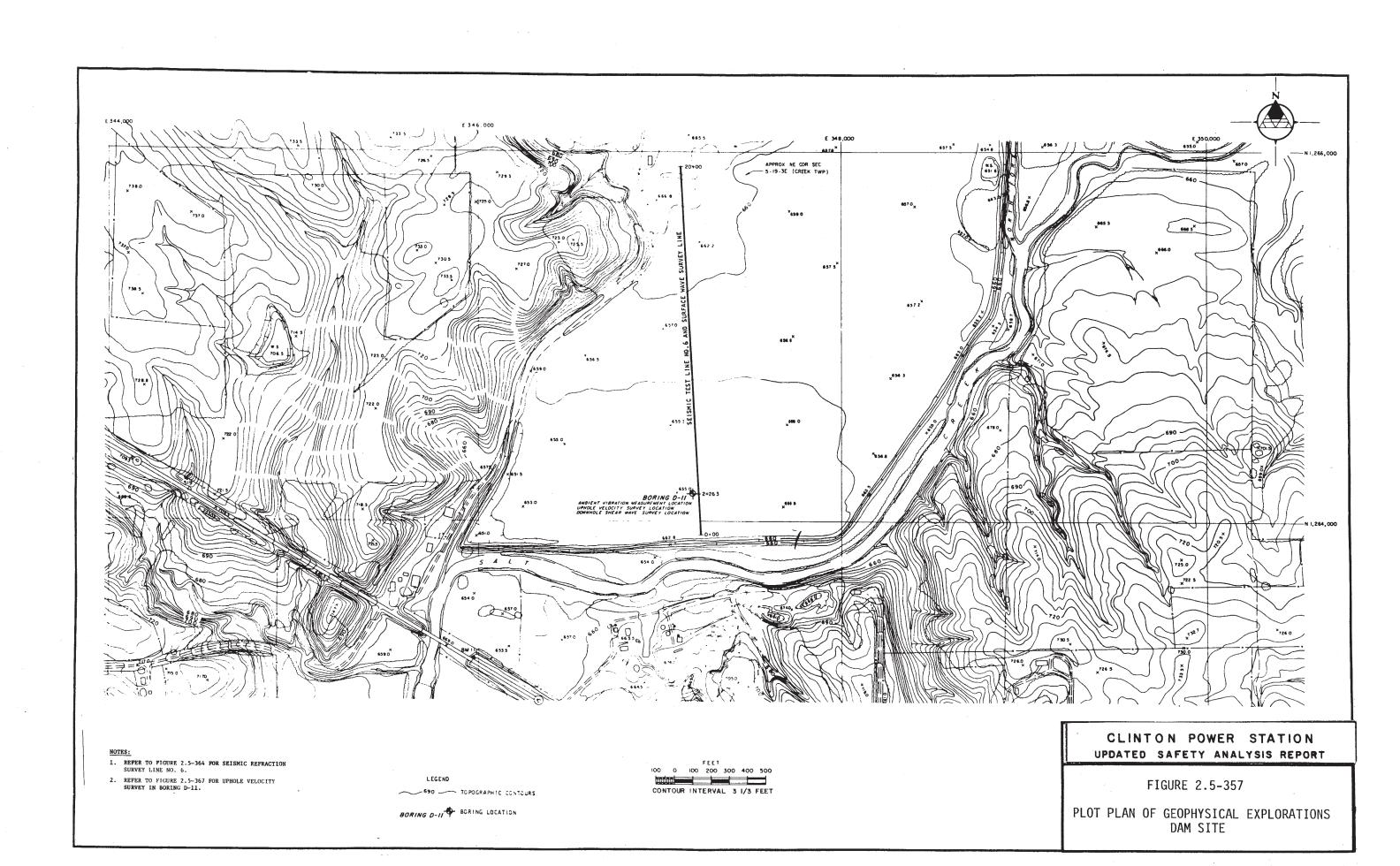
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

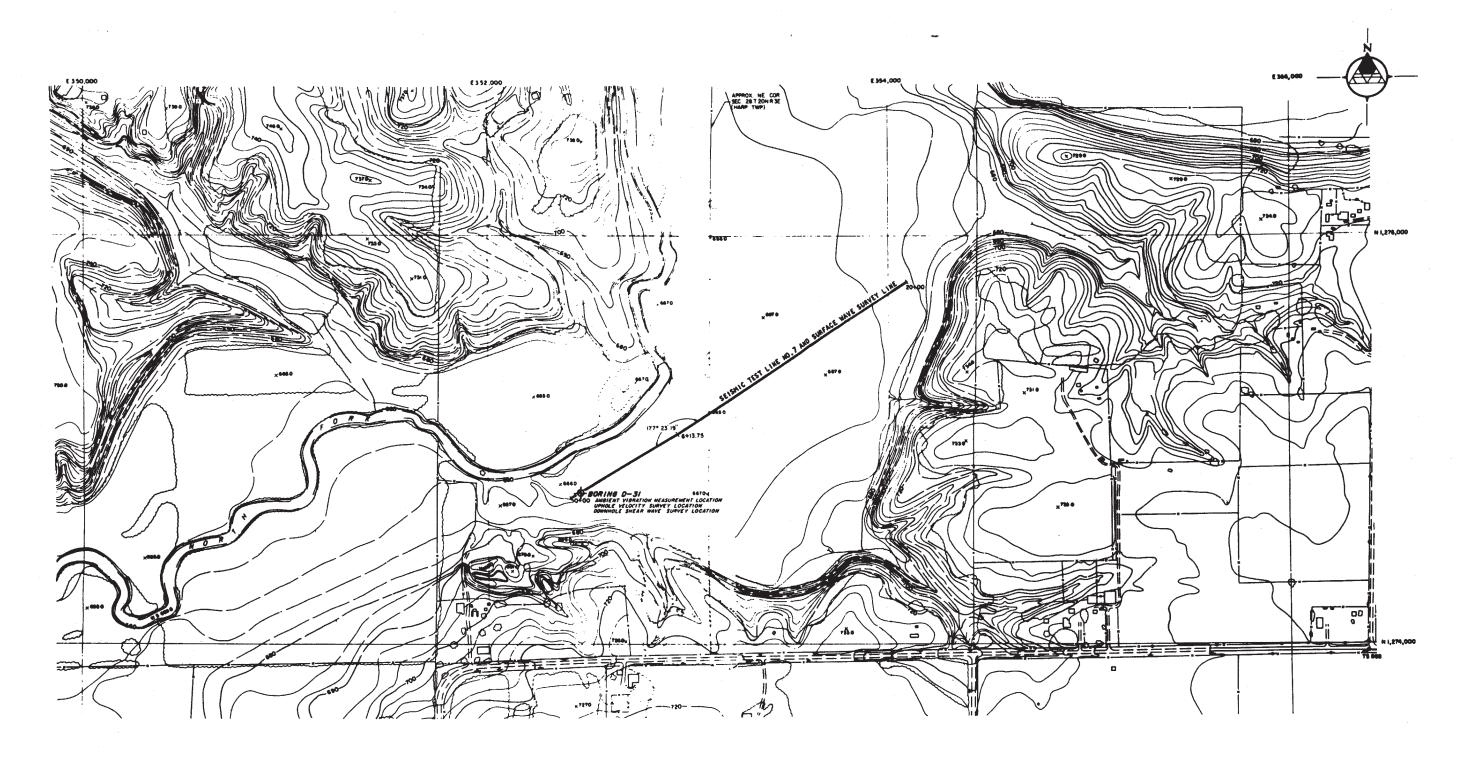
FIGURE 2.5-355

UNIFIED SOIL CLASSIFICATION SYSTEM

STATION SITE







REFER TO FIGURE 2.5-364 FOR SEISMIC REFRACTION SURVEY LINE NO. 7.

LEGENO

- BORING D-3/ BORING LOCATION

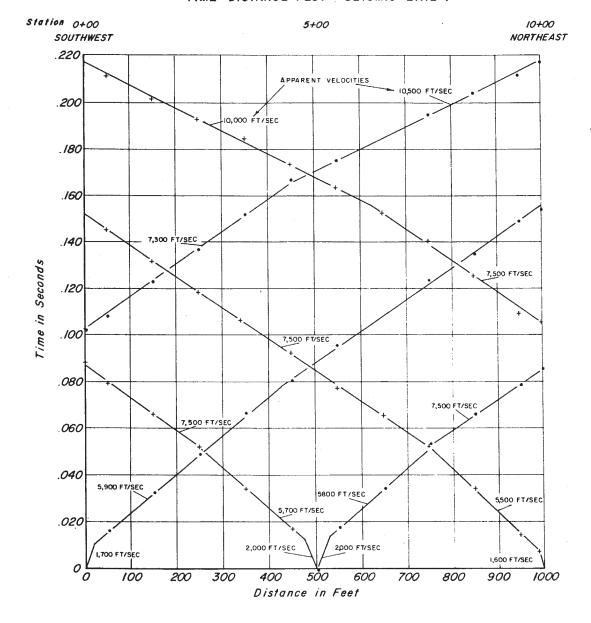
2. REFER TO FIGURE 2.5-368 FOR UPHOLE VELOCITY SURVEY IN BORING D-31.



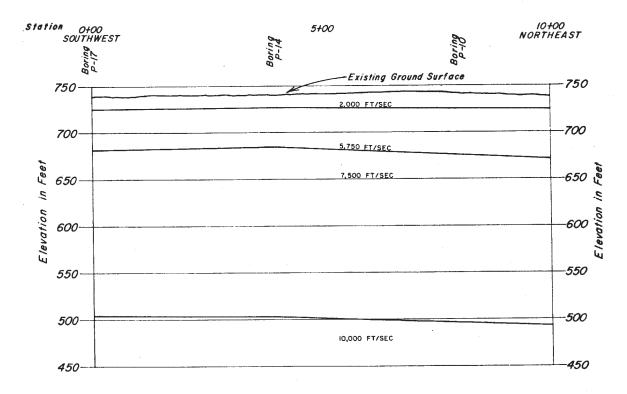
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-358

PLOT PLAN OF GEOPHYSICAL EXPLORATIONS ALONG NORTH FORK OF SALT CREEK



### SUBSURFACE SECTION-SEISMIC LINE I



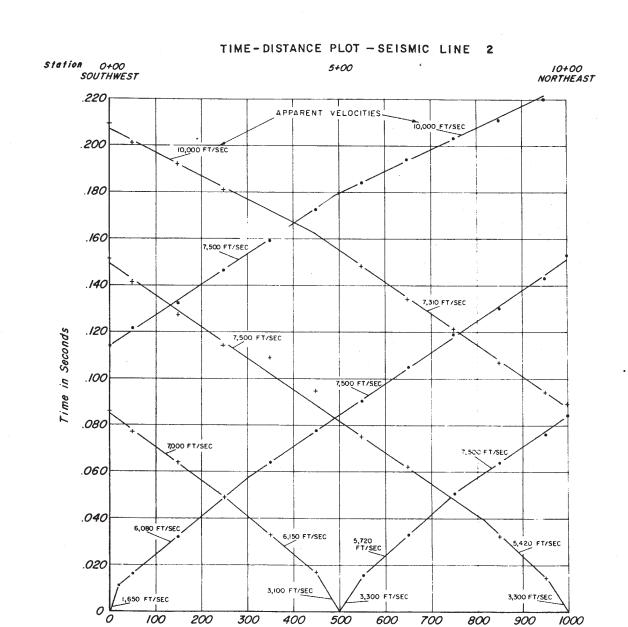
#### NOTES

- 1. TIME DISTANCE PLOTS SHOW INFORMATION COLLECTED FROM SHOT POINTS MADE AT SEVERAL LOCATIONS ALONG A SEISMIC LINE, FOR CLARIFICATION, TWO PLOT SYMBOLS HAVE BEEN USED TO INDICATE THE ORIGIN OF THE ENERGY: FROM THE LEFT (.) FROM THE RIGHT (+).
- 2. THE SUBSURFACE SECTIONS SHOWN PRESENT OUR EVALUATION OF THE MOST PROBABLE SUBSURFACE CONDITIONS BASED UPON OUR INTERPRETATION OF PRESENTLY AVAILABLE DATA, SOME VARIATIONS FROM THESE CONDITIONS MUST BE EXPECTED.
- 3. REFER TO FIGURE 2.5-356 FOR LOCATION OF SEISMIC TEST LINE NO. 1.

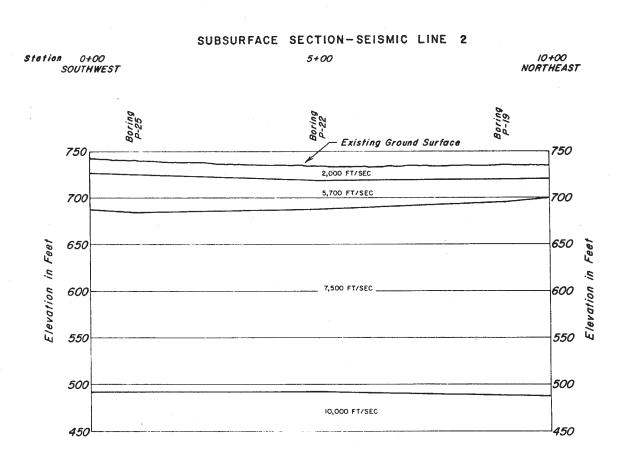
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FIGURE 2.5-359

SEISMIC REFRACTION SURVEY LINE 1 - STATION SITE



Distance in Feet



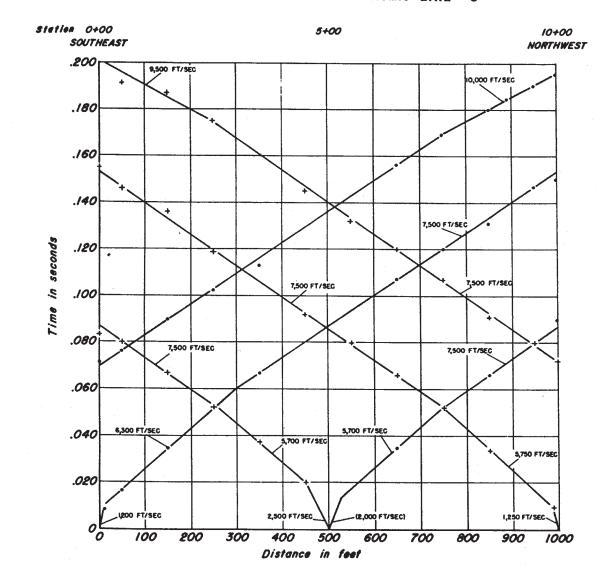
#### NOTE

- 1. TIME DISTANCE PLOTS SHOW INFORMATION COLLECTED FROM SHOT POINTS MADE AT SEVERAL LOCATIONS ALONG A SEISMIC LINE, FOR CLARIFICATION, TWO PLOT SYMBOLS HAVE BEEN USED TO INDICATE THE ORIGIN OF THE ENERGY: FROM THE LEFT (.) FROM THE RIGHT (+).
- 2. THE SUBSURFACE SECTIONS SHOWN PRESENT OUR EVALUATION OF THE MOST PROBABLE SUBSURFACE CONDITIONS BASED UPON OUR INTERPRETATION OF PRESENTLY AVAILABLE DATA, SOME VARIATIONS FROM THESE CONDITIONS MUST BE EXPECTED.
- REFER TO FIGURE 2.5-356 FOR LOCATION OF SEISMIC TEST LINE NO. 2.

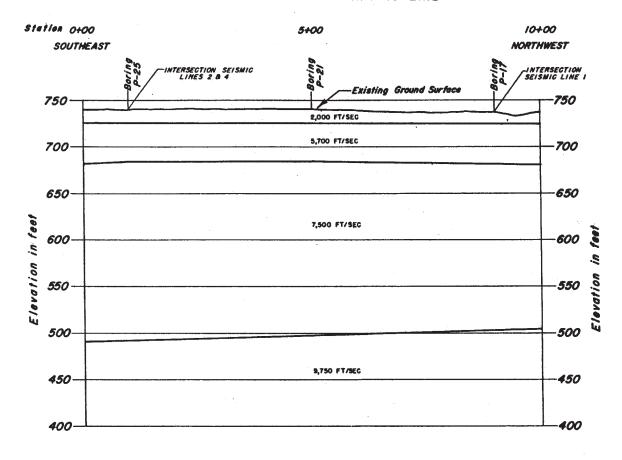
# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-360

SEISMIC REFRACTION SURVEY LINE 2 STATION SITE



### SUBSURFACE SECTION-SEISMIC LINE 3



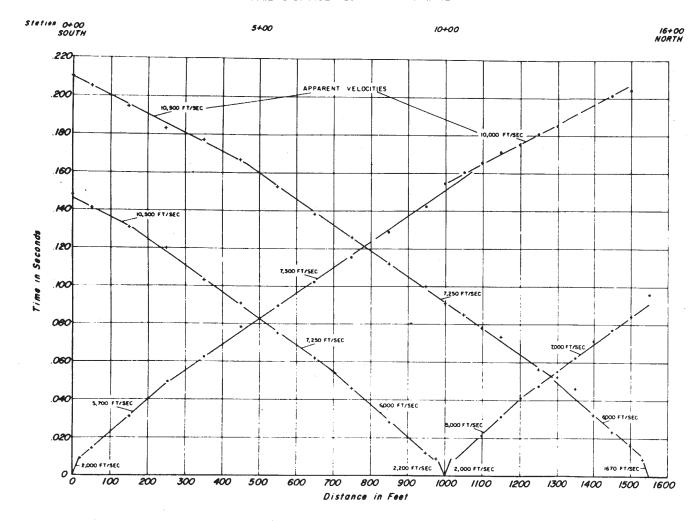
#### NOTES

- 1. TIME DISTANCE PLOTS SHOW INFORMATION COLLECTED
  FROM SHOT POINTS MADE AT SEVERAL LOCATIONS ALONG
  A SEISMIC LINE, FOR CLARIFICATION, TWO PLOT SYMBOLS
  HAVE BEEN USED TO INDICATE THE ORIGIN OF THE ENERGY:
  FROM THE LEFT (.) FROM THE RIGHT (+).
- 2. THE SUBSURFACE SECTIONS SHOWN PERSENT OUR EVALUATION OF THE MOST PROBABLE SUBSURFACE CONDITIONS BASED UPON OUR INTERPRETATION OF PRESENTLY AVAILABLE DATA, SOME VARIATIONS FROM THESE CONDITIONS MUST BE EXPECTED.
- 3. REFER TO FIGURE 2.5-356 FOR LOCATION OF SEISMIC TEST LINE NO. 3.

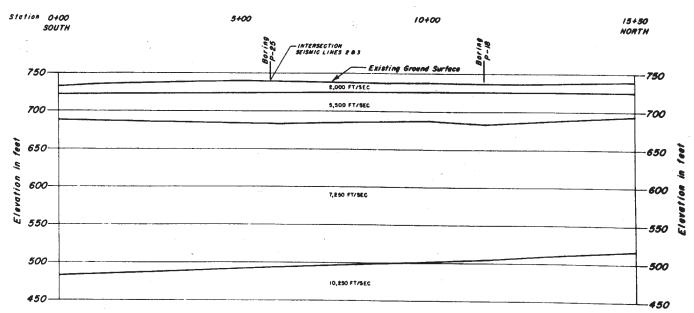
# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-361

SEISMIC REFRACTION SURVEY LINE 3
STATION SITE







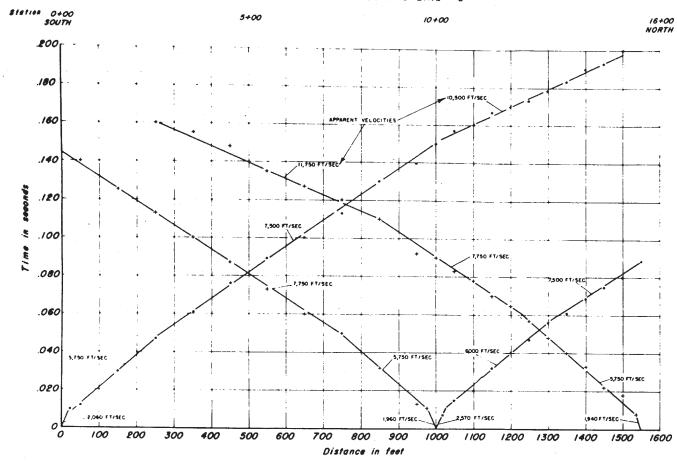
#### NOTE

- 1. TIME DISTANCE PLOTS SHOW INFORMATION COLLECTED
  FROM SHOT POINTS MADE AT SEVERAL LOCATIONS ALONG
  A SEISMIC LINE, FOR CLARIFICATION, TWO PLOT SYMBOLS
  HAVE BEEN USED TO INDICATE THE ORIGIN OF THE ENERGY:
  FROM THE LEFT (.) FROM THE RIGHT (+).
- THE SUBSURFACE SECTIONS SHOWN PRESENT OUR EVALUATION OF THE MOST PROBABLE SUBSURFACE CONDITIONS BASED UPON OUR INTERPRETATION OF PRESENTLY AVAILABLE DATA, SOME VARIATIONS FROM THESE CONDITIONS MUST BE EXPECTED.
- 3. REFER TO FIGURE 2.5-356 FOR LOCATION OF SEISMIC TEST LINE NO. 4.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

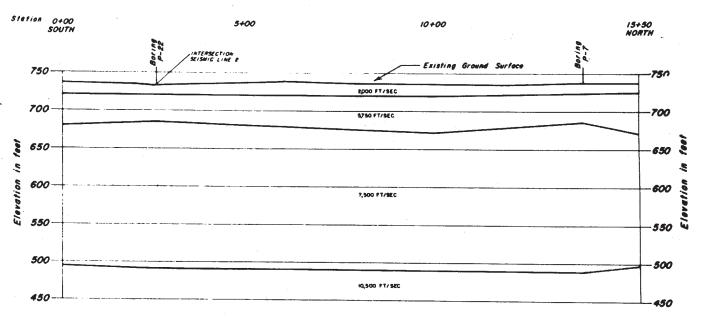
FIGURE 2.5-362

SEISMIC REFRACTION SURVEY LINE 4 - STATION SITE



- 1. TIME DISTANCE PLOTS SHOW IMPORMATION COLLECTED
  FROM SHOT POINTS MADE AT SEVERAL LOCATIONS ALONG
  A SEISMIC LINE, FOR CLARIFICATION, TWO PLOT SYMBOLS
  HAVE BEEN USED TO INDICATE THE ORIGIN OF THE ENERGY:
  FROM THE LEFT (.) FROM THE RIGHT (+).
- 2. THE SUBSURFACE SECTIONS SHOWN PRESENT OUR EVALUATION OF THE MOST PROBABLE SUBSURFACE CONDITIONS BASED UPON OUR INTERPRETATION OF PRESENTLY AVAILABLE DATA, SOME VARIATIONS FROM THESE CONDITIONS MUST BE EXPECTED.
- 3. REFER TO FIGURE 2.5-356 FOR LOCATION OF SEISMIC TEST LINE NO. 5.

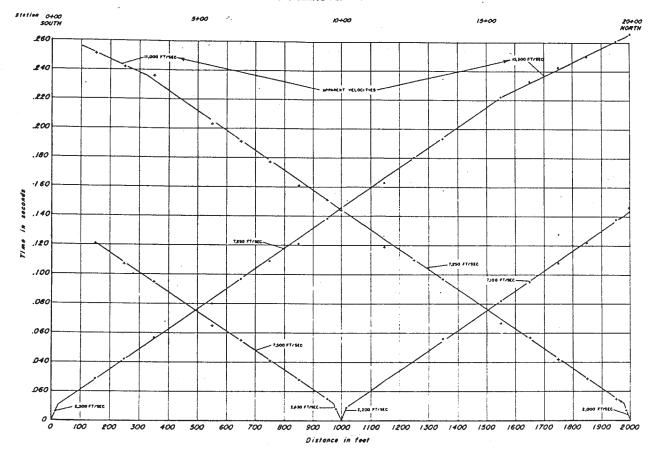
### SUBSURFACE SECTION-SEISMIC LINE 5



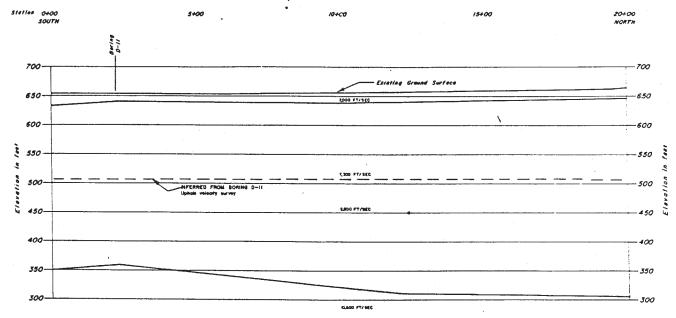
# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-363

SEISMIC REFRACTION SURVEY LINE 5 - STATION SITE



### SUBSURFACE SECTION-SEISMIC LINE



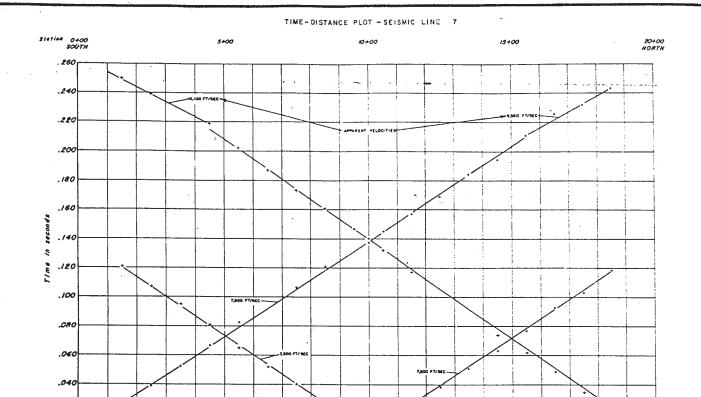
### NOTES:

- 1. TIME DISTANCE PLOTS SHOW INFORMATION COLLECTED FROM SHOT POINTS MADE AT SEVERAL LOCATIONS ALONG A SEISMIC LINE, FOR CLARIFICATION, TWO PLOT SYMBOLS HAVE BEEN USED TO INDICATE THE ORIGIN OF THE ENERGY: FROM THE LEFT (.) FROM THE RIGHT (+).
- THE SUBSURFACE SECTIONS SHOWN PRESENT OUR EVALUATION OF THE MOST PROBABLE SUBSURFACE CONDITIONS BASED UPON OUR INTERPRETATION OF PRESENTLY AVAILABLE DATA, SOME VARIATIONS FROM THESE CONDITIONS MUST BE EXPECTED.
- REFER TO FIGURE 2.5-357 FOR LOCATION OF SEISMIC TEST LINE NO. 6.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-364

SEISMIC REFRACTION SURVEY LINE 6 - DAM SITE



700

800

900

1000

Distance in feet

1100

1200 1300

1400

1500

1600

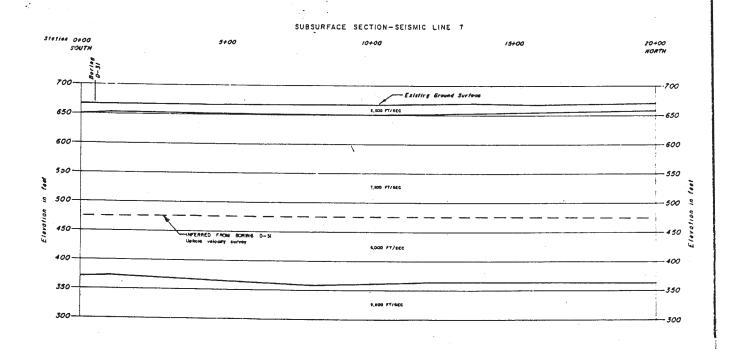
1700

1800

1900

2000

600



### NOTES:

0

100

- 1. TIME DISTANCE PLOTS SHOW INFORMATION COLLECTED FROM SHOT POINTS MADE AT SEVERAL LOCATIONS ALONG A SEISMIC LINE, FOR CLARIFICATION, TWO PLOT SYMBOLS HAVE BEEN USED TO INDICATE THE ORIGIN OF THE ENERGY: FROM THE LEFT (.) FROM THE RIGHT (+).
- THE SUBSURFACE SECTIONS SHOWN PRESENT OUR EVALUATION OF THE MOST PROBABLE SUBSURFACE CONDITIONS BASED UPON OUR INTERPRETATION OF PRESENTLY AVAILABLE DATA, SOME VARIATIONS FROM THESE CONDITIONS MUST BE EXPECTED.
- 3. REFER TO FIGURE 2.5-358 FOR LOCATION OF SEISMIC TEST LINE NO. 7.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-365

SEISMIC REFRACTION SURVEY LINE 7 -SECTION E-E' ALONG NORTH FORK OF SALT CREEK

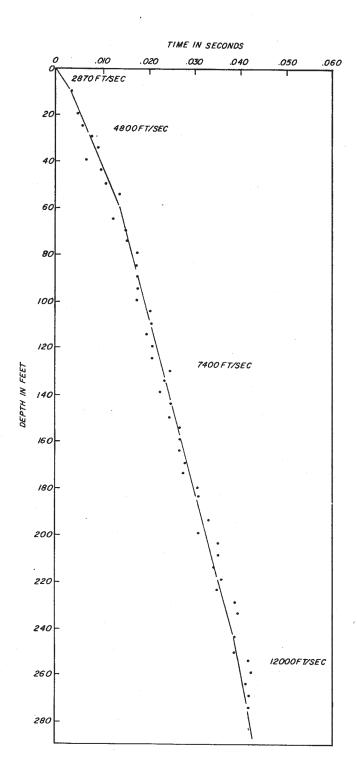


FIGURE 2.5-366

UPHOLE (COMPRESSIONAL) VELOCITY SURVEY -BORING P-14, STATION SITE

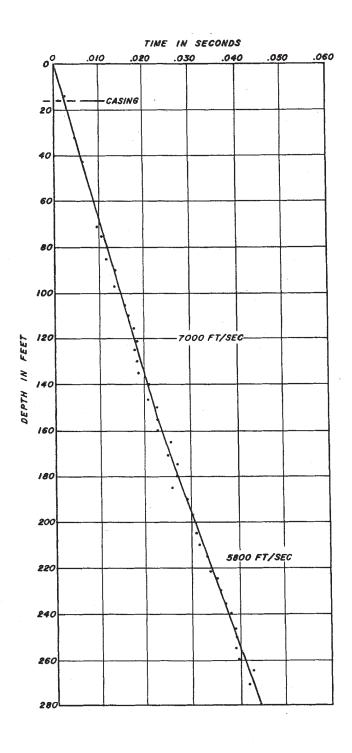


FIGURE 2.5-367

UPHOLE (COMPRESSIONAL) VELOCITY SURVEY BORING D-11A, DAM SITE

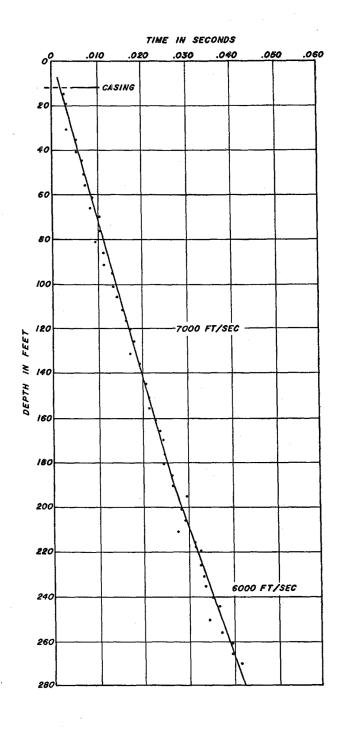


FIGURE 2.5-368

UPHOLE (COMPRESSIONAL) VELOCITY SURVEY -BORING D-31 SECTION E-E' ALONG NORTH FORK OF SALT CREEK

	<b>7-</b>	GRAPHIC COLUMN		UNIT DESCRIPTION	COMPRESSIONAL WAVE VELOCITY FT/SEC	POISSONS RATIO	SHEAR WAVE VELOGITY FT/SEC	(p	GHT													
	7										N3					LOESS, weathered; overlain by organic topsoil WISCONSINAN GLACIAL TILL, weathered to brown, moist	2000	0.37	900	110	130	•
	19											WISCONSINAN GLACIAL TILL, unwedthered	5700	0.48	1100	115	135					
FEET	12	\$\$000°00°00	OVERBURDEN	INTERGLACIAL ZONE; local ALLUVIAL DEPOSITS						·												
AVERAGE THICKNESS IN (NOT TO SCALE)	1!3				OVER	ILLINOIAN GLACIAL TILL; local OUTWASH AND LACUSTRINE DEPOSITS																
AVER	10											LACUSTRINE DEPOSIT (Yarmouthian)	7500	0.46	2100	135	147					
	65				PRE-ILLINOIAN GLACIAL TILL locally underlain by PRE-ILLINOIAN LACUSTRINE DEPOSIT																	
	5 15		BEDROCK	SHALE SILTSTONE	10,500	0.29	5700	155	160													
CLINTON POWER STATION																						

UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-369

NOTE:

REFER TO LEGEND ON FIGURES 2.5-275 AND 2.5-276 FOR DETAILED DESCRIPTION OF OVERBURDEN UNITS.

TYPICAL GEOLOGIC PROFILE SHOWING GEOPHYSICAL PROPERTIES - STATION SITE

		GRAPHIC COLUMN		UNIT DESCRIPTION	COMPRESSIONAL WAVE VELOCITY FT/SEC	POISSONS RATIO	SHEAR WAVE VELOCITY FT/SEC	(pc	GHT f)									
					ŏ≯		*	DRY	WET									
	10			SALT CREEK ALLUVIUM; overlain by organic topsoil	2000	0.37	900	100	122									
S IN FEET	112		ÓVERBURDEN	VERBURDEN	VERBURDEN	VERBURDEN	VERBURDEN	VERBURDEN	VERBURDEN	ÓVERBURDEN	ÓVERBURDEN	ILLINOIAN GLACIAL TILL; local OUTWASH AND LACUSTRINE DEPOSITS						
AVERAGE THICKNESS IN (NOT TO SCALE)	50			PRE-ILLINOIAN LACUSTRINE AND GLACIAL TILL DEPOSITS present locally	7300	0.46	2000	135	145									
AVER	140	140		BEDROCK VALLEY OUTWASH DEPOSIT (Pre-Illinoian Mahomet Valley Deposit)	5800	0.45	1750	109	127									
	5		¥	LIMESTONE														
	10		BEDROCK	SHALE	10,500	0.29	5700	155	160									
				SILTSTONE														
					U	CLINTO	N POWER		ATIO	i i								

NOTE:

* VALUES ARE ESTIMATED

REFER TO BORING D-11 AND LEGEND ON FIGURE 2.5-277 FOR DETAILED DESCRIPTION OF OVERBURDEN UNITS.

FIGURE 2.5-370

TYPICAL GEOLOGIC PROFILE SHOWING GEOPHYSICAL PROPERTIES - DAM SITE

	GRAPHIC COLUMN		UNIT DESCRIPTION	COMPRESSIONAL WAVE. VELOCITY FT/SEC	POISSONS	SHEAR WAVE VELOCITY FT/SEC	UN WEI (pc	SHT f)	
	7	ن	SALT CREEK ALLUVIUM; overlain by organic topsoil	2000	0.37	900	105	125	
FEET		RDEN	ILLINGIAN GLACIAL TILL local OUTWASH AND LACUSTRINE DEPOSITS	·	•				
THICKN (NOT TO	99°°°°°°°°°°°°°°°°°°°°°°°°°°°°°°°°°°°°	OVERBURDEN	LACUSTRINE DEPOSIT (Yarmouthian)  PRE-ILLINOIAN GLACIAL TILL; iocal OUTWASH DEPOSITS	7500	0.46	2100	132	145	
	5		PRE-ILLINOIAN OUTWASH AND ALLUVIAL DEPOSIT						
, 11	0		BEDROCK VALLEY OUTWASH DEPOSIT (Pre-Illinoian Mahomet Valley Deposit)	6000	0.45	1800	106	124	
2	20	BEDROCK	SILTSTONE	10,000	0.28 T0 0.30	5300 T0 5500	155	160	
3	0	38	COAL			N POWE	R G	TATI	0.0

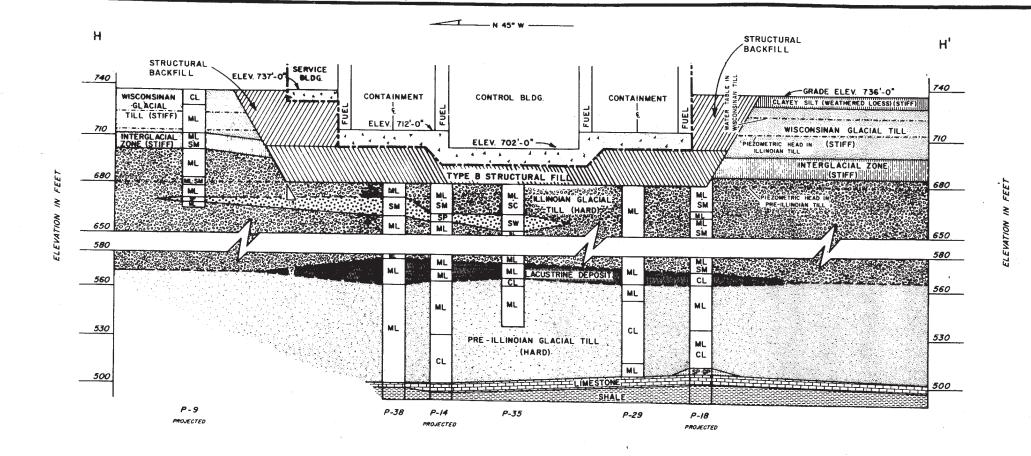
FIGURE 2.5-371

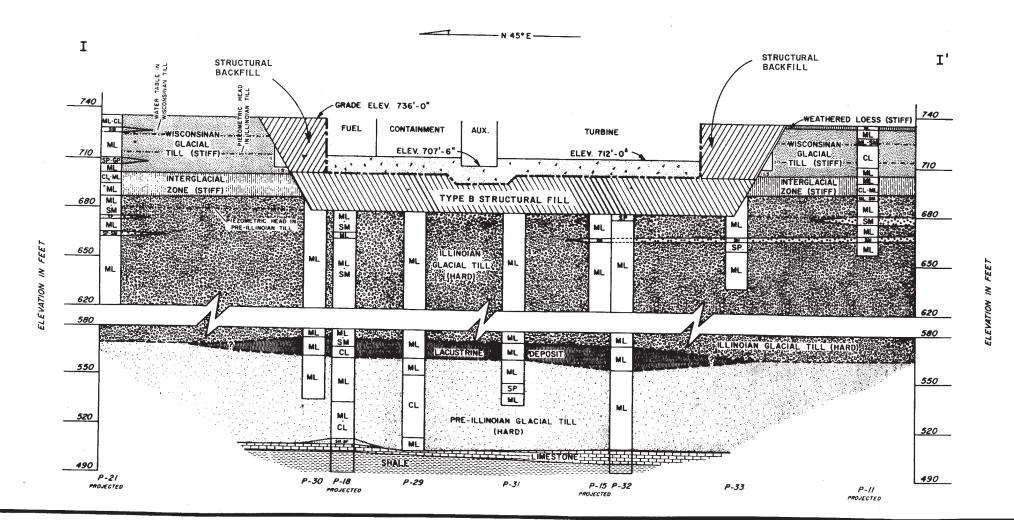
* VALUES ARE ESTIMATED

NOTE:

REFER TO LEGEND ON FIGURE 2.5-278 FOR DETAILED DESCRIPTION OF OVERBURDEN UNITS.

TYPICAL GEOLOGIC PROFILE SHOWING GEOPHYSICAL PROPERTIES - SECTION E-E' ALONG NORTH FORK OF SALT CREEK





### LEGEND

TYPE B GRANULAR STRUCTURAL BACKFILL

TYPE B GRANULAR STRUCTURAL FILL

CONCRETE

LOSS - BROWN TO MOTTLED BROWN AND GRAY CLAYEY SILT OR SILTY CLAY WITH TRACE OF FIRE SAME) WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME WASHINGTON OF THE SAME W

HTTERCACE LIL TOW - HELIDISC DAME CART TO DAME ORGANIC CLAFFY SIT OF METAL STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF THE STATE OF

SAMO - IMBEGUALD DISCOPTINGUES LATES OF ALLEVIAL SAMD AND GAMPEL

(LACUSTRIBE DEPOSIT - BROWNISH-GAMY TO BLACK AND GAM CANYEL SLLT TO

SILT, ORGANIC IN ZOMES; INCLUDES AGRESSION TO BLUTSH-GAMY CLAYEY SLLT TO

PRE-ILLINDIAN GLACIAL TILL - GRAYISM-BROWN TO BROWN SILTY CLAY AND CLAYET SILT WITH SOME SAMD AND GRAYEL, BROWN COLDS AND REATIVELY SCHEMETERS ITS: TENTATIVELY ASSIGNED TO GRASAM GLACIAL STARE ON THE BASIS OF CLAY AMALTSIS BY ILLINDIS STATE GROUNDING STATE

LIMESTONE - CREENICH-CRAY, GRAY AND BROWN, FINE TO COMMSELY CRYSTALLIN SILTY, THIN EXPONE TO MASSIME, MARROUS SHALE PRATYINGS IN ZONES, FOSSIL FRENT

SHALE - GRAY TO DARK GRAY SHALE, CARBONACEOUS TO CALCAREOUS; CLAYEY IN

#### NOTES:

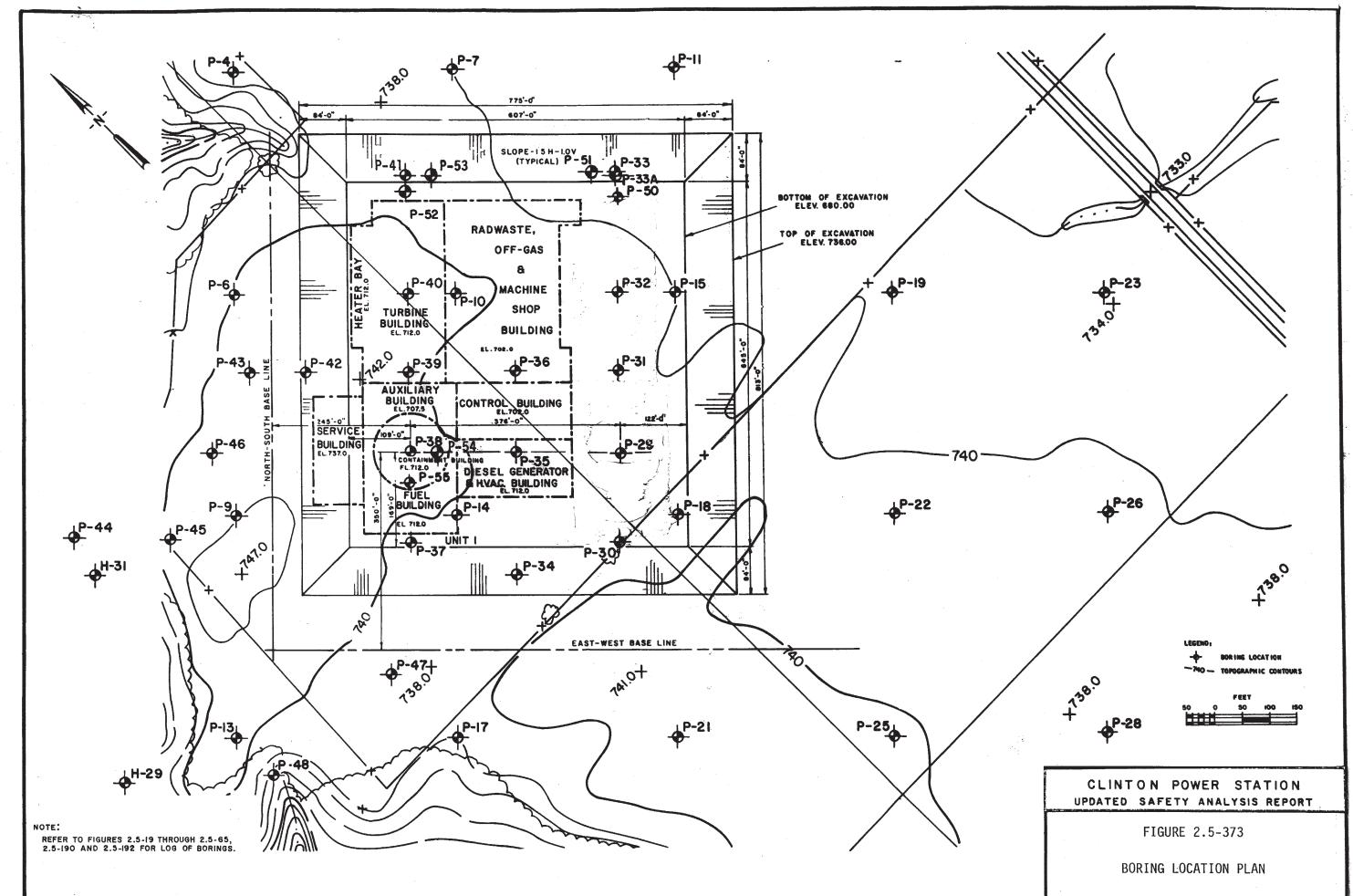
- REFER TO FIGURES 2.5-271 AND 2.5-374 FOR LOCATION OF SUBSURFACE SECTIONS.
- 2. REFER TO FIGURES 2.5-26, 2.5-27, 2.5-29, 2.5-30, 2.5-34, 2.5-37, 2.5-45 THROUGH 2.5-49, 2.5-51 AND 2.5-55 FOR LOG OF BORINGS.
- FOR DETAILED WATER LEVEL DATA REFER TO FIGURE 2.4-36.
- STRATIGRAPHIC NOMENCLATURE OF THE QUATERNARY DEPOSITS USED IN THE FSAR TEXT DIFFERS FROM THAT USED ON THIS FIGURE AND ON THE BORING LOGS, AND IS CROSS REFERENCED ON FIGURE 2.5-274.
- 5. STRUCTURAL BACKFILL FOR THE MAIN PLANT CONSISTS OF TYPE B GRANULAR MATERIAL EXCEPT ON THE NORTH AND WEST SIDES OF UNIT 1. IN THESE AREAS, AND ABOVE APPROXIMATE ELEVATION 720 FEET, TYPES A 5 C COHESIVE MATERIAL WAS USED AS STRUCTURAL BACKFILL.

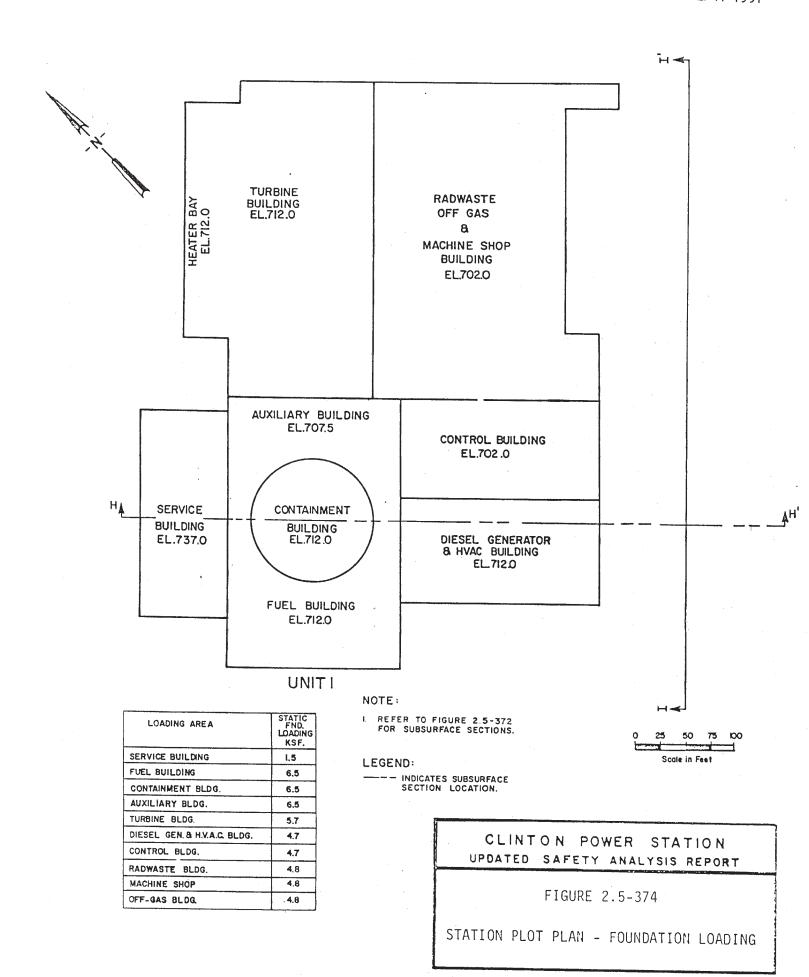


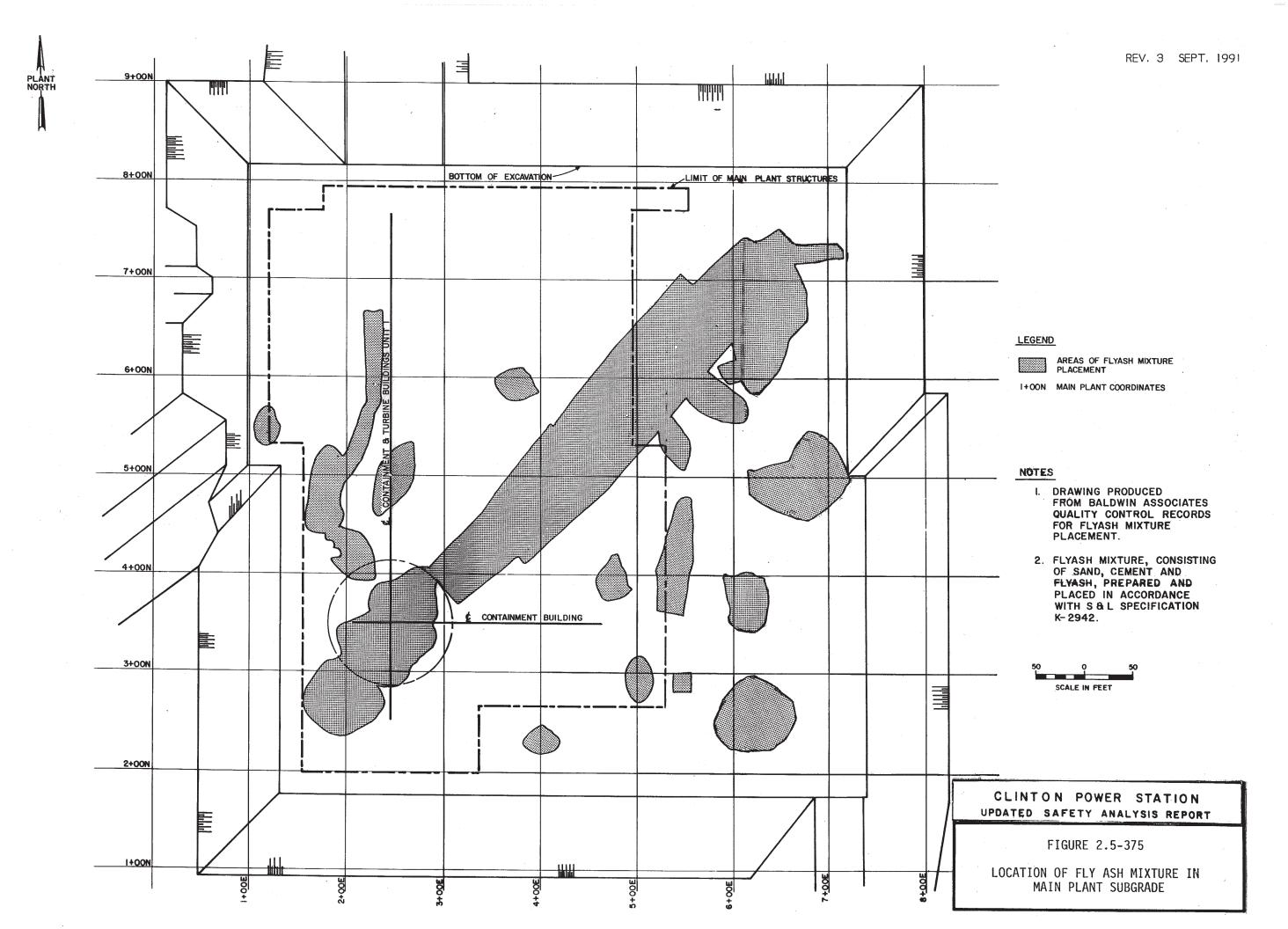
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

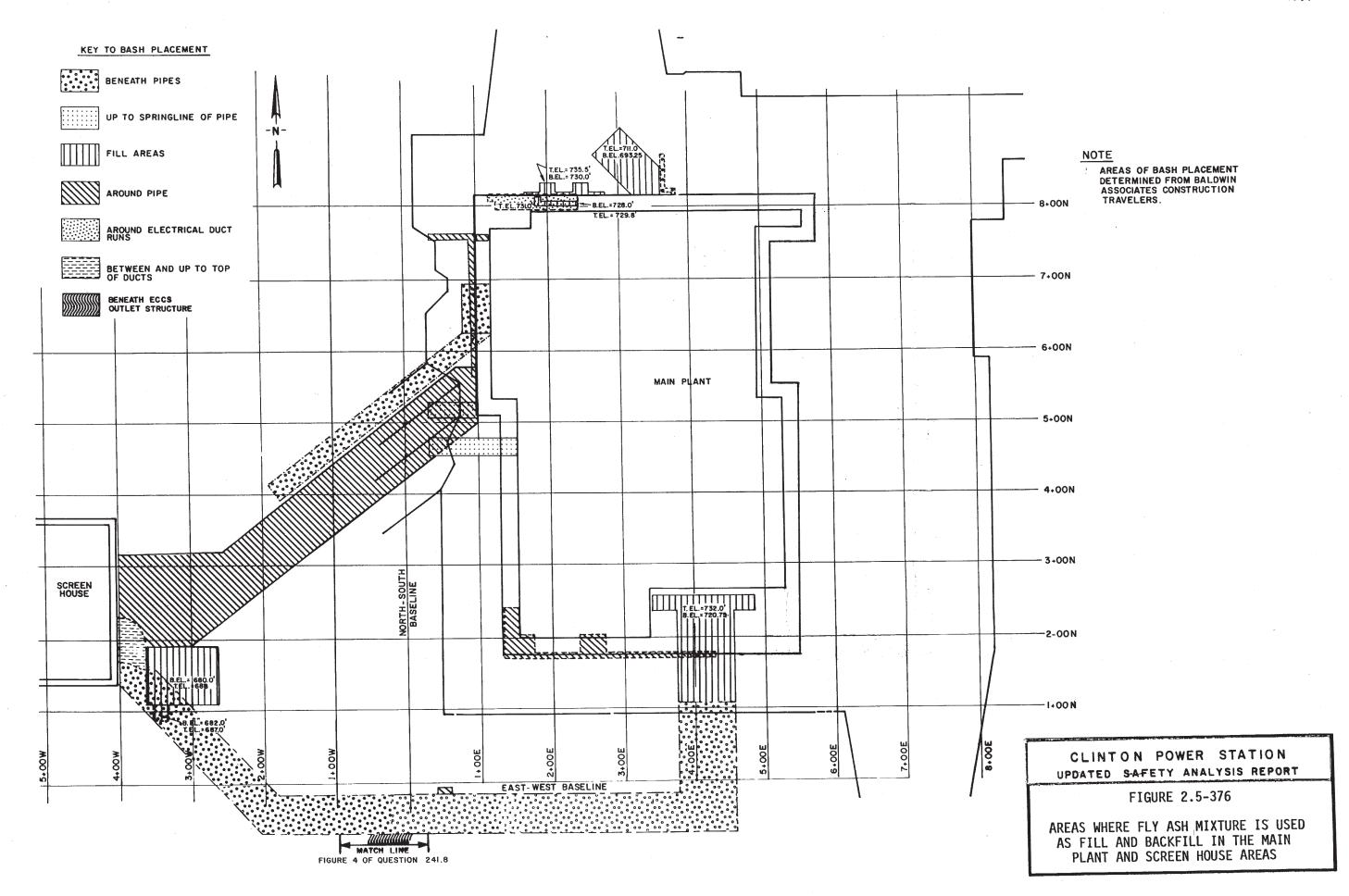
FIGURE 2.5-372

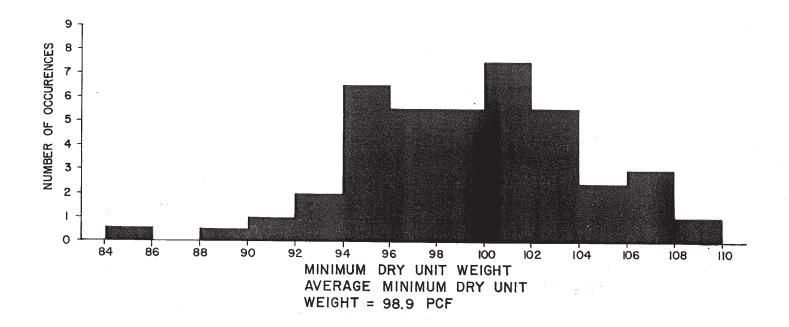
GEOLOGIC SECTIONS H-H' AND I-I' -STATION SITE

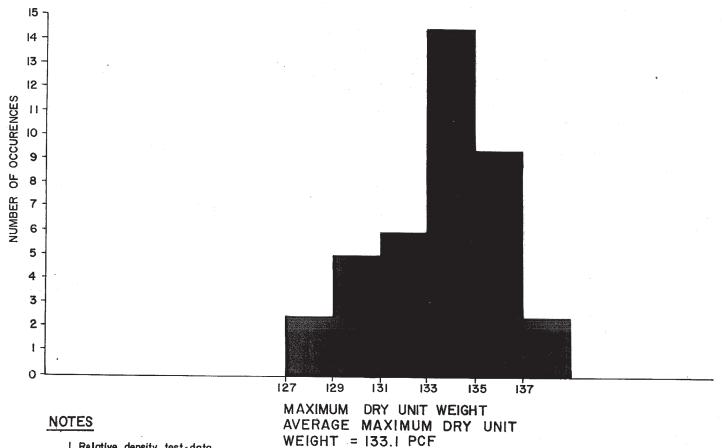








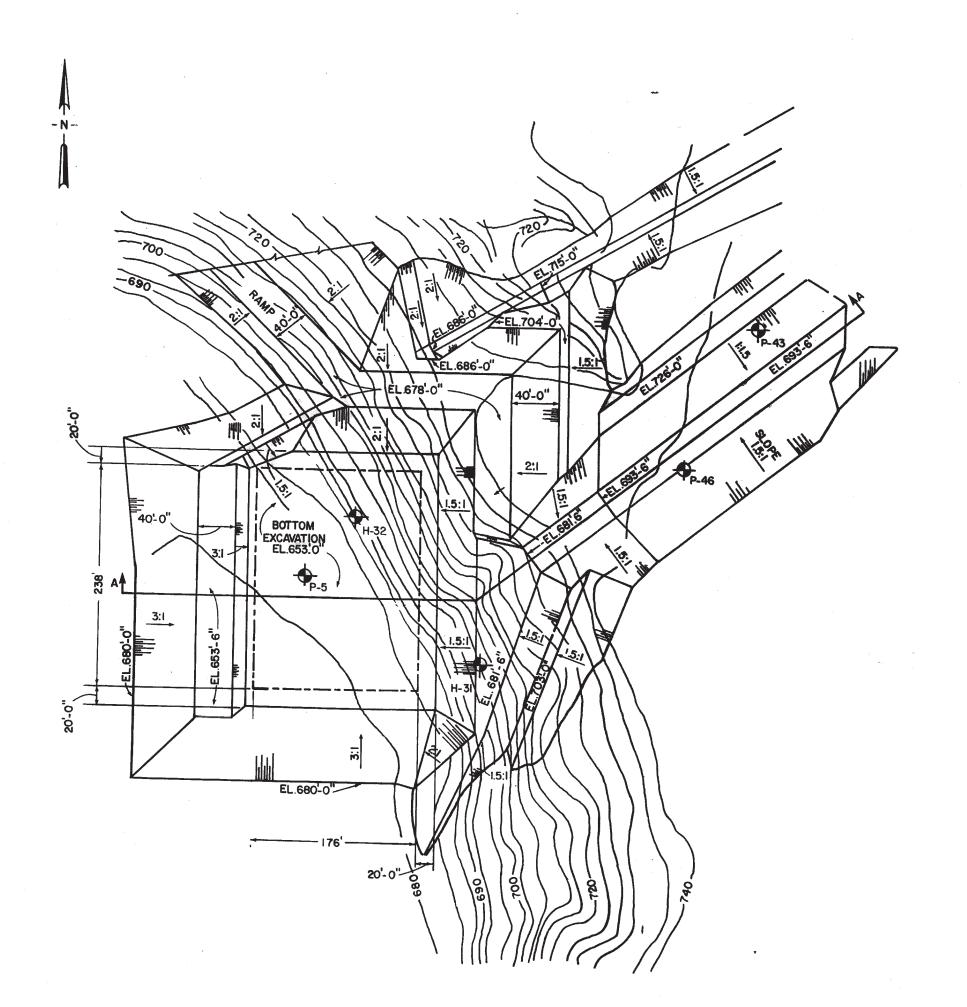




- I. Relative density test-data based on ASTM D2049.
- 2. Number of occurences based on average of two pounds per cubic foot range.

FIGURE 2.5-377

POWER BLOCK RELATIVE DENSITY
TEST SUMMARY



### NOTE

See Figure 2.5-379 for section through screen house.

### LEGEND





CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-378

EXCAVATION PLAN FOR CIRCULATING WATER SCREEN HOUSE

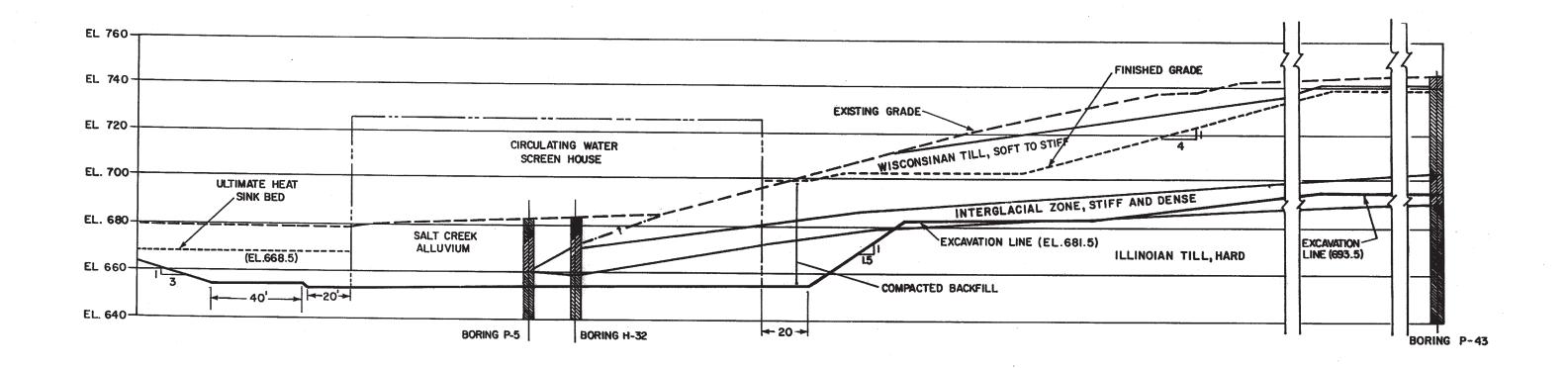




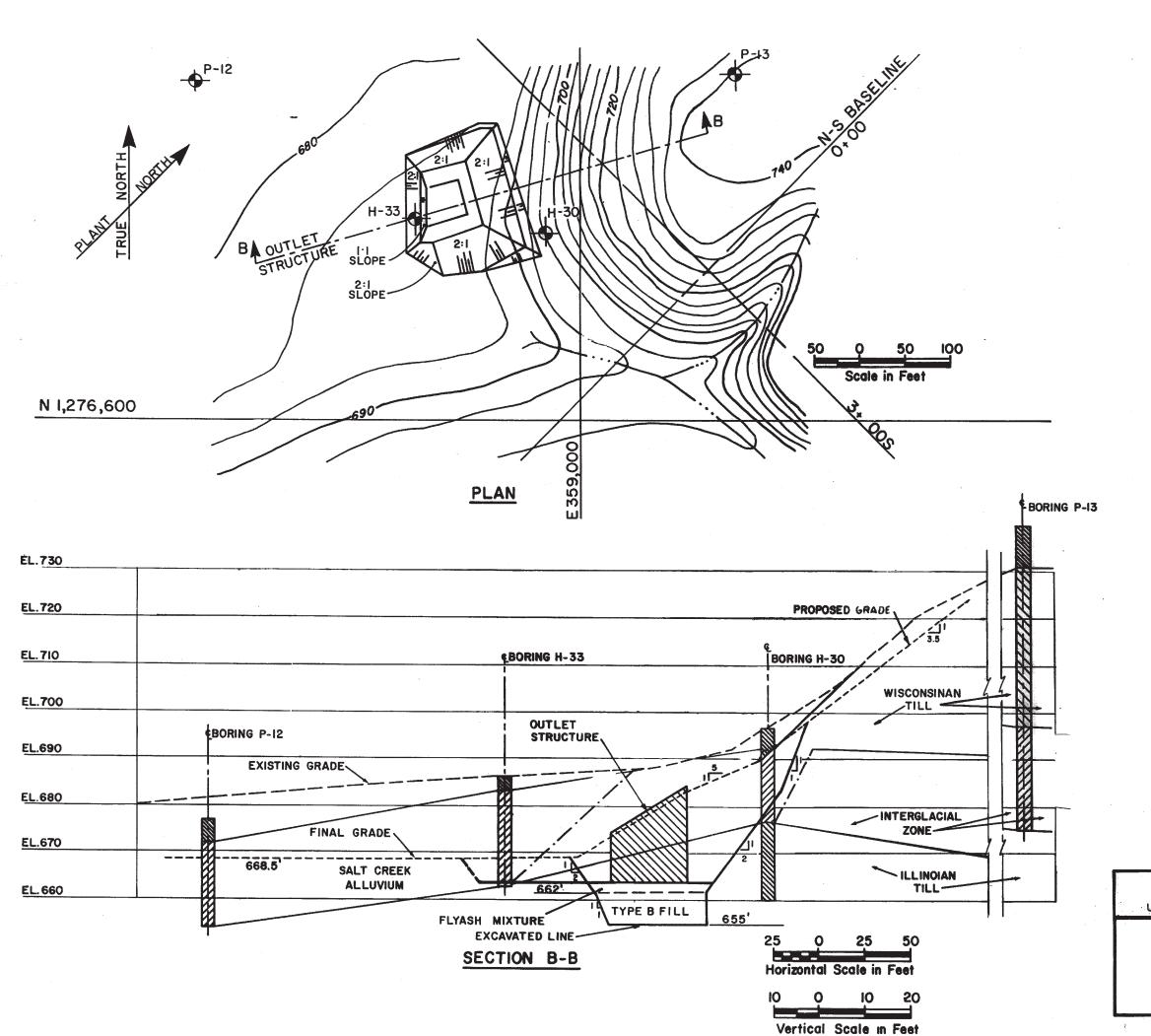
FIGURE 2.5-379

SECTION THROUGH CIRCULATING WATER SCREEN
HOUSE SHOWING EXCAVATION LINE

### CPS/USAR

FIGURE 2.5-380 HAS BEEN DELETED

CHAPTER 02 REV. 12, JAN 2007



### LEGEND



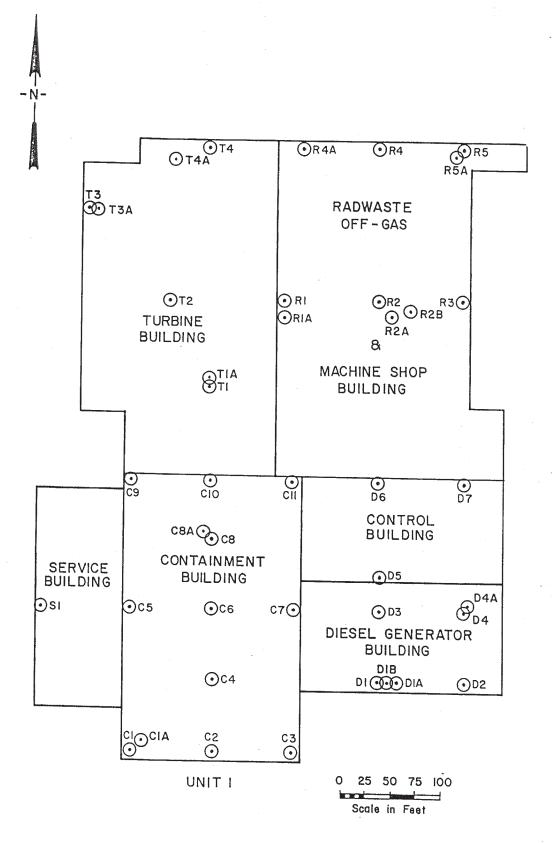
### NOTE

I. The soil profile shows the projection of Borings P-12, H-30 and P-13 along the center line of the outlet structure.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-381

EXCAVATION PLAN AND SECTION FOR OUTLET STRUCTURE



### NOTE

I. OCI SETTLEMENT MONUMENT LOCATION.

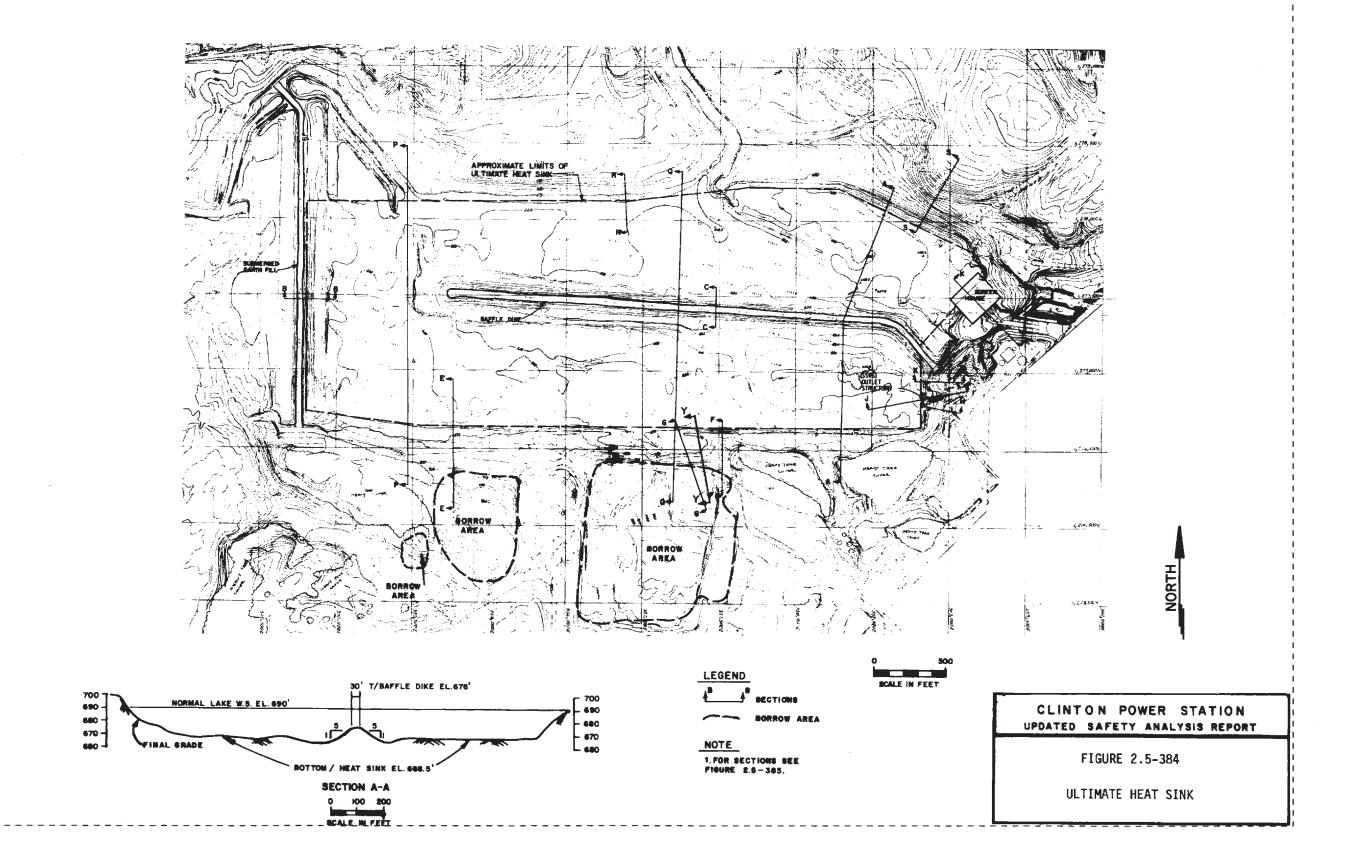
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-382

LOCATIONS OF SETTLEMENT MONUMENTS - MAIN PLANT

### CPS/USAR

Figure 2.5-383 Deleted



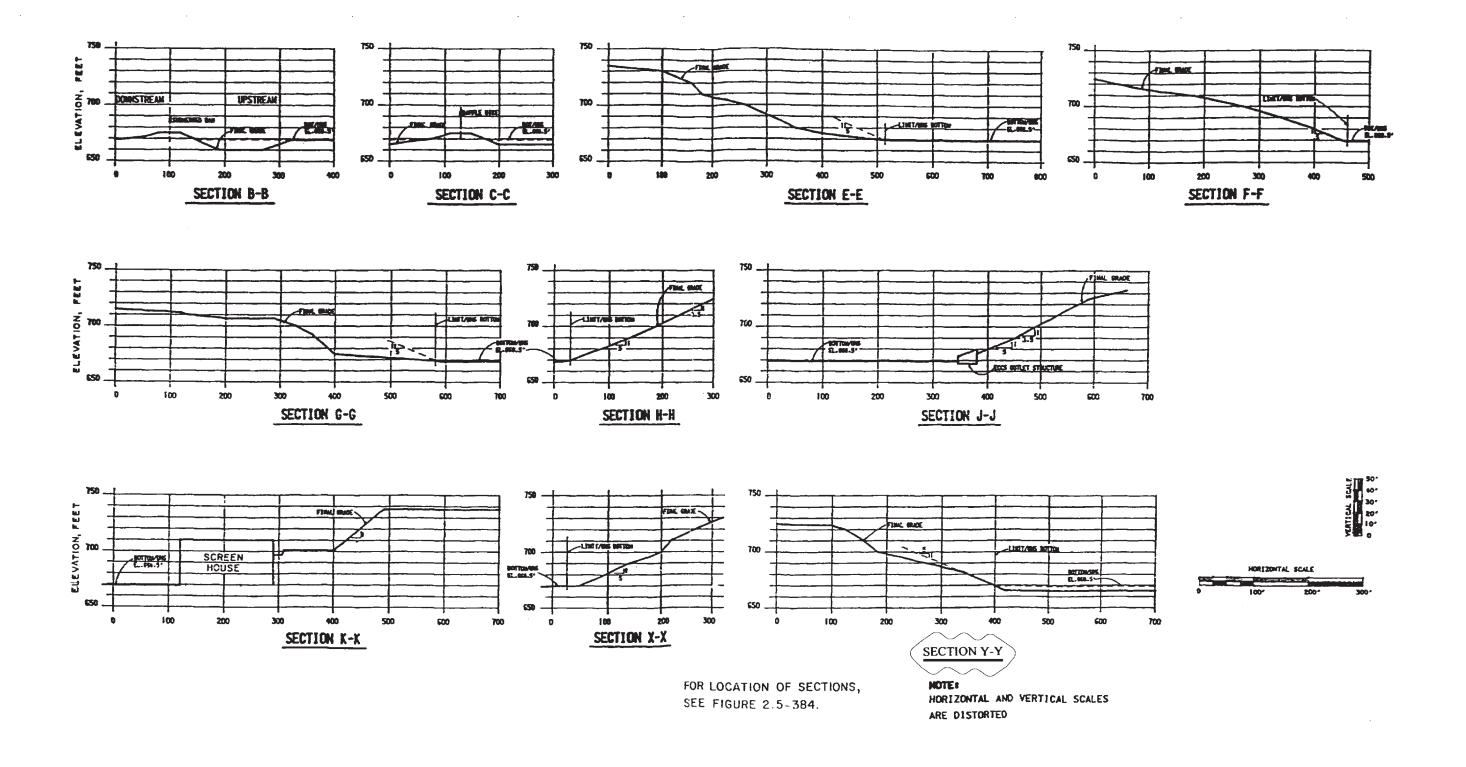
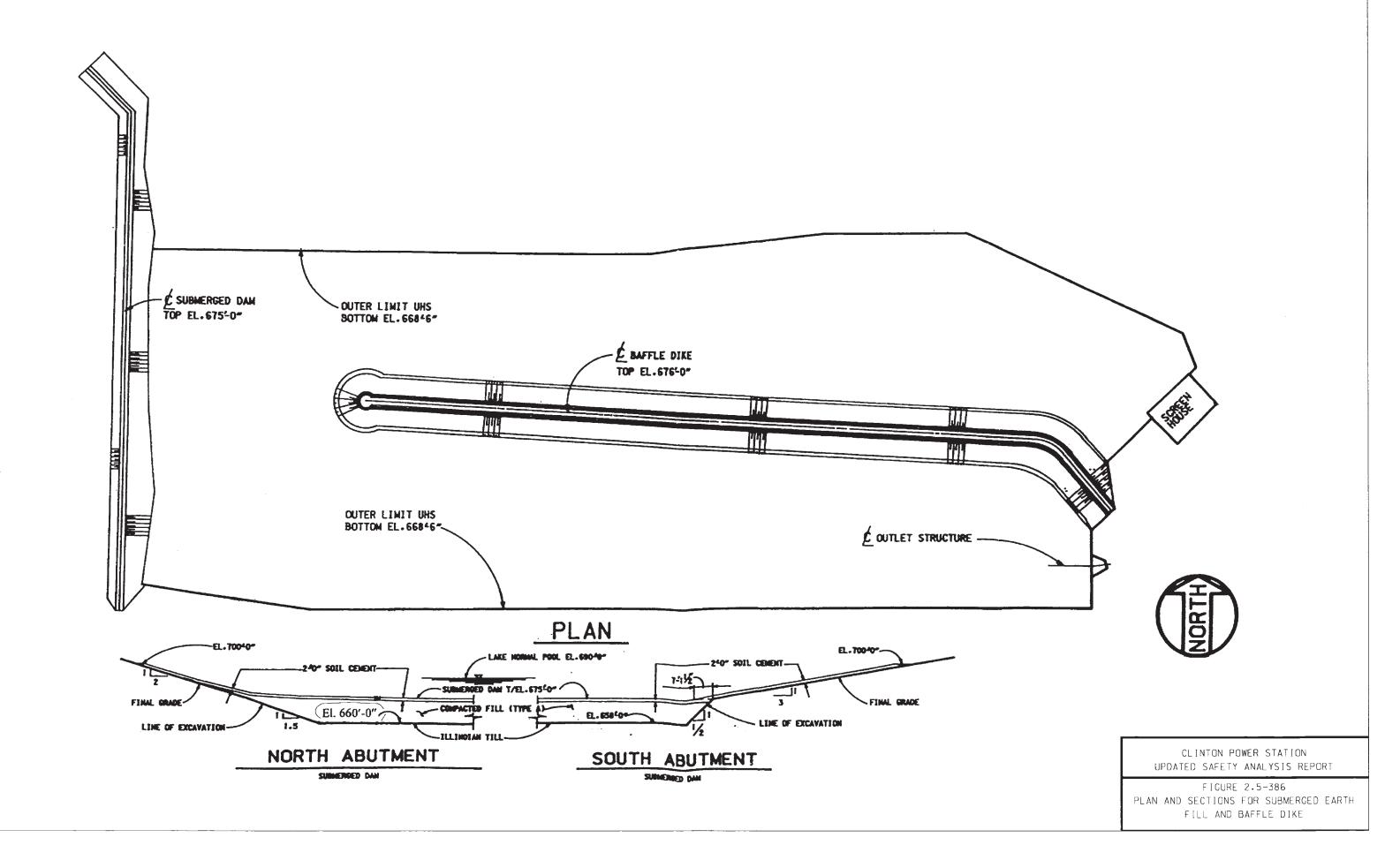
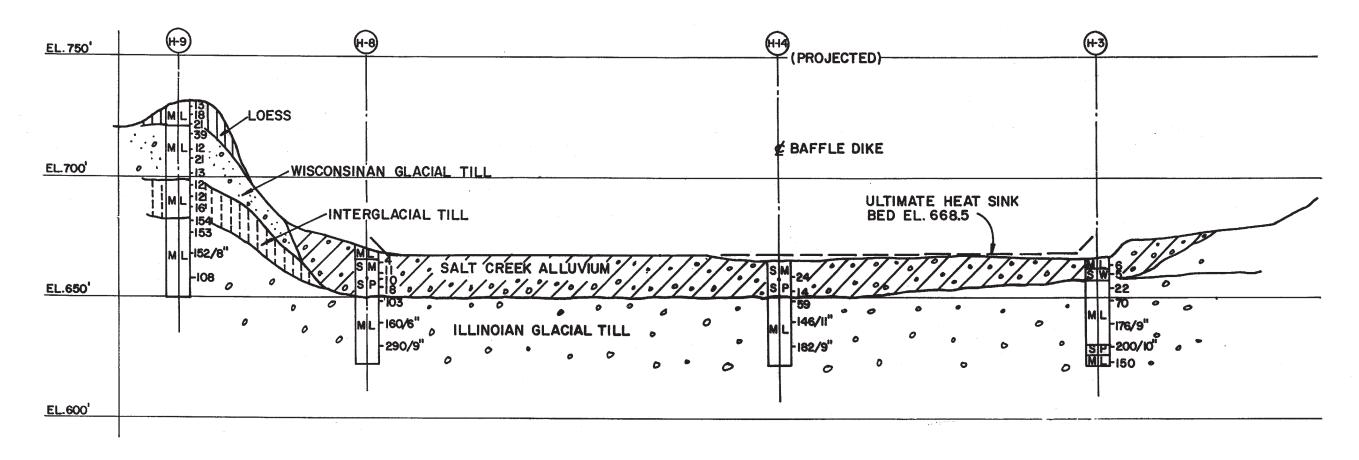
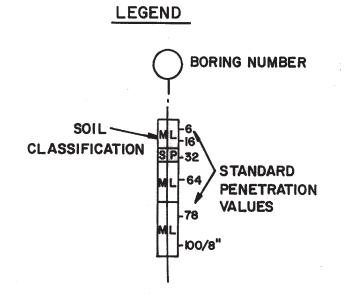


FIGURE 2.5-385

SECTIONS - ULTIMATE HEAT SINK







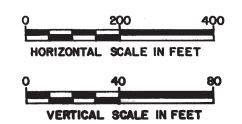
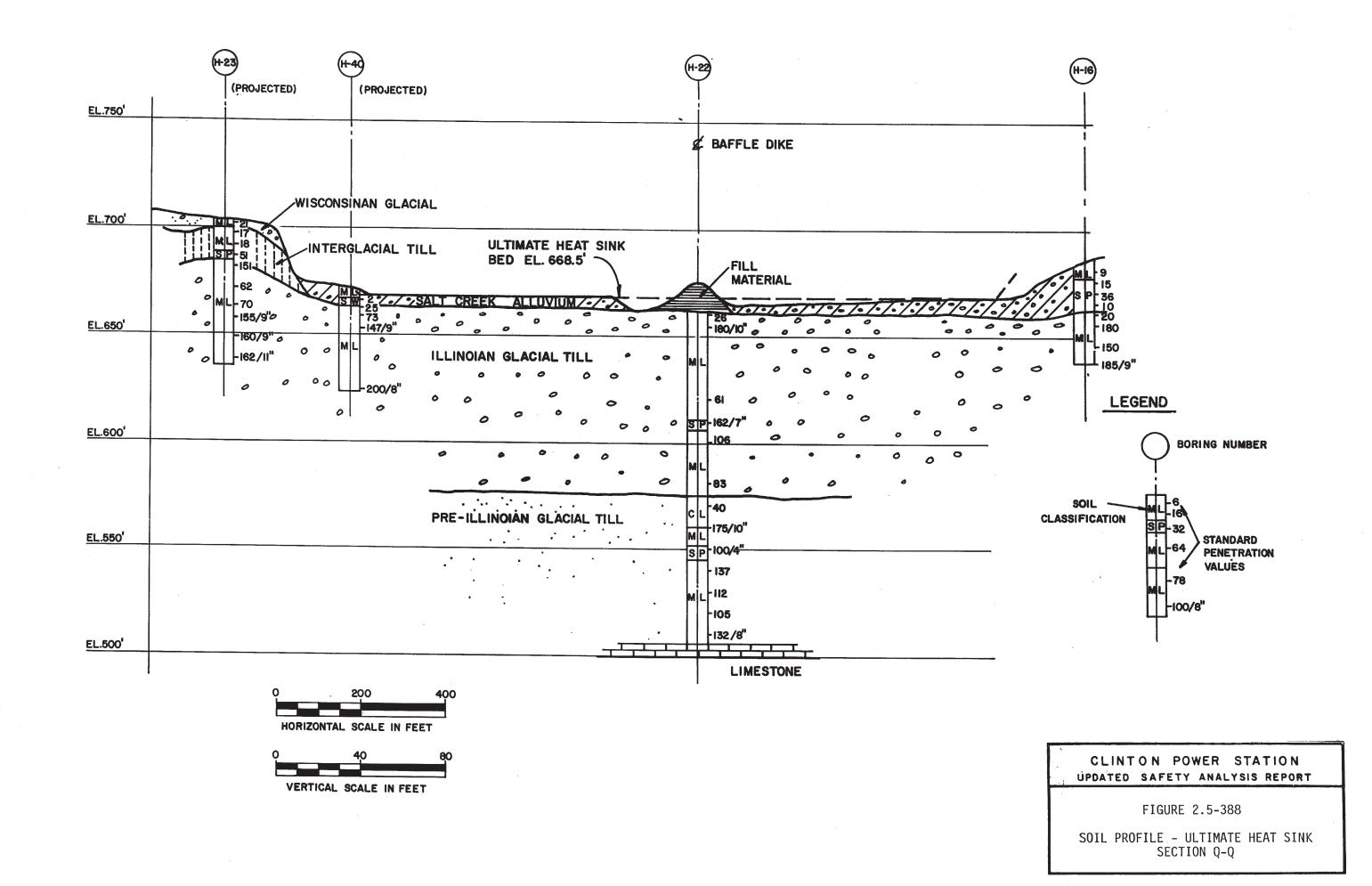
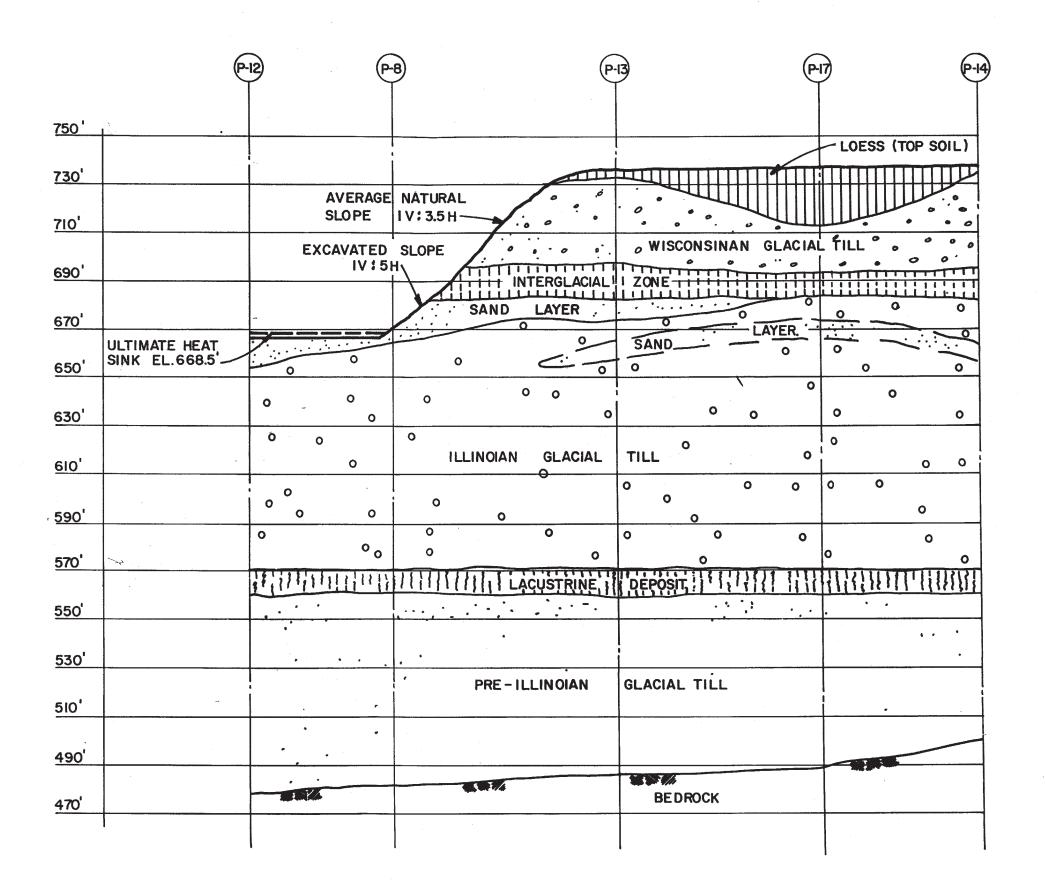


FIGURE 2.5-387

SOIL PROFILE - ULTIMATE HEAT SINK SECTION P-P





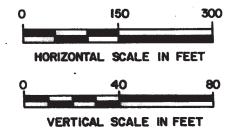
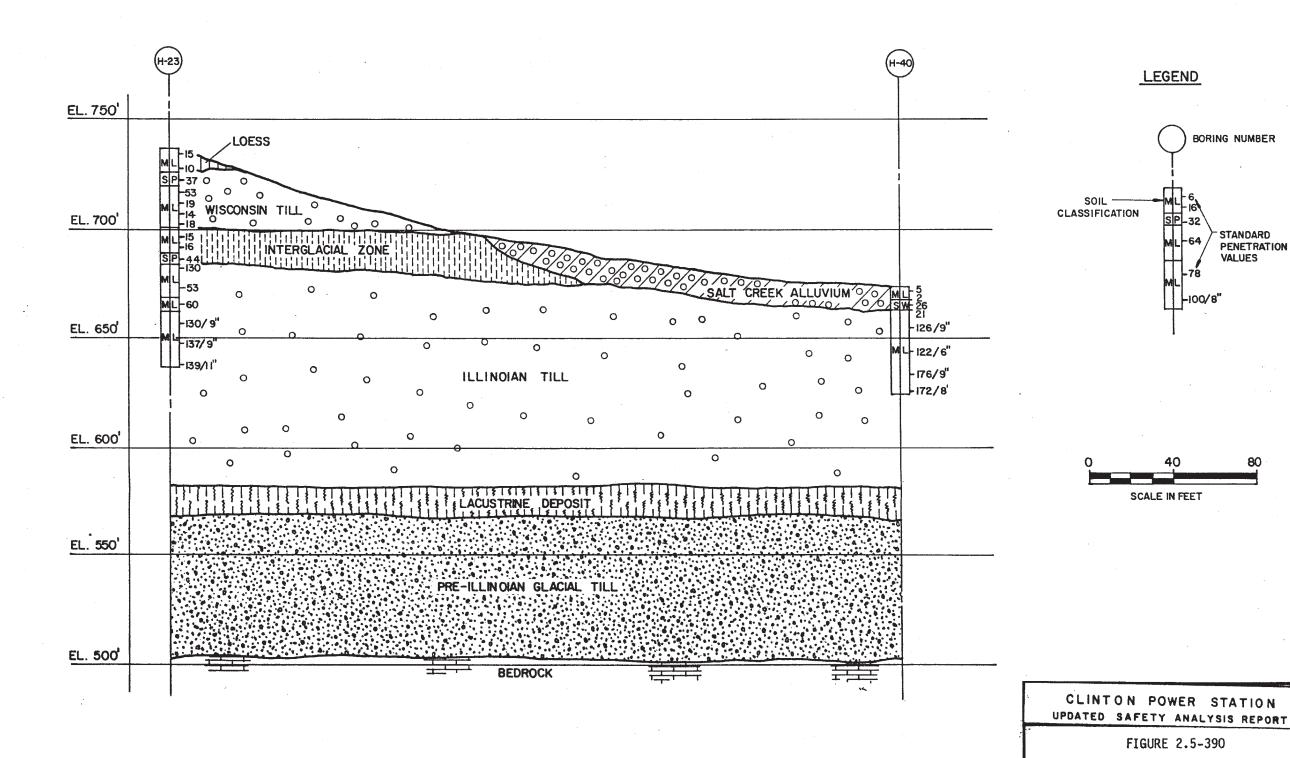


FIGURE 2.5-389

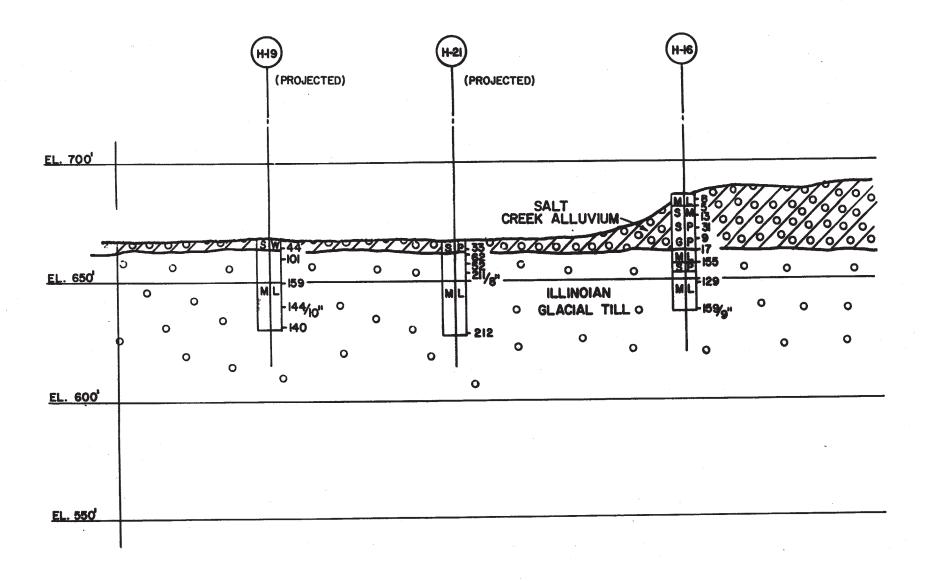
SOIL PROFILE - ULTIMATE HEAT SINK SECTION X-X



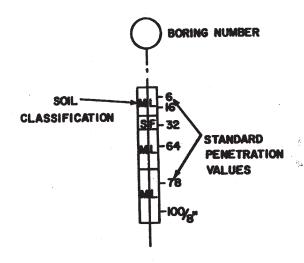
BORING NUMBER

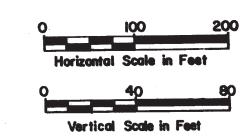
STANDARD
PENETRATION
VALUES

SOIL PROFILE - ULTIMATE HEAT SINK SECTION Y-Y



#### LEGEND

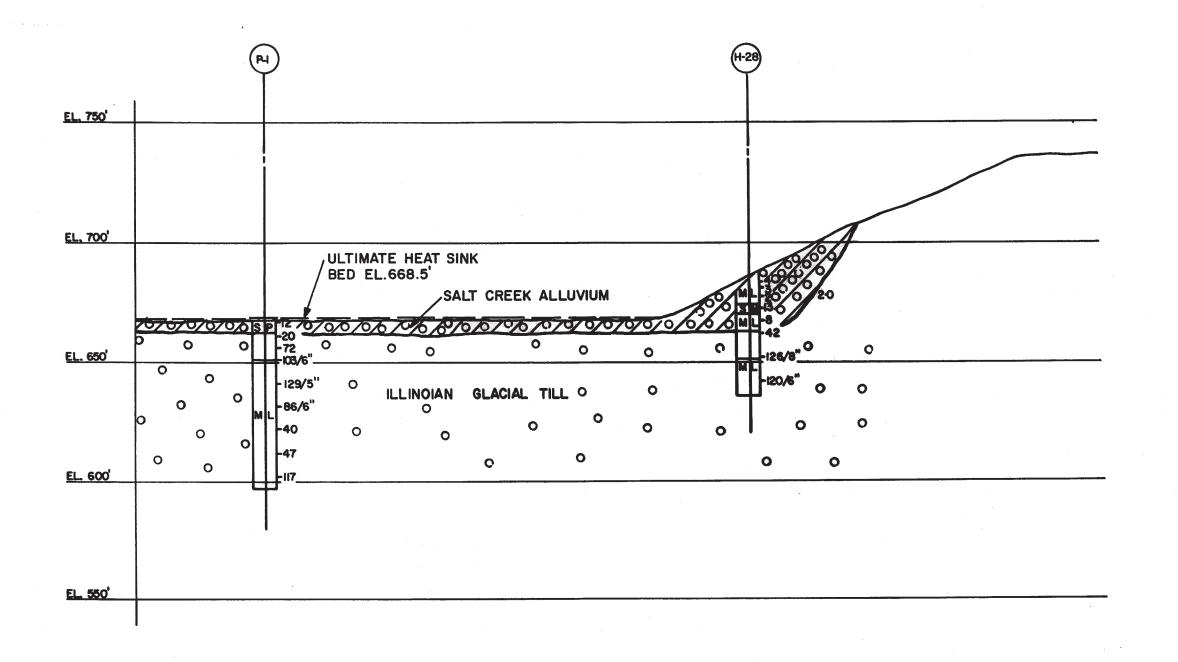




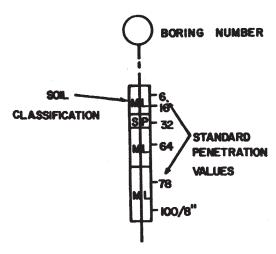
# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

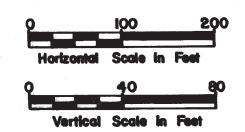
FIGURE 2.5-391

SOIL PROFILE - ULTIMATE HEAT SINK SECTION R-R



#### **LEGEND**





CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-392

SOIL PROFILE - ULTIMATE HEAT SINK SECTION S-S

Γ		s	SOIL DATA					
#	DESCRIPTION	DENSITY (PCF)	C' (PSF)	ø'				
Γ	COMPACTED WISCONSINAN TILL	130	1300#	0*				
2	SALT CREEK ALLUVIUM	120	400	0				
3	SAND DRAINAGE BLANKET	125	0 '	30				
4	SALT CREEK ALLUVIUM (SAND)	125	0	33				
5	ILLINOIAN TILL	140	4000	0				
6	DUMMY LAYER	0	0	0				
7	DUMMY LAYER	0	0	0				

^{*}THESE VALUES ARE TOTAL STRENGTH PARAMETERS

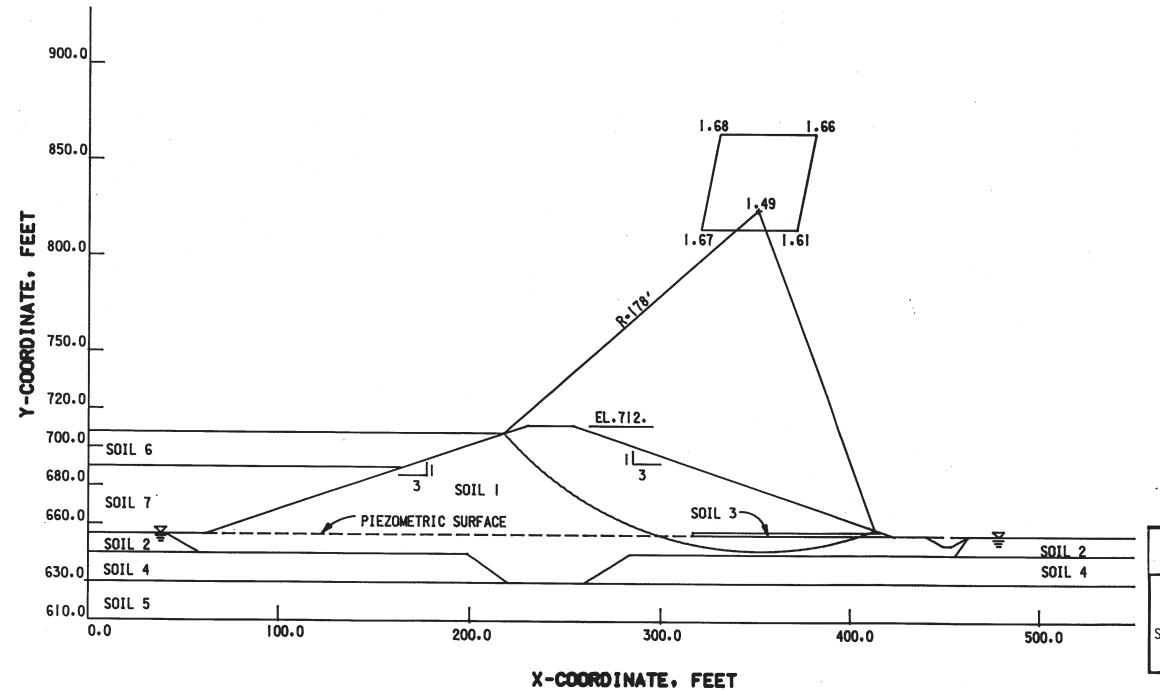


FIGURE 2.5-393

STABILITY ANALYSIS - END OF CONSTRUCTION CONDITION - MAIN DAM

		SOIL DATA					
#	DESCRIPTION	DENSITY (PCF)	C' (PSF)	ø'			
1	COMPACTED WISCONSINAN TILL	130	200	33			
2	SALT CREEK ALLUVIUM	120	400	0			
3	SAND DRAINAGE BLANKET	125	0	30			
4	SALT CREEK ALLUVIUM (SAND)	125	0	33			
5	ILLINOIAN TILL	140	4000	0			
6	DUMMY LAYER	0	0	0			
7	WATER	62.4	0	0			

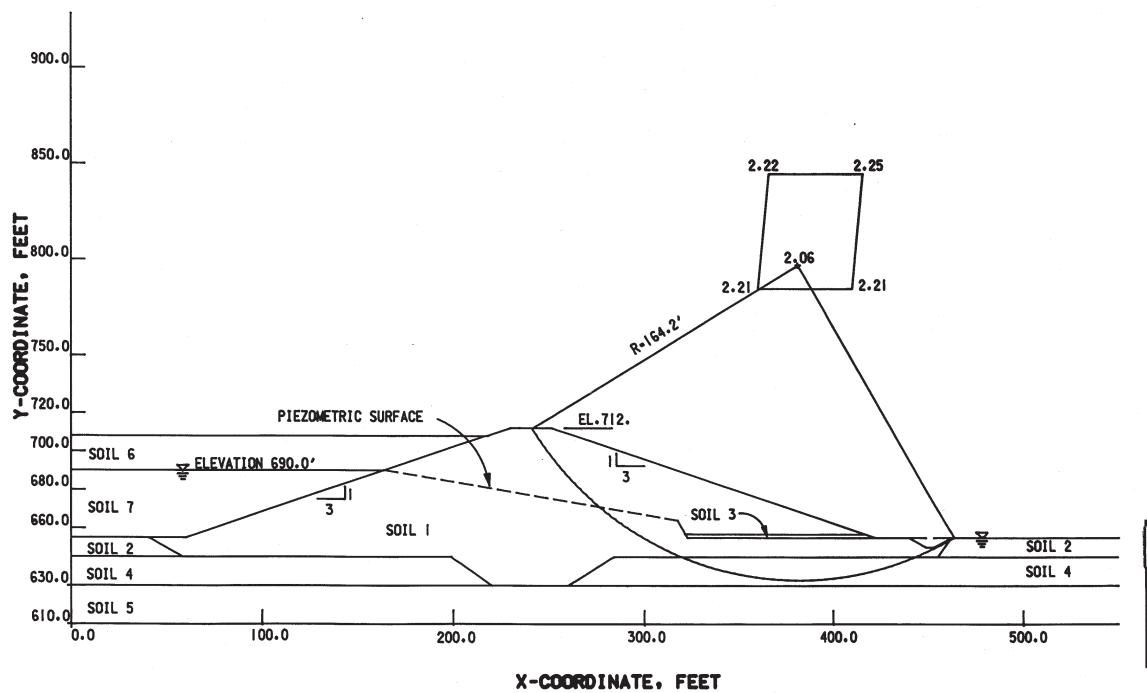


FIGURE 2.5-394

STABILITY ANALYSIS - NORMAL POOL CONDITION - MAIN DAM

		SOIL DATA					
#	DESCRIPTION	DENSITY (PCF)	C' (PSF)	ø'			
1	COMPACTED WISCONSINAN TILL SALT CREEK ALLUVIUM SAND DRAINAGE BLANKET SALT CREEK ALLUVIUM (SAND)	130	200	33			
2		120	400	0			
3		125	0	30			
4		125	0	33			
5	ILLINOIAN TILL	140	4000	0 0			
6	DUMMY LAYER	0	0				
7	WATER	62.4	0				

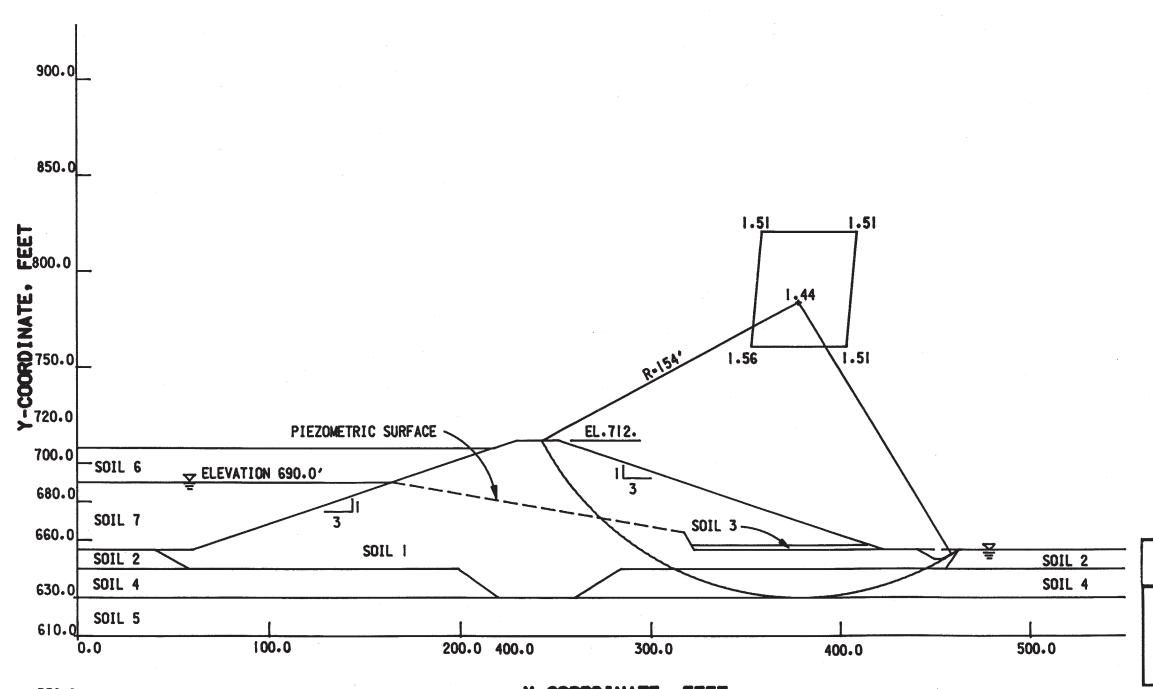


FIGURE 2.5-395

STABILITY ANALYSIS - NORMAL POOL WITH O.1g EARTHQUAKE LOADING CONDITION - MAIN DAM

350.0

X-COORDINATE, FEET

		SOIL DATA					
#	DESCRIPTION	DENSITY (PCF)	C' (PSF)	ø,			
1	COMPACTED WISCONSINAN TILL	130	200	33			
2	SALT CREEK ALLUVIUM	120	400	0			
3	SAND DRAINAGE BLANKET	125	0	30			
4	SALT CREEK ALLUVIUM (SAND)	125	0	33			
5	ILLINOIAN TILL	140	4000	0			
6	WATER	62.4	0	0			
7	WATER	62.4	0	0			

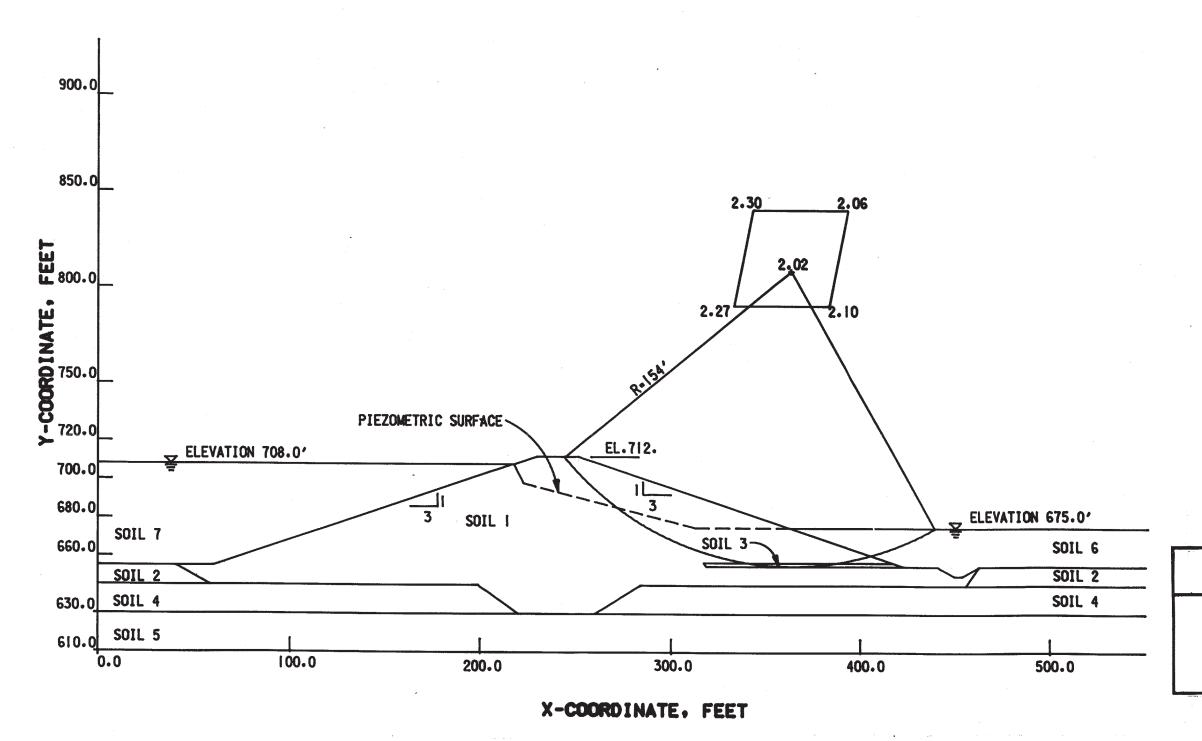


FIGURE 2.5-396

STABILITY ANALYSIS - MAXIMUM POOL CONDITION - MAIN DAM

		SOIL DATA					
#	DESCRIPTION	DENSITY (PCF)	C' (PSF)	ø,			
1	COMPACTED WISCONSINAN TILL	130	200	33			
2	SALT CREEK ALLUVIUM	120	400	0			
3	SAND DRAINAGE BLANKET	125	0	30			
4	SALT CREEK ALLUVIUM (SAND)	125	0	33			
5	ILLINOIAN TILL	140	4000	0			
6	DUMMY LAYER	0	0	0			
7	DUMMY LAYER	0	0	0			

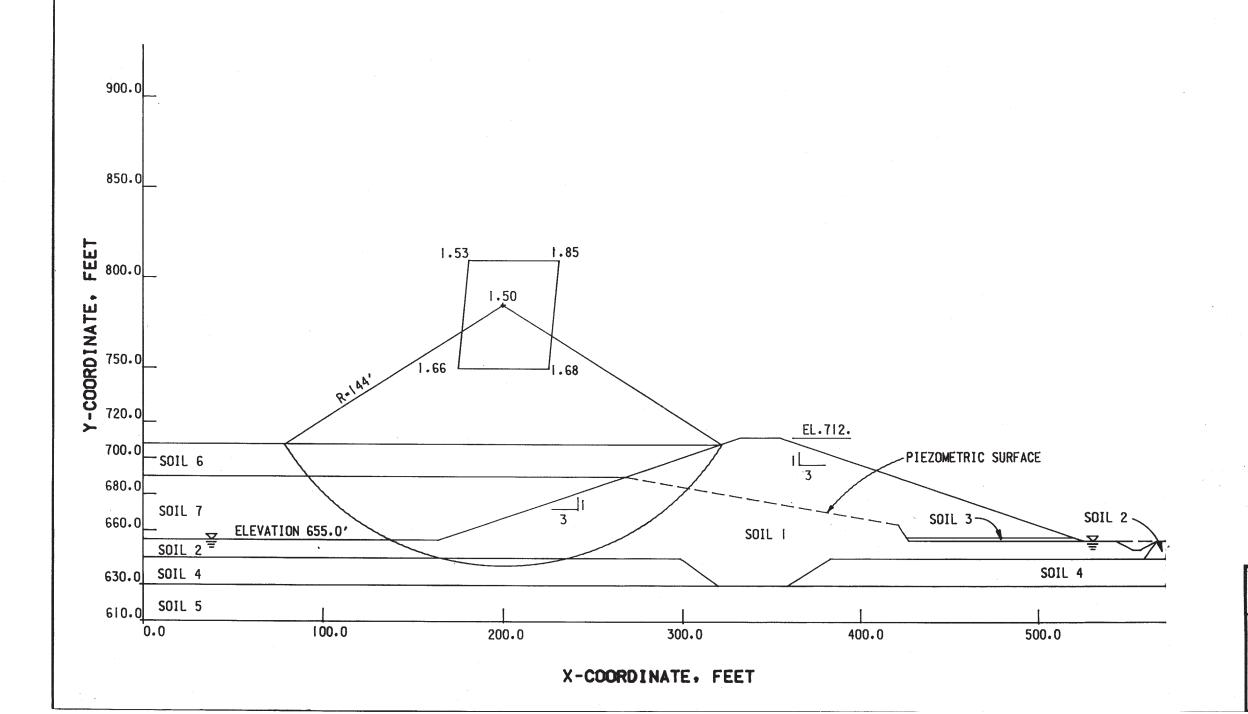
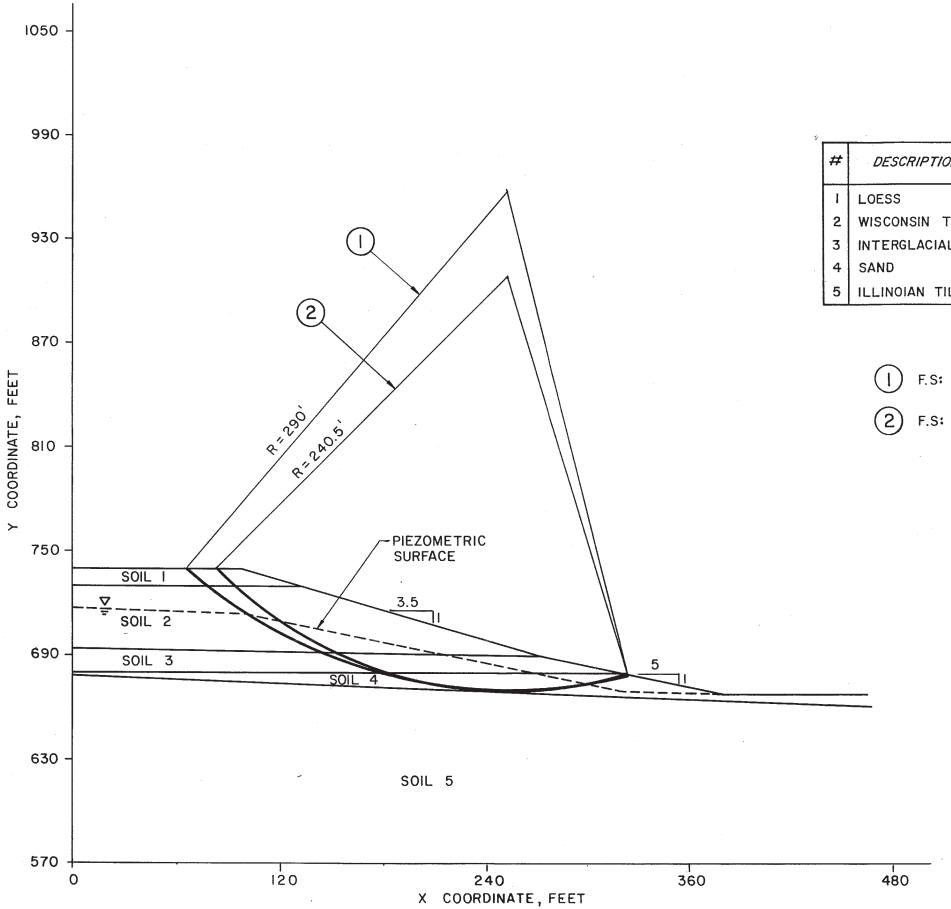
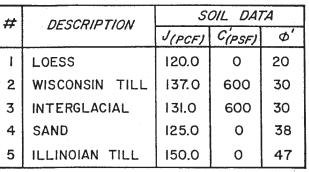


FIGURE 2.5-397

STABILITY ANALYSIS - RAPID DRAWDOWN CONDITION - MAIN DAM

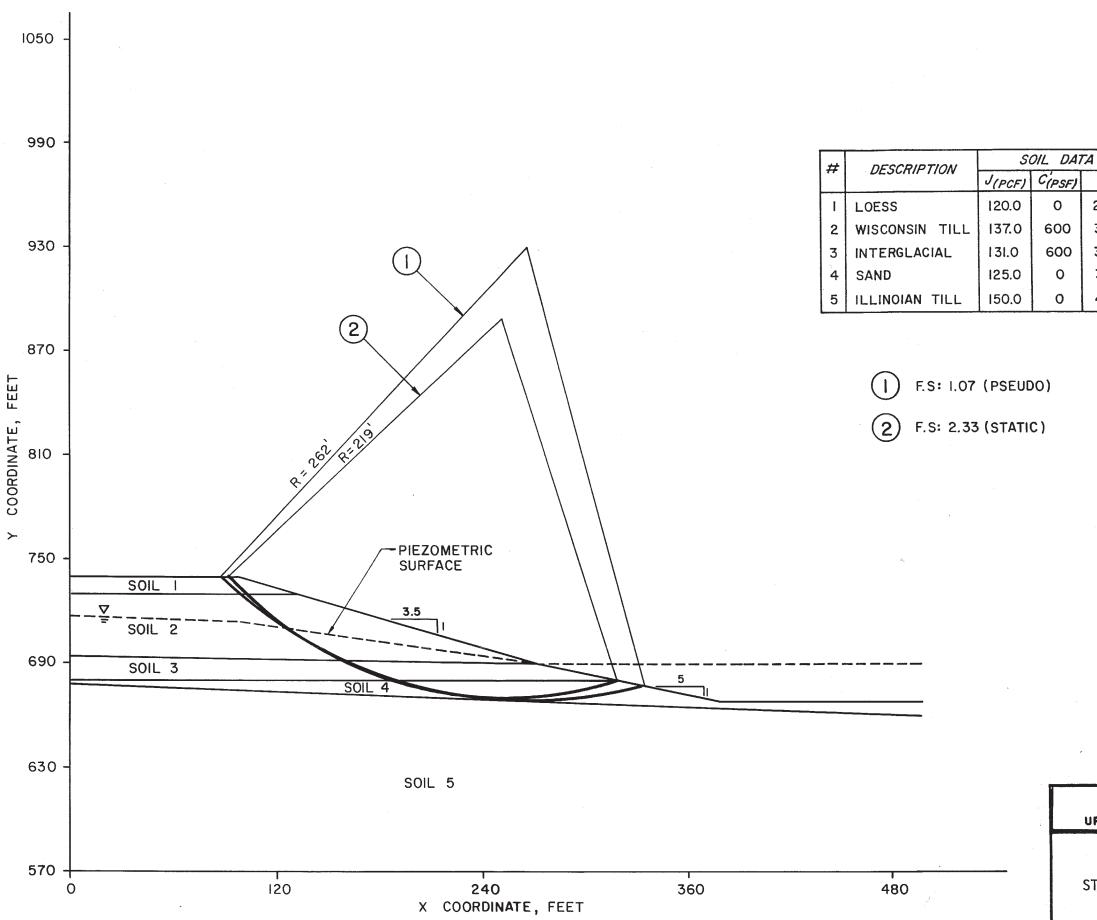




- () F.S: 1.24 (PSEUDO)
- 2) F.S: 2.60 (STATIC)

FIGURE 2.5-398

STABILITY ANALYSIS - END OF CONSTRUCTION CONDITION - SECTION X-X,
ULTIMATE HEAT SINK



ø'

20

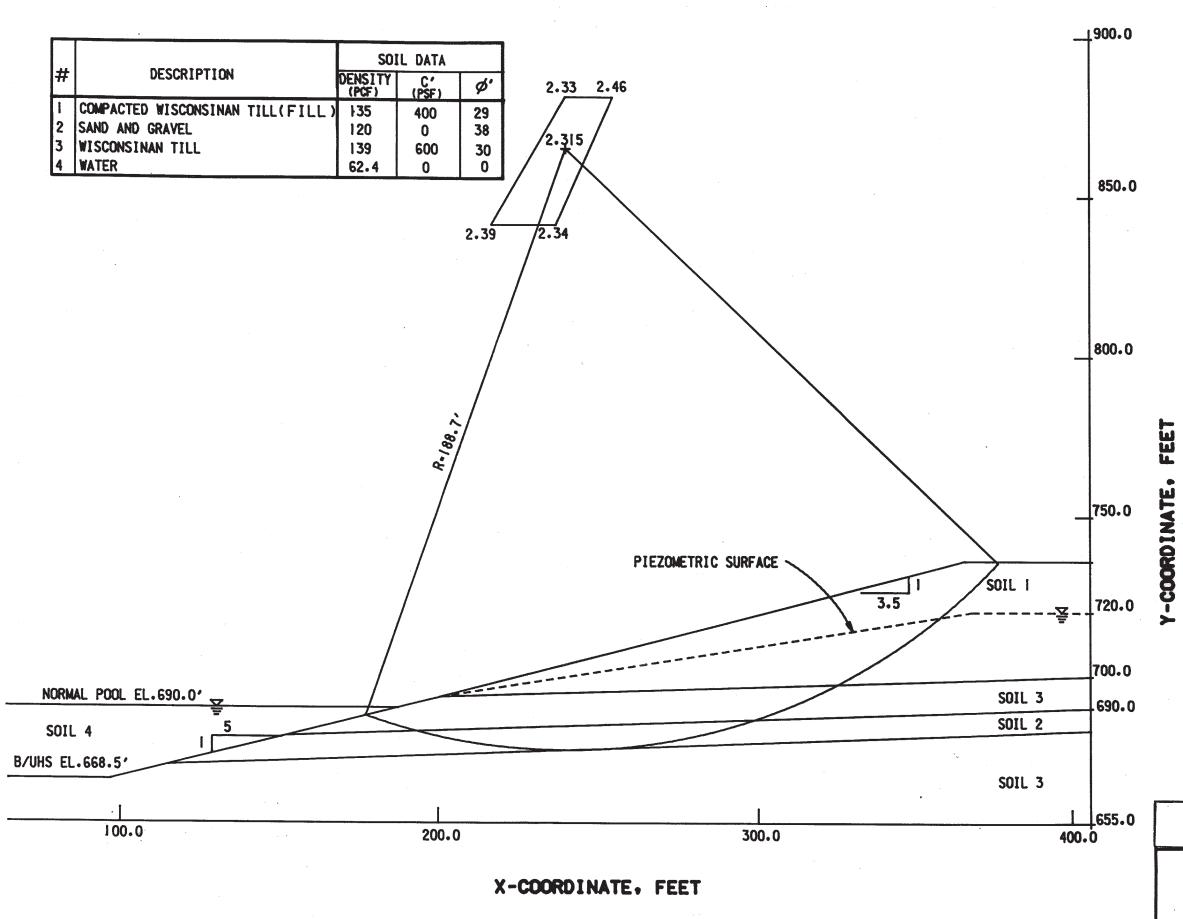
30

30 38

47

FIGURE 2.5-399

STABILITY ANALYSIS - FULL COOLING LAKE CONDITION - SECTION X-X, ULTIMATE HEAT SINK



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-400

STABILITY ANALYSIS - FULL COOLING LAKE CONDITION - SECTION H-H', ULTIMATE HEAT SINK

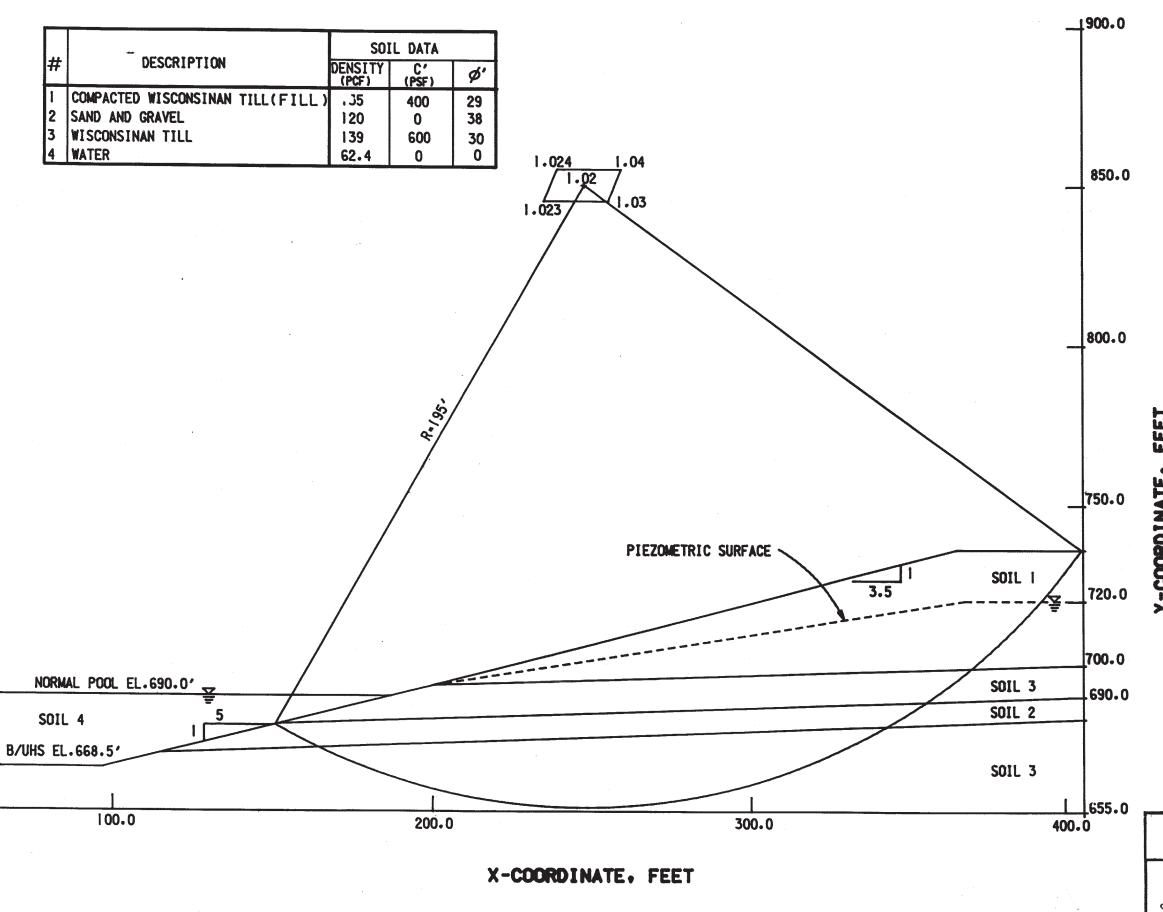
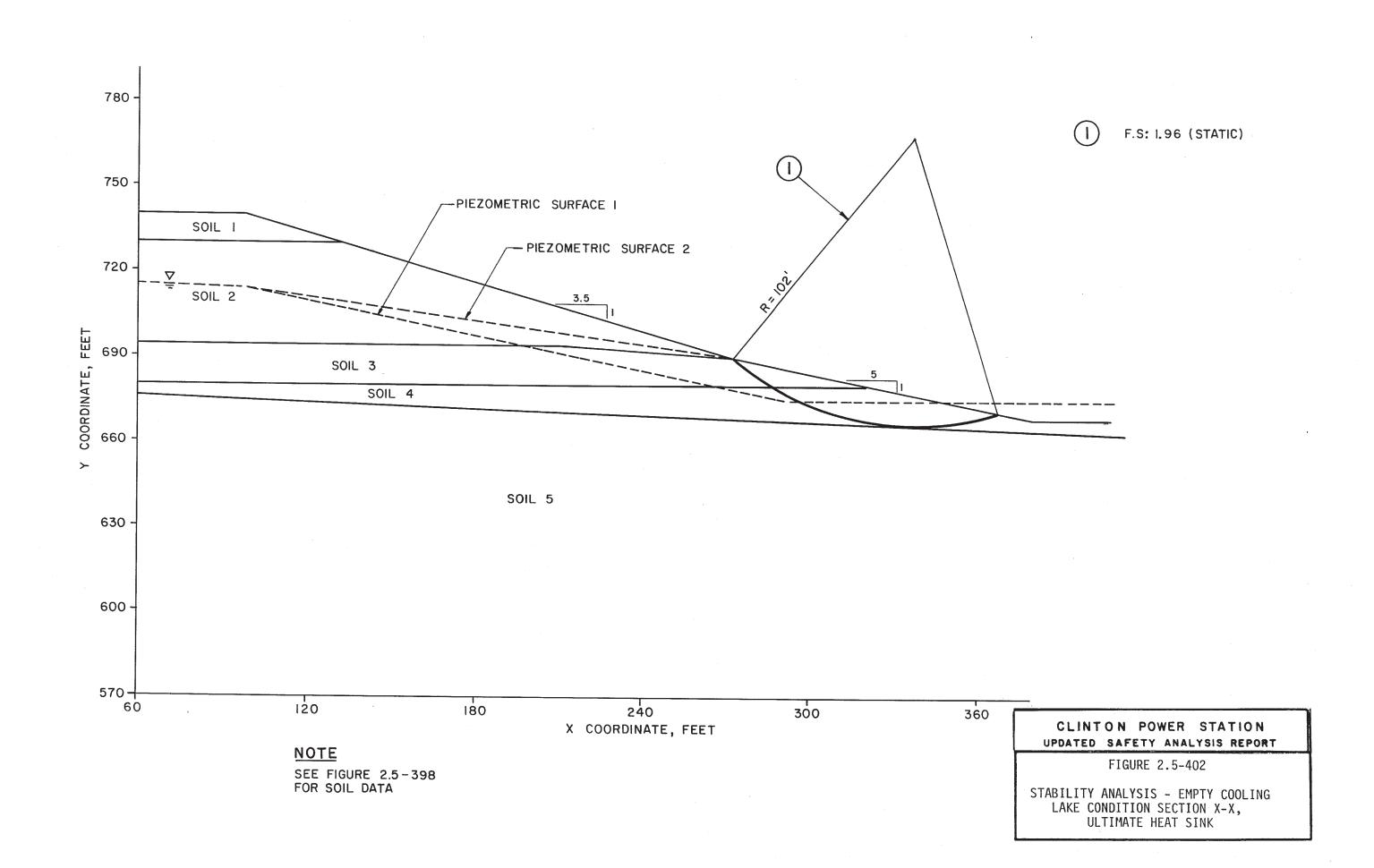
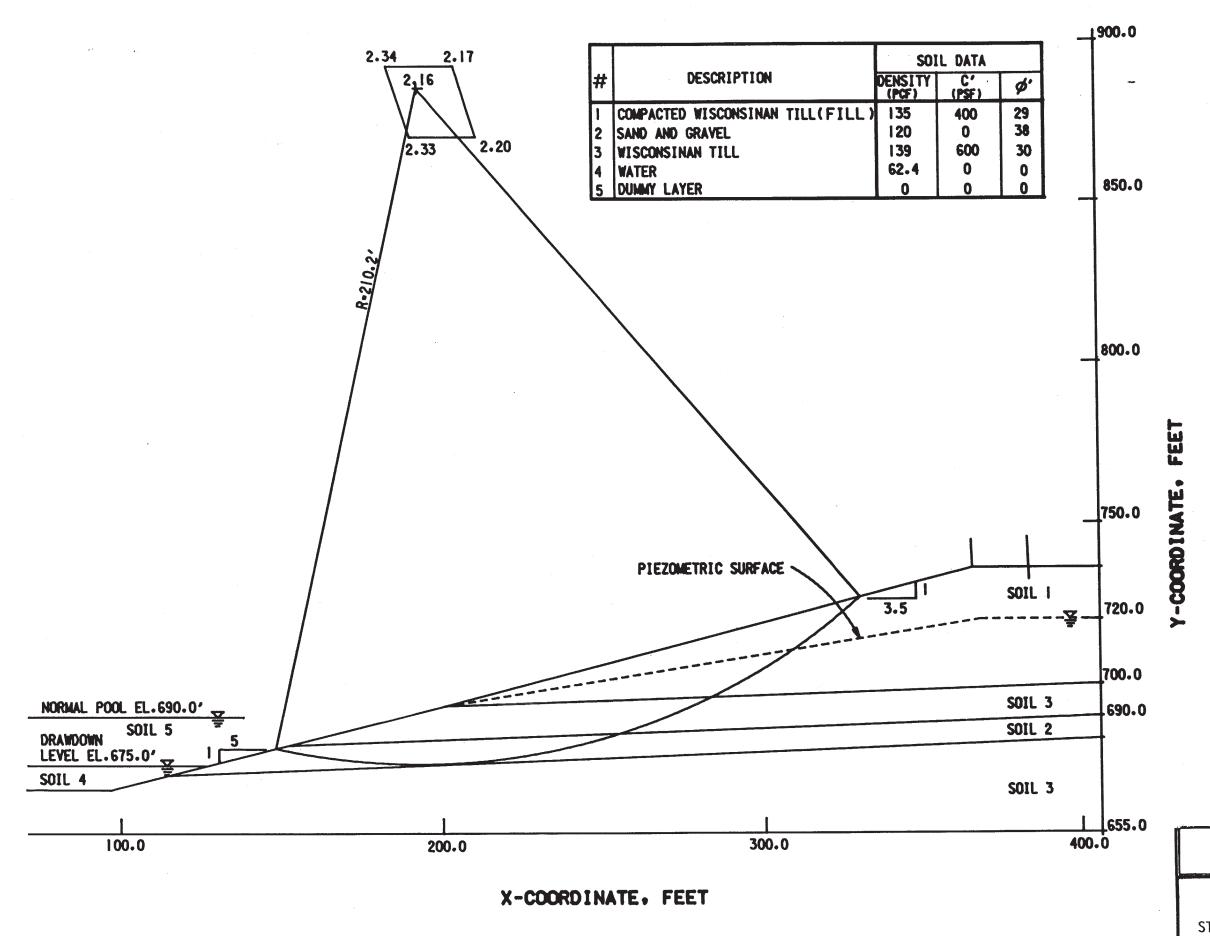


FIGURE 2.5-401

STABILITY ANALYSIS - FULL COOLING LAKE WITH 0.25g EARTHQUAKE LOADING CONDITION SECTION H-H, ULTIMATE HEAT SINK





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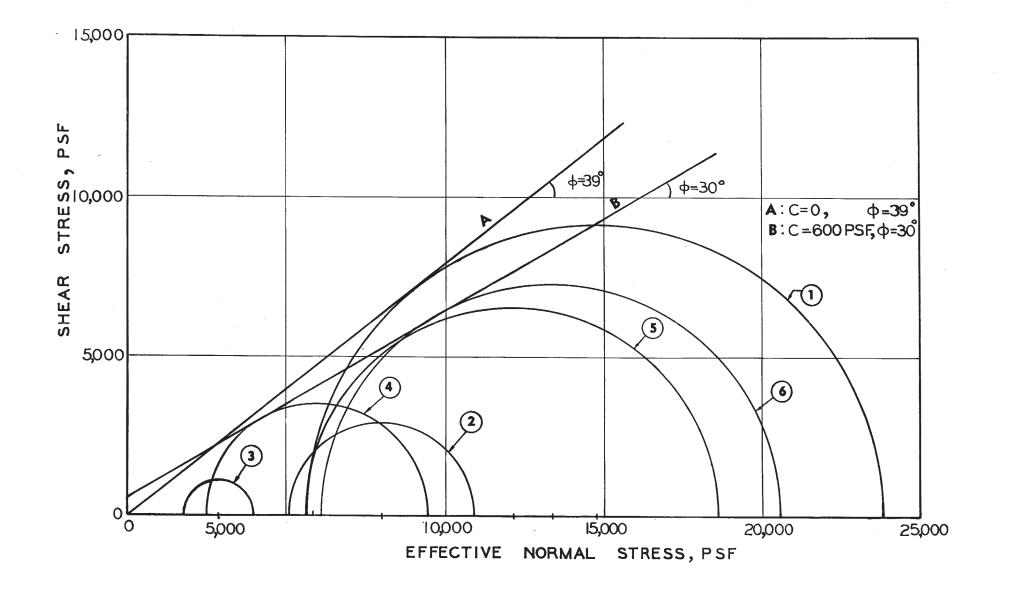
FIGURE 2.5-403

STABILITY ANALYSIS - EMPTY COOLING
LAKE CONDITION SECTION H-H,
ULTIMATE HEAT SINK

#### TRIAXIAL COMPRESSION TESTS

CONSOLIDATED UNDRAINED WITH PORE PRESSURE MEASUREMENTS

KEY	BORING	ELEVATION	SOIL TYPE	BLOW COUNTS/FT.	DRY DENSITY IN PCF
1	H-20	721.8	SP	85	130.3
2	H-38	712.9	ML	20	123.5
3	D-48	709.3	CL	9	123.0
4	H-23	707.3	ML	14	121.4
5	D-48	704.3	CL	21	123.8
6	D-48	689.3	CL	17	108.8



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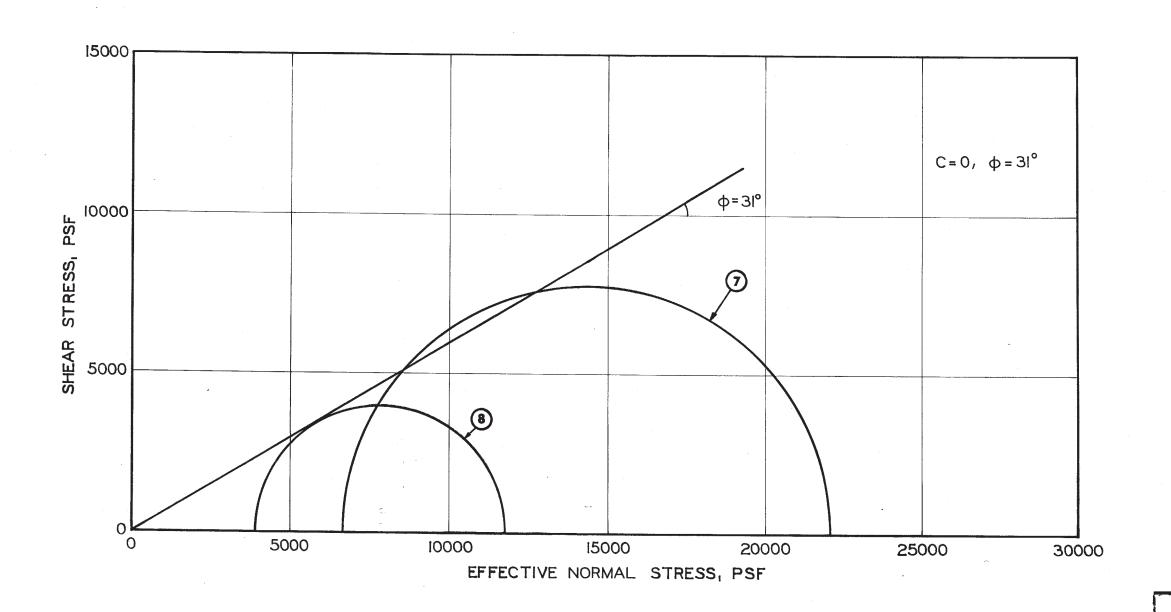
FIGURE 2.5-404

MOHR CIRCLES - WISCONSINAN GLACIAL TILL

#### TRIAXIAL COMPRESSION TESTS

CONSOLIDATED-UNDRAINED WITH PORE PRESSURE MEASUREMENTS

TEST NO.	BORING NO.	ELEVATION	SOIL TYPE	BLOW COUNT/FT.	DRY DENSITY IN PCF
7	H-23	692.3	ML	15	103
8	H-38	687.9	ML	8	_



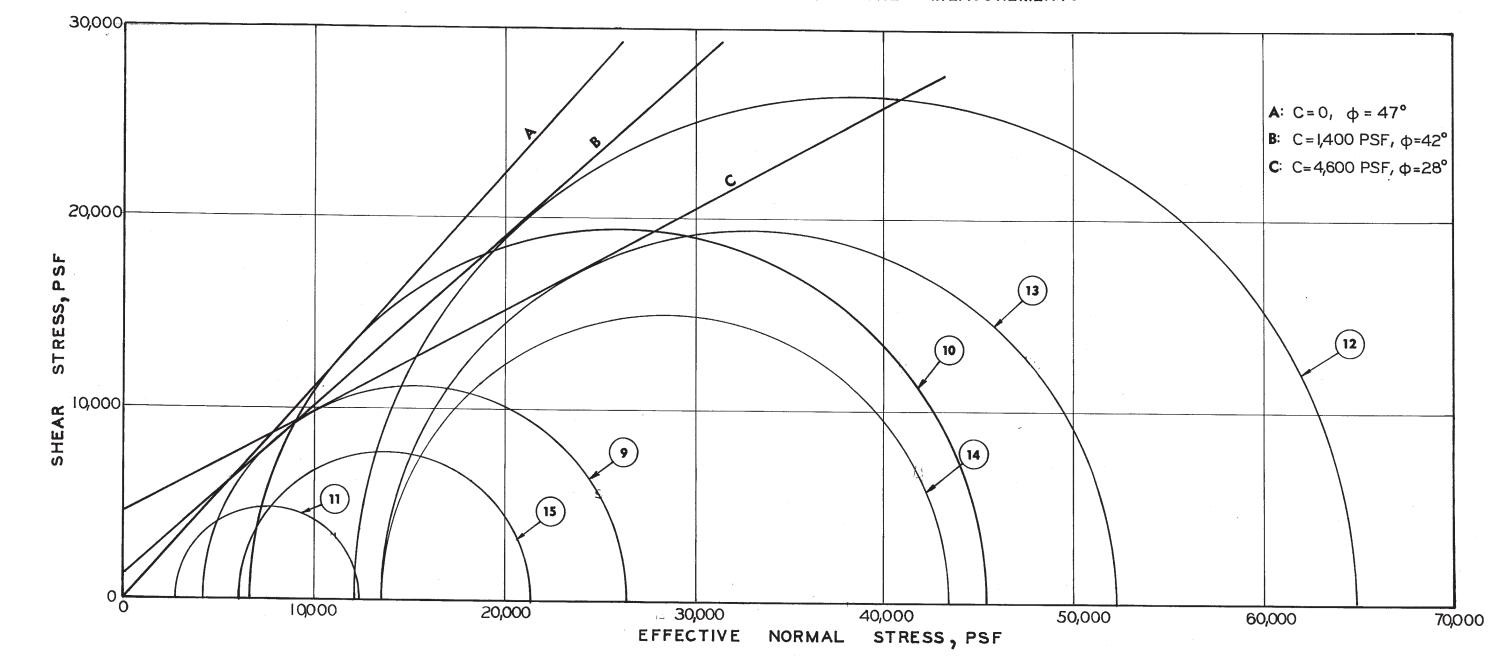
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FIGURE 2.5-405

MOHR CIRCLES - INTERGLACIAL TILL

TRIAXIAL COMPRESSION TESTS

CONSOLIDATED - UNDRAINED WITH PORE PRESSURE MEASUREMENTS



KEY	BORING	ELEVATION	SOIL TYPE	BLOW COUNTS/FT	DRY DENSITY
9	H-38	673.4	ML	154/11"	123.5
10	P-38	648.5	ML	100/6"	138
П	H-3	645.1	ML	60	139
12	H-25	633.7	ML	100/4"	146
13	D-8	631.7	ML	94	135.2
14	D-8	591.7	ML	83	132
15	H- 6	504.3	ML	31	109

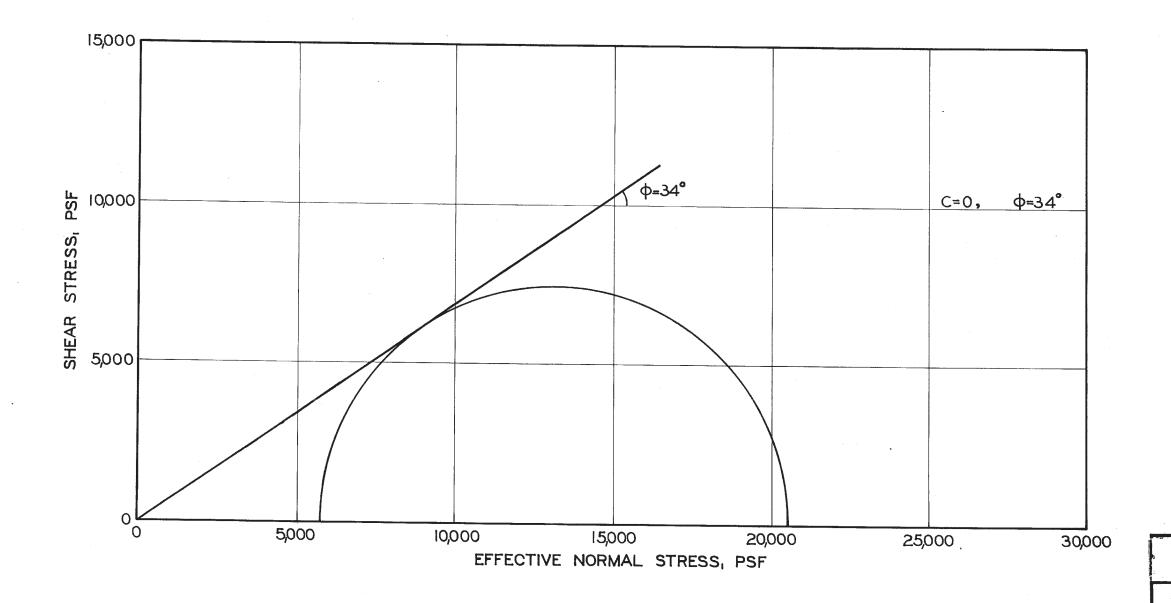
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-406

MOHR CIRCLES - ILLINOIAN GLACIAL TILL

# TRIAXIAL COMPRESSION TEST CONSOLIDATED-UNDRAINED WITH PORE PRESSURE MEASUREMENTS

KEY	BORING	ELEVATION	SOIL TYPE	BLOW COUNTS/FT.	DRY DENSITY
16	P-38	572.9	ML	48	125.9



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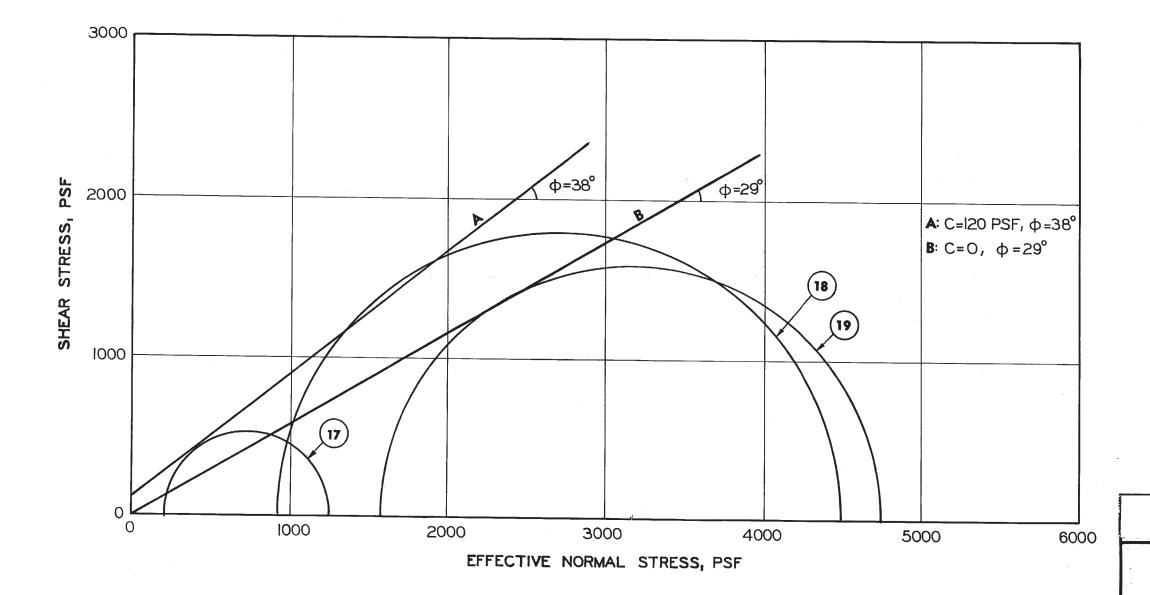
FIGURE 2.5-407

MOHR CIRCLE - LACUSTRINE DEPOSITS

#### TRIAXIAL COMPRESSION TESTS

CONSOLIDATED-UNDRAINED WITH PORE PRESSURE MEASUREMENTS

KEY	BORING	ELEVATION	SOIL TYPE	BLOW COUNT /FT.	DRY DENSITY
.17	H-25	674.7	ML	3	_
18	H-13	673.6	ML	6	102
19	H-24	670.7	SM	2	_



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-408

MOHR CIRCLES - SALT CREEK ALLUVIUM

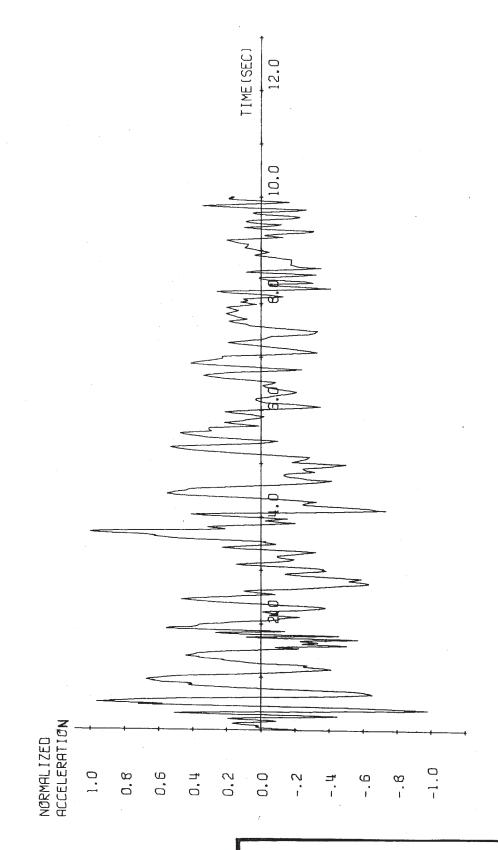


FIGURE 2.5-409

ARTIFICIAL ACCELEROGRAM FOR HORIZONTAL GROUND MOTION

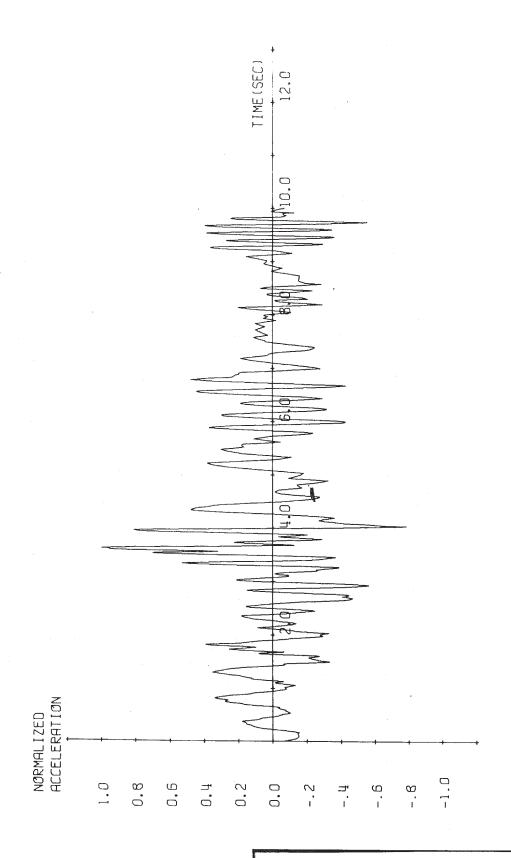
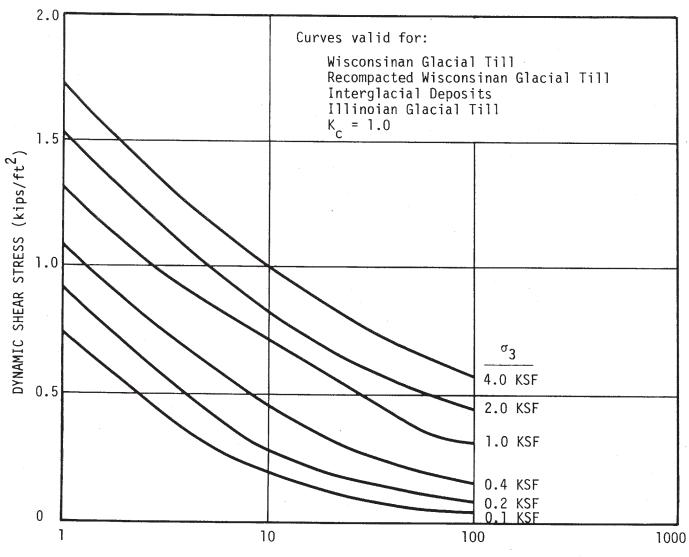


FIGURE 2.5-410

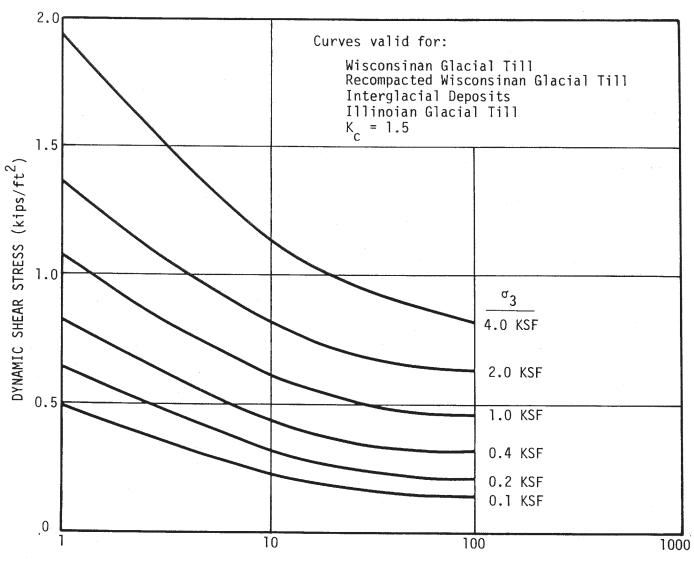
ARTIFICIAL ACCELEROGRAM FOR VERTICAL GROUND MOTION



NUMBER OF CYCLES TO CAUSE 5% STRAIN

FIGURE 2.5-411

DYNAMIC SHEAR STRESS VS. NUMBER OF CYCLES TO CAUSE 5% STRAIN -  $K_{\rm C}$  = 1.0



NUMBER OF CYCLES TO CAUSE 5% STRAIN

FIGURE 2.5-412

DYNAMIC SHEAR STRESS VS. NUMBER OF CYCLES TO CAUSE 5% STRAIN -  $K_{\rm C}$  = 1.5

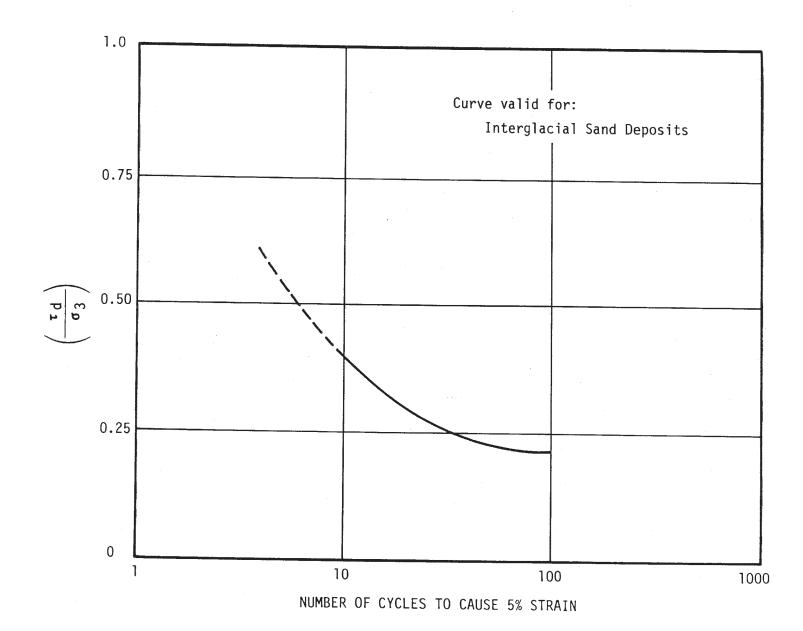


FIGURE 2.5-413

DYNAMIC SHEAR STRESS TO MINOR PRINCIPAL STRESS RATIO VS. NUMBER OF CYCLES TO CAUSE 5% STRAIN

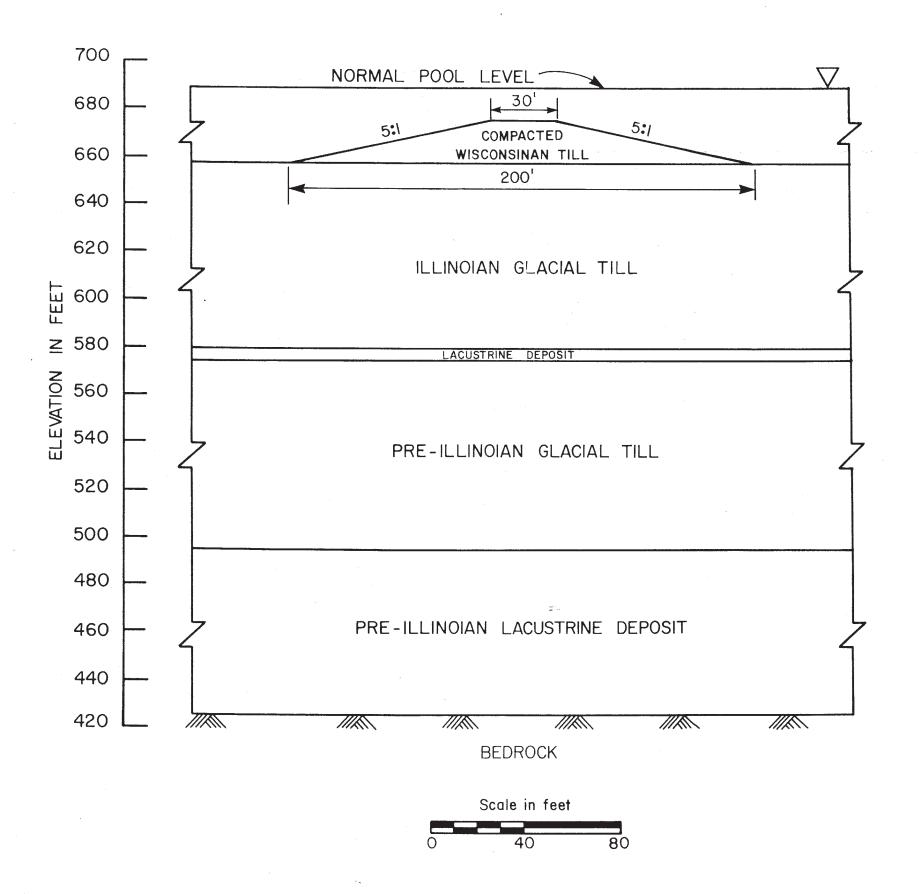


FIGURE 2.5-414

SUBMERGED DIKE - CROSS SECTION ANALYZED FOR SEISMIC STABILITY

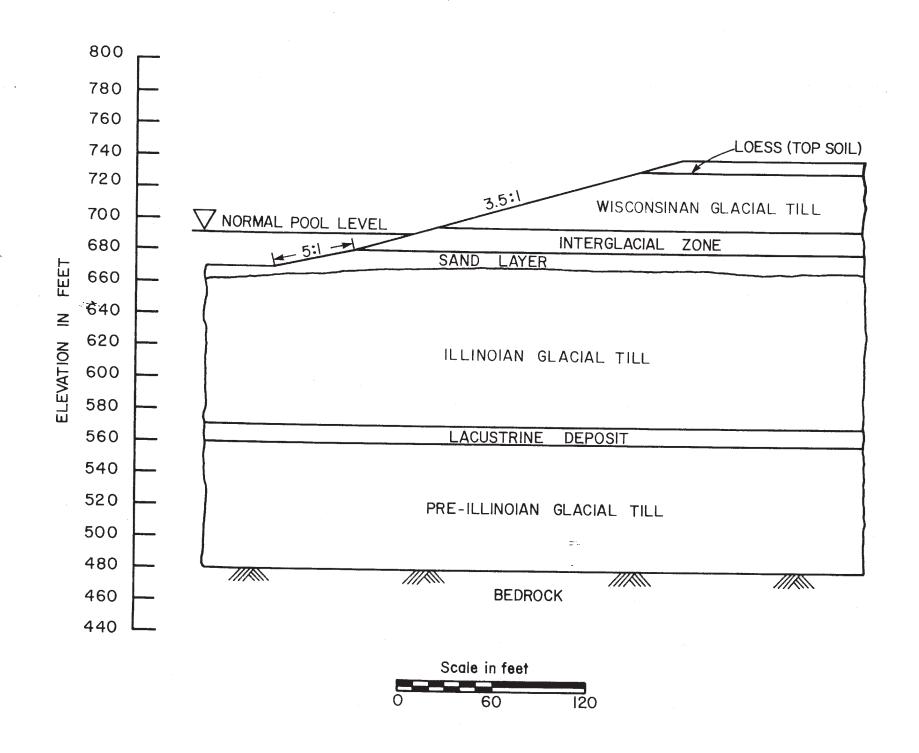


FIGURE 2.5-415

NATURAL SLOPE - CROSS SECTION ANALYZED FOR SEISMIC STABILITY

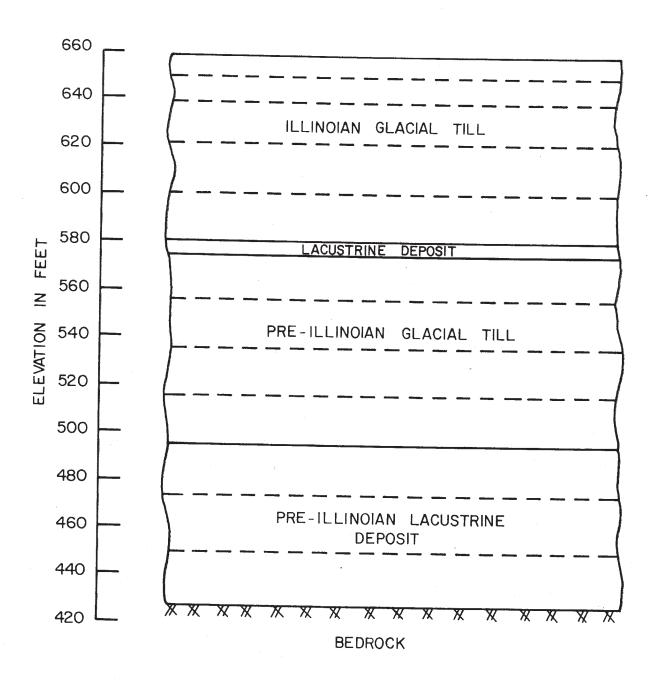
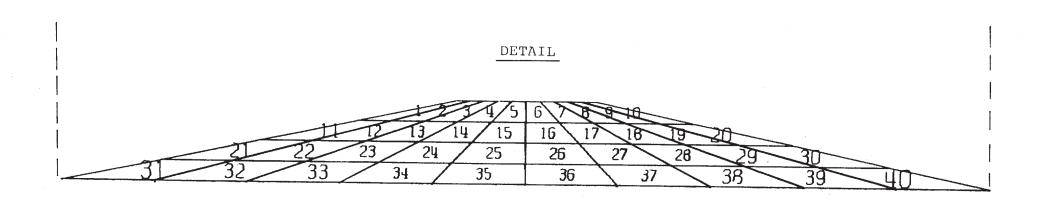


FIGURE 2.5-416

SHEAR LAYER MODEL FOR THE SUBMERGED DIKE FOUNDATION

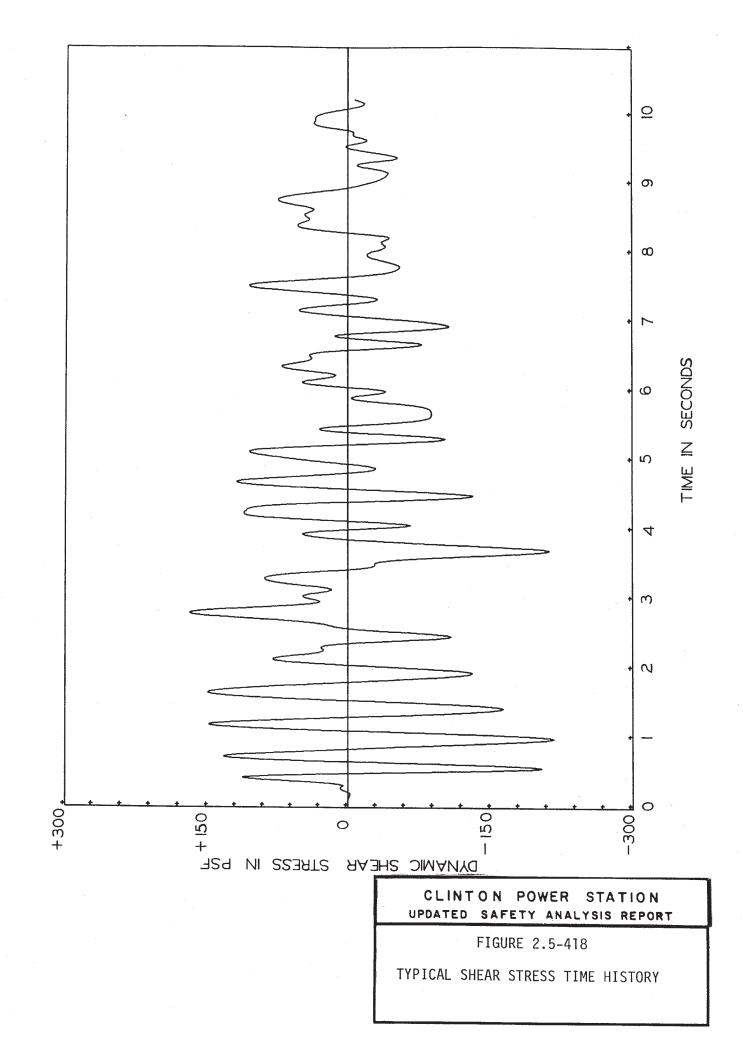


			-	S	ee D	Deta	ail —				
			اً ا					53 54	<u> </u>		
41 59	45	43 44 61 62	45 46 63 64	47 48 65 66	項 67	50 68	51 52	53 54	55 56	5,7	58
77	78		82 84 83	85 87 86	88	69	999291	93 95 94	96 98 27	99	76 100
101	102	103	104	105	106			109	110	111	112
113	114	115	116		118		120	121	122		124
137	126 138	115 127 139	126 140		1 <b>30</b> 142		132 144	1 <b>33</b> 145	134 146	123 135 147	136 1 1 P
149	150	151	152	153	154	155	156	157	158	159	
	162	163	164	165	166	167	166	169	170	171	173
1/3	174	175	176	177	176	179	160	181	182	183	180
185	186	187	188	169	190	191	192	193	194	195	183
197	198	199	200	201	202	203	204	205	206	207	218 -
209	210	211	212	213	214	215	216	217	218	219	538

SCALE= 1./ 66.667

FIGURE 2.5-417

FINITE ELEMENT MODEL FOR THE DIKE



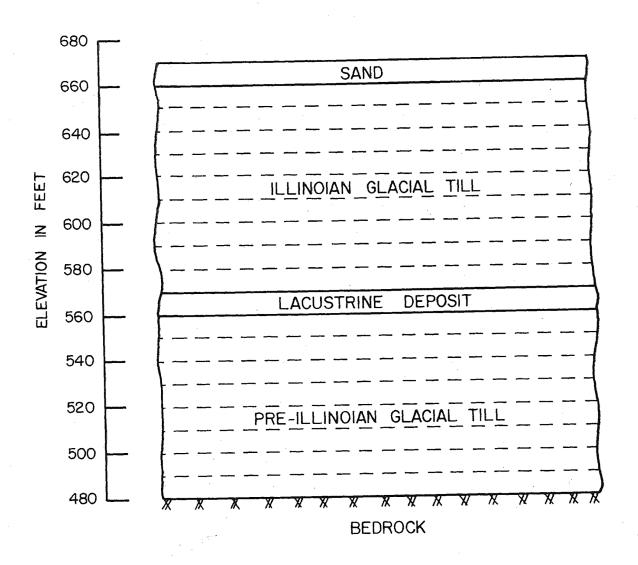


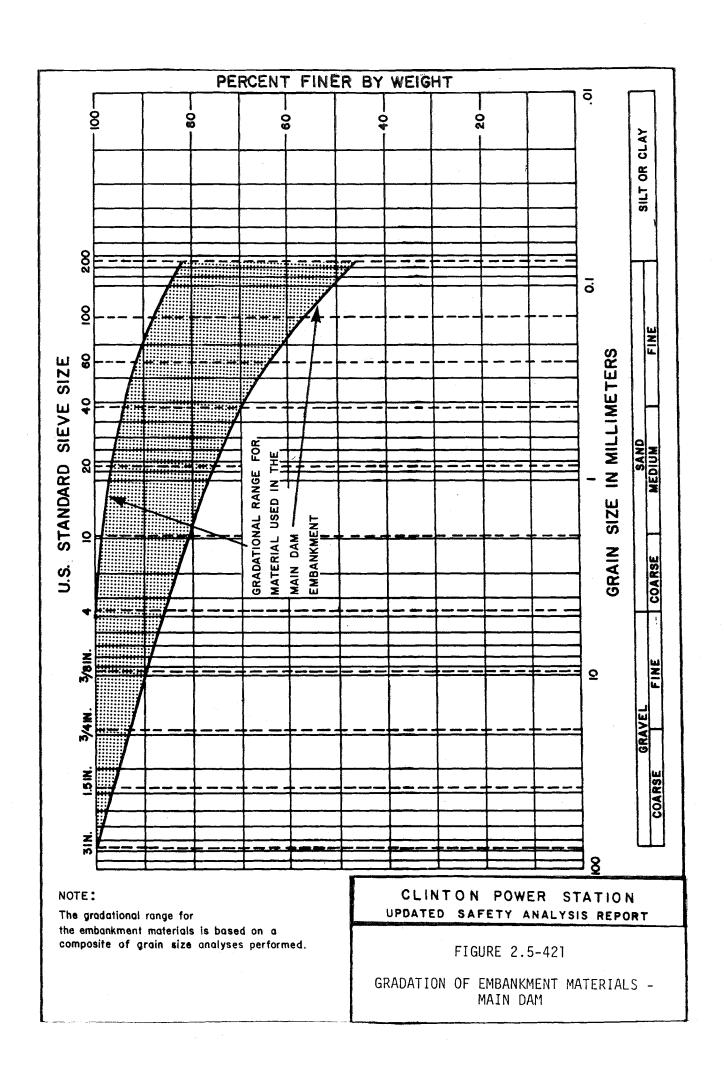
FIGURE 2.5-419

SHEAR LAYER MODEL FOR NATURAL SLOPE FOUNDATION

	131	169	180	191	202	213	224	235	246	257	
o →c/w 4ηγοα 4μω 4ορις		168	179	190	201	212	223	234	245	256	
	128 129 154-77 56	16.7	178	189	200	211	222	233	244	255	
	126 127	166	177	188	199	210	221	232	243	254	
	124 125 124 125 125 125	.1	176	187	198	209	220	231	242	253	
	122 123	164	175	186	197	208	219	230	241	252	
100	120 121	163	174	185	196	207	218	229	240	251	
	118 119	· V	173	184	195	206	217	228	239	250	
	114 115 116 117		172	183	194	205	216	227	238	249	
	113	160	171	182	193	204	215	226	237	248	
	112	159	170	181	192	203	214	525	236	247	

FIGURE 2.5-420

FINITE ELEMENT MODEL USED FOR NATURAL SLOPE STABILITY ANALYSIS



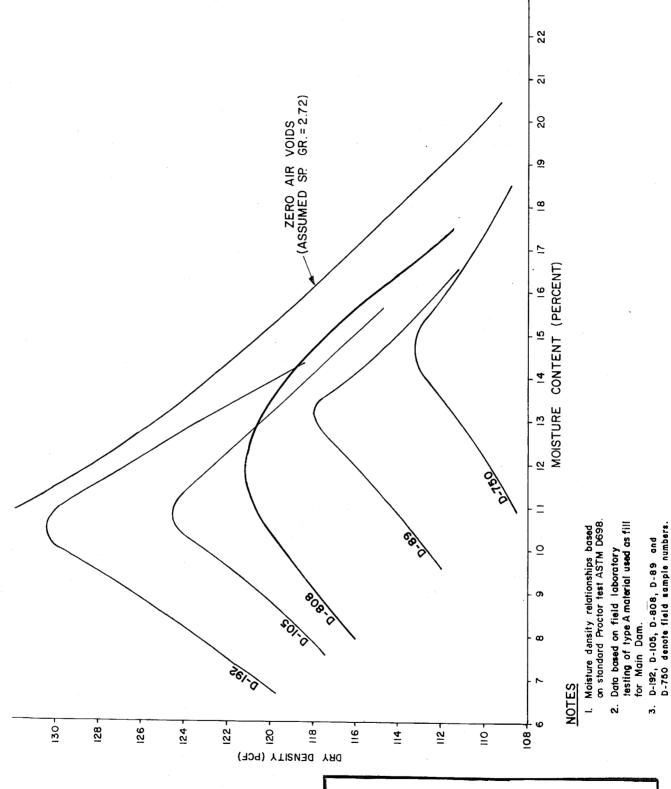


FIGURE 2.5-422

TYPICAL MOISTURE - DENSITY RELATIONSHIP - MAIN DAM

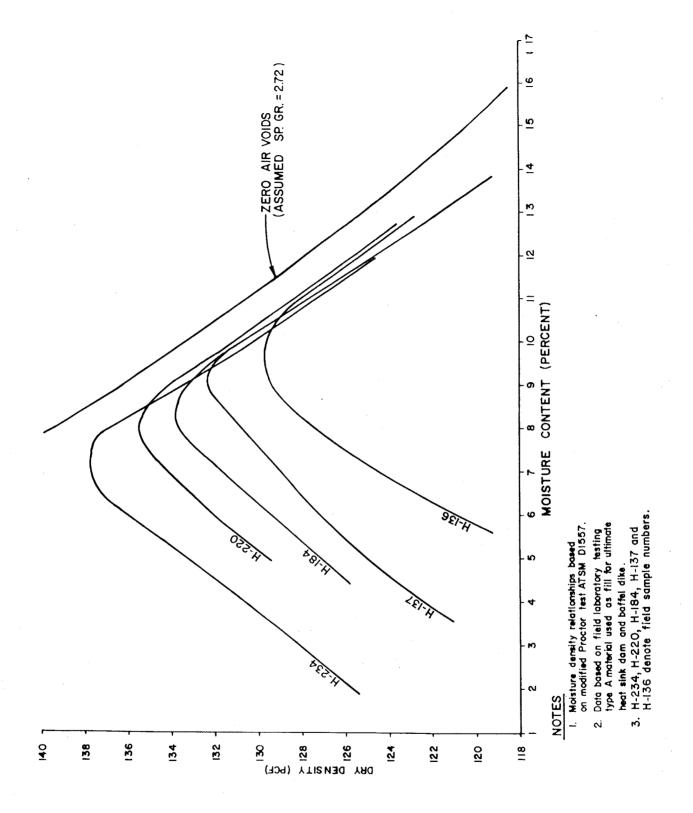
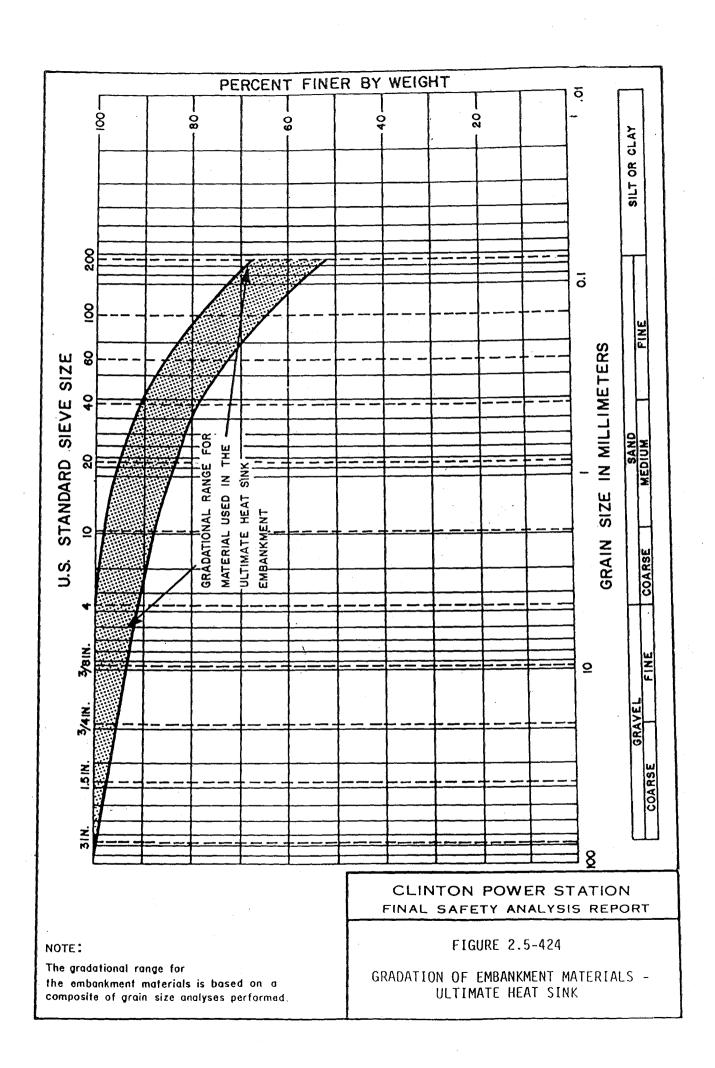
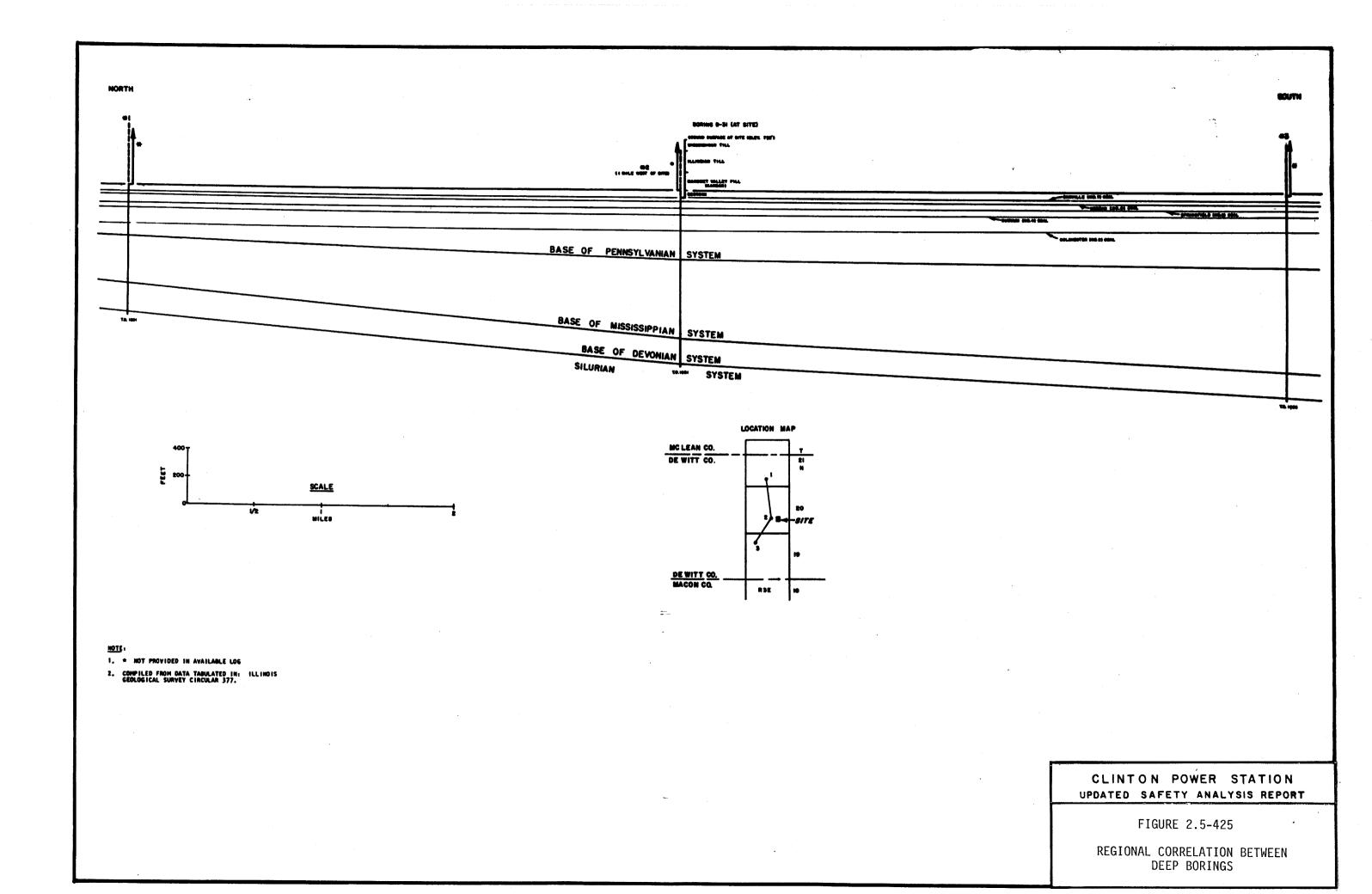
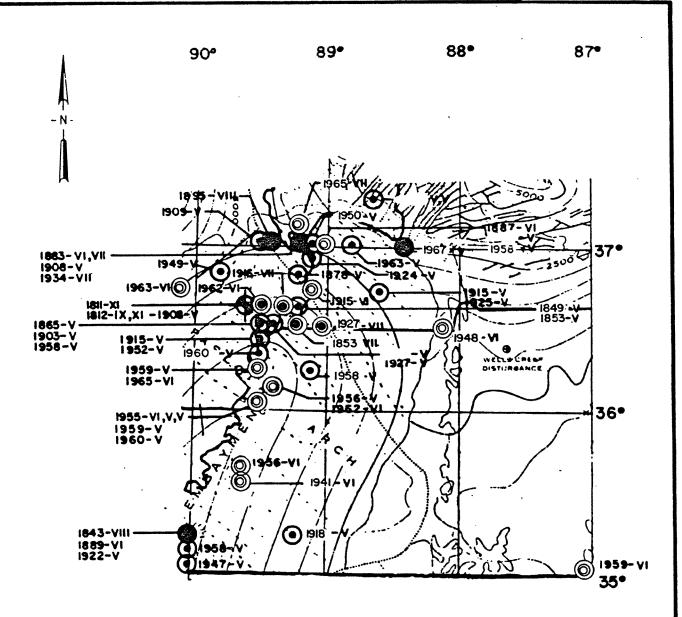


FIGURE 2.5-423

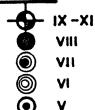
TYPICAL MOISTURE-DENSITY RELATIONSHIPS - ULTIMATE HEAT SINK







### LEGEND:



#### NOTE:

FROM TECTONIC MAP OF THE UNITED STATES, UNITED STATES GEOLOGICAL SURVEY AND AMERICAN ASSOCIATION OF PETROLEUM GEOLOGISTS, 1962.

EARTHQUAKE DATA COMPILED FROM COFFMAN, J.L. AND VONHAKE, C.A. (1973) EARTHQUAKE HISTORY OF THE UNITED STATES, U.S. DEPT. COMMERCE, NOAA, PUBLICATION 41-1 AND FROM OTHER SOURCES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-426

EARTHQUAKE EPICENTER MAP - BELOW 37°

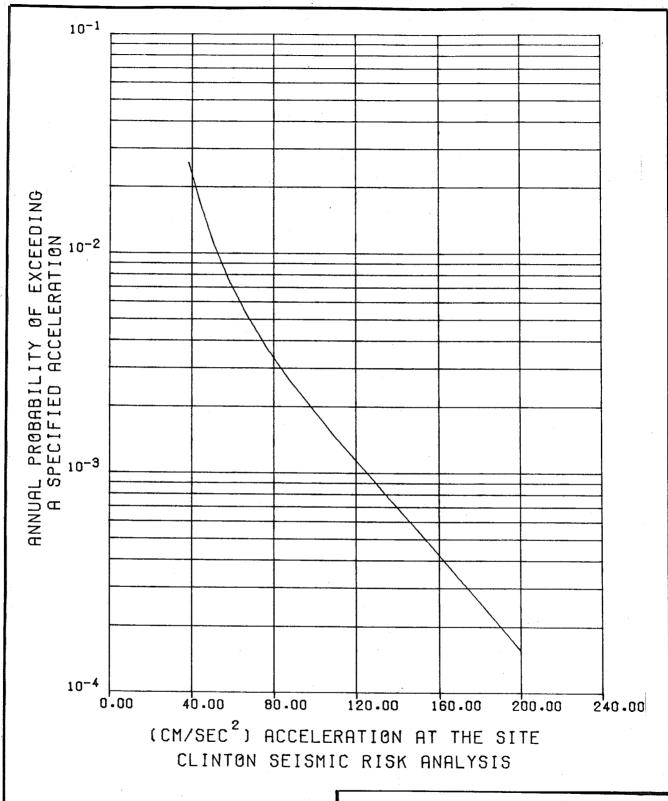
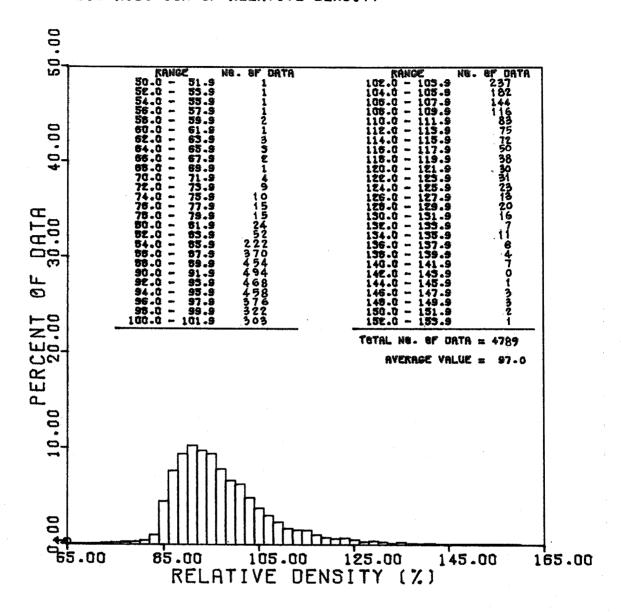


FIGURE 2.5-427
SEISMIC RISK CURVE

#### DISTRIBUTION OF RELATIVE DENSITY



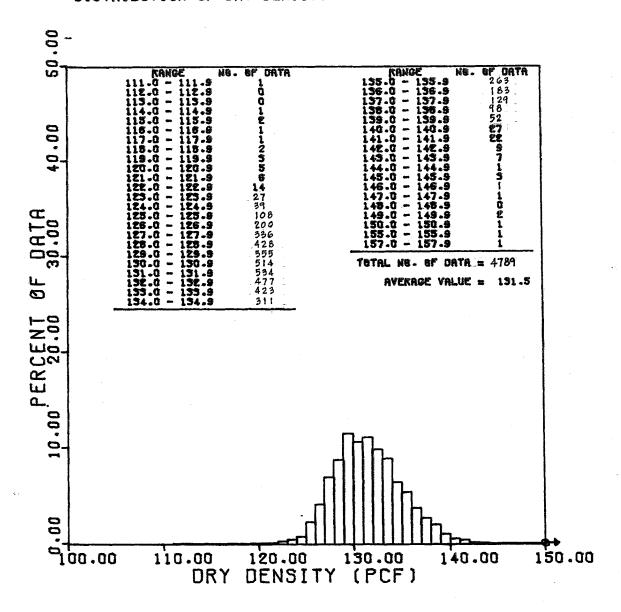
CLINTON STA. COMPACTION TESTS
ALL PASSING DATA
ELEVATIONS 671.00 TO 703.00

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-428

TYPE B GRANULAR FILL DISTRIBUTION OF RELATIVE DENSITY

#### DISTRIBUTION OF DRY DENSITY



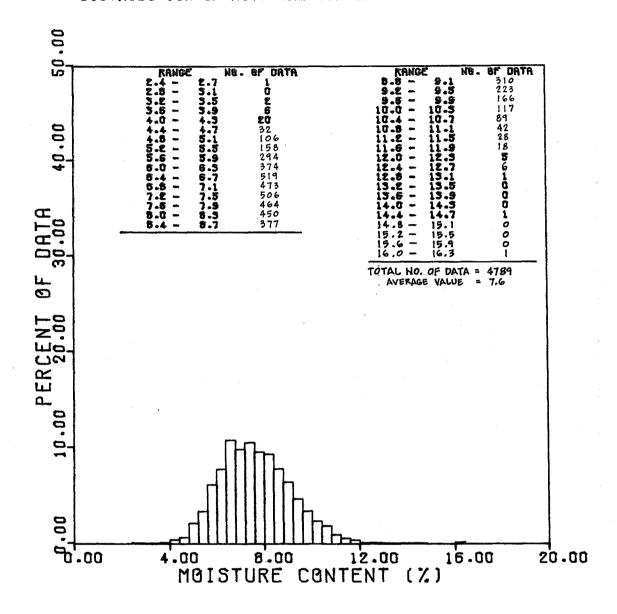
CLINTON STA. COMPACTION TESTS
ALL PASSING DATA
ELEVATIONS 671.00 TO 703.00

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-429

TYPE B GRANULAR FILL DISTRIBUTION OF DRY DENSITY

#### DISTRIBUTION OF MOISTURE CONTENT



CLINTON STA. COMPACTION TESTS
ALL PASSING DATA
ELEVATIONS 671.00 TO 703.00

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-430

TYPE B GRANULAR FILL DISTRIBUTION OF MOISTURE CONTENT

#### LOW DENSITY TESTS BY LIFTS AND GRIDS

		GRID																																				
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	İ	1	2	3	4	5	6	1	2	3	4	5	6	1	2	3	<u>-</u> 4	5	6	1	2	3	4	5	6	1	2	3	- 4	5	6	1	2	3	<u></u> 4	5	6	7
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6	671						$\dashv$							_									_		-													4
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	680	3					$\neg$			-	3																	7	·			<b>-</b>						7
	681				1	2	1									1				3							<u>:</u>	•										1
	682					1	$\neg$	-		-						•	*	1		Ť	1									*							2	ヿ
	683					2	1		1												1	1				1						4				1		7
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 _	685																												1					2				
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ELEVATION	689				4					2												1				1	1				1						1	_
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	102	¥																		<u> </u>					1													

NOTE: The numbers indicate the quantity of low density tests in a grid at a corresponding elevation.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-431

DISTRIBUTION OF IN-PLACE LOW DENSITY TESTS

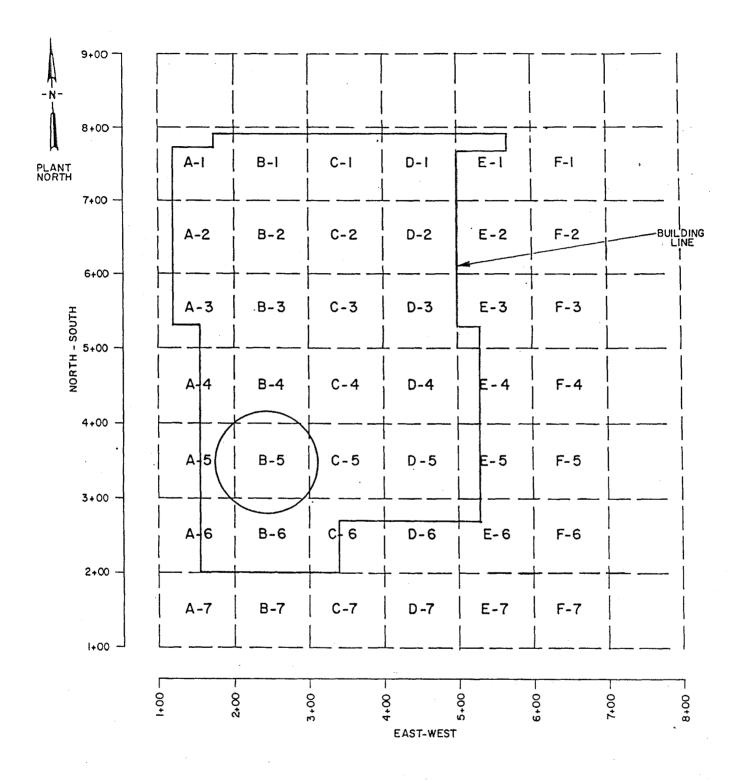
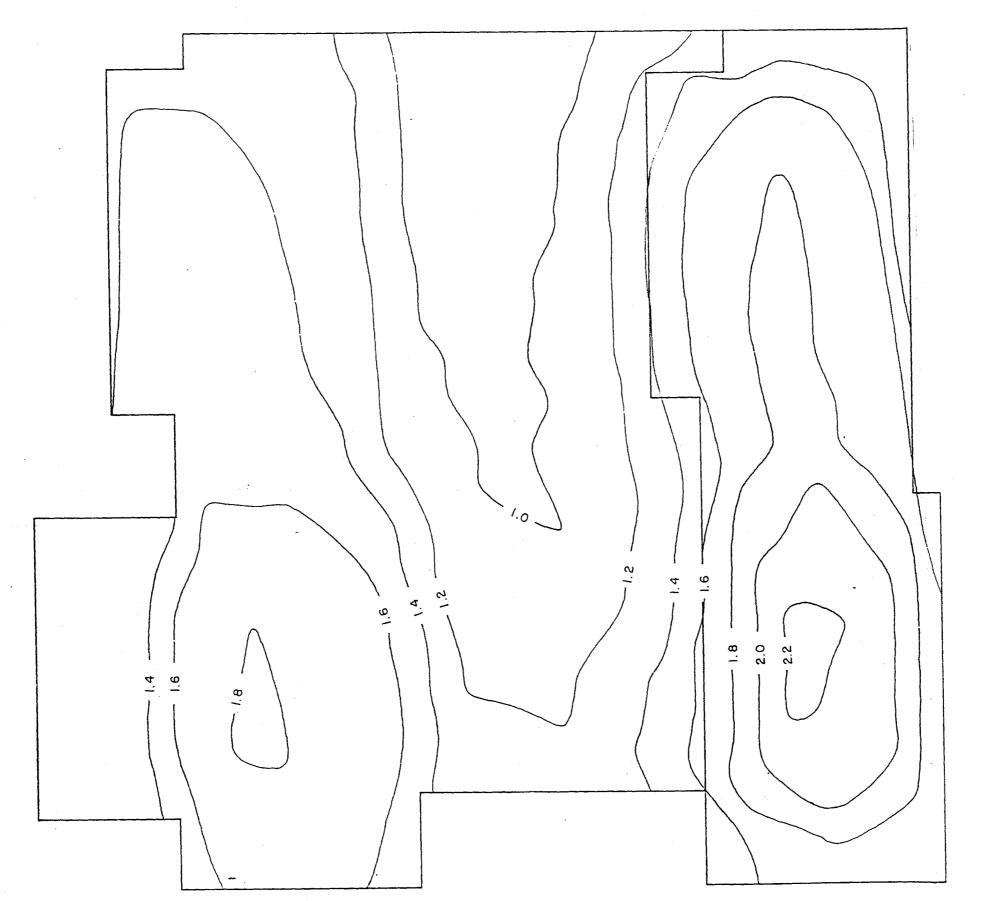


FIGURE 2.5-432

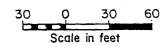
POWER BLOCK GRID SYSTEM





#### NOTE

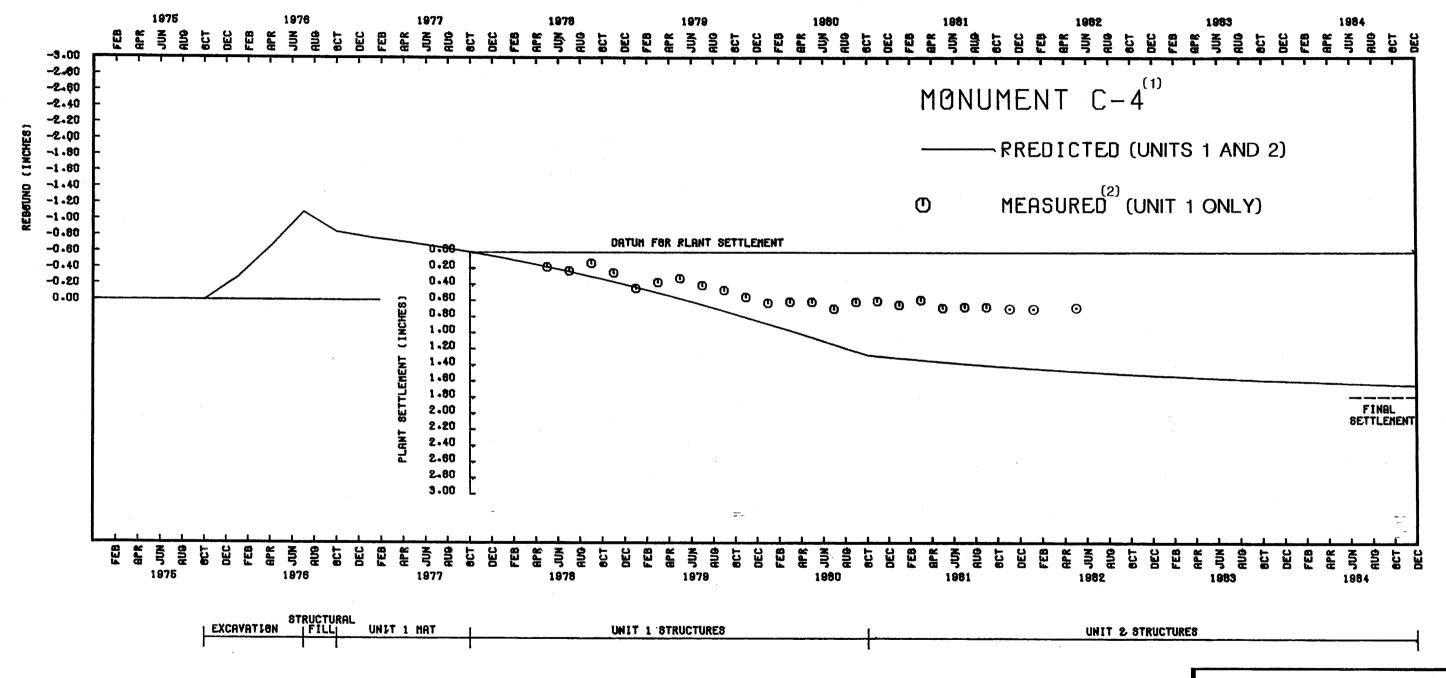
1. Units of settlements are inches



### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-433

CONTOURS OF COMPUTED FINAL SETTLEMENT FOR THE MAIN PLANT



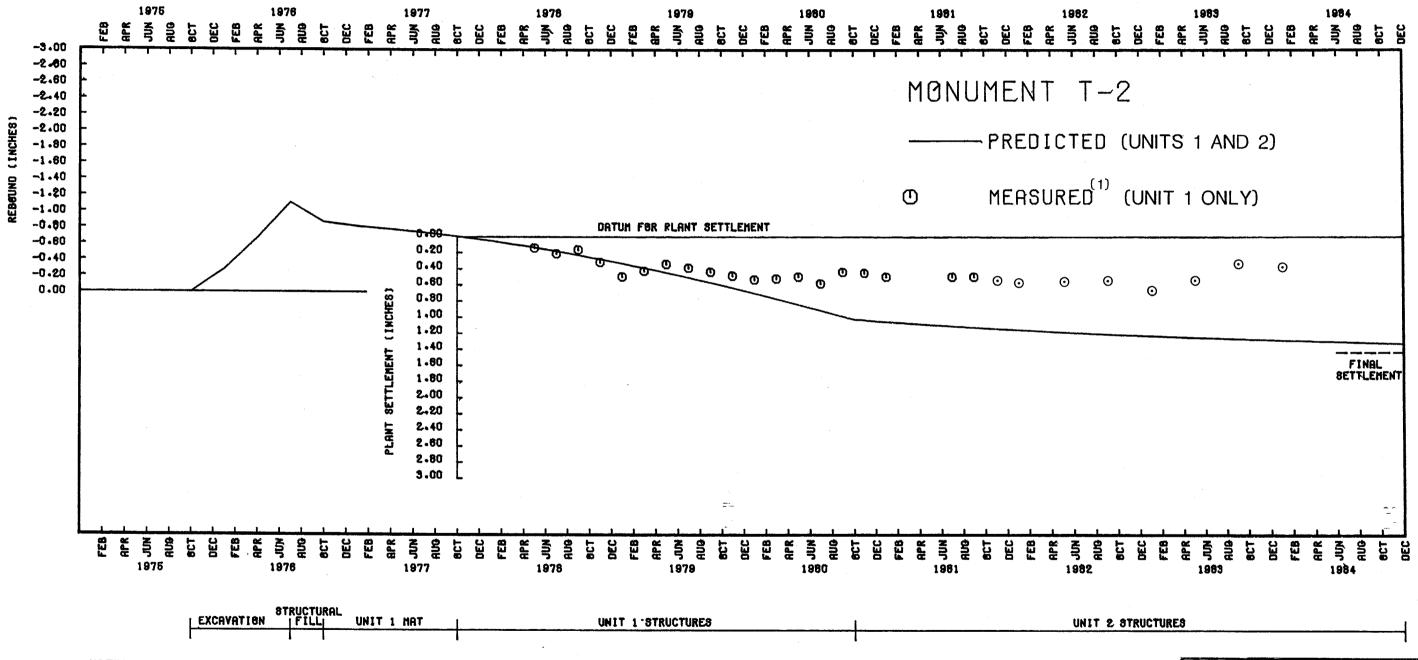
#### NOTES

- 1. THIS MONUMENT WAS REPLACED BY C-4A AFTER SEPTEMBER 1982.
- 2. UNIT 2 HAS BEEN CANCELLED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-434

COMPARISON OF PREDICTED AND MEASURED SETTLEMENT TIME HISTORIES AT SETTLEMENT MONUMENT C4 (CONTAINMENT BUILDING)

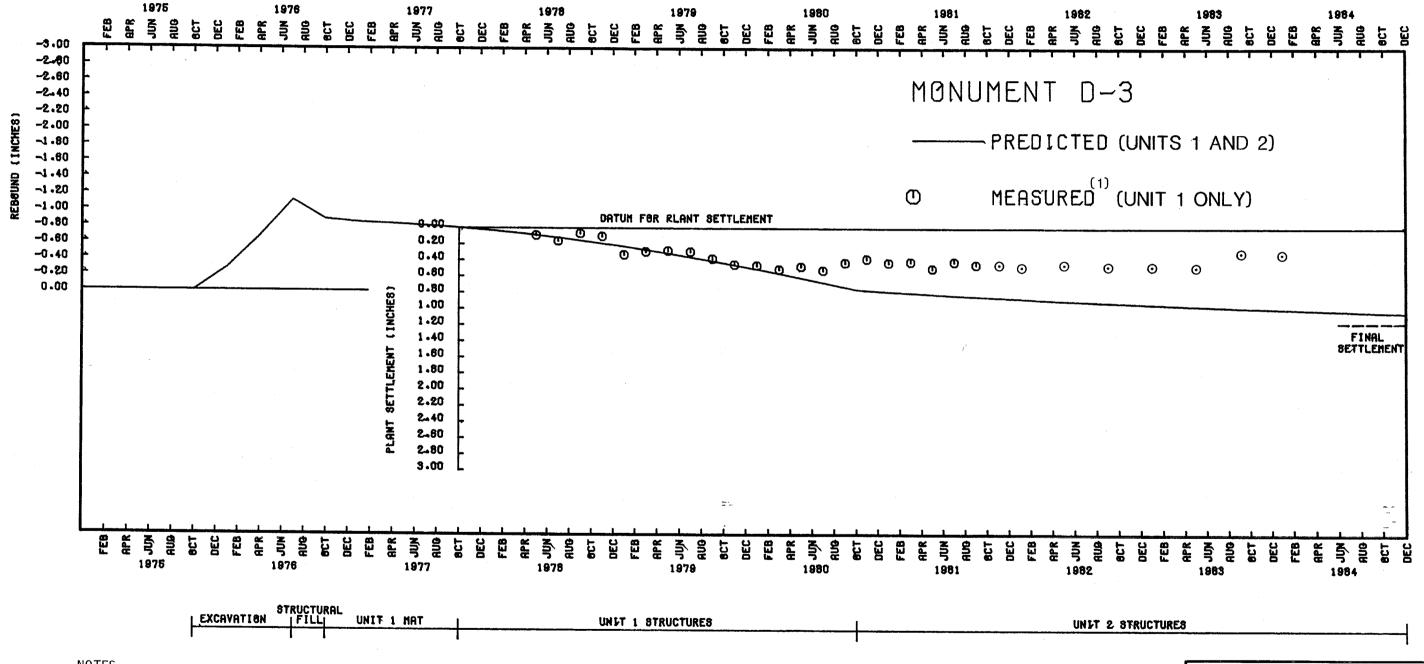


NOTES
1. UNIT 2 HAS BEEN CANCELLED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-435

COMPARISON OF PREDICTED AND MEASURED SETTLEMENT TIME HISTORIES AT SETTLEMENT MONUMENT TI (TURBINE BUILDING)



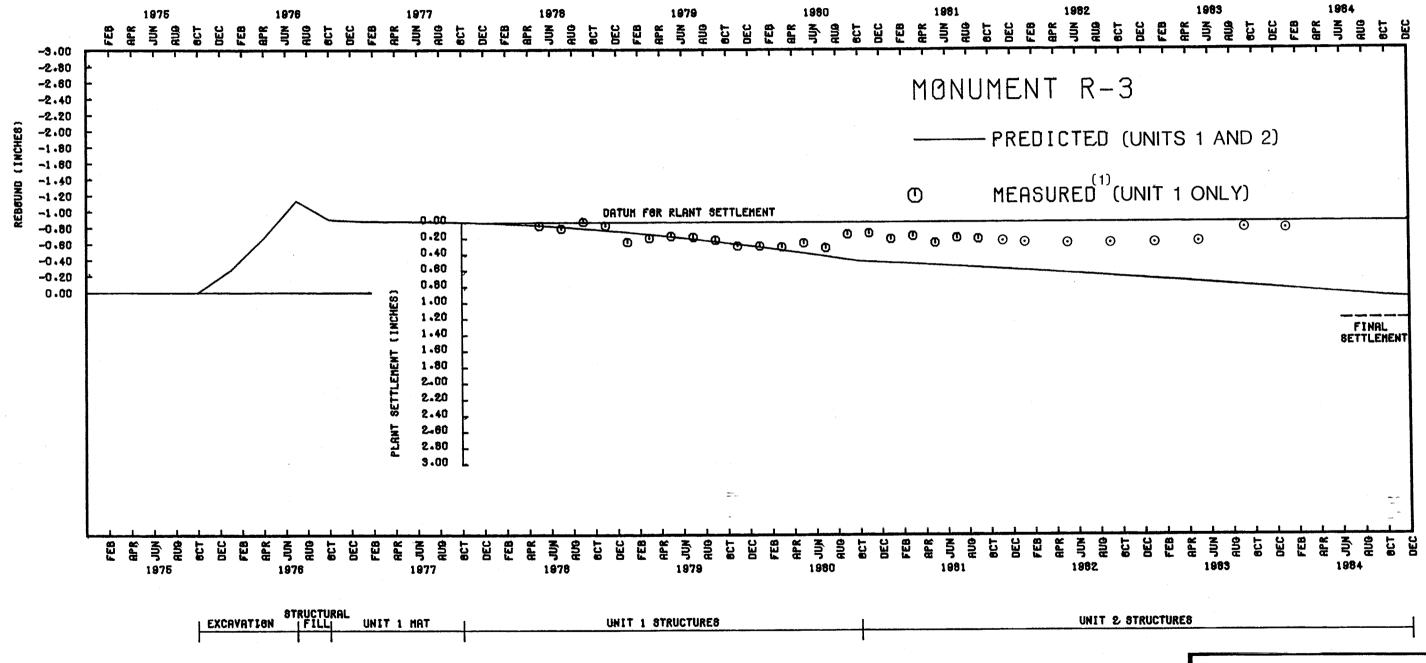
NOTES

1. UNIT 2 HAS BEEN CANCELLED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-436

COMPARISON OF PREDICTED AND MEASURED
SETTLEMENT TIME HISTORIES AT
SETTLEMENT MONUMENT D3
(DIESEL GENERATOR BUILDING)



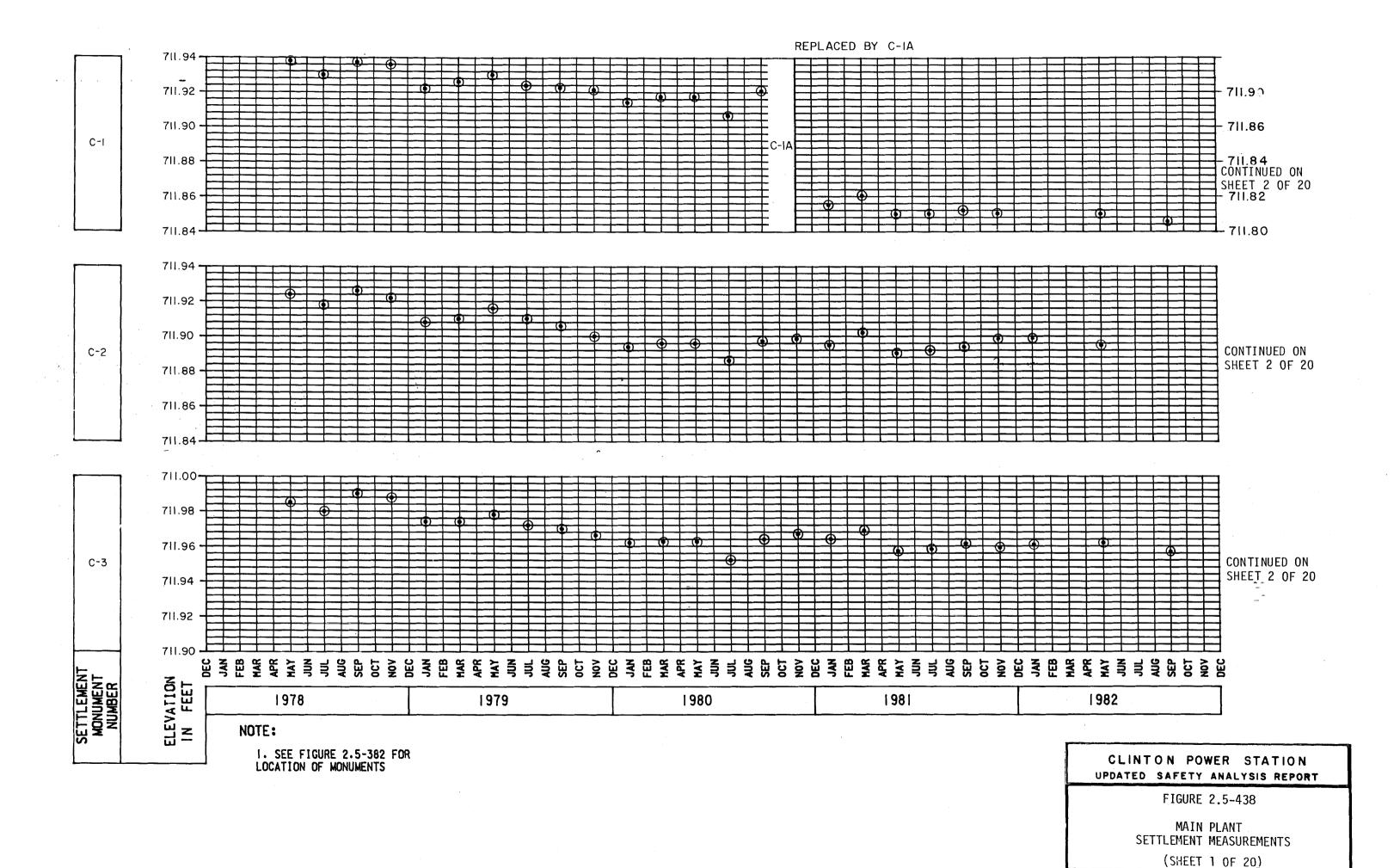
NOTES

1. UNIT 2 HAS BEEN CANCELLED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-437

COMPARISON OF PREDICTED AND MEASURED SETTLEMENT TIME HISTORIES AT SETTLEMENT MONUMENT R3 (RADWASTE BUILDING)



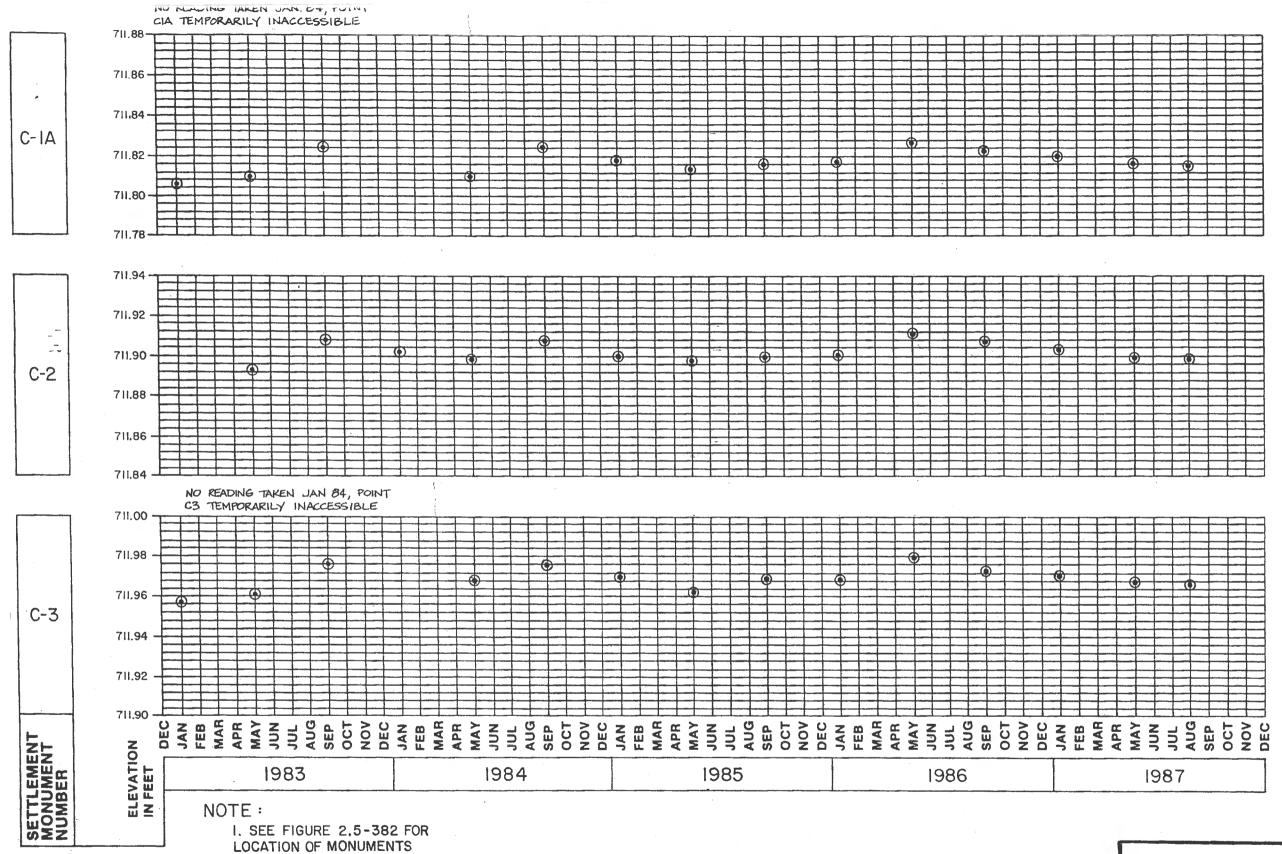
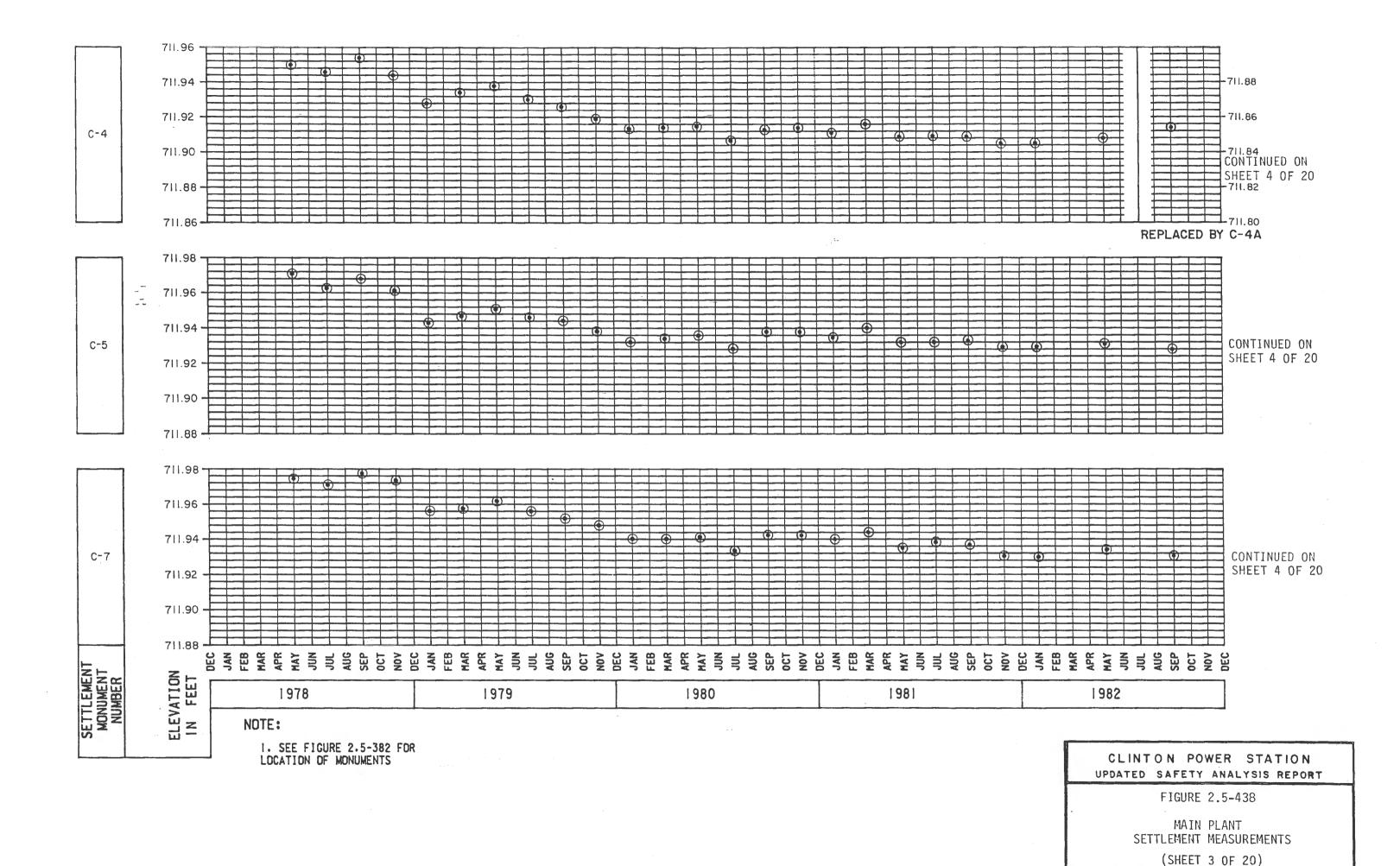


FIGURE 2.5-438

MAIN PLANT
SETTLEMENT MEASUREMENTS
(SHEET 2 OF 20)



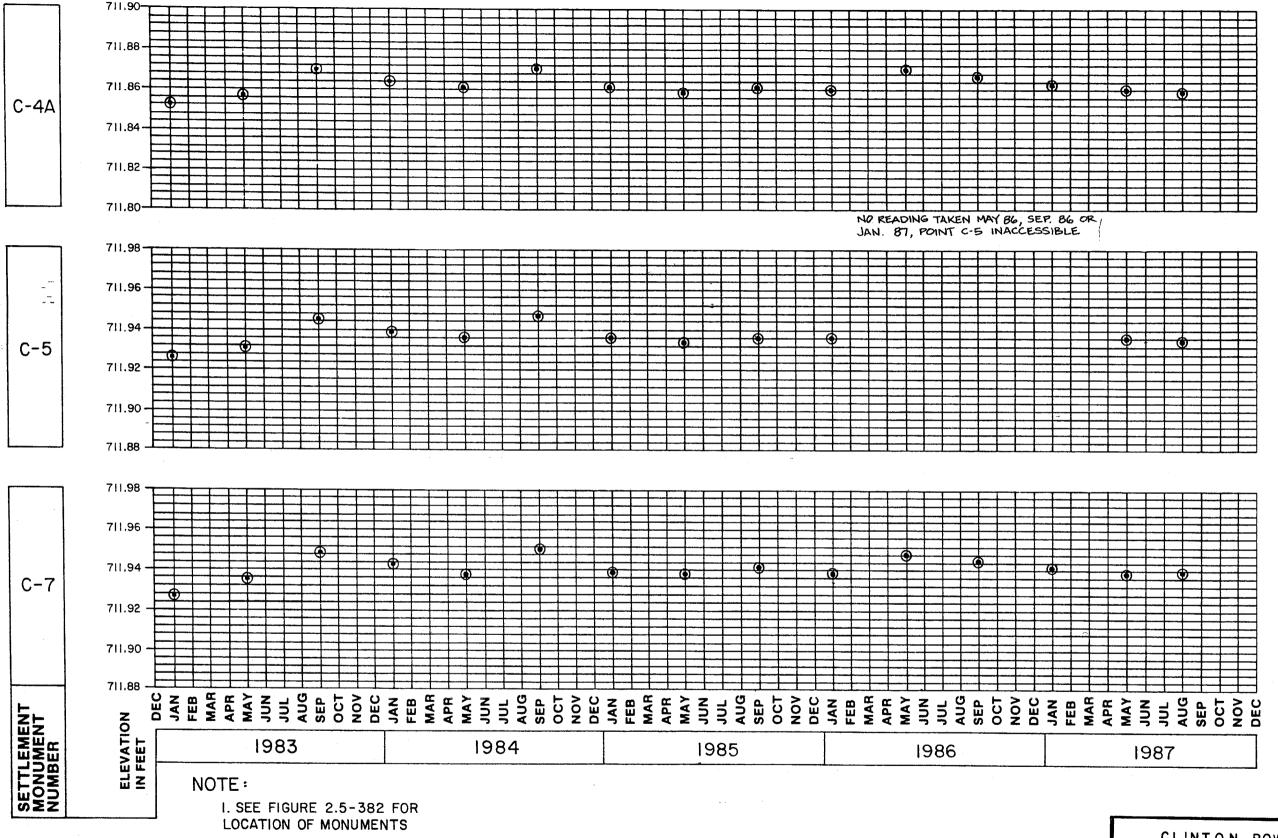


FIGURE 2.5-438

MAIN PLANT
SETTLEMENT MEASUREMENTS
(SHEET 4 OF 20)

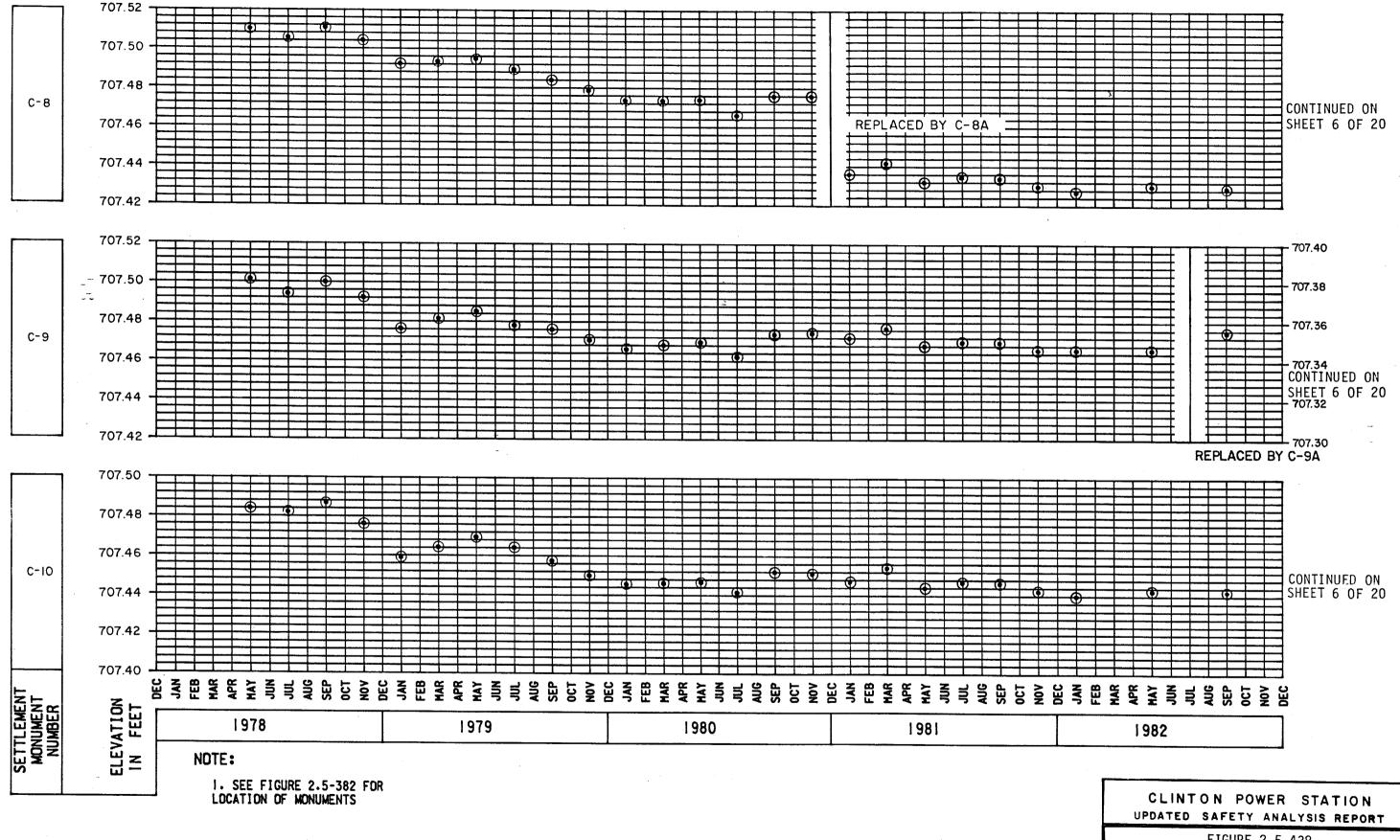
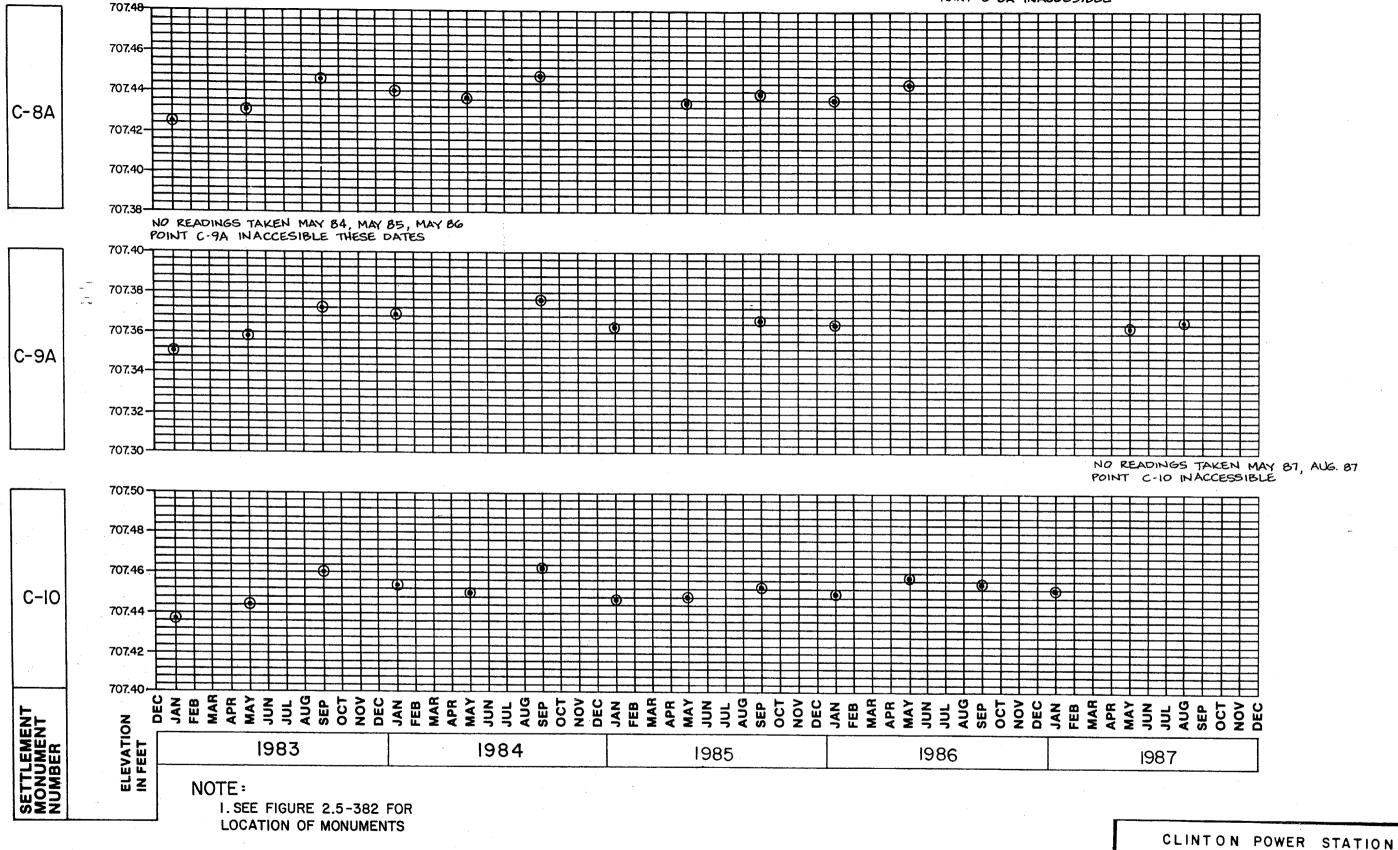


FIGURE 2.5-438

MAIN PLANT SETTLEMENT MEASUREMENTS (SHEET 5 OF 20)

NO READING TAKEN JAN. 85, MAY 87, AUG. 87 POINT C-8A INACCESIBLE

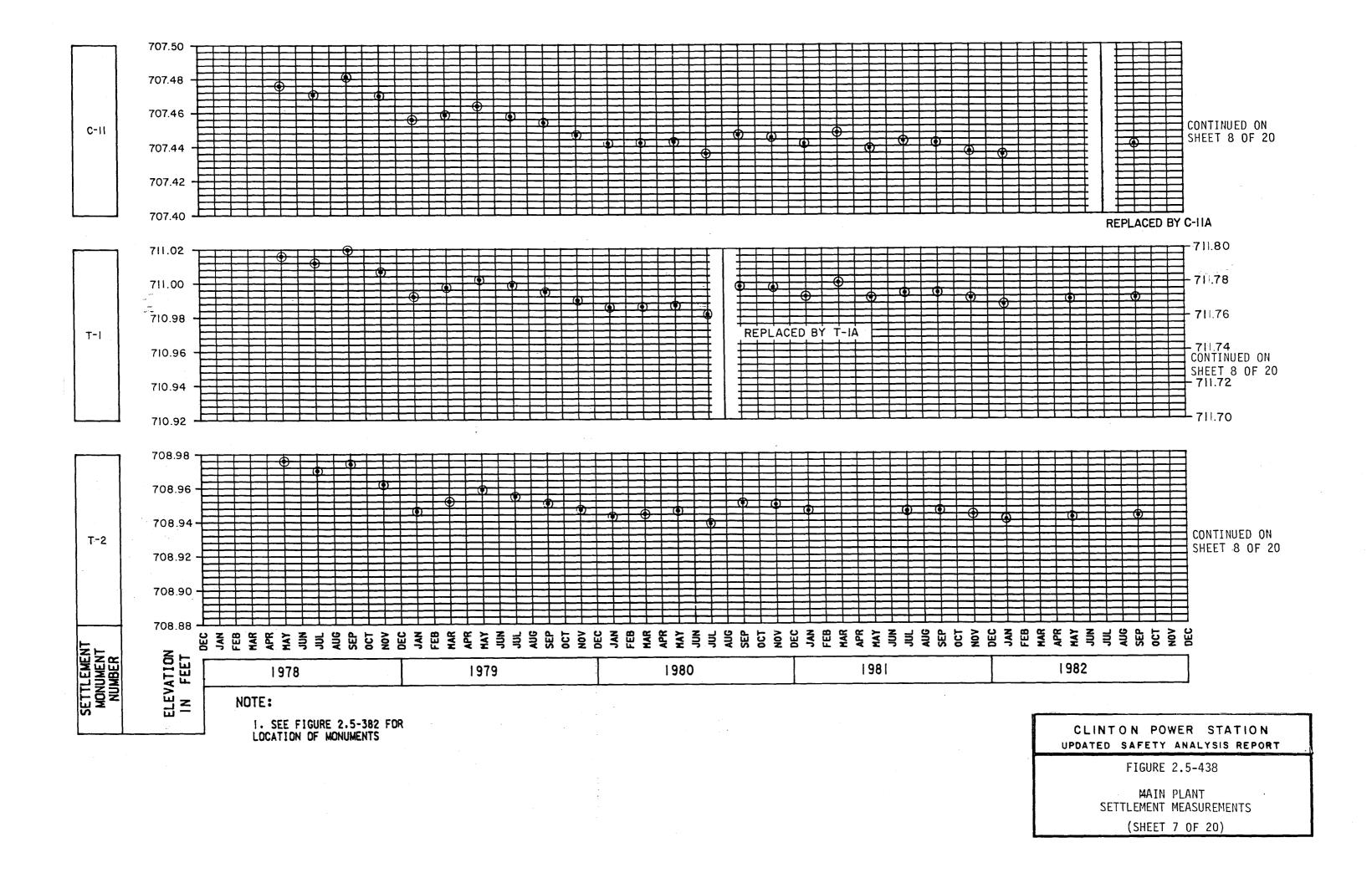


CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-438

MAIN PLANT SETTLEMENT MEASUREMENTS

(SHEET 6 OF 20)



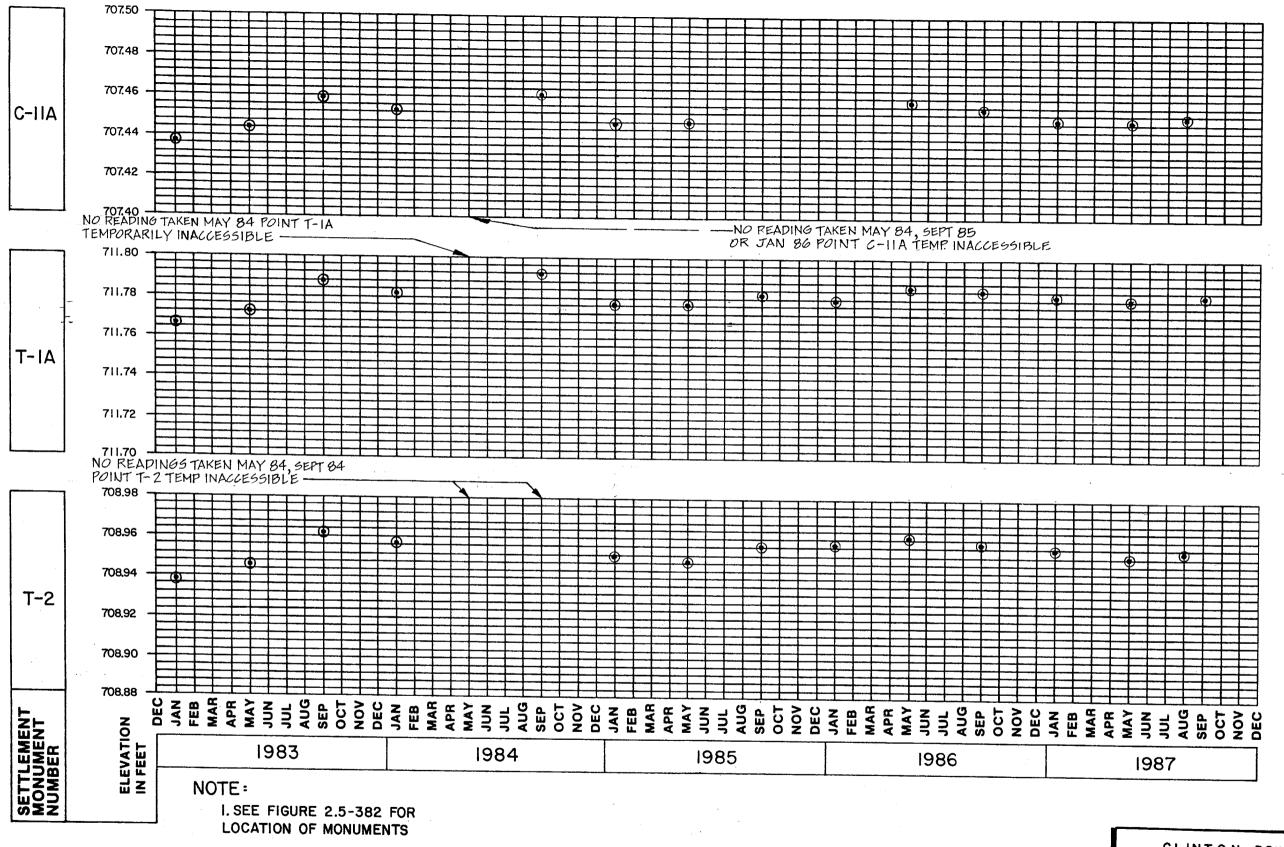
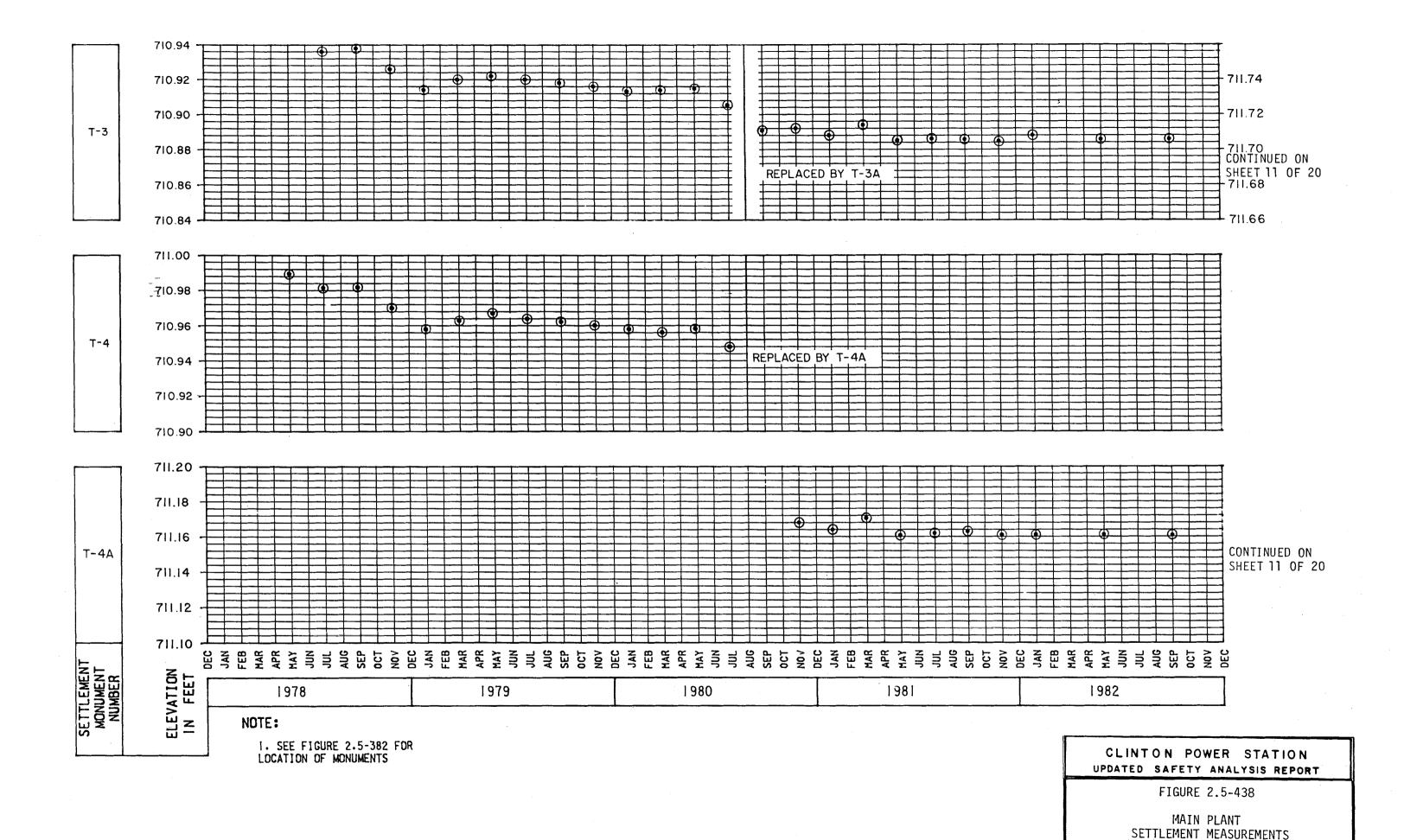
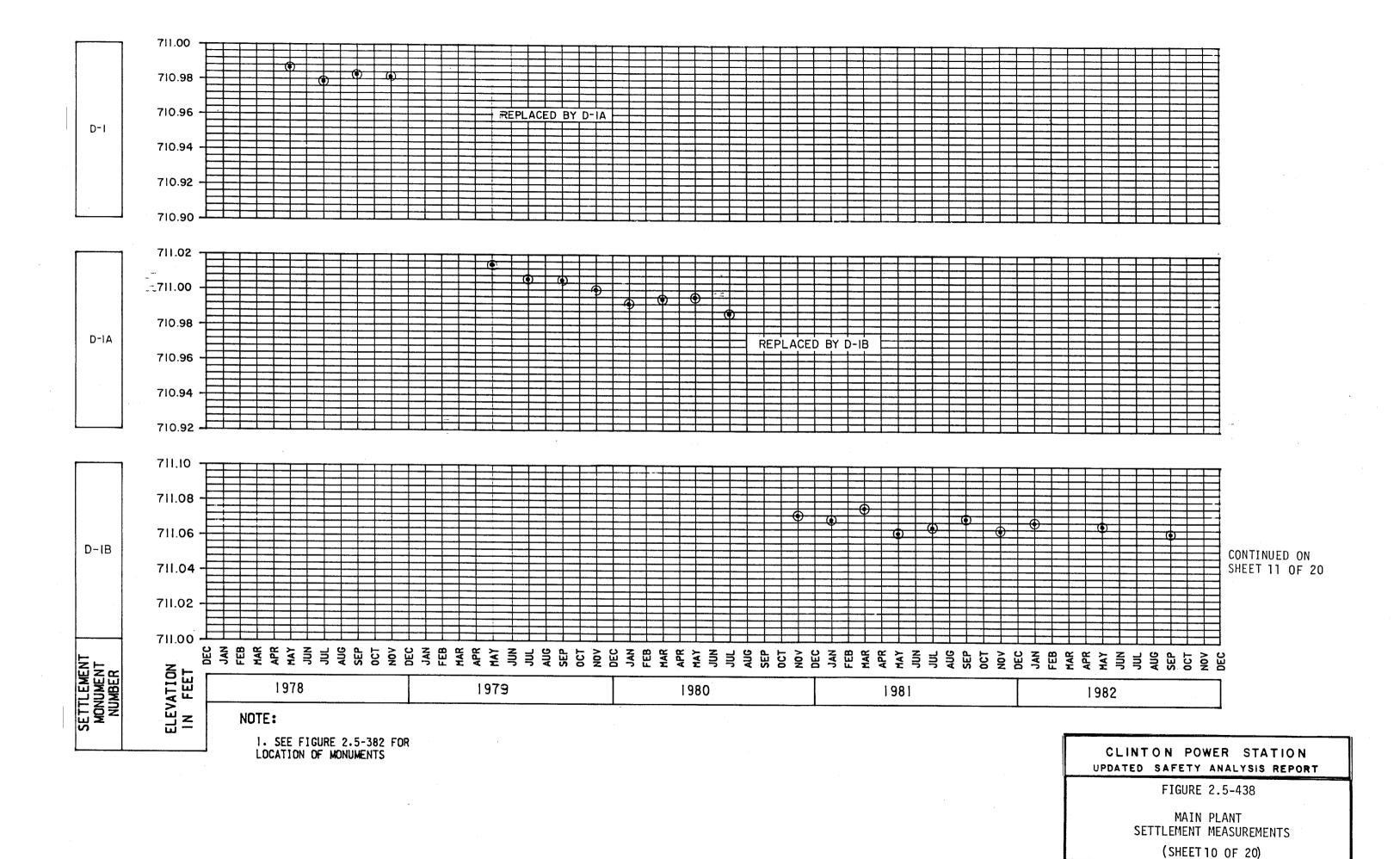


FIGURE 2.5-438

MAIN PLANT
SETTLEMENT MEASUREMENTS
____(SHEET 8 OF 20)



(SHEET 9 OF 20)



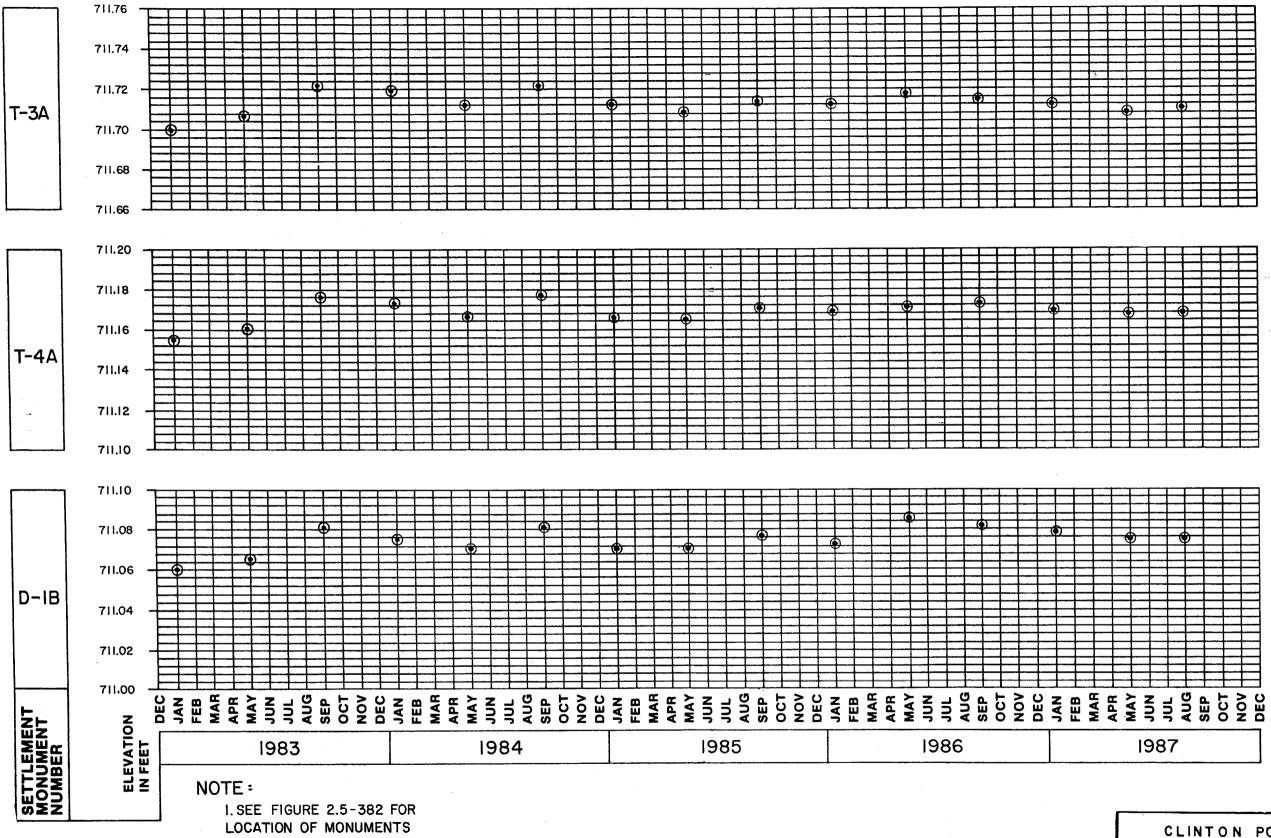
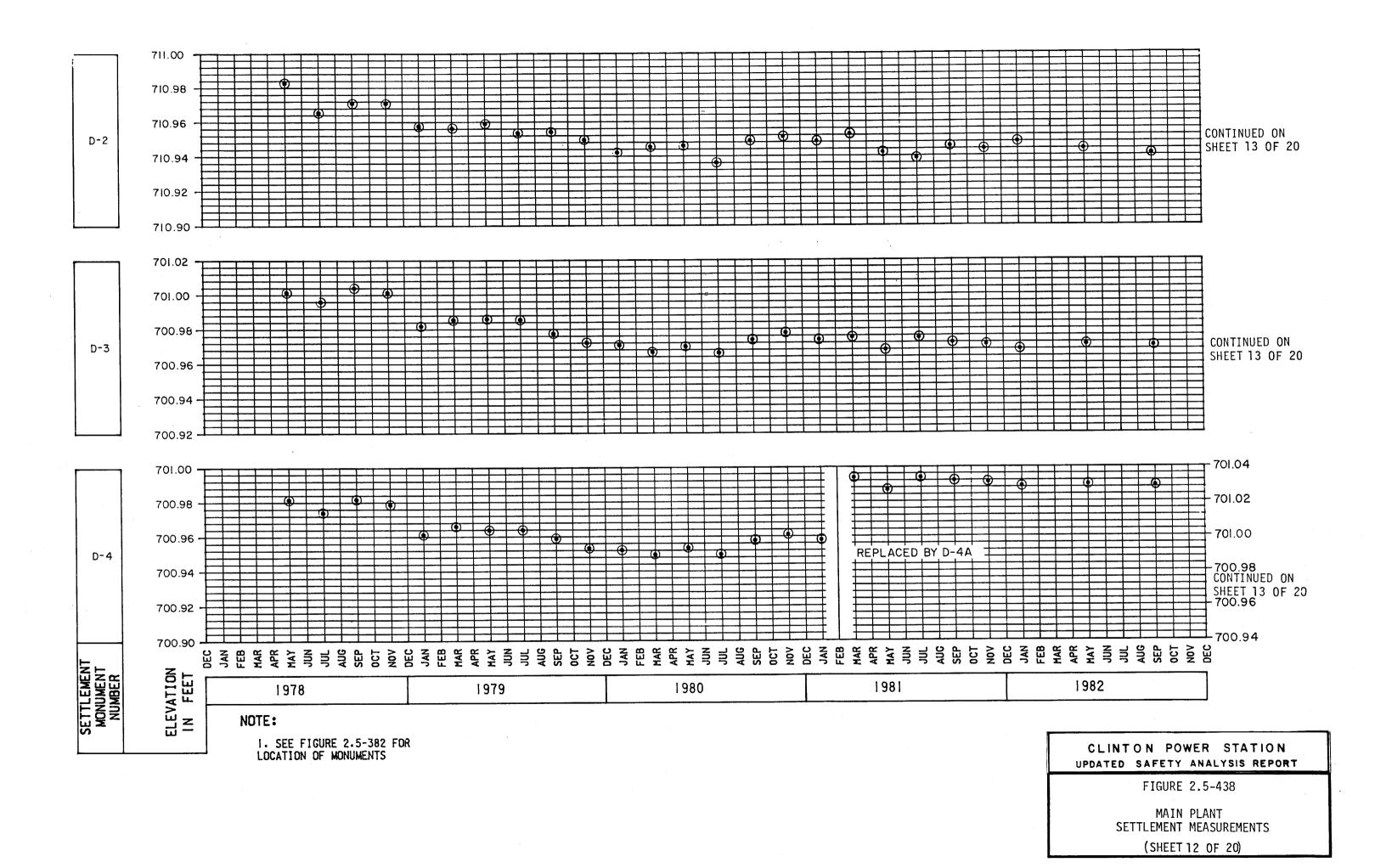


FIGURE 2.5-438

MAIN PLANT
SETTLEMENT MEASUREMENTS
(SHEET 11 OF 20)



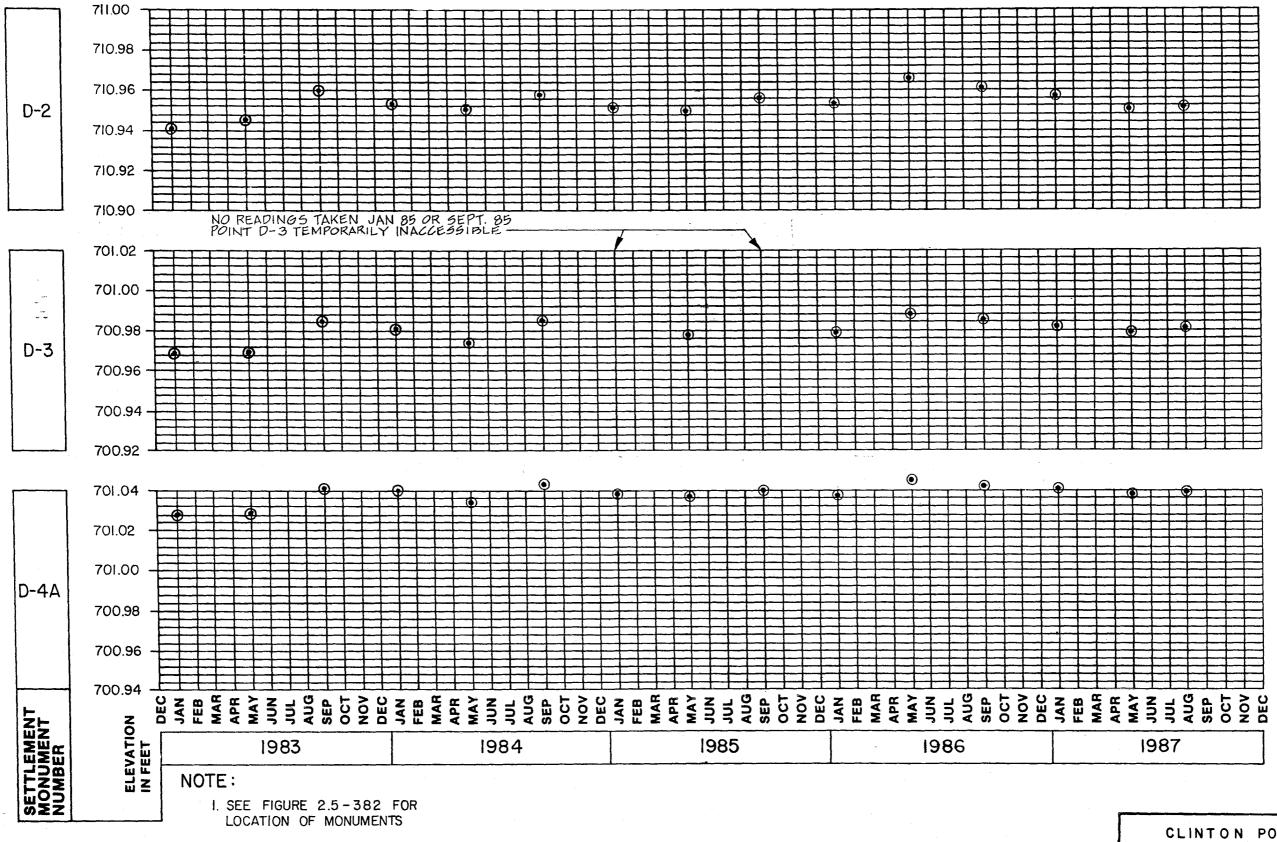
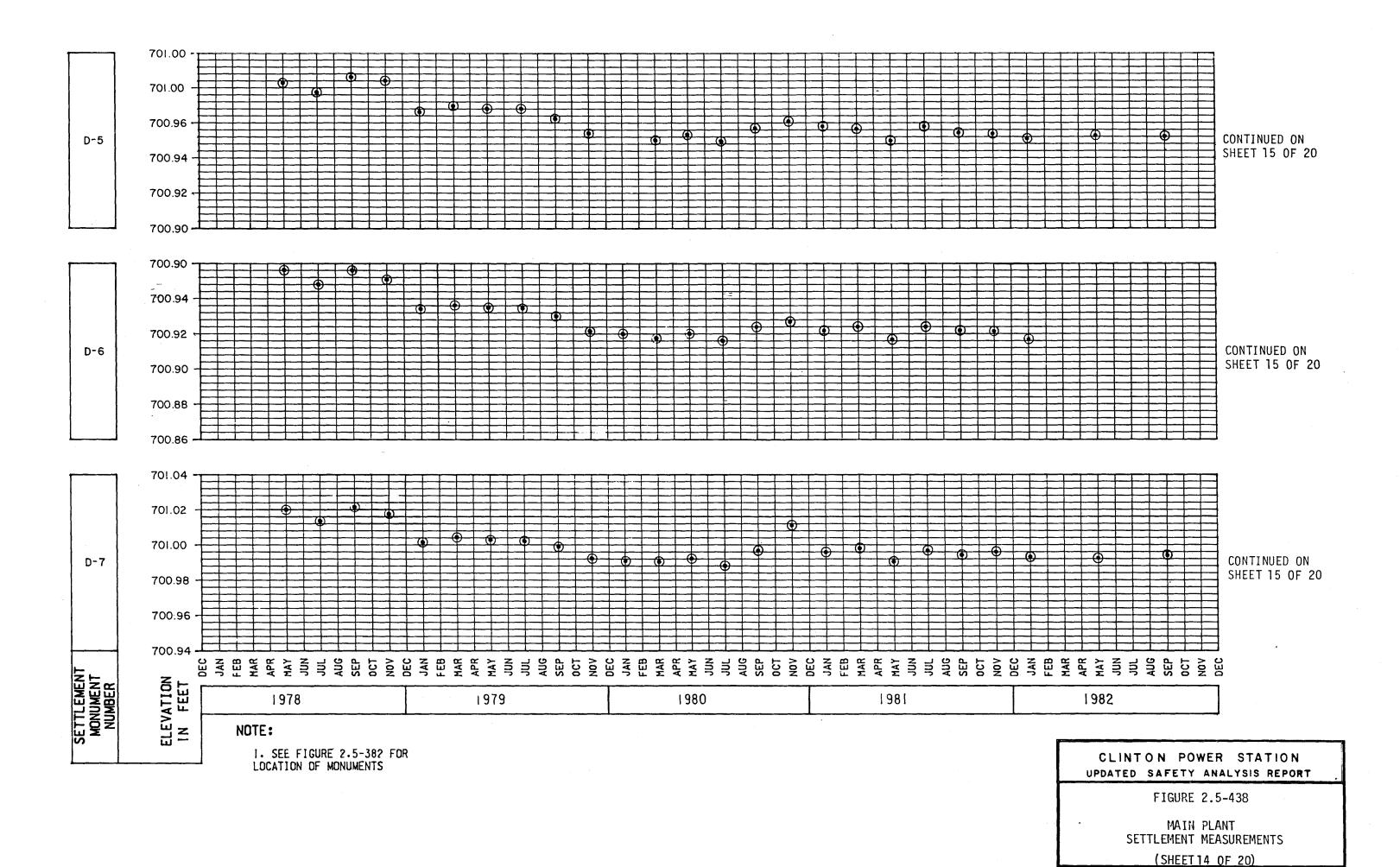


FIGURE 2.5-438

MAIN PLANT SETTLEMENT MEASUREMENTS
(SHEET 13 OF 20)



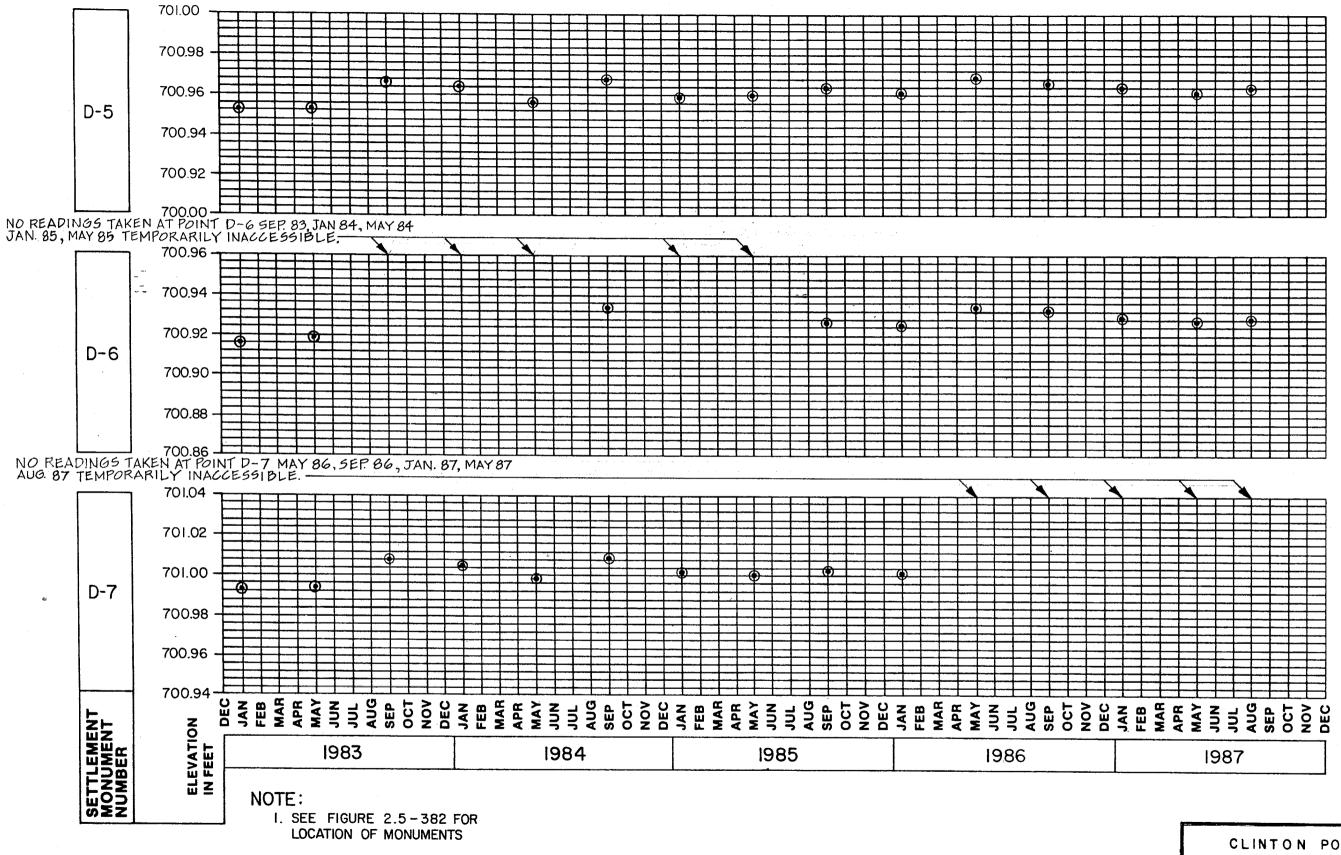


FIGURE 2.5-438

MAIN PLANT
SETTLEMENT MEASUREMENTS

(SHEET 15 OF 20)

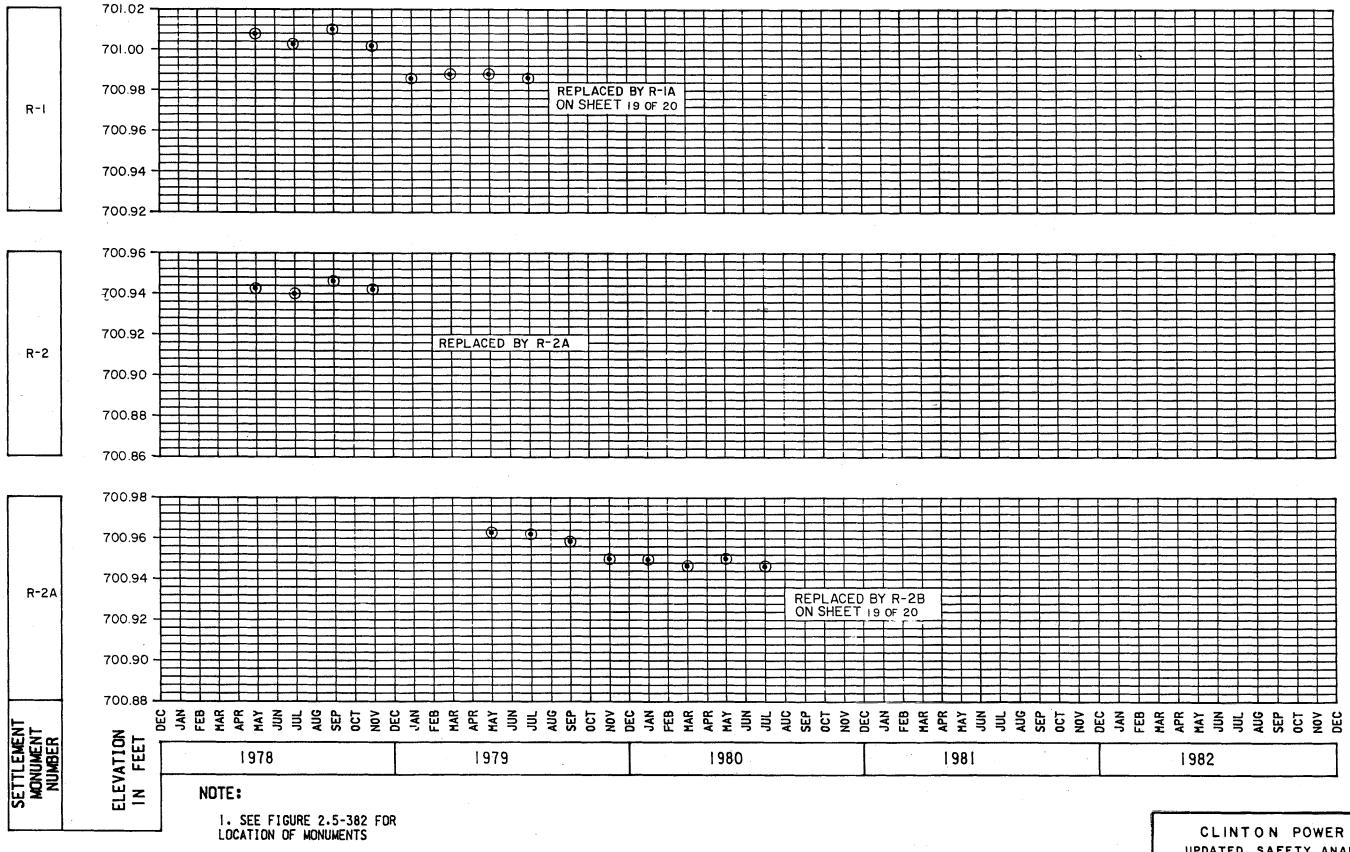
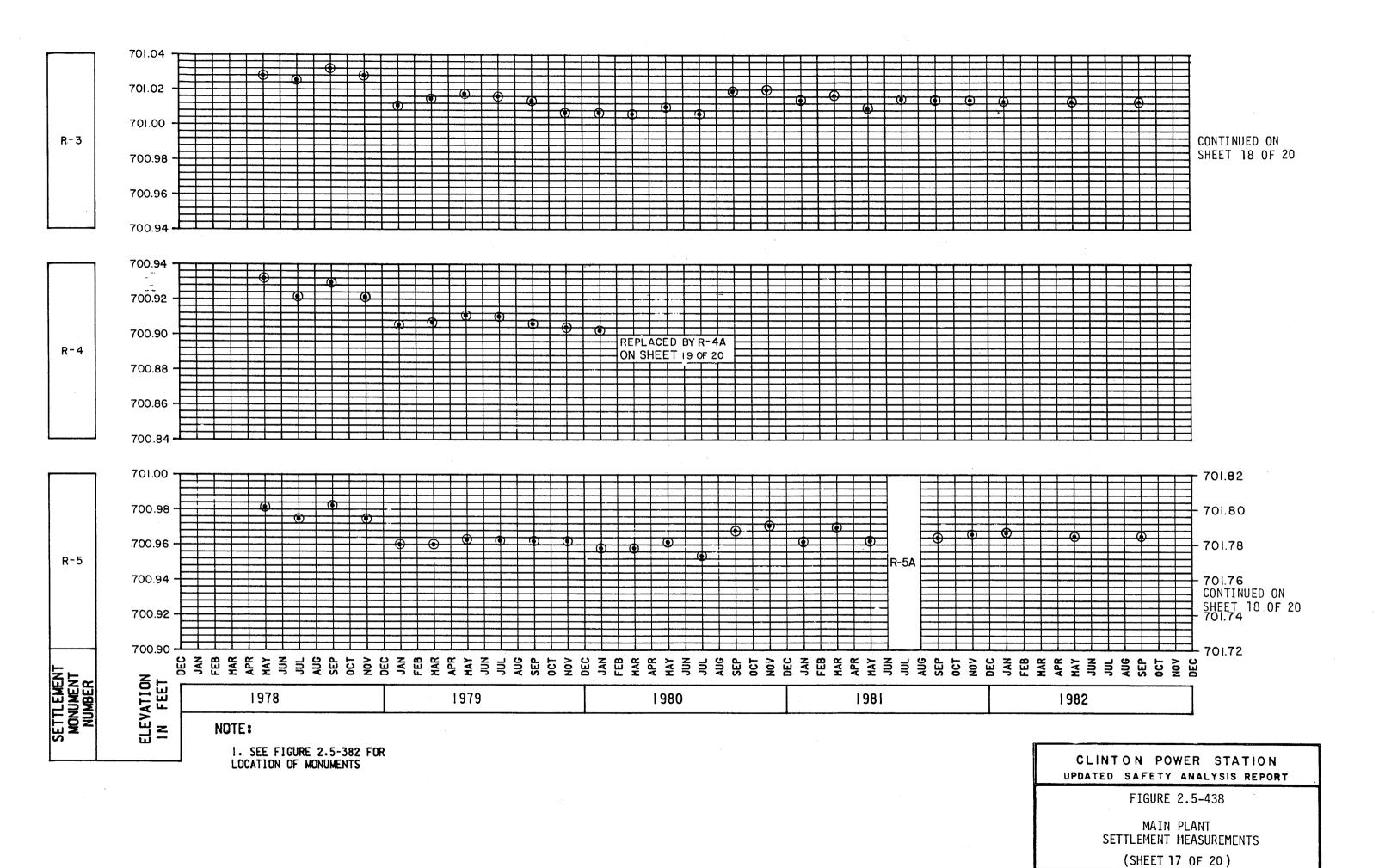


FIGURE 2.5-438

MAIN PLANT SETTLEMENT MEASUREMENTS (SHEET 16 OF 20)



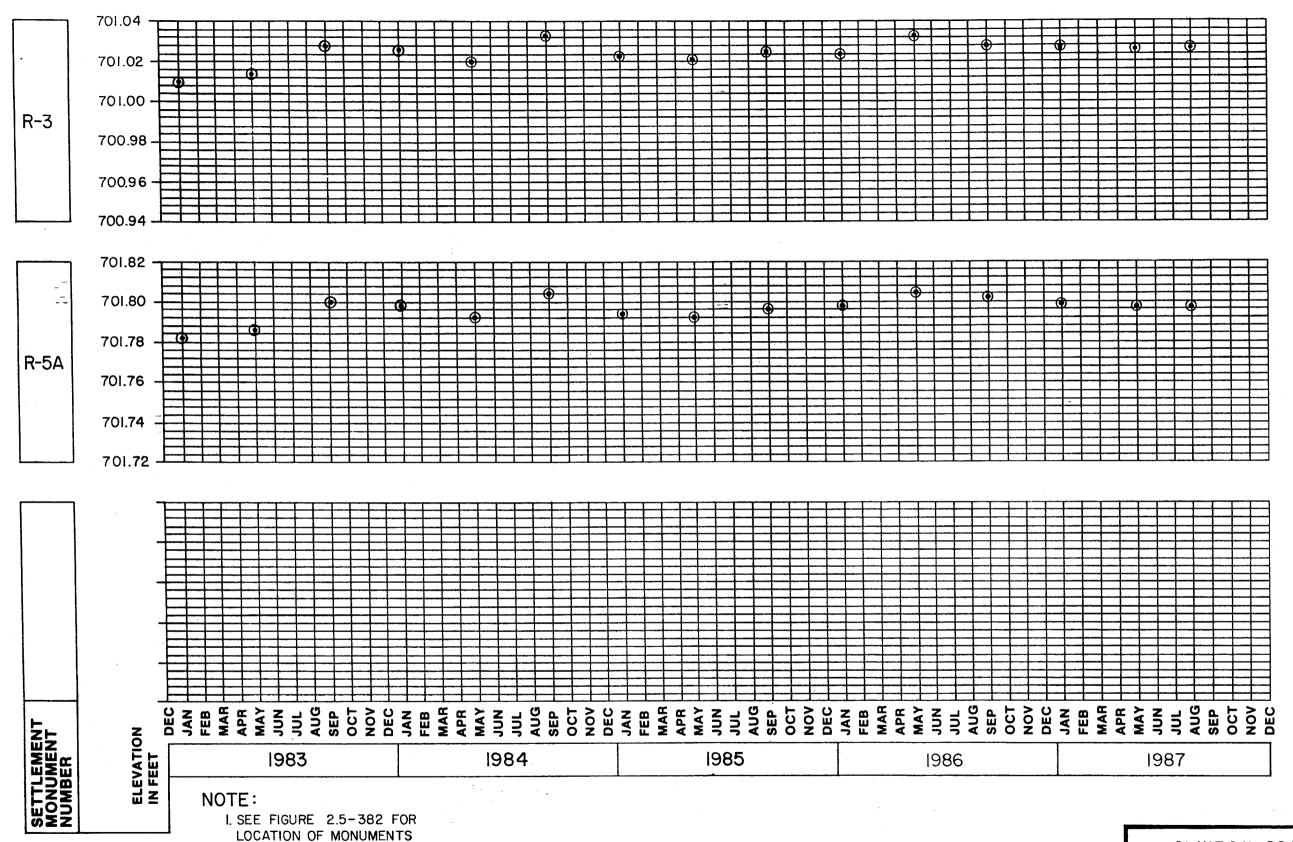
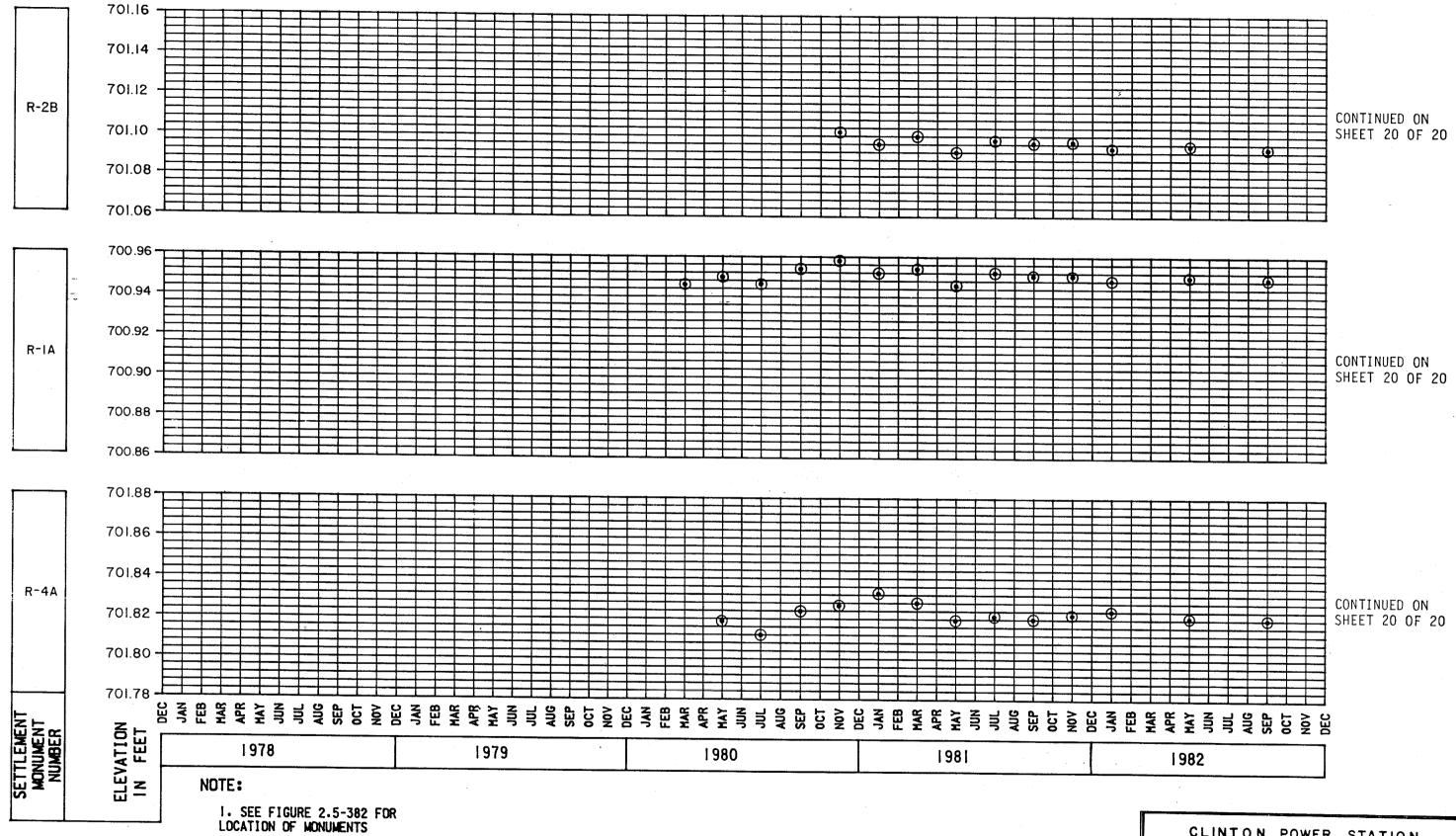


FIGURE 2.5-438

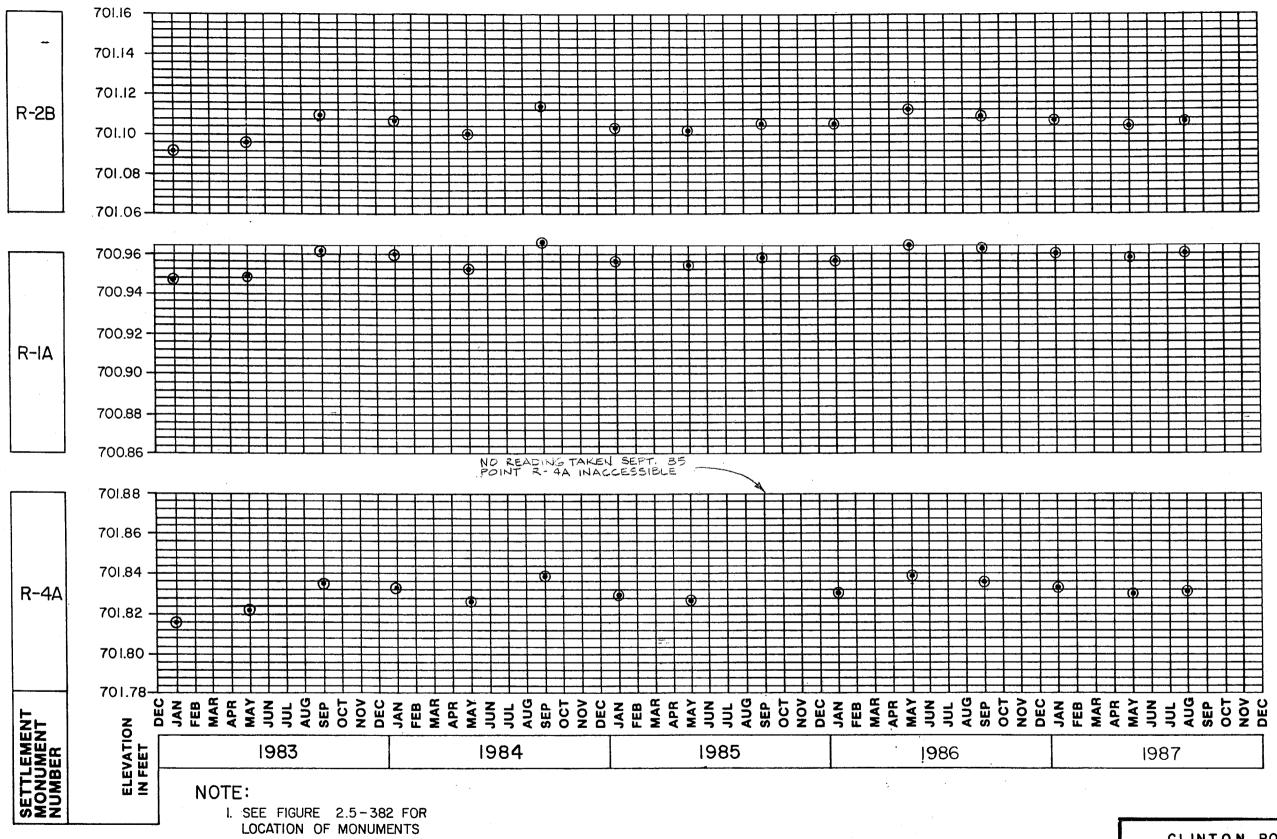
MAIN PLANT SETTLEMENT MEASUREMENTS (SHEET 18 OF 20)



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-438

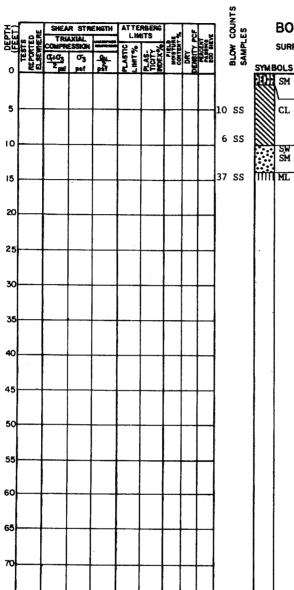
MAIN PLANT
SETTLEMENT MEASUREMENTS
(SHEET 19 OF 20)



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-438

MAIN PLANT
SETTLEMENT MEASUREMENTS
(SHEET 20 OF 20)



#### BORING OW-18

SURFACE ELEVATION 656.5

SLOPEWASH.
Brown, fine sand and silt.

FILL.
Brown, silty clay, some fine to coarse sand, trace silt.
SALT CREEK ALLUVIUM.
Gray, fine to medium sand, trace silt.

DESCRIPTIONS

ILLINOIAN TILL.
Gray, clayey silt and fine to medium sand, trace coarse sand and fine gravel.

BORING COMPLETED AT 15.0 FT. ON 7/16/79. WATER LEVEL AT 9.5 FT. ON 7/16/79.

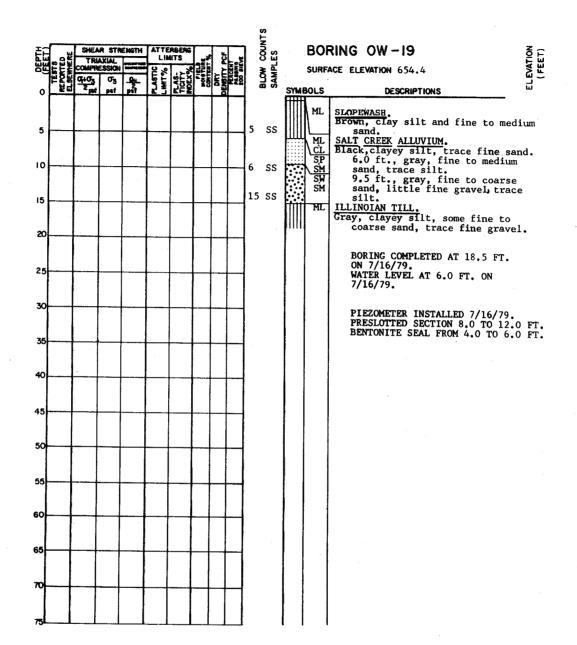
PIEZOMETER INSTALLED 7/16/79. PRESLOTTED SECTION 10.5 TO 14.5 FT. BENTONITE SEAL AT 5.0 TO 7.0 FT.

LOGGED BY: SARGENT & LUNDY

DRILLED BY: RAYMOND INTERNATIONAL

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-439

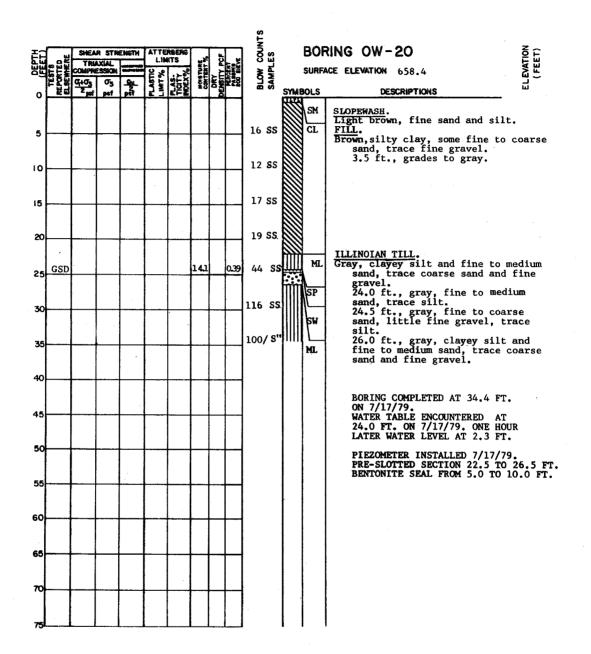


LOGGED BY: SARGENT & LUNDY

DRILLED BY: RAYMOND INTERNATIONAL

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-440



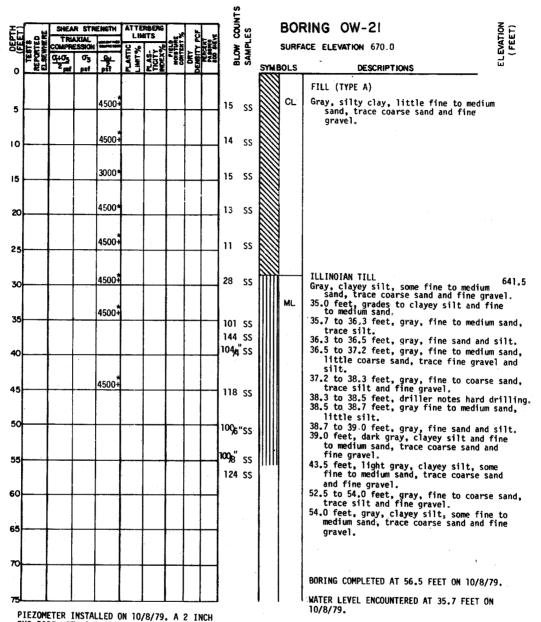
LOGGED BY: SARGENT & LUNDY

DRILLED BY: RAYMOND INTERNATIONAL

TESTED BY: SOIL TESTING SERVICES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-441

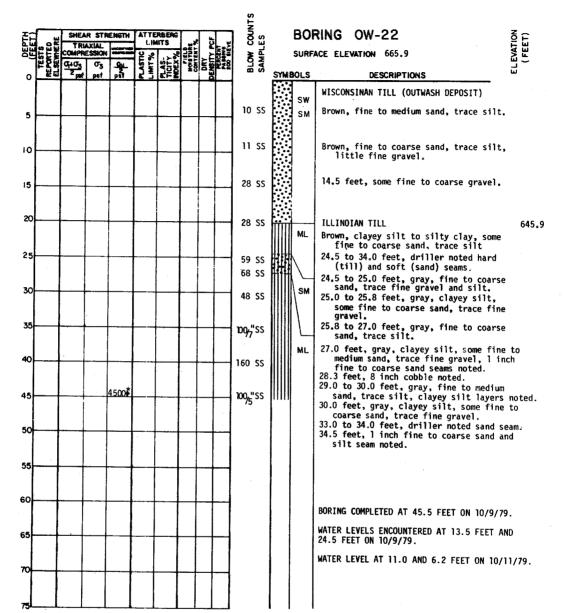


PIEZOMETER INSTALLED ON 10/8/79. A 2 INCH PVC PIPE WITH A 2 FOOT PRE-SLOTTED SCREEN ATTACHED TO A 7 FOOT PERFORATED LOWER SECTION WITH THE END PLUGGED WAS PLACED AT ELEVATION 615; GRANULAR FILL WAS PLACED FROM ELEVATION 615 TO 665; BENTONITE SEAL FROM ELEVATION 665 TO 657; AND GROUT FROM ELEVATION 667 TO 670.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

WATER LEVEL AT 6.2 FEET ON 10/11/79.

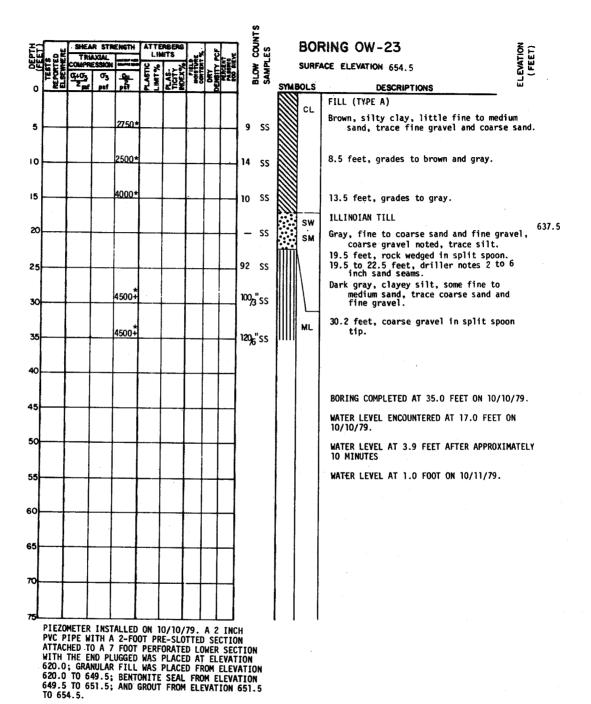
FIGURE 2.5-442



TWO PIEZOMETERS (OM-22A AND 22B) WERE INSTALLED IN BORE HOLE ON 10/9/79. OW-22A CONSISTED OF A 2 INCH PVC PIPE WITH A 2 FOOT PRE-SLOTTED SECTION ATTACHED TO A 7 FOOT PERFORATED LOWER SECTION WITH THE END OF THE LOWER SECTION PLUGGED WAS PLACED AT ELEVATION 621.4; GRANULAR FILL WAS PLACED FROM ELEVATION 621.4 TO 642.9; AND A BENTONITE SEAL FROM ELEVATION 642.9 TO 645.9. OM-22B CONSISTED OF A 2 INCH PVC PIPE WITH THE LOWER 7 FEET PERFORATED AND THE LOWER END PLUGGED WAS PLACED AT ELEVATION 645.9; GRANULAR FILL WAS PLACED FROM ELEVATION 645.9 TO 660.4; BENTONITE SEAL FROM ELEVATION 660.4 TO 663.9; AND GROUT FROM ELEVATION 663.9 TO 665.9.

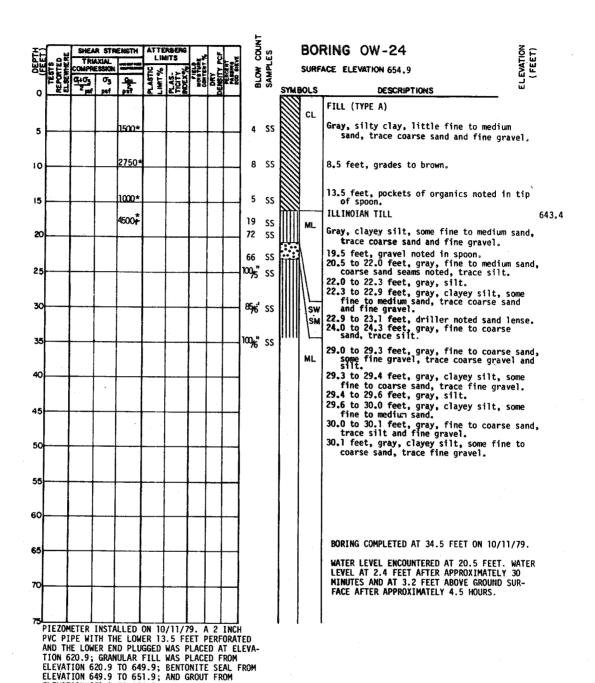
## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-443



## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

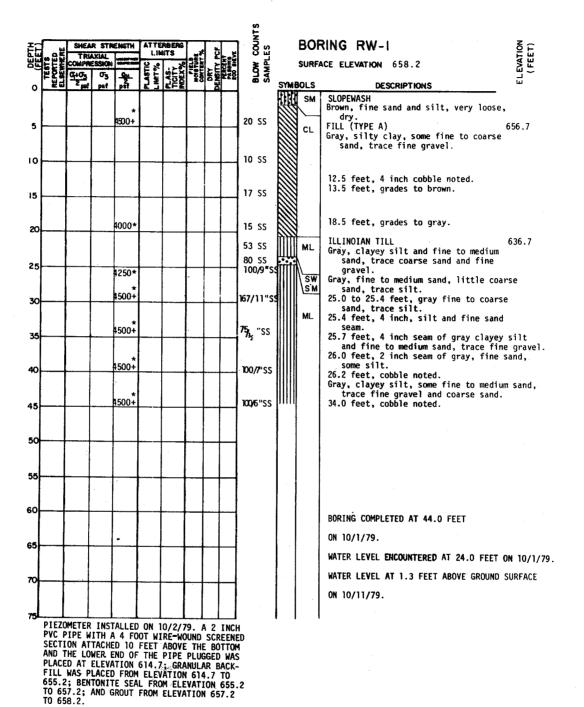
FIGURE 2.5-444



ELEVATION 651.9 TO 654.9

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-445



TO 658.2.

#### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-446

PIEZOMETER INSTALLED ON 10/2/79. A 2 INCH PVC PIPE WITH A 4 FOOT MIRE-WOUND SCREENED SECTION ATTACHED 10 FEET ABOVE THE BOTTOM AND THE LOWER END OF THE PIPE PLUGGED AT ELEVATION 613.4; GRANULAR BACKFILL WAS PLACED FROM ELEVATION 613.4 TO 653.9; BENTONITE SEAL FROM ELEVATION 653.9 TO 655.9; AND GROUT FROM ELEVATION 655.9 TO 656.9.

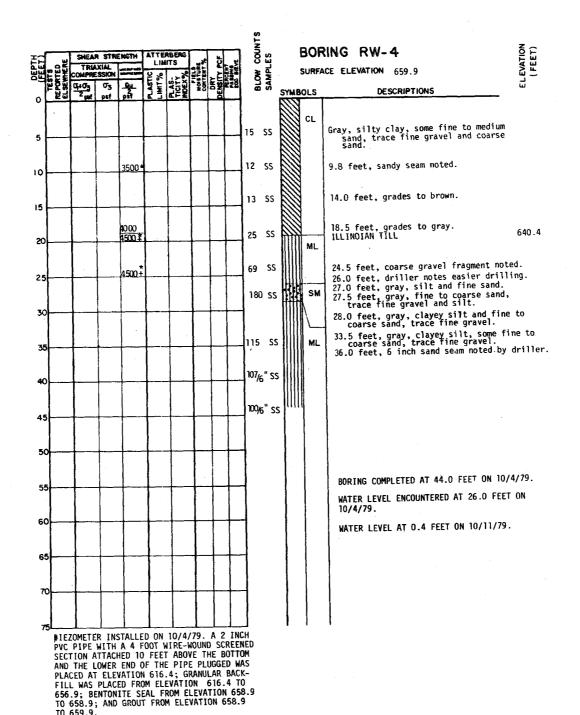
## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-447

PIEZOMETER INSTALLED ON 10/3/79. A 2 INCH PVC PIPE WITH A 4 FOOT WIRE-WOUND SCREENED SECTION ATTACHED 10 FEET ABOVE THE BOTTOM AND THE LOWER END OF THE PIPE PLUGGED WAS PLACED AT ELEVATION 612.3: GRANULAR BACKFILL WAS PLACED FROM ELEVATION 612.3 TO 652.8; BENTONITE SEAL FROM ELEVATION 652.8 TO 654.8; AND GROUT FROM ELEVATION 654.8 TO 655.8.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

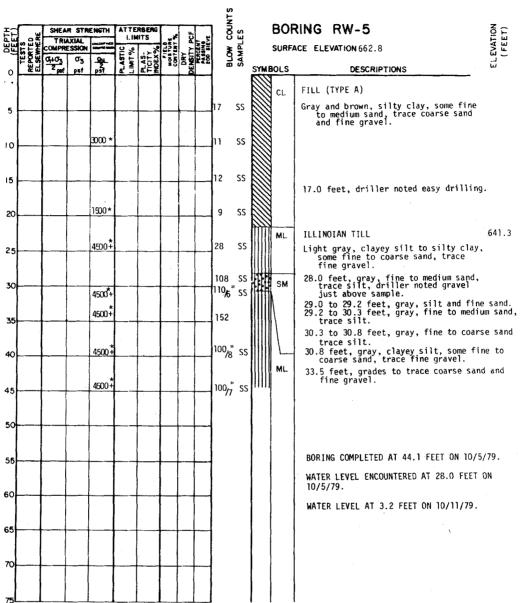
FIGURE 2.5-448



TO 659.9.

#### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

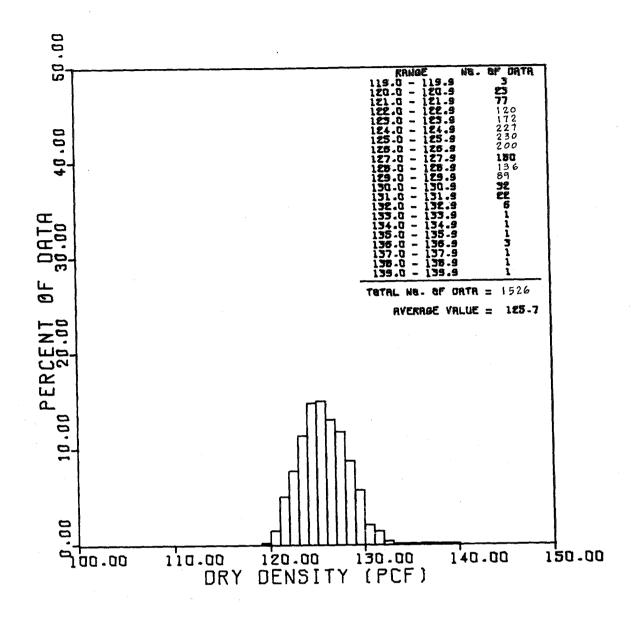
FIGURE 2.5-449



PIEZOMETER INSTALLED ON 10/5/79. A 2 INCH PVC PIPE WITH A 4 FOOT WIRE-WOUND SCREENED SECTION ATTACHED 10 FEET ABOVE THE BOTTOM AND THE LOWER END OF THE PIPE PLUGGED WAS PLACED AT ELEVATION 619.3; GRANULAR BACK-FILL WAS PLACED FROM ELEVATION 619.3 TO 659.8; BENTONITE SEAL FROM ELEVATION 659.8 TO 661.8; AND GROUT FROM ELEVATION 661.8 TO 662.8.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-450

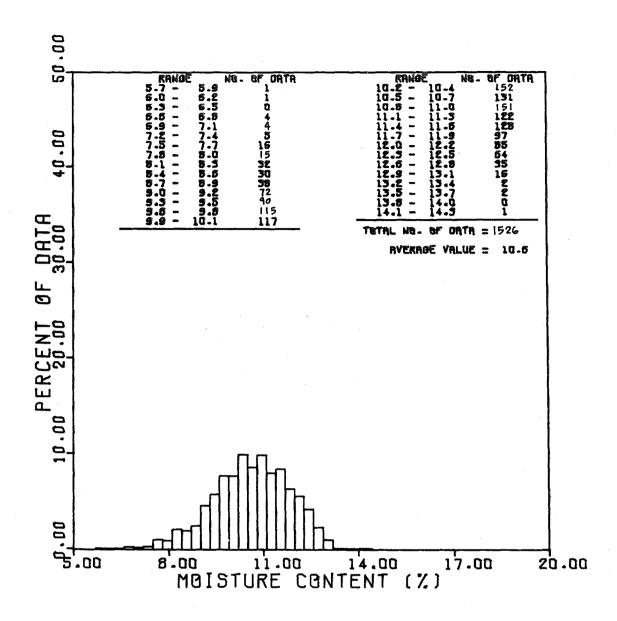


CLINTON STATION IN-PLACE TEST SUBMERGED DAM ELEVATION FROM 645.0 TO 673.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-451

UHS DAM TYPE A COHESIVE FILL DISTRIBUTION OF DRY DENSITY

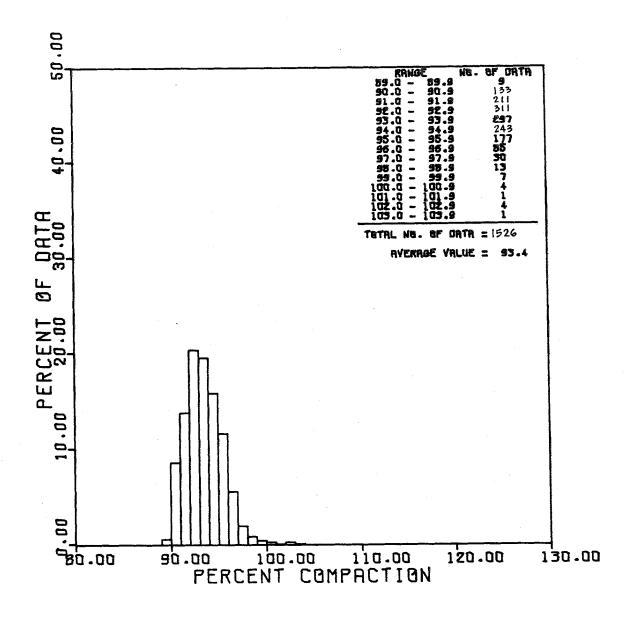


CLINTON STATION IN-PLACE TEST SUBMERGED DAM ELEVATION FROM 645.0 TO 673.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-452

UHS DAM TYPE A COHESIVE FILL DISTRIBUTION OF MOISTURE CONTENT

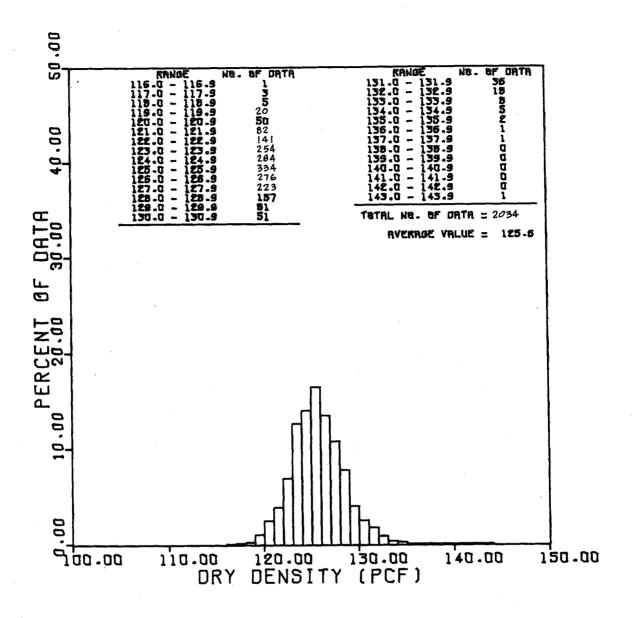


CLINTON STATION IN-PLACE TEST SUBMERGED DAM ELEVATION FROM 645.0 TO 673.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-453

UHS DAM TYPE A COHESIVE FILL DISTRIBUTION OF PERCENT COMPACTION

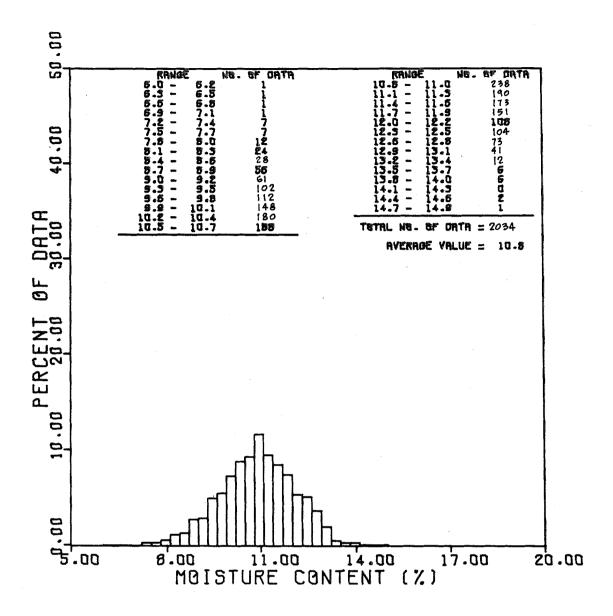


CLINTON STATION IN-PLACE TEST BAFFLE DIKE ELEVATION FROM 649.0 TO 699.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-454

UHS BAFFLE DIKE TYPE A COHESIVE FILL DISTRIBUTION OF DRY DENSITY

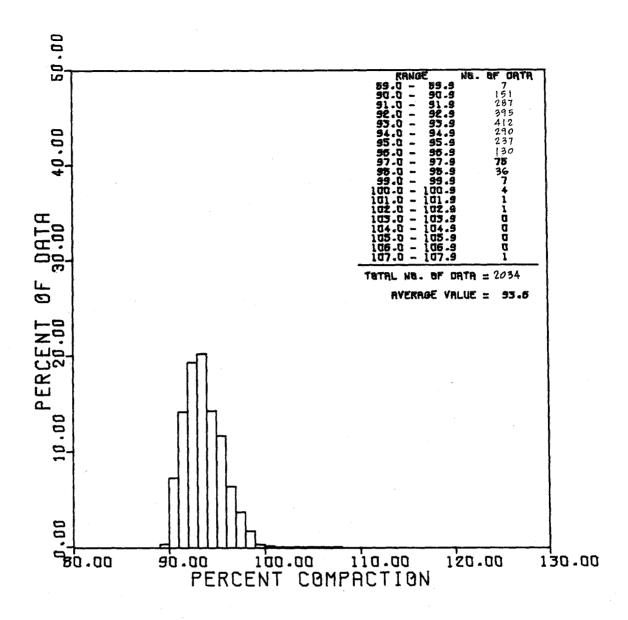


CLINTON STATION IN-PLACE TEST BAFFLE DIKE ELEVATION FROM 649.0 TO 699.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-455

UHS BAFFLE DIKE TYPE A COHESIVE FILL DISTRIBUTION OF MOISTURE CONTENT

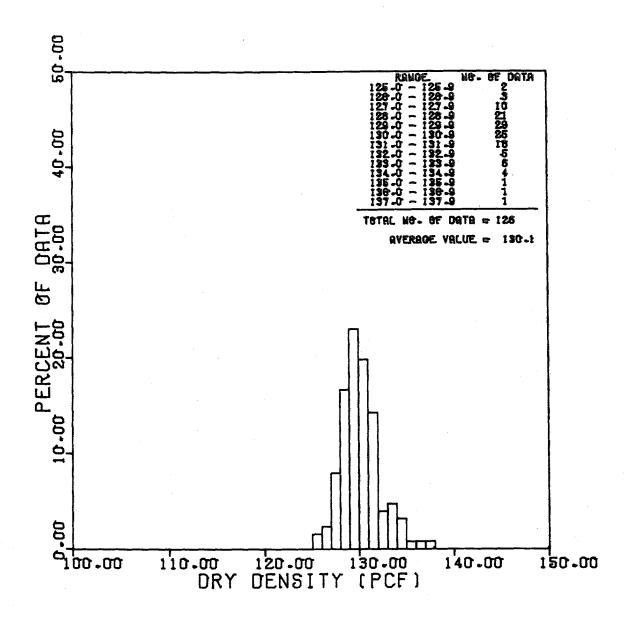


CLINTON STATION IN-PLACE TEST BAFFLE DIKE ELEVATION FROM 649.0 TO 699.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-456

UHS BAFFLE DIKE TYPE A COHESIVE FILL DISTRIBUTION OF PERCENT COMPACTION

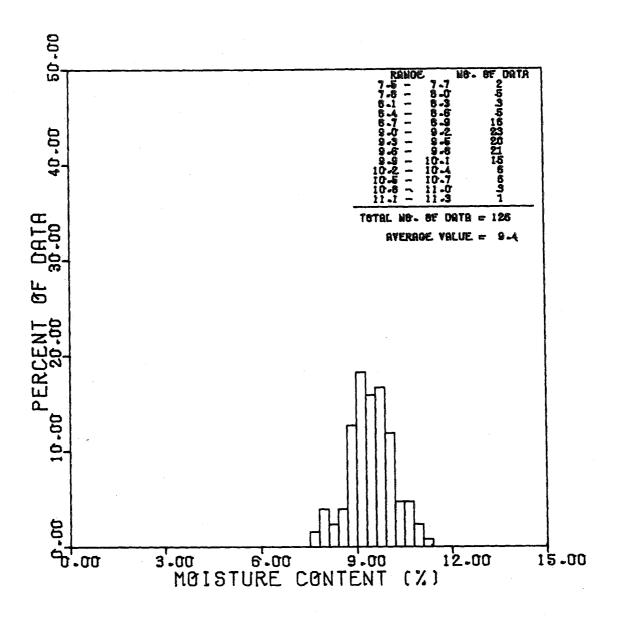


CLINTON STATION IN-PLACE TEST BAFFLE DIKE - SOIL CEMENT ELEVATION FROM 673.0 TO 676.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-457

UHS BAFFLE DIKE SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF DRY DENSITY

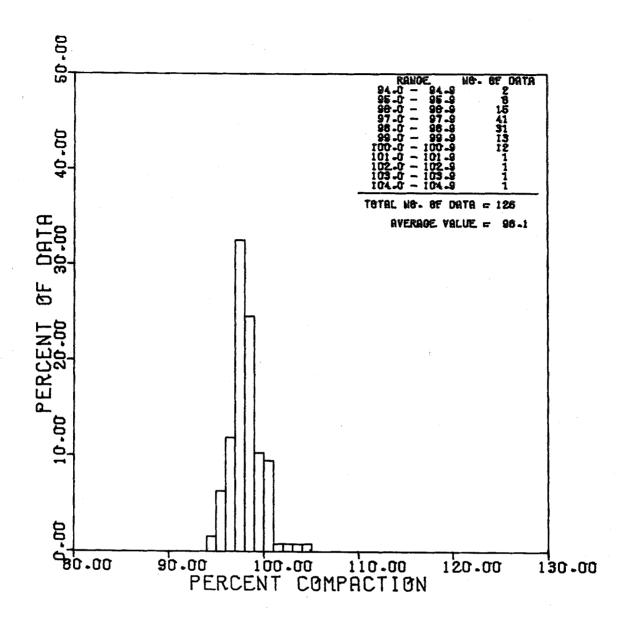


CLINTON STATION IN-PLACE TEST BAFFLE DIKE - SOIL CEMENT ELEVATION FROM 673.0 TO 676.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-458

UHS BAFFLE DIKE SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF MOISTURE CONTENT

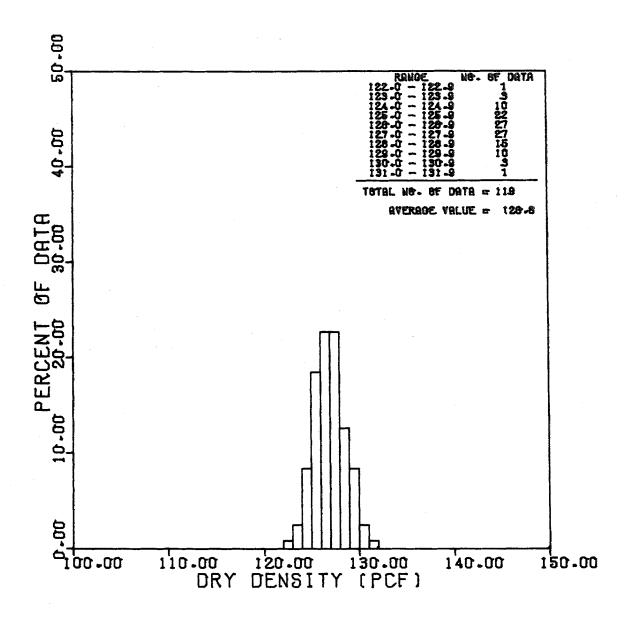


CLINTON STATION IN-PLACE TEST BAFFLE DIKE - SOIL CEMENT ELEVATION FROM 673.0 TO 676.99

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-459

UHS BAFFLE DIKE SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF PERCENT COMPACTION



# CLINTON STATION IN-PLACE TEST SOUTH DAM USDSCR-SOIL CEMENT ELEVATION FROM 649.0 TO 697.99

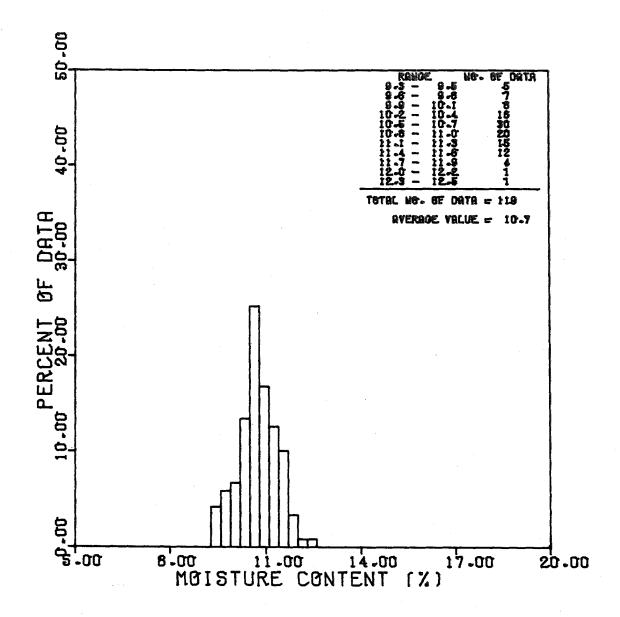
#### NOTE:

ONLY TESTS FOR THE UPSTREAM FACE, CREST AND DOWNSTREAM SLOPE ARE INCLUDED IN THIS ANALYSIS.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-460

UHS DAM SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF DRY DENSITY



CLINTON STATION IN-PLACE TEST SOUTH DAM USDSCR-SOIL CEMENT ELEVATION FROM 649.0 TO 697.99

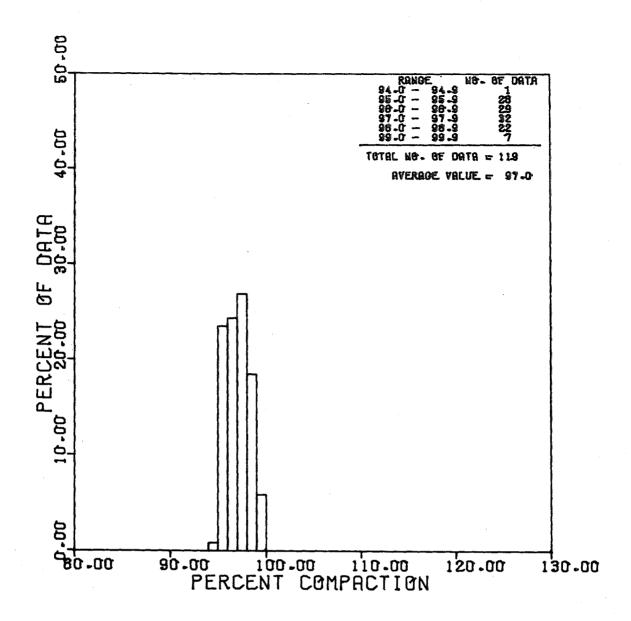
#### NOTE:

ONLY TESTS FOR THE UPSTREAM FACE, CREST AND DOWNSTREAM SLOPE ARE INCLUDED IN THIS ANALYSIS.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-461

UHS DAM SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF MOISTURE CONTENT



CLINTON STATION IN-PLACE TEST SOUTH DAM USDSCR-SOIL CEMENT ELEVATION FROM 649.0 TO 697.99

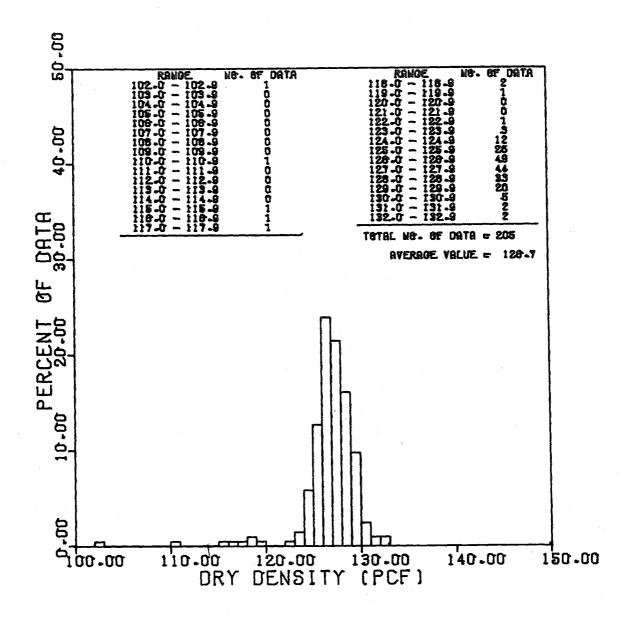
#### NOTE:

ONLY TESTS FOR THE UPSTREAM FACE, CREST AND DOWNSTREAM SLOPE ARE INCLUDED IN THIS ANALYSIS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-462

UHS DAM SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF PERCENT COMPACTION



CLINTON STATION IN-PLACE TEST SOUTH DAM - SOIL CEMENT ELEVATION FROM 649.0 TO 697.99

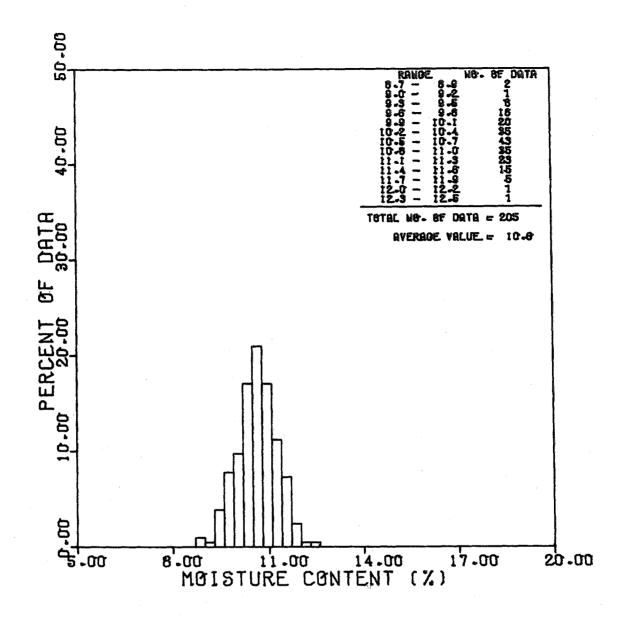
#### NOTE:

TESTS FOR ALL AREAS, INCLUDING THE ABUTMENTS AND DOWNSTREAM FLAT AREA, ARE INCLUDED IN THIS ANALYSIS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-463

UHS DAM SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF DRY DENSITY



CLINTON STATION IN-PLACE TEST SOUTH DAM - SOIL CEMENT ELEVATION FROM 649.0 TO 697.99

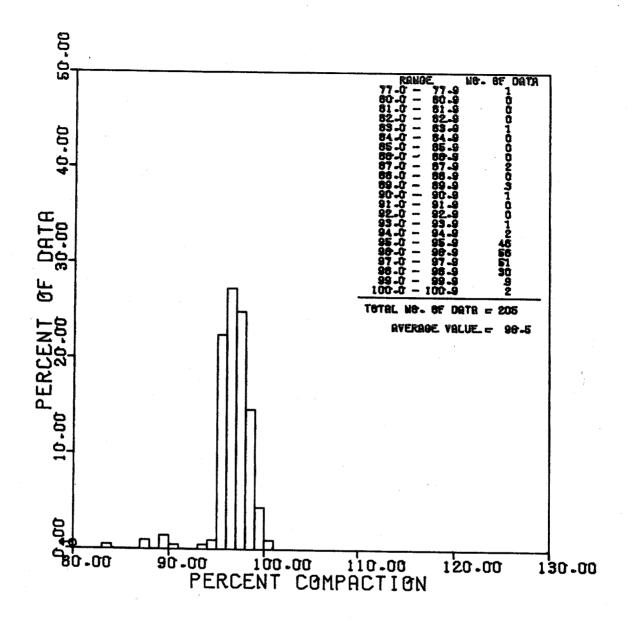
#### NOTE:

TESTS FOR ALL AREAS, INCLUDING THE ABUTMENTS AND DOWNSTREAM FLAT AREA, ARE INCLUDED IN THIS ANALYSIS.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-464

UHS DAM SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF MOISTURE CONTENT



### CLINTON STATION IN-PLACE TEST SOUTH DAM - SOIL CEMENT ELEVATION FROM 649.0 TO 697.99

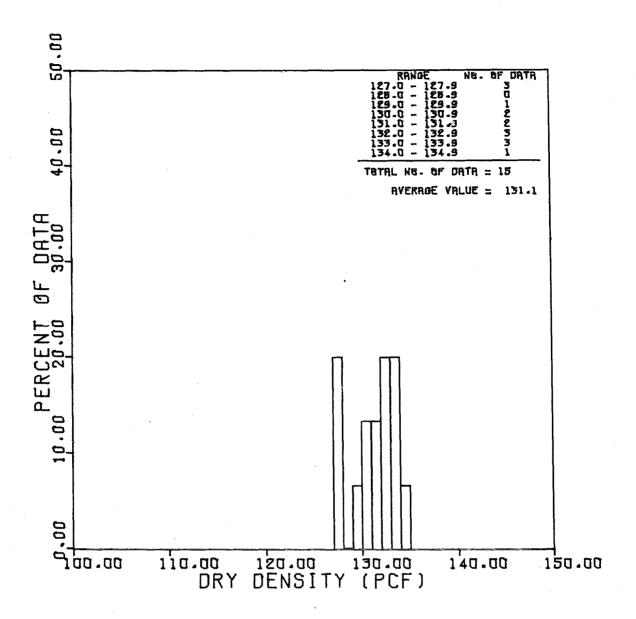
#### NOTE:

Tests for all areas, including the abutments and the downstream flat area are included in this analysis.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-465

UHS DAM SOIL CEMENT SLOPE PROTECTION DISTRIBUTION OF PERCENT COMPACTION

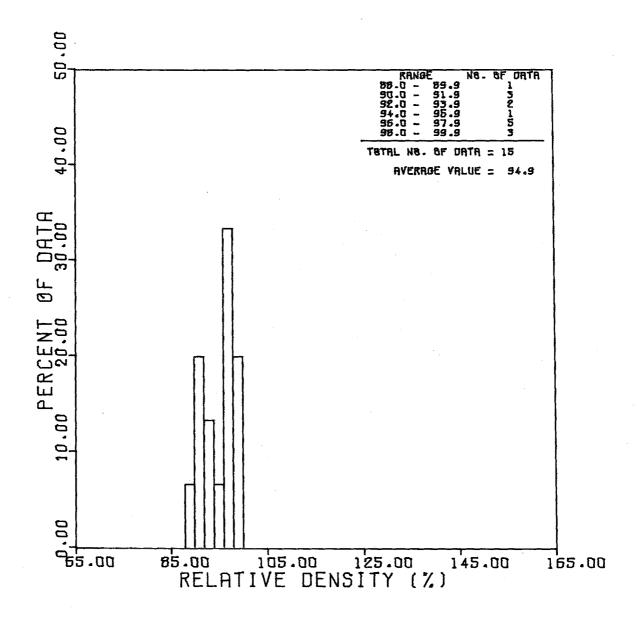


CLINTON POWER STATION
ALL DATA
OUTLET - P & R SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-466

SSWS OUTLET STRUCTURE
GRANULAR FILL - DISTRIBUTION OF
DRY DENSITY

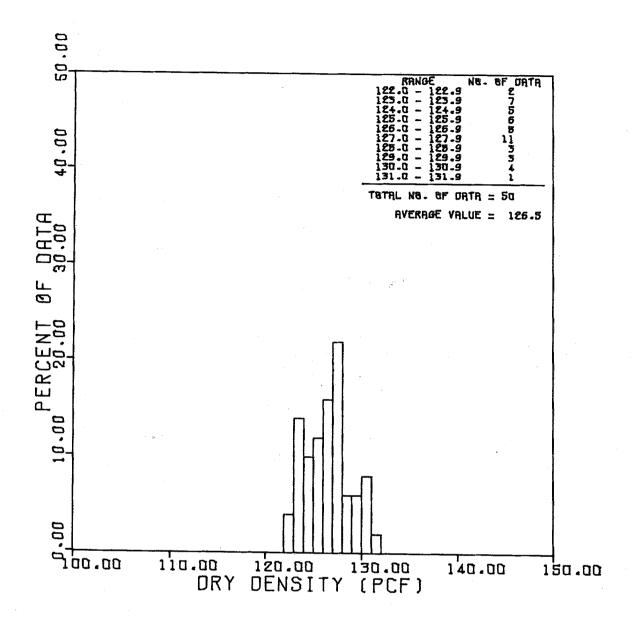


CLINTON POWER STATION
ALL DATA
OUTLET - P & R SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-467

SSWS OUTLET STRUCTURE
GRANULAR FILL - DISTRIBUTION OF
RELATIVE DENSITY

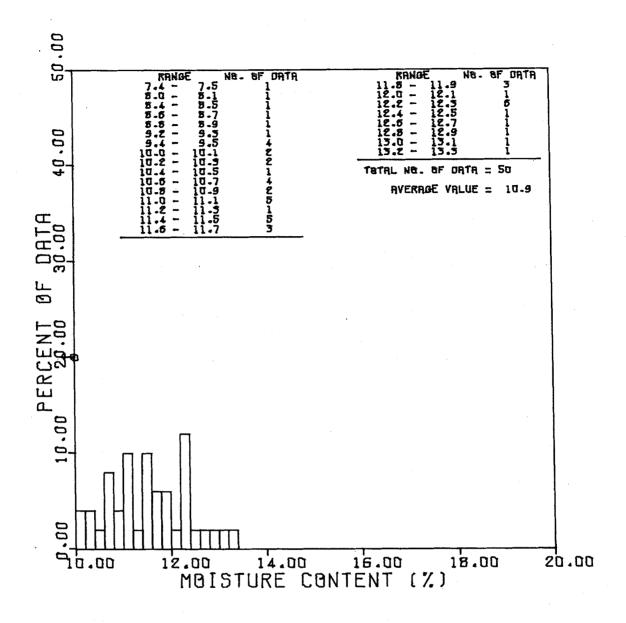


CLINTON POWER STATION
ALL DATA
PIPELINE - PB SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-468

SSWS OUTLET STRUCTURE
COHESIVE BACKFILL - DISTRIBUTION OF
DRY DENSITY

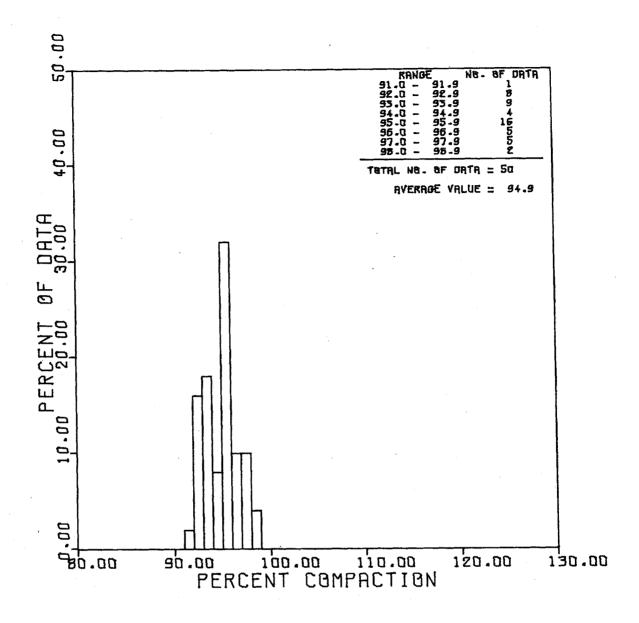


CLINTON POWER STATION
ALL DATA
PIPELINE - PB SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-469

SSWS OUTLET STRUCTURE
COHESIVE BACKFILL - DISTRIBUTION OF
MOISTURE CONTENT

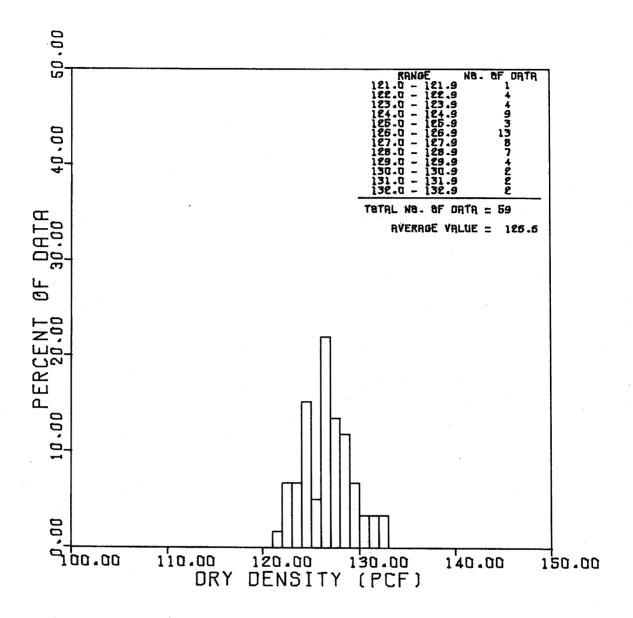


CLINTON POWER STATION
ALL DATA
PIPELINE - PB SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-470

SSWS OUTLET STRUCTURE
COHESIVE BACKFILL - DISTRIBUTION OF
PERCENT COMPACTION

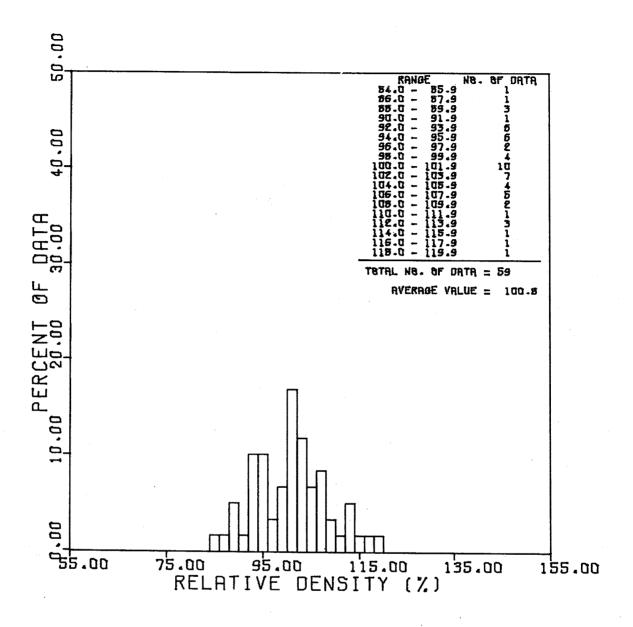


CLINTON POWER STATION
ALL DATA
PIPELINE - P SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-471

SSWS PIPELINE GRANULAR FILL DISTRIBUTION OF DRY DENSITY

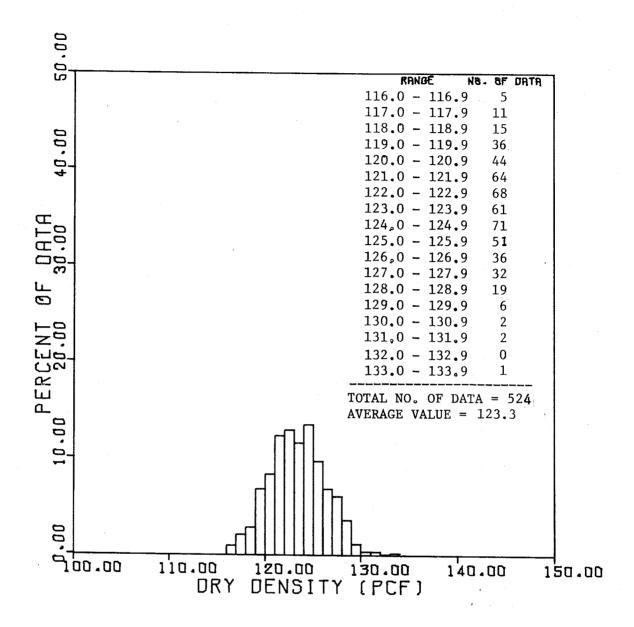


CLINTON POWER STATION
ALL DATA
PIPELINE - P SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-472

SSWS PIPELINE GRANULAR FILL - DISTRIBUTION OF RELATIVE DENSITY

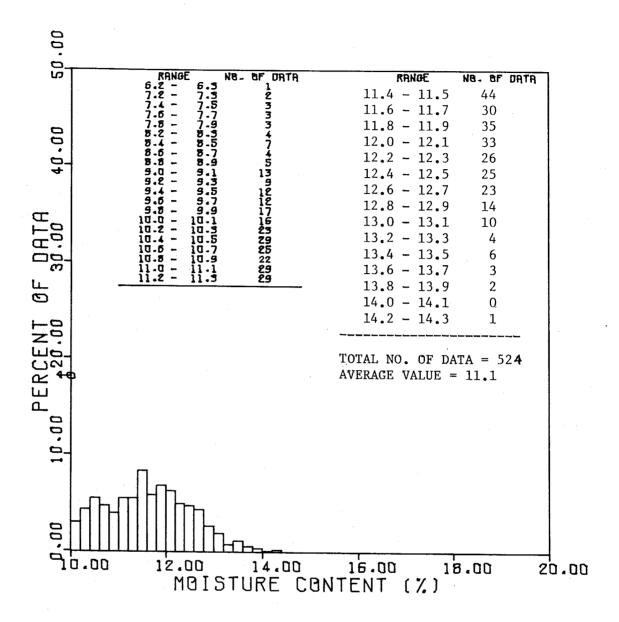


CLINTON POWER STATION
ALL DATA
PIPELINE - PB SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-473

SSWS PIPELINE COHESIVE FILL - DISTRIBUTION OF DRY DENSITY

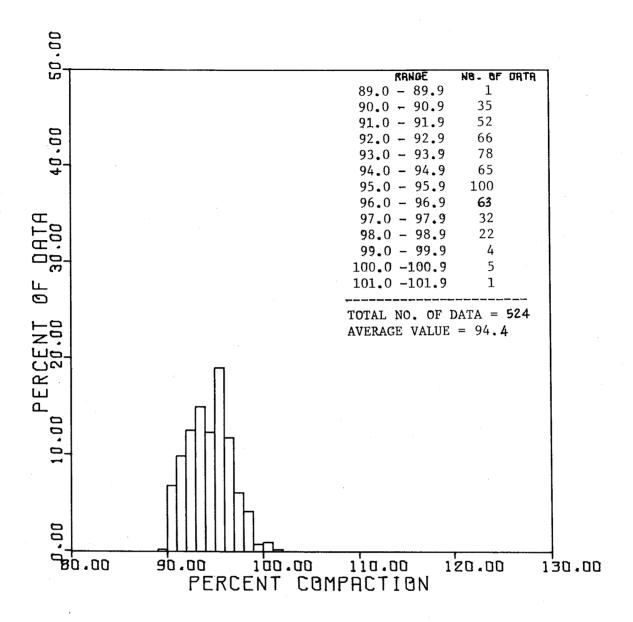


CLINTON POWER STATION
ALL DATA
PIPELINE - PB SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-474

SSWS PIPELINE COHESIVE FILL - DISTRIBUTION OF MOISTURE CONTENT

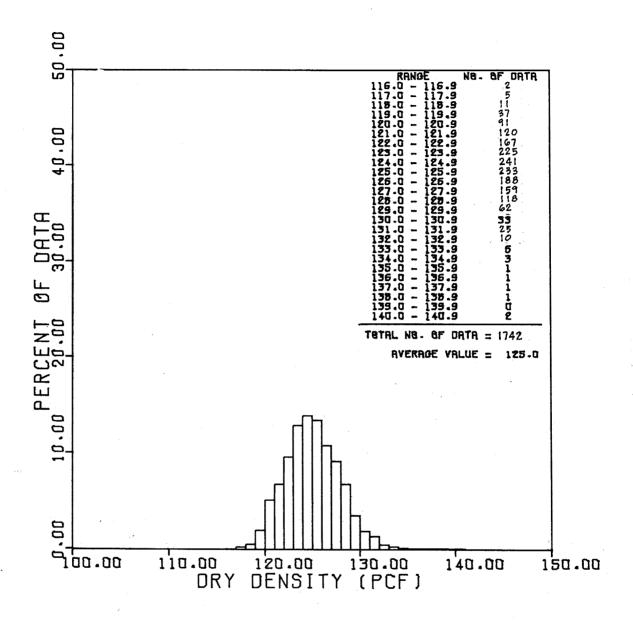


CLINTON POWER STATION
ALL DATA
PIPELINE - PB SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-475

SSWS PIPELINE COHESIVE FILL - DISTRIBUTION OF PERCENT COMPACTION



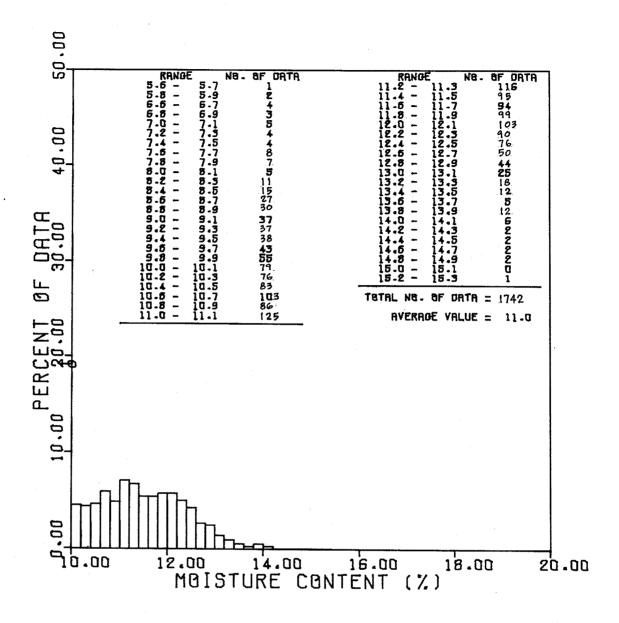
CLINTON

POWER STATION
ALL DATA
SCREEN HOUSE

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-476

SCREEN HOUSE COHESIVE BACKFILL - DISTRIBUTION OF DRY DENSITY



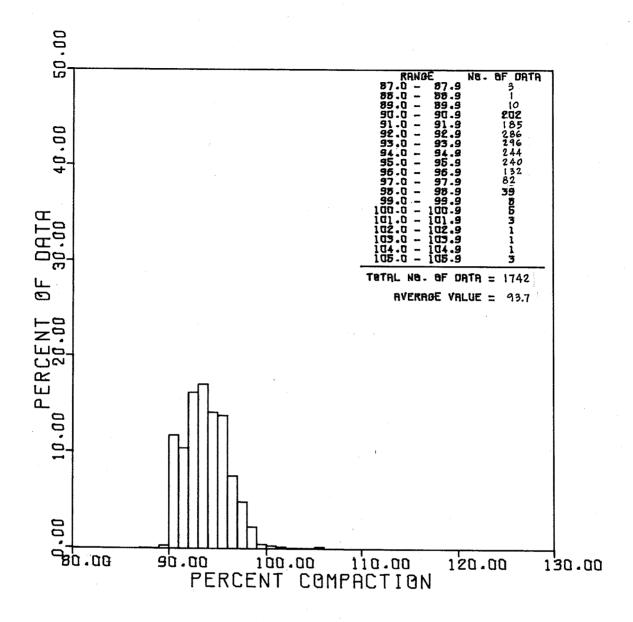
CLINTON

POWER STATION
ALL DATA
SCREEN HOUSE

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-477

SCREEN HOUSE COHESIVE BACKFILL - DISTRIBUTION OF MOISTURE CONTENT



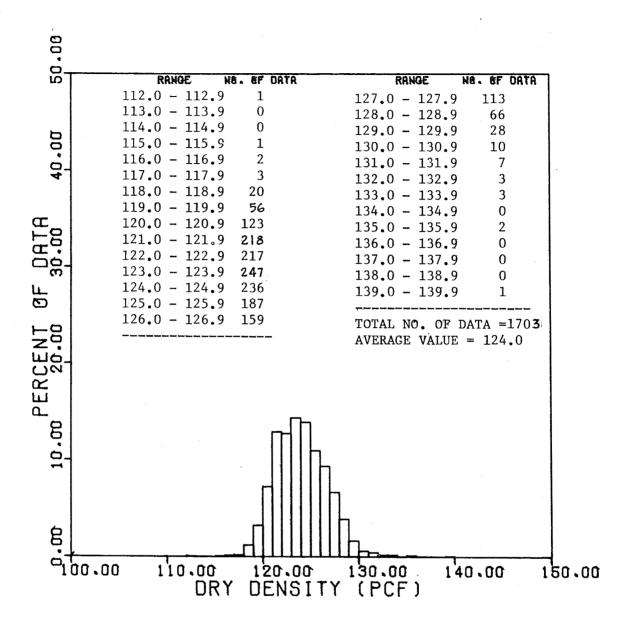
CLINTON

POWER STATION
ALL DATA
SCREEN HOUSE

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-478

SCREEN HOUSE COHESIVE BACKFILL - DISTRIBUTION OF PERCENT COMPACTION

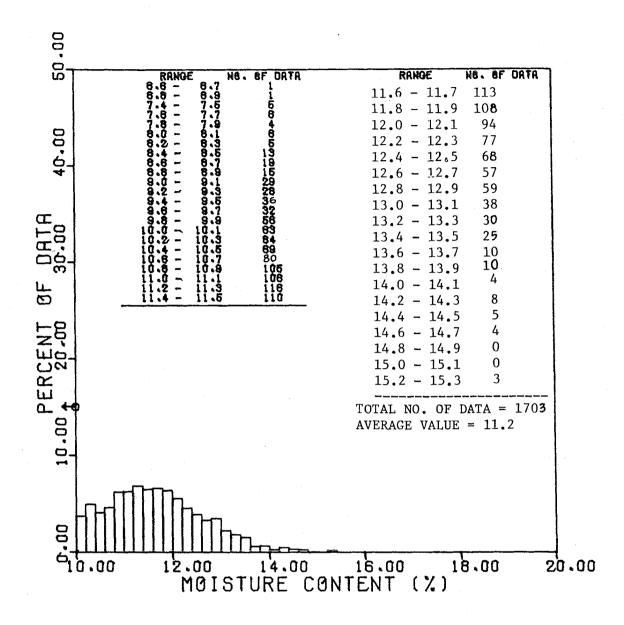


CLINTON POWER STATION
ALL DATA
POWER - PBT SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-479

MAIN PLANT COHESIVE BACKFILL - DISTRIBUTION OF DRY DENSITY

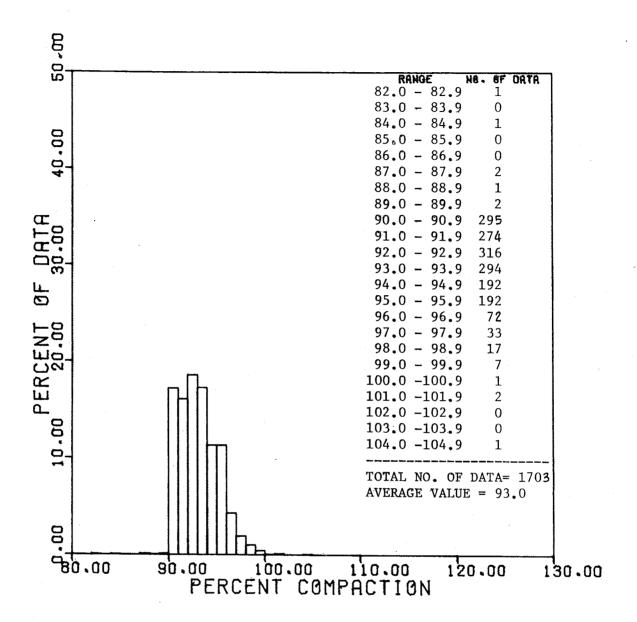


CLINTON POWER STATION
ALL DATA
POWER - PBT SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-480

MAIN PLANT COHESIVE BACKFILL -DISTRIBUTION OF MOISTURE CONTENT

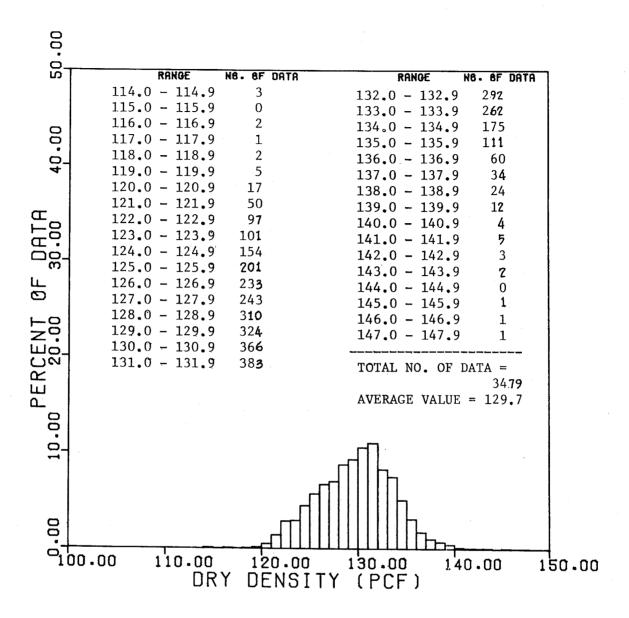


CLINTON POWER STATION
ALL DATA
POWER - PBT SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-481

MAIN PLANT COHESIVE BACKFILL - DISTRIBUTION OF PERCENT COMPACTION

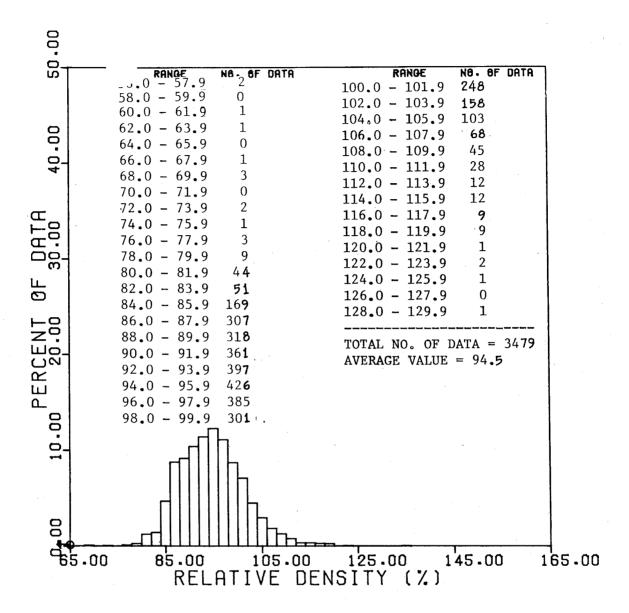


# CLINTON POWER STATION ALL DATA POWER-P SERIES

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-482

MAIN PLANT GRANULAR BACKFILL - DISTRIBUTION OF DRY DENSITY



# CLINTON POWER STATION ALL DATA POWER-P SERIES

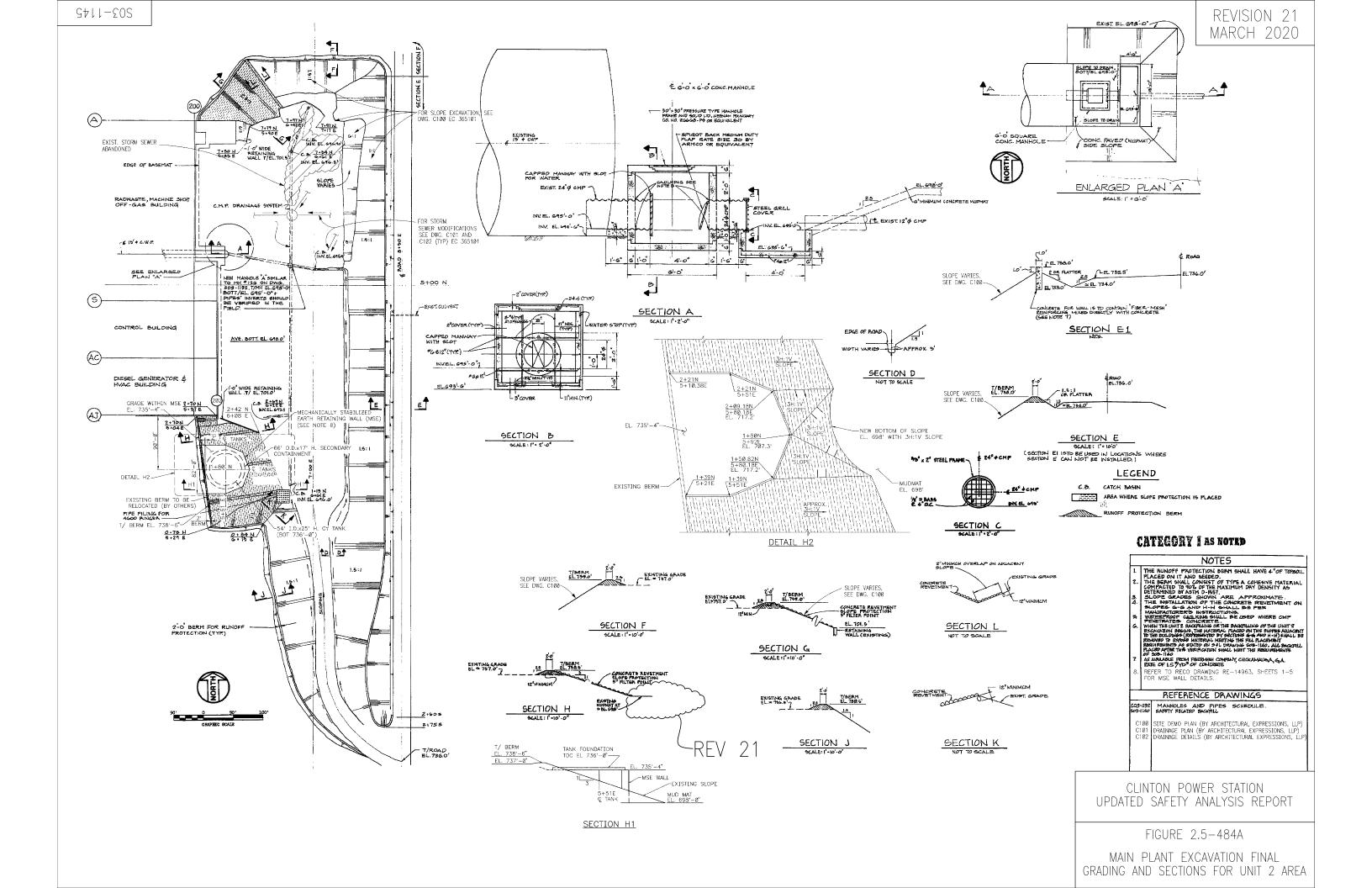
### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

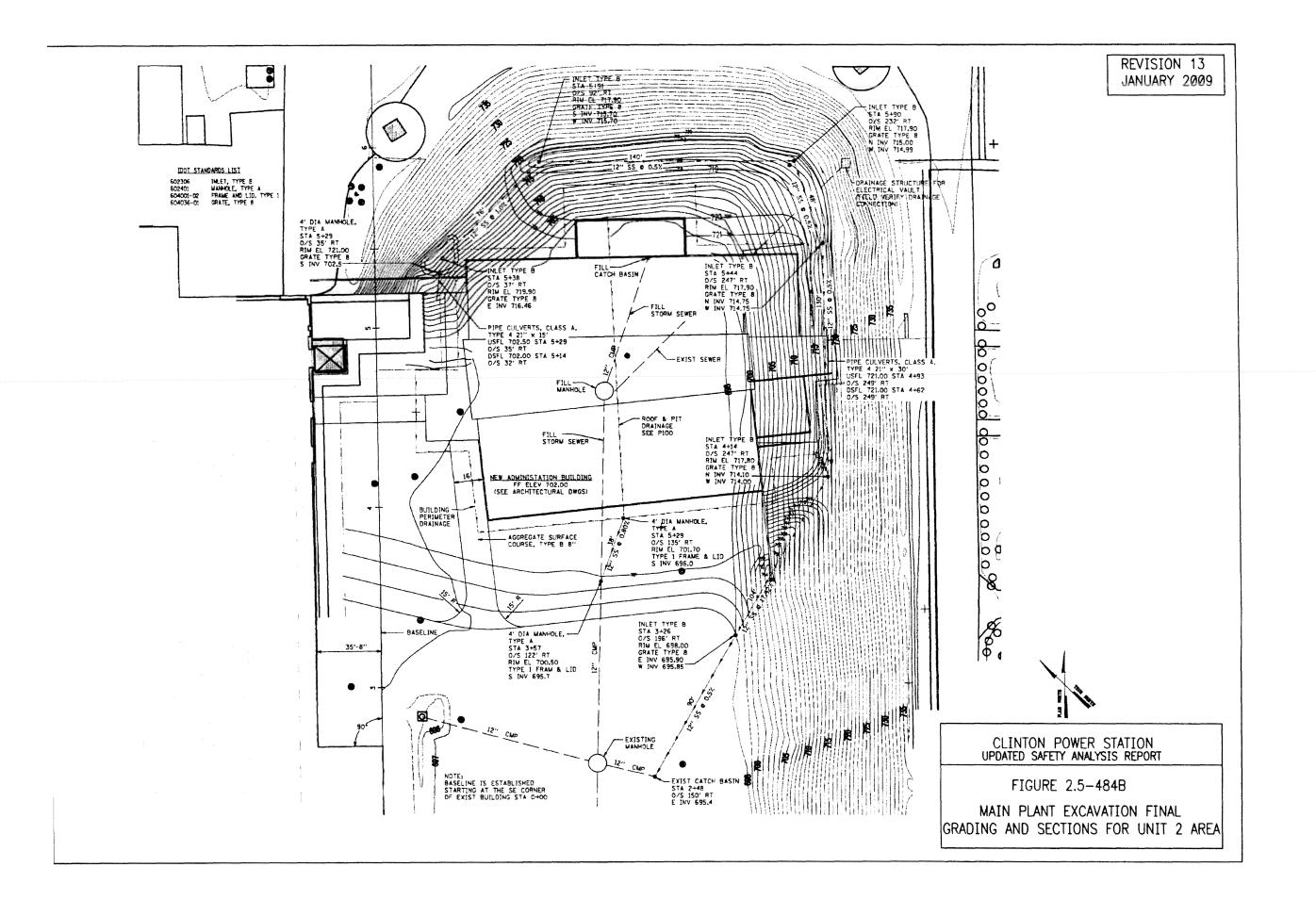
FIGURE 2.5-483

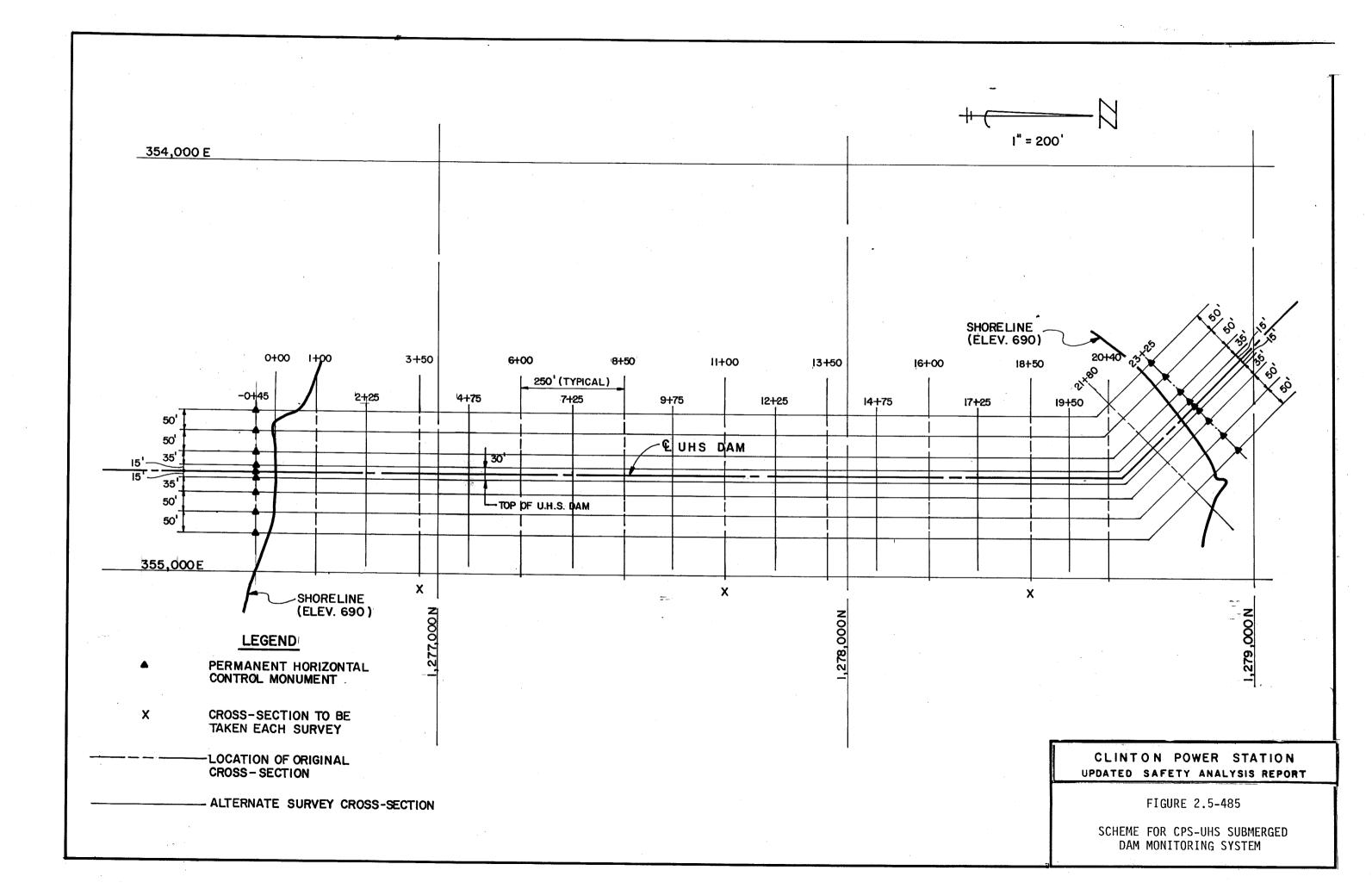
MAIN PLANT GRANULAR BACKFILL - DISTRIBUTION OF RELATIVE DENSITY

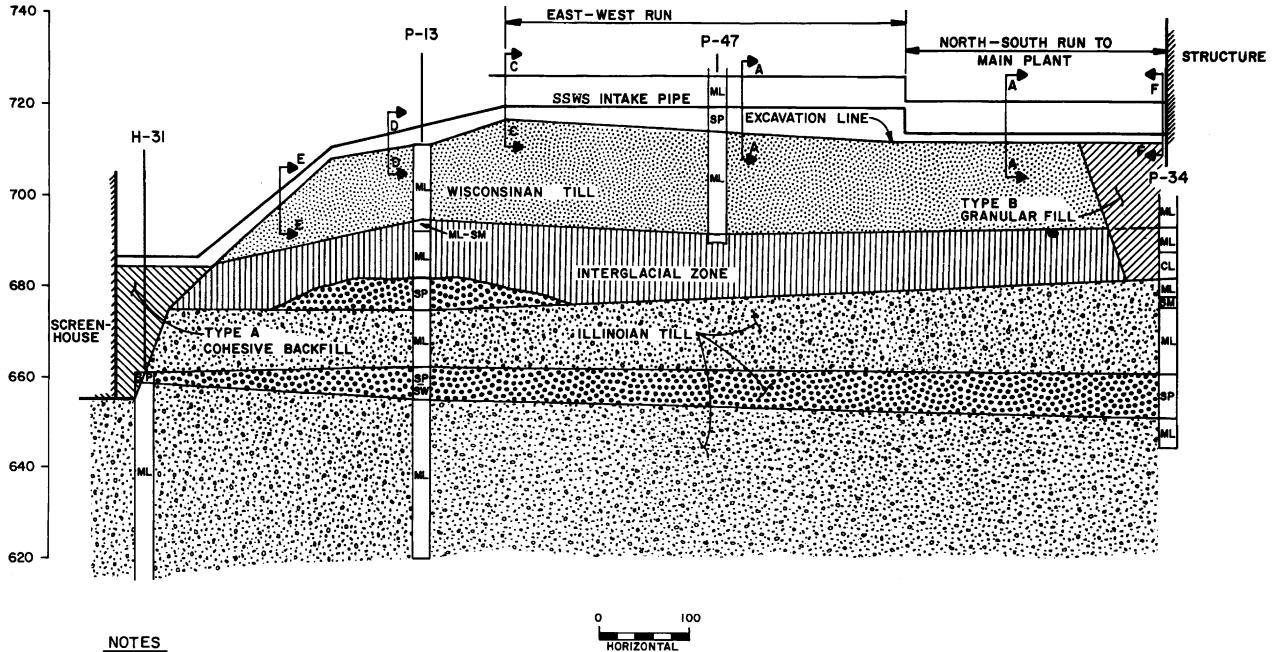
#### CPS/USAR

FIGURE 2.5-484 HAS BEEN DELETED









- 1. REFER TO FSAR FIGURE 2.5-372 FOR GEOLOGIC SECTION BELOW ELEVATION 620 FEET.
- 2. SEE FIGURE C2.5-23 FOR PLAN VIEW OF ECCS PIPELINE EXCAVATION.
- 3. SECTIONS SHOWN ON FIGURE 2.5-488,

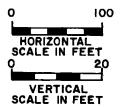
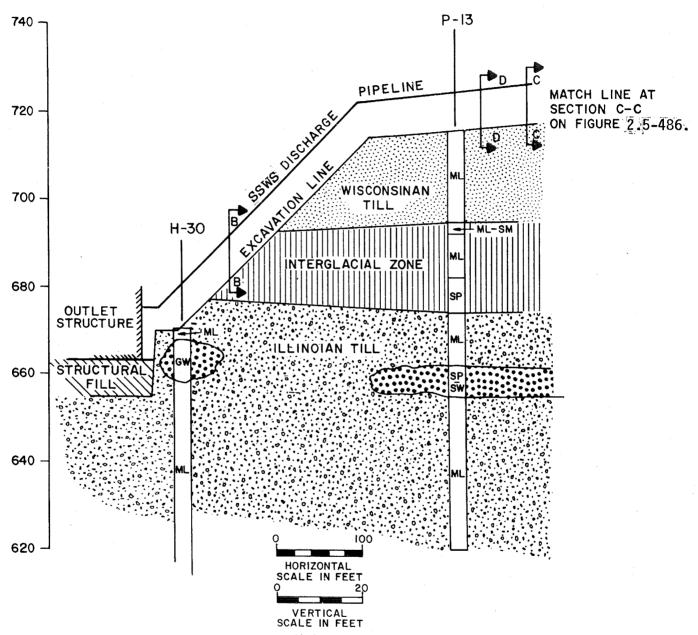


FIGURE 2.5-486

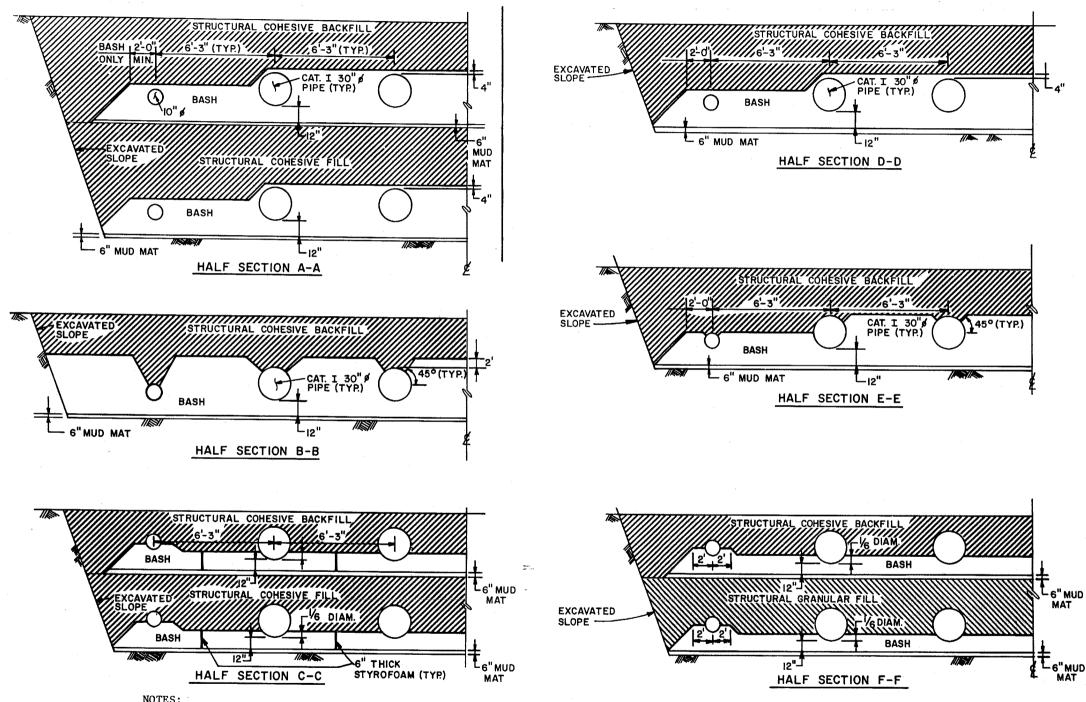
GEOLOGIC PROFILE ALONG (SSWS)
PIPELINE - SCREENHOUSE
TO MAIN PLANT



SEE NOTES ON FIGURE 2.5-486 FOR REFERENCE FIGURES.

FIGURE 2.5-487

GEOLOGIC PROFILE ALONG SSWS PIPELINE - OUTLET STRUCTURE TO MAIN PLANT



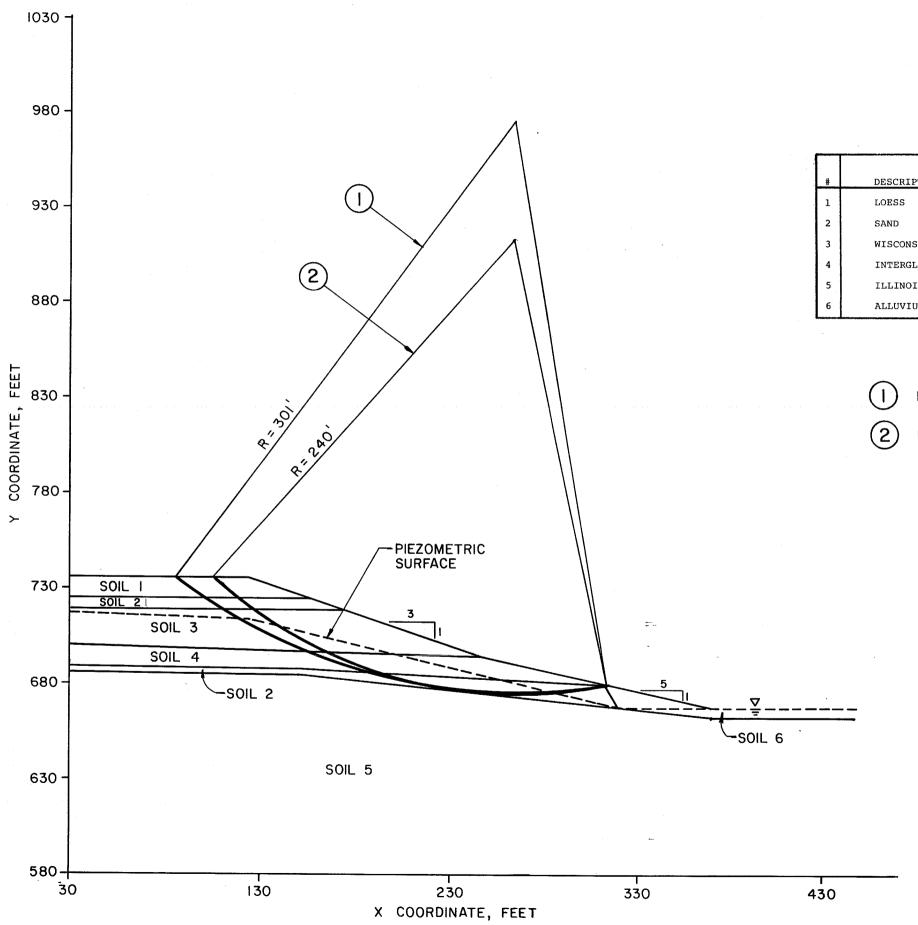
#### NOTES:

- 1. SECTIONS GIVEN ARE HALF SECTIONS AND ARE SYMMETRICAL ABOUT THE CENTERLINE.
- 2. SECTION C-C IS TYPICAL FOR ALL BEND LOCATIONS ALONG PIPELINE.
- 3. SECTION F-F IS FOR AREA IMMEDIATELY ADJACENT TO MAIN PLANT STRUCTURE ONLY.
- 4. LOCATION OF SECTIONS SHOWN ON FIGURES 2.5-486 AND 2.5-487.

#### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-488

TYPICAL CROSS SECTIONS SSWS PIPELINE



Γ			SOIL DATA		
L	#	DESCRIPTION	7 (PCF)	C'(PSF)	ø
	1	LOESS	120.0	0	20
	2	SAND	125.0	0	38
	3	WISCONSIN TILL	137.0	600	30
	4	INTERGLACIAL	131.0	600	30
1	5	ILLINOIAN TILL	150.0	0	47
	6	ALLUVIUM	120.0	120	38

() F.S: 1.21 (PSEUDO)

F.S: 2.42(STATIC)

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE 2.5-489

STABILITY ANALYSIS - END OF CONSTRUCTION CONDITION - SECTION Y-Y, ULTIMATE HEAT SINK

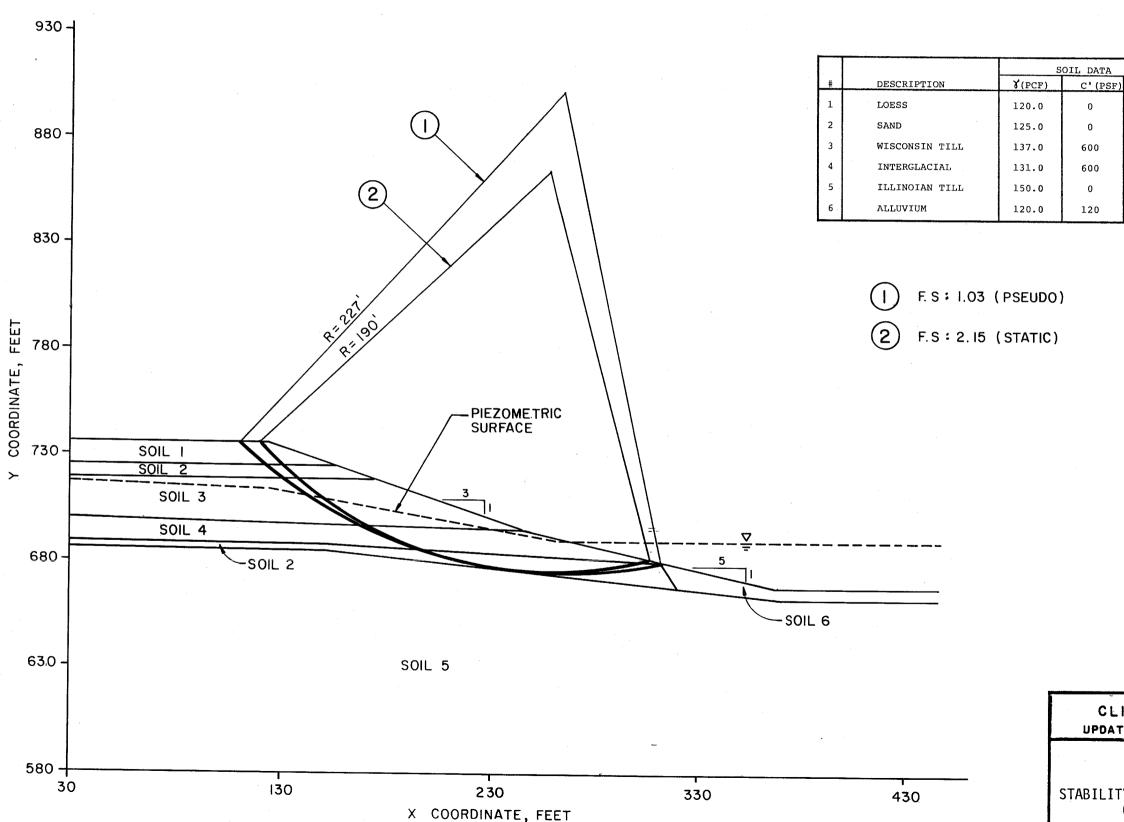
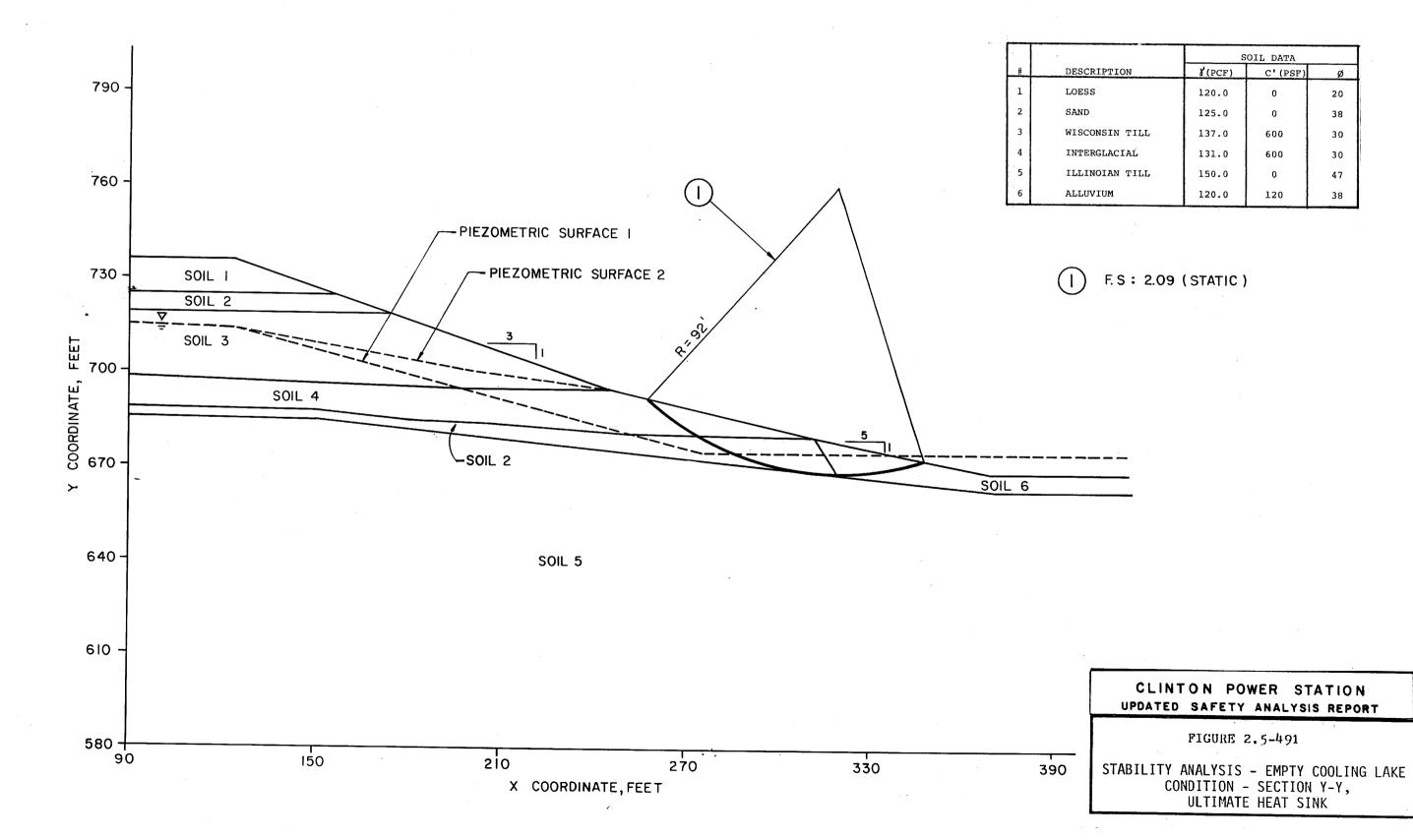


FIGURE 2.5-490

STABILITY ANALYSIS - FULL COOLING LAKE CONDITION - SECTION Y-Y, ULTIMATE HEAT SINK



C'(PSF)

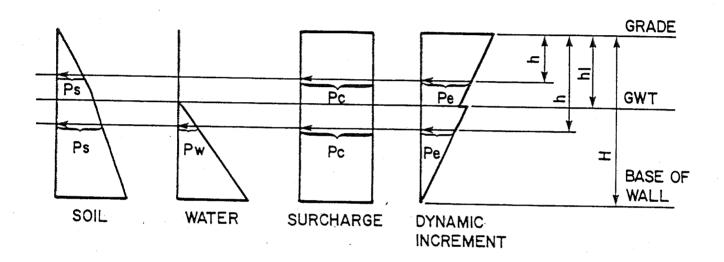
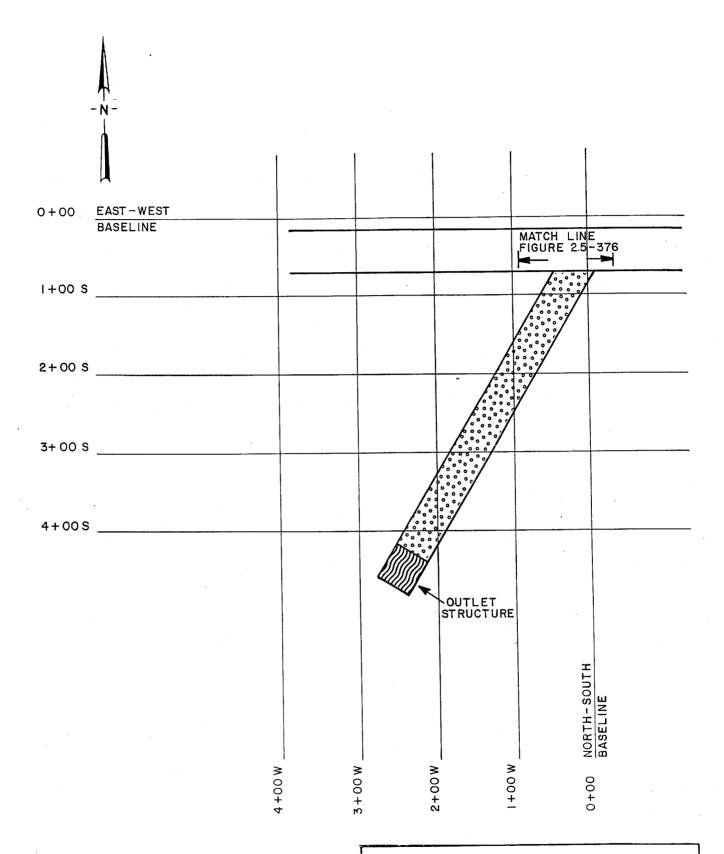


Figure 2.5-492 (Q & R 241.7)

LATERAL SOIL PRESSURES

#### CPS/USAR

Figures 2.5-493 through 2.5-495 Deleted

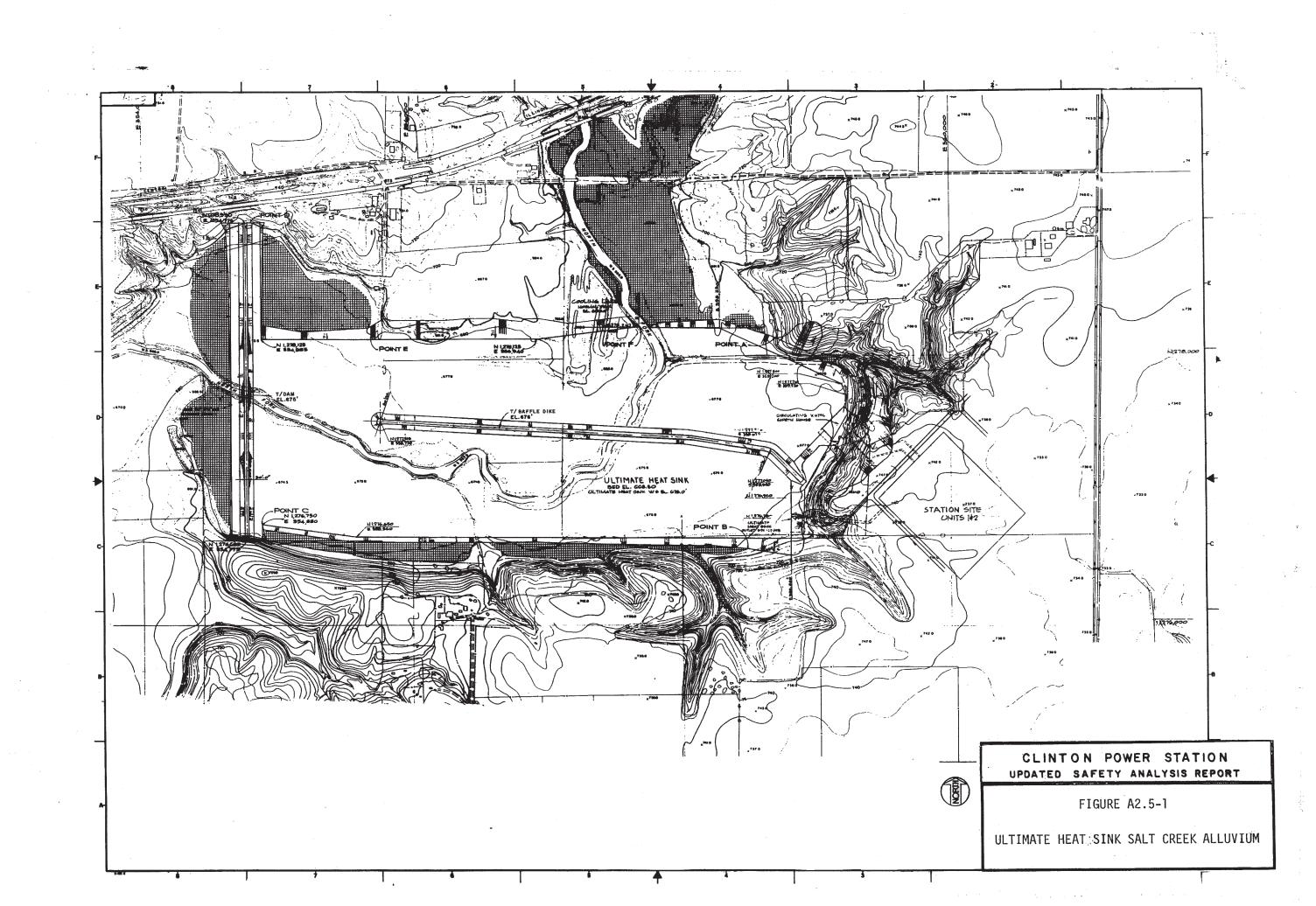


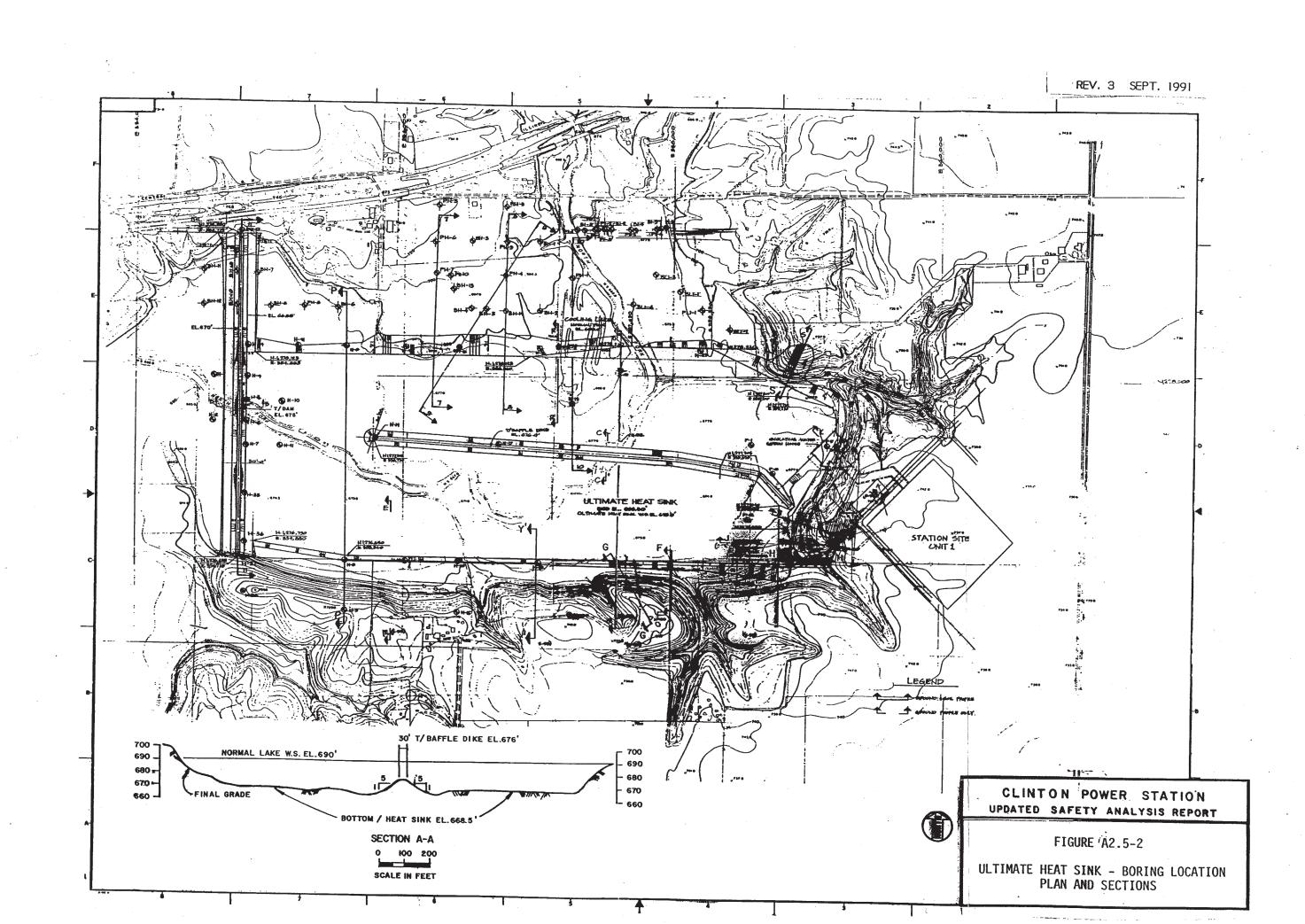
#### NOTE

SEE FIGURE 2.5-376 FOR KEY.

# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure 2.5-496 (O & R 241.8) FLY ASH MIXTURE AS FILL AND BACKFILL FOR THE SSWS DISCHARGE PIPELINE





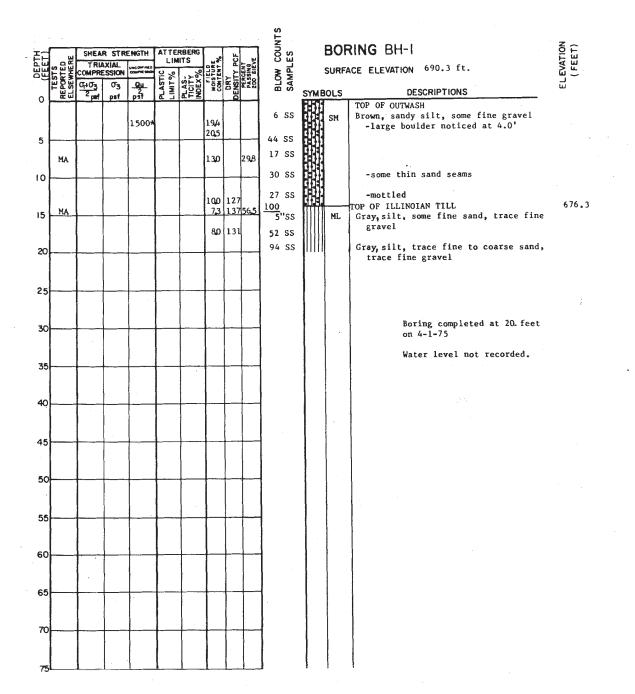


FIGURE A2.5-3

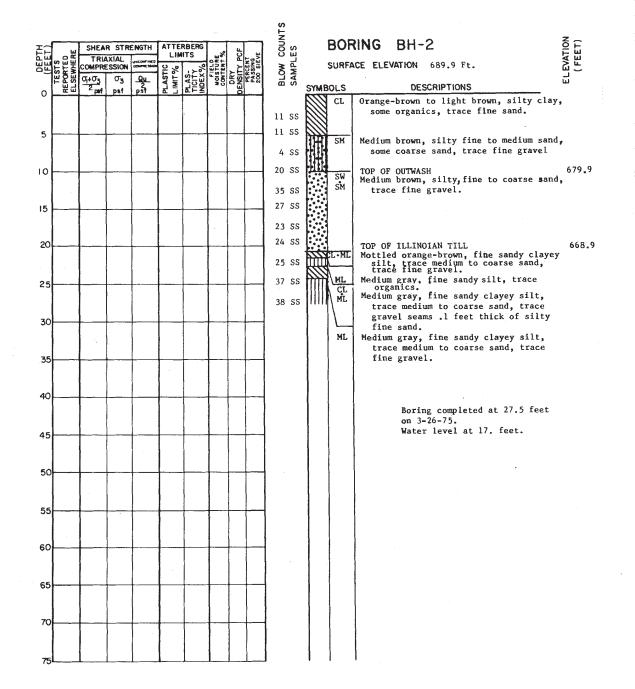


FIGURE A2.5-4

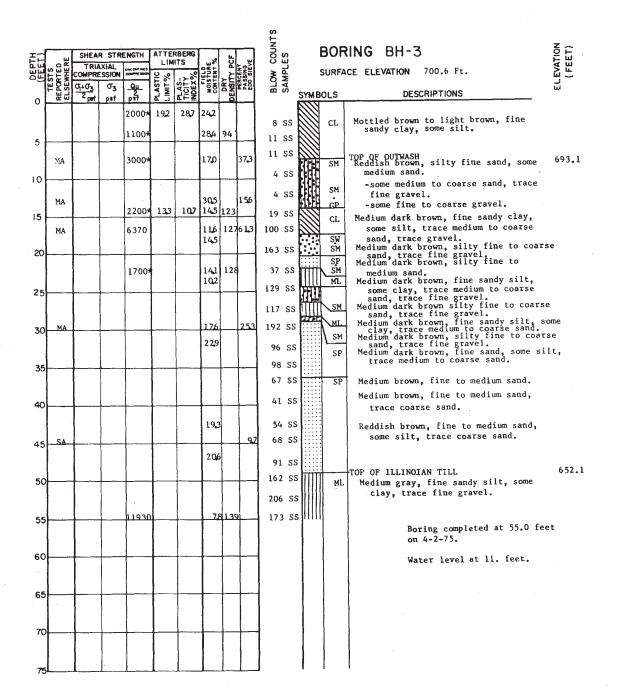


FIGURE A2.5-5

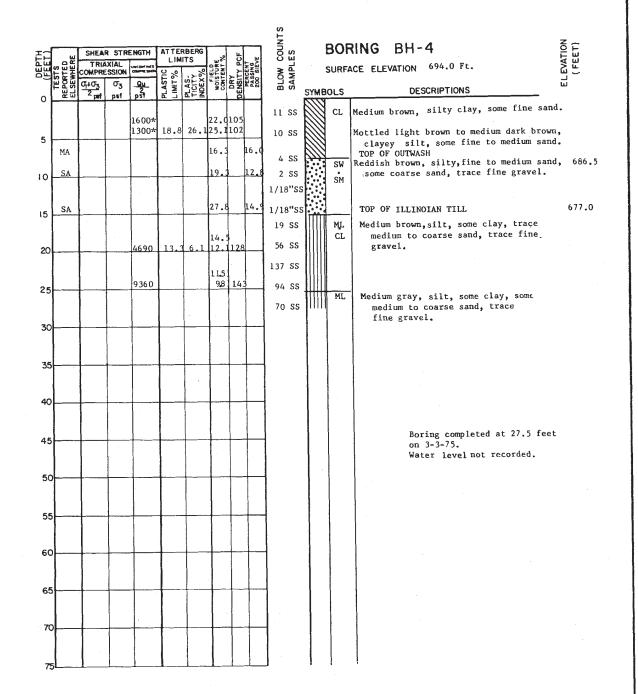


FIGURE A2.5-6

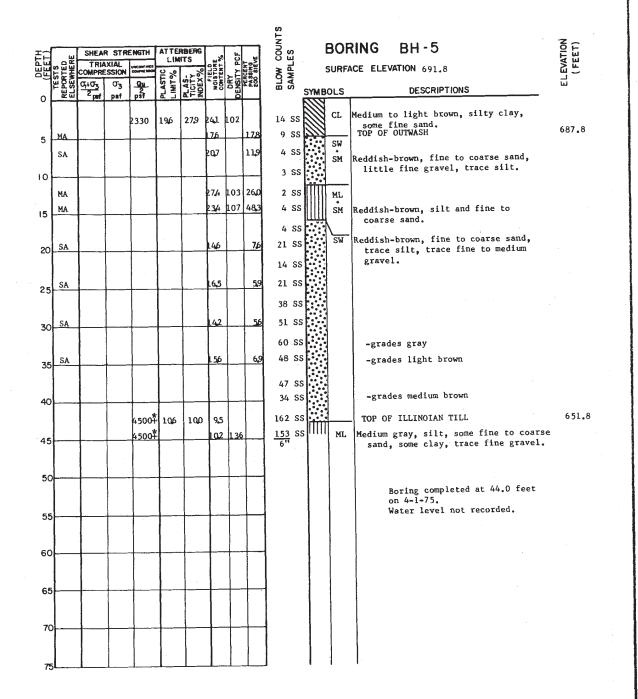


FIGURE A2.5-7

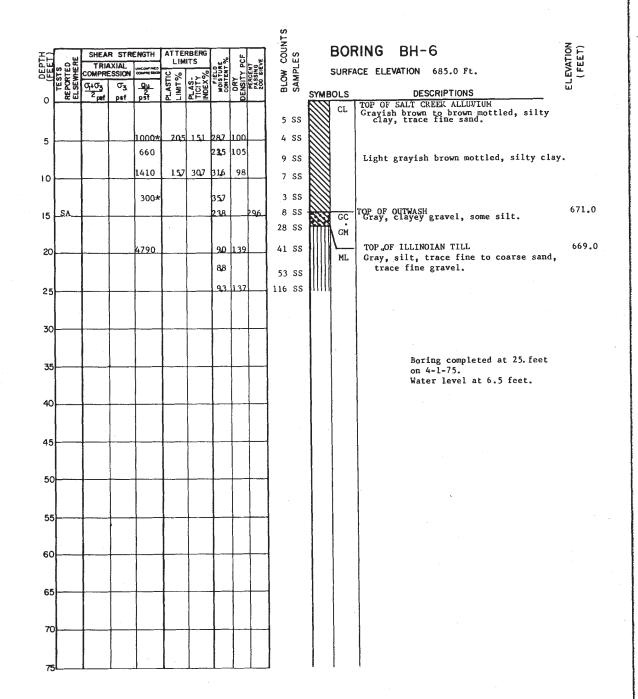


FIGURE A2.5-8

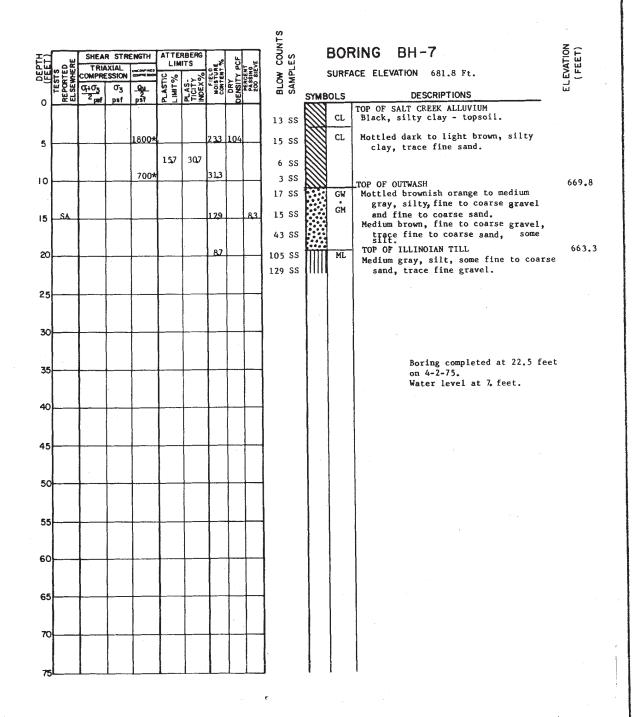


FIGURE A2.5-9
LOG OF BORING BH-7

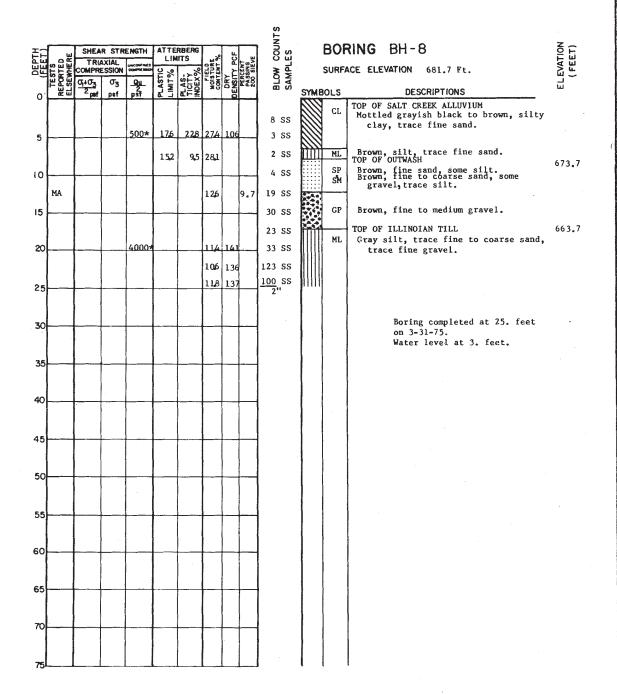


FIGURE A2.5-10

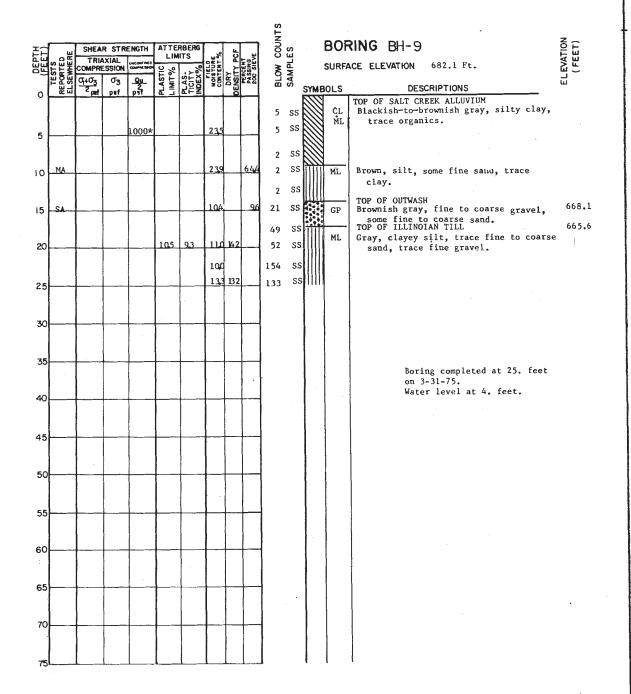


FIGURE A2.5-11

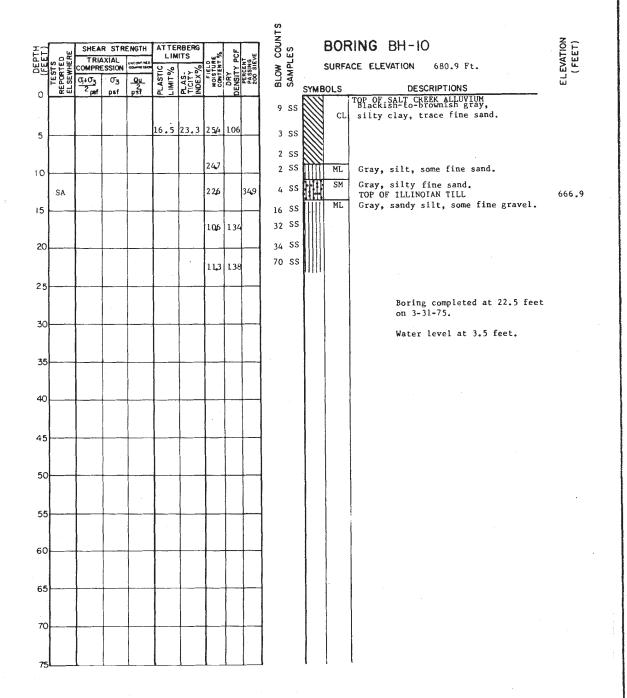


FIGURE A2.5-12

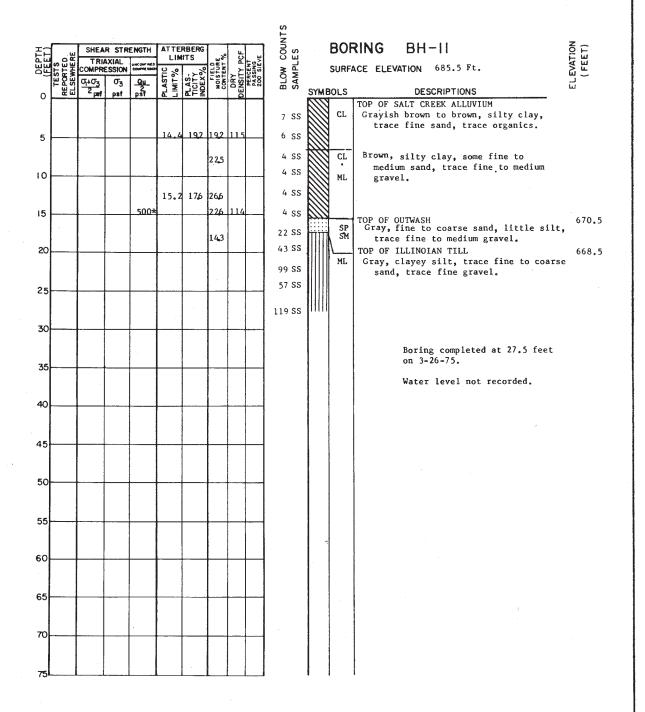


FIGURE A2.5-13

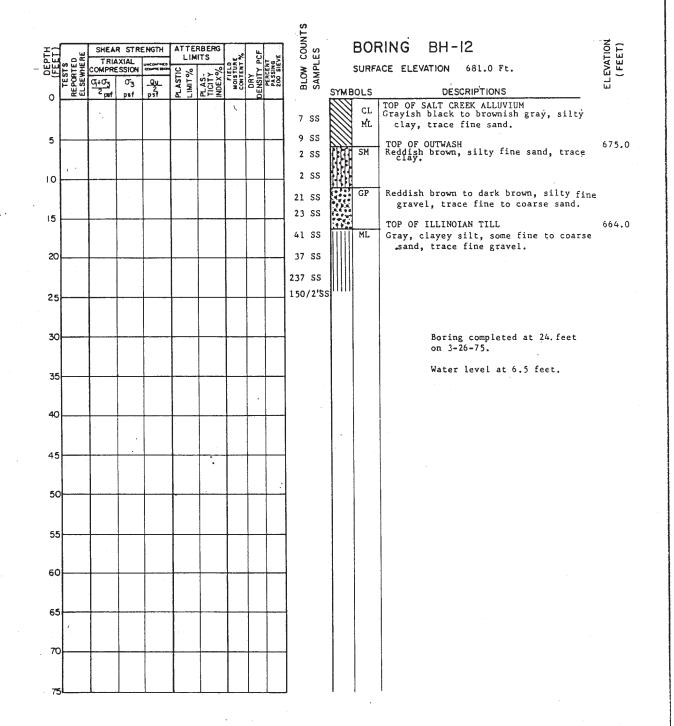


FIGURE A2.5-14

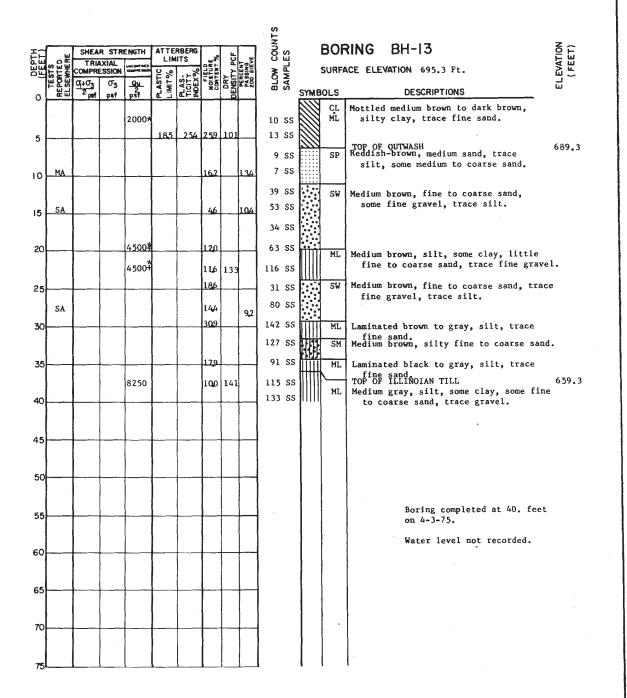


FIGURE A2.5-15

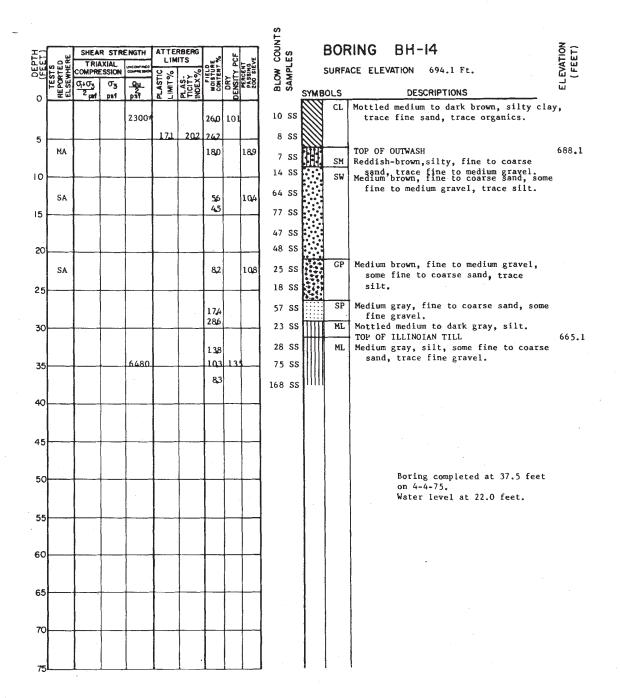


FIGURE A2.5-16

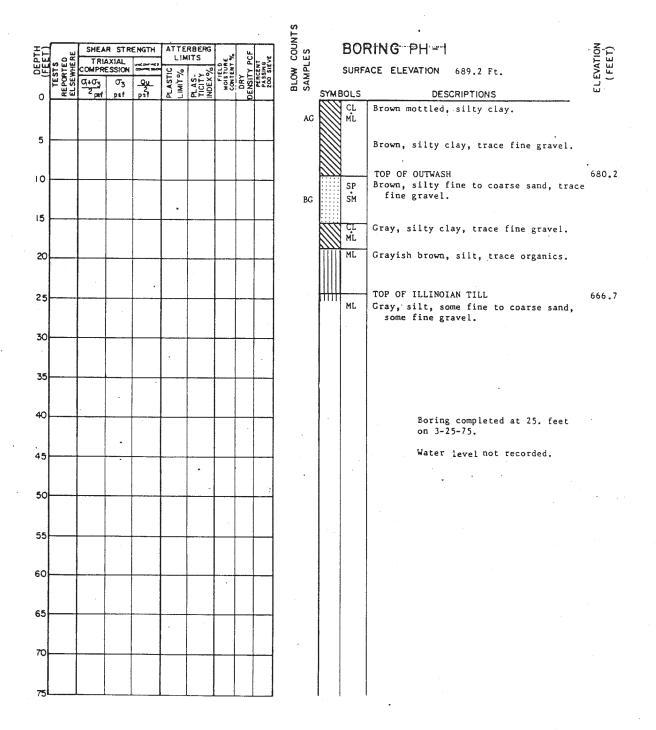


FIGURE A2.5-17

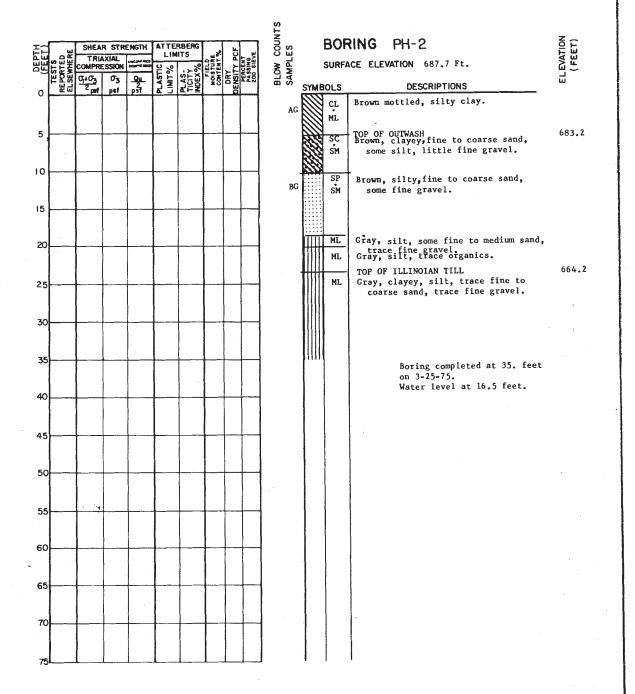


FIGURE A2.5-18

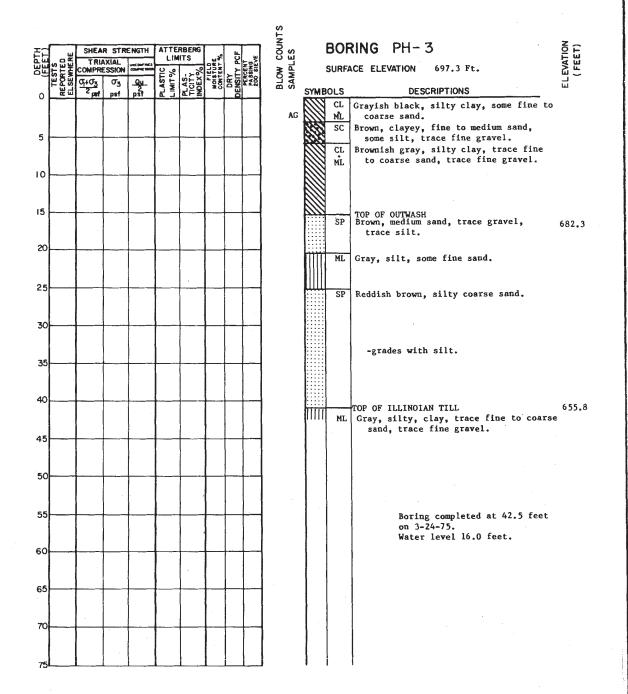


FIGURE A2.5-19

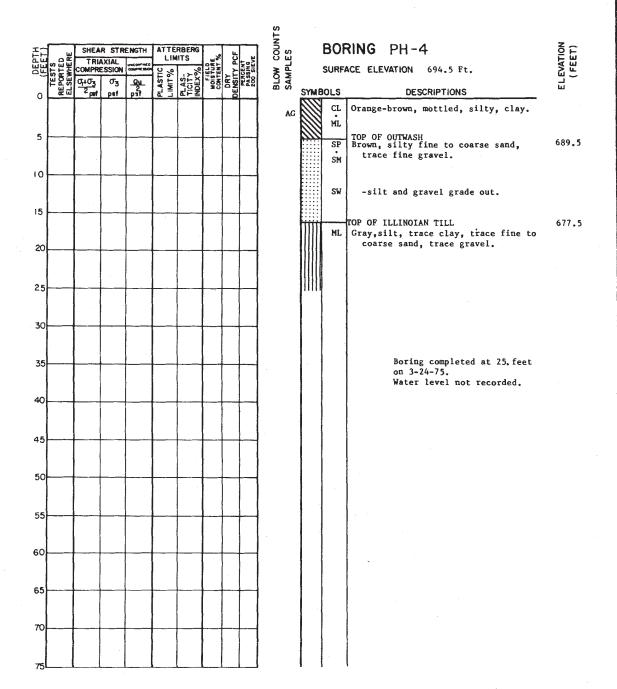


FIGURE A2.5-20

695.6

691.6

FIGURE A2.5-21

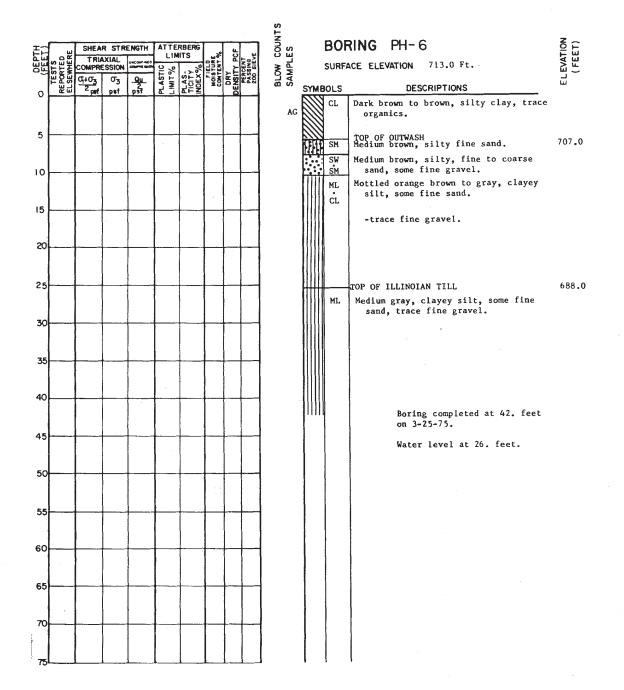


FIGURE A2.5-22

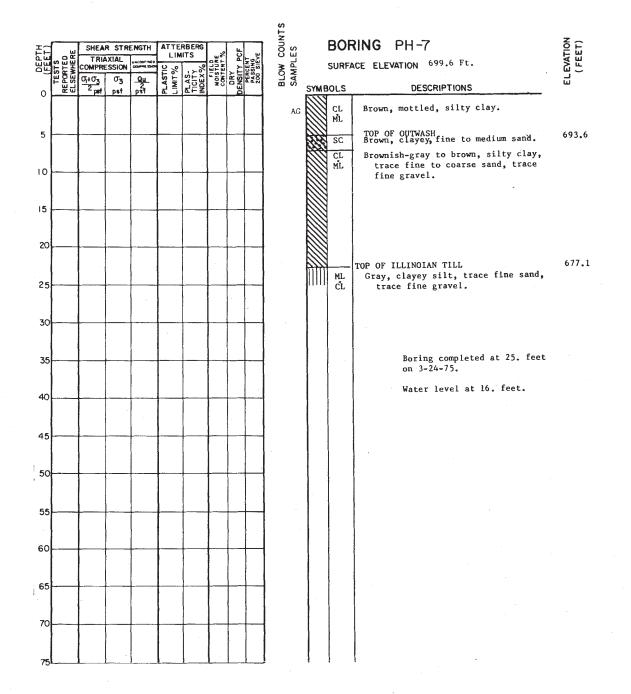


FIGURE A2.5-23

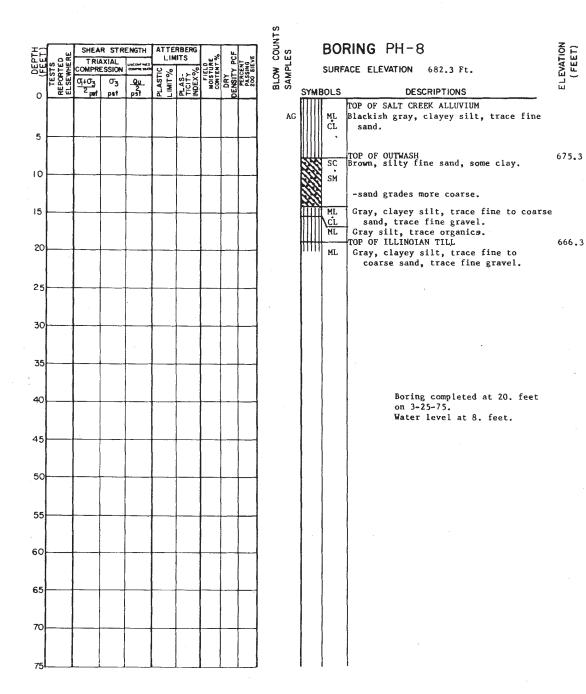


FIGURE A2.5-24

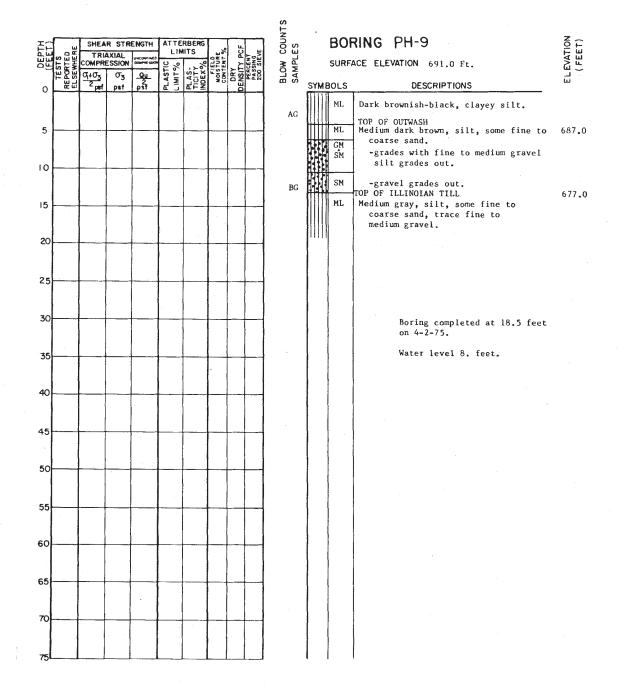


FIGURE A2.5-25

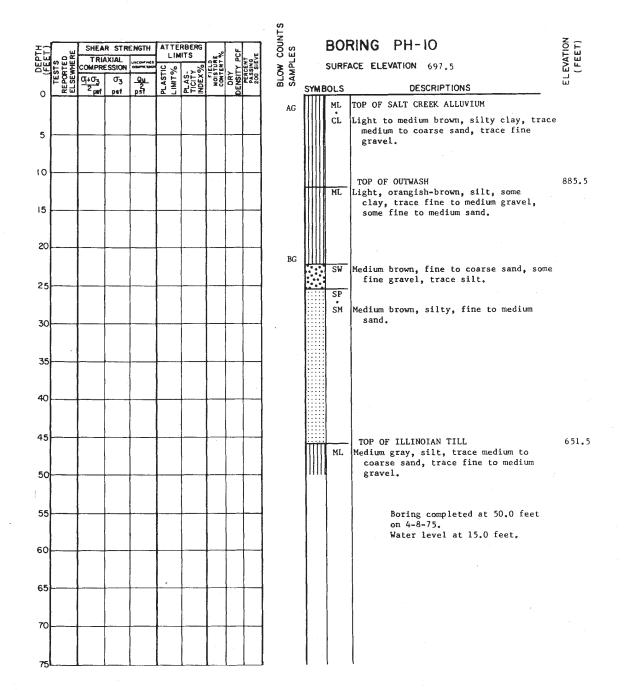


FIGURE A2.5-26

BLOW COUNTS SAMPLES BORING PH-II SHEAR STRENGTH ATTERBERG LIMITS TRIAXIAL SURFACE ELEVATION 679.2 PLASTIC LIMIT% <u>0,+03</u> 2 perf SYMBOLS DESCRIPTIONS TOP OF SALT CREEK ALLUVIUM CL ML AG Dark brown, silty clay, trace fine to medium sand, trace fine gravel, 5 10 15 664.2 TOP OF OUTWASH SP SM Grayish brown, silty, fine sand. 20 -grades to gray. TOP OF ILLINOIAN TILL 657.2 Gray, silt, trace fine gravel, some fine to medium sand. ML 30 Boring completed at 24.5 feet on 4-11-75. Water level at 18.0 feet. 35 40 45 50 55 60 65

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE A2.5-27

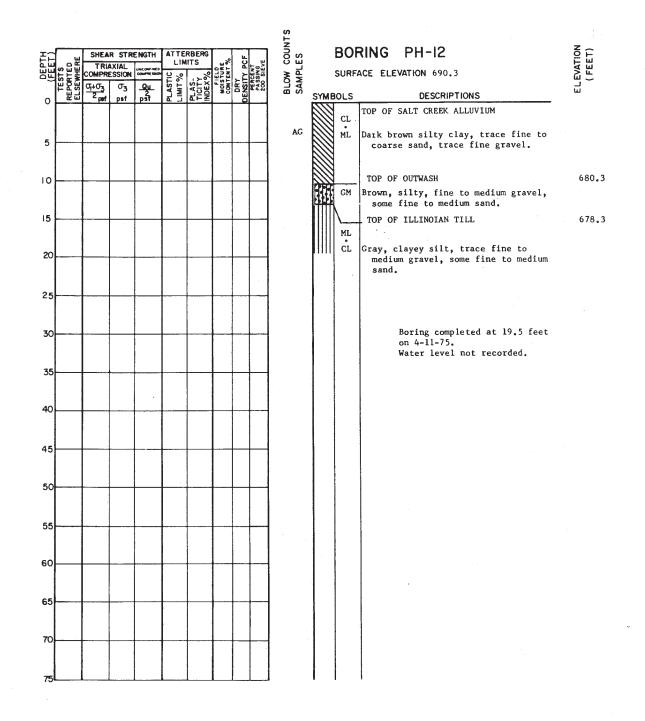


FIGURE A2.5-28

#### Key to Boring Logs

#### Samples

23	Indicates Standard Penetration Test Value
SS	Indicates Sample Obtained Using a Standard Split Spoon
ST	Indicates Shelby Tube Sample Obtained Using a 3.0 inch diameter Shelby Tube
PR	Indicates Sample Obtained Using a Pitcher Sampler (3.0 inch outer diameter)
CR	Indicates Sample Obtained with 4.0 inch outer diameter Core Sampler
ОВ	Indicates Sample Obtained Using an Osterberg Sampler (3.0 inch sample diameter)
AG	Indicates Auger Boring; No Standard Penetration Values or Samples Obtained Unless Specially Noted
BG	Indicates Bag Sample Obtained
HR	Indicates Sample Obtained with High Recovery Barrel

#### **Test Data**

Qu/2	Indicates ½ Unconfined Compressive Strength (Equal to Shear Strength) in P.S.F.
0.50	Value with no Asterisk is Obtained From RIMAC Test
0.50*	Value with Asterisk is Obtained from Pocket Penetrometer

#### Test Reported Elsewhere

С	Consolidation Test
COMP	Bulk Compaction Test
MA	Mechanical Particle Size Analysis (Sieve and Hydrometer)

Wednamed Fattole Olze Analysis (Oleve and Frydrometer

PERM Laboratory Permeability Test

SA Sieve Analysis

TX/CU/PP Consolidate-Undrained Triaxial Compression Test with Pore Pressure Measurement

TX/UU/R Unconsolidated-Undrained Triaxial Compression Test on Remolded Samples

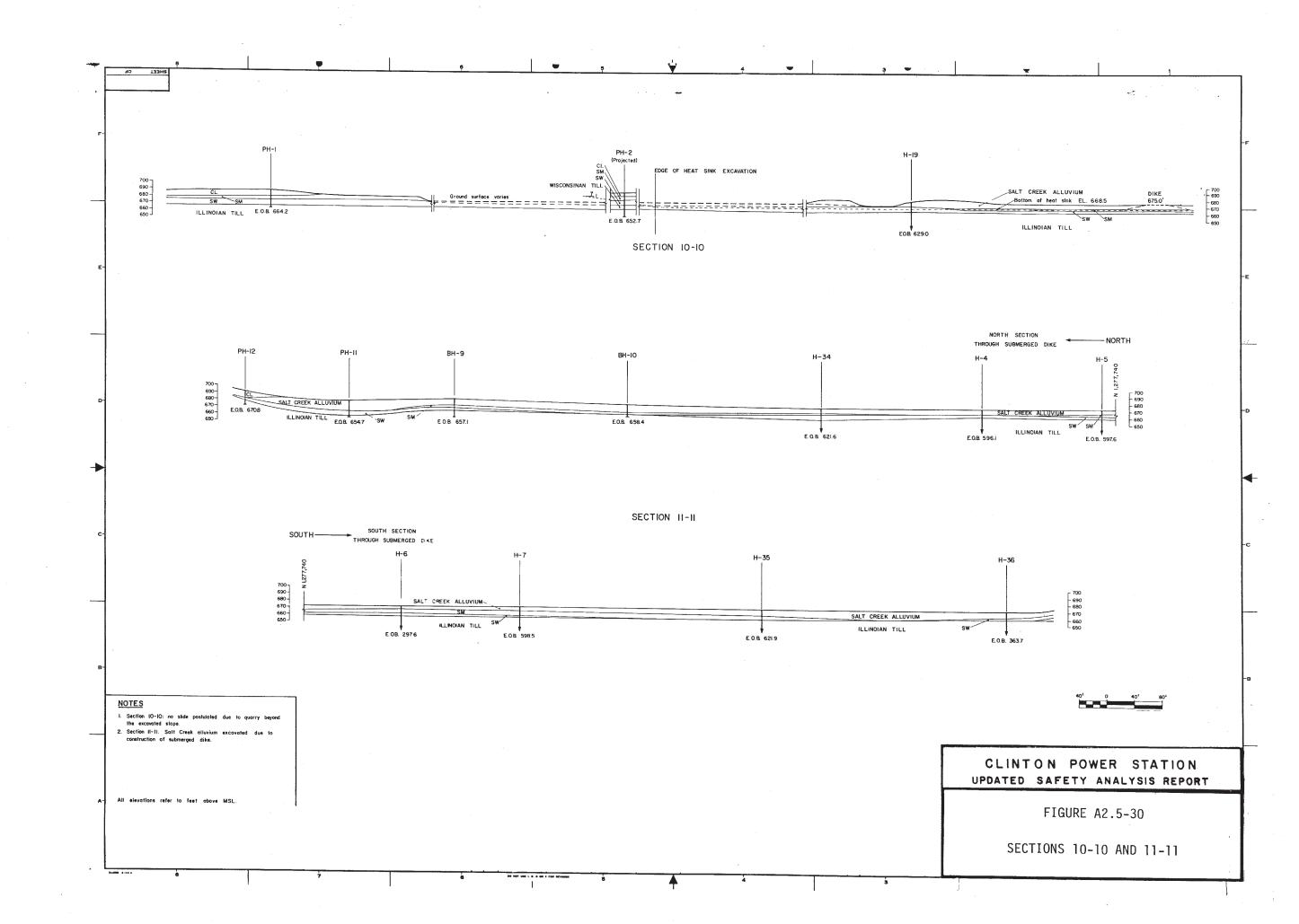
DR Relative Density Test
UC/R Remolded Samples

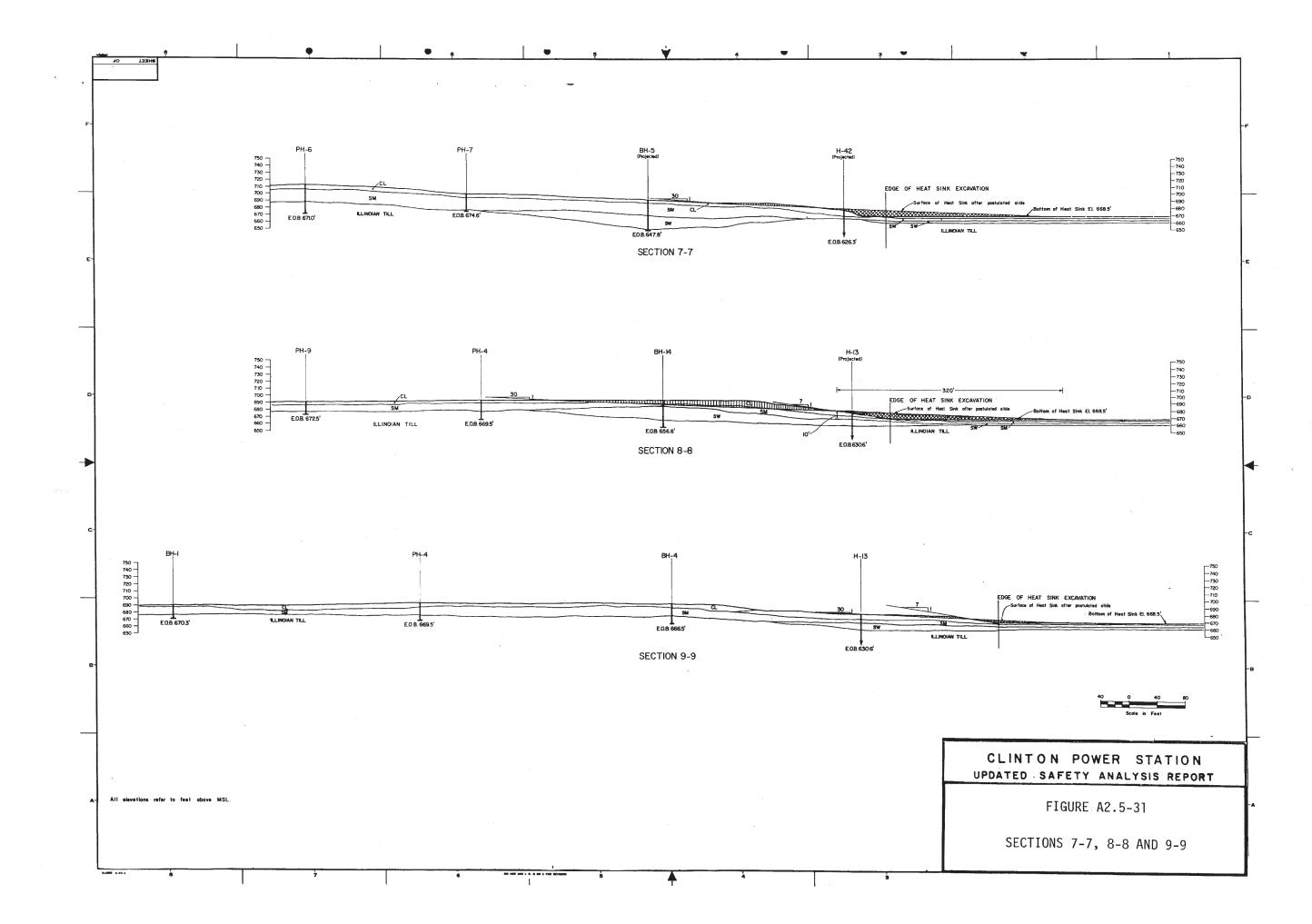
All Elevations Refer to Feet Above Mean Sea Level (MSL)

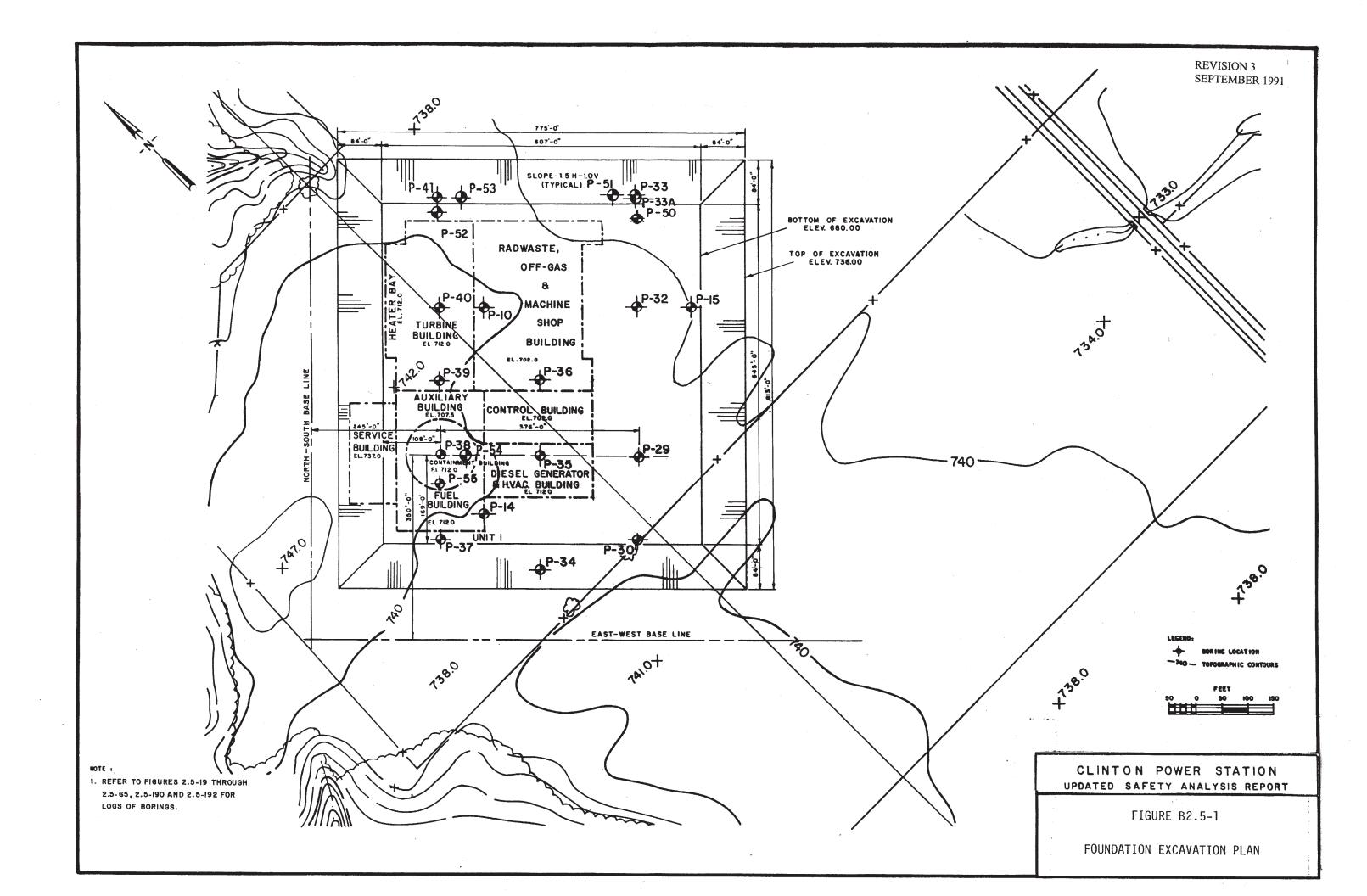
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure A2.5-29

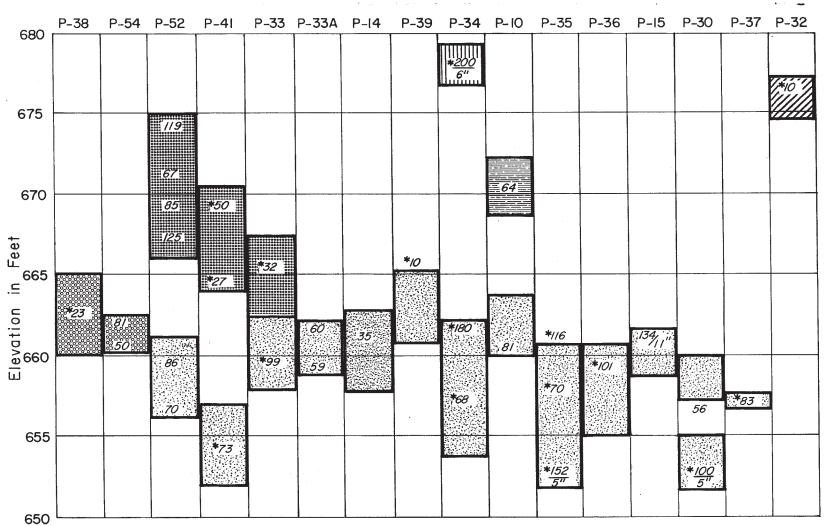
**KEY TO BORING LOGS** 







### BORINGS



### **LEGEND**

Lens I

Lens 2

Lens 3

Lens 4

Lens 5

Lens 6

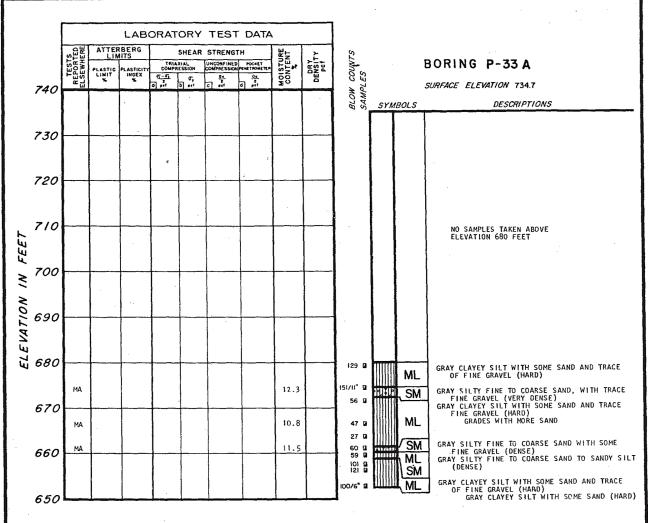
29 Standard split spoon sampler.

*29 Dames and Moore "U" type sampler.

### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE B2.5-2

SUMMARY OF BORING LOGS



BORING COMPLETED AT 82.5 FEET ON 8-6-75. CASING USED TO A DEPTH OF 5.0 FEET. GROUNDWATER LEVEL MEASURED AT 4.3 FEET ON 8-7-75.

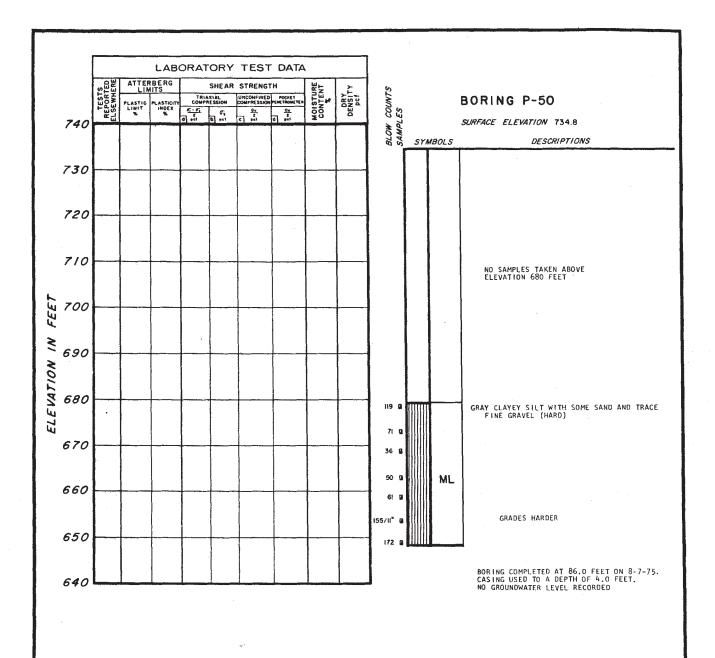
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE B2.5-3

LOG OF BORING P-33A

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.



NOTE

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

FIGURE B2.5-4

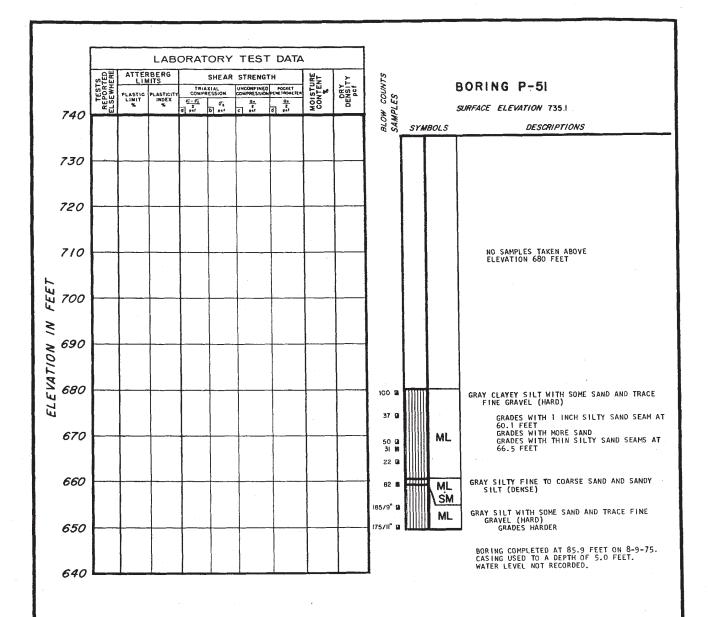
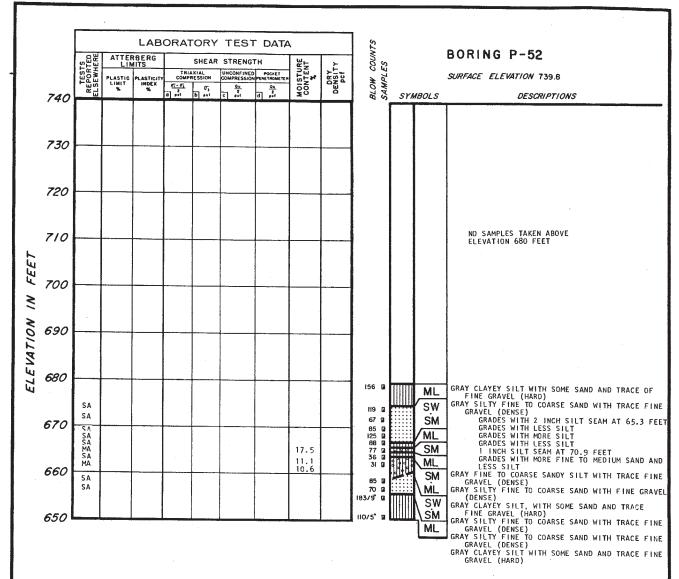


FIGURE B2.5-5

LOG OF BORING P-51

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.



BORING COMPLETED AT 88.9 FEET ON 8-7-75. CASING USED TO A DEPTH OF 5.0 FEET. GROUNDWATER LEVEL NOT RECORDED.

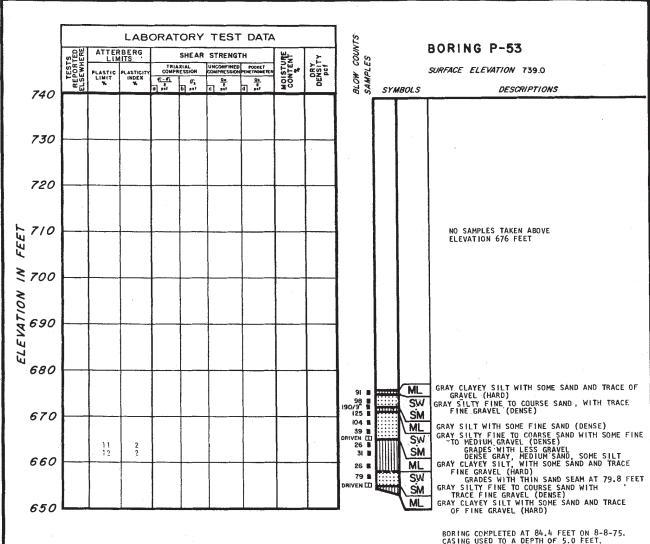
### CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE B2.5-6

LOG OF BORING P-52

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.



BORING COMPLETED AT 84.4 FEET ON 8-8-75. CASING USED TO A DEPTH OF 5.0 FEET. GROUNDWATER LEVEL NOT RECORDED

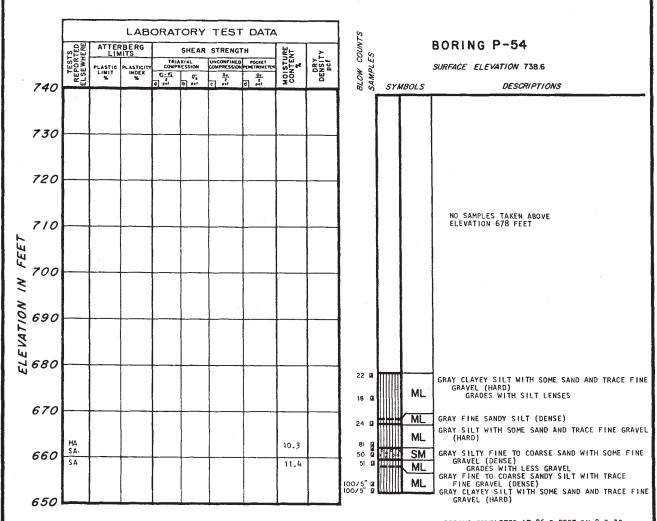
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE B2.5-7

LOG OF BORING P-53

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.



BORING COMPLETED AT 86.0 FEET ON 8-7-75. CASING USED TO A DEPTH OF 5.0 FEET. WATER LEVEL NOT RECORDED.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE B2.5-8

LOG OF BORING P-54

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.

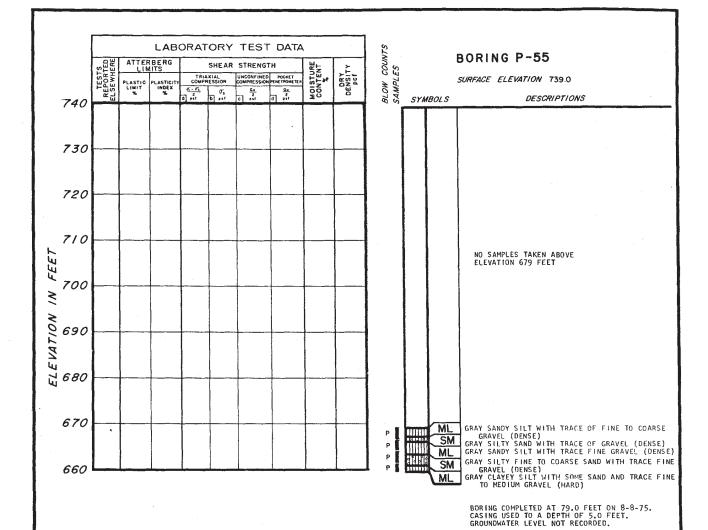
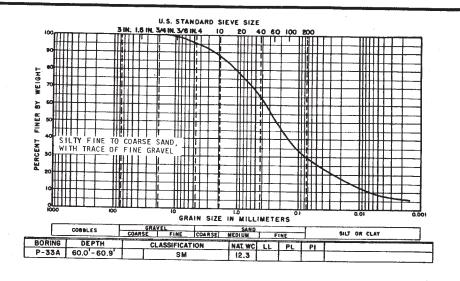


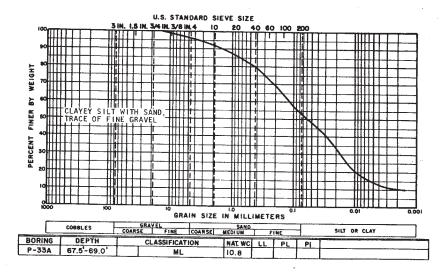
FIGURE B2.5-9

LOG OF BORING P-55

NOTE:

SEE FIGURE 2.5-298 AND FIGURE 2.5-355 FOR EXPLANATION OF SYMBOLS USED ON BORING LOGS.





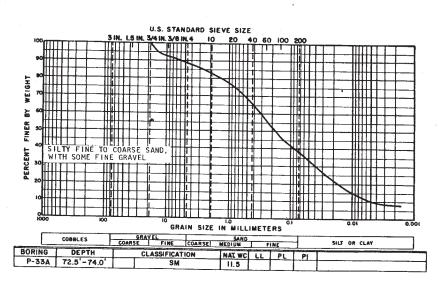
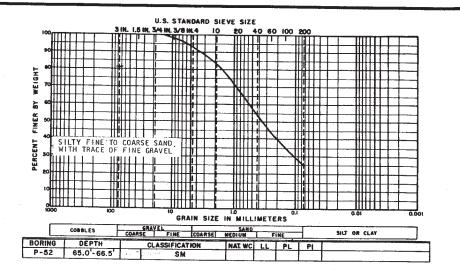
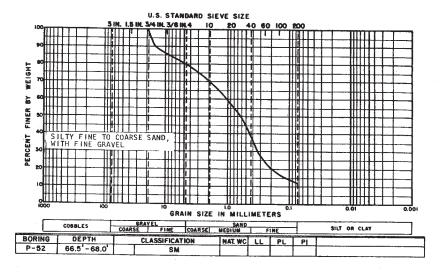


FIGURE B2.5-10

PARTICLE SIZE ANALYSES (BORING P-33A)





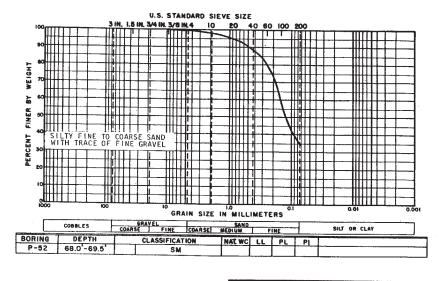
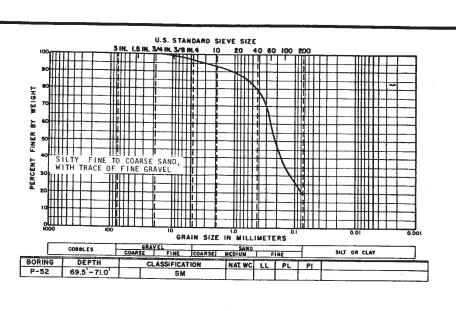
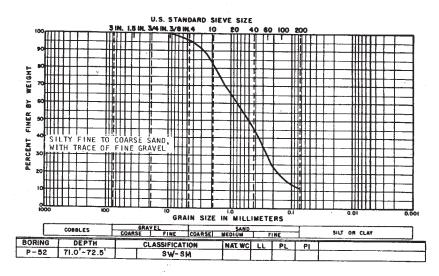


FIGURE B2.5-11

PARTICLE SIZE ANALYSES (BORING P-52)





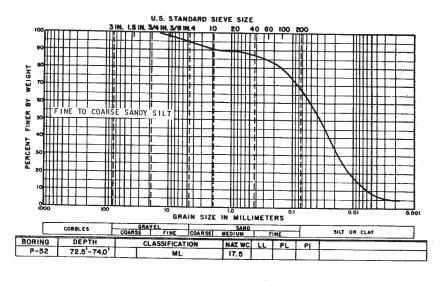
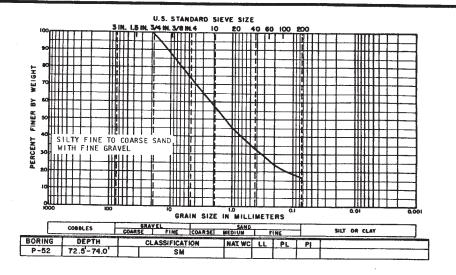
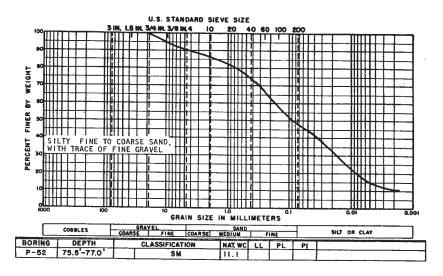


FIGURE B2.5-12

PARTICLE SIZE ANALYSES (BORING P-52)





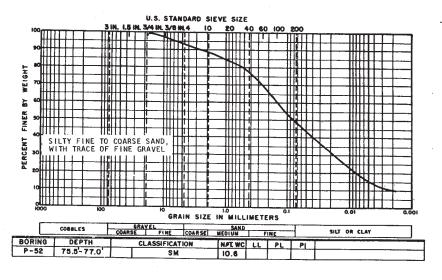
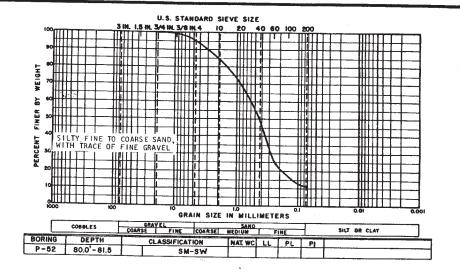
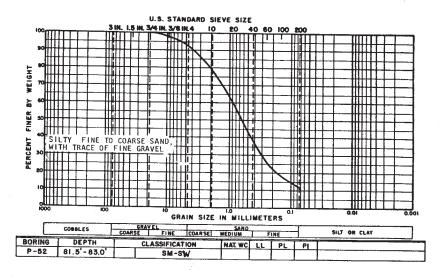


FIGURE B2.5-13

PARTICLE SIZE ANALYSES (BORING P-52)





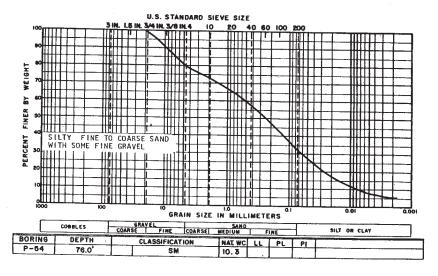
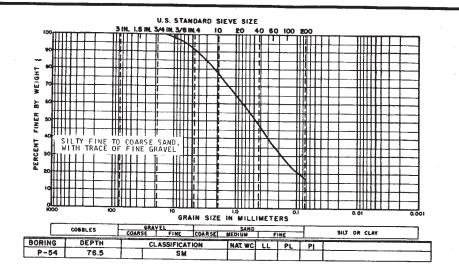
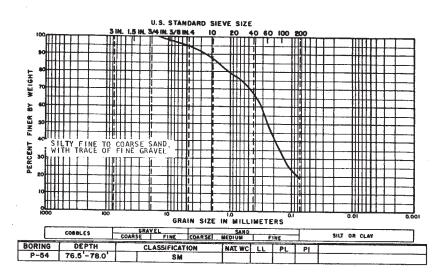
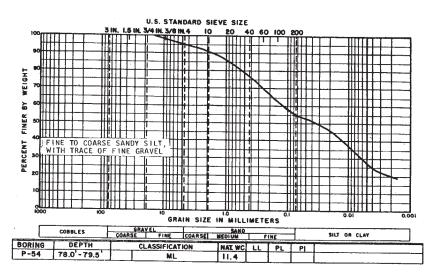


FIGURE B2.5-14

PARTICLE SIZE ANALYSES (BORINGS P-52 AND P-54)



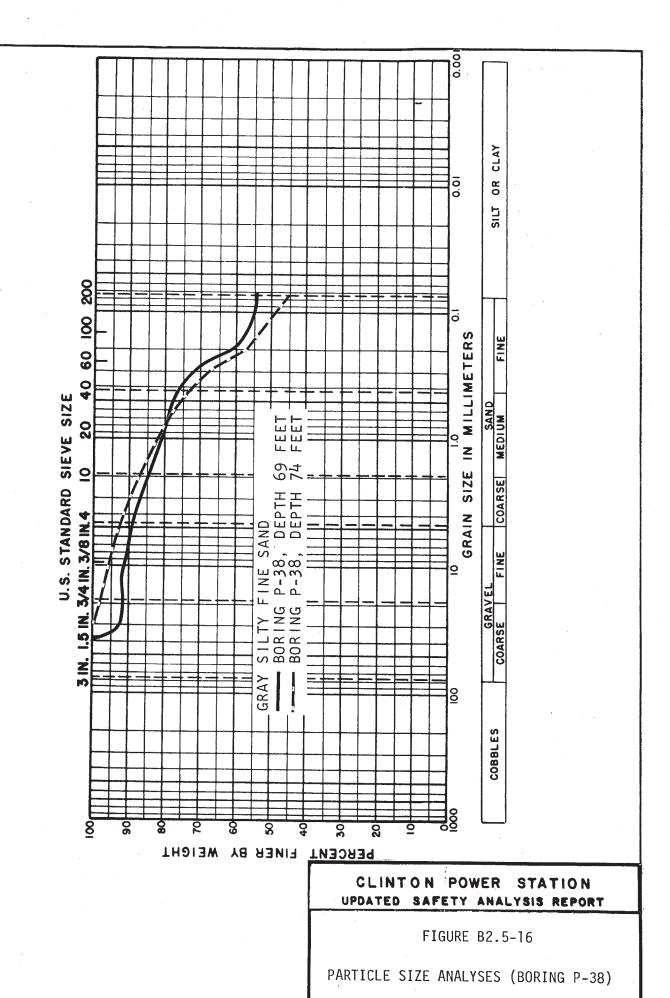




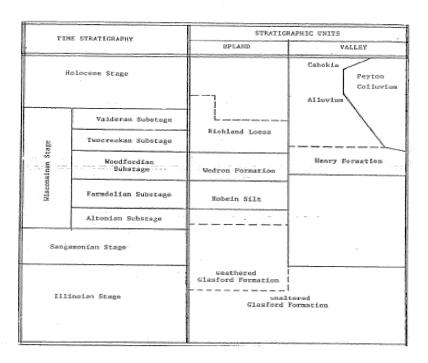
### CLINTON POWER STATION FINAL SAFETY ANALYSIS REPORT

FIGURE B2.5-15

PARTICLE SIZE ANALYSES (BORING P-54)



#### Site Stratigraphic Column



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure C2.5-1 Page 1 of 2

SITE STRATIGRAGHIC COLUMN

#### Site Stratigraphic Column

Stratigraphic Description					
Stratigraphic Unit	Approximate Thickness	General Description			
Cahokia Alluvium	0-35 feet	Alluvium: Poorly sorted silt, clay, and silty sand with lenses of sand and gravel. (CL to SM)			
Peyton Colluvium	0-10 feet	Colluvium: Brown clayey silt with minor amounts of gravel. (ML to CL)			
Richland Loess	0-10 feet	Loess: Brown clayey silt, trace fine sand. (ML to CL)			
Henry Formation	0-33 feet	Glacial outwash: Yellow-brown fine to coarse sand and gravel, with pockets of gray-brown silt, sandy silt, and silty clay. A lag gravel is often present at the base. (SM, SW, SP, SM-SW, SP-SW)			
Wedron Formation	20-55 feet	Till: Brown silty clay to clayey silt, some interspersed fine to coarse sand, trace fine to coarse gravel, with pockets of brown fine sand, sometimes silty, trace fine gravel. Grades to gray clayey silt, some fine to coarse sand, trace fine gravel with pockets of fine to coarse gray sand, trace fine gravel. (ML to CL)			
Robein Silt	0-2 feet	Loess (deposited in water): Dark brown organic silt, trace clay, trace fine sand. Locally consists of peat. (ML to CL)			
Weathered Glasford Formation	10-15 feet	Till: Gray silt grading to clayey silt, trace fine sand. Grades to gray-green silty clay or clayey silt, some fine to coarse sand, trace fine to coarse gravel. More sand and gravel with depth. Slightly to highly calcareous. (ML to CL)			
Unaltered Glasford Formation	90-140+ feet	Till: Dark gray clayey silt, some interspersed fine to coarse sand, highly calcareous, trace fine to coarse gravel with pockets of gray sand, fine to coarse. (ML-CL to SM)			

#### **Notes**

- 1. Vertical scale does not represent either relative thickness or stratigraphic units or relative duration of time interval.
- 2. Excavations for the Clinton Power Station did not extend below the unaltered Glasford Formation.
- 3. Illinoian-age till of the Glasford Formation was subjected to a significant period of weathering during the Sangamonian Stage and Altonian Substage.
- 4. Deposits of Cahokia Alluvium and Henry Formation were not differentiated; reported approximate thicknesses of each unit represents a combined thickness for both deposits.
- 5. The Holocene Stage is represented by a significant period of weathering and development of agricultural soil profiles.
- 6. The Cahokia Alluvium and Henry Formation were mapped as a single unit. The Cahokia Alluvium is Holocene and, quite possibly, in part Valderan/Twocreekan in age; the Henry Formation is Woodfordian (probably early) in age. The Wedron Formation is probably early Woodfordian.
- 7. Locally, the Peyton Colluvium rests directly on the Glasford Formation.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure C2.5-1 Page 2 of 2

SITE STRATIGRAGHIC COLUMN

## Comparison of Terminology Used For The FSAR and PSAR

Time Stratigraphy		Stratigraphic Units			
		FSAR		PSAR	
Holocene Stage		Cahokia Alluvium	Peyton Colluvium	Salt Creek Alluvium or Flood Plain Alluvium and Recent Channel Deposits	
Wisconsinan Stage	Valderan Substage	Richland Loess		Loess	
	Twocreekan Substage		Henry Formation		
	Woodfordian Substage	Wedron Formation		Wisconsinan Till or Wisconsinan Glacial Till	
	Farmdalian Substage	Robein Silt			
	Altonian Substage	Weathered Glasford Formation		Interglacial Zone or Sangamon Interglacial Zone	
Sangamonian Stage				or Sangamon Soil Interval	
Illinoian Stage					
		Unaltered Glasford Formation		Illinoian Till or Illinoian Glacial Till	

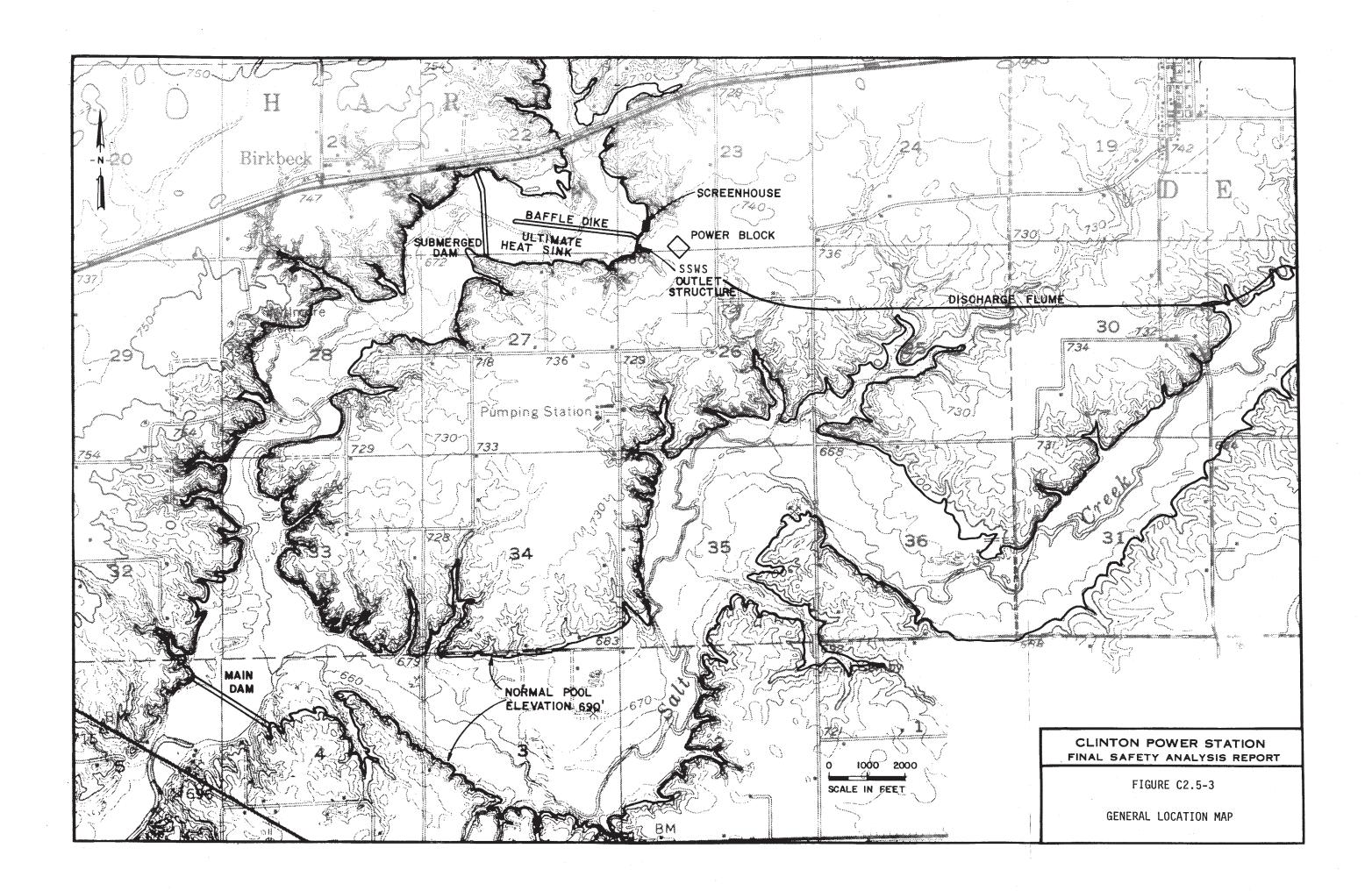
#### **Notes**

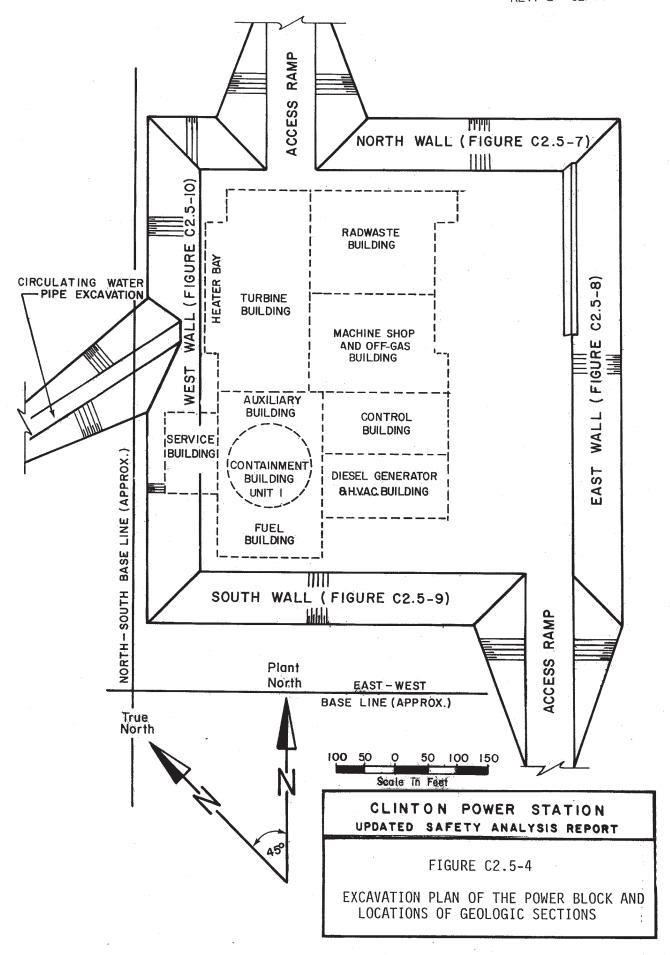
- 1. The Cahokia Alluvium, Peyton Colluvium, and Henry Formation consist of alluvial and outwash deposits and are confined to the valley of the North Fork of Salt Creek.
- 2. Vertical scale does not represent either relative thickness of stratigraphic units or relative duration of time interval.

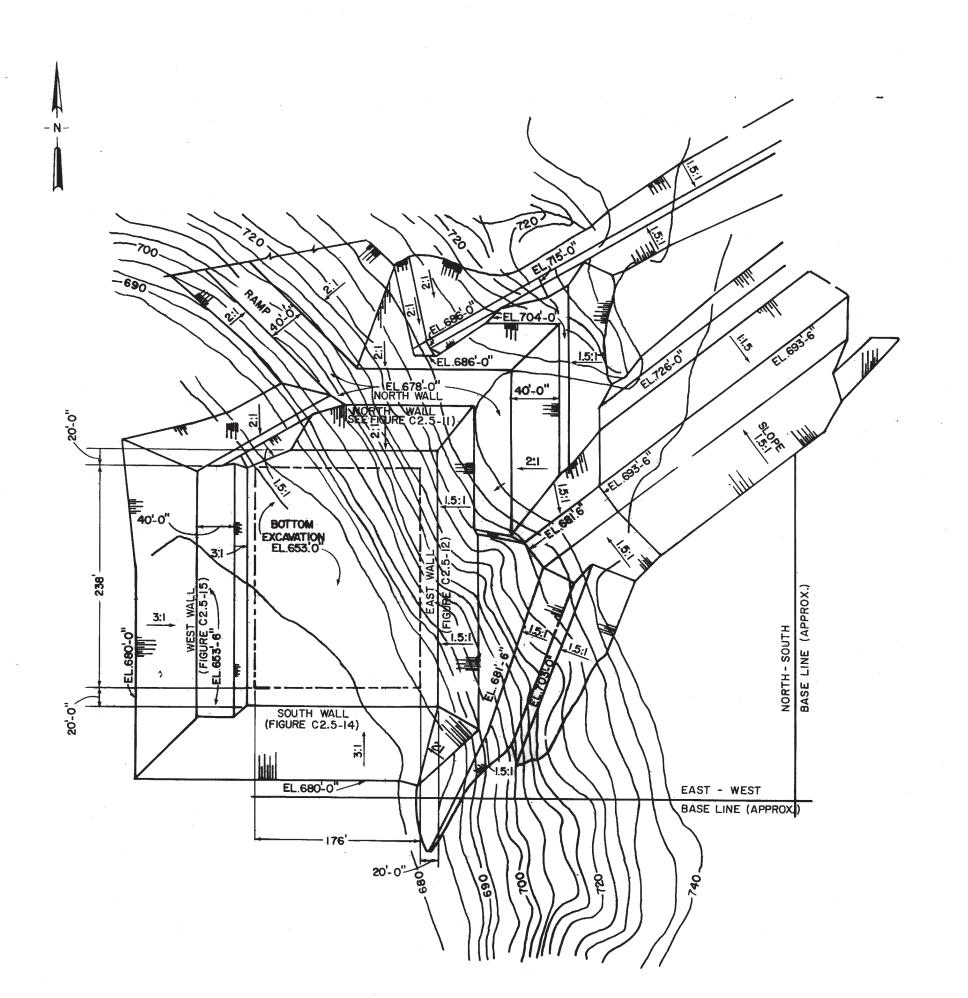
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

Figure C2.5-2

COMPARISON OF TERMINOLOGY USED FOR THE FSAR AND PSAR

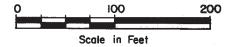






## NOTE

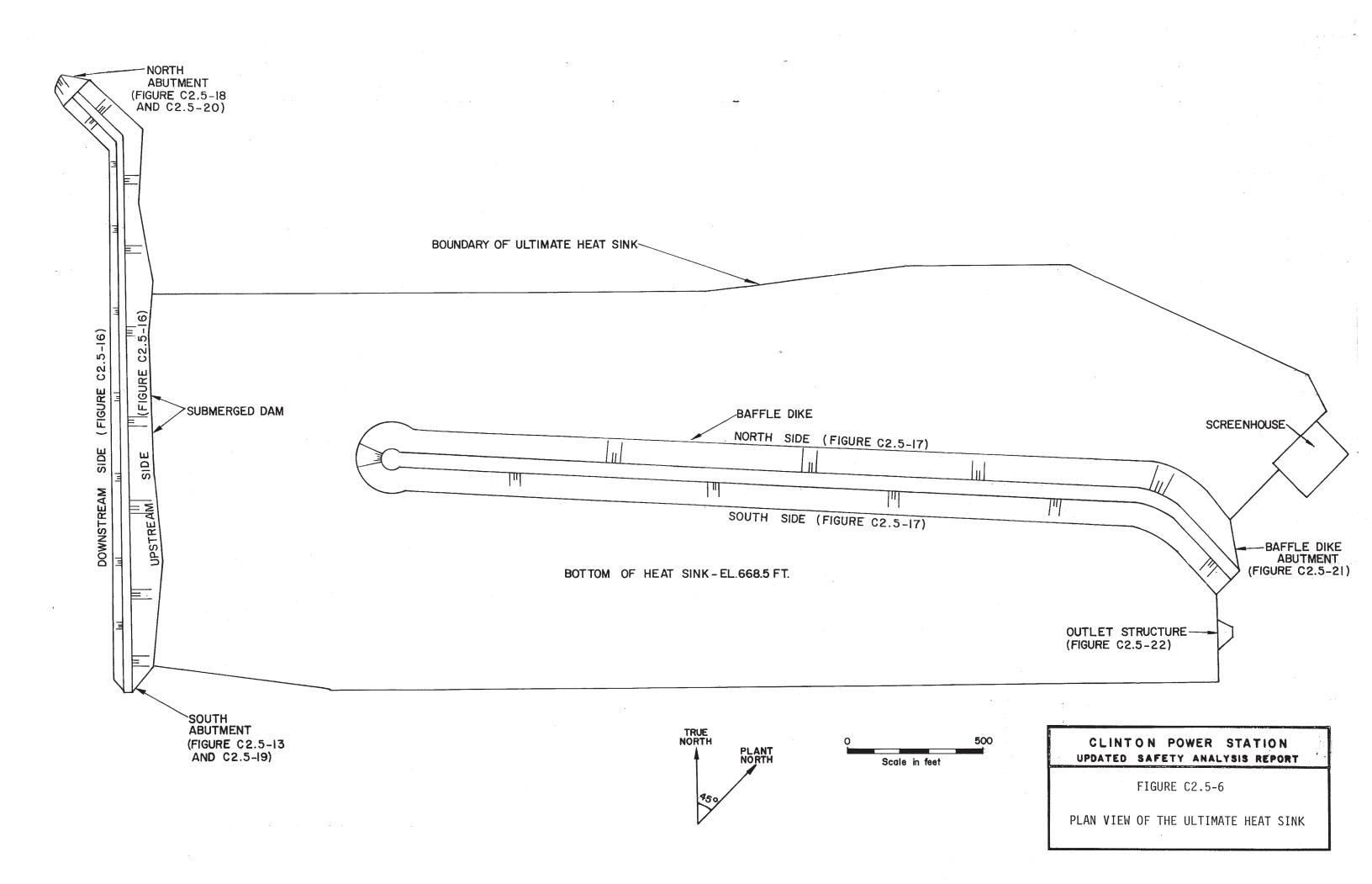
See Figures C2.5-II, C2.5-I2, C2.5-I4 and C2.5-I5 for geologic sections.

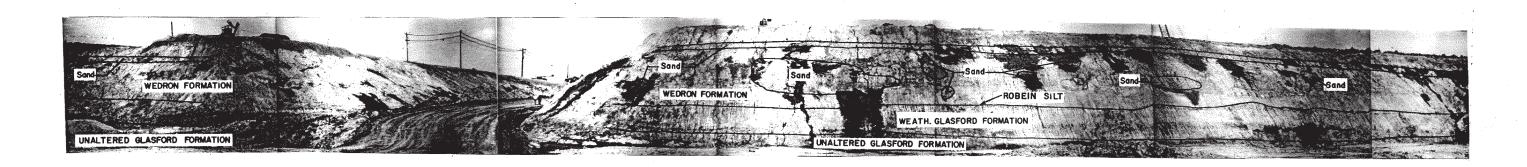


# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

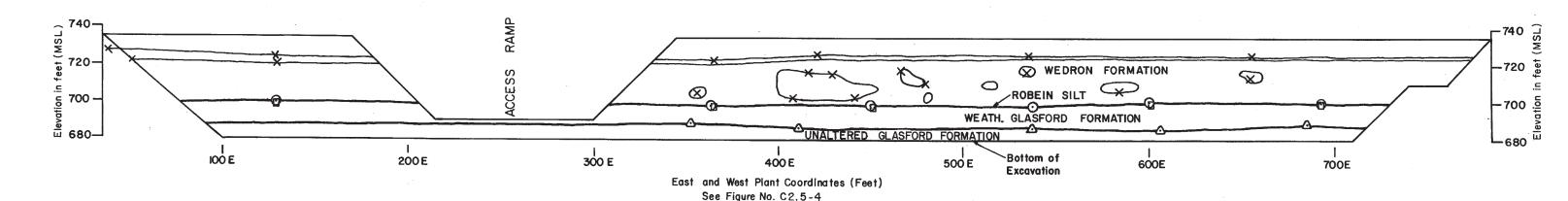
FIGURE C2.5-5

EXCAVATION PLAN FOR THE CIRCULATING WATER SCREEN HOUSE





#### NORTH WALL



### LEGEND

#### STRATIGRAPHIC CONTACTS

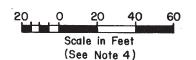
Contacts between formationsOutline of sand deposit

#### REFERENCE POINTS

- Wedron Fm./Robein Silt contact
- Robein Silt/Weath. Glasford Fm. contact
- Weath.Glasford Fm./Unalt. Glasford Fm. contact
- × Sand location

#### **NOTES**

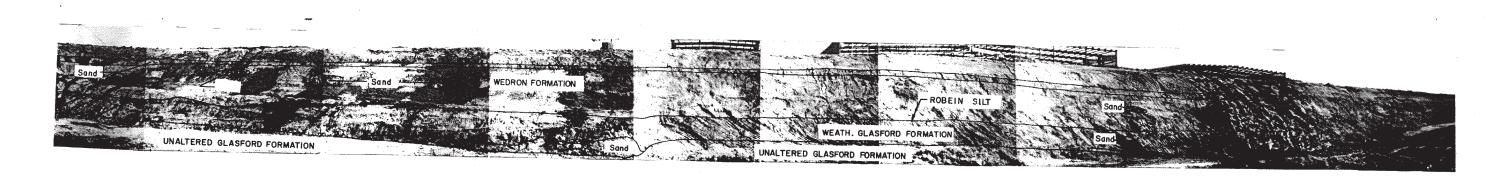
- I. Location of this figure is shown in Figure C2.5-4, see also Figure No. C2.5-3.
- 2. Descriptions and ages of stratigraphic units are presented in Figure C2.5-1.
- 3. Limits of excavation and slopes shown in geologic section are approximations.
- 4. Due to radial photography from center of excavation, photo mosaic is not to scale.
- 5. Contacts between stratigraphic units and limits of sand deposits are approximated between control points.
- 6. The Robein Silt is a generally 2 to 4 foot thick layer of organic silt between the tills of the Wedron and Glasford Formations.



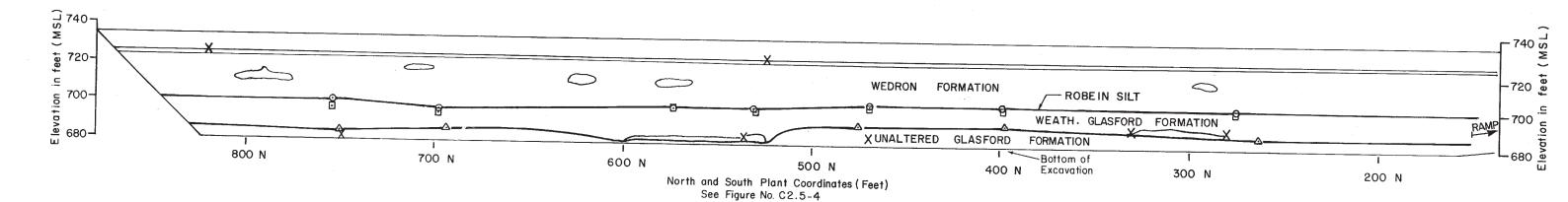
## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-7

GEOLOGIC SECTION AND PHOTO MOSAIC OF THE NORTH WALL OF THE POWER BLOCK EXCAVATION



EAST WALL



## LEGEND

## STRATIGRAPHIC CONTACTS

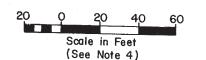
Contacts between formationsOutline of sand deposit

#### REFERENCE POINTS

- Wedron Fm./Robein Silt.contact
- Robein Silt/ Weath. Glasford Fm contact.
- Weath. Glasford Fm./Unalt. Glasford Fm. contact.
- X Sand location

#### NOTES

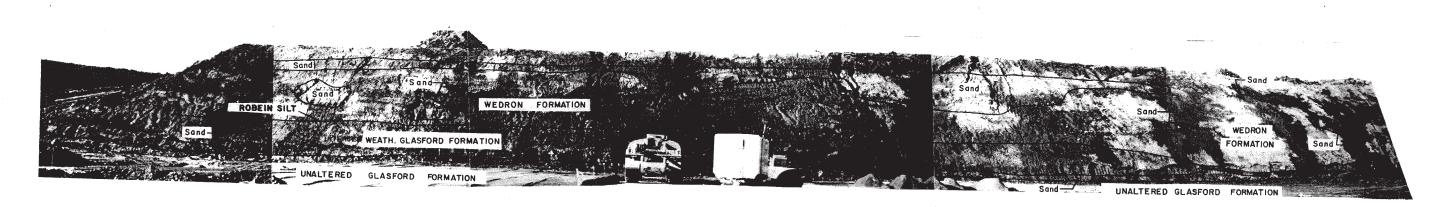
- 1. Location of this figure is shown in Figure C2.5-4, see also Figure No. C2.5-3.
- 2. Descriptions and ages of stratigraphic units are presented in Figure C2.5-1.
- 3. Limits of excavation and slopes shown in geologic section are approximations.
- 4. Due to radial photography from center of excavation, photo mosaic is not to scale.
- 5. Contacts between stratigraphic units and limits of sand deposits are approximated between control points.
- 6. The Robein Silt is a generally 2 to 4 foot thick layer of organic silt between the tills of the Wedron and Glasford Formations.



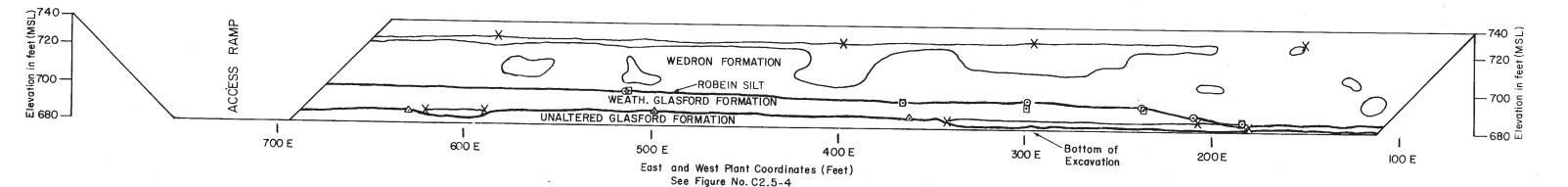
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-8

GEOLOGIC SECTION AND PHOTO MOSAIC OF THE EAST WALL OF THE POWER BLOCK EXCAVATION



## SOUTH WALL



## LEGEND

## STRATIGRAPHIC CONTACTS

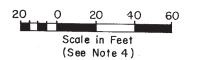
Contacts between formationsOutline of sand deposit

#### REFERENCE POINTS

- Wedron Fm./Robein Silt contact
- Robein Silt/Weath.Glasford Fm. confact.
- Weath. Glasford Fm. / Unalt. Glasford Fm. contact
- $\times$  Sand location

#### **NOTES**

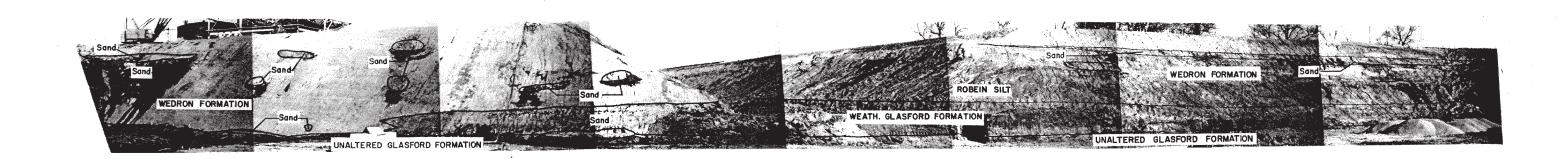
- I. Location of excavation shown in Figure C2.5-4, see also Figure No. C2.5-3.
- 2. Descriptions and ages of stratigraphic units are presented in Figure C2.5-1.
- 3. Limits of excavation and slopes shown in geologic. section are approximations.
- 4. Due to radial photography from center of excavation, photo mosaic is not to scale.
- 5. Contacts between stratigraphic units and outlines of sand deposits are approximated between control points.
- 6. The Robein Silt is a generally 2 to 4 foot thick layer of organic silt between the tills of the Wedron and Glasford Formations.



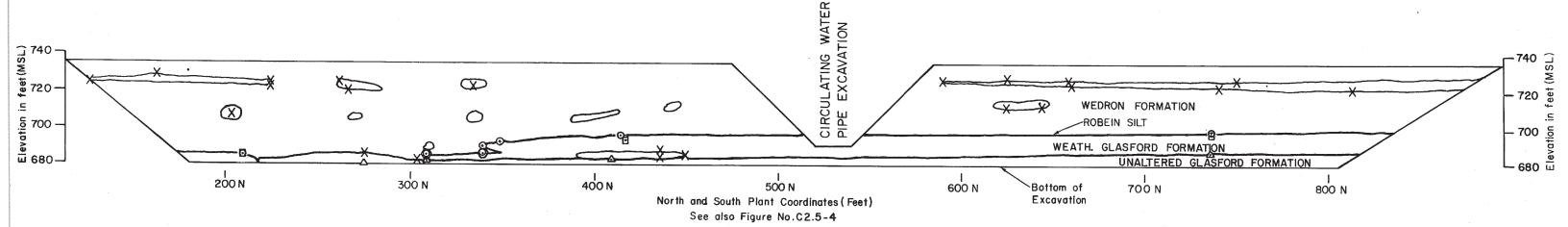
# CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-9

GEOLOGIC SECTION AND PHOTO MOSAIC OF THE SOUTH WALL OF THE POWER BLOCK EXCAVATION



#### WEST WALL



## **LEGEND**

#### STRATIGRAPHIC CONTACTS

Contacts between formations

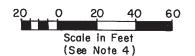
Outline of sand deposit

#### REFERENCE POINTS

- Wedron Fm./Robein Silt contact Robein Silt/Weath. Glasford Fm. contact.
- Wedron Fm./Weath.Glasford Fm. contact.
- Weath. Glasford Fm. / Unalt. Glasford Fm.contact.

## **NOTES**

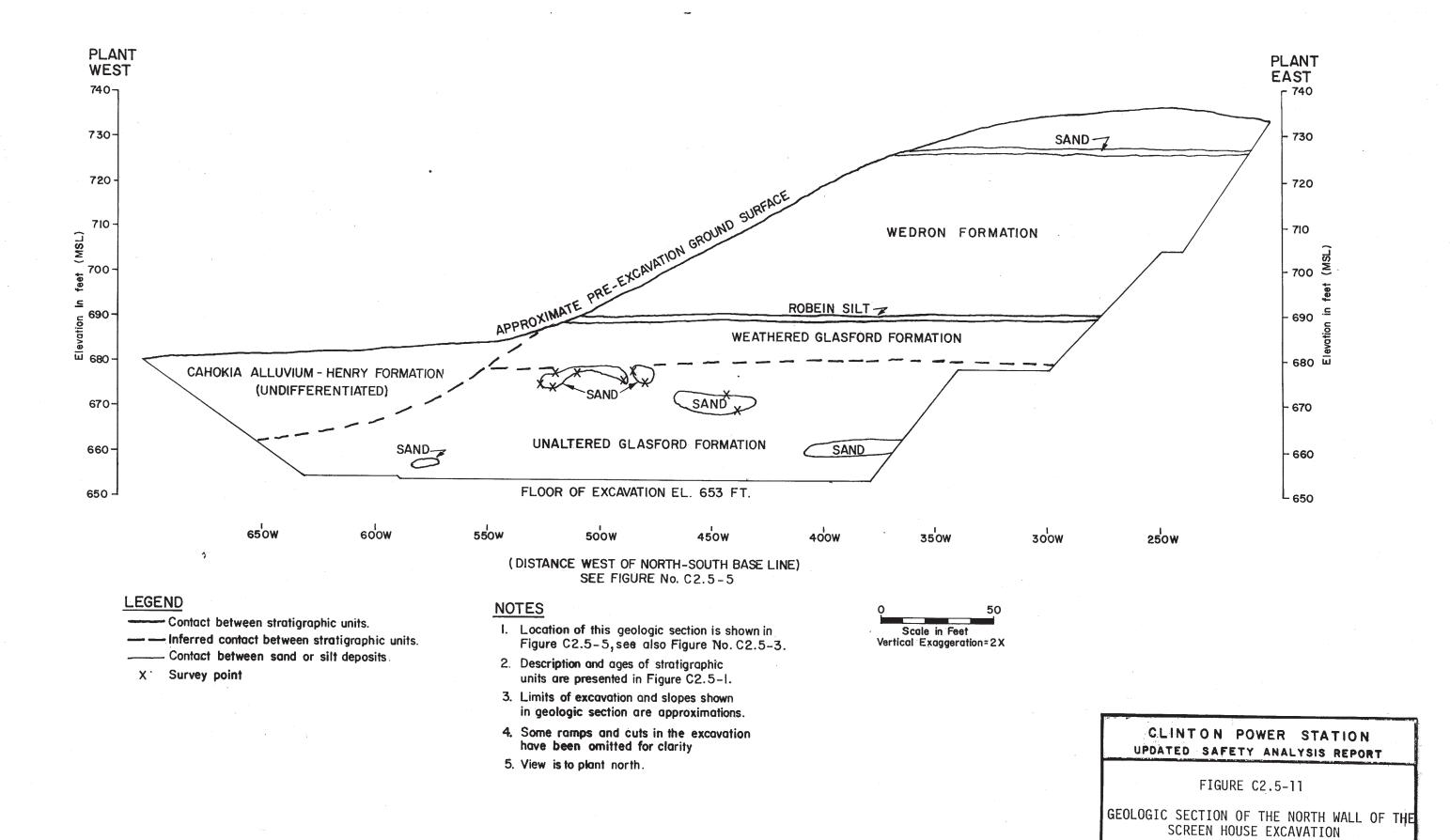
- 1. Location of excavation shown in Figure C2.5-4, see also Figure No. C2.5-3.
- 2. Descriptions and ages of stratigraphic units are presented in Figure C2.5-1.
- 3. Limits of excavation and slopes shown in geologic section are approximations.
- 4. Due to radial photography from center of excavation, photo mosaic is not to scale.
- 5. Contacts between stratigraphic units and outlines of sand deposits are approximated between control points.
- 6. The Robein Silt is a generally 2 to 4 foot thick layer of organic silt between tills of the Wedron and Glasford Formations.
- 7. The Robein Silt has been locally removed by erosion between approximately 220N to 300N.

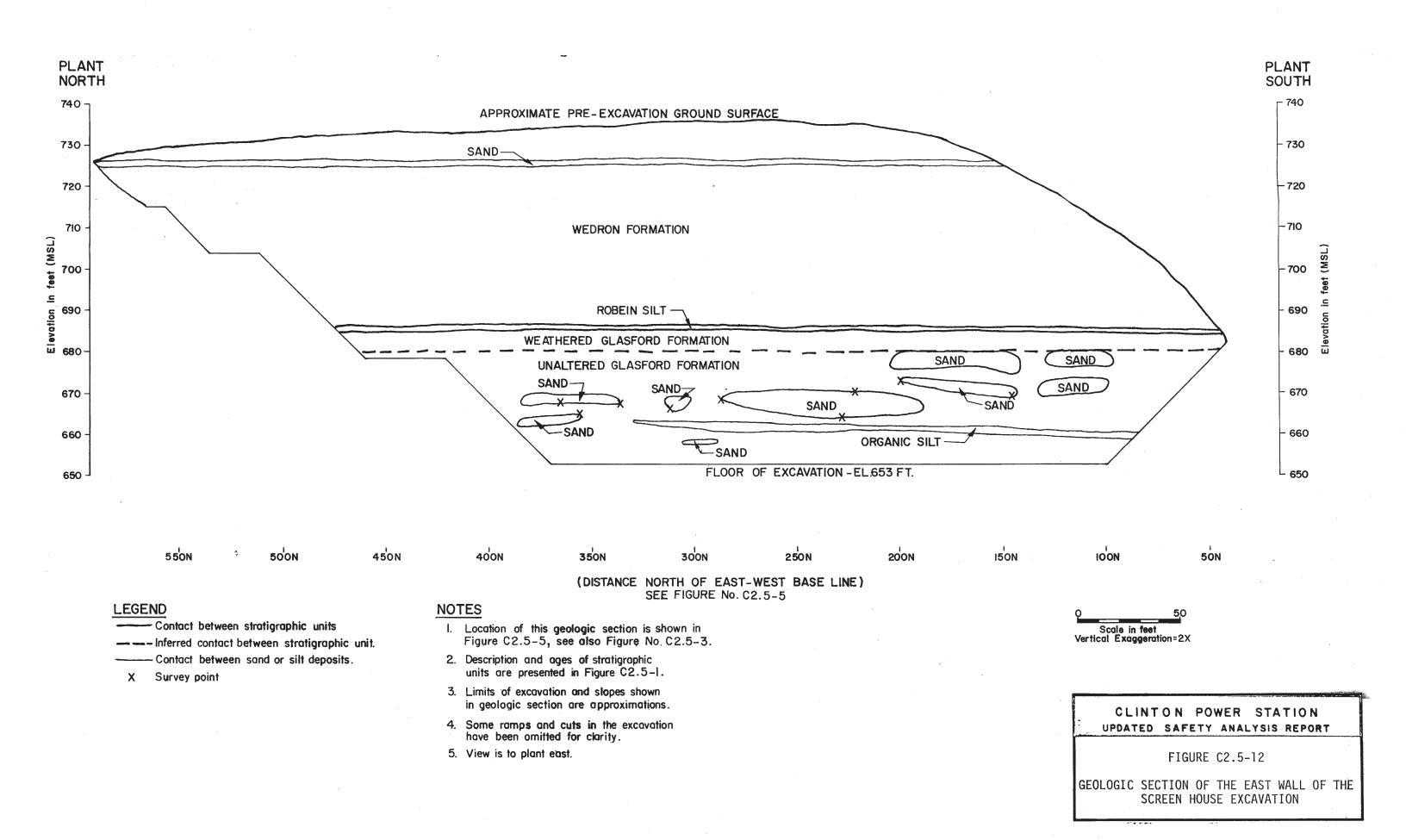


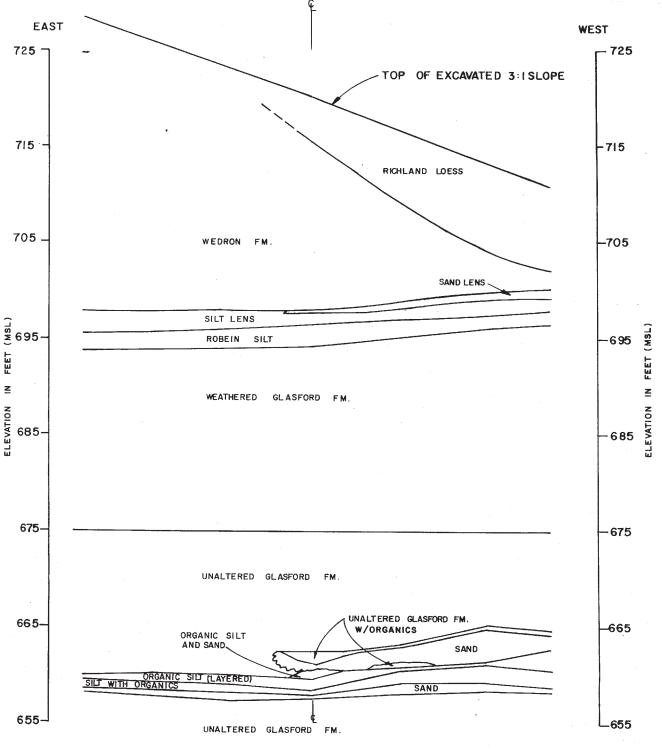
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-10

GEOLOGIC SECTION AND PHOTO MOSAIC OF THE WEST WALL OF THE POWER BLOCK EXCAVATION







#### NOTES

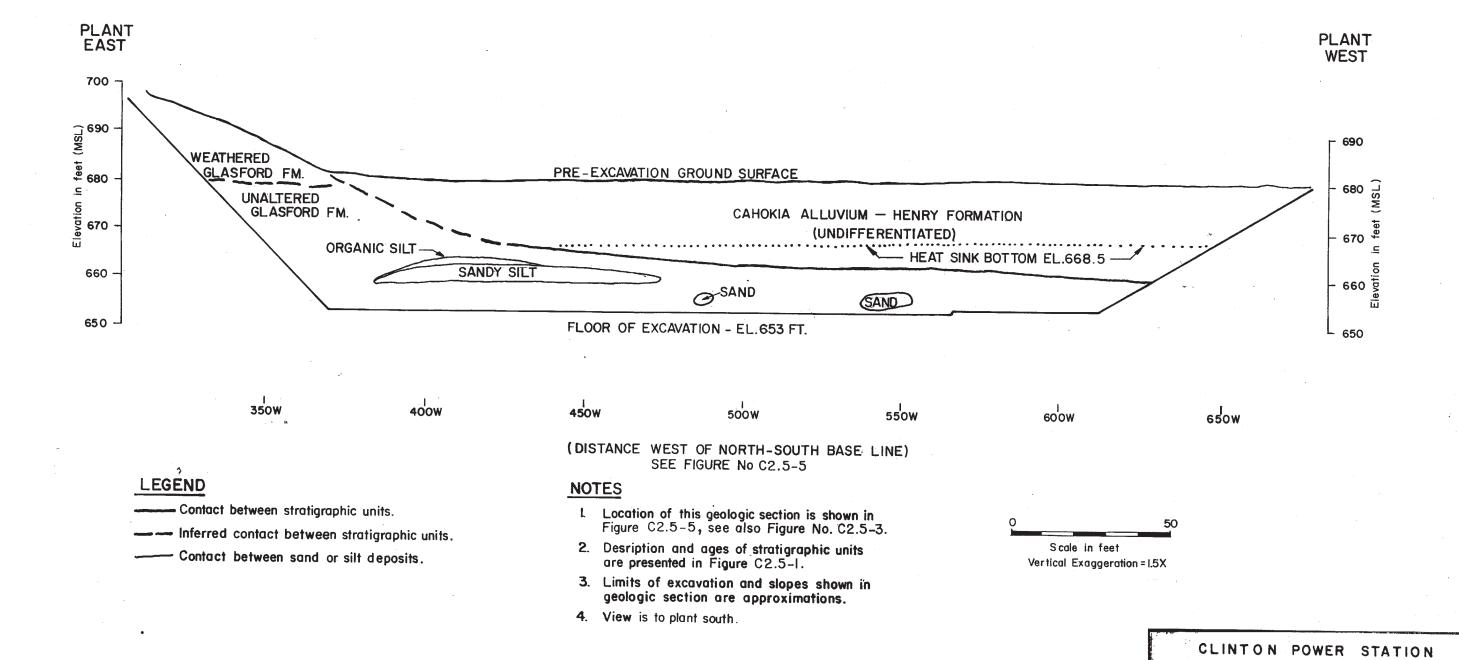
- REFER TO FIGURE C2.5-2 AND C2.5-1 FOR DESCRIPTIONS AND AGES OF STRATIGRAPHIC UNITS.
- 2. REFER TO FIGURES C2.5-3, C2.5-6 AND C2.5-19 FOR LOCATION OF GEOLOGIC SECTION.



CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-13

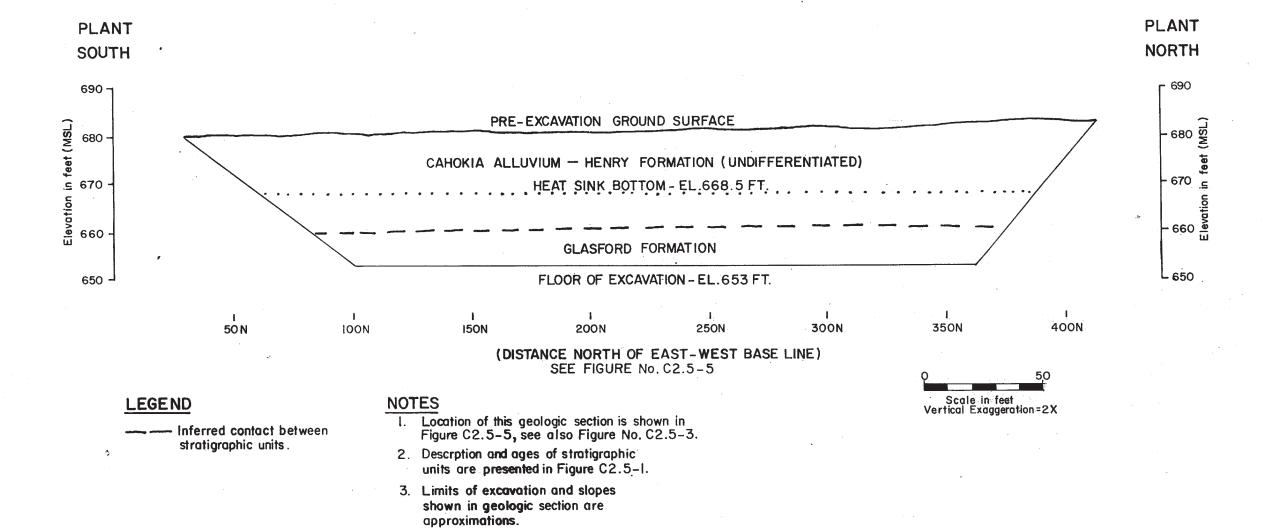
GEOLOGIC SECTION OF THE SOUTH ABUTMENT OF THE ULTIMATE HEAT SINK DAM EXCAVATION



UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-14

GEOLOGIC SECTION OF THE SOUTH WALL OF THE SCREEN HOUSE EXCAVATION

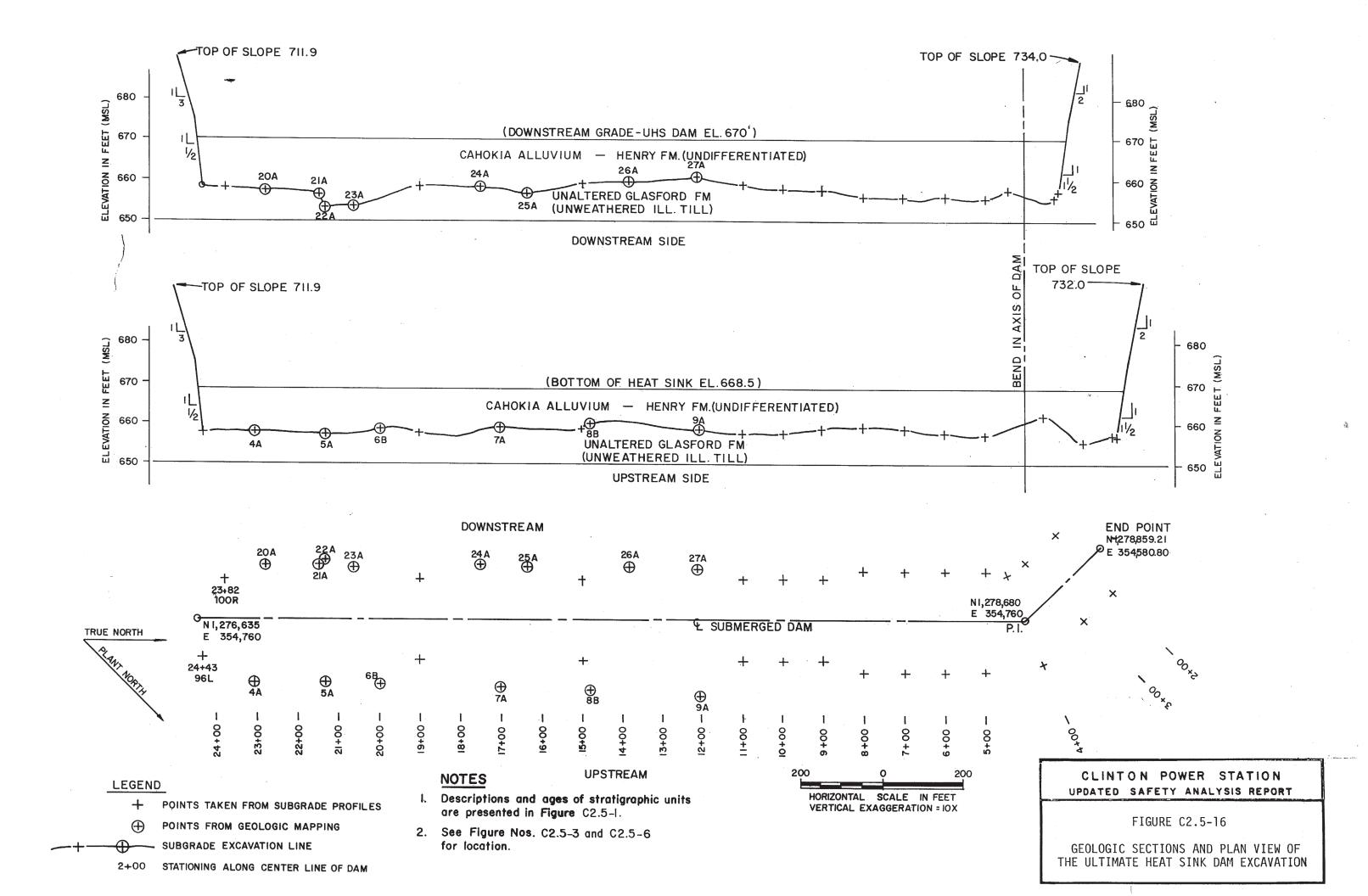


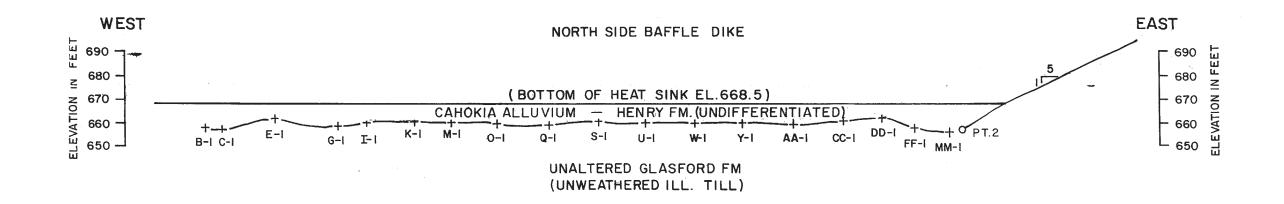
4. View is to plant west

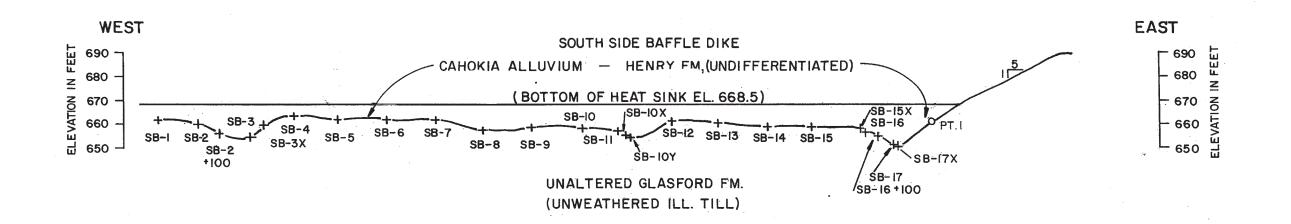
CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

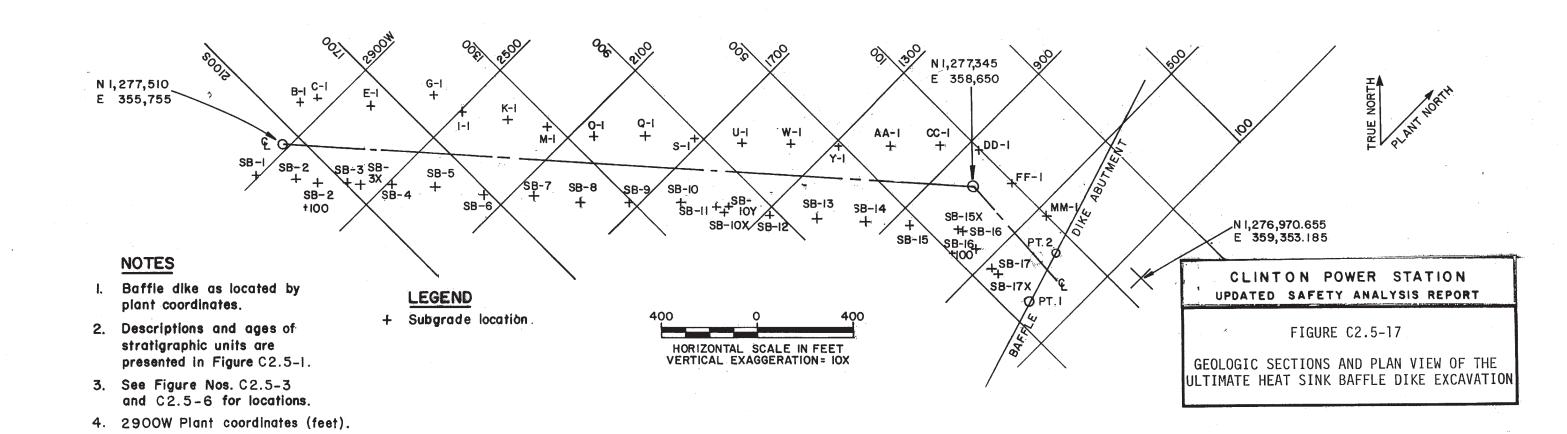
FIGURE C2.5-15

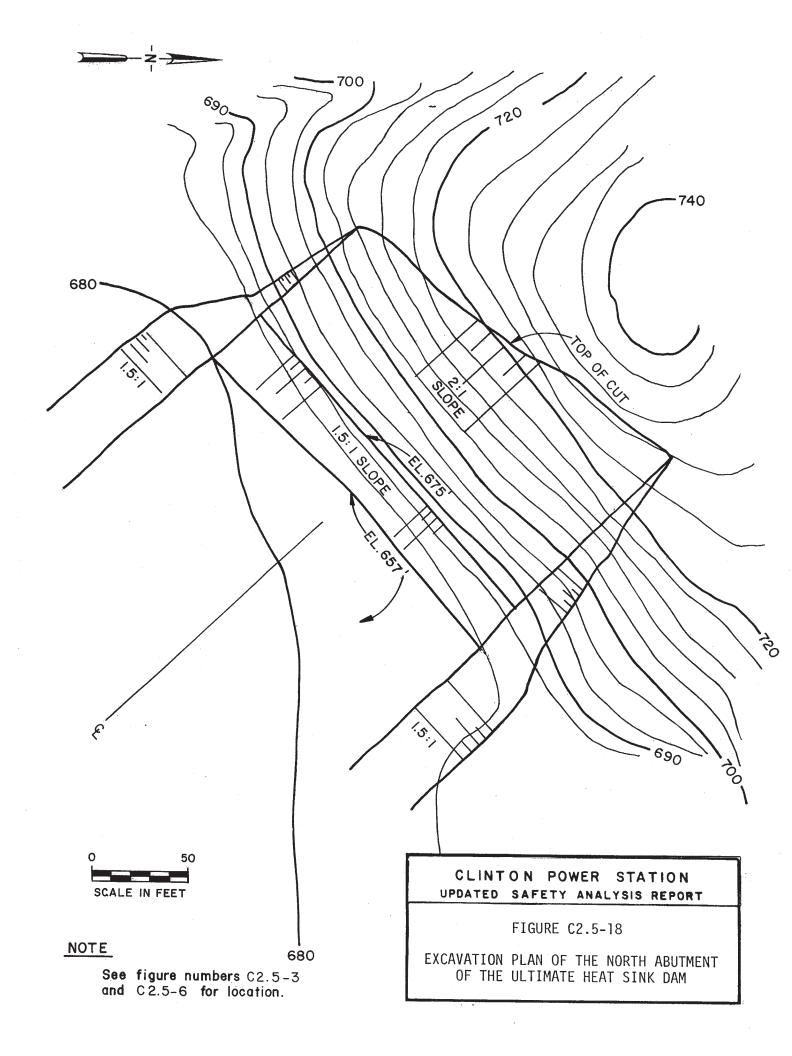
GEOLOGIC SECTION OF THE WEST WALL OF THE SCREEN HOUSE EXCAVATION

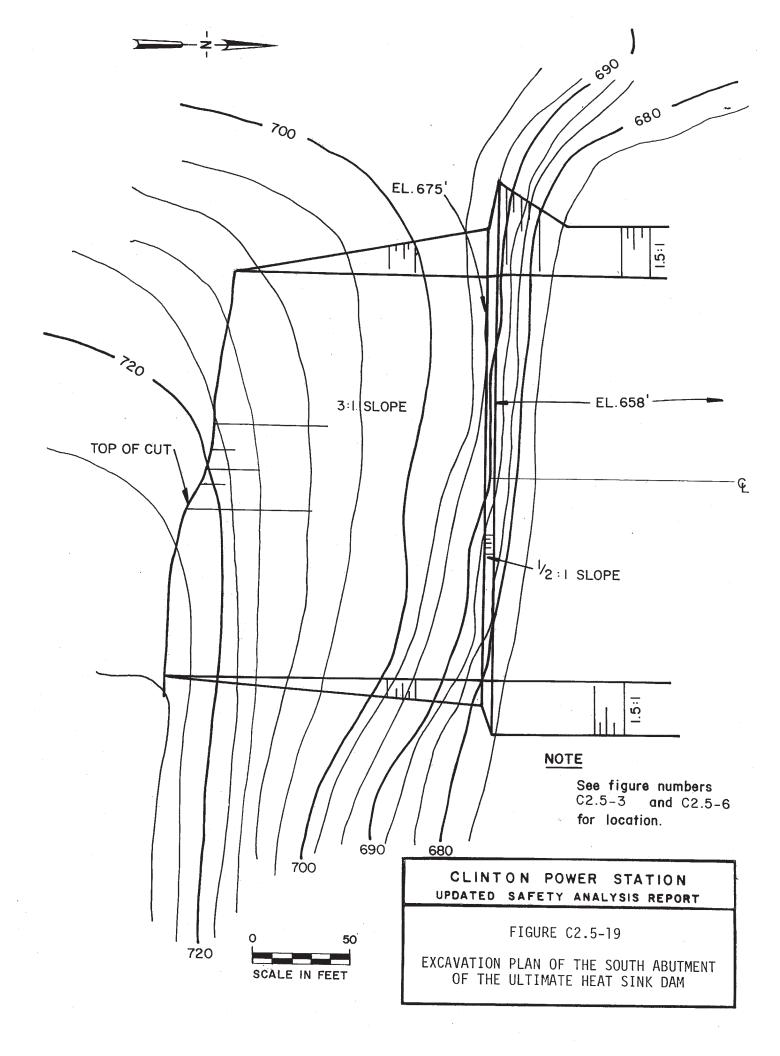


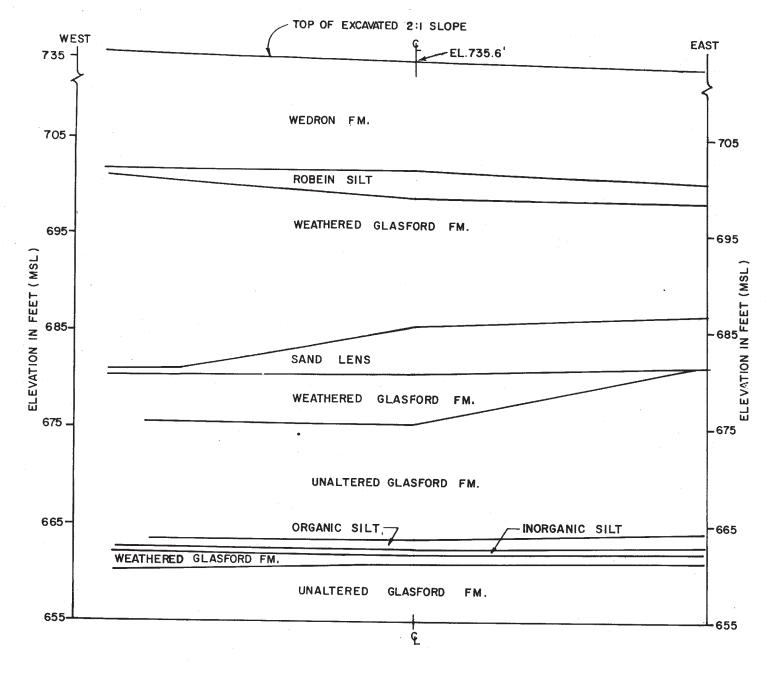












## 0 10 20 30

HORIZONTAL SCALE IN FEET VERTICAL EXAGGERATION = 3X

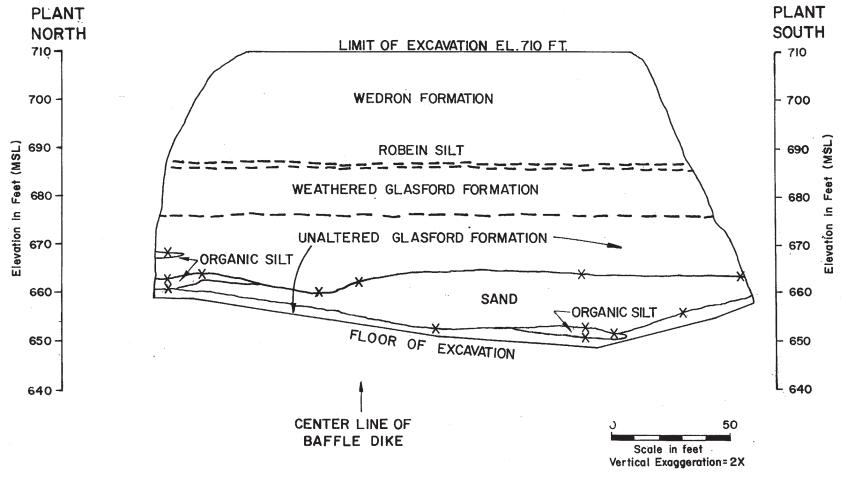
#### NOTES

- I. REFER TO FIGURE C2.5-1 FOR DESCRIPTIONS AND AGES OF STRATIGRAPHIC UNITS.
- 2. REFER TO FIGURES C2.5-3, C2.5-6 AND C2.5-18 FOR LOCATION OF GEOLOGIC SECTION.

## CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-20

GEOLOGIC SECTION OF THE NORTH ABUTMENT OF THE ULTIMATE HEAT SINK DAM EXCAVATION



## LEGEND

- ——— Inferred contact between stratigraphic units.
- ----- Contact between sand or silt units.
- X Survey point

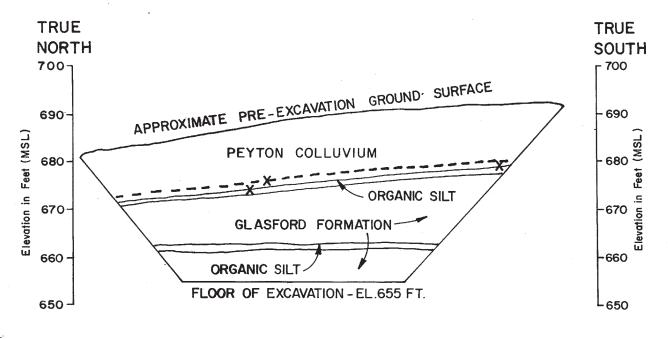
## NOTES

- I. Location of this geologic section is shown in Figure C2.5-6, see also Figure No. C2.5-3.
- 2. Description and ages of stratigraphic units are presented in Figure C2.5-1.
- Limits of excavation and slopes shown in this geologic section are approximations.
- View is to plant west.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-21

GEOLOGIC SECTION OF THE BAFFLE DIKE ABUTMENT EXCAVATION



## **LEGEND**

- --- Inferred contact between stratigraphic units
- ---- Contact between silt units
- X Survey point

## NOTES

- Scale in feet Vertical Exaggeration = 1.5 X
- I. Location of this geologic section is shown in Figure C2.5-6, see also Figure No. C2.5-3
- 2. Description and ages of stratigraphic units are presented in Figure C2.5-1.
- 3. Limits of excavation and slopes shown in this geologic section are approximations.
- 4. View is to true east.

CLINTON POWER STATION UPDATED SAFETY ANALYSIS REPORT

FIGURE C2.5-22

GEOLOGIC SECTION OF THE SSWS OUTLET STRUCTURE EXCAVATION

