

BECHTEL ASSOCIATES PROFESSIONAL CORPORATION
MIDLAND PLANT UNITS 1 & 2
JOB 7220

ADMINISTRATION BUILDING
FOUNDATION SETTLEMENTS
ALONG COLUMN LINE 0.4

Prepared by:

8408140570 840718
PDR FOIA
RICE84-96
PDR

GEOTECHNICAL SERVICES
December, 1977.

SB705862

INTRODUCTION

Early in September, 1977, we were requested by project engineering to assist in reviewing conditions surrounding footing settlements during construction of the Midland Project Administration Building. The foundation location plan for this building is shown in Figure 1. The affected foundations are those along Column Line 0.4.

The following data are presented to enable construction and engineering in evaluating the settlement of these footings.

BACKGROUND

The original ground at the Midland site was at approximately Elevation 608 in the vicinity of the administration building. After ground surface preparation, plant area fill was placed to approximately Elevation 634. An excavation was later made to about Elevation 610 to accommodate construction of the steam tunnel. Figure 2 shows a cross-section of the tunnel and the approximate excavation scheme. After construction of the tunnel, the west side of the tunnel excavation was backfilled to approximate Elevation 620 to construct the foundations along Column Line 0.4 of the administration building. After foundation construction, the remainder of the excavation was backfilled with sand to grade as shown in Figure 2.

During the early part of September, Geotech was made aware of settlements along the Column Line 0.4. The settlement data are given in Table 1.

FIELD OBSERVATIONS

During the week of September 19-23, 1977, several site reviews were made by engineering, construction, and Geotech personnel. These took place before

and after the removal of the subject footings.

Upon removal of Column PA 0.4, it was noted that the soil under and adjacent to it was soft. This was confirmed by pushing a $\frac{3}{4}$ " steel bar with little effort approximately two feet into the ground, by walking on the soil and noting its spongy characteristics, and by pushing of a shovel with little effort.

Tests taken at that time in and adjacent to PA 0.4 included moisture content, density, and unconfined compression. These tests also were taken at Column LN 0.4.

After these field observations, it was decided that two borings should be taken to further evaluate the conditions along Column Line 0.4.

At that time, Bechtel Construction's decision was that all affected footings be removed.

BORINGS

On September 27 and 28, 1977, two test borings were completed at footings LN 0.4 and HT 0.4. At footing LN 0.4, standard penetration tests (SPT) and shelby tubes (ST) were taken. At footing HT 0.4, standard penetration tests were taken.

Borings included visual inspection and description of soils, Qp tests (compressive strength of soil by the pocket penetrometer method) and any visual observations of water conditions (loss or gain).

Samples for proctor testing were also taken as shown in log of holes,
LNA, LNB, and HTA.

The boring logs are shown on Figures 3 through 7.

TESTING PROGRAM

Shelby tubes taken from Boring LN were submitted to U. S. Testing
Laboratory for unconfined compressive tests.

Samples taken at foundations PA 0.4 and LN 0.4 were also taken by U. S.
Testing personnel and unconfined compression tests were made. Results of
testing are given in Table 2.

It was also decided to run Proctor tests on the samples taken directly
under and adjacent to footings in order to determine the standard to be
used in calculating the in situ percent compaction. These results are
found in Figures 8, 9, and 10.

The Proctor curve in Figure 8 was used to calculate the in situ percent
compaction using the in situ dry density data reported by the Field. This
information is compared in Figure 9 with the percent compaction previously
reported. This comparison shows that the percent compaction was in all
cases lower than that previously determined.

In order to illustrate the effect of a reduced percent compaction on the
strength of soil, three of California Bearing Ratio (CBR) tests previously
made on three identical samples of the Midland soils are presented in Figure
11. The samples were compacted at three levels of compaction effort, which

resulted in compactive energies of 56,000 ft-lb/ft³, 20,000 ft-lb/ft³, and 12,400 ft-lb/ft³, respectively. It is seen that the pressure values for a penetration of 0.1" at the maximum dry density reduced from 94.5 psi to 5 psi by reducing the compactive energy from 56,000 ft-lb/ft³ to 12,400 ft-lb/ft³.

CONCLUSION

Based on available data the material under and adjacent to the subject footings, (Elevation 618-622) had insufficient bearing capacity to support the foundations.

The backfilled other than the soil in question (below 618) appears adequate and this conclusion is supported by SPT borings and compression tests.

SB705866

Administration Building
Anchor Bolts for Col. Line 0.4
Top Bolt Elev. 634' - 2-1/2"
Per Dwg. 981, Rev. 1, Sec. D

The Columns and Grade Beam
For Column Line 0.4 Shows
Settlement Per As Built
Elevations Taken 8-23-77

<u>Column</u>	<u>Elevation</u>	<u>Settlement (ft)</u>
P _a	634.10	0.11
N _k	634.03	0.17
M _p	634.01	0.20
L _N	634.05	0.16
K _p	634.02	0.19
K _B	633.93	0.28
J _F	633.93	0.28
H _T	633.92	0.29

SB705867

MIDLAND UNITS 1 & 2
ADMINISTRATION BUILDING EXCAVATION
UNCONFINED COMPRESSION TESTS

Sample No.	Sample Location	Sample Elevation	*Unconfined Compression Strength Lbs Per Sq Ft	**Allowable Bearing Value Lbs Per Sq Ft	Percent Strain	Remarks
1	PA - .0'	622.0	730	625	20.0	
2	PA - .04	621.0	487	420	20.0	
A	PA - .04	612.0	1984	1709	6.7	
B	PA - .04	611.0	633	546	20.0	
3	LN - .04	622.0	9.4	789	12.0	
4	LN - .04	621.0	2081	1792	5.0	
ST-1	Boring LN	617.5	4241	3653	10.3	
ST-2	Boring LN	615.5	2145	1849	20.0	
ST-3	Boring LN	603.0	5945	5123	9.1	
ST-4	Boring LN	597.5	3137	2704	20.0	
ST-5	Boring LN	588.0	2837	2423	20.0	

SBG 3369

Figure 1

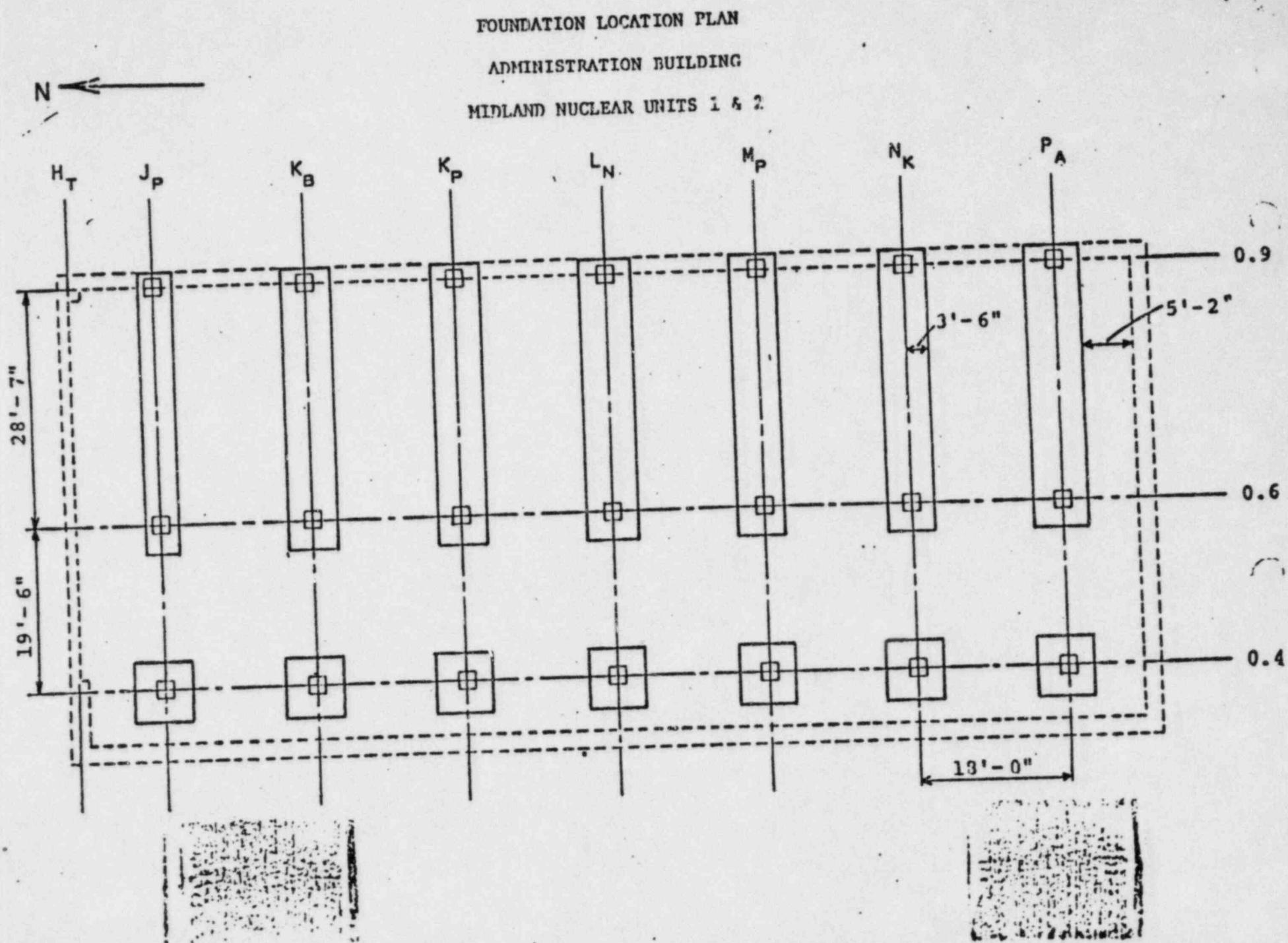
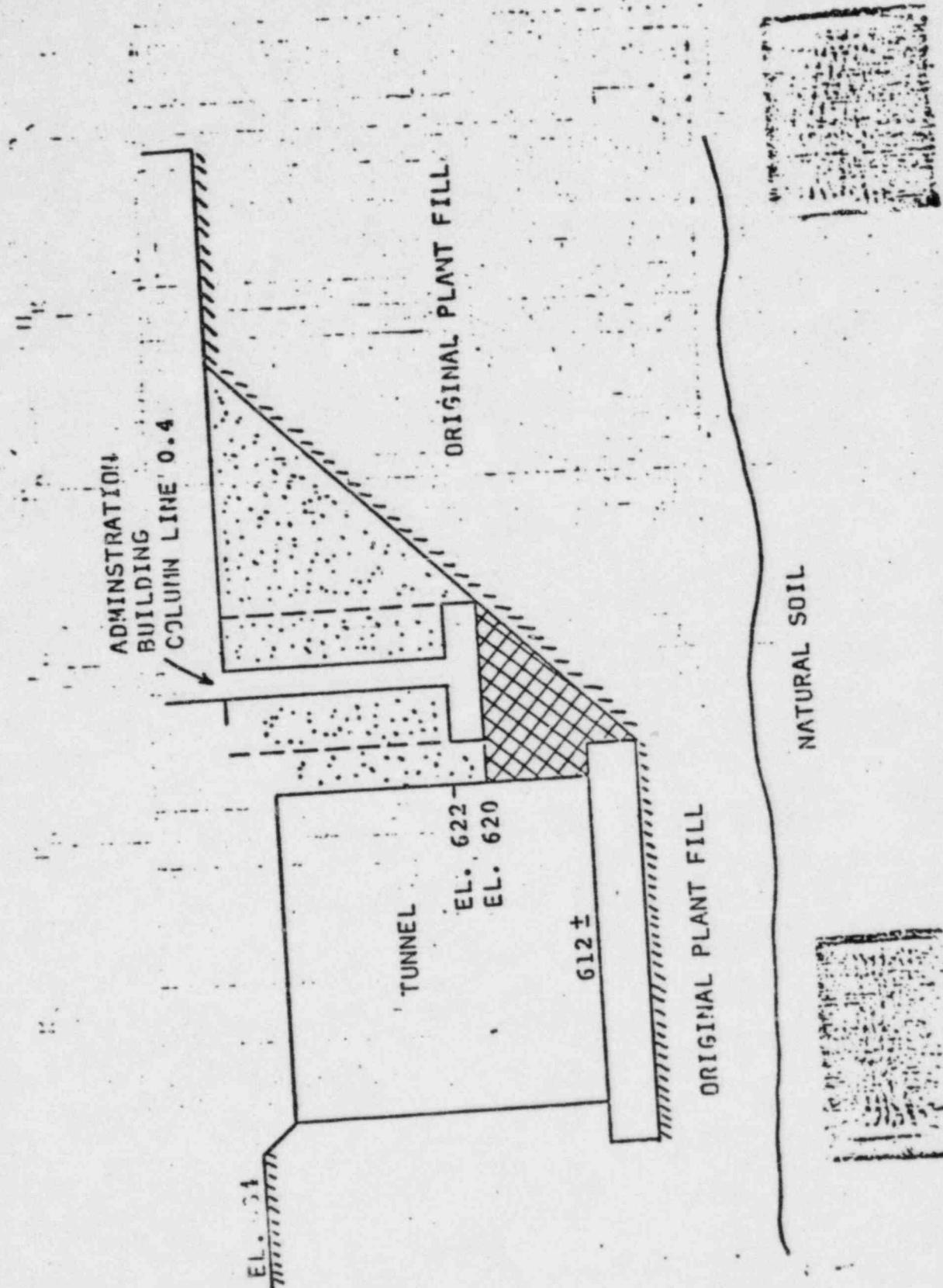


Figure 2



SB705870

BORING LOG				PROJECT MIDLAND NUCLEAR PLANT 7220				JOB NO. 1 or 1		HOLE NO. LN		
SITE ADMINISTRATION BLDG.				COORDINATES APPROX. 2' E OF FOOTING & 0.4-LN				ANGLE FROM HORIZ. 90°		BOREHOLE		
9/27/77	COMPLETED 9/28/77	DRILLER SINGLETON (ABEL DRILL.)	CME-550	DRILL MAKE AND MODEL	HOLE SIZE 5"	OVERBURDEN (FT) —	ROCK (FT) —	TOTAL DEPTH 43.5'				
CORE RECOVERY (%)		CORE DIA. (IN)	NUMBER OF CASING	GROUND EL. 622.0	DEPTH/EL. GROUND WATER (SEE NOTES COL.)	DEPTH/EL. TOP OF BOR.						
SAMPLE NUMBER WEIGHT/FT		CASING LEFT IN HOLE: DIA./LENGTH		LOGGED BY JERRY B. GIVENS								
140#/18"		NONE										
SAMPLE TYPE NO.	SAMPLE NO.	SAMPLE LENGTH (IN)	SAMPLE DIA. (IN)	PENETRATION BLOWS	ELEVATION	DEPTH IN FEET	SOIL TYPE	DEPTH IN METERS	DESCRIPTION AND CLASSIFICATION			NOTES DRI. WATER LEVEL. WATER RETURN. CHARACTER OF DRILLING, ETC.
									100%	10%	5%	
9/27/77												
2' ST	2'	1.5'	—	—	—	—						
2' ST	2'	0.9'	—	—	—	—						
2' SS	1.5'	1.0'	35	11	13	22						
2' SS	1.5'	1.2'	37	13	17	20						
2' ST	2'	0'	—	—	—	—						
2' SS	1.5'	0.5'	28	10	14	14						
2' ST	2'	1.0'	—	—	—	—						
2' SS	1.5'	0.9'	31	17	16	15						
2' ST	2'	1.7'	—	—	—	—						
2' SS	1.5'	0.5'	19	6	9	10						
2' ST	2'	0'	—	—	—	—						
2' SS	1.5'	0.2'	10	4	5	5						
2' ST	2'	1.3'	—	—	—	—						
2' SS	1.5'	0.2'	20	8	9	11						
2' SS	0.9'	100+	24	100+	—							
2' SS	0.8'	100+	71	100+	—							
2' SS	0.8'	100+	8	100+	—							
2' SS	0.5'	0.2'	100+	103	—							
9/28/77												
2' ST	2'	2.5'	—	—	—	—						
2' ST	2'	4.5'	—	—	—	—						
2' ST	2'	7.5'	—	—	—	—						
2' ST	2'	10.5'	—	—	—	—						
2' ST	2'	13.5'	—	—	—	—						
2' ST	2'	16.5'	—	—	—	—						
2' ST	2'	19.5'	—	—	—	—						
2' ST	2'	22.5'	—	—	—	—						
2' ST	2'	25.5'	—	—	—	—						
2' ST	2'	28.5'	—	—	—	—						
2' ST	2'	31.5'	—	—	—	—						
2' ST	2'	34.5'	—	—	—	—						
2' ST	2'	37.5'	—	—	—	—						
2' ST	2'	40.5'	—	—	—	—						
2' ST	2'	43.5'	—	—	—	—						
TOTAL DEPTH = 43.5' EL. BOTTOM = 578.5.												
WATER LEVEL 5.5' AFTER CASING PULL HOLE BACKFI. WITH SOIL A: COMPLETION												
SPLIT SPOON AT A SINGLE TUBE: B = BENTONITE; P = PITCHER; O = OTHER				SITE ADMINISTRATION BLDG.				HOLE NO. SR705871 LN				

Figure 4

BECHTEL

BORING LOG				PROJECT MIDLAND NUCLEAR PLANT				JOB NO. 7220	SHEET NO. 1 or 1	HOLE NO. LNB		
SITE ADMINISTRATION BLDG.				COORDINATES Z' WEST OF LNA				ANGLE FROM HORIZ. READING 90°				
DATE 9/26/77	COMPLETED 9/29/77	DRILLER SINGLETON (ABEL DRILL)	DRILL NAME AND MODEL CME-550	HOLE SIZE 5"		OVERTURE (FT.)	ROCK (FT.)	TOTAL DEPTH 5'				
CORE RECOVERY (FT%)		CORE BOXES	SAMPLES	EL TOP OF CASING	GROUND EL.	DEPTH EL. GROUND WATER (SEE HOLE "LN")	DEPTH EL. TOP OF ROCK					
SAMPLE HAMMER WEIGHT/FALL N/A				CASING LEFT IN HOLE: DIA./LENGTH NONE				LOGGED BY: JERRY B. GIVENS				
SAMPLE TYPE AND DIAMETER	SAMPLE ADVANCE LENGTH CORE RUN	SAMPLE RECOVERED CORE RECOVERY %	SAMPLE BLOWS PER CORE RECOVERY %	PENETRATION BLOWS			ELEVATION	DEPTH IN	SOIL TYPE	B M G R	DESCRIPTION AND CLASSIFICATION	NOTE: DRI. WATER LEVELS, WATER RETURN, CHARACTER OF DRILLING, ETC.
				1 st	2 nd	3 rd						
BULK	2.5'	2.5'	—	—	—	—	622	619.5	A. 2.5' 5'	N.Y.	0'-2.5' SAND BACKFILL 2.5'-5' COMPACTED CLAY	5" AUGER TO 5'; TOOK BULL SAMPLE FOR COMPACTION TEST FROM 2.5'-5' AND COMBINED IT WITH BULL SAMPLE FROM HOLE LNA
							617				TOTAL DEPTH = 5' EL. BOTTOM = 617	HOLE BACKFILL WITH SOIL AFTER COMPLET REFER TO BORING LOG "LN" FOR MORE INFO. CONCERN ING SOIL PROFILE
RE = SPLIT SPOON BY A SHELBY TUBE B = BENTONITE P = PITCHER R = OTHER				SITE ADMINISTRATION BLDG.				HOLE NO. LNB				

SB705873

BEECH

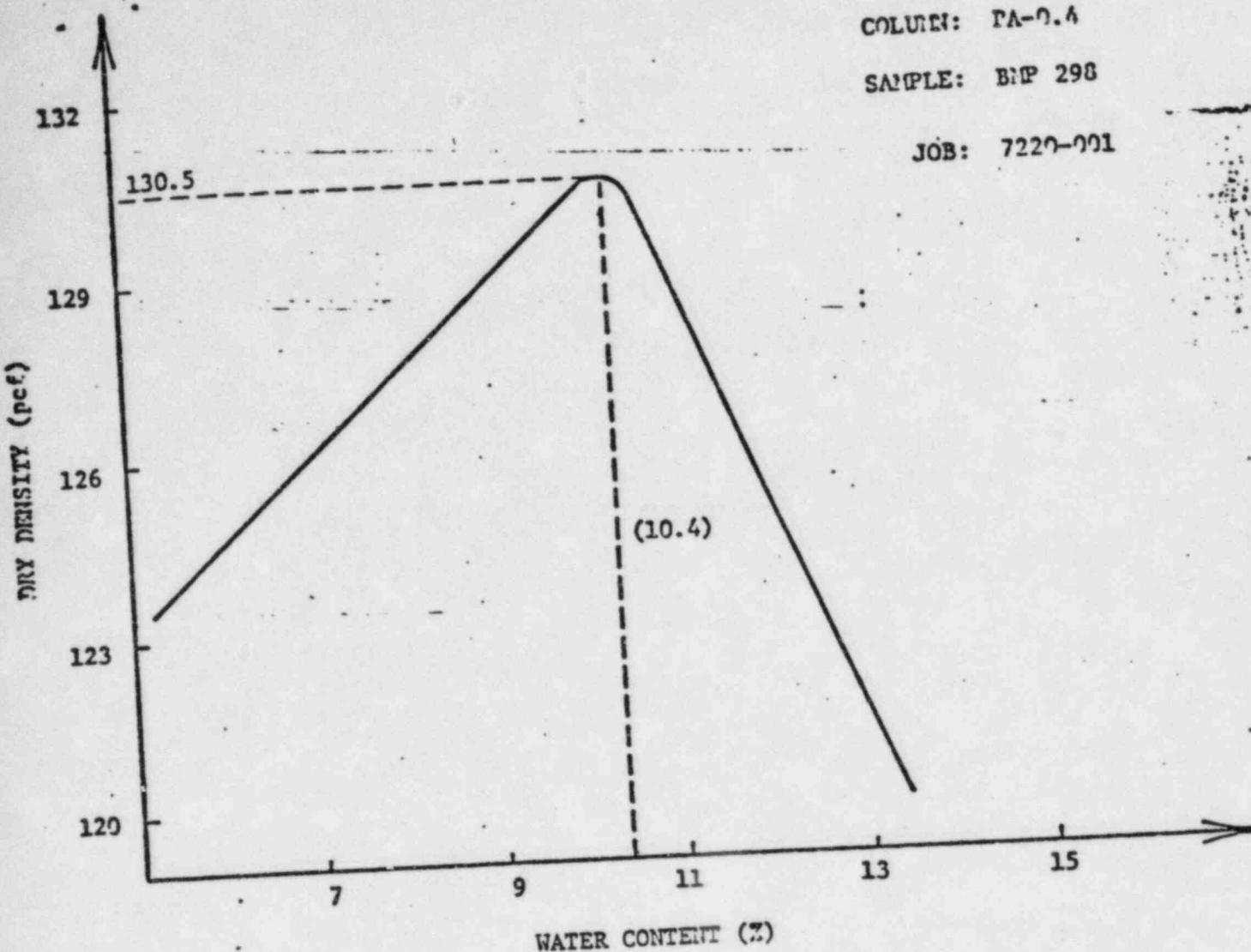
00 = SPLIT SPORNS; ST = SMALLY TURNED
0 = DENNISON; P = PITCHARD; S = OTHER

ADMINISTRATION SWG.

SB705875

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LOCATION: Administration Building
 COLUMN: PA-0.4
 SAMPLE: BMP 298
 JOB: 7220-001



Std. Compaction Data Used			Field Data		Original Calculated % Compaction		From Above Data % Compaction	
Name	$\gamma_d(\max)$	W_o	$\gamma_d(f)$	W_{of}				
BMP 262	123.9	11.8	117.5 120.5	17.5 13.3	94.0 97.0		90.0 92.3	
BMP 269	127.3	10.0	127.5	13.3	101.6		97.7	
BMP 270	124.6	11.1	118.7	16.7	95.7		91.0	
BMP 273	117.0	15.2	103.5	19.5	92.7		83.1	

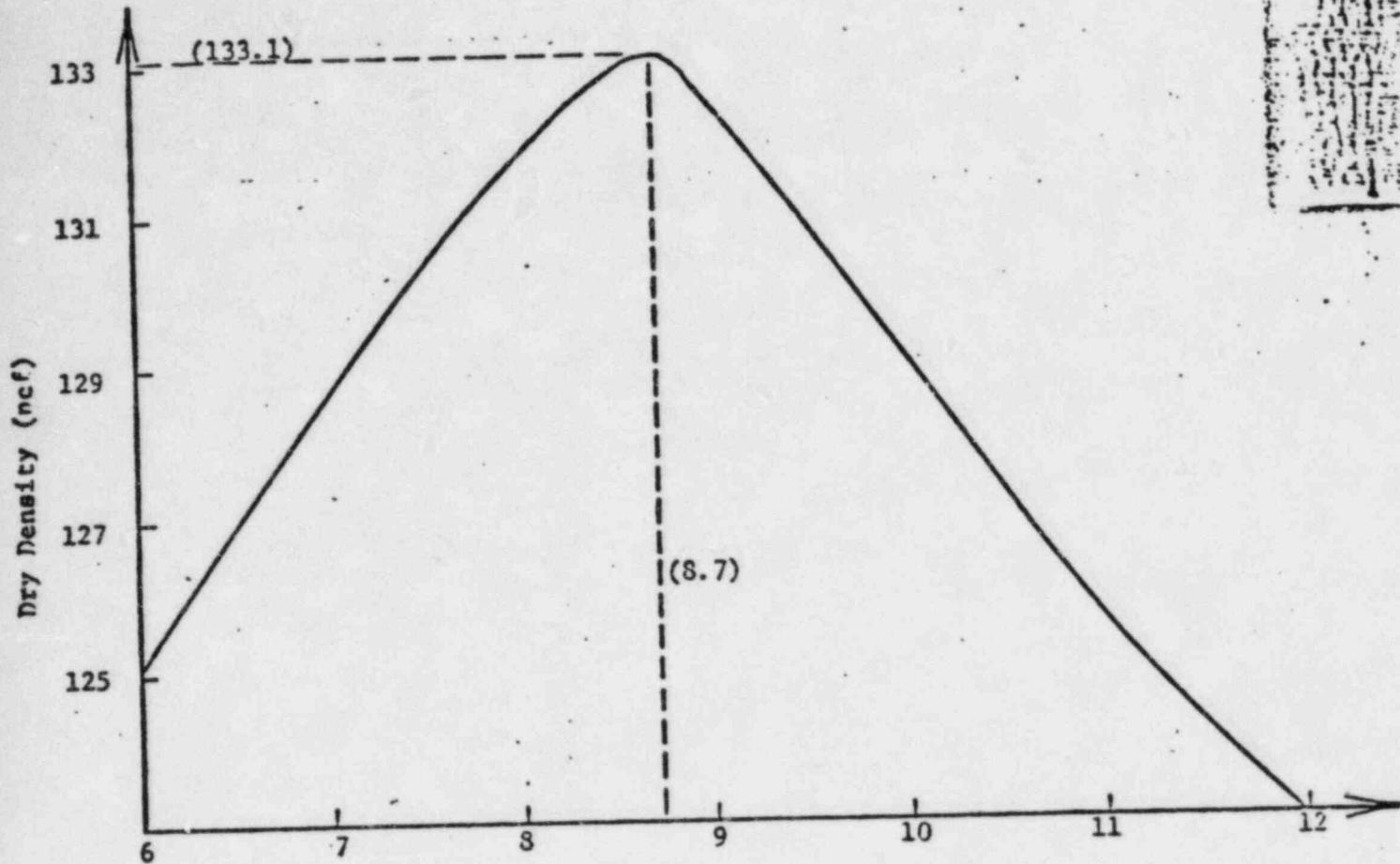
$\gamma_d(\max)$ = Maximum dry density as determined for a particular compaction test
 W_o = Corresponding optimum water content

$\gamma_d(f)$ = Field dry density
 W_{of} = Corresponding field moisture content

Fig 8

SB705876

Figure 9



MOISTURE CONTENT %

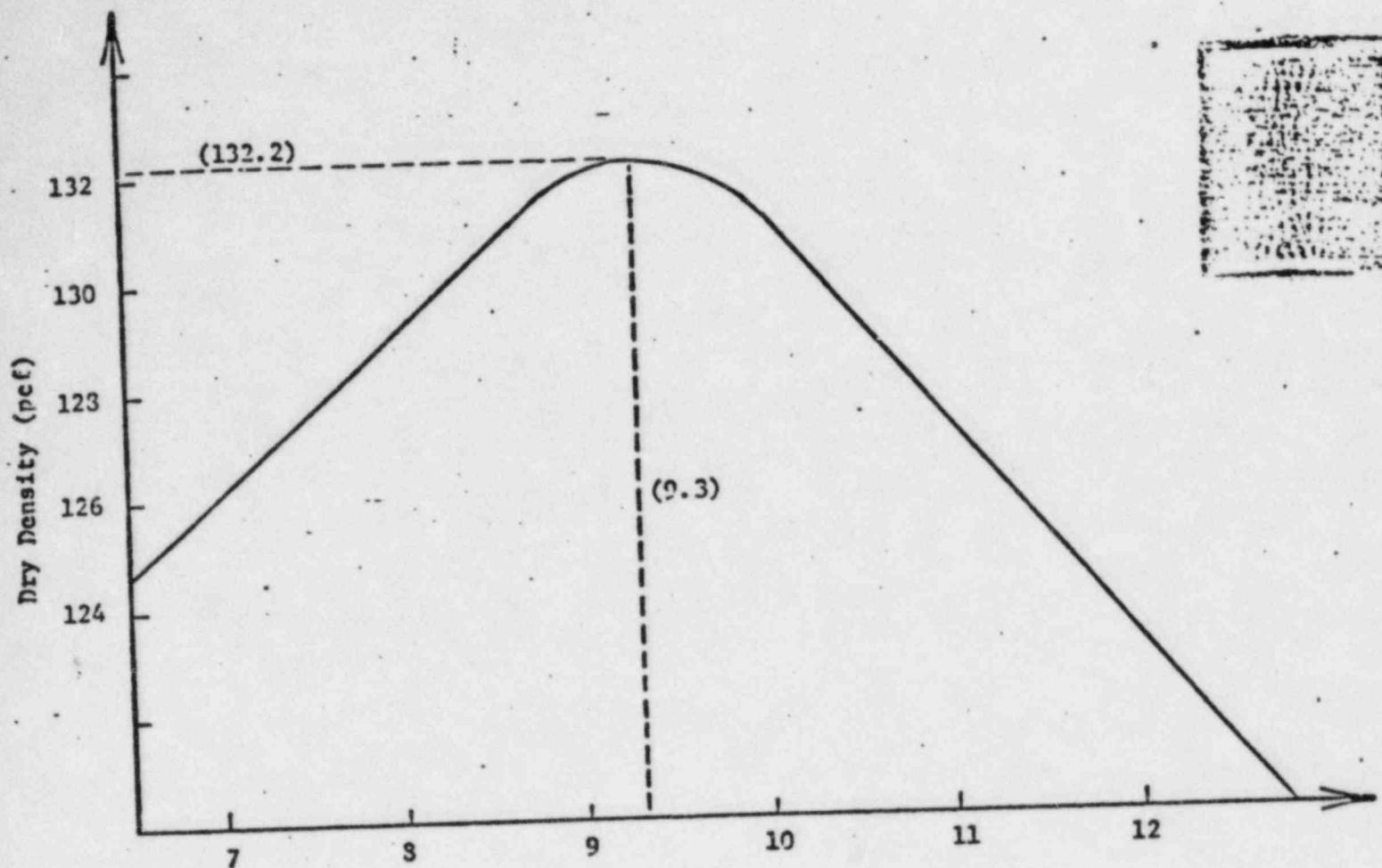
LOCATION: Administration Building

COLUMN: LN-0.4

SAMPLE: BMP-299

JOB: 7220-001

SB705877



MOISTURE CONTENT %

LOCATION: ADMINISTRATION BUILDING

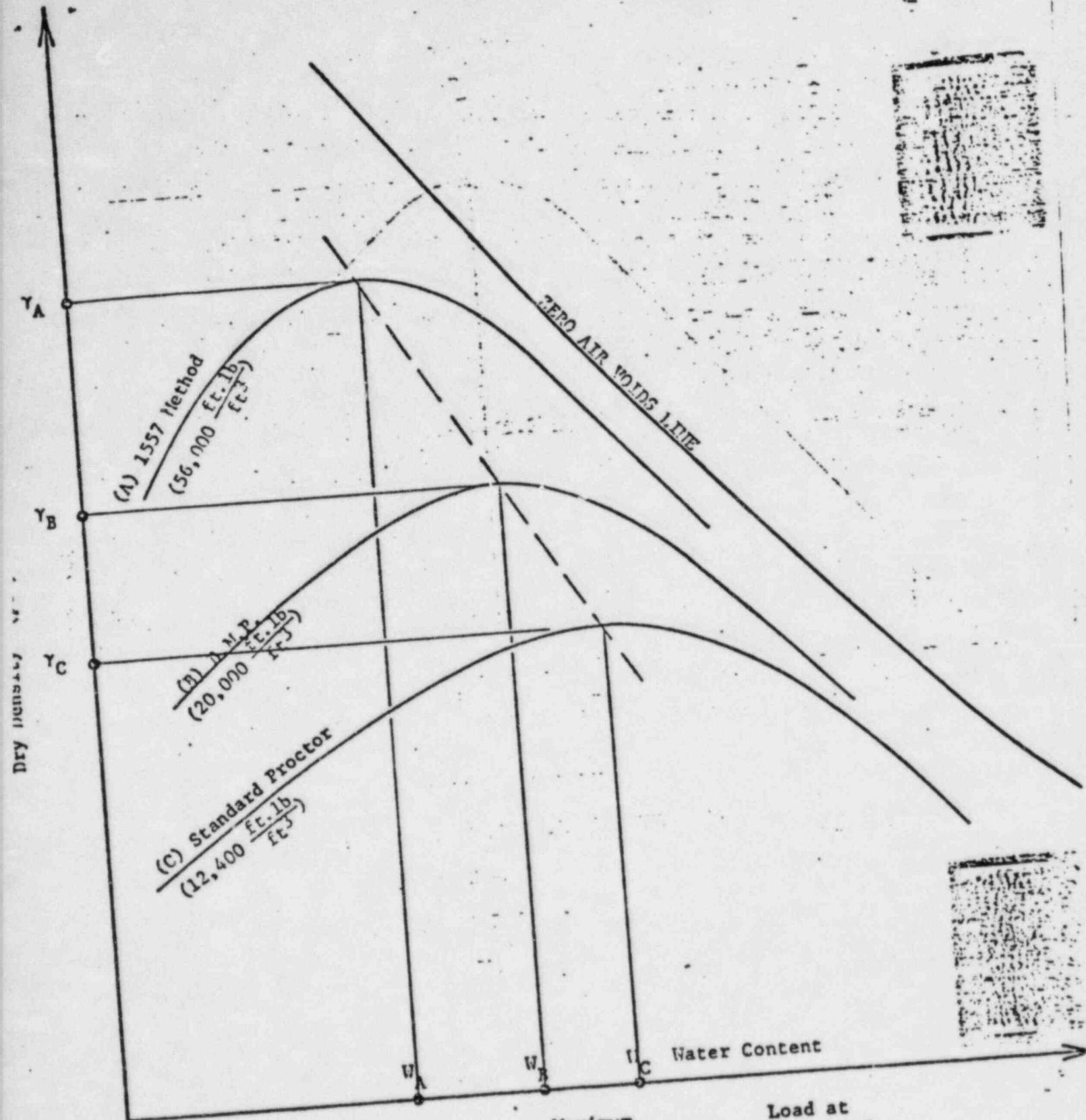
COLUMN: HT-0.4

SAMPLE: BTP-300

JOB: 7220-001

SB705878

COMPARISON OF 3 DIFFERENT STANDARDS



	<u>Optimum Water Content</u>	<u>Maximum Dry Density</u>	<u>Load at 0.1" Penetration</u>
A	12.7(%)	124.5(pcf)	94.5 (psi)
B	14.0(%)	117.0(pcf)	57.2 (psi)
C	15.0(%)	112.8(pcf)	5 (psi)

SB705873



CONSUMER
POWER
COMPANY

General Offices: 212 West Michigan Avenue, Jackson, MI 49201 • (517) 788-0550

September 18, 1981

William Paton, Esq.
Counsel for NRC Staff
U. S. Nuclear Regulatory Comm.
Washington, D.C. 20555

Dear Mr. Paton:

Attached hereto are copies of the documents requested by Joseph Kane with respect to the investigation following the Administration Building grade beam settlement. Included are copies of borings "D" and "E".

I look forward to seeing you again in October.

Very truly yours,

James E. Brunner

J. Kane
Re'd 9/21/81

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II

Paton/Thessia
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PE

BORING LOG

PROJECT MIDLAND NUCLEAR PLANT

JOB NO.

7220 1-1

HOLE NO.

E

SITE
EVAPORATOR AND AUX. BOILER

COORDINATES
AT FOOTING 8-CA

ANGLE FROM HORIZ. BEARING

90°

DESIGN COMPLETED DRILLER
S/20/77 E/20/77 SINGLETON (CASEL DRILL) CME-550

DRILL NAME AND MODEL

HOLE SIZE

OVERBURDEN (FT.)

ROCK (FT.)

TOTAL DEPTH

36.5'

CORE RECOVERY (%)

CORE BOXES SHIPPED

EL TOP OF CASING

GROUND EL

DEPTHL GROUND WATER

DEPTHL TOP OF ROCK

14

633

(SEE NOTES COL.)

SAMPLE NUMBER, WEIGHT/NULL

CASING LEFT IN HOLE: DIAM/LENGTH

LOGGED BY:

JERRY B. GIVENS

140# / 15"

NONE

SAMPLE TYPE
AND DISCR.

SAMPLE
NUMBER

SAMPLE
TYPE

SAMPLE
NUMBER

PENETRATION
BLOWS

ELEVATION

DEPTH

GRAPHIC LOG

IN
FEET

IN
METERS

DESCRIPTION AND CLASSIFICATION

NOTES ON:
WATER LEVEL,
WATER RETURN,
CHARACTER OF
DRILLING, ETC.

TYPE AND DISCR.	SAMPLE NUMBER	SAMPLE TYPE	SAMPLE NUMBER	PENETRATION BLOWS	ELEVATION	DEPTH	GRAPHIC LOG	IN FEET	IN METERS	DESCRIPTION AND CLASSIFICATION	NOTES ON: WATER LEVEL, WATER RETURN, CHARACTER OF DRILLING, ETC.
Z'551.5' 1.5"	30	15	19	11	629.5	5.5	0'-3.5' SILTY SAND, TAN (BACKFILL)	5"	1.5	"AUGER TO 3.5'; DRILLING IN 4"	
Z'551.5' 1.5"	30	15	19	11	629.5	5.5	3.5--5' CONCRETE MORTAR		1.5	TRI-CONE AND RE-	
Z'551.5' 1.5"	25	12	11	12	623.5	12.5	5'-6' X 1.5'-5.5' CLAYEY SAND TO SANDY CLAY, GREY, VELL STIFF, SLIGHT TO #19.0 = 4.5" TSF		3.75	CIRCULATING WATER	
Z'551.5' 1.5'	33	10	15	18	623.5	12.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		3.75	= 2.0# = 4.5" TSF	
Z'551.5' 1.5'	30	10	15	15	621	12.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		3.75	#3.2# = 4.5" TSF	
Z'551.5' 1.5'	58	10	21	37	617.5	17.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		5.25	#5.0# = (NONE-SAND)	
Z'551.5' 1.5'	54	13	24	32	615	17.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		5.25	#4.5# = 4.5" TSF	
Z'551.5' 1.0'	32	10	15	17	612.5	20.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		6.25	#7.0# = 4.5" TSF	
Z'551.5' 1.0'	22	11	12	15	608.5	24.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		7.5	#9 HAD PROBLEMS	
Z'551.5' 1.0'	43	10	21	22	607	25.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		7.65	WITH "DONUT" PUSHER, DRILLING IN 20 WHEN POUNDING,	
Z'551.5' 1.0'	59	15	24	35	605	26.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		8.15	TOOL DRILLED TO 5.5# AND HEATED?	
Z'551.5' 1.5'	55	12	13	45	601.5	28.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		8.75	AFTER #3-PE DROVE CASING	
Z'551.5' 1.0'	55	12	13	45	600	29.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		9.25	TO 21' BECAUSE	
Z'551.5' 1.0'	51	62	65	-	596.5	30.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		9.75	WATER WAS	
Z'551.5' 1.0'	51	-2	-2	-	596.5	30.5	1.5'-2' X 1.5' X 1.5' CLAY, GREY, VELL STIFF, SLIGHT MOISTURE		9.75	RUNNING UPSIDE	
										AND OUT HOLE	

TOTAL DEPTH = 36.5'
EL. BOTTOM = 596.5

#10# = (SAND)
#11# = 4.5" TSF
#12# = (SAND)
#13# = (SAND)
#14# = (SAND)

WATER AT 9.4 WHILE DRILLING
WATER LEVEL
AT 5.1 AFTER
DRILLING

* SPLIT SPONGE AT A SINGLET TUNNEL
** BERMISON: P = PITCHMENT S = OTHER

GIVE

EVAPORATOR AND AUX. BOILER BLDG.

HOLE NO.

E



Given in 3/26/81 Deposition

UNITED STATES
NUCLEAR REGULATORY COMMISSION
WASHINGTON, D. C. 20555

11/86

MAR 17 1981

Docket Nos. 50-329/330

MEMORANDUM FOR: Frank J. Miraglia, Chief
Licensing Branch No. 3
Division of Licensing

THRU: *JM* James P. Knight, Assistant Director
for Components and Structures Engineering
Division of Engineering

FROM: George Lear, Chief
Hydrologic and Geotechnical Engineering Branch
Division of Engineering

SUBJECT: MIDLAND - CHANGES IN REMEDIAL ACTION AS INDICATED
IN CONSUMERS FEBRUARY 27, 1981 TELECON

Plant Name: Midland Plants, Units 1 and 2

Licensing Stage: Post CP

Docket Numbers: 50-329/330

Responsible Branch: LB No. 3; D. Hood, LPM

Review Status: Continuing

The information provided in the March 13, 1981 memorandum to files on the above subject which was prepared by D. Hood is the basis for the following request by the Geotechnical Engineering Section.

(1) To minimize the potential for future differences in the review of the Midland project, we suggest that DOL arrange a telecon between Consumers Power Co. and their consultants with the Corps of Engineers and the GES prior to Consumers beginning the explorations at the Midland site. The objectives of the call would be:

- a. To make arrangements to have a Corps representative at the site when certain borings and sampling operations are underway.
- b. To discuss CPCo plans for selecting the locations and depths where undisturbed samples will be taken.

In addition to the above request for arranging the telecon, DOL is also requested to have Consumers Power Co. provide one copy of the PSAR to the U.S. Army Corps of Engineers, Detroit District.

8103231078 2pp

Frank J. Miraglia

-2-

MAR 17 1981

Consumers Power Co. is also requested to provide a list of reports completed by Dames and Moore for the Midland project which have not been provided to the NRC. The intent of this request is to determine what information exists beyond that provided in Appendices 2A and 2B of the FSAR documents.

Original signed by George Lear

George Lear, Chief
Hydrologic and Geotechnical
Engineering Branch
Division of Engineering

cc: R. Vollmer
J. Knight
G. Lear
L. Heller
D. Hood
H. Levin
W. Paton
N. Gehring, COE
J. Kane

OFFICE	HGEB:DE	HGEB:DE	HGEB:DE	A/D/CSE:DE			
SURNAME	JKane/mc	LHeller	GLear	JPKnight			
DATE	3/16/81	3/16/81	3/16/81	3/17/81			



UNITED STATES
NUCLEAR REGULATORY COMMISSION
WASHINGTON, D. C. 20555

J.Kone
Reid 3/13/81
Copy sent COE on
3/13/81

MAR 13 1981

Docket Nos.: 50-329/330

MEMORANDUM FOR: File

FROM: D. Hood

SUBJECT: FEBRUARY 27, 1981 TELECON REGARDING CHANGES IN REMEDIAL ACTIONS FOR MIDLAND SOIL SETTLEMENT

At 11:15 a.m. on February 27, 1981, Messrs. J. Cook, G. Keeley, and others of Consumers Power Company called Messrs. R. Vollmer, F. Miraglia (Acting for R. Tedesco), H. Levin, and D. Hood of NRC to report certain decisions and changes intended to expedite resolution of the soil matter on Midland Plant, Units 1 and 2.

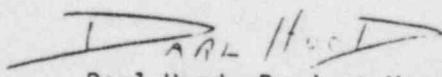
- (1) Mr. Cook has today authorized all borings, exploration and testing requested by the Staff's letter of June 30, 1980, Request 37, as subsequently amended by R. Tedesco letter. The Staff and the Corps of Engineers will be invited to participate as requested in the staff letter. Samples will be sent to an independent laboratory and results of analyses will be provided to the Staff.
- (2) For the Diesel Generator Building, the program will measure the pre-consolidation pressure of the boring sample and this will be correlated by analysis to what the surcharge program should have done. An error analysis of the uncertainty of this empirical data associated with borings will also be provided. Consumers would appreciate an opportunity to discuss these results with the staff prior to conclusion of the staff review.
- (3) The proposed remedial action for the Service Water Building has been changed. The use of piles has been dropped and a Bin Wall concept (essentially an extension of the entire North wall down to till) will be adopted. Underpinning was found to provide little seismic margin. A conceptual design package, including seismic discussions, will be presented for the new fix the first week in April.
- (4) The fix for the Aux. Bldg. remains the same, however more caissons might possibly be added if found to be needed. Other possibilities for lateral loads are being reviewed in the event such should be needed. A potential 50.55(e) report on the Aux. Bldg. seismic analysis was issued February 20, 1981.

8103260883 12pm.

MAR 13 1981

- 2 -

- (5) The drilling of wells for the permanent dewatering system may prove to be a pacing schedule item if a lengthy hearing results. Mr. Cook would like to explore with the Staff the possibility that the drilling of these wells might be acceptable to the staff prior to completion of the hearing. Mr. Cook noted that wells can always be plugged if necessary at some later date.
- (6) Two reports by Weston Geophysical, one for the seismic response spectra at the original ground surface and another on the probabilistic seismic hazards study will be forwarded March 2, 1981. A third report, covering the response spectra at the top of the fill will be forwarded later. A meeting on the first two reports is requested.
- (7) A 50.55(e) report on the BWST cracks was issued February 20, 1981. Five options are being considered at this time.



Darl Hood, Project Manager
Licensing Branch No. 3
Division of Licensing

cc: J. Kane
L. Heller
W. Paton
G. Lear
J. Knight
F. Schauer
F. Rinaldi
R. Bosnak
A. Cappucci
R. Gonzales
R. Jackson
J. Kimball
J. Gilray
R. Shewmaker
E. Gallagher
R. Knop
F. Miraglia
R. Vollmer
H. Levin
R. Tedesco

Note to Files:

Follow up w/ HGEB memo to DCL

ROUTING AND TRANSMITTAL SLIP

Date

4/22/81

12/86

TO: (Name, office symbol, room number,
building, agency/Post)

Initials

Date

1. L. Heller

2. G. Lear

3. J. Knight

4. D. Hood

5. W. Pator

Action	File	Note and Return
Approval	For Clearance	Per Conversation
As Requested	For Correction	Prepare Reply
Circulate	For Your Information	See Me
Comment	Investigate	Signature
Coordination	Justify	

REMARKS

Inevitable questions arise after reading the attached memorandum.

Why wasn't NRC notified of this test pile program in April 1980?

Why wasn't this document available in the deposition papers of either Afifi, Dhar or Ferris?

Why did Dr. Davissen mislead NRC in his deposition of January 14, 1981 (Pages 76 to 79)?

I think the problem in driving this pile and buckling the pile further reinforces HGE's requests to have Consultants at the site during the difficult underpinning work.

DO NOT use this form as a RECORD of approvals, concurrences, disposals, clearances, and similar actions.

FROM: (Name, org. symbol, Agency/Post)

Room No.—Bldg.

J. Kane

Phone No.

5041-102

G. U. S. G. P. O. 1977-241-53C/3090

OPTIONAL FORM 41 (Rev. 7-76)

Prescribed by GSA

FPMR (41 CFR) 101-11.206

Bechtel Associates Professional Corporation

Inter-office Memorandum

To: S. S. Afifi Date: 12 May 1980
Subject: Report of Pile Driving Operations From: L. A. Kendall
for Service Water Test Pile Of: Geotechnical Services
April 23, 1980

Copies to: S. L. Blue w/o At: Ann Arbor 10 D 5
A. Boos w/a
H. H. Burke w/a
L. H. Curtis w/a
B. Dhar w/a
W. R. Ferris w/a
J. O. Wanzeck w/a
1320, 3410

INTRODUCTION

Driving operations for the test pile to be installed near the northeast corner of the Service Water Building at the Midland jobsite were scheduled to begin early in the morning of Wednesday, April 23, 1980. M. T. Davisson (consultant) and J. O. Wanzeck (Geotech) were on hand to monitor the installation. The weather was cool (50°F) and clear with a light breeze.

OPERATIONS

The pre-drilling operation was not begun until late morning (10:30 a.m.) due to a mechanical problem with the drilling crane. Once started the pre-drilling operation proceeded without incident to a depth of 30 feet. The test pile was then dropped into the approximately 16 inch diameter hole with a free fall of 31 feet. Driving began after lunch and after the cushion had been repaired (the mycarta/aluminum stack was replaced). Once started, driving continued for 5 minutes before pile damage forced an interruption. (The top of the pile bent during driving and had to be removed). Driving resumed after a 20 minute delay, and continued for 12 minutes until a sudden change in pile resistance occurred. Canonie's Supt. G. Thorne felt that the pile had been broken. Consultant M. T. Davisson wanted to be sure that it was a broken pile, and not a soft soil layer, that caused the change in resistance. Driving was therefore briefly resumed. The very low resistance encountered as well as the sound of the hammer on the pile convinced all observers that the pile was broken.

RESULT

After driving operations were stopped and the pile was cut off, the location of the top of the shear was found to be at elevation 608. Measurements

Bechtel Associates Professional Corporation

Page 2
12 May 1980
S. S. Afifi

also determined that approximately 3 feet of pipe is buckled beneath the shear. As the pipe is in a Q-area, it will be concreted and left in place.

L. A. Kendall
L. A. Kendall

LAK/nm
Attachment

STRUCTURE SW GROUP NO. TEST PILE NO. 1

GROUND ELEV. 634 FT.

CUT-OFF ELEV. 636 FT.

TIP ELEV. 590 FT.

PAY LENGTH FT.

PILE TYPE 2725 x 500 TRIPSS LENGTH 75.25 FT

HAMMER & MANDREL VULCAN TYPE Q10

INTERNAL INSPECTION & REMARKS:

* PILE RENT DURING DRIVING: 19' 3 1/8" CUT-OFF

** PILE BROKE DURING DRIVING

DRILLING TIME

DRIVING TIME

START FINISH TIME

START FINISH TIME

1:35 PM 11:30 AM 55

1:53 AM 1:58 AM 5

AM PM AM PM

2:20 AM 2:32 AM 12

AM PM AM PM

2:38 PM 2:39 PM 1

TOTAL 55

TOTAL 18

PUNCH MARK ELEV.

SEE FALL 31 FT.

PUSHED 0

RETAP INFO.

UPLIFT

BATTER

NO. OF DOWNS	FT.	NO. OF BLOWS	NO. OF BLOWS PER INCH						
15	42	61 1/10"							41+
49	43	6							8
50	44	8							9
69									10
83									10
86									11
95									2
90									↓
67									1
72									↓

PILE INSPECTOR

L. KENDALL

ORIGINATOR LARIE KENDALLDATE 4-25-80

CALC. NO.

REV. NO.

CHECKED

DATE

JOB NO.

7220-101

SHEET NO.

PROJECT MIDLANDSUBJECT SW TEST PILEMEASUREMENTSLOCATION OF PILE: S 4983
E 800DRIVING BOOT: $1.5'' \times 14\frac{3}{16}''$ CUSHION: SMALL STACK $(7\frac{3}{4}'' \times 14'')$ 4 MYCARA $(13\frac{3}{16}'' \text{ OD}, 3'' \text{ ID } \times 1'')$ 3 ALUMINUM $(13\frac{3}{4}'' \text{ OD}, 2\frac{7}{8}'' \text{ ID } \times \frac{1}{8}'')$ PENNY $(2'')$ FIRST CUT-OFF: $19' 3\frac{1}{2}''$ FOUND TO BE $\frac{3}{8}''$ WALL AT TOP, $\frac{1}{2}''$ WALL AT BOTTOMFINAL CUT-OFF: $10' 10''$ ($\frac{1}{2}''$ WALL)

FINAL ELEV. OF PIPE = 6.36 FT

PIPE BUCKLE ~3 FT

TOP OF BUCKLE / SHEAR AT EL. 6.08 (28' BELOW TOP OF PIPE)

→ NOTE: HAMMER STROKE FOUND TO BE APPROXIMATELY $2'' 2\frac{1}{2}''$ LONG

ROUTING AND TRANSMITTAL SLIP

TO (Name, office symbol or location)

~~W. Lator, DEQP~~
~~D. Flood, NRR~~
~~J. Kepler, RII~~
~~L. Heller/J. Kane, NRR~~
J. Gillray, NRR
F. Rinaldi, NRR
A. Cappucci, NRR
G. Gallagher, IE
G. Fiorelli/K. Neidu, RII

INITIALS	CIRCULATE
DATE	COORDINATION
INITIALS	FILE
DATE	INFORMATION <input checked="" type="checkbox"/>
INITIALS	NOTE AND RETURN
DATE	PER CONVERSATION
INITIALS	SEE ME
DATE	SIGNATURE

REMARKS

Attached is updated listing
on log of depositions
related to the Midland Hrg.

X C:
Brad Jones
J. Kane
G. Lear
J. Kinball
R. Gonzales

Given in
3/26/81
Deposition

Do NOT use this form as a RECORD of approvals, concurrences,
disapprovals, clearances, and similar actions.

FROM (Name, office symbol or location)

Stewmiller

DATE	3/13/81
PHONE	27551

5041-101

OPTIONAL FORM 47
AUGUST 1967
GSA FPMR (41CFR) 100-11.208

GPO 0-15-81418-1 419-018

J. Kane Rec'd 3/24/81

S. Kane
13/86

3/13/81

MIDLAND HEARING

Prehearing Conferences

12/14/78 Prehrg. Conf. pp. 1-233

9/10/80 Prehrg. Conf. pp. 234-408

1/28/81 Prehrg. Conf. pp. 409-690

1/29/81 Prehrg. Conf. pp. 691-826

2/24/81 Corrections to Transcript of 1/28/81

<u>Depositions Of</u>	<u>Date</u>	<u>Pages</u>	<u>Exhibits</u>
G. Gallagher (NRC-IE)	11/17/80	1-116	1-8
	11/18/80	117-345	
	12/16/80	346-528	
K. Naidu (NRC-IE)	2/26/81	1-174	1
G. Fiorelli (NRC-IE)	2/17/81	1-152	1-11
J. Keppler (NRC-IE)	1/6/81	1-164	1-7
	1/16/81	165-248	8-13
R. Shewmaker (NRC-IE)	1/19/81	1-165	1-28
H. Thornburg (NRC-IE)	2/20/81	_____	_____
D. Hood (NRC-LPM)	10/7-8/80	1-314	1-21
Transcript Corrections	12/3/80 2/19/81 12/29/80	315-407 _____ 1-4	26-32
Transcript Corrections	2/17/81	1-6	
L. Heller (NRC-HGEB)	10/9/80	1-139	1-6
	12/4/80	139-247	
	12/5/80	248-353	
J. Kane (NRC-HGEB)	10/14/80	Vol. I, 1-154a	1-17
	10/15/80	Vol. II, 1-209	
	10/16/80	Vol. III, 1-94	
	12/2/80	Vol. IV, 1-185	
	12/3/80	Vol. V, 186-357	
	12/4/80	"Vol. VI, 358-406	
Transcript Corrections	12/5/80	1-9	
J. Simpson (USACE)	11/19/80	1-145	1-17

MIDLAND HEARING

<u>Depositions Of</u>	<u>Date</u>	<u>Pages</u>	<u>Exhibits</u>
W. Otto (USACE)	1/19/81	1-165	1-7
R. Erickson (USACE)	1/20/81	1-85	1-4
H. Singh (USACE)	12/18-19/80	Vol. I, 1-215	A, 1-6
	1/21/81	Vol. II, 216-343	
	1/22/81	1-209	
		1-100	
F. Rinaldi (NRC-SEB)	1/6/81	1-162	1-16
P. Huang (NSWC)	1/9/81	1-54	1-3
J. Matra (NSWC)	1/7/81	1-114	1-4
A. Cappucci (NRC-MEB)	1/22/81	1-114	1-14
J. Brammer (ETEC)	1/22/81	1-61	1-4
W. Chen (ETEC)	1/21/81	1-131	1-10
J. Gilray (NRC-QAB)	2/20/81	1-54	1-3
G. Keeley (CPC)	10/23/81	1-87	1 & 2
T. Cooke (CPC)	10/22/80	1-64	1-3
D. Horn (CPC)	10/21-22/80	1-174	1-3
		175-245	
T. Thiruvengadam (CPC) (formerly Bechtel)	12/11/80	1-61	1 & 2
W. Ferris (Bechtel)	12/10-11/80	1-252	

MIDLAND HEARINGS

<u>Depositions Of</u>	<u>Date</u>	<u>Pages</u>	<u>Exhibits</u>
S. Afifi (Bechtel)	10/29/80	1-123	1-6
	10/30/80	124-223	& Includes
	10/31/80	224-261	Informal Discovery Documents
B. Dhar (Bechtel)	12/17/80	1-160	Informal Discovery Documents
R. Peck (Bechtel Consult)	1/13/81	1-140	1-5
	1/14/81	141-205	
A. Hendron (Bechtel Consult)	1/27/81	1-165	1-33
	1/28/81	166-230	
M. Davisson (Bechtel Consult)	1/14/81	1-147	1-8
C. Gould (Bechtel Consult)	1/8/81	1-120	1 & 2