

101 California Street, Suite 1000, San Francisco, CA 94111-5894

415 397-5600

October 3, 1984 84042.030

Mrs. Juanita Ellis President, CASE 1426 S. Polk Dallas, Texas 75224

Subject: Responses to Cygna Design Control, Pipe Support, and Pipe Stress

Questions

Comanche Peak Steam Electric Station Independent Assessment Program - Phase 3 Texas Utilities Generating Company

Job No. 84042

Dear Mrs. Ellis:

Enclosed please find copies of additional responses to Cygna design control, pipe support and pipe stress questions.

This should complete the transmittal of responses received to date for the Phase 3 Independent Assessment Program. We shall be transmitting responses associated with the Phase 4 Independent Assessment Program in the near future. Feel free to call me if you have any questions or wish to discuss the enclosed documents.

Very truly yours,

n. H. Williams

N. H. Williams

Project Manager

dmm

Attachments

cc: Mr. S. Treby (NRC), w/attachments

Mr. S. Burwell (NRC), w/attachments

Mr. D. Wade (TUGCO), w/o attachments

Ms. J. Van Amerongen (TUGCO), w/o attachments

Mr. D. Pigott (Orrick, Herrington & Sutcliffe), w/o attachments

8411060436 841003 PDR ADOCK 05000445 A PDR 2222 - Per S. Burwell See Attached

Attachments

- L. M. Popplewell (TUGCO) letter to N. Williams (Cygna), "CPSES Cygna Review Questions (Pipe Supports)," June 28, 1984.
- L. M. Popplewell (TUGCO) letter to N. Williams (Cygna), "CPSES Cygna Review Questions (Pipe Supports)," July 2, 1984.
- R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69245, "Additional Response to Cygna letter 84042.007 dtd. 6/23/84," July 12, 1984.
- 4. L. M. Popplewell (TUGCO) letter to N. Williams (Cygna), "CPSES Cygna Review (Pipe Supports)," July 12, 1984.
- 5. R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69250, "Followup Information from G&H," July 13, 1984.
- R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69249, "Pipe Stress Review - Mass Participation," July 13, 1984.
- R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69296," Cygna IAP Phase 3 Report, July 27, 1984.
- 8. R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69303, "Tapered Transition Joint SIF," July 31, 1984.
- 9. R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69316, "Revised Mass Participation Fraction Sensitivity Study," August 3, 1984.
- J. T. Merritt (TUGCO) memorandum to J. B. George (TUGCO), "CPSES Document Control Center," September 6, 1984.
- 11. R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69369, "Transition Joint SIF at Equipment Nozzle Connections," August 23, 1984.
- 12. R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69368, "Mass Participation," August 23, 1984.
- 13. R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69373, "Mass Participation," August 24, 1984.
- R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69339, "Mass Participation," August 10, 1984.
- 15. L. M. Popplewell (TUGCO) letter to N. Williams (Cygna), "Comanche Peak Steam Electric Station, Phase III Action Items," August 29, 1984.

- 16. J. B. George (TUGCO) letter to N. Williams (Cygna), CPPA #40439, "Comanche Peak Steam Electric Station, Cygna Phase III, Independent Assessment Program," August 16, 1984.
- 17. R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69454, "Mass Participation," September 14, 1984.
- 18. R. C. Iotti (EBASCO) letter to N. William (Cygna), 3-Z-17 (6.2), ETCY-1, "U-Bolt Cinching Testing/Analyses Program Phase 3 Open Items, Additional Information as follow-up to Meeting of 9/13/84," September 18, 1984.
- 19. R. E. Ballard (G&H) letter to J. B. George (TUGCO), GTN-69359, "Transition Joint SIF at Equipment Nozzle Connections," August 17, 1984.

PROJECT FILE

TEXAS UTILITIES SERVICES INC.

P. O. BOX 1002 - GLEN ROSE, TEXAS 76043

June 28, 1984

CYGNA Energy Services 101 California Street San Francisco, CA 94111

DATE LOGGED:

YGNA

Attention: Ms. Nancy Williams, Project Manager

FILE:

JOB NO :

COMANCHE PEAK STEAM ELECTRIC STATION CPSES CYGNA REVIEW QUESTIONS (PIPE SUPPORTS)

Reference: 1) June 26; Telecon between D. Rencher (TUGCO) and

CROSS REF. FILE

J. Minichiello (CYGNA),

Dear Ms. Williams:

Below is TUGCO's response to the above referenced telecon.

Telecon of June 26 Regarding Support MS-1-003-04-C72S:

The question of structural acceptability of the 1"x7"x12" washer plate (item 17) is still open. In lieu of performing detailed calculations and finite element analyses to demonstrate acceptability, the problem will be simplified by making two (2) conservative assumptions:

- 1) Assume the washer is 1"x6"x6" and is centered over the insert. The rear bracket to plate to tube steel connection is separate.
- 2) Assume there is no weld between the 1"x6"x6" washer and the tube steel.

With the above assumptions, Table 7 of Section 20 of the NPSI Structural Design Manual (attached) may be applied directly. This table states that a 1" thick plate may be used for insert tension loads less than or equal to 17.5 kips. Based on the detailed calculation performed by NPSI, tension in the insert is 18.2 kips. This apparent slight overload is acceptable, however, for the following reasons:

1) Because of installation tolerances, skew angles of less than 5° are generally not considered in support design. Consideration of the 2.3° skew on this support increased the tension load in the insert from 15.8K to 18.2K. 15.8K would have been perfectly acceptable to use for design.

- 2) Conservative assumptions were made by NPSI in their sizing calculations for washer plates (e.g.: point load at center of plate and neglecting stiffening affect of tube steel). Actual stress in a 1" plate subjected to a 17.5K load is well below allowable limits.
- Conservative assumptions were made on this support (see items
 and (2) in first paragraph).

Based on the above reasoning, the 1"x7"x12" washer plate on MS-1-003-004-C72S is capable of performing its function as intended. This is further demonstrated by the fact that the support is installed and continuously subjected to its full design load and has not shown any signs of high stress.

If there are any further questions or comments, please contact Mr. George Grace at extension 500.

Very truly yours,

TEXAS UTILITIES GENERATING COMPANY

L. M. Popplewell Engineering Manager

GG/amd

TEXAS UTILITIES GENERATING COMPANY

CYGNA

JOB NO :

DATE LOGGED .

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8404 2 PF

LOG NO .:

CYGNA Energy Services FILE: 101 California Street

2.1.1 mr. cR

San Francisco, CA 9411cRoss REF. FILE &

8/6/84

July 2, 1984

ATTENTION: Ms. Nancy Williams, Project Manager

COMANCHE PEAK STEAM ELECTRIC STATION CPSES CYGNA REVIEW QUESTIONS (PIPE SUPPORTS)

Reference: 1) June 29; Telecon between D. Rencher (TUGCO) and J. Minichiello (CYGNA)

Dear Ms. Williams:

Below is TUGCO'S response to the above referenced telecon regarding Flare Bevel Welds.

CYGNA Questions:

- a) It is our interpretation of existing weld standards that the effective throat of a flare bevel weld (te) is 5/16R. Assuming R=2t, then te=5/3t. What justification does TUGCO have for using te=t in flare bevel weld design?
- b) Please provide documentation which instructs engineers how to calculate the effective throat of a flare bevel weld with a fillet cap.

TUGCO Response:

- a) Per AWS D1.1, 19/9 edition, figure 10.13.1.3B, an effective throat of t is specified (see attached). Based on the geometry of the joint, test is a reasonable value.*
- b) Calculation of weld effective throat (in any joint) is based on the shortest distance from the root of the weld to the face of the weld. For a flare bevel with a fillet cap, the engineer uses this approach in calculating te. The example supplied to CYGNA for MS-1-003-013-C72k. shows this calculation is done.

TEXAS UTILITIES GENERATING COMPANY

P. O. BOX 1002 GLEN ROSE, TEXAS 76043

* Please note that the ASME code does not address flare bevel weld design.

If there are any further questions or comments, please contact Mr. George Grace at extension 500.

Very truly yours,

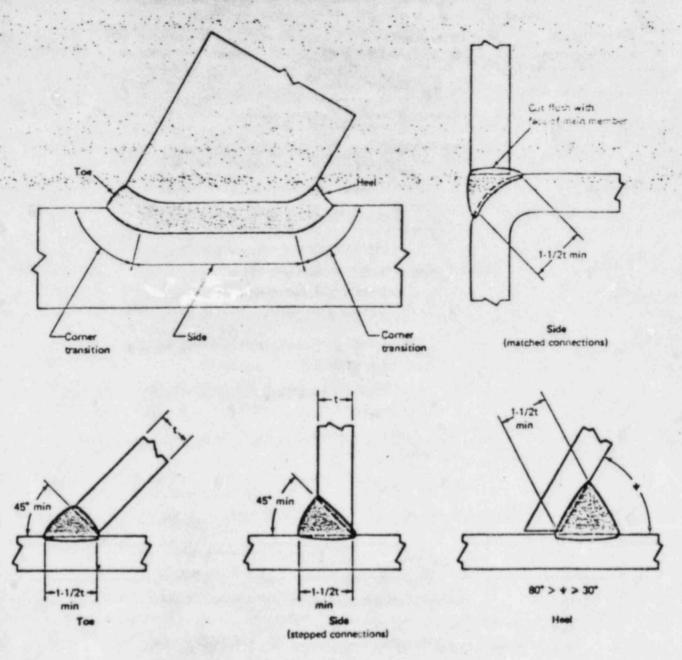
TEXAS UTILITIES GENERATING COMPANY

L. M. Popplewell

Engineering Manager

aku for

GG/jrf



Notes

- 1. t = thickness of thinner section
- 2. Depth of bevel = t
- 3. Root opening 0 to 3/16 in. (4.8 mm)
- 4. Not prequalified for ♥ under 30°
- 5. Effective throat " t
- Joint preparation for corner welds shall provide a smooth transition from one detail to another.
 Welding shall be carried continuously around corners, with corners fully built up and all starts and stops within flat faces.

Fig. 10.13.1 3B—Partial joint penetration prequalified box connections made by shielded metal arc, gas metal arc, or flux cored arc welding

Gibbs & Hill, Inc.

PROJECT FILE

11 Penn Plaza New York, New York 10001 212 760- 4438 Telex:

Domestic: 127636/968694 International: 428813/234475

A Dravo Company

Distribution

W. Williams

S. Bibo

D. Smedley

84042 PK

GTN- 69245

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project Gen. Mgr.

July 12, 1984

CYGNA

JOB NO:

DATE LOGGED:

LOG NO.:

FILE:

CROSS REF. FILE 2.1 mc. CR. Log

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY
COMANCHE PEAK STEAM ELECTRIC STATION
G&H PROJECT NO. 2323
ADDITIONAL RESPONSE TO CYGNA LTR 84042.007 DTD 6/23/84
REF: GTN-69190 DATED JUNE 29, 1984

In Gibbs & Hill's response to CYGNA Energy Services letter of June 23, 1984, via the referenced letter, we indicated that we were continuing to search files for documentation supporting Concern No. 2b of their letter. We are providing the following additional response concerning Management Review Evaluation Reports.

CYGNA's Finding

2b. Management Review Evaluation Reports could not be found for the time period from 1974 through 1976. (1) This requirement has been established since September 1974 in G&H Procedure QA-4...it appears that these activities form an integral part of the G&H corrective action system. (2)

Please determine if documentation exists for.., Management Review Evaluations from 1974 through 1977 (1) as required by G&H Procedure QA-4. (Emphasis Added)

TRANSMITTED BY TELECOPIER

7-13-84

GTN-69245

Gibbs & Hill's Response - Statements (1) and (2) above are somewhat misleading, since they give the impression that:

- No management review was performed during the period 1974 through 1977. There is also inconsistency between the dates mentioned under Statement (1), as reported by CYGNA.
- The Management Review function forms an integral part of Gibbs & Hill's corrective action system. This implied that the corrective action system was not duly performed or completed.

In response to item (1), it is to be noted that the requirement to perform the management review function was included in G&H Procedure QA-4, Rev. 3 dated September 1974. Accordingly, the first round of management reviews was expected to take place in 1975 (i.e., within a year of issuing the procedure). This was done in August 1975. Although we have been unsuccessful, so far, in retrieving this 1975 Report, we can demonstrate that the 1975 management review was indeed performed. By examining the cover sheets and part of the check lists of the 1976 management review reports (see attached copies), it is stated: "Date of previous management review: August 1975. This demonstrates that the 1975 review was indeed performed.

Also, it is to be noted that the checklist used for the management review included a provision to check and verify corrective actions of previous reviews. This was done, as evidenced in the 1976 Report. In other words, any action which was recommended as a result of the 1975 review was verified in the 1976 review. This completes the action and demonstrates that this function was done as required by the G&H Program.

Further, manage ont reviews of successive years (i.e., 1976 and later) were performed and the reports are on file and were presented to CYGNA's representative.

In response to item (2), the statement that "those activities form an integral part of G&H corrective action system", this statement misrepresents the intent of the management review function and discredits G&H's corrective action system. It further implies that G&H did not correct those activities. Since this is not the case, we would point out that correction of deficiencies identified by audits, surveillance or any other means, were dealt with, corrected and verified as part of G&H's audit/surveillance program which was already in place. The

GTN-69245

management review objective was to assess part performance to improve future operation, under the QA program, and strengthen the preventive action measures.

With this response we believe that we have addressed to our satisfaction all concerns raised by CYNGA's letter 84042.007.

If we can provide additional information in this regard, please advise.

Very truly yours,

GNEBS & HILL Inc.

Robert E. Ballard, Jr. Director of Projects

REBa-MSM:1c

1 Letter + 1 Attachment

CC: ARMS (B&R Site) OL + 1A

N. Williams (CYGNA, Calif.) 1L+1A (telecopied)

S. Bibo (CYGNA, MA) 1L+1A D. Wade (TUSI Site) 1L+1A

AUDIT NO. 76-1 DATE - DLY 76,1776

PAGE 1 OF 5 OCT. 29,1976 REV.

DEPARTMENTAL MANAGEMENT AUDIT

DATE OF PREVIOUS AUDIT:	AUGUST 27, 1975
NEXT AUDIT SCHEDULED FOR:	MAY 1977
	EII
PERSONNEL AUDITED AND TITL	E. E. HOROVITZ - SUFY, MECH. ENGINEER
	M. FITTER - Sa. If C ENGINEER
	THERE - DR. THE ENGINEER
	A. SCARIMENTA- LEND MECH LENGTH THE
	LITERALIMANTAL FERDINGSHI DELINITAL

REFERENCE AUTHORITY: GIBBS & HILL QUALITY ASSURANCE MANUAL PROCEDURE QAI-G

AUDIT NO. 76-1 DATE JULY 24, 1976

PAGE 3 OF 5

C. NE	EW ITEMS OR SUGGESTED IMPROVEMENTS (SEE PAGE 2)	A For ADDITION	مرا بم
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D. PR	(Quait chelchat item 3.20).	YES NO	
	WERE DEFICIENCIES OF THE PREVIOUS AUDIT SATISFACTORILY RESOLVED?	<u> </u>	
	REMARKS:		
2.	WAS NECESSARY CORRECTIVE ACTION TAKEN TO PREVENT REPORTED DEFICIENCIES?		
		- ,	
		,	
3.	REMARKS: 5.2 : to F. 1 4.		
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DEPAR	TMENT_	1	Electri	cal	
AUDIT	NO	2	DATE	10/12/76	
PAGE_	1 OF	5			

DEPARTMENTAL MANAGEMENT AUDIT

DATE OF PREVIOUS AUDIT:	1975
NEXT AUDIT SCHEDULED FOR:	1977
PERSONNEL AUDITED AND TITLE:	L.E. O'Brien - Senior Electrical Engineer
	J.A. Walsh - Lead Design Engineer
	Engineers & Designers (Electrical)
	OPPD - Fort Calhoun 2 . Job No. 564

REFERENCE AUTHORITY: GIBBS & HILL QUALITY ASSURANCE MANUAL PROCEDURE QAI-G

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NEW	ITEMS OR SUGGESTED IMPROVEMENTS:	
1.	(3.0B) Procedure QAII-B.7 should be amended to	ensure that a copy of
	the SAR sign-off record is given to the job eng	ineer.
2.	(3.5D) QAII-B.6 para. 6.4(d) should be clarifi	ed to resolve the dif
	ference in interpretations between QA and engin	eering.
3.	OAII-E provides for distribution of specs and a	addenda to all but the
	(c	ont'd on back)
PRE	VIOUS AUDIT STATUS:	YES NO
1.	WERE DEFICIENCIES OF THE PREVIOUS AUDIT SATISFACTORILY RESOLVED?	N/A
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	REMARKS: There were no deficiencies in Audit N	
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2.	REMARKS: There were no deficiencies in Audit N WAS NECESSARY CORRECTIVE ACTION TAKEN TO	o. 1.
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	WAS NECESSARY CORRECTIVE ACTION TAKEN TO PREVENT REPORTED DEFICIENCIES? REMARKS: There were no Corrective Action Reque	o. 1. N/A sts in Audit No. 1.

DEPARTMENT Electrical

AUDIT NO. 2 DATE 10/12/76

DEPAR	TMENT	St	ructur	al		
AUDIT	NO	2	DATE	June	1976	
PAGE	1	OF_	5			

DEPARTMENTAL MANAGEMENT AUDIT

DATE OF PREVIOUS AUDIT:	August 7, 1975
NEXT AUDIT SCHEDULED FOR:	
PERSONNEL AUDITED AND TITLE	Job Engineer I.K. Shah, Senior Engineer -Valdecaballeros
	J.G. Ortiz, Senior Engineer - Ft. Calhoun 2
	C.S. Chen, Senior Engineer - Ft Calhoun 2
	A. M. Kenkre, Senior Engineer -Comanche Peak

REFERENCE AUTHORITY: GIBBS & HILL QUALITY ASSURANCE MANUAL PROCEDURE

DEPARTMENT_Structural
AUDIT NO. 2 DATE June 1976
PAGE 2 OF 5
MANAGEMENT AUDIT REPORT
SUMMARY OF THE AUDIT ANALYSIS:
Valdecaballeros and Ft. Calhoun were fully audited. Comanche Peak was audited as a review of the previous management audit
of August 7, 1975. A. M. Kenkre for Comanche Peak, I. K. Shah
for Valdecaballeros and J. G. Ortiz and C. S. Chen for Ft.
Calhoun 2 were interviewed. Conformance within the Department
to the QA Manual was found satisfactory as regards procedures
and understanding. Valdecaballeros and Ft. Calhoun 2 are in
a stage where all procedures are not yet auditable.

QA PROGRAM REVIEW:

IN VIEW OF THIS AUDIT AND REVIEW OF THE QA PROGRAM AND CORPORATE PROCEDURES:

	<u>x</u>
REMARKS:	
ARE THE EXISTING PROCEDURES ADEQUATE AND IN SUFFICIENT DETAIL TO MEET THE REQUIREMENTS	
OF APPLICABLE REGULATORY GUIDES, CODES AND STANDARDS FOR NUCLEAR SAFETY-RELATED SYSTEMS STRUCTURES AND COMPONENTS	x
OF APPLICABLE REGULATORY GUIDES, CODES AND STANDARDS FOR NUCLEAR SAFETY-RELATED SYSTEMS	<u>x</u>

nostol file

TEXAS UTILITIES GENERATING COMPANY

July 12, 1984

CYGNA Energy Services
101 California Street
San Francisco, Califo

Attention: Ms. Nancy Williams

Project Manager

CYGNA GIEN ROSE TEXAS 76043

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8/6/187/ G. Bjorkman

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102 Welliams

8/9422

COMANCHE PEAK STEAM ELECTRIC STATION CPSES CYGNA REVIEW (PIPE SUPPORTS)

REF: 1) April 19, 1984 letter to N. Williams (CYGNA) from L. Popplewell (TUGCO)

2) July 11, 1984 Telecon between J. Minichiello (CYGNA) and J. C. Finneran (TUGCO)

Dear Ms. Williams:

This letter responds to CYGNA's concerns regarding "bumpers" on supports on the main steam line and on weld calculations for composite sections.

As previously discussed in Reference 1 above (Page 8, Question 3), TUGCO believes that these support configurations are acceptable and we do not agree with CYGNA's assessment of these supports. However, in order to satisfy CYGNA's concerns, we have re-analyzed the stress problems for the pipes with these supports completely removed from the analysis. This evaluation results in no over-stressed piping or supports. Therefore, in the event these supports would behave in an unstable manner (which TUGCO does not believe will happen) and in the event that the bumpers would not perform their intended function, there would be no detrimental effects on these piping systems.

Per Reference 2 regarding the main steam supports with composite sections, the calculation packages for these supports did not consistently include a calculation of the appropriate stresses in the welds between structural members and cover plates for composite sections. However, all the subject welds were acceptable for all stresses. In order to satisfy CYGNA's concerns in this regard, we have reviewed all supports on the 18" feedwater lines and 30" service water lines to determine if composite sections were utilized. We only found one other support on a feedwater line where a composite section was used. The weld stresses in this support were acceptable.

If there are any further questions regarding the above issues, please contact Mr. J. C. Finneran at the site at Extension 521.

CYGNA Energy Services Page 2. July 12, 1984

. . 1 .

Very truly yours,

TEXAS UTILITIES GENERATING COMPANY ENGINEERING DIVISION

L. M. Popplewell
Project Engineering Manager

LMP/JCF/GG/cp

cc: D. H. Wade

J. C. Finneran

Gibbs & Hill, Inc.

PROJECT FILE

11 Penn Plaza New York, New York 10001 212 760-4438 Telex: Domestic 127636/968694 International: 428813/234475

A Dravo Company

Distribution J. Minchello N. Williams

GTN-69250

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project Gen. Mgr.

July 13, 1984 YGNA JOB NO : DATE LOGGED : LOG NO . : FILE: CROSS REF. FILE

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY COMANCHE PEAK STEAM ELECTRIC STATION G&H PROJECT NO. 2323 FOLLOWUP INFORMATION FROM G&H REF 1: CYGNA COMMUNICATIONS REPORT DTD 7/2/84 REF 2: GTN-69233 DTD 7/10/84

By copy of this letter to Nancy Williams of CYGNA, attached please find a response (to supplement that given in ref. 2) to question 1 of the referenced CYGNA Communications Report.

Should you have any questions, please contact either Steve Lim or Henry Mentel.

Very truly yours,

GIBBS & HILL, Inc.

REBa-HWMe: 1c

1 Letter + 1 Attachment

ARMS (B&R Site) OL + 1A CC:

Robert E. Ballard, Jr. Director of Projects

N. Williams (CYGNA, Calif.) 1L 1A (telecopied)

G. Grace (TUSI Site) 1L 1A

D. Wade (TUSI Site) 1L

L. Weingart (CYGNA, Calif.) 1L 1A

SUPPLEMENTARY RESPONSE TO ITEM 1 OF CYGNA TELECOPIED QUESTIONS ON JULY 2, 1984

In reference to the minor differences in snubber loads reported in the computer printouts and the calculation book, the analyst in his or her judgment deemed the load changes to be small and as such would have no impact on the support designs. As a consequence, the calculation book was not updated to reflect these new loads and these minor load changes were not reported in the pipe support vendor certification.

11 Penn Piaza New York, New York 10001 212 760- 4438 Domestic: 127636/968694 International: 428813/234475 A Dravo Company

PROJECT FILE

GTN-69249

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project Gen. Mgr.

JOB NO : DATE LOGGED: LOG NO .: FILE: CROSS REF FILE

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY COMANCHE PEAK STEAM ELECTRIC STATION G&H PROJECT NO. 2323 PIPE STRESS REVIEW - MASS PARTICIPATION REF 1: CYGNA LETTER 84042.008 DTD 6/24/84 REF 2: GTN-69176 DTD 6/29/84 -MASS PARTICIPATION FRACTION SENSITIVITY STUDY

By copy of this letter to Mr. Leo Colborne of CYGNA Energy Services, attached is one (1) magnetic tape of the input files for those problems selected per reference 2. The tape has been prepared according to the format specified in reference 1.

CYGNA will verify that this tape is readible and will contact Gibbs & Hill regarding G&H personnel traveling to their Boston office. It is presently anticipated that Henry W. Mentel and Steve Lim will be making that trip. Henry Mentel should be contacted regarding travel plans and if there are any questions.

Very truly yours,

HAMME REBa-HWMe:1c 1 Letter

CC: ARMS (B&R Site) OL

G. Bjorkman (CYGNA Boston) 1L

N. Williams (CYGNA Calif.) 1L

Dravo D. Wade (TUSI Site) 1L G. Grace (TUSI Site) 1L

GIBBS & HILL,

Robert E. Ballard, Jr. Director of Projects

L. Colborne (CYGNA Boston) 1L + Tape T12779

11 Penn Plaza New York. New York 10001 212 760- 4438 Telex 12 CAYONA Domestic ! Internation A Dravo CompanyNO : DATE LOGGED: July 27, 1984 LOG NO. : CROSS REF. FILE Texas Utilities Generating Company

NOTED AUG 06 1984 MINILIAMS

Distribution

N. Williams

D. Smedly

S. Bibo

J. Minuhillo

M. Shulman

84042 PF

Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project Gen. Mgr.

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY COMANCHE PEAK STEAM ELECTRIC STATION G&H PROJECT NO. 2323 CYGNA IAP PHASE 3 REPORT

Gibbs & Hill has performed an overall review of the Phase 3 Report with particular focus on the Results and Conclusions Section and with an eye towards established action plans for problem resolution. In view of this we offer the following comments:

Section 5 - Results & Conclusions -Page 5-10, Last Paragraph

Regarding NCR's - Gibbs & Hill does not review NCR's on a routine basis; only when presented for our review via formal correspondence or as part of a DCA/CMC request.

Section 3.3 - Develop Checklists -Pages 3-2, 3-3

Establishes the checklist identifiers, i.e., PI-mm where mm = 05 to correspond to Stress Problem 1-023A. In Appendix 1, the identifiers have been used as the stress problem numbers in several cases.

> THURST IN THE STATE OF 7-27-84

Gibbs & Hill, Inc.

GTN-69296

. .

-2- July 27, 1984

Example: Checklist No. PI-05

Problem No. AB-1-005; Rev. 1

AB-1-005 - Should be AB-1-023A.

The problem numbers referred to on the pipe stress checklists should be reviewed and corrected accordingly.

Please advise of any questions.

Very truly yours,

GIBBS & HILL, Inc.

Robert E. Ballard, Jr. Director of Projects

REBa-SMMa:lc l Letter

CC: ARMS (B&R Site) OL

N. Williams (CYGNA CA) 1L

D. Wade/G. Grace (TUGCo Site) 1L (telecopied)

11 Penn Plaza New York, New York 10001 NOTED AUG 0 6 1984 RECEIVE 212 760-CYGNA Tolox Domestic: 127636/9 International: 42881 334 No : A Dravo Company DATE LOGGED: LOG NO .: FILE: GTN- 69303 31, 1984 CROSS REF. FILE Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project Gen. Mgr.

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY COMANCHE PEAK STEAM ELECTRIC STATION G&H PROJECT NO. 2323
TAPERED TRANSITION JOINT SIF REF: GTT-10424, DATED 7/11/84

Subsequent to the G&H intentions outlined in referenced GTT, G&H has completed Part A of the plan of action pertaining to the Stress Intensification Factor (SIF) for tapered transition joints at equipment nozzle connections. The results are as follows:

- 1. Problem AB-1-61C, Rev. 0 CYGNA's sample considered node points 378, 420 and 439. The G&H node points where stress intensification factors (SIF's) were input are 377, 419 and 438. Node 377 represents the end of a flange and contains an SIF of 1.9. Nodes 419 and 438 represent the end of an elbow to flange connection and contain SIF's of 3.5 and 4.271 respectively. Therefore, the analysis contains the appropriate stress intensification factors at the equipment nozzle connections.
- 2. Problem AB-1-151B, Rev. U CYGNA's sample considered node point 1 which represents an embedded portion of pipe and not the piping to equipment nozzle connection for the spent fuel pool cooling water pump. Node point 1 does not require an SIF. The node point at the equipment nozzle connection is

node point 83 which represents the end of a flange and contains an SIF of 1.9. Node 1 is the node point of the discharge nozzle connection for adjacent problem AB-1-151A. Therefore, the analysis contains the appropriate stress intensification factor at the equipment nozzle connection.

- Problem AB-1-57, Rev. 0 CYGNA's sample considered node points 76 and 116 which are at the equipment nozzle connections to the reactor coolant drain tank heat exchanger and the excess letdown heat exchanger respectively. Due to the weld configuration, a stress intensification factor at these locations is not required. Westinghouse equipment drawings 501B572 and 501B574 indicate that these 4-inch nozzle connections utilize a butt weld end prep configuration. G&H Specification, 2323-MS-43B, requires that "all sharp changes in sections of any weld shall be eliminated" which results in a flush weld between the equipment nozzle and the adjoining pipe. Figure NC-3673.2(b)-1 of Subsection NC to Section III of the ASME Code states that for a butt-weld that has been reworked flush, the SIF is equal to 1.0. Thus, the stresses are not intensified at these equipment nozzle connections; ustification for not considering an SIF at the said locations is therefore provided.
- 4. Problem AB-1-167-B, Rev. CYGNA's sample considered node point 204 which is the piping connection to a flexible connector. This connector is made out of neoprene and utilizes stainless steel clamp assemblies to connect the adjacent piping. An SIF at this connection is therefore not required.
- 5. Problem AB-1-40, Rev. 0 CYGNA's sample considered node point 34 which is the equipment nozzle connection to the regenerative heat exchanger. Due to the nozzle weld end configuration shown on Atlas Industrial Manufacturing equipment drawing D-4313-7, a stress intensification factor at this location should have been considered. However, the nozzle weld end configuration can be considered as a tapered transition and when the associated SIF of 1.9 maximum (per Figure NC-3673.2(b)-1 of Subsection NC to Section III of the ASME Code) is applied at this location, the stress results are still within the Code allowables and are as follows:

GTN- 69303

```
Eq. 8 = 7184 psi 17,200 psi

Eq. 9 (upset) = 8141 psi 20,640 psi

Eq. 9 (emergency) = 8786 psi 30,960 psi

Eq. 10 = 5185 psi 27,800 psi

Eq. 11 = 12369 psi 45,000 psi
```

Omission of the SIF at the equipment nozzle connection does not adversely affect the analysis.

6. Problem AB-1-150G, Rev. 0 - CYGNA's sample considered node points 1 and 17. Node point 1 is at the connection to the thermal regeneration demineralizer and node point 17 is in the vicinity of the resin fill opening. Due to the nozzle weld end configuration shown on Westinghouse drawing 271C900 and the flange arrangement shown on drawing FSM00143, a stress intensification factor at these locations should have been considered. However, when an SIF of 1.9 maximum is applied at these locations, the stress results are still within the Code allowables and are as follows:

Node Point 1

Eq.	8	=	2047	psi	4	17,200	psi
Eq.	9 (upset)			-		20,640	-
Eq.	9 (emergency)					30,960	
Eq.	10					27,800	
Eq.	11					45,000	

Node Point 17

```
Eq. 8 = 1215 psi \( \) 17,200 psi 

Eq. 9 (upset) = 1215 psi \( \) 20,640 psi 

Eq. 9 (emergency) = 1215 psi \( \) 30,960 psi 

Eq. 10 = 0 psi \( \) 27,800 psi 

Eq. 11 = 1215 psi \( \) 45,000 psi
```

Omission of the SIF's at the said connections does not adversely affect the analysis.

In conclusion, G&H agrees that three (3) of the ten connections do not contain a stress intensification factor. The analyses involved are AB-1-40, Rev. 0 (node point 34) and AB-1-150G, Rev. 0 (node points 1 and 17). Based upon our findings, G&H will

GTN-69303

July 31, 1984

perform Part B of the plan of action outlined in the reference GTT. In order to complete this review in a timely manner and since it involves considering a formidable amount of analyses (approximately 272) the following methodology will be followed:

- Determine which analyses contain equipment nozzle connections.
- Determine if a stress intensification factor was or was not considered at the connection.
- 3. If the analysis contains equipment but no SIF was considered, the stress results at the applicable node point will be multiplied by the appropriate maximum SIF.
- 4. If the resulting intensified stresses remain within the allowable limits, the analysis will remain acceptable.
- If the resulting intensified stresses exceed the allowable limits, the piping to nozzle mismatch will be considered to arrive at a decreased SIF.
- 6. If the stresses still exceed the allowable limits, equipment and weld end prep detail drawings will be reviewed to obtain possible relief.

Results of the above plan of action should be available by August 17, 1984.

If you have any questions, please contact H. W. Mentel (x6302) or F. A. Colucci (x5203).

Very truly yours,

GIBBS & HILL, Inc.

& In marao

R. E. Ballard, Jr. Director of Projects

REBa-HWM/FAC:1c 1 Letter

CC: ARMS (B&R Site) OL

D. H. Wade (TUSI Site) 1L
N. Williams (CYGNA CA) 1L

G. Grace (TUSI Site) 11.

PROJECT, FILE

Gibbs & Hill, Inc.

11 Penn Plaza New York, New York 10001 212 760- 4 4 3 8 Telex: Domestic: 127636/968694

International: 428813/234475

A Dravo Company

CYGNA

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August 3, 1984

GTN-69316

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project Gen. Mgr.

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY
COMANCHE PEAK STEAM ELECTRIC STATION
G&H PROJECT NO. 2323
REVISED MASS PARTICIPATION
FRACTION SENSITIVITY STUDY

REF 1: GTN-69162 DTD 6/26/84 REF 2: GTN-69279 DTD 7/20/84 REF 3: GTN-69176 DTD 6/29/84

Attached please find the revised plan of action regarding Mass Participation along with a listing relating functional schedule milestone dates. Should you have any questions, call this office.

Very truly yours,

GIBBS & HILL, Inc.

of the manaur

Robert E. Ballard, Jr. Director of Projects

Approved:

M.A. Vivirito

Vice President Power Engineering

REBa-HWMe:1c

G. Bjorkman (CYGNA MA) 1L 1A

l Letter CC: ARMS (B&R Site) OL N. Williams (CYGNA CA) 1L 1A D. Westbrook (TUSI Site) 1L 1A

_ D_ Wade (TUSI Dallas) 1L 1A (telecopy)

Drays Westbrook (TUSI Site) IL la (telecopy) NASSAL TED BY TELECOPITA

REVISED MASS PARTICIPATION FRACTION SENSITIVITY STUDY

GTN-69162 dated June 26, 1984 established the Gibbs & Hill plan of action for the Mass Participation Fraction Sensitivity Study. Item 3 in that GTN stated that upon completion of the re-analysis of the five (5) selected problems a preliminary report would be issued and an assessment made with regard to additional required work. The status report issued under GTN-69279 dated July 20, 1984 generalized as to the type of additional work or followup actions required. Those followup actions were:

- a. Need for an expanded review (more problems)
- Further re-analysis of the five (5) selected problems (with refinements to reduce loads)
- c. Submittal of revised loads to PSE to check support adequacy.

Based upon the preliminary information gathered to date, Gibbs & Hill has deemed it appropriate to expand the review of the piping analyses. In essence the original plan of action is now revised with changes being made in review criteria and in the scope (number of problems) of the Study. The revised plan of action for the Study is as follows:

- 1. The scope of the Study will be expanded beyond the originally selected five (5) problems. A representative sampling will be made of those problems which are considered to contain the extreme parameters in regards to high frequency response. In GTN-69176 dated June 29, 1934, Gibbs & hill presented Charts 1, 2 & 3 nighlighting the mass particiation percentages calculated for the 'x', 'y' and 'z' directions for the 200 stress problems in its initial survey. A review of these Charts show that 18 problems exhibited an 'x' mass fraction under 10 percent, 28 problems a 'y' mass fraction under 10 percent; four (4) problems a 'z' mass fraction under 10 percent. An initial representative sample will be drawn from these worst case percentages with the following additional parameters being considered:
 - a. The first natural frequency and number of modes considered in the original as-built analysis
 - b. The pipe size, schedule and weight
 - c. The number of anchors and pipe supports
 - d. Concentrated weights in the analysis, i.e., valves
 - c. The building(s) to which the piping is supported
 - f. The Response Spectra utilized (refined versus unrefined)

g. Seismic anchor movements.

A selection of the actual problems to be analyzed considering the above parameters is being prepared and will be submitted on August 8, 1984.

2. The problems in the representative sample will be re-analyzed utilizing the ADLPIPE computer program version C (consistent with the version used in the original as-built analysis). Introduced into this re-analysis will be a static seismic analysis based upon the respective 'x', 'y' and 'z' Zero Period Acceleration (ZPA) associated with the stress problem. The loads obtained in this manner will be compared with those originally derived in the dynamic analysis performed during the as-built program (which utilized a frequency cutoff of 33 Hertz). The two (2) sets of loads (original dynamic/ZPA) will be compared and the higher values used to check the support designs.

The above method outlined is currently an acceptable industry method of checking the adequacy of the piping system and its support designs in regard to high frequency responses. In addition in a telephone conversation with Nancy Williams of CYGNA Fnergy Services placed on Tuesday, July 31, 1984, Ms. Williams concurred that this type of check of the effect of ZPA was the method used by CYGNA's reviewer during the independent audits, and is acceptable to CYGNA.

- a 10 percent acceptance criteria. If the new total support load is within 10 percent of the original no further work will be performed. For those cases above a 10 percent increase in load Gibbs & Hill will make refinements to the analysis. Such refinements will consist of:
 - Use of refined response spectra, if not originally utilized
 - b. Use of a more specific ZPA (for the factoring of resultant loads). Presently the acceleration associated with the cutoff frequency of 33 Hertz is being conservatively utilized as the ZPA. There can be a reduction in ZPA at higher frequencies. Besides a more accurate ZPA, credit will be taken of the relative location of the piping system supports being analyzed with respect to the supporting building(s) to lower any undo conservatism inherent in the enveloped Response Spectra curve.
 - c. Use of refined seismic anchor movements.

4. With the refined re-analysis a check will be made of the support design load margin for those supports still having a greater than 10 percent total load increase.

Gibbs & Hill is optimistic that utilizing the above step by step evaluation of the ZPA effect on the worst case representative sample will dispense with the mass participation issue. Following is a simplified summary of the recommended actions.

For each of the problems contained in the worst case representative sample (mass fractions less than 10 percent):

- Step 1 Analyze the ZPA effect
- Step 2 Compare the resulting loads with those of the original dynamic analysis (as-built)
- a. If the original support design loads are higher, no further evaluation is required
- b. If the ZPA associated loads result in a total design load increase of less than 10 percent, no further evaluation is required
- c. If the ZPA associated loads constitute a greater than 10 percent increase in total loads proceed to Step 3.
- Step 3 Perform a refined analysis of the ZPA effect (more specific ZPA and SAM)
- Step 4 Same as Step 2, a & b -- no further analysis; if c -- proceed to Step 5
- Step 5 Evaluation of the design load margin in the supports.

Depending on the outcome of the results of the above analyses, additional sampling analysis may be required.

MASS PARTICIPATION SENSITIVITY STUDY - FUNCTIONAL SCHEDULE MILESTONES

- Problem Selection/Data for Refinement
 (note 30 problems) August 3, 1984 August 8, 1984
- Reanalysis Incorporating ZPA Effect for comparison
 (first 15 problems) August 8, 1984 August 15, 1984
- Reanalysis Incorporating ZPA Effect for Comparison
 (second 15 problems) August 15, 1984 August 22, 1984
- 4. Refined Reanalysis as Required
 August 22, 1984 August 30, 1984
- 5. Report to PSE Supports with Load Increase to Check Margin
 August 17, 1984 - August 31, 1984

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TEXAS UTILITIES SERVICES INC.

OFFICE MEMORANDUM

To J. B. Seorge			Gleo, Pose, Texas	Sept. 6, 1983
Subject	COMANCHE !	PEAK STEAM ELECT	RIC STATION	- Store
	CPSES I	DOCUMENT CONTROL	DATE LOGGED:	8/7/8V # 72
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The following is submitted in response to your request regarding the historical path and current status of the project document control program.

In May of 1982, project management directed the re-evaluation of the CPSES document control system in an effort to strengthen the system and improve its overall efficiency. The first step in this process was a realignment of supervision with an initial charge to evaluate DCC in terms of efficiency with respect to the total control and distribution process.

The immediate results of this evaluation were to increase efficiency by the proper organization of DCC manpower. These included establishing priorities, specific task sequences, and job descriptions. In parallel, equipment inventories were re-evaluated considering capability, cost effectiveness, operator training, and maintenance (including history and contract commitments). The results, when implemented in conjunction with new equipment purchases, enabled DCC to recover some document control functions previously managed outside DCC, and effect current file retrievability, reproduction, and distribution.

The second phase of the evaluation consisted of an integrated review of the total distribution process. It was clearly recognized that centralizing drawing and design change control would strengthen the system and provide a more positive means of control. In late 1982, the concept of managing these controls by a limited number of DCC-managed satellites was originally proposed.

The "satellite concept" -- although in preliminary outline form -- was observed during the CAT investigation by NRC personnel. Although the DCC effort was not found by CAT to be deficient, the team commented the satellite approach would minimize the "general risk" inherent to the existing program and simultaneously enhance positive control.

The first satellite was implemented in May of 1983 to support the Startup and Startup Support Groups. Full implementation of the satellite program was accomplished August 1, 1983, with the operation of five (5) total satellites supporting CPSES.

J. B. George Sept. 6, 1983 Page 2

During the implementation phase of the satellite system, CYGNA began their Independent Assessment Program at Comanche Peak. Several observations were noted indicating inconsistencies in DCC's design change records. The inconsistencies resulted, in part, from start-up difficulties of the satellite system and from human error. Additional confusion was created because of the Tack of understanding initially, by CYGNA, of the function of the Design Change Tracking Group (DCTG).

The DCTG is the engineering group charged with maintenance of the CMC/DCA Master Index used for tracking/statusing the engineering/design review of design changes. This group is also the primary Engineering-to-DCC interface.

The original CMC/DCA Master Index was maintained by Gibbs & Hill to track their internal design review effort. Because of the manner in which this index had been maintained, a comparison of applicable design changes in that document would appear, at face value, to be discrepant with DCC's manual design change logs. The DCTG is currently reviewing each design change for completeness and accuracy with regards to the status of design changes and the proper drawing references. When this purging effort is completed, a computer data base will exist such that DCC's manual design change tracking system may be eliminated. The merging of the two systems is scheduled to occur on October 15, 1983, and will eliminate all discrepancies from the past.

Until such time as the merger is made, DCC's manual design change tracking system remains as the controlling mechanism for design changes. In order to assure that identified discrepancies are corrected and that positive controls are in place, an independent monitoring team which reports directly to DCC Management, has been established. This team constantly rotates within the DCC system assessing and "auditing" distribution control. The team's scope includes each drawing, specification, procedure, and associated changes entered in the control system. These personnel have been delegated no production responsibility except auditing.

The above actions, all combined, will ultimately result in a strengthened DCC system and will provide the positive controls that are necessary.

sst. Project General Manager

JTM: pew

Rec. 8/27/84

Gibbs & Hill. Inc.

11 Penn Plaza New York, New York 10001 212 760- 4438 Telex: Domestic: 127636/968694 International: 428813/234475 A Dravo Company

N. Williams wo/a L. Weingart w/a J. Minichello wo/a

August 23, 1984

JOB SO

LOG NO .:

FILE:

DATE LOGGED!

GTN-69369

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J.B. George

Vice President/Project Gen. Mgr.

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY COMANCHE PEAK STEAM ELECTRIC STATION G&H PROJECT NO. 2323 TRANSITION JOINT SIF AT EQUIPMENT NOZZLE CONNECTIONS REF: GTN-69359 DTD 8/17/84

By copy of this letter to Nancy Williams of CYGNA, attached is a copy of the Gibbs & Hill Calculation 2323-EQ-SIF referred to in the above reference. As can be seen all nozzles are acceptable. In those instances where a calculation check was performed it should be noted that the taper transition SIF of 1.9 was applied across the board without account made for the pressure term or for the .75 factor in equations 8, 9 and 11. This across the board application was done for expediency and due to the low magnitude of stresses found in the majority of cases. In several instances this approach was not applicable hence the pressure and .75 factor was accounted for.

August 23, 1984

Gibbs & Hill feels that this calculation more than adequately completes the plan of action on this item. Note that not included with this transmittal is the calculation attachments to 2323-EQ-SIF which are copies of the related ADLPIPE analyses microfilm. These can be made available upon request.

Very truly yours,

GIBBS & HILL, Inc.

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Robert E. Ballard, Jr. Director of Projects

REBa-HWMe:lc 1 Letter

CC: ARMS (B&R Site) OL

N. Williams (CYGNA, CA) 1L 1A

D. Wade (TUSI Site) 1L G. Grace (TUSI Site) 1L

Calculation Cover Sheet

G&H Job No. 2323-046 Client TUS!

Calculation Number 2323-EQ-SIF

Number of Sheets in Original Issue 82

Subject Tapered Transition Joint S.I.F. At Equipment

- Nuclear Safety Related
- □ Non-Nuclear Safety Related—QA Program Applicable
- □ Non-Nuclear Safety Related

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Subject Tapered Transition Joint S.I.F. At Equipment

Sheet No. 1 Calculation Number 2323- EQ-SIF

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Purpose: The purpose of this calculation is to verify that the stresses at equipment nozzle connections satisfy the requirements of the ASME Code allowables (see Messages 2)

Table of contents	Sheet No.
1) References	2
2) Information Matrix	3
3) Analysis Results	II .
4) Conclusion	82
5) Calculation Attachments	

Subject Topered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - Ea - 51F

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Sheet No. 2

References :

- 1) ASME Code Section III 1974
- 2) GTT-10424 DATED 7/11/84 "PLAN OF ACTION
 ADDRESSING EQUIPMENT NOTTLE CONNECTIONS AND
 THE CONSIDERATION OF A SPRESS INTENSIFICATION
 FACTOR FOR A PAREFED TRANSITION JOINT,"
 - 3) GTN 69303 OATED 7/31/84 PART " OF
 PIERINGATIONED PLAN ABOVE
 - 4) 67N-69338 DATED 8/10/84 STATUS PEPOAT

Subject Topered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ-SIF

Sheet No. 3

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Information Matrix

No.	Prob.	Equipment	S.T.F. Accounted For IN ORIGINAL AMALYSIS	S.I.F. Effect Accounted For	Comments
1	1-1	Yes	Yes	MIA	H/A
2	1-2	Yes	Yes	N/A	N/A
3	1-3	Yes	Yes .	N/A	N/A
4	1-4	Yes	Yes	NIA	N/A
5	1-5	Yes	Yes	NIA	N/A
6	1-6	Yes	Yes	N/A	N/A
	1-7	Yes	Yes	NIA	N/A
7 8	1-8	Yes	Yes	N/A	N/A
9	1-9A	No	N/A	NIA	N/A
10	1-98	No	NIA	NIA	N/A
	1-90	No	NIA	NIA	N/A
11	1-90	No	NIA	NIA	N/A
- 15	1-10A-1		Yes	NIA	N/A
13	1-106		N/A	NIA	N/A
14	1-10 C		NIA	N/A	N/A
15	1-100-1	\ \/ -	No	Yes	See Calc. Page 11
- 16	1-11A	Yes	No	Yes	See Colc. Poge 12
17		Yes	No	Yes	See Calc. Page 13
18	1-11B	Yes	Yes	N/A	N/A
19	1-110		Yes	NIA	N/A
50	1-12A-		NA	N/A	N/A
51	1-12B	70	NIA	NIA	N/A
55	1-150			NIA	N/A
53	1-15 E		N/A Yes	NIA	N/A
- 24	1-19A		Yes	NIA	N/A
52	1-198			NIA	NIA
56	1-190	1 1/ -	N/A	NIA	N/A
27	1-51-		Yes		N/A
- 58	1-234	NO	NA	NA	N/A
- 29	1-236	No No	NA	AN	N/A
- 30	1-530	No .	N/A	N/A	N/A
- 31	1-830		N'IA	1 77	NIA
32	1-24	No	N/A	Yes	See Calc. Poge 14
- 33	1-27-		No.	Yes	See calc. Page
- 34	1-28-		No		N/A
35	1-29 K		N/A	N/A	
36	1-291		N/A	N/A	N/A
37	1-29 M	1 NO	N/A	I N/A	F-166, 7-8

Subject Topored Transition Joint S. I. F. At Equipment

Calculation Number 2323- EQ- SIF

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No.	Prob.	Equipment	S.I.F. Accounted For IN ORIGINAL ANAITSIS	S.I.F. Effect Accounted For	Comments
38	1-29N-1	NO	NIA	N/A	N/A
39	1-290	No	N/A	MIA	N/A
40	1-296	No	N'/A	NIA	N/A
41	1-295	NO	N/A	A/A	N/A
92	1-29T	No	NIA	N/A	H/A
43	1-294	No	NIA	NIA	NIA
44	1-29V	No	NA	N/A	N/A
45	1-29W	No	N/A	N/A.	N/A
46	1-29X	No	NIA	N'A.	N/A
47	1-29Y	No	N/A	17/7	N/A
48	1-297	No	I N/A	NIA	I A Same
- 49	1-30-1	Yes	No	Yes	See Calc Page 20
- 50	1-31	Yes	No	Yes	See Calc. Foge 21
51	1-32	Yes	Yes	NIA	N/A
52	1-33	Yes	Yes	NA	N/A
- 53	1-34A	Yes	No	Yes	See caic. Foge 22
54	1-34B	Yes	Yes	N/A	N/A
- 55	1-34C	Yes	No	Yes	See caic. Page 23
36	1-35A	No	N/A	N/A	N/A
57	1-358-1	No	N/A	N/A	N/A
58	1-35C	No	NIA	MIA	N/A
59	1-35D	No	N/A	N/A	N/A
60	1-35E	No	NIA	N/A	~/A
61	1-35F	No	N/A	N/A	N/A
62	1-36	No	N/A	N/A	N/A
63	1-37B	No	NIA	A/H	N/A
64	1-37W-1	No	N/A	M/A	N/A
65	1-37 X	No	N/A	N/A	N/A
66	1-37Y	No	N/A	N/A	N/A
67	1-377	No.	N/A	NIA	N/A
68	1-40	Yes	No	Yes	See GTN 69303, 7/31/84
- 69	1-42A	Yes	No .	Yes	See Calc. Page 24
70	1-A2B	No	NIA	H/A	N/A
- 71	1-45Q	Yes	Yes	N/A	N/A
72	1-45R	Yes	Yes .	N/A	N/A
73	11-455	Yes .	Yes	NIA	N/A
74	1-45T	Yes	Yes	N/A	N/A
	king Metho	d# 1 Line by the the	olong ulason Results compared		F-166, 7-82

Subject Topered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ-SIE Sheet No. 5

Revision	Organi	Date	Rev.	Dete	Rev.	Dete	Rev.	Deta	Rev.	Date
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Checker	nin	6/21/84								

No.	Prob.	Equipment	S.I.F. Accounted For IN ORIGINAL ANALYSIS	S.I.F. Effect Accounted For	Comments
75	1-ACA	Yes	Yes	N/A	N/A
-76	1-468	Yes	No	Yes	See Caic. Page 25
- 77	1-478	Yes	No	Yes	See Calc. Page 26
78	1-51A	Yes	Yes	AIM	N/A
79	2-51A	Yes	Yes	N/A	N/A
80	1-51C-1	No	N/A	N/A	NIA
19	1-51D	Yes	Yes	N/A	N/A
29	2-510-1	Yes	Yes	N/A	MA
23	2-526	Yes	Yes	NIA	NIA
- 84	1-524	Yes	No	Yes	See Chia Page 27
25	2-524	Yes	Yes	N/A	NJA
66	1-52V-1		Yes	N/A	N/A
87	1-52W	Yes	Yes	NIA	N/A
99	1-52 X	Yes	Yes	N/A	N/A
69	1-52Y	No	NIA	NIA	N/A
90	1-522	No	NIA	NIA	N/A
91	1-55A	Yes	Yes	NIA	N/A
92	1-550	Yes	Yes	NIA	N/A
93	1-55C		Yes	N/A	N/A
94	1-550-1		Yes	N/A	N/A
95	1-57	Yes	N/A	N/A	See GTH 69303, 7/31/84
-96	1-58	Yes	No	Yes	See Calc. Page 28
97	1-59A	Yes	Yes	N/A	N/A
98	1-59B		Yes	N/A	N/A
99	1-59C		Yes	NIA	N/A
- 100	1-590	Yes	No	Yes	See Calc. Poge 30
-101	1-60	Yes	No	Yes	See Calc. Poge 32
102	1-61A-2		No	Yes	See Calc. Page 37
103	1-61B-1		Yes	N/A	A/A
104	2-618	1	Yes	N/A	N/A
105	1-616		Yes	N/A	SEE GTN 69303, 7/31/84
- 106	1-610	Yes	No	Yes	See Calc. Poge 39
107	1-61E	Yes	Yes	N/A	N/A
108	2-61E-1	1	Yes	N/A	N/A
109	1-61F	No	N/A .	N/A	N/A
110	1-62A	Yes	Yes	NIA	N/A
111	1-628-		Yes	NIA	N/A
	king Metho		cking uislion Results compared		F-166, 7-82

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ-SIF-

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	RUV.	Date
Chactery Metros	1			><		><		$>\!<$		><
Preparar -	-tac	8 11 84								1
Checker	Hom.	8/27/84								-

NO.	Prob.	Equipment	S.I.F. Accounted For IN ORIFINAL ANALYSIS	S.I.F. Effect Accounted For	comments
112	1-62C-1	Yes	Yes	N/A	N/A
113	1-620-1	Yes	Yes	N/A	N/A
114	2-620	Yes	No	Yes	See Calc. Page 40
115	1-62E	Yes	Yes	N/A	N/A
116	1-62F	Yes	No	Yes	See Calc. Poge 41
117	1-626	No	N/A	N/A	N/A
118	1-62 X-1	Yes	Yes	N/A	N/A
119	1-62Y	No	NIA	MIA	N/A
120	1-622-1	Yes	Yes	NIA	N/A
121	1-63A	Yes	Yes	N/A	N/A
-122	1-638	Yes	No	Yes	See Cale. Page 42
123	2-638	No	NIA	N/A	combined with AB-1-638
124	1-63C	No	N/A	N/A	
125	1-63D	Yes	Yes	N/A	N/A
126	1-64A	Yes	No	Yes	See Calc. Page 43
127	1-64B	No	N/A	N/A	N/A
128	1-64C	No	N/A	N/A	N/A
129	1-640	No	N/A	N/A	N/A
- 130	1-64E	No	MIA	N/A	N/A
131	1-64F	Yes	No	Yes	See Calc. Poge 44
132	1-65	Yes	Yes	NIA	N/A
133	1-66A-1	Yes	Yes	N/A	NIA
134	1-66 B-1		Yes	N/A	N/A
135	1-66C	Yes	Yes	NA	N/A
136	1-67T	No	N/A	I N/A	N/A
137	2-67T	No	N/A	A/A	A/A
- 138	1-67U	No	N/A	NIA	N/A
139	1-67V	Yes	Yes	NIA	N/A
140	1-67×	No	N/A	A/A	N/A
141	2-67X	No .	N/A	N/A	N/A
142	1-67Y	No	N/A	AIM	N/A
143	1-672	Yes	Yes	N/A	N/A
144	1-68T	No	N/A	N/A	N/A
- 145	2-68T	No	N/A	N/A	H/A
146	1-68 U-		N/A	NIA	N/A
147	1-68V-1		Yes .	NIA	N/A
148	1-68×	No.	N/A	N/A	N/A
	king Metho		acting suitation Results compared		F-166, 7-82

Subject Topered Transition Joint S. I. F. At Equipment

Calculation Number 2323- EQ-SIF

Revision	Circana	Date	Rev.	Date	Rov.	Date	Flev.	Date	Rev.	Date
Chactery Marros	1	><		><		><		><		><
Preparer.	-fac	8/11/84								
Checker	GIAM	4/25/64								

No.	Prob.	Equipment	S.I.F. Accounted For IN ORIGINAL MAYSIS	S.I.F. Effect Accounted For	Comments
149	2-68×	No	N/A	. N/A	N/A
150	1-68Y	No	N/A	N/A	N/A
151	1-682	Yes	Yes	N/A	N/A
152	1-69	Yes	Yes	.N/A	N/A
153	1-70	Yes	Yes	N/A	M/A
154	1-71A	Yes	Yes	N/A	N/A
155	1-718	Yes	Yes	N/A	N/A
156	1-72	NO	N/A	N/A	N/A
157	1-73	No	N/A	N/A	N/A
158	1-74	No	N/A	N/A	N/A
159	1-75	Yes	Yes	N/A	N/A
- 160	1-76A	Yes	No	Yes	See Calc. Poge 46
161	1-76B	No	NA	N/A	N/A
162	1-77	Yes	Yes	N/A	N/A
163	1-78	Yes	Yes	N/A	N/A
164	1-79A	No	A/A	N/A	N/A
165	1-798	No	N/A	N/A	N/A
166	1-79C	No	N/A	N/A	N/A
167	1-79 D	No	N/A	N/A	N/A
168	1-79E	No	N/A	N/A	N/A
169	1-79F	Yes	Yes	N/A	N/A
170	1-80A	No	N/A	N/A	N/A
- 171	1-80B	Yes	No	Yes	See Colc. Poge 48
זרו	1-80C	No	N/A	H/A	N/A
173	1-80D	No	N/A	NIA	N/A
174	1-81	Yes	Yes	N/A	N/A
175	1-86A	Yes	Yes	N/A	N/A
176	1-86B	No	N/A	N/A	N/A
177	1-86C	No	N/A	N/A	N/A
- 178	1-87A	Yes	Yes	A/A	N/A
179	1-878	No	N/A	N/A	N/A
180	1-87C-1	No	N/A	N/A	A/M
181	1-88C	Yes	No	Yes	See Cald. Page 49
182	1-88D	Yes	Yes	N/A	NA
183	1-88E	Yes	No	Yes	See Calc. Page 50
184	1-88W	No	N/A	N/A	N/A
	1-88×	No	N/A	I N/A	N/A
Check	ing Metho	3 Identical Calcula	clang Lieson Results compared ston Results compared and results of computer with comsepon	ding inputs and results of similar codes	F-166, 7-82

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323-EQ-SIF

Revision	Original	Date	Rev	Dete	Rev.	Date	Rev.	Dete	Rev.	Dete
Chectory	1	> <		><		$>\!<$		><		><
Preparer.	-fac	8/11/84						-		1
Checker	9/20	8/27/84								-

187 188 189 190	-887 -882 -89 -90 -91	70 725	N/A	N/A	N/A
187 188 189 190 191	-882 -89 -90 -91	Yes	N/A		
188 189 190	1-90			N/A	N/A
189 1	1-90	¥	No	Yes	See Calc Pose 52
190	1-91	Yes	No	Yes	See Calc. Poge 58
191		No	N/A	N/A	N/A
	1-92A	Yes	No	Yes	See Calc. Poge 60
	1-92B	No	N/A	N/A	N/A
	1-93A	Yes	No	Yes	See Calc. Poge 61
	1-93B	Yes	Yes	N/A	N/A
	1-94	No	N/A	N/A	N/A
	1-95	Yes	No	Yes	See Calc. Poge 63
	1-96A	No	N/A	N/A	N/A
	1-96B	No	N/A	NIA	N/A
	1-96C	No	N/A	N/A	H/A
	1-960	No	N/A	N/A	N/A
	1-97A-1	Yes	Yes	N/A	MA
	1-978	No	N/A	N/A	N/A
	1-976	No	N/A	N/A	N/A
	1-970	No	N/A	N/A	N/A
_	2-970	Yes	Yes	N/A	H/A
	APP-5	Yes	Yes	N/A	N/A
	2-998	Yes	Yes	N/A	N/A
	-135A-1	No	N/A	H/A	N/A
	-1358	No No	N/A	N/A	N/A
	1-135C	Yes	Yes	N/A	N/A
	1-135D	-	Yes	N/A	N/A
-	-135 E-1	\ \/ -	Yes	N/A	N/A
	-135 F	1 1 -	Yes	NIA	N/A
	1-150 F	Yes	No	Yes	See Calc. Page 64
	2-150F	Yes	No	Yes	See Calc. Page 66
	1-1506	Yes	No	Yes	See GTH 69303, 7/31/84
	2-1506	Yes	No	Yes	Correlated to AB-1-1500
	1-150H	Yes	No	Yes	See Calc. Page 68
	2-150 H	1	No	Yes	Correlated to AB-1-150 H
	1-150I		No	Yes	See Calc. Page 70
	2-150 I		No	Yes	See Calc. Page 72
	1-1505		No	Yes	See Calc. Page 74 F-166, 7-82

Subject Topered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIF

Revision	Orgnei	Date	Rev.	Date	Rev.	Desire	Rev.	Deste	Rev.	Date
Chactery Marrod	1	><		><		><		$>\!\!<$		><
Preparer_	-sac	8/11/84								
Checker	ninn	8/21/44								

No.	Prob.	#quipment	S.I.F. Accounted For M ORIGINAL MALYUS	S.I.F. Effect Accounted For	Comments
223	2-1503	Yes	No	Yes	See Calc. Page 76
224	1-151A	Yes	Yes	N/A	N/A
-275	1-151B	Yes	Yes	N/A	See GTN 69303, 7/31/84
226	1-151C-1	Yes	Yes	NA	N/A
227	1-1510-1	No	N/A	N/A	N/A
328	1-152	Yes	Yes	N/A	N/A
-229	1-153	Yes	No .	Yes	See Calc. Page 78
230	1-154	Yes	Yes	N/A	N/A
231	1-155	Yes	Yes	N/A	N/A
232	1-156	Yes	No	Yes	See Calc. Poge 79
233	1-157A	No	N/A	N/A	N/A
234	1-1578	Yes	Yes	N/A	N/A
285	1-1570	No	NIA	NIA	N/A
236	1-158A	No	N/A	N/A	N/A
237	1-1588	Yes	Yes	N/A	N/A
238	1-158C	No	N/A	N/A	N/A
239	1-163-1	Yes	Yes	N/A	N/A
240	1-165 A-	Yes	Yes	N/A	N/A
241	1-165 BH		Yes	N/A	N/A
242	1-165C	Yes	Yes	N/A	N/A
-243	1-165 D	Yes	Yes	N/A	H/A
244	1-165E	Yes	Yes	NIA	N/A
245	1-165F	Yes	Yes	N/A	M/A
246	1-165G	Yes	Yes	AIM	N/A
247	1-165H	Yes	Yes	N/A	N/A
248	1-166A	No	N/A	N/A	N/A
249	1-166 B	No	N/A	N/A	N/A
250	1-166C	No	NIA	N/A	N/A
251	1-166 D	No	N/A	N/A	N/A
252	1-1674-1	Yes	N/A	NIA	Same as AB-1-1678-1
253	1-1678-1	Yes	N/A	N/A	See GTH - 69 303, 7/31/84
254	1-167C	Yes	N/A	N/A	Same as AB-1-169 8-1
255	1-1670-1	Yes	N/A	N/A	Same as AB-1-167 B-1
256	1-167E-1		N/A	N/A	Same as AB-1-1678-1
257	1-167F-1		N/A	N/A	Same as AB-1-1678-1
258	1-168	No	NIA	N/A	N/A
259	1-169	No	N/A	N/A	N/A
The second secon	ing Method	# 1 Line-by-tire chec	Aung lation Results compared from Results compared	ding inputs and results of similar codes	F-166, 7-82

Subject Topered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ- SIF

Revision	Crigina	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Chadeng Method #	1							$>\!<$		><
Preparer_	Jac.	18/11/84								
Checker	2000	8/23/64								

No.	Prob.	Equipment	S.I. F. Accounted For IN PRIGNAL ANTIN'S	Accounted	Comment.	s
260	1-170	No	N/A	H/A	N/A	
561	1-171	No	N/A	N/A	N/A	
565	1-172	No	N/A	N/A	N/A	
263	1-174	No	MA	N/A	N/A	
264	1-175	No	MIA	MA	N/A	
265	1-178A	Yes Yes	Yes	H/A Yes	See Calc. P	09e 80
266	1-178B	No	NO N/A	N/A	N/A	3000
268	1-179	No	NA	NIA	N/A	
269	5-181	Yes	Yes	N/A	N/A	
270	1-186	No	N/A	N/A	N/A	
27:	1-188	No	N/A	N/A	N/A	
272	1-189	No	N/A	N/A	N/A	
				positivity and		
	1					
		4.1				
					4 3 400 11 14 14 1	
Check	ing Method	3. Identical Calculat	lang lation Results compared on Results conspared inthinsults of cumputer with consepon			F-166, 7-82

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ-SIF Sheet No. 11

Flevision	Cingrai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Method	1			><		><		$>\!\!<$		\sim
Preparac_	Soc.	9/15/84								
Checker	2600	8/11/84	100000							

Problem AB-1-10 D-1

MAXIMUM STRESS RESULTS

Condition	Equition	Allowable Stress	Calculated Stress	tiode	S.I.F.	Description
Normal and Upset	8	15000	5156	687	1.9	
Normal and Upset	9	1.2 Sh	3825	687	1.9	
Emergency	9	1.8 s _h	4254	687	1.9	
Faulted	9	2.45h	4254	687	1.9	
Normal and Upset	10	5 _A	2851	687	1.9	
Normal and Upset	11	s _A + s _h	7541	687	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323 - EQ - SIF Sheet No. 12

Revision	Original	Date	Rev.	Date	Rev.	Dete	Rev.	Date	Rev.	Date	
Decking Method 2	1			><		><		><		><	
Preparet_	40C	8/15/84									
Checker	NAM	8/21/100									

Problem AB-1-11A

MAXIBUM STRESS RESULTS

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	1108	99	1.9	
Normal and Upset	9	1.2 Sh	1828	99	1.9	
Emergency	9	1.3 Sh 27000	2086	99	1.9	
Faulted	9	2.45h	2086	99	1.9	12
Normal and Upset	10	5 _A	12527	99	1.9	
Normal and Upset	11	s _A + s _h	13634	99	1.9	

Gibbs & Hill, Inc. Job No. 2323-046 Client Tus!
Subject Tapered Transition Joint S.T.F. At Equipment

Calculation Number 2323- EQ-SIF

Sheet No. 13

Revision	Cingrai	Date	Rev.	Date	Rev.	Date	Rev.	Dute	Rev.	Date
Chectory Market	1	><						><		><
Preparec_	GAC	2/15/04								
Checker	20012	8/2/64						1379		

Problem AB-1-11B

MAXIBUE STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	1096	506	1.9	
Normal and Upset	9	1.2 Sh	4399	506	1.9	
Emergency	9	1.8 s _h	5214	506	1.9	
Faulted	9	2.45h	2744 5214	306	1.9	
Normal and Upset	10	22500	2475 4703	506	1.9	
Normal and Upset	11	5 _A + 5 _h	3053	506	1.9	

Client TUS! Gibb's & Hill, Inc. Job No. 2323-046

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ-SIE

Sheet No. 14

- I-I	Circural	T Date T	Rev	T Date 1	Rev.	Date i	Rev.	Date	Rev.	Date
Revision	Bake	Valo	THOY.	—		—				
Checking Method 9	1			2						1
Preparer -	Jac	8/11/84				-		-		+
Checker	26000	8/21/84				-		-		++

Problem AB-1-27, Rev. 1

MAXIMUM STRESS RESULTS

Condition	Equation	Allowable	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	16600	4666	1	1.9	
Normal and Upset	9	1.2 Sh	4059	١	1.9	
Emergency	9	1.8 Sh 29880	8449	1	1.9	
Faulted	9	2.45h	8449	1	1.9	
Normal and Upset	10	5 _A .	2400	1	1.9	
Normal and Upset	11	s _h + s _h 44250	3717	1	1.9	

Job No. 2323-046 Client TUS! Gibbs & Hill, Inc.

Subject Topered Transition Joint S.I. F At Equipment

Calculation Number 2323- EQ- 51F

Sheet No. 15

	Name	Date	Bev	Date	Rev.	Date	Rev.	Date	Rev.	Dete
evision	200.00	Date	7404.	S		><		><		$\geq \leq$
reparer_	Sac	8/11/84								-
hecker	200011	11/23/84				-		-		+

Problem AB-1-27, Rev. 1

MAXIEUM STRESS RESULTS

Plant	Equation	Allowable Stress	Calculated Stress	tiode	S.I.F.	Description
Normal and Upset	8	16600	6555	51	1.9	
Normal and Upset	9	1.2 Sh	9460	51	1.9	
Emergency	9	1.8 Sh 29880	53-5	51	1.9	
Faulted	9	2.45h 39840	10118	51	1.9	
Normal and Upset	10	SA. 27650	1759	51	1.9	
Normal and Upset	11	s _k + s _h 44 250	8313	51	1.9	

Subject Topered Transition Joint S.I. F. At Equipment

Calculation Number 2323- EQ-SIF

Sheet No. 16

Revision	Organa	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Tracking a	1	><		><		><		$>\!\!<$		><
Preparer_	-ac	8/11/84	MARK			1		-		++
hecker	Ten.	4/7/1/84								++

Problem AB-1-27, Rev. 1

MAXIBUE STRESS RESULTS

Plant Condition	Equation	Allowable Stress	Calculated Stress	Node	S. I.F.	Description
Normal and Upset	8	5h 16600	3777	32	1.9	
Normal and Upset	9	1.2 Sh	2899	32	1.9	
Emergency	9	1.8 Sh 29880	3452	32	1.9	
Faulted	9	2.45h 39840	3982 7452	32	1.9	
Normal and Upset	10	5 _A .	1699	32	1.9	
Normal and Upset	11	s _h + s _h 44250	8875	32	1.9	

Subject Tapered Transition Joint S.I. F. At Equipment

Calculation Number 2323- EQ- SIF

Sheet No. 17

Revision	Crigna	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Nectors Medical	7	><		><		><		$>\!\!<$		\sim
Preparer .	-Aac	8/11/84								-
Chacker	2000	6h1/84								

Problem AB-1-28, Rev. 1

MAXIEUM STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	16600	4799	42	1.9	
Normal and Upset	9	1.2 Sh	2837 5390	42	1.9	
Emergency	9	1.8 S _h	5681	42	1.9	
Faulted	9	2.45h 39840	5681	42	1.9	
Normal and Upset	10	5 _A .	6411	42	1.9	
Normal and Upset	11	s _k + s _h	11212	42	1.9	

Subject Tapered Transition Joint S. J. F. At Equipment

Calculation Number 2323 - EQ- SIF

Sheet No. 18

O-deles	Organia	T Date T	Rev	Date	Rev.	Date	Rev.	Date	Rev.	Date
Revision Checking	bon.e	1		><		><		><		><
Prepared_	FOC	18/11/84						-		-
Checker	2001	\$ /21/84				-				+

Problem AB-1-28, Rev. 1

MAXIMUM STRESS RESULTS

Plant Condition	i Equation	Allowable Stress	Calculated Stress	Node	5.I.F.	Description
Normal and Upset	В	16600	2458	501	1.9	
Normal and Upset	9	1.2 Sh	5278	501	1.9	
Emergency	9	1.8 s _h	5520	501	1.9	
Faulted	9	2.45 _h	5520	501	1.9	
Normal and Upset	10	5A.	273 519	501	1.9	
Normal and Upset	11	Sh+ Sh 44421	5189	50	1.9	

Subject Topered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - 51F Sheet No. 19

Revision	Organi	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Dete	
Revision	7			><		><		><		><	
Preparec_	fac	8/11/84								-	
Checker 19	MAN.	12/2//89						-		-	

Problem AB-1-28, Rev. 1

MAXIEUM STRESS RESULTS

Plant	Equation	Allowable Stress	Calculated Stress	Node	5.1.F.	Description
Normal and Upset	8	5h 16600	6878	631	1.9	
Normal and Upset	9	1.2 51.	2807 7233	831	1.9	
Emergency	9	29860	7406	831	1.9	
Faulted	9	2.45h 39840	7406	631	1.9	
Normal and Upset	10	5 _A .	1301	183	1.9	
Normal and Upset	11	5x+5h 44421	9350	831	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ-SIF

Sheet No. 20

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Revision	Chigana	Date	Rev.	Date	Rev.	Dete	Rev.	Dete	Rev.	Date
Chectory Method 3	1	><		><		><		><		><
Preparer-	Sec	2/15/84								
Checker	960m	8/2:/84								

Problem AB-1-30-1

MAXIMUM STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	1668	44	1.9	
Normal and Upset	9	1,2 sh	1502	44	1.9	
Emergency	9	29880	3245	44	1.9	
Faulted	9	2.45 _h	3245	44	1.9	
Normal and Upset	10	s _A 27650	1384	44	1.9	
Normal and Upset	11	s _h + s _h	4298	44	1.9	

Client Tus! Job No. 2323-046 Gibbs & Hill, Inc.

Subject Tapered Transition Joint S.I.F. A+ Equipme

Calculation Number 2323 - EQ-SIF

Sheet No. 21

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Checking Method #		><		><		><		><		><	
Preparer	Soc	8/16/84									
Checker	NOIN	8/16/84									

Problem AB-1-31

MAXIMUM STRESS RESULTS

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	16600	1621	44	1.9	
Normal and Upset	9	1.2 s _h	5789	44	1.9	
Emergency	9	1.8 S _h	7978	44	1.9	
Faulted	9	2.45h 39840	7978	44	1.9	
Normal and Upset	10	5 _A	8960	44	1.9	
Normal and Upset	11	s _h + s _h	10581	44	1.9	

Subject Tapered Transition Joint E.I.F. At Equipment

Calculation Number 2323- EQ- SIF

Sheet No. 22

Revision	Orgnei	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
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Preparer_	Anc	8/12/64								
Checker	2/011	2/2/84								

Problem AB-1-34A

MAXIMUM STRESS RESULTS

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	18300	1307	278	1.9	
Normal and Upset	9	1.2 sh 219:60	2225	278	1.9	
Emergency	9	1.8 s _h	2434	278	1.9	
Faulted	9	2.45h 43920	2434	278	1.9	
Normal and Upset	10	5 _A .	3331	278	1.9	
Normal and Upset	11	s _k + s _h · 46375	7636	276	1.9	

Subject Tapered Transition Joint S.I. F At Frimpment

Calculation Number 2323- EQ-SIF

Sheet No. 23

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Berdeinn	Original	Date	e T	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Dete
Revision	1	>	\leq		><		><		><		><
Preparec	さのこ	€1.7	8-1						-		-
Preparex_ Checker	18411	8/21/	84								-

Preblem AG-1-34C

MAXIBUN STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S. I.F.	Description
Normal and Upset	8	16600	2523	450	1.9	
Normal and Upset	9	1.2 Sh	11600	250	1.9	
Emergency	9	1.8 S _h	13899	150	1.9	
Faulted	9	2.45h 39840	13899	450	1.9	
Normal and Upset	10	5 _A .	20317	450	1.9	
Normal and Upset	11	s _A + s _h	23980	450	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - 51F

Sheet No. 24

Revision	Ongeral	Date	Rev.	Date	Rev.	Date	Rov.	Date	Rev.	Date
Chectury leastings in	1	><		><		><		><		><
Preparer -	DOC	8/15/mg		The state of						
Chacker	Non	8/21/69								

Problem AB-1-AZA

MAXIBUN STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h 15900	10230	91	1.9	
Normal and Upset	9	1.2 Sh	12992	91	1.9	
Emergency	9	1.8 S _h	13479	91	1.9	
Faulted	9	2.45 _h	13479	91	1.9	
Normal and Upset	10	5 _A	1169	91	1.9	
Normal and Upset	11	s _A + s _h 43 375	11398	91	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323-EQ-SIF

Sheet No. 25

Revision	Ongrai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checung Method #	1	><		><		><		><		><
	-A-C	8/16/84								
Checker	Non.	1 /21/84								

Problem AB-1-46B

MAXIEUM STRESS RESULTS

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
formal and Upset	8	16200	5664	128	1.9	
Normal and Upset	9	1.2 Sh	407E 7748	128	1.9	
Emergency	9	29160	8480	128	1.9	
Faulted	9	2.4 Sh 38880	8480	128	1.9	
Normal and Upset	20	5 _A	3487	128	1.9	
Normal and Upset	11	s _A + s _h	9150	128	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Caiculation Number 2323- Ea-SIF

Sheet No. 26

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rav.	Date
Checking Method #	1	><		><		><		><		><
Preparer_	fac	2/12/84							THE REAL PROPERTY.	
Checker	Han	8/21/84	THE STATE OF							

Problem AB-1-47B

MAXIMUM STRESS RESULTS

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17200	4630	262	1.9	
Normal and Upset	9	1.2 S _h	5305	262	1.9	
Emergency	9	1.8 s _h	11598	565	1.9	
Faulted	9	2.45 _h	11598	262	1.9	
Normal and Upset	10	5 _A	14288	262	1.9	
Normal and Upset	11	s _A + s _h	9957	262	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323 - EQ - SIF

Sheet No. 27

Revision	issue.	Dete	Rev.	Date	Rev.	Date	Rev.	Dete	Rev.	Date	
delrod 3										><	
Preparer	Soc	8/12/84									
Checker	JAM A	8/2/1/4									_

Problem AB-1-524

"MAXIMUM STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and	1 8	Sh	608			
Upset	104	17800	1155	1100	1.9	
Normal and Upset	9	1.2 Sh	308			
		21360	1155	1100	1.9	
Emergency	9	1.8 S _h	608			
		32040	1155	1100	1.9	
Faulted	9	2.4 Sh	608			Production of the last of the
		42720	1155	1100	1.9	
Normal and Upset	10	S _A	0			
-pace		26950	0	1100	1.9	
Normal and	11	SA+ Sh	608		1.9	
Upset	11	44750	1155	1100	1.7	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ-51F

Sheet No. 28

Revision	Organa	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Markou	1	$>\!<$		><		><		><		><
Preparer	HAC	8/12/04								-
Checker	exaci.	8/12/84								-

Problem AB-1- 58

MAXIEUM STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	1729	97	1.9	
Normal and Upset	9	1.2 Sh	3667	97	1.9	
Emergency	9	1.8 s _h	5045	97	1.9	
Faulted	9	2.4 Sh 36000	5045	97	1.9	
Normal and Upset	10	5 _A .	1036	97	1.9	
Normal and Upset	11	s _A + s _h	2765	97	1.9	

Subject Tapered Transition Joint S.I. F At Fquipment

Calculation Number 2323- EQ-51F

Sheet No. 29

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Checking Mathod F	T	\sim		><		><		><		><	
Preparer -	-C	8/12/84		DIES SEE							
Checker	219.7	8/21/84									

Problem AB- 1-58

MAXIBUM STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	tiode	S.I.F.	Description
Normal and Upset	8	15000	1393	65	1.9	
Normal and Upset	9	1.2 Sh	2586	65	1.9	
Emergency	9	27000	3091	65	1.9	
Faulted	9	2.4 Sh 36000	3091	65	1.9	
Normal and Upset	10	5 _A .	1410	65	1.9	
Normal and Upset	11	5 _A + 5 _h	2803	65	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipmen

Calculation Number 2323 - EQ-SIF

Sheet No. 30

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Interface I	T	><		><		><		><		><
Preparec_	-gac	2/15/84								
Checker	gram	9/32/44								

Problem AB-1-59D

MAXIEUM STRESS RESULTS

Condition	Equation	Allowable Stress	Ca'culated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	4352	472	1.425	
Normal and Upset	9	1.2 Sh	18201	472	1.425	
Emergency	9	1.8 s _h	16486	472	1.425	
Faulted	9	2.4 Sh 36000	21697	472	1.425	
Normal and Upset	10	5 _A	17119	472	1.9	
Normal and Upset	11	s _A + s _h	13362	472	1.9	

* Exceeds allowable. See next page. Pressure Stress = PDo = (2500)(1.9) = 4226 psi

P = 2500 Ps . 00 = 1.4 in. tn = .281 in.

S.I.F. for Eq 849 = (1.9)(.75) = 1.425 S.I.F. for Eq 10 = 1.9 Eq 11 = Eq 8+ Eq 10

Checking Method # 1 Une by the challen Results compared 2 Administration Calculation Results compared 5 species (Calculation Results compared

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ- 51F

Sheet No. 31

Revision	Ongrai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Method #		><		><		><		><		
Preparer_	Sac	8/17/84								
Checker	76000	-0/23/29								

Problem AB-1-59 D Considering pipe mismaten to obtain a revised Stress Intensification Factor

MAXIMUM STRESS RESULTS

Plant Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	4389	472	1.293	
Normal and Upset	9	1.2 Sh	14033	472	1. 293	
Emergency	9	1.8 Sh 27000	20078	472	1.293	
Faulted	9	2.4 Sh 36000	20078	472	1.293	
Normal and Upset	10 (2)	5 _A	15533	472	1.724	
Normal and Upset	11	S _A + S _h	13362	472	1.724	

S.I.F. = 1.3 + 0.0036 Do + 3.6 & (ASME Code Section & 1974, Fig NC-3673, 2(b)-1 (.201) + 3.6 (.0313) | 1974, Fig NC-3673, 2(b)-1 (.201) | 1974 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936 | 1936

F-166, 7-82

S.I. F for Eq 10 = 1.724 Eq 11 = Eq 8 + Eq 10

Alternative Calculation Results compared dentical Calculation Results compared Communication pages and results of computer with comesponding inputs and results of similar codes.

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323-EQ-SIF Sheet No. 32

Da inion	Crignal	Date	Rev.	Date	Rev.	Date	Rev.	Dete	Rev.	Date
hevision	1	><		><	Bursha	><		><		$\geq \leq$
Preparer	Lac	2/12/84								-
Checker	240	8/17/84				1		+		+

Problem AB-1-60

MAXIBUE STRESS RESULTS

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	3739	47	1.9	
Normal and Upset	9	1.2 Sh	4393	47	1.9	
Emergency	9	1.8 Sh 27000	4742	47	1.9	
Faulted	9	2.4 Sh 36000	4742	47	1.9	
Normal and Upset	10	22500	1638	47	1.9	
Normal and Upset	11	5 _A + 5 _h	5377	47	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIE

Sheet No. 33

Revision	Organal	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
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Preparer_	Jac	2/2/84								-
Checker	Am.	10/22/44								

Problem AB-1-60

MAXIBUM STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	3722	,	1.9	
Normal and Upset	9	1.2 S _h	3956	1	1.9	
Emergency	9	1.8 Sh	4106	1	1.9	
Faulted	9	2.4 Sh 36000	4106	1	1.9	
Normal and Upset	10	s _A .	2244	1	1.9	
Normal and Upset	11	5A+ 5h 37500	5966	1	1.9	

Subject Tapered Transition Joint S.I. F. At Equipment

Calculation Number 2323- EQ-51F

Sheet No. 34

Revision	Organa	Date	Rev.	Date	Per:	Date	Rev.	Date	Rev.	Date	
Revision Studing	1			><		><		><		><	
Preparer .	SOC	8/12/04								-	_
Checker	Om.	4/27/89						-		-	_

Problem AB-1-60

MAXIEUR STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	3713	48	1.9	
Normal and Upset	9	1.2 Sh	3929	48	1.9	
Emergency	9	1.8 S _h	4098	48	1.9	
Faulted	9	2.4 S _h	4098	48	1.9	
Normal and Upset	10	5 _A .	2405	48	1.9	
Normal and Upset	11	s _h + s _h	6118	48	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIE

Sheet No. 35

Revision	Organe	Date	Rev.	Date	Rev.	Date	Rev.	Date	Plav.	Date	
Stacking Seption 9	1	><		><		><		><		><	
Preparer	Soc	8/12/84								-	
Checker	min.	8/11/84								-	

Problem AB-1-60

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	3832	1541	1.9	
Normal and Upset	9	1.2 Sh	2292 4355	1541	1.9	
Emergency	9	1.8 S _h	2486	1541	1.9	
Faulted	9	2.45 _h	4723	1541	1.9	
Normal and Upset	10	5 _A .	4587	1541	1.9	
Normal and Upset	11	s _A + s _h	8421	1541	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323-EQ-51F

Sheet No. 36

Revision	Cingmus	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Revision Pecting	1			><		><		$>\!<$		><	
Preparec.	Enc.	18/01E4								-	
Checker	you	8/27/84								-	

Problem AB-1-60

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and	8	15000	4908	174	1.9	
Normal and Upset	9	1.2 Sh	3190 7581	174	1.9	
Emergency	9	1.8 Sh	9282	174	1.9	
Faulted	9	2.45h	9282	174	1.9	
Normal and Upset	10	5 _A .	3484	174	1.9	
Normal and Upset	11	5A+ 5h 37500	11525	174	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - 51F

Sheet No. 37

Revision	Chigana	Date	Rev.	Dete	Rev.	Date	Rev.	Date	Rev.	Date	
Checking	1	><		><		><		><		><	
Preparer-	dec.	efisiea	I di la								
Preparer 26	264m	8/11/84									

Problem . AB-1-61A-2

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	5183	1	1.9	
Normal and Upset	9	1.2 Sh	5719	1	1.9	
Emergency	9	27000	5824	1	1.9	
Faulted	9	2.4 Sh 36000	5824	1	1.9	
Normal and Upset	10	SA 22500	758	1	1.9	
Normal and Upset	11	S _A + S _h	5941	1	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ-SIE

Sheet No. 38

Revision	Marie .	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Revision hedro	1										
Preparer -	doc-	2/15/04									
Thecker ?	16/2/2	8/22/01				1 1				+	

Problem AB-1-61A-2

Condition	Equation	Allowable Stress	Calculated Stress	liode	S.I.F.	Description
Normal and Upset	8	5h	3656	24	1,9	
Normal and Upset	9	1.2 Sh	3927	24	1.9	
Emergency	9	27000	3982	24	1.9	
Faulted	9	2.4 Sh 36 000	3982	24	1.9	
Normal and Upset	10	s _A	889	24	1.9	
Normal and Upset	11	s _A + s _h	2392	24	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ-SIF

Sheet No. 39

Revision	Ongrei	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
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Properer_	-Jac	8/12/84	10.00							
Checker	Jum	16/22/84								

Problem AB-1-61D

Condition	Equation	Allowable Stress	Calculated Stress	tiode	S.I.F.	Description
Normal and Upset	8	15000	4623	462	1.9	
Normal and Upset	9	1.2 Sh	5134	462	1.9	
Emergency	9	27000	5377	462	1.9	
Faulted	9	2.45h	5377	462	1.9	
Normal and Upset	10	5 _A .	547	462	1.9	
Normal and Upset	11	5 _A + 5 _h	5168	462	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323-EQ-SIF

Sheet No. 40

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Checking Method #				><				><		><	
Preparer -	SOC.	8/16/64	Eg Alle								
Checker	9614	14/13/84					FEG.				

Problem AB-2-620

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
formal and Upset	8	15000	1486	2103	1.9	
Normal and Upset	9	1.2 Sh	4826	2103	1.9	
Emergency	9	27000	2746	2103	1.9	u
Faulted	9	2.45 _h	2746 5217	2103	1.9	
Normal and Upset	10	SA 22500	232	2103	1.9	
Normal and Upset	11	s _A + s _h	1720	2103	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323-EQ-51F

Sheet No. 41

Revision	99.0	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Steel oc 3	1			\sim							
Propanek	Jac	8/15/64									
Checker	SHAN Y	0/11/04				1		+		+	

Problem AB-1-62F

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	1828	1296		
Normal and Upset	9	1.2 Sh	2700	1296	1.9	
Emergency	9	27000	3209	1296	1.9	
Faulted	9	2.4 Sh 36000	3209	1296	1.9	
Normal and Upset	10	5 _A	3730	1296	1.9	
Normal and Upset	11	5 _A + 5 _h	5558	1296	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ-51F

Sheet No. 42

Preparer	><				1	 -	
Prenarer	STATE STATE STATE OF				-	-	
Loberton and Co. C	2/15/84						
Preparer 60C Checker 191917	8/22/64		1	-			-

Problem AB-1-63 C/B

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	1720	1	1.9	
Normal and Upset	,	1.2 Sh	2008	1	1.9	
Emergency	9	27000	2107	1	1.9	
Faulted	9	2.4 Sh	2107	1	1.9	
Normal and Upset	10	SA 22500	386	1	1.9	
Normal and Upset	11	s _A + s _h	2105	1	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ-51E

Sheet No. 43

Revision	Chighres	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
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Ртерален	Sac	2/15/84									
Prepares Sec Checker 2/2	non	8/22/84									

Problem AB-1-64A

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.1.F.	Description
Normal and Upset	8	5h	1752	760	1.9	
Normal and Upset	9	1.2 Sh	2943	760	1.9	
Emergency	9	27000	3238	760	1.9	
Faulted	9	2.4 Sh 36000	3238	760	1.9	
Normal and Upset	10	5 _A	2073	760	1.9	
Normal and Upset	11	5A+ 5h	3825	760	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipmen

Calculation Number 2323-EQ-SIF Sheet No. 44

Revision	Crigoral	Date	Rev.	Date	Rev.	Dete	Rev.	Date	Rev.	Date	
Checking	1									><	
Preparer -	O-C	8/15/84									
Preparer— Checker	Nm	8/22/84								1	

Problem AB-1-64F

Note: Stresses taken from node 1067

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	3348	1068	1.9	
Normal and Upset	9	1.2 S _h	3870	1068	1.9	
Emergency	9	27000	2202	1068	1.9	
Faulted	9	2.4 Sh 36000	2202	1068	1.9	
Normal and Upset	10	5 _A	2335	1068	1.9	
Normal and Upset	11	s _A + s _h	5683	1068	1.9	

Subject Tapered Transition Joint S.T.F. A+ Equipmen

Calculation Number 2323 - EQ - SIE

Sheet No. 45

Revision	Chiganal	Date	Rev.	Date	Rev.	Date	Rev.	Dete	Rev.	Date
Name of	1			$>\!<$		><		><		><
Preparer.	SOC	12/15/84								
Chec.er W. Zo.	6/ - m	8 /21/84							HILLS:	

Problem . AB-1-64F-Note: Stresses taken from node 5837

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	3566	5838		
Normal and Upset	9	1.2 Sh	2193	5838	1.9	
Emergency	9	27000	4598	5838	1.9	
Faulted	9	2.4 Sh 36000	2420 4598	5838	1.9	
Normal and Upset	10	5 _A	871 1655	5838	1.9	
Normal and Upset	11	s _A + s _h	2748 5221	5838	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipmen

Calculation Number 2323- EQ-SIE

Sheet No. 46

Revision	Dirganas	Date	Rev.	Date	Rev.	Data	Rev.	Date	Rev.	Date
hecturing test noo a	1									>
reparer	Soc	8/15/84								
Preparer Ga Checker 7/4	2/11/11	8h: 164								

Problem . AB-1-76A

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	10024	500	1.9	
Normal and Upset	9	1.2 Sh	5680	500	1.9	
Emergency	9	27000	5797	500	1.9	
Fiu red	9	2.4 Sh 36000	25844	500	1.9	
Formal and Upset	10	SA 22500	13943	500	1.9	
Normal and Upset	11	5A+ 55 37500	19219	500	1.9	

Equation 10 exceeded however, equation 11 is less than 37500 psi. There fore, o.K.

Gibibs & Hill, Inc. Job No. 2323-046 Client Tus!
Subject Tapered Transition Joint S.T.F. At Equipment

Calculation Number 2323-EQ-SIE Sheet No. 47

Revision	Cingma	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Matrice	1									
Ртерагег	Sac.	8/15/84								
Checker	2/m	14/21/84								

Problem - AB-1-76A

MAXIEUM STRESS RESULTS

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	3974	260	1.425	
Normal and Upset	9	1.2 Sh	7798	260	1.425	
Emergency	9	27000	7597 9838	260	1.425	
Faulted	9	2.4 Sh 36000	19825	260	1.425	
Normal and Upset	10	5 _A	26849*	260	1.9	
Normal and Upset	11	s _A + s _h	31024	260	1.9	

than 37500 psi . Therefore O.K.

Pressure stress = 3500 psi

Chacking Mathad # 1 Jesus and

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ - SIF

Sheet No. 48

Revision	Ongmai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Memor	1	><		><		><		><		><
Preparer_	4-6	8/10/84						-		-
Checker	Jum	8/10/84				+		-		++

Problem AB-1-80B Note: Stresses taken from node 5908

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	7712	5905	1.9	
Normal and Upset	9	1.2 Sh	8801	5905	1.9	
Emergency	9	1.8 Sh 27000	4722 8972	5905	1.9	
Faulted	9	2.4 Sh 36000	4722 8972	5905	1.9	
Normal and Upset	10	S _A	3215	5905	1.9	
Normal and Upset	11	s _A + s _h	10929	5905	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ-SIE

Sheet No. 49

Revision	Organa	Date	Rev.	Oate	Rev.	Date	Rev.	Date	Rev.	Date
Chectury Method 8	1	$>\!<$				><		$>\!<$		><
Preparer-	es-c	2/15/24								
Checker	ofn:	4/22/64								

Problem . AB-1-88C

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h 17800	2240	1	1.9	
Normal and Upset	9	21360	2348	1	1.9	
Emergency	9	1.8 S _h	2573	1	1.9	
Faulted	9	2.4 Sh A2720	2573	1	1.9	
Normal and Upset	10	5 _A	3197	1	1.9	
Normal and Upset	11	s _A + s _h	8314	1	1.9	

Subject Tapered Transition Joint S. I.F. At Equipment

Calculation Number 2323- EQ - SIF

Sheet No. 50

Revision	Orgnal	Date	Rev.	Date	Rev.	Dete	Rev.	Date	Rev.	Date
Theolog Approx 3	1	><		><		><		><		><
	Bac.	2/3/84								-
Checker	Non	8/27/84								-
	1									

Problem AB-1-88 E

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17800	5506	1	1.9	
Normal and Upset	9	21360	4689	,	1.9	
Emergency	9	1.8 s _h	2745 5216	1	1.9	
Faulted	9	2.45h 42720	5216	1	1.9	
Normal and Upset	10	5 _A .	6192	1	1.9	
Normal and Upset	11	s _h + s _h 45750	8398	1	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIF

Sheet No. 51

Revision	Cingoral	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Dete
Chectury aternocy	1					><	H	$>\!\!<$		><
Preparer	LOC	8/13/84								
Checker	afron	8/22/34	1							

Problem AB-1-86E

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17800	2269	409	1.9	
Normal and Upset	9	1.2 Sh 21360	3688	409	1.9	
Emergency	9	1.8 S _h	3971	409	1.9	
Faulted	9	2.45h 42720	3971	409	1.9	
Normal and Upset	10	5 _A .	1670	409	1.9	
Normal and Upset	11	s _k + s _h 45750	3939	409	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ-51F Sheet No. 52

Revision	Cingrai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Dete
Oraciure Mac vad 8	1	><		><		><		><		><
Preparer -	Hac	3/13/84								
Checker	offer	8/13/84								

Problem AB-1-89

Plant	Equation	Allowable Stress	Calculated Stress	tiode	S.I.F.	Description
Normal and Upset	8	17800	1967	810	1.9	
Normal and Upset	9	21360	2265	810	1.9	
Emergency	9	1.8 S _h	2331	810	1.9	
Faulted	9	2.45h 42720	2331	910	1.9	
Normal and Upset	10	5 _A .	1759 3342	810	1.9	
Normal and Upset	11	s _A + s _h 45750	5309	810	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIF

Sheet No. 53

Revision	Organa	Date	Rev.	Date	Rev.	Date	Rev.	Desta	Rev.	Date
Precising Author	1			><		><		\sim		><
Preparer.	Jac	2/3/84						1		-
	Nm.	6/22/84								-

Problem AD-1-89

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	1790	1	1.9	
Normal and Upset	9	21360	1363	1	1.9	
Emergency	9	1.8 s _h	2852	1	1.9	
Faulted	9	2.45h 42720	1501	1	1.9	
Normal and Upset	10	27950	786	1	1.9	
Normal and Upset	11	s _k + s _h 45750	1729 3285	1	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - Ea - SIF

Sheet No. 55

Revision	Organa	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Chactery staffee 9	T	$>\!\!<$		><		><		\sim		><
Preparer_	Sec	18/13/84			14,441					
Checker	2000	8/22/84								

Problem AG-1-89

Plant Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	1742	144	1.9	
Normal and Upset	9	1.2 Sh	2134	144	1.9	
Emergency	9	1.8 S _h	1178	144	1.9	
Faulted	9	2.45h 42720	1178	144	1.9	
Normal and Upset	10	5 _A .	1559	144	1.9	
Normal and Upset	11	s _k + s _h 45750	2146	144	1.9	

Subject topored Transition Joint S.I.F. At Equipment

Calculation Number 2323-EQ-SIF

Sheet No. 56

Revision	Organi	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Dete
Checury Heatres	1	><		><		><		$>\!<$		\times
Preparer_	Soc	13/13/84			HITCH					
Checker	Her.	4/12/64								

Preblem AB-1-89

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17800	1340	43	1.9	
Normal and Upset	9	21360	2565	43	1.9	
Emergency	9	1.8 s _h	2865	43	1.9	
Faulted	9	2.45h 42720	2865	43	1.9	
Normal and Upset	10	5 _A 27950	735 139 7	43	1.9	
Normal and Upset	11	s _h + s _h 45750	2736	43	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIF

Sheet No. 57

Revision	Cingmai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Dete
Revision	1			$>\!<$		><		$>\!\!<$		><
Preparer .	-d-c	2/13/84						-		+
Preparer	Non.	8/20/84								+

Problem AB-1-89

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Norma. and Upset	8	5h 17800	1264	222	1.9	
Normal and Upset	9	1.2 sh 21360	1275	222	1.9	
Emergency	9	32040	2643	222	1.9	
Faulted	9	2.45h 42.720	2643	222	1.9	
Normal and Upset	10	5 _A .	811	222	1.9	
Normal and Upset	11	s _k + s _h	2073	222	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ-SIF

Sheet No. 58

Revision	Ongrasi	Date	Rev.	Date	Rev.	Deia	Rev.	Date	Rev.	Date	
Checking Method	1	><		><		><		><		><	
Preparet_	Jac.	8/13/84						1			
Preparec_ Checker	Nm	16/23/94									

Problem AB-1-90

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	18400	1524	117	1.9	
Normal and Upset	9	1.2 Sh 22080	1701	117	1.9	
Emergency	9	1.8 Sh 33120	939	117	1.9	
Faulted	9	2.45h 44160	939	דוו	1.9	
Normal and Upset	10	SA. 28100	1691	117	1.9	
Normal and Upset	11	s _A + s _h	3215	117	1.9	

Subject Topered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIF

Sheet No. 59

Revision	Onginal	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Disching Method 9 Preparer -	1			><		><		><		><	
Preparer -	SOC	8/13/84									
Checker	Hm.	2/21/84									

Problem AB-1-90

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	18400	1518	84	1.9	
Normal and Upset	9	22080	1820	84	1.9	
Emergency	9	33120	1925	84	1.9	
Faulted	9	2.45h 44160	1925	84	1.9	
Normal and Upset	10	28100	5158	84	1.9	
Normal and Upset	11	s _k + s _h 46500	3644	84	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- 50-51F

Sheet No. 60

Revision	Cingnai	Date	Rev.	Date	Rev.	Date	Rev.	Dete	Rev.	Date
Revision Checking Mathod	1	$>\!\!<$		$>\!<$		><		$>\!<$		><
Prepares_	Sac	2 13 84								
Checker Am	Nm.	8/22/84								

Problem AB-1-92 A

Plant	Equation	Allowable Stress	Calculated Stress	tiode	S.I.F.	Description
Normal and Upset	8	17200	1528	588	1.9	
Normal and Upset	9	1.2 Sh 206:40	904	288	1.9	
Energency	9	1.8 s _h	1799	588	1.9	
Faulted	9	2.45h 41280	1799	588	1.9	
Normal and Upset	10	5 _A .	3559	588	1.9	
Normal and Upset	11	s _k + s _h	5086	588	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323-EQ-SIF

Sheet No. 61

Revision	Organa	Date	Pev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Chectory Method 9	1			$>\!<$		><		><		><
Preparer	-fac	8/13/84								
Checker	am	4/22/84								

Problem AB-1-93A

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	2196	387	1.9	
Normal and Upset	9	21360	7409	387	1.9	
Emergency	9	1.8 s _h	17011	387	1.9	
Faulted	9	2.45 _h	8953	387	1.9	
Normal and Upset	10	5 _A .	478 908	367	1.9	
Normal and Upset	11	s _h + s _h 45750	3107	387	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ- SIF

Sheet No. 62

Revision	Organa	Date	Rev.	Date	Rev.	Date	Rev.	Dete	Rev.	Date
Checking Method #	7	$>\!<$		><		><		><		><
Preparer	Jac.	2/13/84								
Checker	2/100	8/77/14								

Problem AB-1-93A

Plant Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17 800	2343	433	1.9	
Normal and Upset	9	1.2 Sh	7858 14930	433	1.9	
Emergency	9	1.8 Sh 32040	17262	433	1.9	
Faulted	9	2.4 Sh 42720	9285	433	1.9	
Normal and Upset	10	5 _A .	1476	433	1.9	
Normal and Upset	11	s _h + s _h 45750	2708 5145	433	1.9	

Gibbs & Hist, Inc. Job No. 2323-046 Client Tus!
Subject Tapered Transition Joint S.T.F. A+ Equipment
Calculation Number 2323-EQ-SIF Sheet No. 63

Revision	Orgnai	Date	Rev.	Date	Rev.	Date	Rev.	Dete	Rev.	Date	_
Chectury Method 3	1							><		>	_
Preparer	Sac	e/15/84									_
Checker	NA	8/1/84		1		1		1			_

Problem AB-1-95

Condition	Equation	Allowable Stress	Calculated Stress	Noue	S.I.F.	Description
Normal and Upset	8	5h	2075	70	1.9	
Normal and Upset	9	1.2 S _h	3591	70	1.9	
Emergency	9	27000	3948	70	1.9	
Faulted	9	2.4 Sh 36000	3948	70	1.9	
Normal and Upset	10	S _A 22500	3456 6566	70	1.9	
Normal and Upset	11	s _h + s _h	8641	70	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIF

Sheet No. 64

Revision	Crigne	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Method 9	1					><		$>\!<$		><
Preparer-	Won	8/16/84								
Checker	Non	8/22/84								

Problem AB-1- 150 F

Plant Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	18 760	3758	1	1.9	
Normal and Upset	9	22512	4893	1	1.9	
Emergency	9	1.8 s _h	5368	t	1.9	
Faulted	9	2.45 _h	5388	1	1.9	
Normal and Upset	10	S _A	2744	1	1.9	
Normal and Upset	11	s _A + s _h 46950	6502	1	1.9	

Subject Tapered Transition Joint S.T.F. A+ Equipment

Caiculation Number 2323- EQ-51F

Sheet No. 65

Revision	Organa	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Method	1									><
Preparer- Checker	Goc	2/10/84								
Checker	Am.	4/22/84								

Problem AB-1-150F

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	18760	2309	רו	1.9	
Normal and Upset	9	1.2 s _h	2309	17	1.9	
Emergency	9	1.8 s _h	2309	17	1.9	
Faulted	9	2.45 _h	2309	רו	1.9	
Normal and Upset	10	28190	0	17	1.9	
Normal and Upset	11	s _A + s _h	2309	17	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323 - EQ - SIF

Sheet No. 66

Revision	Ongmai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Chectury Method 9	1									
Preparer	bac	2/16/84								
Checker	Nan	E/16/84				1				_

Problem AB-2-150 F

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	18760	3747	1	1.9	
Normal and Upset	9	1.2 sh	5134	ı	1.9	
Emergency	9	1.8 s _h	2940 5586	1	1.9	
Faulted	9	2.45h 45024	5586	1-	1.9	
Normal and Upset	10	28190	2274	1	1.9	
Normal and Upset	11	5 ₄ + 5 _h 46950	3169	1	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323 - EQ-51F Sheet No. 67

Revision	Chibra	Date	Rev.	Date	Rev.	Dete	Rev.	Date	Rev	Date	
Checking Method 8	1					><		><		><	
Preparer_	Sac	e/16/84									
Checker	Jac Nan	2/22/84									

Problem AB-2-150F

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	18760	2309	.17	1.9	
Normal and Upset	9	22512	2309	17	1.9	
Emergency	9	1.8 s _h	2309	17	1.9	
Faulted	9	2.45 _h	2309	17	1.9	
Normal and Upset	10	5 _A	0	17	1.9	
Normal and Upset	11	s _h + s _h 46950	2309	17	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIF

Sheei No. 68

Revision	Onginal	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Checking Method #	1							><		><	
Preparer -	Sin.	2/16/94									
Checker	0/100	Kh1/84									

Problem AB-1-150H

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17200	3743	1	1.9	
Normal and Upset	9	1.2 s _h	5126	1	1.9	
Emergency	9	1.8 S _h	5590	1	1.9	
Faulted	9	2.4 Sh 41280	5590	1	1.9	
Normal and Upset	10	S _A 27800	11222	1	1.9	
Normal and Upset	11	s _A + s _h	5966	1	1.9	

Subject Tapered Transition Joint S.T.F. A+ Equipment
Calculation Number 2323 - EQ-SIF Sheat No. 67

Revision	Chigaras	Date	Rev.	Date	Rev.	Date	Rev.	Date	Flex.	Date
Party I	1			><						><
Preparer	Joc.	e/16/84			A Company					
Checker	Nas	2/27/84								

Problem AB-2-150F

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	18760	2309	17	1.9	
Normal and Upset	9	22512	2309	17	1.9	
Emergency	9	1.8 s _h	2309	17	1.9	
Faulted	9	2.45h 45024	2309	17	1.9	
Normal and Upset	10	5 _A	0	17	1.9	
Normal and Upset	11	5 _A + 5 _h 46950	2309	17	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number	2323.	EQ-SIF	Sheet No. 68
			DI 10011101 60 C

Revision	Ongmail	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Method if	1	><		><		><		><		><
Preparer -	de c	2/16/94								
Checker	0/20	8/2/84								

Problem AB-1-150H

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	В	17200	3743	1	1.9	
Normal and Upset	9	1.2 Sh 20640	5126	1	1.9	
Emergency	9	1.8 s _h	5590	1	1.9	
Faulted	9	2.4 S _h	5590	1	1.9	
Normal and Upset	10	S _A 27800	1169	1	1.9	
Normal and Upset	11	s _A + s _h	5966	1	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ-SIF

Sheet No. 69

Revision	Onginal	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Method #	1					>>		><		
Preparec	Sec.	2/6/84								-
Checker	Nm	101/84		1.						

Problem AB-1-150 H

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17200	2309	17	1.9	
Normal and Upset	9	20640	2309	17	1.9	
Emergency	9	1.8 S _h	2309	רו	1.9	
Faulted	9	2.4 Sh 41280	2309	17	1.9	-
Normal and Upset	10	S _A 27800	0	רו	1.9	
Normal and Upset	11	S _A + S _h	2309	רו	1.9	i e y

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ- SIF

Sheet No. 70

Revision	Ongria!	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Checking Method #	1									
	300	8/16/84								
Preparer : Checker	after	8/21/64								

Problem AB-1-150I

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and	8	sh	1936			
Upset		17200	3678	1	1.9	
Normal and Upset	9	1.2 Sh	2678			
opace		206:40	5088	1	1.9	
Emergency	9	1.8 Sh	2-59			
		30960	5565	1	1.9	
Faulted	9	2.4 Sh	2929			
		41280	5565	1	1.9	
Normal and Upset	10	SA	1464			2 1000
apsec		27800	5850	1	1.9	
Normal and	1,,	SA+ Sh	3420			
Upset	11	45000	6498	1	1.9	

Subject Tapered Transition Joint S.T.F. A+ Equipment

Calculation Number 2323-EQ- SIF

Sheet No. 71

Revision	Ongina'	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Checking Metroc #	1	><		><		><		><		><	
Preparer_	-Sac	8/16/84		The second							
Preparer_ Checker	gpm.	8/22/84		1							

Problem AB-1- 150I

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.1.7.	Description
Normal and Upset	В	17200	2309	17	1.9	
Normal and Upset	9	1.2 Sh 20640	2309	רו	1.9	
Emergency	9	1.8 S _h	2309	רו	1.9	
Faulted	9	2.45 _h	2309	7	1.9	
Normal and Upset	10	S _A	0	רו	1.9	
Normal and Upset	11	5 _A + 5 _h	2309	רו	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323-EQ-SIF

Sheet No. 72

Revision	Ongina	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	617
Division of	1			><		$>\!<$				><	
Preparec. Checker	Jec.	2/16/84					Bill Live				
Checker	opm	8/m /44									

Problem AB- 2-150 I

Plant Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	В	17200	3705	1	1.9	
Normal and Upset	9	1.2 Sh 20640	4769	1	1.9	
Emergency	9	30960	5219	1	1.9	
Faulted	9	2.4 Sh 41280	2747 5219	1	1.9	
Normal and Upset	10	SA 27800	2367	1	1.9	
Normal and Upset	11	S _A + S _h	3186 6053	1	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ- SIF

Sheet No. 73

Revision	Ongrai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Revision Orecard Menocia	1	><		><		><		><		><
Preparer -	Soc	2/16/84								
Checker	2/202	8/11/84				Paralle de la constante de la				

Problem AB- 2-150 I

Plant Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17200	2309	רו	1.9	
Normal and Upset	9	1.2 Sh 20640	1312	17	1.9	
Emergency	9	1.8 S _h	2309	רי	1.9	
Faulted	9	2.45 _h	2309	די	1.9	
Normal and Upset	10	5 _A	0	רו	1.9	
Normal and Upset	11	S _A + S _h	1215	17	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ-SIF

Sheet No. 74

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Revision Chadeng Method P	1									><	
Preparer-	SOC	2/16/84									
Checker	am	8/22/84									

Problem AB-1-150J

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h 17200	3787	1	1.9	
Normal and Upset	9	206.40	2731 5189	1	1.9	
Emergency	9	1.8 S _h	5641	1	1.9	
Faulted	9	2.45h 41280	5641	1	1.9	
Normal and Upset	10	S _A 27800	2189	ı	1.9	
Normal and Upset	11	5 _A + 5 _h	3145 5976	1	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipme.

Calculation Number 2323- EQ-SIE

Sheet No. 75

Preparer de c e/16/84 Checker Nan gif 44	Revision	issue	Date	Rev.	Date	Rev.	Date	Rev.	Date	Hev.	Date
Preparer	effect ?	1									
	reparer -	doc-	2/16/84								
Checker Wan sit 84	hecker	Non	84 44			-	+				-

Problem AG-1-150 J

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F. Descripti
Normal and Upset	8	17200	2309	וח	1.9
Normal and Upset	9	1.2 s _h	2309	17	1.9
Emergency	9	1.8 s _h	2309	17	1.9
Faulted	9	2.45 _h	2309	17	1.9
Normal and Upset	10	5 _A	0	17	1.9
Normal and Upset	11	S _A + S _E	2309	17	1.9

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323- EQ-SIF Sheet No. 76

Revision	Ongmai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Ohrsoking Memod #	1	><		><		> <		\sim		
Preparer-	Sec	8/16/84								
Checker	9/1002	8/22/89		1	GLIET.					

Problem AB- 2-1505

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	5h	3743	1	1.9	
Normal and Upset	9	1.2 Sh 206:40	2533	1	1.9	
Emergency	9	1.8 S _h	2767 52 57	1	1.9	
Faulted	9	2.4 Sh 41280	5257	1	1.9	
Normal and Upset	10	SA 27800	2562	1	1.9	
Normal and Upset	11	5 ₂ + 5 _h	6010	1	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323-EQ-51F

Sheet No. 77

Revision	Ongmal	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
hecking Aethod #	1 Sami							><		><	
reparer_	Sac	2/16/84									
hacker	AMANI	8/10/84									

Problem AB-2-1505

Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	17200	2309	17	1.9	
Normal and Upset	9	1.2 Sh 20640	2309	רו	1.9	
Emergency	9	1.8 S _h	2309	רו	1.9	
Faulted	9	2.45h 41280	2309	רי	1.9	
Normal and Upset	10	5 _A	0	רי	1.9	
Normal and Upset	11	S _A + S _h	2309	17	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323-EQ-SIF Sheet No. 78

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date
Chactory Method 9	1			><		><		><		><
Preparer.	Boc	2/13/84								
Preparer Checker	7421.	R 127 164								

Problem AB-1-153

Plant	Equation	(1) Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	9255	1151	1.9	
Normal and Upset	9	1.2 Sh	5124 9736	1151	1.9	
Emergency	9	27000	5259 9992	1151	1.9	
Faulted	9	2.4 Sh	21979	1151	1.9	
Normal and Upset	10	5 _A	14451 27457*	1 151	1.9	
Normal and Upset	11	5 _A + 5 _h	19322	1151	1.9	

^{*} Equation 10 exceeded however, equation 11 is less than 37500 psi. Therefore o.K.

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323 - EQ- SIF

Sheet No. 79

Revision	Ongmai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Checking Method #	1	><		><		><		$\geq <$		><	
Prepai -	Joc	2/14/84									
Checker	11/34	8/33 KG					14				

Problem AB-1-156

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	3198	698	1.9	
Normal and Upset	9	1.2 Sh	2256	698	1.9	
Emergency	9	1.8 S _h	2429	698	1.9	
Faulted	9	2.4 Sh 36000	4615	698	1.9	
Normal and Upset	10	5 _A	9627	698	1.9	
Normal and Upset	11	s _A + s _h	12823	698	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323 - EQ - SIF

Sheet No. 80

Revision	Ongrai	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Checking Method if	1									><	
Preparer -	Sac.	2/16/84		6871.00							
reparer - Checker	2000	8/22/89									

Problem AB-1- 170 B

Plant	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	В	15000	56 Z	238	1.9	
Normal and Upset	9	1.2 Sh	562	238	1.9	
Emergency	9	27000	296 562	238	1.9	
Faulted	9	2.4 Sh 36000	296 562	238	1.9	
Normal and Upset	10	S _A 22500	0	238	1.9	
Normal and Upset	11	SA+ Sh 37500	296 562	238	1.9	

Subject Tapered Transition Joint S.I.F. A+ Equipment

Calculation Number 2323- EQ - SIF

Sheet No. 81

Revision	Original	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Checking Memod #	1	><		><		><		><		><	
Preparec.	Sac	8/16/84									
Checker	Offm	2/23/84									

Problem AB-1-178 B

Plant Condition	Equation	Allowable Stress	Calculated Stress	Node	S.I.F.	Description
Normal and Upset	8	15000	562	231	1.9	
Normal and Upset	9	1.2 Sh	562	231	1.9	
Emergency	9	1.8 S _h	296 562	231	1.9	
Faulted	9	2.4 Sh 36000	296 562	231	1.9	
Normal and Upset	10	SA 22500	0	231	1.9	
Normal and Upset	11	S _A + S _h	562	231	1.9	

Subject Tapered Transition Joint S.I.F. At Equipment

Calculation Number 2323 - EQ - SIF Sheet No. 82

Revision	Chiganas	Date	Rev.	Date	Rev.	Date	Rev.	Date	Rev.	Date	
Chacking Mathod	1	\times									
Preparer_	Sec	8/17/84									
Checker	non	8/17/84									

Conclusion :

The results of this calculation are acceptable and responsive to the purpose of this calculation

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Gibbs & Hill, Inc.

11 Penn Plaza New York, New York 10001 212 760- 4438 Telex Domestic: 127636/968694 International: 428813/234475 A Dravo Company

Rec. 8/27/84
Distribution

Nullillisins
L. Weingart
D. Minichiello

August 23, 1984

GTN-69368

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project Gen. Mgr.

JOB NO : DATE LOGGED: LOG NO . : FILE: CROSS

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY COMANCHE PEAK STEAM ELECTRIC STATION G&H PROJECT NO. 2323 MASS PARTICIPATION

REF 1: GTN-69316 DTD 8/3/84 REF 2: GTN-69339 DTD 8/10/84

With regards to the outlined plan of action in reference 1, attached is the supplement to the problem sample established in reference 2. Attached is a list of the additional sample (15 problems). Whether or not these 15 problems will be utilized is dependent upon the results of the first sample.

Should you have any questions, contact either Henry W. Mentel (x6302) or Steve Lim (x5212).

Very truly yours,

GIBBS & HILL, Inc.

of In maran

Robert E. Ballard, Jr. Director of Projects

MMam

REBa-HWMe:1c

1 Letter + 1 Attachment CC: ARMS (B&R Site) OL

D. Wade (TUSI Site) 1L

G. Grace (TUSI Site) 1L

Williams (CYGNA CA) 1L 1A

G. Bjorkman (CYGNA MA) 1L 1A

Attachment to GTN-69368

PROBLEM NUMBER	PIPE SIZE(S)	BUILDING(S) ¹	x	MF Y	Z	CURVES ² A/B	NUMBER OF SUPPORTS
AB-1-11C	6, 8, 10	s	.249	.123	.522	A	25
AB-1-29K	6	С	.186	.585	.195	A	15
AB-1-37B	6, 8, 10	c	.603	.182	.573	A	22
AB-1-45Q	3/4, 1-1/2, 2	RI	.202	.120	.272	A	18
AB-1-51C-1	3	S, A, C	.043	.157	.234	A	22
AB-1-52Z	1, 2	S, A, C	.625	.133	.155	A	34
AB-1-64A	12, 8, 6, 4	S	.430	.111	.186	A	51
AB-1-68V-1	10	S	.150	.635	.559	A	6
AB-1-86B	10	F	. 497	.123	.156	A	5
AB-1-90	3, 4	A	.597	.189	.161	А	29
AB-1-92A	3, 4	S	.207	.120	.181	A	12
AB-1-96A	2	S	.265	.023	.118	А	9
AB-1-150I	3, 6	A	.451	.108	.174	А	2
AB-1-166C	1-1/2	S	.085	.002	.213	A	5
AB-1-174	3/4, 4	s, c	.604	.141	.334	A	

Notes

- 1. S = Safeguards C = Containment RI = Reactor Internal Structure A = Auxiliary F = Fuel
- 2. A = Unrefined Response Spectra
 B = Refined Response Spectra

Gibbs & Hill, Inc.

11 Penn Plaza New York, New York 10001 212 760- 4438 Domestic: 127636/968694 International 428813/234475 A Dravo Company

August 24, 1984

GTN-69373

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project

JOB NO : DATE LOGGED: LOG NO . : FILE: CROSS REF. FILE

Gentlemen:

TEXAS UTILITIES GENERATING COMPANY COMANCHE PEAK STEAM ELECTRIC STATION G&H FROJECT NO. 2323 MASS PARTICIPATION

REF 1: GTN-69339 DTD 8/10/84 REF 2: GTN-69368 DTD 8/23/84

Attached for review by TUGCo Pipe Support Engineering (PSE) are the results of the Mass Participation Fraction Sensitivity Study for 15 of the problems from the referenced samples (14 from reference 1 and 1 from reference 2). These problems are as follows:

AB-1-88X	AB-1-71B	AB-1-165F
AB-1-19B	AB-2-52U	AB-1-166B
AB-1-27-1	AB-1-61F	AB-1-167E-1
AB-1-29U	AB-1-96C	AB-1-178B
AB-1-29Y	AB-1-156	AB-1-51C-1

Marked on the printouts attached are those support mark numbers which require PSE review. Note that only those supports identified with the mark numbers require review and that it is only these which see a load increase (Note - support called out regardless of the magnitude of the increase). The loads to be utilized by PSE from the printouts are those labeled "ZPA" next to "U" (Upset) and "E" (Emergency) to the right of each mark number.

*** · · **

Based upon a cursory review, Gibbs & Hill feels that these load increases should not present a problem, however during the PSE review for design load margin if such is not the case, Gibbs & Hill should be notified for the purpose of consideration of additional refinement. At the completion of the review it is requested that the attached printouts be returned along with PSE documentation verifying the acceptability of the load increases.

Very truly yours,

GIBBS & HILL, Inc.

&m manane

Robert E. Ballard, Jr. Director of Projects

REBa-HWMe:lc l Letter

CC: ARMS (B&R Site) OL

J. Finneran (TUSI Site) 1L + Printouts

D. Wade (TUSI Site) 1L G. Grace (TUSI Site) 1L

N. Williams (CYGNA CA) 1L

G. Bjorkman (CYGNA MA) 1L

KLScheppel MAVivirito, Rrallard, HwMentel SLim smilarano/0 , TDHawkins/H apinig, Outgoing

chler/CMJan, PTHuang,

International: 428813/234475 A Drave Company

GTN- 69339

August 10, 1984

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project Gen. Manager

Gentlemen:

N.WILLIAMS J.MINICHIELLI L.WEINGART 84042PF

TEXAS UTILITIES GENERATING COMPANY COMANCHE PEAK STEAM ELECTRIC STATION G&H PROJECT NO. 2323 MASS PARTICIPATION REF: GTN-69316 DATED 8-3-24

With regards to the outlined plan of action in the reference GTN, the initial problem sample (15 problems) has been performed. Attached is a listed of the problem numbers.

Note the following

- 1) Of the fifty (50) problems exhibiting a mass fraction under 10 percent (18x, 28y, 4Z) there are 39 unique problems. From these 39, 15 were selected for the first sample.
- These 15 problems cover a range of pipe sizes from 3/4 inch up to and including 24 inch.
- The auxiliary, containment, fuel and safeguard buildings are considered; with all original analysis being performed with unrefined response spectra curves.
- 4) A total of 320 supports are contained in these 15 problems.

TRANSITION BY THECOPICE 8-10-8

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e hog

The next set of 15 problems is being prepared.

Should you have any questions, contact either Henry W. Mentel (x6302) or Steve Lim (x5218).

Very truly yours,

GIBBS & HILL, INC.

In maran

Robert E. Ballard, Jr. Director of Projects

REBa-HWMe-SL:sce 1 Letter, 1 Attachment

cc: ARMS (B&R Site) OL

D. Wade (TUSI Site) 1L

D. Westbrook (TUSI Site) 1L G. Bjorkman (CYGNA Ma) 1L, 1A N. Williams (CYGNA Ca.) 1L, 1A

G. Grace (TUSI Site) 1L

Problem Number	Pipe Size(s)	Bldg(s)1	х	MF Y	Z	Curves2 A/B	Number of Supports	
AB-1-88X	4	С	.373	.003	.547	A		
/AB-1-19B?	4, 13, 1.5	C&S	.693	.071	.416	Α	5	
AB=1=27=17	16, 10, 6, 4	S	.818	.023	.717	A	52 46	
AB=1=29U/	6, 4	С	.170	.000	.328	A	46	
AB-1-29Y7	8, 6	С	.264	.010	.236	A	42	
AB-1-71B	6	S	.355	.097	.600	A	10	
AB-1-5207	2	A	.057	.082	.756	A	18	
(AB=1261F)~	10	A	.020	.332	.198	A	7	
AB=1=87C-1	10	F	.556	.099	.112	Α	6	
(AB-1-96C)	2	S	.240	.259	.045	A	9	
AB-1-1567	12	S	.356	.026	.284	A	5	
AB-1-165F7	3, 2, 3/4	S	.087	.000	.101	A	26	
(AB-1-166Bv)	1.5	S	.099	.001	.217	A		
AB=1-167E=1	24	S	.015	.208	.280	A	5	
AB-1-178B	12,6	A&C	.462	.058	.270	A	39	
			5/18	12/28	1/4		320	

Notes: 1) A= Auxiliary, C= Containment, F= Fuel, S= Safeguards

²⁾ A= unrefined response spectra B= refined response spectra

TEXAS UTILITIES GENERATING COMPANY

P. O. BOX 1002 · GLEN ROSE, TEXAS 76043

August 29, 1984

CYGNA Energy Services 101 California Street Suite 1000 San Francisco, California 94111

Attention: Ms. Nancy Williams, Project Manager

Subject:

Comanche Peak Steam Electric Station

Phase III Action Items.

Reference:

1) Phase III of the CYGNA Independent Assessment Program

2) TUGCO letter dated August 16, 1984, from J.B. George

(TUGCO) to N.H. Williams (CYGNA)

Dear Ms. Williams:

In reference 2, TUGCO committed to review and to provide a schedule of implementation for all recommendations proposed by CYGNA in Phase III of The Independent Assessment Program. These are provided below.

CYGNA Recommendations:

CYGNA suggested that the pipe stress group send all changes in support loads to the pipe support group.

TUGCO agrees that if any reanalysis or calculations are conducted by the Stress Analysis Group all load increases will be transmitted to Pipe Support Engineering. A change will be made to the 'as-built' procedure to ensure compliance.

CYGNA stated that pipe support designers note any simplifying assumptions when doing support designs.

TUGCO believes an engineer should list simplifying assumptions that are significant to the design of the support. The test of a well designed support is the capacity of the support to take the load, not the ease with which the design calculations can be followed. Nevertheless, we will reiterate to our engineering personnel the desirability of stating simplifying assumptions in the calculations.

CYGNA	1
JOB NO :	84042
DATE LOGGED:	8/3//84
LOG NO.:	#82
FILZ:	2.11 Bic. av
CROSS REF. FILE	21 pic. cri log

Distribution

N. Williams

J. Minichiello

L. Weingart

C. Wong

84042 PF

GBJOLLMAN

In addition to the above comments CYGNA had suggested making changes to six hanger drawings. All of the changes suggested except one had discrepencies of such a minor nature that no changes were deemed warranted. (The change was completed for drawing MS-1-002-004-C72K.)

If there are any questions with the above recommendations, please contact Ms. J. Van Amerongen at (817) 897-4881, ext. 500.

Very truly yours,

Texas Utilities Generating Co. Engineering Division

L.M. Popplewell

Project Engineering Manager

CC: J.C. Finneran

H. Harrison

D. Wade

J. Van Amerongen

H. Mentel

LMP/GG/bh

Distultation

9. Williams

M. Shulman

9. Minichillo

C. Wang

J. Weingaut

84042 PF

TEXAS UTILITIES GENERATING COMPANY

P. O. POX 1002 · GLEN ROSE, TEXAS 76043

100 CYGNA August 16, 1984 JOB NO : 9/12/84 DATE LOGGED: #85 CYGNA Energy Service LOG NO .: 2.1.1 mc. cr FILE: Smc. CR LOG San Francisco, Californias AddlibiLE

Attention: Ms. Nancy Williams, Project Manager

Subject:

Suite 1000

Comanche Peak Steam Electric Station

CYGNA Phase III

Independent Assessment Program

Dear Ms. Williams:

101 California St.

TUGCO has reviewed the CYGNA Phase III Independent Assessment Program Report and is providing the following general comments:

First, it is TUGCO's position that the Phase 3 review and resulting report should be independent of other CYGNA activities. Any reference in the Report to further CYGNA activity to be conducted as part of the Phase IV review is inappropriate. These are two seperate phases with independently defined activities. The documents that result should be comprehensive of the activities that were conducted for each phase (or previous phases), not activities that will or may be conducted. All references in the Phase III Report to Phase IV should, therefore, be deleted. Moreover, CYGNA's reference to TUGCO's objectives in conducting an Independent Assessment Program, and references to SIT or CAT reviews, are outside the scope of CYGNA's evaluation. TUGCO's position is that all discussions relating to the program objectives should be limited to the objectives of the Phase III Report only.

Second, the report does not present a consistent position regarding CYGNA's "programmatic" findings. CYGNA states on page 1-5, "Except as reflected in the PFR's, CYGNA did not detect any type of a programmatic breakdown on the Comanche Peak Project." Then, in the conclusion, page 5-20, CYGNA repeats the statement from page 1-5 and also states, "CYGNA did not find any evidence of a programmatic breakdown at CPSES."

It appears CYGNA intends to state that they found no evidence of a "programmatic" breakdown at CPSES, although some isolated instances of potentially unsatisfactory technical judgements which require further analysis to resolve, were noted. The cited references should be revised to state clearly CYGNA's position.

Third, in Appendix E, CYGNA did not specify which of its review criteria were TUGCO licensing committments, which were developed by CYGNA based on requests or suggestions from the ASLB hearings or which were based on CYGNA's own judgement. The additional criteria (including those derived from the Walsh-Doyle allegations) which were not specifically part of the CPSES design criteria resulted in several "unsatisfactory" marks on the checklist. However, none of these unsatisfactory checks resulted in a potential finding and only four were raised to an observation of which two were considered isolated and one was not considered a valid observation. This indicates that CYGNA believes TUGCO's design practices resulted in acceptable designs, irrespective of their satisfaction of additional criteria which were

derived from the hearings. Accordingly, we recommend that CYGNA provide specific conclusions regarding the validity of the additional criteria resulting from the Walsh-Doyle allegations and the unsatisfactory checklist items that resulted from these additional criteria.

Finally, all recommendations proposed by CYGNA will be reviewed for appropriate action. The exact action to be taken and the schedule of implementation will be provided to CYGNA.

In addition to the above comments, a number of specific comments were identified during the review and are included in the attached list.

Very Truly Yours,

J.B. George

Vice Pres./Project Gen. Manager

cc: David Wade George Grace Bill Horin John Finneran

ARMS

JBG/GEG/jf

ATTACHMENT

ITEM	(PAGE)	DESCRIPTION
1.	(1-4)	The time period encompassed by the IAP indicated requires further clarification. Calculations and drawings for pipe supports, for example, were provided to CYGNA with dates that preceded 1980.
2.	(Exhibit 1.4) (6 of 8) (p. 5-8) (p. 5-19) (PS-02, Att. A, 1 of 1)	PS-02. The main steam supports that were identified with bumper supports that were considered unstable were those without cinched U-Bolts, that is support MS-002-003-C72R and MS-004-003-C72R, on stress problems AB-1-23B and AB-1-23D, respectively. TUGCO considers these support configurations isolated because there were only four of these type of supports in the plant. This concern is identified throughout this report.
3.	(4-4, Exhibit 4.4)	The quality assurance program reviewed was TUGCO's, not TUSI's.
4.	(5-6)	Reference to observation PI-00-01 is incorrect in this discussion of the pipe support review. PI-00-01 is a pipe stress observation, not a pipe support item
5.	(5-13)	The sentence stating that "trending of the audits was performed by TUGCO on a quarterly basis" is unclear as to the exact action being considered. Reference should be made to Appendix G, not F.
6.	(Appendix C) (p. 13)	Item (d) did not include Mr. Wheaton as an affiant of this affidavit.
7.	(Appendix E) (5 of 19)	3.1. The correct code of record is ASME BPV Code section III, sub-section NF, 1974 Edition through Winter 1974 Addenda.
8.	(7 of 19)	No stiffness requirements must be met at CPSES in designing a class 2 and 3 pipe support. The applicable criterion is the 1/16" deflection guideline. The review criterion used here is not a requirement at CPSES and should be identified as an additional criterion that CYGNA developed. Furthermore, the criterion that was used is an acceptable industry standard.
9.	(7 of 19)	The criterion used at CPSES is that the allowable total diametrical gap for rigid frames is 1/8" plus or minus 1/16". In addition, the last sentence regarding proper thermal tolerances requires further clarification. It is unclear as to what CYGNA intends to state.

ITEM	(PAGE)	DESCRIPTION
10.	(8 of 19)	4.1.5-At CPSES, the criteria for rod hangers is that the maximum swing angle is equal to or less than 5°. Offsetting of the hanger is done to accommodate the pipe movement if the swing angle exceeds 5°, not if the total movement of the pipe is two inches.
11.	(8 of 19)	4.1.6-For snubbers, offsetting is done to accommodate pipe movement if the swing angle exceeds 5°, not if the total movement of the pipe is two inches. Midpoint of thermal travel is not required to be at the midpoint of the snubbers total travel.
12.	(11 of 19)	4.2-CYGNA states that the friction load is the product of a friction factor and the dead and thermal loads, but cannot be less than the dead load. TUGCO disagrees with this statement because the product can be lower than the dead load. Loads generated by thermal expansion include temperatures from maximum and minimum operating conditions, not the normal operating temperatures as CYGNA has stated.
13.	(Appendix G) (PS-01, 1 of 1)	Attachment E should read EE.
14.	(PS-08) (1 of 1)	The requirement that is referred in this observation is not part of the committed to ASME Code '74 Edition through Winter '74 Addenda.
15.	(Appendix J) (Note 1)	CYGNA states that TUGCO and NPSI committed to reviewing each welded attachment analysis against final pipe support loads. The committment made in item 3 of the June 8th Telecon is not being done as a result of a CYGNA request, but rather as part of the normal design practice performed by NPSI. (Note, the referenced communication report is in error.)
16.	(Note 12)	Typographical error in item (d) which reads 'T=1.45"' it should be 'T=1.25"'.
17.	(PS-05)	#25. Unsatisfactory is marked without explanation as to why the MS-46A specification was not met.
18.	(PS-024) (2 of 10)	#4. Gaps considered unsatisfactory, but no explanation was provided.
19.	(PS-036)	#4. Gap considered unacceptable without explanation.
20.	(3 of 10)	#6, #7, and #8. Support is a rigid support. These items are criteria for spring supports, rod hangers and snubbers and should be marked "N/A".

ITEM	(PAGE)	DESCRIPTION
21.	(PS-069) (1 of 10)	#2. This support is considered stable without "bumpers" since the U-Bolts are cinched. See comment 2.
22.	(PS-070) (1 of 9)	#2. This support does not have "bumpers", there- fore, there would not be any calculations for the "bumper" portion.
23.	(PS-071) (8 of 10)	#21. Engineers were directed to decrease the section properties by 5%, not increase them. All 12 effected supports were reviewed and are satisfactory.
24.	(PS-080) (2 of 8)	#4. Gap accommodation for thermal and seismic movements in the unrestrained direction is not applicable to a trapeze support. CYGNA has marked this criterion as unsatisfactory without an explanation.
25.	(PS-081) (9 of 9)	#24. Unsatisfactory mark indicated without explanation regarding whether the appropriate buckling lengths was used in the calculation.
26.	(PS-083) (10 of 10)	#25. Item 25 explanation refers back to itself.
27.	(PS-087) (2 of 10)	#1. Sketch is accurate. Plan elevation is center- line of pipe.
28.	(PS-089) (4 of 9)	#10. U-Bolt was satisfactory in accordance with ITT-DRS-137S which is an acceptable method for qualifying the U-Bolt.
29.	(PS-099) (1 of 8)	#2. U-Bolts are cinched; therefore, bumpers are not required for stability. See comment 2.
30.	(PS-099) (5,6, of 8)	#14, #15. These items regarding standard embedments and support attachments were considered unsatisfactory and the comments referenced Item 1 which stated that a higher applied load was used for design, a conservative assumption, please provide appropriate explanation of these unsatisfactory marks
31.	(PS-100) (1 of 9)	#1. Attachment 1 is part of an NCR and is used only as a reference document in the calculations. There are actually two welds 2-3/4" long, so the designer used a 5" weld length as a conservative input for the calculation.
32.	(PS-106) (7 of 9)	#18. No explanation of unsatisfactory mark regarding inclusion of inertia loads.

ITEM	(PAGE)	DESCRIPTION
33.	(PS-106) (7 of 9)	#20. Stiffness is not required to be determined for a spring support.
34.	(PS-107) (7 of 8)	Typographical error: checks are not aligned correctly.
35.	(8 of 8)	#24. Typographical error: checks are not aligned correctly.
36.	(PS-115) (5 of 9)	#11. Hilti-bolt was qualified with a factor of safety of 4.8, not 4.0.

Gibbs & Hill, Inc 11 Pern Plaza New York, New York 10001 212 760- 4438 Tester: Dumentic: 12763e/968694 International 428813/234476 HOTED SEP 1 7 1964 LANGUAN 2.1.1 mc. CR A Drews Company 2.1 mc. 42 Lo GTN- 69454 September 14, 1984 Texas Utilities Generating Company Post Office Box 1002 Glan Rose, Texas 76043 N. Williams Attention: Mr. J. B. George 9. Minichiello Vice President/Project Gen. Mgr. 84042 PF Gentlemen: M. Shulmar TEXAS UTILITIES GENERATING COMPANY COMANCHE FEAK STEAM BLECTRIC STATION GAH PROJECT NO. 2323 MASS PARTICIPATION REF 1: CYGNA LTR 84042.016 DTD 8/25/84 REF 2: GTN-69316 DTD 8/3/84 REF 3: GTN-69279 DTD 7/20/84 By copy of this letter to Nancy Williams of CYGNA Energy Services, please find attached the plan of action for the review of the overall effect of low mass participation fractions. This plan was verbally discussed with Namey Williams on Friday, September 7, 1984. should you have any questions, call this office. Very truly yours, GIBBS & HILL, Inc. of My marano REBL - HWMe: 1c Robert E. Ballard, Jr. 1 Letter Director of Projects CC: ARMS (BER Site) OL 1A D, Wade (TUSI Site) 1L 1A (telecopied) R. Jotti (Ebasco NY) 1L 1A N. Williams (CYGNA CA) 1L 1A G. Bjorkman (CYGNA MA) 1L 1A H. Levin TENERA Md) 1L 1A (telecopied) Approved M. A. Vivirito VP Power Engineering AND ROSE LEVELED MIL STANKED

SEISMIC ANALYSIS OF PIPING EFFECT OF HIGHER ORDER MODES/MASS PARTICIPATION

Plan of action for review of the overall effect of low mass participation fractions.

Based upon CYGNA's response (CYGNA letter 84042.016 dated August 25, 1984) to Gibbs & Hill's revised mass participation fraction sensitivity study (outlined in G&H letter GTN-69316 dated August 3, 1984) Gibbs & Hill has elected to revise its plan of action, with various expansions in detail and scope, as detailed below.

The pertinent facets of the plan are:

- A) That, as opposed to a sampling approach, Gibbs & Hill intends to perform a full scale evaluation of all of the large bore piping stress analyses originally in Gibbs & Hill's scope of as-built analysis (272 stress problems)
- B) These 272 atress problems will be selectively acreened and accounted for as detailed below.
- C) All subsequent analyses will be performed on Gibbs & Hill's IBM ADLPIPE Version D, which is automated to account for the effect of higher order modes (specifically the "ZPA" effect).

The detailed steps of the plan are as follows:

- Evaluation of the sample of five (5) problems established in GTN-69176 (utilizing unrefined response spectra curves for comparison with the original as-built).
- 2) Based upon the results of these first five (5) problems, develop preliminary plots correlating percent increase in support loads with percent mass participation.
- 3) Completion of verification of Gibbs & Hill's IBM ADLPIPE Version D.
- 4) Expand the sample of five (5) with approximately 30 representative problems (essentially those established in GTN-69339 and 69368), including a full-range variation in mass participation, pipe size, geometry, location, etc.

- 5) Using the total sample of approximately 35 problems, finalize the load increase versus mass participation plots. Based upon consideration of this data and support margin information, criteria will be developed including parameters such as minimum mass necessary to include 90 percent of response, line sizes/support types with sufficient margins, margins due to other loads/load combination, to screen all large bore problems and identify candidate supports requiring additional review.
- 6) Those problems/supports identified as requiring additional review would be re-run using ADLIPIPE Version D to evaluate the significance of potential load changes (runs would be made utilizing the refined response spectra curves).
- 7) If potential problem supports are identified, further refinement is possible in the applicable response spectra, i.e., use of localized curves relative to the specific piping and piping support location, as opposed to floor elevation response spectra.

The above plan will answer the global question of the effect of low mass participation on piping supports since all piping stress problems and their related supports will either be screened out as not being a problem or be fully evaluated.

Rec'd & 91 FROJECT FILE EBASCO

Two World Trade Center, New York, N.Y. 10048

September 18, 1984 3-2-17 (6.2) ETCY-1 Distribution

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W. Welliams

84042 PF

Miss Nancy H. Williams
Project Manager
CYGNA Energy Services
101 California Street
Suite 1000
San Francisco, California 94111-5894

Subject: Comanche Peak Steam Electric Station

U-Bolt Cinching Testing/Analyses Program-Phase 3 Open Items Additional Information as follow-up to Meeting of 9/13/84

Reference: Letter 84042.015 from N.H. Williams to J.B. George dated August 23, 1984.

Dear Miss Williams:

Enclosed please find the information which we agreed to provide as follow-up to the meeting of 9/13/84 at the Ebasco offices in New York.

The information is provided in the form of three attachments.

Attachment A is a numerical example of how the stresses in Tables H,I,N and O of the Affidavit were obtained. This example was discussed during the meeting and provides an answer to question 1 of the Attachment to the referenced letter.

Attachment B is a summary table of the maximum element stresses obtained by the finite element analyses. The stresses are given for all load cases and for all specimens analyzed. This table is necessary to perform calculations such as that given in Attachment A for all other pipes/U-bolt specimens.

Attachment C is a copy of the friction test data handed out during the meeting, which is provided in reply to questions 14 and 15 of the Attachment to the referenced letter.

Finally, we would like to clarify an item in your question 12. For the 10" sch 40 stainless pipe u-bolt specimen, your referenced letter quoted a value of preload of 3606 which is very low for the applied torque of 100 ft. 1bs. This value of preload was obtained from p. 66 of Attachment 1 to our Affidavit.

If you refer to p 64 and 65, articles 5.0 and 6.0 you will note that the creep test was performed right after the thermal cycling test without retorquing the

CYGNA	(10			
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DATE LOGGED:	9,20/84			
LOG NO.:	#87			
FILE:	2.1.1 mc. cr.			
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u-bolt ie the u-bolt nut torques placed prior to the thermal cycling test were maintained at the beginning of the creep test. The torque corresponding to the value of 3606 lbs is therefore not 100 ft-lbs but whatever torque remained at the end of the thermal cycling test which was begun with 100 ft. lb torque. Therefore, the 3606 lb preload should not be included in the variation of preload with torque.

Please call if you have additional questions.

Very truly yours,

Robert C. Iotti

Chief Engineer Applied Physics

RCI:ab

cc: Mr. D. Wade (TUGCO)

Mr. J. Van Amerongen (Ebasco/IUGCO)

Mr. R. Ballard (G&H)

Mr. W.H. Horin (Bishop/Cook)

J. Finneran (TUGCO)

W. Lapay (Westinghouse)

ATTACHMENT A

1. Please provide a detailed numerical breakdown of how the stresses in Tables H, I, N and O of the Affidavit (reference 3) were obtained The easiest way to show how the stress intensity is obtained is to refer to the figure VII-2 of Attachment 3 of the Affidavit which defines it as the maximum of either the absolute difference between the major principal stress or minor principal stress and zero, or the algebraic difference of the two principal stresses, and to apply this figure to an actual example. The example chosen is the 4" sch 160 pipe. For the elements having the largest circumferential and longitudinal stresses, the finite element analyses determined that the principal stresses are virtually identical to the circumferential and logitudinal stresses (see Attachment 3 of Affidavit at page 57). The longitudinal, circumferential, major and minor principal stresses for the highest stressed piping element of the 4" sch 160 pipe are given for both the inside and outside surfaces and for the maximum load case in the table of p. 58 of Attachment 3 to the Affidavit. These values are reported below:

	Long.stress(ksi)	Circum.Stress(ksi)		Stress (ksi) Minor
4" sch 160 inside	10.49	44.79	44.78	10.50
Outside	-26.65	-34.07	-26.63	-34.08

where the negative sign denotes compressive stresses.

A confirmation of the max. circumferential stress can be found in the table of page 71 of Attachment 3 of the Affidavit for element 627.

Note that on that table, there is no distinction regarding the surface at which the maximum stresses occur. For instance, the 44.79 ksi tensile circumferential stress occurs on the inside surface, while the -26.65 ksi compressive longitudinal stress occurs on the outside surface of element 627. To the local stresses computed by the finite element analysis one must add the longitudinal equation 9 pressure and piping moment stresses. These are available from the table on page 56 of attachment 3 of the Affidavit.

They are:

Longitudinal	Pressure Stress		4.8 ksi		
	Moment stress		+	12.146	ksi
	Moment stress		+	22.49	ksi

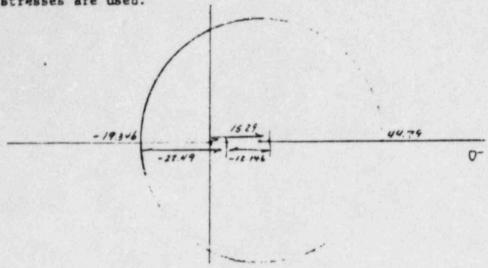
Adding the longitudinal pressure to the stresses previously tabulated we obtain:

Principal Stresses

	Major (Circumferential)	Minor (Longitudinal)
4" sch 160 Inside	44.79	15.29
Outside	-34.07	-21.85

To add the piping moment stresses to the longitudinal (minor principal) stresses, we choose the sign which will produce the largest stress intensity.

This is seen in a Mohr circle depicted below, where inside surface stresses are used.



Thus the total stress intensity is given by 44.79- (-19.346) = 64.136 ksi, which is the total stress intensity given on page 59 of Attachment 3 of the Affidavit or in table H of page 60 of the Affidavit.

For comparison purposes the stress intensity derived for the cutside surface is:

Maj. Princ. (Circumferential) stress = -34.07

Minor Princ. (Longitudinal) stress = -26.63 + 4.8 + 12.146 + 22.49 -56.466

The max. stress intensity is thus 56.47 ksi.

Using the alternative signs would have produced a stress intensity of 34.07 + 12.8 = 47.5 ksi which is lower.

As shown above, the highest stress intensity occurs on the inside surface.

To determine the primary and secondary stress intensities, several alternatives are available. The most straightforward determines the primary stress intensity from the principal primary stresses and derives the secondary stress intensity by subtraction of the primary from the total. For the example chosen we proceed as follows:

(i) The secondary portion of the circumferential stress is obtained as the stress due to thermal expansion by subtracting the circumferential stress due to preload + thermal given on page 59 of Attachment 3 of the Affidavit as -39305 psi, from the circumferential stress due to preload alone, which is given in the preceeding page as -26091 psi. These occur on the outside surface. The primary circumferential stress becomes -34.07 + 13.214= -20.856 ksi.

- (ii) The primary longitudinal stress is similarly derived by considering only the equation 9 piping moment stress, ie neglecting the equation 12 stress and subtracting the difference between the longitudinal stress due to preload + thermal and that due to preload only which equals -6.5 ksi. The longitudinal stress thus becomes -21.85 12.146 + 6.5 = -27.496 ksi.
- (iii) Thus the primary stress intensity is -27.5 ksi and the secondary stress intensity becomes 56.47 -27.5 = 28.97 ksi.

Similarly we obtain the primary and secondary stress intensities for the inside surface.

- (i) Primary circumferential 44.79 0.81 = 33.98 (10.81 is difference between preload + thermal and preload only circumferential stresses for inside surface and these do not appear in any table, but are available from the computer output)
- (ii) Primary Longitudinal = 15.29 + 12.146 -4.24 = 1.096 where again 4.24 is the difference between the longitudinal stress due to preload + thermal and that due to preload only.

Please note that the primary stress intensity is thus 35.1 ksi instead of the value of 31.6 reported on page 59 of the Attachment 3 to the Affidavit.

(iii) The secondary stress intensity then becomes

64.14 - 35.1 = 29.04 ksi instead of the 32.54 ksi reported.

The difference between the numbers occurred when inadvertently the outside secondary circumferential stress was subtracted from the inside total circumferential stress.

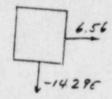
To explain the values appearing in Table N and O, we again will use an example and will employ the 4" sch 160 specimen as the example.

To determine the primary membrane portion of the U-bolt preload, push, and pressure stress, the stress state at the inside and outside of the pipe element surface is averaged. This stress state includes the mechanical longitudinal stresses due to the other (non-local loads).

As previously explained the primary circumferential stress on the outside surface is -20.856 ksi, and the corresponding primary circumferential stress on the inside surface is 33.98 ksi, with an average circumferential stress of 6.56 ksi.

The primary longitudinal stress on the outside surface is -27.496 ksi and that of the inside surface is -1.096 ksi, resulting in an average stress of -14.296 ksi.

The resulting stress intensity is then computed from the stressblock



and hence is equal to 20.86

3

Round off errors in this calculation results in the difference from the 20.99 reported in table N.

The values listed in Table 0 are Equation 10 values and include primary plus secondary stress intensities derived on the basis that loads which derived previous: for Table I are amended to subtract the portion of the stress that is due to preload.

For the 4" sch 160 inside surface the circumferential and longitudinal stresses due to preload alone are given in the table on page 68 of Attachment face they are -26.091 and -13.47.

The stress block for the outside surface can be modified to read

With the stress intensity (eq. 10) being -39.0 ksi.

The stress block for the inside surface is then

ATTACHMENT B

Tabulation of Inside and Outside Surface maximum stresses for all Load Cases

Londing strest	C.C.	14" Ksi		10: 405 K		10°-80 Ki		32 MSKsi	
Lond CASE	JOHNEE	Long	CiRC	LONG	Circ	Lows	CRC	Long	CIRC
Prilond (P)	Tuside	8.18	21.76	8.83	38,7/	8.19	23.02	1.80	3.57
Freinno	Outside	-1347	-24 0	-2525	-48.53	-16.27	-29.89	-3.4/	-4.92
P+ Theemal (TH)		1242	32.57	11.16	48.08	8.74	27,05	8.35	15.5
ra Micemai (in)	Outside	1-19.97	-39.3	1-33.72	-6064	1-19,41	1-34.40	-14.81	-21.5
PATH + Pressure P		12 85	42.91	11.59	58.59	19.10	132.96	11.33	34.3
41H 411E3500C41	Outside	-2018	-31.41	1-34.43	-5465	1-19.95	7-30%	19.85	-15.5
PATH+PR+Push	INSIDE	1022	WX	10.78	72.71	10.24	143.15	19.58	147.1
ורנותיוגיוטטא	Outside	-2615	-34.0	-48.50	-734	-30,27	1-4438	-31.01	1-34.1

Subject: Priction Test Preload Data

Presented Delow are the preloads measured at the torque values during the friction tests. Data report the sum of preload in the two U-bolt legs. Use of value indicated to approximate preload per leg.

Thus the stress intensity equals 50.60 ksi. The value reported on Table 0 is 50.8 ksi.

TORQUE	PRELOAD				
10 ft-1b	1358 1b				
20	2209				
30	3459				
40	4131				
50	4804				
60	6661				
70	8743				
80	9448				
90	9736				
100	10920				
110	12458				
120	13354				
130	15373				
130	16654				
130	17934				

32" CS

TORQUE	PRELOAD
40 ft-1b	736 1b
60	603
100	1981
120	1507
140	2583
180	3618
200	6503
220	7493
240	8785
700	28422
1170	34019

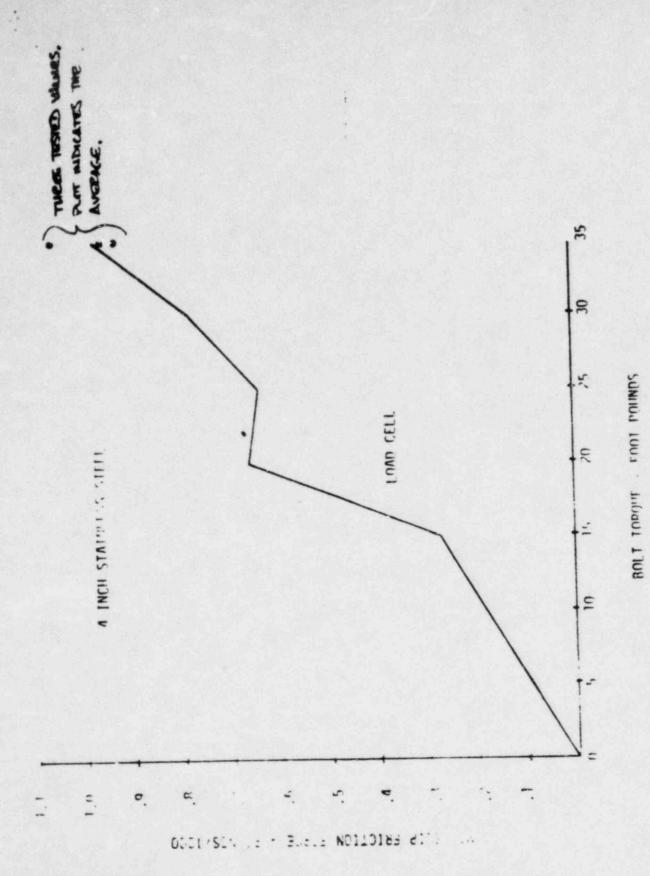


FIGURE 9



FIGURE 10

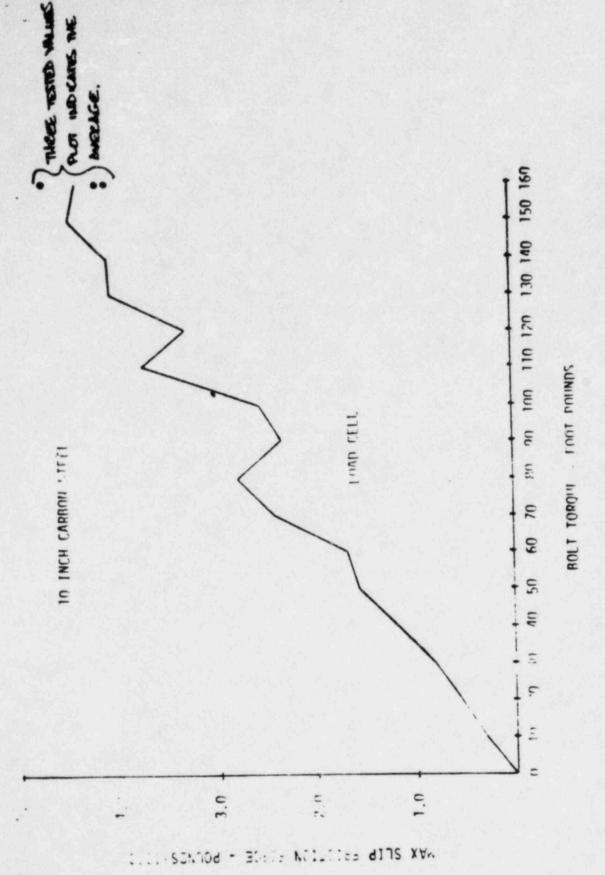


FIGURE 11

BOLT TORONE - FOOT POUNDS

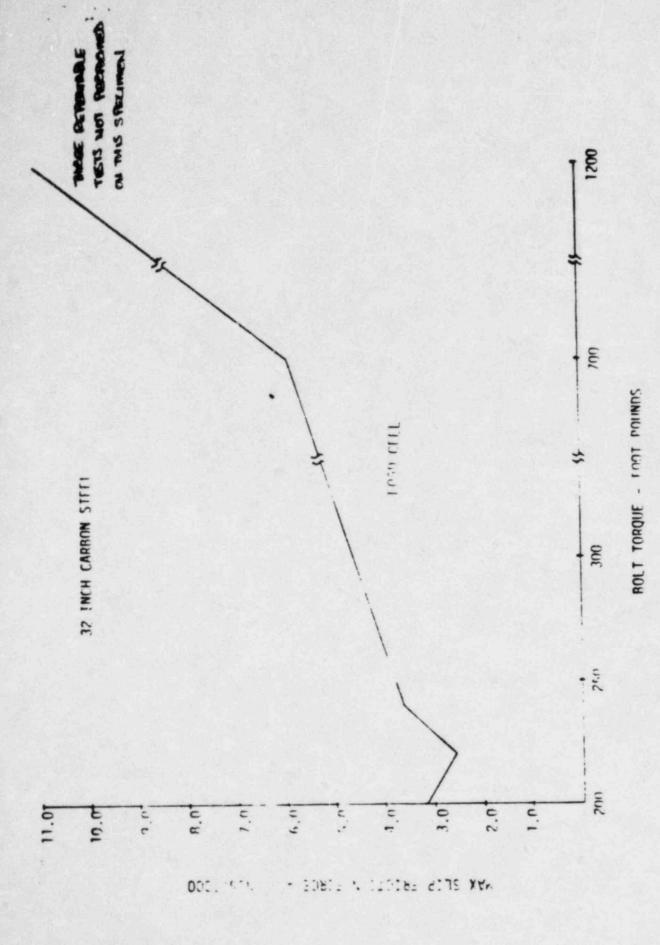


FIGURE 12

PROJECT FILE

11 Penn Plaze
New York, New York 10001
212 760- 4 4 3 8
Telex:
Demaylic: 127636/988994
Nethanisonal: 428613/234475
A Dravo Company

Distibution:

NWilliams

JMinichiello

August 17, 1984 Lukingart

Froject File

84042

GTN- 69359

Texas Utilities Generating Company Post Office Box 1002 Glen Rose, Texas 76043

Attention: Mr. J. B. George

Vice President/Project G

Gentlemen:

CYGNA	No
JOB NO :	84042
Doc Mgr.	- 8/20/84 # 75
FILE:	2.1.1 mc. ca
CROSS REF. FILE	2.1 Inc. UR Log

TEXAS UTILITIES GENERATING COMPANY
COMANCHE PEAK STEAM ELECTRIC STATION
G&H PROJECT NO. 2323

TRANSITION JOINT SIF AT EQUIPMENT NOZZLE CONNECTIONS

REF 1: GTN-69338 DATED 8/10/84 REF 2: GTN-69303 DATED 7/31/84

By copy of this letter to Mancy Williams of CYGNA, please be advised that the subject G&H expanded review has been completed and all issued analyses are found to be acceptable.

Gibbs & Hill has reviewed all the as-built analysis problem flow diagrams in the G&H New York scope (272) and has determined that 119 analyses do not contain spripment and therefore do not require that an SIF be considered. ADLPIPE computer input, output and calculation book reviews showed that 100 analyses considered stress intensification factors at the equipment nozzle connections and are acceptable.

Additional hand analysis was required on 44 problems which were determined to be acceptable. The results are contained in calculation 2323-EQ-SIF to be issued by August 22, 1984.

For expediency, equipment and weld end preparation detail drawings were not reviewed. Had they been utilized, the magnitude of the additional hand analysis performed might have been minimized.

The remaining nine (9) analyses were found to be acceptable, either by correlation, weld type or by the use of flexible type connectors (see reference 2).

Checking of the calculation is now in progress. A copy of the calculation will be provided upon completion of the checking.

or F. A. Colucci (x5203).

Very truly yours,

GABBS & HILL, Inc.,

Robert E. Ballard, Jr. Director of Projects

REBa-HWMe-FAC:1c

1 Letter

CC: ARMS (BER Site) OL

D. H. Wade (TUSI Site) 1L (telecopy)

N. Williams (CYGNA CA) IL (telecopy)

G. Grace (TUSI Site) 1L

TO: DOCUMENT CONTROL

FROM: S. B. Burwell x 27563

SUBJECT: Cygna Review (Phase 3) Comanche Peak

Attacked is the following document:

October 3,1984 - 84042,030
Responses to Cygna Design Control, Pipe Support
and Pipe Stress Questions.
Cygna (Williams) to CASE (Ellis)