

DEPARTMENT OF THE ARMY DETHOIT DISTRICT. CORPS OF ENGINEERS BOX 1027 DETROIT, MICHIGAN 40231

2 4 MAR 1980

NCEED-T

SUBJECT: Interagency Agreement No. NRC-03-79-167, Task No. 1 - Midland Plant Units 1 and 2, Subtask No. 1 - Letter Report (INTERIM)

THRU:

Division Engineer, North Central ATTN: NCDED-Z (James Simpson)

TO:

U.S. Nuclear Regulatory Commission ATTN: Dr. Robert E. Jackson Division of Systems Safety Mail Stop P-314 Washington, DC 20555

1. The Detroit District team which provides geotechnical engineering support to the NRC has reviewed most of the available documents concerning plant fill at the Midland Plant. The team met with the NRC staff, the applicant Consumers Power Company (CPCO) and its consultants, participated in a site visit to observe site conditions and discussed the proposed remedial measures planned for Category I structures placed on plant fill. Since final design computations have not yet been provided for remedial measures, we feel it would be most expeditious for all parties to submit this INTERIM letter report to provide our initial event of the femetial measures and recommend procedures to settle any unresolved discuss, conderns and questions. The Bistrict also feels it is important to accomplish a thorough review which just a cannot be done quickly.

2. The Bistrict's investigation to date has been centered mainly around the proposed remedial measures or other action for the Category I structures placed on fill materials. The review to date includes at least a preliminary look at all data received through Amendment 74. The initial indication of issues unresolved to date fall under the following four general types with subtopics as noted:

I. Soils:

a. Groundwater

- 4. 5. Borings and testing
- b. d. Settlement/Consolidation
- Ed. Seisnic comes guturtmeral Conterm

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SUBJECT: Interagency Agreement No. NRC-03-79-167, Task No. 1 - Midland Plant NCEED-T Units 1 and 2, Subtask No. 1 - Letter Report (INTERIM) cl. e. Attace structures constructed on fill not covere 4. f. Quality control II. Remedial Measures for Category I Structures on Fill a-Dewatering 2 b. Diesel generator building d. Service water building G. d. Borated water tanks d. d. Underground diesel fuel tanks . Underground utilities - g. Aux. building - Feedwater valve pit -III. Geology 2. Genan www Depth to rock b. Layer formation c. -- Fault and seismic history d. Crustal Tebound e .- Interpretation of borings, test pits, lab tests, etc. b. Origan rivermere Livery Miscellaneous or General. IV. and anditional date 3. Questions, uncertainties and daresolved issues are stated in the following pages. These are in addition to the responses and to be received from the applicant concerning additional design support information to the 10 CFR 50.54 (f) questions concerning structures on plant fill. The source of each question, concernent issue is indicated at the end. of one change to tablet I. Soils Q. & Borings and testing

(1) Who and what are the qualifications of the persons(s) who classified samples of all borings, driller or geologist? Were samples tested in a lab? Are samples still available? Where are the results, many appear to be missing? Source - site visit, various documents and general concern.

(2) Are final fill elevations available at completion of fill placement and prior to construction? Were additional borings taken prior to start of construction? If so, where are they? Source - Incl to CPCO letter 2 Nov 79.

(3) Have all investigative borings for the plant fill problem been completed? If not, what are the locations of the remaining borings to be taken? Source - site visit 28 Feb 80.

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· SUBJECT: Interagency Agreement No. NRC-03-79-167, Task No. 1 - Midland Plant Units 1 and 2, Subtask No. 1 - Letter Report (INTERIM)

c. Settlement/Consolidation

(1) Why have allowable soil bearing stresses not been discussed? What are the related soil spring constants so that settlement vs. load is quickly discerned? Source - site visit 27 Feb 80.

(2) Has the bearing value of the glacial till been determined and have settlements been estimated for this bearing stratum based of all building loads, particularly the reactor building? Source - site visit 27 Feb 80.

d. Seismic concerns

(1) New soil properties or new materials used for backfill should be used in the revised seismic analysis for determination of structural adequarcy. Has this been accomplished and documented? Source - Question 262.13, Q&R 2.5-22.

e. Misc. structures constructed on fill not covered in Paragraph II below.

(1) Sand pockets have been noted in cooling pond dikes which lead toward the river. What are the adverse impacts (groundwater piping, leading to dike failure)? Was the dike properly compacted? Source - general concern.

(2) Have all structures on fill be investigated for settlement? If not why not? Have all buildings on fill been checked for cracking? Source - interim SER.

(3) The design of the Water Service Building retaining wall is critical and partially category I. This design should be provided for CofE review. What is the cause of the wall settlements noted during the 27 and 28 Feb 30 site visits? Source - site visit 28 Feb 80.

f. Quality control

(1) Why are there so many shrinkage cracks (assuming these are shrinkage cracks)? Is this simply poor quality control? Will cracks be repaired in same? Source - site visit 23 Feb 80.

II. Remedial Measures for Category I Structures on Fill

a. Dewatering

(1) Why not utilize a slurry cutoff wall or trench across the primary recharge zone near the service water building in addition to pumped wells plarned? Use of clay slurry wall would provide positive cutoff. Source - site visit 27 Feb 80.

NCEED-T SUBJECT: Interagency Agreement No. NRC-03-79-167, Task No. 1 - Midland Plant Units 1 and 2, Subtask No. 1 - Letter Report (INTERIM)

(2) What is the dewatering well gravel pack design? Does it vary with soil layer type? Source - site visit 27 Feb 80 (dewatering).

(3) Will groundwater piping occur from cooling pend to well points over time? Any preventative measures proposed to stop this? What about piping along piles, building foundations or caissons? Source - site visit 27 Feb 80 (dewatering).

(4) Will weep holes in retaining wall at the service water wilding be plugged since these are no longer necessary with the dewatering anned? This could be a likely source of possible groundwater piping in the future. Source - site visit 27 Feb 80 (dewatering).

(5) What are the test results concerning incrustation of the dewatering system as well as fines removal (additional settlement) concerns? Source - site visit 27 Feb 80 (dewatering).

(6) What is the final dewatering plan; number of wells, spacing, location, depth, diameter, pumping rates, recharge rates, back-up systems, etc? Source - site visit 27 Feb 80 (dewatering).

(7) Are there any known problems of operations of the dewatering system due to presence of gas pockets in the area? What about fire/explosion hazards with the electrical controls? Source - general concern, prior experience.

(8) Has the need for localized dewatering in sand fill lenses been analyzed for structures other than the D.G. building? Source - site visit 27 Feb 80 (dewatering).

. (9) Upon reaching a steady state in dewatering, a geophysical survey should be made to confirm the position of the water table and to insure that no perched water tables exist. Source - site visit 27 Feb 80 (dewatering).

b. Diesel generator building

(1) Provide additional verification that surcharge loading has indeed solved the settlement problem. Additional borings, if taken, should indicate higher blow counts when compared to borings taken prior to application of pre-load. Settlement analysis should be made on samples from new borings. The drop in groundwater levels, implying heavier soil unit weights, and diesel plus seismic vibrations should be considered in the settlement and seismic analyses. Source - Q&R 2.5-22 and site visit 27 and 23 Feb 80.

(2) How was sand surcharge placed inside D.G. building? Was it compacted? How was it removed? What was in-place unit weight of sand used?

24 MAR 1980 Luceragency Agreement No. NRC-03-79-167, Task No. 1 - Midland Plant Units 1 and 2, Subtask No. 1 - Letter Report (INTERIM) emoval of sand by heavy equipment cause cracking? Source - Q&R 2.5-21, (3) Establish cracking history as a function of construction and rge activities. Source - site visit 27 Feb 57 (Anal. Investigations). (4) What is the explanation of the "hump" in the settlement en the two western-most generator bays on the south side of the building? (5) Since certainty that the surcharge provided proper compaction not yet been satisfactorily that the surcharge provined proper compactions confidence to the structure could be setablished by e - Incl. to CPCO letter 2 Nov 79. not yet been satisfactorily demonstrated to the Corps, and as a nedge inst liquefaction, confidence in the structure could be established by (6) Are post surcharge borings and related test data available? erpinning. Source - site visit 27 Feb 80. so, these should be provided. Source - CPCO letter 2 Nov 79. (7) Why is there no floor in D.G. building? Will a floating slab V(8) Further, explanation of the 1 1/2" of additional settlement e placed later? Source - site visit 28 Feb 80. earthquake, 3/4" static load and 1/4" dewatering) is requested. The eartnquake, 3/4 static load and 1/4 dewatering) is requested. The 1/2" additional settlement due to earthquake loads must be reviewed by WES. Source = 048 2.5=34 0362-17 (1) Corrective piles are to be 100 ton piles. How is this load Source - Q&R 2.5-34, Q362.17. (1) Corrective piles are to be 100 ton piles. How is this load to be developed and established, by tip elevation or blow count? Discuss the pile load test(s) to be performed. Resolve the problem of possible differential settlement that could occur between the pile supported and and pile load test(s) to be performed. Resolve the problem of possible differential settlement that could occur between the pile supported end and the contine placed on fill. Source mains while 28 Feb 80 (Daviseon differential settlement that could occur between the pile supported en the portion placed on fill. Source - site visit 28 Feb 80 (Davisson (2) What computations show that sufficient lateral stability is (2) What computations show that sufficient lateral stability is what 28 Feb 80 (Dawteeon presentation) (3) Would building lateral stability be improved by plugging the presentation). (3) Would building lateral stability be improved by plugging the site visit 23 Feb 80 (Davisson presentation). site visit 23 Feb 80 (Davisson presentation). - (1) The soil test investigation repose at the tank farm should be provided for our review. Bearing capacities should be determined from plate load tests. The vield point of the foundation should be determined. Source our review. Bearing capacities should be determined from plate The yield point of the foundation should be determined. Source load tests. The yield point of the for site visit 27 Feb 80 (remedial work). 5

. SUBJECT: Interagency Agreement No. NRC-03-79-167, Task No. 1 - Midland Plant Units 1 and 2, Subtask No. 1 - Letter Report (INTERIM)

(2) Why has the ring foundation cracked? Is this crack in the area of the reported broken air line? The diagonal crack did not appear to be a shrinkage crack. Source - site visit 27 and 28 Feb 80.

(3) Why not increase the test load for the tank by a surcharge in addition to filling the tank with water? Source - Interim SER.

(4) Since applicant agreed that broken air line may have degraded the foundation material, the tests taken in this area must be conclusive. Source - NRC 28 Aug 79 Memo.

(5) What are the residual settlement predictions and the consequences thereof? Source - Interim SER.

e. Underground diesel fuel tanks

(1) What are the settlement predictions on these tanks and will X these then function properly? Any differential settlement expected? Source -Interim SER.

(2) Does differential settlement reduce the fuel storage X capacity? If so, how much? Source - general concern, Interim SER.

(3) Are these tanks designed against "bouyancy?" Source -Interim SER.

f. Underground utilities

(1) Why not inspect the interior of water circulation piping with video camera with sensing devices to show pipe cross-section, infiltration and slope? Source - site visit 27 Feb 80.

(2) Have electrical duct banks at D.G. building been damaged? Have these been inspected after it was shown they were severly loaded? Direct observation of the western-most duct would seem appropriate and easy at this time. Source - site visit _7 and 28 Feb 80.

(3) Have all Category I underground utilities, ducts, pipes etc. been profiled? This would seem to be the only positive way to be certain no damages to pipes or utilities have been sustained. What about corrosion to buried pipes or chemical attack of concrete pipes underground? What about stress induced by differential settlement? Are all stress levels below allowable and what about added stress due to res. 'ual settlement? Source site visit 27 Feb 80 (evaluation of piping).

(4) Will all utility settlements be monitored throughout the plant lifetime, particularly at connections? Source - site visit 27 Feb 80 (evaluation of piping).

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NCEED-T

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(5) Passing a "rabbit" through conduits is not a suitable safeguard or insurance that conduits are undamaged. Source - Interim SER.

(6) During our site investigation on 28 February 80 it was noted that the "rattle space" had been reduced at several buildings. How will these defects be corrected? Source - site visit 28 Feb 80.

(7) The category I pipelines (outlets, inlets) for return water at the emergency heat sink could be covered by a slope failure during a seismic event. The applicant should anlayze the sideslope to determine if a sufficient factor of safety exists. Source - site visit 27 and 28 Feb 80.

g. Auxiliary building - feedwater valve pits

(1) Seismic analysis of this area is needed concerning change from fill to lean concrete. Source - Questions 362.13 Q&R 2.5-23.

II. Geology

c. Fault and seismic history

(1) Canadian faulting of major magnitude exists at Sault Ste. Marie and Sudbury, Ontario. Why were these not considered in the FSAR? Source - FSAR Figure 2.5-27.

e. Interpretation of borings, test pits, lab tests, etc.

(1) Many undisturbed soil samples were taken, yet no test results or reports are available for many of these. Why not? Source - general review of documents.

IV. Miscellaneous or General

(1) Why are there so many shrinkage cracks, especially in plant area structures placed on fill? This appears to be more than a coincidence. Source - site visit 28 Feb 80.

(2) Will C.P.Co. consultants Peck, Davisson, Gould & Hendron submit summary statements to NRC concerning their presentations at the 23 Feb 80 site visit? Source - site visit 28 Feb 80.

(3) Cooling pond dikes have been repaired due to erosion. This dike design should be provided for CofE review. Source - site visit 28 Feb 80.

(4) Will the applicant provide minutes of the 27 and 23 Feb 80 meeting?

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(5) The entrance road to the plant appeared to be below cooling pond elevation. Would access to the plant be impaired for emergency vehicles in the event of a dike failure? Source - site visit 27 and 28 Feb 30.

(6) What provisions will be made to insure the dewatering system will be maintained in proper operating condition? Source - site visit 27 Feb 80 (dewatering).

(7) Some of the cracks noted appear to be enlarging with time. The width of these cracks would be already in excess of a shrinkage crack. Source - site visit 27 and 28 Feb 80.

4. The District's recommendations concerning questions, uncertainties and unresolved issues presented above are given when possible and appropriate below in a like numbered paragraph:

I. Soils

e. Miscellaneous structures constructed on fill not covered in Paragraph II below.

(2) All structures, including utilites should be checked for settlement and cracking.

f. Quality control

(1) Undertake a comprehensive analysis on cracked structure. Statements that all cracks are due to shrinkage or do not effect structural integrity are not sufficient.

II. Remedial Measures for Category I Structures on Fill.

a. Dewatering

(1) Consider benefits of using clay slurry cutoff wall in conjunction with pumped dewatering.

(3) Analyze possible groundwater piping along the paths indicated.

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(4) Analyze pros and cons of plugging weepholes.

(5) Consider a control panel in control tower area to indicate plant groundwater level in the critical plant areas so that monitoring can be easily accomplished.

2 4 MAR 1980

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b. Diesel generator building

(1) Take additional borings and tests to prove surcharge worked.

f. Underground utilities

(2) Conduct an visual inspection of at least one (the westernmost) electrical duct bank at the D.G. building.

(3) Investigate any category I utilities not investigated.

(5) Provide additional assurance the category I utilities have not been over stressed.

(6) Establish minimum rattle space criteria and restore as necessary.

(7) Analyze outlet pipes located in heat sink pit side slopes for seismic event (SSE & OBE).

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5. If you have any questions concerning this interim letter report, please contact Mr. N.A. Gehring at FTS 226-6793.

POR THE DISCHICT ENGLISHER:

P. MCCALLISTER Chief, Engineering Division

DEPISTENT OF THE ARTT RETADIT DISTRICT, CORPS OF REGIMERS P. O. BOX 1027 DEFECT, MICHIGAN 48231

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SUBTOT: Interspeacy Agroament No. MIC-13-79-167, Stricethly Letter

Dr. Inhert I. Jackson 6.5. Fuclear Repulatory Commission - Division of Systems Safety Sail Stop 7-314 Vashington, DC 20353

Cear Dr. Jacksont

This is the fourth of the bi-monthly letters required by subject interagener Agreement. The states of the items requested in the agreement is provided in the following paragraphe.

Efforts Completed Paring the Period 20 May to 20 July 1965

Midland. The seismic analysis was received from the USAS Materwars Superimot Station (NTS) on 3 June 1920. ISAT revision no. 28 was received on F June 1980. On 14 May and 20 June 1960, draft latter reports more informality furnished to Bau and per Morth Cantral Division (NCD) office for sensent. The letter report, subtask no. 1, was called to MAC through SGD on 7 July 1980.

Bailing. On 27 May and 15 June 1980, draft latter reports were informally formished to MSC and MCD for commut. On 10 July 1980, the latter report, sustant no. 1, was miled to MRC through NGL. On 29 and 30 May 1980, MINSCO responded to RCP's and requests for information Joi.01 through 362.79 and 362.30, respectively were received. On 3 June 1980, a Jraft clarification statement was received from MRC to further define the Detroit District work effort required under subject agreement. John Grundstron, Bailly Lead reviewer for Detroit District, was in Machington for discussions with the MRC and to attend a Z-day pile driving seminar.

Anount of Funds Executed

Site		From 1 May 80 to 30 Jam	30 Curvlative
Balland manually	Total	\$14,695.40 13,175.00 \$27,570.54	\$ 61,957.80 42,404-35 - 104,342-15
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SHAUECT: Interagency Agroement No. TRC-C1-79-167, Bi-Moathiv Letter

Problems Esconstered of Articipatad.

We letter of 30 Jone 1980 to 200, Division of Contracts, indicates that at Mer Sistrict's rate of aspenditure in Parch, April and May 1980 (appresimately \$20,000/month), that as stditicael \$40,000 over and shows the 1144,000 Sweinste would be meded through the end of 27 80.

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Hidland. Funding aft acceptance, subtast no. 1 (latter report) of task 1 (attland) is complete.

Sailly. Sunding WC accestarcs, Thousand and 1 (letter report) of tast 2 (Sailly) is ecupiece.

Plane for the West Assorting Pariod 21 Jaly 80 - 21 September 195.

midland. Lattiete work on the draft Safety Ivaluation Report (Sam).

Bailty. Initiate wort on traft SEA.

General. A schedule of subtask activities based on the District's procress Entrang' 30 June 1980 is actached as Incl 1 for your information.

Sincerely,

1 isel Schedule

MCDED-G

Chief, Ingineering Division

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Liquifaction Potential

- 1. ER 1110-2-1806 (30 Apr77) 600d outline of found investig. Says " In-situ & Dr should be determined. For dyn. and., make downhole, cross hole & surface geophysical tests to est. in-situ compression & shear moduli. Determine critical void ratio. PZ probe may be used in sate material to assess dilative or contractive in-situ properties.
- 2. Seed ASCE proceedings Apr 72. The crux of this orticle is the: Liq. can occur in dry or moist materials as well as sat.
- 3 Seed Winterforn & Foug, pg 72:.
 (a) Liq can occur in <u>any zone</u> at <u>any depth</u> (implied that the the source of the small.)
 (b) Liq of upper layers may result from induced Liq of depth which causes upward by. gradients if conditions are right.

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Name: Hari Narain Singh

Address: 34174 Koch Avenue Sterling Heights, Michigan 48077

Professional Licenses:

AX5-12 18-50 DATES-187 4 "SAND OVER MUCH (MININAT, FLA. P/D)

Registered Structural Engineer - Pennsylvania - 1970, 15552E.
 Registered Civil Engineer - Fennsylvania - 1978 15552E.

Education:

(1) B.S. (Civil) - 1956 - University of Patna, India
(2) M.S. (Civil) - 1969 - University of Colorado, Boulder, U.S.A.
Completed 30 additional semester hours beyond M.S. degree.

(3) (Geotechnical) - Wayne State University DETROIT (Presently working for Ph.D. degree)

Professional Experience:

A. October 1978 to Present: Civil Engineer, U.S. Army Corps of Engineers, Detroit, Michigan.

B. April 1978 to September 1978: Civil Engineer (bridges & foundation) Arizona State Highway Department, Phoenix, Arizona.

C. March 1970 to March 1978: Civil Engineer, Pennsylvania Department of Transportation, Franklin, PA 16323

D. September 1965 to September 1969: Graduate student and Research Assistant, University of Colorado, Boulder, U.S.A.

E. May 1959 to July 1965: Assistant Professor of Civil Engineering, Department of Industries, Government of Bihar State, India. Posted at the Ranchi School of Engineering (1959-1961) and the Regional Institute of Technology, Jamshedpur, India.

F. April 1958 to April 1959: Assistant Civil Engineer, Government of India (Tripura Administration), India.

G. July 1956 to April 1959: Engineer Assistant (Civil), Government of Bihar State, India.

<u>Summary of Experiences</u>: Twenty-four (24) years experience in civil engineering activities which include teaching, design, construction and maintenance. Completed design and reviewed design for more than fifty (50) bridge structures and their foundations. Carried out soil explorations and foundation investigations for structures.