## BEFORE THE ATOMIC SAFETY AND LICENSING BOARD

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VOLUME II

## APPEARANCES :

SHAM, LINCOLN \& BERLE (By Mr. Ronald Zamarin and One First National Plaza Chicago, Illinois 60603 and

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212 West Michigan Avenue Jackson, Michigan 49201 Appearing on behalf of Consumers Power Company

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Appearing on behalf of the Nuclear Regulatory Commission

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# Cross Examination by Mr. Zamarin, continued 

## Afterncon Session

Cross Examination by Mr. Zamarin, continued



HARI NARAIN SINGH, having

CROSS EXAMINATION (continuing)
BY MR. ZAMARIN:
Q It is now eleven o'clock. We delayed beginning this morning because there were certain administrative matters with regard to discovery and scheduling that we have been attampting to work out and, of course, you understand, Mr. Singh, that you are still under oath and still sworn to tell the truth today?
A I do.
Q All right. I have been advised by $M r$. Paton that you have a clarrification to an answer that you gave yesterday that you would like to make, and if you just want to go ahead and tell us what that is, please?

A When I work in Pennsylvania, Departrent of Transportation, then I design bridges and foundations and at the same time I reviewed. Okay, in Arizona I supervise a crew who is doing foundation investigations, so I watch sample taking.
Q And that was disturbed sampling?

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2 A Yes.
3 Q I have here what has been marked Consumers Exhibit Number 4 for identification as of today's date, and I'd like to show you and Mr. Paton that Exhibit and I am going to ask you af you have had an opportunity to review it, to tell me what 1 is.

A Yes. I prepared it.
Q You did it? What is it?
A It is a computation of the pressure, computation of the pres sure below the foundation, the footing level along the depth of the fill material because of the surcharge load.
Q Okay, and why did you prepare that document?
A Because I wanted to see how the pressure varies below the footing.

Q Wat did you see?
A I did it and saw it varies.
Q All right, and of what significance is that variation to you:
A Well, that it will affect the settlement.
Q How?
A Because the settlement depends upon the pressure.
Q I mean "how" in a quantitative sense?
A If there is less pressure there will be less settlement.
Q What conclusions did you reach on the basis of the calculations and the analysis of the information in Exhibit Number 4

A The conclusion was I found the settlement pressure. I didn' calculate the settlament.

## Q Why not?

A It was not required. I don't know how the piezometers are.
Q Why did you do the calculations in Exhibit 4 if you then didn't do anything with them?

A Because it was to be compared with the one exhibit we were given.

Q Okay. Now we are getting somewhere. It was to be compared with this Figure 2 ?

A What is that, let me see that figure.
Q Wait a minute, I have my notes on it. Do you have an unmark copy?

It is of View Graph Number 5 as attached to Consumers Exhibit Number 12, for identification, as of October 15th, 1980 , and that's the deposition of Joseph Kane and I have got here a clean copy, I am getting a clean copy.

All right. I have what I am going to gi to you, and it is Figure 2 to the December 14, 1980 submitta by Consumers Power Company, and that submittal was Consumers Exhlbit Number 8, for identification, as of October 15 in th Kane deposition, and this Figure 2 was included with that an itwes also a part of Exhibit Number 12 of that same depositi which was a Joe Kane document.

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Is that the graph that you saw that then prompted you to do the calculations contained in Exhibit 4 ?

A It looks like that.
Q Okay, and what was it that you were trying to either verify or check or look at with regard to that Figure 27

A Actually the dirt loads shown here in this graph was much mori than I have got in one of your tables, not tables, my correction, it is in question number four -- I am not sure, but I got a table in which it is given at the time of surcharge the dead load weight at 2.2 keps.

Q All right, and what is shown on this graph as far as dead loac
A Eere I found much more than that.
Q What?
A Dead load.
Q What did you find?
A Well, I can measure here, one, two, almost three keps.
Q And did you ever find out why in a table that was 2.2 keps and why here that that load is over three?

- A Well, you have to fumish this. I don't know. Whatever information I have got I have to do it according to that.

Q Did you ever find out what you perceived to be what the cause was for which you perceived to be a difference in dead load?

A I don't see anything.
Q Does that mean you haven' $t$ found it?

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A I have not found out.
Q So you still don't know?
A No, I still don't know.
Q Did you ever ask aaybody?
A My liaison in NRC, I asked Joe Kane.
Q And what did Joe Kane tell you?
A I say that is here - I am telling $h i m$, and I don't remembe the table number, but it is written there.

Q What did Joe kane tell you?
A He is not aware of why it is.
Q Did he say he'd find out?
A No. How should I find out?
Q No, did he, did Joe kane tell you that he'd find out foz $y$ and let you know?
A Perhaps he will make contact with applicant and then -(Interposing) : Perhaps he would, that is a possibility. he tell you that he would do anything to find out?

A No, he didn't tell me.
Q Did you want him to get that information for you?
A Sure, I would like to have that, but I didn't tell him tha I want it.
Q Why not? Did you assume that he knew that you vanted it?
A Because, no, because we are waiting for some information $f$ Consumers ?ower. There were a lot of designs and calculat

24 Q Did it say any more about that?
25 A No, I don't remember.

## SINGE

2 \& All right, and how do your calculations that you have got in would be at various elevations based upon 2.2 keps, is that

## 17 A Right.

18 And then I'Il bet you went and you made ancther one of these

25 Q And that graph is what we have as View Graph Number 6. It:

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> Consumers Exhibit Number 12 as of October 15,1980 , in the Kane deposition, right?
> Yes.

All right, and what that graph is, that is a graphic depic tion of your calculations as contained in Exinibit 4, right:

A Uh-huh.
Q Good. After you did these calculations and you did this gz that Joe Kane has got marked Exhibit 12 for his deposition, View Graph Number 6 , and I see here where it says 2.2 keps, Table 4.1, so maybe that's where you got the 2.2 keps?

A Yes.
Q What did that tell you?
A That the load was 2.2 .
Q Well, heck, you knew that apparently by looking at Table 4. You didn't have to go through all that stuff on Exhibit Number 4. You plotted that graph out or at least drew the graph?

A Uh-huh.
Q What does that graph say to you?
A It say that if I add this whole lot then the total lot at $t$ time of surcharge is less at the surface from this graph (indicating). That's what it states.

Q Okay. That would pretty much follow if you had a 2.2 keps and you had a 3.2 keps load, you pretty well know that the
stress would be less for the 2.2 keps load, right?

## A Yes.

So what in addition to that revelation did your graph of the calculations you used did this Exhibit 4 sh iw you?

A Just difference of the stresses it shows. That's all, the difference of the stresses between what is the graph I got in -- what is the number?

Q All right, the graph that you got --
A (Interposing): From the application.
From View two. I see what you did. It looks like what you did on here was that you laid your graph over the graph that was in Pigure two from the applicant?

A Oh, yes. I don't lay, but I compare what is on the top here, this thing (indicating).
Q What I mean you put both graphs on one or both of the data?
A Yes.
Okay, so basically what you were trying to do here was get a graphical depiction of what the stresses are at difi $\mu$ rent elevations and what it would like at 2.2 and what it weuld look like at 3.4 , I belleve is what you show as calculated at 628, right?

A Uh-huh, yes.
Q All right. Taking just the information using the 3.4 keps, or sheuld it be 3.5 keps? I notice you have here the dead
load equals 3.5 , and then here you have it equals 3,4 . Is that because you got the 3.4 off of Figure 2 but you calcule ed it at 3.5?

A Approximately calculated it. It was 3.5 and hare it was 3.4 and I measure it.

Q On Figure 2 ?
A Just approximately.
Q So what you did is you used the 3.4 on this graph, right?
A I think I used 3.5 -- maybe 3.4 , it doesn't make very much difference, but --

Q (Interposing) : Okay, what's a tenth of a kep, huh?
A Well, I am not aware of all the load what is there because I don't know how much pre-load is there and how much is there (indicating) because I just approximace it to see, and whatever the drawing shows I take that.

Q Okay. Based fust upon the 3.4 keps dead load that is used 1 Pigure 2 of Consumers -- you have got a little graph there, what does that tell ynu? What kind of information does that give you with regard to your review of the soils issues at Midland?

A That 3.5 keps I have written, that's after completion of the building when all is there, but $I$ don't know at the time of surcharge whether this load there is 3.4 or 5 , that $I$ have : way to know.

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2 Sure you do, you could ask somebody.
3 A Well, I can communicate with NRC.
40
5 A Yes, that is why I asked that, how it is the 3.55 after I seen Sure, and did anybod

8 A No.
9 Q Who did you ask, Joe Kane again?

A No, no, he didn't.
15 ( But you expected him to get back to you with that informatio:

17 A No, I didn't expect these things because ultimately when $I$ 18 submitted this thing, that result, then he wanted some more cause there will be some kind of interrogatories.

21 Q So you knew if you had any problems concerning any of this

3 (Interposing): Excuse me, I thought you were done.
4 A So he wanted to compile all the questions but $I$ don't know whether he will ask somebody and give it to me. I assume $t$

6 R Right, but he never did give it to you, whether he asked sor body or not, right. I mean you don't knuw?

8 A No, I don't know what he is doing after that.
9 He hasn't given you one lick of information that you asked $f$

A I didn't want --

1
2 (Interposing) : You didn't want the information? cant was given all the questions.

A Okay.
Yes, and you come up with a figure of 3.2 and a figure 3.4 I'11 bet as the lead reviewer you'd like to know which one the one that was $=1 g h t ?$

17 , A Yes, I asked him.
18 O Okay, and you assumed when he said that he didn't know tha 19 would find out and he'd let you know, didn't you?

23 A Expect him to get it somehow, no doubt about that.
24 Q Okay, in fact the reason that you mentioned it tc him was let him know that you had that problem with the informati.
so that ultimately you would get that information so that you

A Yes.

Q As you sit here now you still haven't gotten that information from Joe Kane, Ifght?

A Yes.

Q Do you remember when it was that you mentioned to Joe Rane if that there was this problem with the data?

A The same day Dr. Peck gave the demonstration, the same time I say I had never seen such kind of load there.
Q. That was back in around the 14th of September, 1980?

A No.

Q No?

A It was on 30 th of August, about the end of August sometime.
Q I see, so it was in August of 1980 ?
A But at that time I didn't get the picture, the submission. I got that after 15 days.

Q Ckay, so what you have been getting back fron Joe kane when you have been mentioning to bim that you had these problems or you need this additional information is simply that he keeps asking you to tell him about more problens that you have but he hass't given you any other information yet, right?
A Right.
Q Looking just at Figure 2 --

2 A (Interposing): Uh-huh.
3 (Continuing): And this is Pigure 2 to that September $14 \pm$

A (Interposing): Yes. 1980 submittal by Consumers --

Q What conclusions, if any, can you draw from that graph wit regard to the surcharge program, assuming that the informa contained on that graph is accurate?

A If the information given on this graph is accurate then $I$ assume that the dead load of the building and all the dead load of the building is less than - I don't know, probably about -- okay, just a second, if all the dead load includin the machine lozd is -- I can't read this. I can't say here if that is 4.50 or something.

Q It looks about like 4.4.
A 4.4.
Q You ara taking the dead load plus the live load, right?
A Yes, 4.4 approximately then at the time of surcharge there $w$ some excess load then the final load is going to come on the building, if assuming that 4.4 is corrset.

Q Okay. How much less than the 4.4 could you have and $s$ till have the load that was on the building during the surcharge exceed the final load?

A I have no way to know what is the final load on the building MR. ZAMARIN: Could you read back, pleasc

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that answer to that questionand the answer preceding?
(Whereupon the Reporter read back the last two answers.)
(By Mr. Zamarin, continuing): Okay, what is it that you as a reviewer want to see with respect to the relationship between the load during surcharge and the final load?

The loading of the surcharge has created compression in the foundation. It has some kind of compressibility constant and you can determine out of that and if the final load is more than the load of the surcharge load that constant, it depends upon the load.

Q So what you then want to see is that the load during the surcharge period was at least as great as the final load on that building was going to be, is that right?
A No, I didn't get your question correctly.
Q What you want to see is that the load of the buileing --
A (Interposing) : Uh-huh.
Q (Continuing) : At surcharge was at least as much as the final load is going te be on that building, or in other words that the final load isn't going to be more than the load during surcharge, is that right?
A If the final load is going to be more on the surcharge, definitely, then the compressibility will be different.
Q Okay.

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2 A (Continuing) : Because this soil is not over consolidated for that part of the load of the building. It would be more. So what you want to see as reviewar is that the load during surcharge is not less than the final load?

A That's not the only criteria, not less than the final load. That's one criteria.

Q All right, give me another one?
A Another one I would like to see the surcharge load should be at least some in excess of the final load.

Q How much?
A Nell, assume 1.5 times the final load. Normally I seen in all the surcharge it is generally twice, sometimes 1.5. Actually surcharge in some cases is ceEined as the load more than the permanent load in some instances, especially the Navy and even in Stanley Johnson paper 90-70 he defines what the surcharge over the permanent load is going to be.
0 All right. You say or you give the figure 1.5. What about 1.27

A I say, yes, I never seen. I have seen two. I don't remember ever seeing 1.5 , so it could be very critical.
Q It what?
A It might be critical for certain reasons if the dead load suddenly increases then you will be in the critical zone, okay so in some cases you don't know. Take the example of the snow

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load on the building, because sometimes the load might two or three months on top of the roef, and it might i the load on the building, so in engineering designs yo some margins.

How much snow are you talking about when you say that snow on the roof is an environmental load to be an irnp. facter?

A We design $50-150$ pounds per square foot in building breakwaters between the lock, we take 150 pounds per sc foot.

Q How much snow would it take to exert 150 pounds per sqi
A It becomes snow and then it becomes ice, it depends on happens completely, and I am telling you it is not impc but it might.

Q Okay, well, just snow, how many feet of snow would it $t$ exert 150 pounds per square inch or square foot?

A It can become ice, and snow is very light.
Q It comes down out of the sky as snow.
A Snow is very light, one inch of snow -- well, generally foot of snow -- maybe is equal to one inch of water nor so it would take tremendous amounts, I can guess it mig 100 feet.

Q Okay, and that ice has to come from somewhere, so you w need all that snow building up and turning, that 100 fe

of snow turning into ice?
A Due to compressibility. It doesn't have to become ice, but -(Interposing): Okay, sure, as it compresses or as that pressure occurs, whatever, it becomes ice and it would take 100 feet of snow to become ice to give you that kind of pressure?

A The entire season, yes, naturally.
Q okay. We wers talking about the factor by which the surcharge total load during surcharge ought to exceed the final load, and I asked you if in your opinion 1.2, for example, of the load during surcharge to the load that would be there after campletion of the building, in your opinion would be sufficient?

A No.
Q No?
A No, as an engineer there has to be a margin of safety dependin upon the type of the structures.

Q okay.
A (Continuing): Well, this particular structure, this is nuclear power plant, it is very important, the safety of the structure to insure safety of the structure in all conditions and I would not go less than 1.5. That is my opinion.

Q Okay. What do you base that on, just kind of a gut feeling?
A My experience.

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2 Y Yur experience in what?
3 A In designing.
Surcharging?
A No, designing structures.

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respect to the surcharge load as opposed to the loading durinc the iffetime of the structure should be 1.5 ? In other words, can you tell me a textbook that might say that?

The surcharge load is equal to the total load of the building and, definitely, I am getting a margin of safety of one, factor safety of one, and I know I am going to increase the margin of safety to 1.5 to insure more safety to the structure What I am saying is unless we assume that the .5 that you are increasing it in going from 1.5 is magic somehow, upon what do you base your opinion that 1.2 wouldn't be enough but 1. . is what ought to be there? I mean, can you give me the name of a book or the name of a paper or name of a study or the name of anybody or anything upon which you base that opinion?

A Most of the factor of safety I have been using in any corps of Engineers, in all kind of structures is 1.5 and more.

Q Okay. In other words, if you were to design something you would design for 1.5 times the expected load, right?
A That is not factor of safety. There is a difference between load factor and factor of safety.

Q Tell me what the difference is between the load factor and the factor of safety?

A Load factor defines, considers the increase, the percentage of load, the factor of safety compares with the stresses.

Q Okay. All right, so when you design then are you telling me

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that you always design for a 1.5 factor of safety, 1.5 times the anticipated stress?

And more.
Regardless of the type of structure?
One point - no, I say it can go above 1.5 , but never less than 1.5 I never used in my life.

Okay. With regard to your surcharge program in deciding on the ainount of surcharge load, and I know you have never had any experience with that, but do you know if there is a diffe ent practice perhaps in the engineering field with regard to whether you would load to a factor of 1.5 as opposed to 1.2 with respect to the stress?

A No, I didn't get your question. Would you repeat it? MR. ZAMARIN: Read it back, please (to Reporter).

A Stress of what?
Q Well, I would assume that the stress that that load, the surcharge load and the structure that is being loaded would impose upon whatever it is that is holding the building up, th soil or whatever?
A The question is not 1.2 and 1.5 -- I didn't understand what you mean by the stress in surcharge.

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You told me that you always design things $s$ so that you have factor of safety of -- you design something for one and a half times the stress, at least one and a half times the stress that is anticipated that that structure, whatever it that you are designing will ever be subjected to, right?

A Uh-huh.
Q Am I correct in that that was your statement?
A It means that I - okay, let me see this thing.
Q Yes.
A Whenever I design a structure I have come across these thin the stress used has a margin of safety of 1.5 and more.

Q Okay. Do you know whether in developing pre-load programs the engineering ifeld -- I am not talking about drawing boa: design : om the ground up, I am talking about in surcharge programs such as we had at the Diesel-Generator Building, do you know whether engineering practice is to calculate the 1 , to be applied to that to provide a margin of safety of 1.2 rather than 1.5 ?

A No. I have even seen there was a margin of safety of $t w$.
Q Ckay, you have been reading something where there was a mar? of safety of two, right?

A Yes. I can give example, that was in the Mayport Airfield. Their surcharge was approximately twice the actual -

Q (Interposing): All Iight, that was for what, a runway?


Runway.
Q For a runway, so you read about that. Did you ever read about any other surcharge programs as to what the factor of safety was?

A
I read other programs but I never saw any factor of safety there.

Q So you really don't know then other than that they used the factor of two at the Mayport Airfield what is commonly accepted in the engineering field, do you?

A It is commonly accepted, it is a good practice accepted in general programs, but I have never seen less. I read a couple moza papers and I don't remember those things, but I didn't see less surcharge in the actual load and even one less than the actual load surcharge was always more than the actual dead load.

Q Okay, iight, the surcharge is always more, okay, so that would be a safety factor of one?

A More is not one. You have to have --
Q (Interposing): You say it is never less. If it is not less it has got to be at least one.

But I haven't seen anything which is less than one and even one I have not seen because I say it might be more than that and how much more than that --

Q (Interposing) : I an trying desperately to uncerstand your last
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couple of answers. You are saying that you have never seer a surcharge where they use a load during surcharge that wa: less than the final loai, right?

A Final load, right.
Okay, and you read about one where the surcharge load, the pre-load was twice what the final load would je, right?

A Uh-huh.
Q You have to say yes or no?
A Yes.
Q He can't record the bobbing of your mustache. Do you know strike that, and the only surcharge about which you have in formation with respect to the load and a factor of safety was at Mayport Airfield?

A Go ahead.
Q And, therefore, you don't know what the accepted standard 1 in the engineering ileld with regard the safety factor for surcharges, isn't that right?

A Well, I discuss with my supervisors.
Q Your superviscr being?
A Bill otto.
Q What did he say?
A He say it is always more than that.
Q Always more than what?
A Is more than one always the ratio of these things.


2 O Okay. What else did he say?
3 A That's all he says.
4 Q Okay. It is always more than one?
5 A Uh-huh.
60 equal to the final load, right? I know you would.

## Yes.

 should be at least ene. how you drive on the freeway.A Sure.

So you do know that at least in the engineering field that it is accepted practice that the surcharge load must be at least

That is my opinion if $I$ would surcharge $I$ would surcharge 1.5 .

Q And you know some people when they drive around the interstate and the speed limit is 55 that they drive 30 , but what $I$ am asking you is whether - what $I$ am asking you is whether it isn't true that all you know about the standard in the engineering practice with regard to surcharge is that you use a load that is at least equal to the final load, isn't that right?
A You say at least final load. I as an engineer told you that it shouldn't be less than 1.5. I am not agreeing to that it

Q I am not asking what Hari Singh would do and I am not asking

Q What I am saying is, I am asking you about your knowledge of the standards in the engineering industry and you have told

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me that your supervisor; Mr. Otto, told you that it has to b
at least a safety factor of one, Iight, at least?
A He says more than one.
Q Okay, so that means that it has to be at least one?
A No, I don't say he say at least. He said more than one. MR. PATON: That's pretty clear. He didn't say -- he said it has to be more than one. He did nosay -

A (Interposing) : Ie did not say at least.
Q So it has got to be at least 1.0001 , right?
A I will not agree to that.
Q What the heck did he tell you? What did you understand him to be saying?

A He said that it should be more than one.
Q More than one? Okay, so all you know then about the standarc in the industry is Mr. Otto told you that it had to be more than one, isn't that right?

A It is more than one, yes.
Q And that's really the extent of your knowledge about what the standard is in the engineering industry, right?

A What was that, please?
MR. ZAMARIN: Would you read it back?
(Whereupon the Reporter read back the previous question.)


A Okay, I will answer that. He give me that more than one and gave me paper to read and I read that paper and I found that it was two.

So you know that somebody did two, right, and that was at the Mayport Airport?

Okay, that's why he give it to me, and I go and raad, and I asked because actually it should be more than one, and I read this paper and get the details, I read it and found two and then I accepted that.

0 So you saw two in that paper, right?
A Yes.
Q So other than what you read in that one paper about what they did at Mayport Airfield and Mr. Otto telling you that it should be more than one, you have no knowledge of what the standard is in the engineering industry, isn't that right? A Okay, standard of engineering, the factor of safety is almost In any design I have come across, I found is more 1.5 , so based on that I am basing this thing in every case that the safety falls, and I have the margin of safety of 1.5 , and I have seen some surcharge done in the past based on that, too. Q Tell me about the surcharge you have seen done in the past?

A That I read the paper.
Q So when you say you have seen them done in the past you are talking about that Mayport Airfield?

A Uh-huh.
Q Okay. My question, however, is other than the paper you read about the Mayport Airfield and Bill Otto's statement that it should be a safety factor of more than one you have no knowledge of what the standard is with regard to the safety factor in surcharge or pre-load prograns in the engineering field?

A In engineering field for suricharge I don't think there is one standard like that, that they use just one factor of safety.

Q Becuase you don't know what the standard is, right?
A In my knowledge there is no standard, but in practice there is that factor of safety.

Q In what?
A For everything there is a factor of safety and that should be 1.5. I have not seen -- I have practiced 24 years and I have never seen a factor of safety anywhere less than 1.5 .

Q All you have seen about surcharging though is the paper on the Mayport Airfield?

A Yes, and that was two there.
Q Okay, but you haven't seen anything else or have any other knowledge of what the engineering standard would be on surcharge, your knowledge is limited to what you read in that Mayport paper and Bill Otto's comment that it should be more than one, right?

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MR. PATON: I object to the question because the question has been asked repeatedly and the Kitness has repeatedly stated that based on his engineering practice and his experience of 24 years he has stated that he would apply generally in the engineering field a figure of 1.5. I think his answar is clear.

MR. ZAMARIN : That is his practice. Perhaps you have missed the question, too. The question is -MR. PATON (Interposing): No, I didn't miss it.

MR. ZAMARIN (Continuing): It is the standard in the industry not what Earry Singh does when he is designing a trapeze or a bridge, it is what the standard is in the industry and I belleve, Mr. Singh, that you have testified that your only knowledge of the surcharging and what is done in the surcharging is your experience with reading the paper on the Mayport Airfield and Bill otto, who is your supervisor, saying that it should be greater than one. Now was that what I understood you to be saying with regard to surcharge, and 11 I am wrong tell me and wa will go down and I will want every other instance thatyou have upon which you base knowledge of the standard with regard to a surcharge program. That's all.

UR. PATON: All right, is that your

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question?
MR. ZAMARIN: Yes.
MR. 'PATON: Okay, I object to the question industry practice based on what he has seen in 24 years of engineering and I think his answer is clear and his answer is saying that is more than his thought, it is what he has experienced and, therefore, that what he is saying is generally accepted engineering practice.

A Can I add something to this?
(By Mr. Zamarin, continuing) : On what? Do you know what my question is?

A Yes.
Q Do you remember what my guestion is?
A Yes, factor of safety.
Q No.
A (Continuing): I talked to Bill otto and that was just a casual talk, it was not -- he said, "Well, at least you don't want less than that, there should be some factor of safety more than one."

2 Ee didn't say, "You don't want less than," did he?
A No, no, he said more than one. He said it should be more than one but I mean this was just a casual talk, it was not a serious talk, so I will not consider that.


Good, I won't hold lim to it.
A He gave me a book and told me to see this thing, that there it is.

Fine.
A And I take that as correct.
Listen carefully to my question, and my question is that other than your casual talic with Bill otto where he said it should be more than one and gave you a book --

A (Interposing): He said the book contains sometiing.
Q Yes, okay, sure, and yru went and you read this one paper about the Mayport Airfield which was in the book that $B 111$ otto gave to you following your casual conversation with him?

A Uh-huh.
Q All right, and other than that conversation with $3 i l l$ otto that you told us about and what you read about in that Mayport Airfield paper, isn't it true that you don't have any knowledge of what the engineering standard is regarding the safety factor for surcharges?

A Okay --
Q (Interposing): Isn't that true?
A I read lot of paper but they have never mentioned what is that, and if I am going to calculate it $I$ can find that.

Q I an not asking what you know now. Isn't it true that that is the extent of your knowledge in that area with regard to

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standards for surcharges?

Sure, it mean I saw that, but I have read a lot of other papers.
But the extent of your knowledge about the factor of safety in surcharge programs is based on the Mayport Airfield paper that you read and your easual conversation with Bill Otto, isnt that true?

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A Please, may I ask you to read back again?
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(Whereupon the Reporter read back the previous question.)

Can you read it again?
Whereupon the Reporter read back the previous question.)

A Surcharge - strike that out, pager reporting surcharge load, I have read more than one, but in those papers this factor of safety has not been mentioned so I assume that, I assume that the factor of safety as asked by me should be one is not correct, it should be more than 2.5 . Once they don't mention it doesn't mean that that is one.
Q I am trying to tilink of a question that would match that answer, Mr. Singh. That wasn't my question. My question simply is that isn't it true that your own knowledge of what the engineering industry standard is for safety factor in surcharge progrars is what you read in that Mayport Airfield


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paper and what B111 Otto said to you as you have described to us in that casual conversation? Isn't that it?

A Let me say I have not read only one paper, I have read other papers, but they dor't mention it, and that doesn't mean that I have only that experience. I have other experience. I am not asking you for your experience, I am not asking you whether you are illiterate and didn't read them, I am asking you for what information you have with regard to the industry standard with regart to the safety factor in surcharge programs and isn't it true that that knowledge is limited to what you read in the Mayport paper and what Mr. Otto sald to you during that casual conversation you have described?

A The new standard in my knowledge for the surcharge or the factor of safety what is used in other normal engineering practice should be used.

Q Okay.
MR. PATON (Intarpesing): That is, I submit, that is an answer.

Q Okay, so what you are saying is that to your knowledge there isn't any standard?

A No.
Q But that you would use the general standard used in design in the engineering industry, right?

A Good, yes.

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Okay. My question is though that when you say that you don't believe or you don't have any knowledge of any standard that is because the only knowledge you have with regard to safety factors that were used or are used in the industry is what you read in that Mayport paper and what 3111 Otto might have mentioned in this conversation, right?

Okay, I read, I read lot of other papers regarding this and they don't mention this thing, factor of safety, and that means I assume they use the normal engineering practice taking that as a factor of safety that should be used in surcharge, tof MR. ZAMARIN: Would you read the question back, please?
(Whereupon the Reporter read back the previous question.)

Q Okay, my question is other than -- strike that.
Isn't it true that the only knowledge that you have of what might be an industry standard of the engineering field with regard to surcharge prograns and the factor of safety to be employed therein is the paper on the Mayport Airfield and your conversation with Bill otto that you have described to us?

A Nould you read it again to me?
(Whereupon the Reporter read back the previous question.)

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MR. PATON: Jugt a minute. Can I ask Eor a clarification and that is that you are asking him to eliminate from his answer the information he gave you before that he would use a standard from general engineering? You are asking for a standard that applies only to or limited to surcharging ?

MR. ZAMARIN: Yes. The statement he made about other sources is in the record.

MR. PATON: Fine, I am just trying to clarify it if that helps the Witness any.
A In my knowledge there is no standard, there is no standard for factor of safety for surcharge load. What is practice in other engineering, civil engineering field is the same factor of safety which should be applicable on the surcharge load.

MR. ZAMARIN : Would you read the question back? That is not responsive.

MR. PATOM: It's not? Can we go off the record? To me that was the perfect answer.

MR. ZAMARIN: Okay, off the record.
(Wheraupon there was a short discussion held off the record after which the Reportir read back the previous question.)
A You waiting for my answar?
Q Yes, we are waiting for you.

A I look in the manuals of the Corps of Engineers which is one of the leaders in this kind of thing and consult with B111 otto, and any manual giving the details of surcharge and I didn't ifnd anything regarding factor of safety, so I assume that the leader in this area don't have so there is nothing existing regarding factor of safety of surcharge load and the other factor of safety, the factor of safety in other area of civil engineering should be used.

MR. PATON: Let me talk to the Witness for a minute. Let me ask $h i m$ on the record or off the record MR. ZAMARIN: No, stay on the record.

MR. PATON: All zight, I am trying to -I think I uncerstand what you want. It's difificult and I have been trying to, off the record, assist and, you know, I think the Witness has answered your question but apparently it is not exactly the form in which you want it.

MR. ZMMARIN: I would think that he could give a yes or no answer.

MR. PATON: Let me just ask you, would you be satisfied if he were to say that he had no knowledge from another scurco as to whether or not in fact there is any industry standard that relates strictly to the factor of safety for surcharging?

MR. ZAMARIN: Sure, he can say that but

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that's really not the answer to my question.
MR. PATON: That's not enough?
MR. ZAMARIN: NO.
MR. PATON: No? I am really not sure what
else you want besides that.
MR. ZAMARIN: My question is very straight forward. I want h im to say it is limited to what he has already said. He can answer that question yes or no. If he answers yes that's it. If he answers no I am going to ask him to list all the things. Te has to answer that one first and that's a yes or no.

MR. PATON: I am really trying to get over the hump here because we are taking a lot of time on this.

MR. ZAMARTN: Yes, it has been 25 minutes now that I am trying to get an answer.

MR. PATON: Let me ask you this one question. If he wera to say that he had no other knowledge from any other source other than talking to $B 111$ otto and reading that paper you would then be able to conclude in your own mind that the answer to the question you were trying to get is, yes, that his knowledge is limited to that, is that correct?

MR. ZAMARIN: Sure. We are not playing twenty questions.

MR. PATON: I know what your problen is,
you are not happy with the form in which you are getting the answer.

MR. ZAMARIN: He is not answering my question. Sure what he may be giving me one might be able to through deduction conclude what the answer is but I think I am entitled to a straight forward answer to my question and he can answer it yes or no. If he says no, that the statement in my question is not true then I will go ahead and I will ask him for all of the other factors of safety he knows about until I know that he doesn't know any and I will ask my question until he answers yes that what $I$ state in my question is true.

MR. PATON: You don't have any trouble with srriving at what is already on the record?

MR. ZAMARIN: That's right, I don't, but somebody some day down the road might read the transcript that isn't as smart as I am.

MR. PATON: That's true and -- well, all I can do is let you and Mr Singh try to work it out.

MR. 2AMARIN: Would you read that question
back again?
(Whereupon the Reporter read back the previous question.)

A
I search for other iiterature including all the manuals of the U. S. Corps of Engineers. I never found anything and in that

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case that's the only thing I know and it is limited to that.
MR. PATON : Now, is that your answer? He said it was iimited to that. Isn't that your answer?

MR. ZAMARIN: Would you read the question back, please? It can be answered yes or no and there is enough in there, my question is complete enough so that it makes it easy for a yes or no answer and this is nonsense that we keep getting stuff where the witness just doesn't want to answer.

MR. PATON: I want to make a statement on the record and the statement is that we have been struggling with this for some time. It is my opinion that the Witness has answered the question, apparently not in the precise form that Mr. zamarin would like. I think we are trying to cooperate and I would ask the witness in order to satisfy Mr. Zamarin, I tink this is what he wants, that when you listen to this question do so extremely carefully and if you can answer the question with a yes or no do so. Now you can explain your answer but try to answer it if you can first with a yes or no, 1f you can. Do you understand what I am saying to you?
A Say it again, the last part.
MR. PATON: All right, I think what Mr. Zamarin wants you to do is to answer the question, if possible, with a yes or no. Do that first. Listen to the question carefully and if you can answer it with a yes or no do so and then you can explain that if you want to, if you want to explain why you say yes or no you can do so.

A Good.
MR. PATON: Try to answer it yes or no if you can, okay?

A Yes, I know, you know.
(Whereupon the Reporter read back the previous question.)

MR. ZAMARIN: Do you understand?
A Uh-huh. Yes, but I search for the entire manuals of the United States Corps of Engineers regarding the surcharge and factor of safety and I consulted NRC people and wanted anywhere the written factor of safety and I found nowhere written what is the factor of safety for surcharge load.

Q Okay, so then other than the manuals that you looked at and the paper on the Mayport Airport that you read and your casual conversation with Bill otto, you have no --
A (Interposing) : No, and also -- oh, go ahead.
Q DiC I leave something out? I will start over again. Well, what did I leave out?

A I consulted NRC personiel, too.
Q All right. Who in the NRC did you consult?
A I wanted to know if they know anything abovt that. I had conversations about that with Joe Kane.

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2 Q What dic Joe tell you?
3 A It is nowhere written regarding surcharge.
4 Q When did he tell you that?
A Long ago I talk to him.A Two months ago.
Where was it, on the telephone?
A It was on telephone.

What did you say? Did you say, "Joe, do you know what factor of safety there should be in a surcharge"?

A No, it was a discussion when we saw that (indicating).

## Q Figure 27

A Applicant's presentation on the 28 th of August at Midiand and then we saw something and what should be factor of safety, so we look through different papers and so I didn't find anything defined anywhere so $I$ guess what is normal is used in engineering, civil engineering practice should be.
Q So Joe couldn't give you any more information on that either?
A No, he didn't show me any paper where at that time I might have found that, and after that I didn't consult him.
Q Okay, so whether than -- isn't it true that other than your asking Joe Kane if he knew what the factor of safety should be and him saying no, he didn't know, and your referring to the Corps manuals and not being able to find anything and your

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casual conversation with Eil Otto in which he said it ought to be more than one and that paper you read about the Mayport Airport you don't have any knowledge with regard to what might be a standard in the engineering industry with regard to surcharge loading?

Yes. Yes.
What was the purpose of the safety of factor -- strike that.
What was the purpose of the factor of safety that you read about at the Mayport Afrfiwld?

A Any structure design you provide to make sure of safety. That's normal practice for engineers because there are s lot of unknown factors in construction material, variabilfty and to svard against all this contingencies engineers use a factor of safety.

Q What was the purpose, do you know, of surcharging to twice the final load at the Mayport Airfield?

A Because they are over consolidated for the less load.
Q What were they trying to do to the soil?
A Compress it.
Q Was the purpose there just to accelerate settlement?
A Yes, that was one of them.
Q What were the others?
A I assume that otherwise to get it over consolidated so that the inal settlement would be less, the over consolidated

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load, the settlersent would be less.
What was the magnitude of the surcharge at the Mayport Airfield:
A It was ten feet of soil higher than the final grade.
What was the magnitude of that load?
I don't know. They have given two -- they mentioned this, but I don't know. No, I am sorry, it was ten feet above the ground level and it was -- I don't remember, I don't exactly remember now, but it was ten foot higher than the final grade so you can calculate. I assume it was the weight of the soil. I would assume 110 or 120 multiplied by ten, so I would assume 120 multiplied by twelve would be 1200 points.

Q 1.2 keps?
A Yes, more than that.
Q What kind of soil did they use?
A Oh, I don't know what kind of soil they used.
Q It wasn't ten feet of peat, was it?
A No, I don't have any idaa of the soil.
Q What area of the airport was surcharged?
A It was extension of this.
Q It was what?
A They were going to extend the airport from, I guess, it was for four hundred feet to 8,000 feet. It was swampy area that they were surcharging.

Q They were surcharging a swampy area?

2 A Yes, because they are going to build the airport there, extend it.

Q Okay. You think there is a difference in surcharging a swampy area as opposed to the load that you want to impose as opposed to maybe surcharging the Diesel-Generator Building?

A I think the principle is the same.
Q That is the same but as far as the amount of load you wouldn't i. see any difference, would you?

A Amount of load, you say? I don't understand your question.
Q Can you tell me where I could find that Mayport Airfield paper?
a I don't know. I have a copy of that.
Q You have a copy of that?
A Because B111 Otto give it to me.
Q Could I look at it for a minute after Iunch?
A I think I have one here.
Q Oh, maybe I can look at it before lunch. What else have you got there? off the record.
(Whereupon there was a short discussion held off the record.)

A (Continuing): You might request, I think Bill otto has a copy of that. I give it back.
Q You gave it back to him?
A Yes, I gave it to him.
MR. PATON: We will attempt to get that
for you.
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MR. ZAMARIN: Thank you.
(By Mr. Zamarin, continuing): Dees the factor of safecy that we have been talking about or does factor of safety depend on what it is that you as an engineer are trying to accomplish?
A I didn't get the intent of your question.
Q Okay. What $I$ mean is this. Are certain things that you do, for example, you might have a factor of safety with regard to slope stability or with regard to bearing capacity?
A Uh-huh.
Q (Continuing) : And there are generally accepted factors of safety within the engineering field for those two elements, for example, that are quite different, aren't they?
A They might be different, yes.
Q They might? They do, don't they? Don't you typically use maybe a factor of safety of three for one of those and one and a half for the other?

A That's right.
Q Right. Okay, so it does depend on what you are trying to do and what you are setting out to accomplish as to what the factor of safety ought to be, right?
A Yes, but I have never seen less than 1.5 for anything.
Q Okay. All right, you could be surprised theugh, couldn't you?
A Not if I see something somewhere written, some documentation,

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$$ but I will not accept somebody's mouth.

Q Sut you also haven't seen very much about the surcharge and the factor of safety used, right?

No, I saw all these papers and went to ths manuals.
But they didn't say anything?
Didn't say anything. I can calculate for the load they have given, you can calculate and I believe in that.

Q Okay, but if you consider that to be so important why didn't you do that calculation to find out what factors of safety have been used?

A Where?
Q Where? In the papers you read?
A They were not evident, not enough information.
Q No data?
A Not enough information given, but in some cases $I$ think I did in some places, in that Mayport paper.

Q Other than Mayport though you didn't at least do any kind of a calculation to tell you what the factor of safety was, right?

A Mayport give information.
Q Okay, but you didn't do any calculations with regard to those other papers as to what the safety factor was even though that was important to you?

A But the information might develop very quickly for that.

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Q So that when you said you could do the calculations on that you meant if the information was available, but it is not available in those papers, right?

A If available I would do it.
Q But there wasn't that information in those papers, right?
A No, not adzquate. There was some but not adequate in my opinion.

Q Okay. When you refer to a safety factor that you use of 1.5 is that usually in the design of a structure?

A Generally. I remember it was in slope, it was when I design a retaining wall.

Q Uh-huh, so --
A (Interposing) : This is in sliding, I remember it is in sliding of retaining walls. I am referring to the Pennsylvania Highway Depart ent mainly-

Q There it was 1.5 and that was with regard to overturning or sliding?

A I think it was silding.
Q Okay. What is it for overturning?
A It might be two, but it is more than 1.5.
Q Now what is it for bearing capacity?
A Bearing capacity depends, maybe two, three, I have seen some manuals say three, but $I$ don't remember where it is. I immediate'y refer to the manual and find it out. Did you use a safety factor ir predicting settlement on an: of the retaining walls that you have been involved with?

A Rotaining walls? I never predict settlement.
Q Why not?
A It in not in design. Generally we design for bending and shear and all such things, because I never have problems wit I calculate it.

Q Did you ever predict settlement for the bridges you designe
A One in India I did. I don't know whether the bridge was bi or not. I left.

Q Did you use a safety factor in predicting settlement?
A In settlement I never used it.
MR. ZAMARIN: This would be a gcod Eime
break for lunch.
(Wherevpon the Ceposition was recessed
until 1:15 o'clock, P.M.)

## AFTERNOON SESSION

CROSS EXAMINATION (continuinc):
Q (By Mr. Zamarin): With reģard to Figure 2 of that Septembe 14th Consumers subrittal, and that again was marked as Exhibit Number 8 at the Kane deposition on October 15th, is determining the factor of safety that was associated with $t$ loading depicted on that chart, would you measure the stres

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at the mid point in the fill?
A I would not measure at mid point. Normally most of the sett. ment will occur on the top portion so I will measure above the middle somewhere. At this stage $I$ can't make a snap decision. I have to study more.

What do you have to study more?
A Normally it is taken where the most -- where we see the most area where the settlement will occupy.

Q In your opinion that would be somewhere in the top half of the lavel of the fill?

A Yes, somewhere there.
Q And in making that calculation I'd like to go through with you on Pigure 2 how you would go about dolng that, just so I understand how you would use this graph. Okay, if for example we were going to take the level of 618 --

A (Interposing) : Uh-huh, okay, 618.
Q See it?
A Uh-huh.
Q How would you go about from the information contained on that graph in using the data that is represented on that graph determining the factor of safecy?

A In this case suppose -- I am assuning this is correct.
Q Uh-huh.
A I am not telling it is correct.

2 Q That's right. I am saying assuming that the information on the graph is correct, right.

4 A (Continuing) : It will take this total ioad here (indicating
5 O Okay, wait. When you say the total load here, I am going te describe it.

7 A Total stress.
8 Q Total stress, and by that --
9 A (Interposing): I am assuming on calculating on this 618.

A Assuming this drawing, Figure 2 -- what is the Exhibit number?

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It is Figure 2 in the September 14th submittal.
All right, this submittal at 618 I will measure the total stress. I don't know exactly, but approximately 5.1 kep, that's my estimat. I am using.

Sure.
A (Continuing): And $I$ an assuming that this will be the final load on the structure.

When you say "this" you are referring to line number five? Line number five.

Q And that would represent the final load of the structura?
A No, it is trace developed by the final load given by the app. cant.

Q Okay.
A (Continaing) : Which I have not verified that final load is correct or not, but based on that the stress will be, oh, 4.3, 4.3.

Q And then would you divide 5.1 by 4.3 to determine what your -
A (Interposing) : Yes, that would be.
Q (Continuing) : Factor of safety is?
1 A Yes.
Q Excuse me just one minute.
Okay, before you go away from that, that
figure, I want to ask you a couple of questions. It seems to me in determining the stress associated with the Einal load
you have included the weight of the soil and should you be doing that?

A No, the weight of the soil it has been included here (indica: ing), the weight of the soil that includes total pressure. Okay, that's right, and in yous opinion in determining the effective stress of the structure you would include the stres associated with the weight of the soil?

A Yes, both are present.
Both are what?
A Both are present at that time so I would include that one. Q In a situation such as the Mayport Airfield in coming up with a factor of safety of two do you know whether they included the weight of the soil along with the stress associated with the load?

A At twice the load they have calculated. I assume that have calculated the load is twica, the load they have calculated. I calculated it on the surface and there will be more at the top of the surface.

Q There would be more what?
A More on the surface, on the top surface.
Q Yes, so it is your opinion that when they report a factor of safety of two that they have included in their calculations as you have is. taking it off this graph in Pigure 2 the stress associated with the weight of the soil?

2 A I think they have calculated at the surface of the runway.
3 Oh, I sae, I see, so ther wouldn't be any soil weight at th surfach, is that what you are saying, so, therefore, it kind of washes itself out?

A No, here at the ground level -- well, let's see here, at the ground level then they calculate 20 feet, the surcharge, I believe it is 20 or 22, approximately.

Q Yes.

A (Continuing) : What is going to be the actual load, ten feet, they say the runway level was ten feet and they loaded up to 20 Eeet high so at that surface they calculate it.

Q At that surface, and by that you mean -
A (Interposing): Where the ground level was originally, so in this case the footing of the building, and so I would calculate at that ievel in the same way I will not consider any load for the soil here (indicating).

Q For the soil here? Okay, so if you are calculating it at the level of the footing --

A (Interposing) : Yes.
Q (Continuing): -- you wouldn't include any soil weight that would contribute or resuit in stress, right?

A Dnder the footing definitely, but there is soil by the side of the footing that will contribute stresses below, you know, the footing. The footing there is below the top level.

2 Q Bow thick was the compressible layer that they were dealing 3 With at Mayport Airfield, do you know?

4 A I don't know.
3 Q Was that information provided in the ifterature?
6 A I read it, probably 14 feet.
7 Q I am sorry, what?
8 A I am not sure.
9 Q Let me mark this as an exhibit and I am marking it as Consumt

Okay, have you had a chance to look that

## over?

A I want to find out the thickness of the muck.
Q I think it is about ten feet or something.
MR. PATON: He said 14 feet.
But that is not the thickness of the muck.

Q (By Mr. Zamarin, continuing): Okay, well, let's set the aside for a minute and let me ask you something else. I going to give you a piece of paper and you have a little eil there that is just a little stub, and you may take $t$ one --
(Interposing): That's big enough for me.
Okay. All right, I want you -- I am going to give you s assumed facts. They don't apply to Midland. They don't apply to anything I am aware of, and I want you to show how we would go about determining a pre-load amount usin this 1.5 safety factor.

Let's assume a structure that is 50 by 50 feet, that's 50 feet square, okay?

A Yes.
Q And it has a weight of three keps per square foot.
A Weight of the structue.
Q Yes, 3 keps per square foot. Okay, and you have a compr, ble layer 100 feet thick and you want to surcharge this : ture with sand in order to achieve your 1.5 factor of sa:

A Surcharge with sand?
$Q$ Sand, yes.
A How much sand, yes.
Q How many feet of sand would we pile up on that 50 by 50 structure in order to achieve your 1.5 factor of safety?
1.

A Okay，I need some more information to calculate this thing． First you have to determine the distribution of the pressure under the foundation which you cannot do without，without some more information．

Well，I can do it，I can make it up，so I will give you a distribution of pressure．Use a two to one distribution，two vertical to one horizontal．

Two horizontal，one vertical？
No，two vertical and one horizontal．
It will be somewhare－－of a total of 100 feet thickress，it will dissipate somewhere in between．

Q What will？
A That stress，the entire thickness will not be affected by that load．

Q By what lcad？
A The load you are putting．
Q You mean the load of the structure？
A Load of structure，you know，once they put on the load，the entire load will not go one way 100 feet，it will go out．

Q Well，you can put on a heavy enough loadso that it can go 100 feet，can＇t you？

A les，but you want a factor of safety．
Q Right．
A Factor of safety，so－－

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(Interposing): Right.
MR. PATON: Let me ask a question clarify
ing. Are you pre-loading this before there is any structure there or after?

MR. ZAMARIN: No, no, you have a structur
MR. PATON: You have a structure, okay, fine.

MR. ZAMARIN: I started out by saying you have a structure there.

MR. PATON: The structure is there.
Q okay.
A That was 3 keps per square foot and he wants to surcharge it.
MR. PATON: Sure, but I wasn't sure
whether the structure was there or not.
Q (By Mr. zamarin, continuing): What I want to know is how mue of a surcharge we would have to put on that to provide your 1.5 factor of safety?

A It is not that easy for me with the data given as to how much I load, how much of a load I put here. The depth will be going down more and more. Suppose I put four tons it will be maybe eight foot deep, suppose I put 12 tons, myybe 24 feet, so you can't do this in such a way. You need at least one or two days to figure out the distribution. You can't assume like that.

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2 Q Okay, so you'd need one or two days in order to --
3 A (Interposing): Sure, you have to study this. This is not going in one day and one second and making a snap decision. Come on, Hari, I am not asking for a snap decision. But you are asking factor. I can calculate factor of safety at this level, that is reasonable, at the top of it. What would it be at the surface? Oh, that is very easy. okay.

A Your load is only 3 keps.
Q Three keps, right.
A For the building.
Q Yes?
A So I will put dcwn 1.5 and multiply by three, and I will neec 4.5, and I measure for that, for how much sand you need, and it depends on the weight of the sand. Suppose the sand weigh 120 pounds, then you put the 500 divided by 120 it will be. That is the number of feet of sand you have to put on top of the stracture?

A Yes.
Q So then what you want to do is you want to put one and a half keps worth of sand on top of this 3 keps structure so you'd have a total stress at that level of four and a half, right?

A Yes, using the three and then the one and a half, $u c$ that is

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no problem, I can do this. It is obvious, but if you go below that -- this I will accept that one maybe because that is very easily done.
okay. Would that then, providing that type of surcharge that you just described, provide in your opinion the factor of safety that we are talking about this morning that would be required with regard to surcharge program?

A You say -- yes, the factor of safety. I am sorry, I didn't hear you. Read the question, please.
(Whereupon the Peportar read back the previcus question.)

A For Micland, yes.
Q I asked you a little earlier if you had a chance to take a 100 at the Corps letter of July 7, 1980?

A Yes, uh-huh.
Q And have you had a chance to just glance at that?
A I glance this thing, but - okay.
Q Can you tell me to the best of your recollection into which of the questions in that letter you had i'uput and what the extent of your input was for each of those?

A There are a lot of overlaps in these questions, four and five people preparing because we used to study and everybody was writing questions and then we combine together and then Ifc

## SINGH

a lot of overlapping so $1=$ 's hard to pull out from any particular question any one sentence by me. I gave idea and some other people gave idea and I have some rough approximi 1dea, and I have written that -- give me that, excuse me, $s$ me those papers I have some marks approximately. Somewhere I put some numbers. Yes, in question number 39 I have inpt in one and two. You can read from here, one and two is written here (indicating), one, two, three, four, five, it marked here question 39.

Q Okay.
And there is mark there which shows one and two. There are only two suppositions that I put in there.

Q Jkay. With regard to all of those cuestions in that Corps report you only had input into this (indicating)?

A No, this particular question $I$ have two and I have other questions, too. Question Number 40 , I believe I have input in, let's see, 40 is the main head and then the small one $i$. parenthesis. Porty, small one, two, and four. All right.

A (Continuing) : And some others, there are some others where I have little input earlier and there I can't extract all these things.

Forty-one I have input, 41 is the main head, the nain question and then subhead 41 , one and that 0 :

## SINGH

is in parenthesis, 41, two.
Q Okay.
A (Continuing): And then 42 , same thing, 42 , one, 42, two. In 44 I don't have input in 44.

Okay .
Forty-five, I belleve I don't have input, naybe few lines, but I don't retamber exactly.

Forty-six, the question regarding cooling
pond, I have some input in that.
What input did you have into 46 ?
A Cooling pond, I say.
の I know, but like what did you say? what did you do that enced up getting sorething of you into 46?

A Forty-six? Regarding the stability of the dikes and there is some category one pipe underneath.

Category one pipe?
A Yes, that is -- I assume is right because that is discharge pife.

Q Really what your input was there is you said, "Hey, I'd like to know the stability of the dikes because there may be eategory one pipe there," and that was the kind of input you had into 46 ?

A Yes.
Q Was that the extent of it?


A Yes, that was this kind of thing.
Q Do you have any reason to doubt that the dikes are stable?
No, I wanted to -- no, once you give me something to check I have to check. I don't have reason or no reason. I want to verify.

Q Okay, continue. What about after question 46?
A Forty-seven, this is question three in that and 48 , no, on 48 I don't think so. Maybe some input but I cannot extract completely.

Q I didn't hear what you said with regard to 47 . Did you have any?

A Yes, I told you three.
Q Oh, three? okay, okay.
A In 48 I don't see any. It mightbe, but $I$ am not positive.
Q Were there any other questions?
A Not as close as I can figure out, but there may be a few ines here and there.

Q Okay. What was your input to sio parts one and two of question $39 ?$

A I think it is bearing capacity. Let me see. You asked which question?

Q Thirty-nine, one and two.
A Thirty-nine, you want me to read this thing for you?
Q Well, when you said you had input did you prepare the question?


2 A I prepared the question.
3 Q oh, okay. Why did you want to know what you asked about in Does dewatering have an effect on bearing capacity?

10 A It first goes on the settlement. information is given to me and the factor of safery. do you?

A I saw this was given in a table and the table is not enough BETZANO SUMMERS. INE.


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2 for me to review.
3 Q Why not?
4 A I want to see how the calculation is done, how much shear
5 strength parameter, so I want the complete computations.
6 Q Did you have any problem with any of the values that were con
7 tained in that table of the FSAR?
8 A There is no question of problem, problem is $I$ am a reviewer
9 and whatever you say it is, you say it is three and $I$ can't

Is that type of review required by Reg Guide 170 ?
12 A

Q You think that is the zeason, but you do say in your opinion You think it is required by Reg Guide 170 , is that true or not?

A I am not familiar with Reg Guide 170.
Q Are you familiar with the Standard Review Plan?
A Yes, I have seen that plan.
Q And is this type of verification of information or the backup

## SINGH

information such as is contained in the PSAR table required by the Standard Reviev Plan?

It doesn't specifically speak anything. So in ycur opinion it is not required by the Standard Review Plan?

A I think, in my opinion, this is required to see all the conputations.

Q Required by what? By the Standard Review Plan?
A I think yas.
MR. PATON: I didn't hear the last questior
MR. ZAMARIN: Read it back.
(Whereupon the Reporter read back the previous question and answer.)

Q (By Mr. Zamarin, continuing) : You said, yes, you think it is required by the Standard Review Plan?

A Yes, anything comes to me for review I am a reviewer then $I$ will be going to put my signature on it and anything when I put my signature I am taking responsibility for something and then I would like to see information for what I do.

Q I understand that and really what I am asking you is is there a requirement in the Standard Review Plan that says you must do chis checking of the backup calculations?

A No, it is not written there.
Q Okay. This question you are talking about deals with a
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containment, doesn't 1 t?
A Let's see what is there. Reactor building foundation. Yes, who told you that was within the scope of your review?

I didn't ask. I am a reviewer so I have to satisfy ryself.
Well, I mean obviously there was some limitation. You wouldn't start looking at anything, didn't soniebody else tel you what structures you are supposed to be looking at? They say you are technical engineer, the foundations, bearin capacity.

Q For everything out there on the site, is that what your unde standing is?

A No, what is furnished to me, just like Auxiliary Building a. all these things, so --

Q (Interposing): I am talking about the containment right now on the Auxiliar Building?

A That Reactor Building, reactor foundations.
Q Who told you that you should be looking at the Reactor Build foundations? That is not founded on plant fill, is it?

A No, it is not on plant fill.
Q Why are you looking at it?
A Euh? Okay.
Q I was waiting for you to answer. I didn't realize that you had?

A Okay, I will tell this. This question was partly formed by

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me and partly formed by NRC, sc you will have to satisfy HRC. too. I have input into that.

Okay. Realiy my question is when you first got tills assignment --

A (Interposing): Uh-huh. (Continuing): Were you told that you were supposed to be looking at the foundations and matters that related to the plant fill at Midland?

A No, for all category one structures, that was my understanding.
Q I see, and who was it that gave you that understanding or told you what it was that you were supposed to be doing?
A I got my supervisor.
Q Mr. Otto?
A Yes.

Q Okay. Have we pretty much gone through now your reasons for asking sub parts one and two of question 39 ?

A Yes. Now I see, Reactor Building is category one structure.
Q Yes. It is category one?
A That's right.
Q And you took your job to be looking at category one structures, sight?
h That's right, that's right. It doesn't matter whether it is reacter or what it is if it is category one structure.

Q Were you told to look at any non-category one structures?

## SINGE

2 A No. Certain things I felt very close to category one struc3 ture, so -- category two, and that question came to my mind.

4 I never askcd that question a couple of places.
5 Q So you then took it upon yourself to question the categoriza6 tion of some of the structures out there?

7 A Yes, sure, because there is category one pipe or category one 8. certain things and imaediately I felt that I should point out 9 to NRC. I didn't ask them, I pointed it out to NRC.

10 Q You pointed it out to Joe Kane?
11 A I wrote to NRC, not Joe Xane.
12 Q Who in NRC?

3 A Let me see. 41, I have input in that.
4 Q What was your input in question $41 \%$
5 A Last sentence.
6 Q When you say 41 we are talking about sub part one, right?
7 A Yes. Give the question to ne.
I am asking you if really you wrote those questions?

Q It says furnish the computation details for evaluating magnitude of vibration for Diesel-Generator Building including magnitude of existing forces, whether they are constant or frequency dependent.

Why did you ask this question 40 , sub part two, for bearing capacity computations?

> Do you understand my question?

A No, what are you talking, you say question 40 ?
Q In 40, sub part two. Oh, I see, you want to see the question?
A Sure.
Q All right. What I want to know was --
A (Interposing): Question 40 , sub part two.
Q Yes. The bearing capacity. Do you see that?
A Yes.
Q Nas it that you wanted calculations for the bearing capacity or that you wanted new borings?
A In this particular case they have given graph and that graph, from what was that prepared, that graph, I don' $£$ know.

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$$ So all you wantad in that question was the calculations that were used in preparing that graph?

A Let me see this to read completely. Okay, this I ask from the new borings which I have requested.

Okay, so you wanted new borings?

A Yes.
Q Anc new bearing capacity calculations, you weren't interested in that question on the computation or the bearing capacities in the table you aiready had or the graph you already had, is that right?

A Okay, assume the - I assumed that after pre-load the characteristic of the soil in the Diesel-Generator Building had changed, it had become denser than before, and naturally there would be some change in the shear strength parameter, and it would be more realistic that you give the new results.

Q Would you expect the shear strength after the pre-load to increase or decrease from that that it was before the pre-load?

A It will increase.
Q Increase? Can I have that back (indicating).
Who ordginated the request for the addi-
tional borings at the Midland sita?
A It was originated before I joined this. I can't tell the name. I don't know these names.

Q With regard to question number 40 , sub part four, that referre

to a contour map showing the settlement configuration of the Diesel-Generator Building that had been furnished by Consumers In February of 1980 that indicated that the base of the building apparently had warped through differentfal settlements and, therefore, concluded that additional stresses would be induced on the various components as a result of that.

Was there any input from the structural
consultant into that question or was that your question?
A That's foundations, so that is mine. Excuse, me, the question was asked before by some other reviewer I replaced. I changed this language but the question was the same, but I take full responsibility for this.

Q Eave you communicated at all or transmitted any information to or received any from a constructural consultant with regard to stresses induced in the Diesel-Generator Building by the differential settlement?

A I pointed out and sent to NRC this thing and they must have done this thing. I son't know.
Q You don't know whether they did or not?
A No.
Q Question 41 , for -- strike that.
I'd like you to take a look at question 44.
Tell me if you have any idea who wrote that question?
A Oh, I don't have any iciea. It was written before I took over BETZAND SUMMERS. INE.

$3 i l l$ otto, you mean?
Bill Otto, yes.
Okay. Can I have that back (indicating)?
of the stability of the dikes in the PSAR?

A Yes. I never wrote a letter, I say just I would like to see BETZANE SUMMERS. INC.
a copy of the PSAR, perhaps they didn't send one to the corps of Engineers.

> Do you know if anyone ever asked the applicant to send the PSAR to the Corps?

A Frankly I didn't ask anything.
Q Do you know if anyone asked the applicant to send the PSAR to the Corps?

A I don't know.
Q But you did ask Joe Kane for a copy of the PSAR?
A Yes, numerous times I see and I said I want to see this thing, get a copy of that.

Q Were you awars at the time that you prepared question 46 with regard to the emergency cooling pond that there was an analysis of stability of the dikes in the PSAR?

A PSAR I have not seen I told you.
Q I know, but were you aware - well, strike that. Then, I take it, since youhadn't seen the PSAR that at the time that you prepared question 46 you didn't know that there was a discussion of the cooling pond dike stability in the PSAR, did you?

A Let me see that. Okay, this question originally was formulated before I came, but I put some input in that and becaus? we didn't have any soil strangth parameter to verify the stability, so it was requested.
Q At th
aware
you?

A No, I don't, if is present in PSAR, if already conceived about the dike in the PSAR as to the height, strength and shear strength at that level, because I don't know, it looks to me that PSAR is tow preliminary at that time the shear strength was determined.

Q Do you recall what my question was?
A Yes.
Q It was whether at the time you provided any input into question 47 you were aware that there was a discussion 0 the dike in the PSAR. You weren't, were you?

A No, I haven't seen this PSAR, so naturally --
Q (Interposing) : And you hadn't seen it and you also weren't aware that there was a discussion of the dike in there, right?
A No, I wasn't.
Q Okay. Had you reviewed the discussion of the dike in the FSAR at the time you provided input into question 46 ?
A Yes, I have seen the FSAR.
Q What was your problem then with the information that was contained in the PSAR with regard to the cike?

A In the FSAR you have given all final results. From final results I couldn't conclude that this structure is safe. I
would like to see the computations and actual shear strength and how shear strength was determined.

You sure you are not from Missouri?
No.
Q You indicated that you prepared sub part three to question47, and that question relates to dewatering, doesn't it?

A Let me see that. Yes, yes, last few sentences in that paragraph.

Q The last few sentinces referring to including the supporting data for what is asked here and the locations and that kind of stuff?

A Yes.
Why were you fooling around with the dewatering?
That is part of geotechnical too, and it has to do with the hydrologic.

Q Who prepared the rest of that question?
A Some was prepared by - well, perhaps before I join this thing, by Willis Walker.

Q Willis Walker?
A Yes.
Q Ee is from the Tulsa Distric:, isn't he?
A Yes.
Q What elements of dewatering is covered by the geotech as opposed to the hydrologic section?

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Well, when I am student in my school I cover applied soil mechanics, permeability, ali these things come under that, wells.

Well, I mean, what can happen when it comes time to review a dewatering plan, are you going to do it or are the people in hydrologic going to do it? How are you going to decide who does it?

A Perhaps NRC will decide that hydrologic will do that. Q Let's assume they don't or haven't. How would you decide?

A I would not decide because they will direct me to do that and I will confine myself to their direction.

Q I want to refer you again to Figure two in the September 14 th. 1980 submittal by Consumers Power once again. This is included as View Graph 5 of Consumers Exhibit 12 at the Kane deposition of October $15 t h$, and I want to go through with you agsin how you would go about detemining factor of safety at a point, for example, at elevation 618. The reason why I want to go through it again is I went through this with Mr. Kane in his deposition and he indicated that in making that determination you don't take into account the weight of the soil and that's also my understanding of the way it woris, and in doing that if you would go back to my example of the 100 feet of soil --

A (Interposing): Üh-huh.
(Continuing): If you go down, for example, 50 feet into that soil and calculate the weight of the soil you come up with an almost impossible figure for the amount of load.

A Yes, sure.
All right, so in doing the calculations at 618 feet on Figure 2 would you agree that it would be appropriata to take, in determining the load during surcharge, to take the load indicated between line one and line two for the elevation at 618 and add to that --

A (Interposing): Yes. (Continuing) : -- the load indicated between line five and line three, okay, and then divide that by the load incicated between line one and line four, so that what you are doing is you are adding the stress due to the dead load at the tine of surcharge plus the stress due to the surcharge and dividing that by the stress due to the dead load plus live load of the structure after the surcharge when it is in service?

A You mean to deduct this load? Yes.
Q Well, yes -- not to decuct that load necessarily because you are not deducting it, you are just not adding the weight of the soil as an additional stress after the building is completed.

A As I told you before this thing needs scme time for study and you ask me within a minute to make a snap decision that

may be completaly not right, and last time you asked me how much pressure will be created at this level, and --

Q (Interposing) : Yes, and that's why I an coming back to it now because you have had some time to think about it.

No, no, I didn't think about it. I was busy in answering your questions.

Q I mean now you have some -- I am asking you now to take some time and think about it and see if what I fust suggested doesn't make sense to the way you calculate the factor of safety?

A That time which you are giving myself is not enough, but I will tell you some what happened.

Q Okay.
A Actually the most it will be at that level -
Q (Interposing) : When you say at that level you ara ealking now at tha footing level?

A Footing level and then if you consider at that level, that zone that you want to find out this factor of safety, I assume that he has to deduct this one (indicating).

Q Okay, that's zight. Okay, so if you are doing this at elevation 618 you would deduct the weight of the soil?

2 Even here (indicating), this one it would be more reasonable to deduct that.

Q That's right. Even at the footing level it would be more

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representative?

A That's yight, but you don't give me time to think. I will take even one day to thini about that.

Q Okay. I am not trying to rush you. That's why I am coming back to it because I don't want to do anything that is unfair.

Okay, so tell me would it be correct then in deternining the factor of safety at elevation 618 to take the load indicated by the distance between line one and line two and add to that the load indicated between line five and Iine three, that's the live load in thera, all right, so what we would do here is we would add, for the enumerator we would take the dead load during the surcharge program plus the stress associated with the surcharge itself, right, we would add those two together to get the stresses of the structure plus surcharge?

A Enumerator, okay.
Q What about -- okay?
A Uh-huh.
Q Is that okay?
A Uh-huh.

Q And then what we would do is we, for the denominator, the thing that we are going to divice is that we would then take the stress resulting from the dead load glus the live load? A Uh-huh.
 by referring to the numbers associated with these lines on the graph the enumerator would be the load associated distance between line one and line two plus the distance between line five and line threa and the denominator would be the distance between line one and line four - well, line one and line two plus the distance between line two and line four?

I don $t$ know how they wrote this thing, it is all messed up. Well, no, someone will look at this chart -(Interposing) : Why not say that this line (indicating) -well, go ahead.

MR. PATON: May I interrupt here, please, just for a moment, let me ask the witness if he thinks he could follow this better after a break, if we had about a five minute break and he looked at the chart, would that assist you in any way?

A That might help, yes.
MR. PATON: Okay, let's take a five minute oreak and you can look at the chart and if you want we can even decide on some clear way to in this.

A I understand what he asked.
MR. PATON: Yes, I understood his question also. I think maybe you might need a couple of minutes to look at it, is that right?

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A No，I don＇t need this，no．I can look at this，but it i messed $p$ at the same time．
（By Mr．zamarin，continuing）：Let＇s start again and I $w$ break it down this time into two separate guestions firs describing what $I$ am doing and then with reference to th on the chart，okay？

In order to determine the factor of at elevation 618 is it correct that what we would do is． would take the stress attributable to the dead load of $t$ structure－－

A（Interposing）：Uh－huh．
Q（Continuing）：－－and add to that the stress attributabl the surcharge，to the sand－－

A（Interposing）：You want me to correct here or later？
Q Correct here if there is a problem．
A Dead load of the structure at the time of surcharge？
Q That＇s right，ot the time of surcharge．
A That you add to that（indicating）．
Q All right，we take the dead load of the structure at the Time of surcharge and add to that the stress associated the surcharg？loading？

A Okay．


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And then we divide that by the stress resulting from the dead load of the structure after the structure is complet

A (Interposing): Completed structure.
Q (Continuing) : - plus the live load of the structure --
a (Interposing) : Including machinery and everything.
That's right, okay, and then when you do that mathenatica corputation you would come out with a factor of --
(Interposing): No, no, you have to add some more.
Q What?
A To the dewatering.
Q Okay, and then you would subtract what fiom the denominati
A Denominator, you would have to add this one too.
Q You would have to add the stress attributable to the effer of the Cewatering?

A Yes.
Q Right?
A Uh-huh.
Q Now, I know that I am rushing you, but I want to give thi: to you to think about --

A (Interposing): No, I know.
Q All right, when you dewater -- okay, really what you are doing is you are still dealing simply with the weight of soil, right?

A Ye3, the soil what is coming from here to here (indicatine
you see, the pressure douile up by the soil, this dewaterin so that pressure you have to add because it came after, aft this soil is already existing there. That is what you are not considering there.

MR. RAMARIN: shy don't we take about a five minute break.

> (Wh reupon there was a short recess aft which the deposition again continued.) where $=$ don't know if structure was built or not after that. I read settlement calculations in the schools in my graduat course.

Q That's right, that was yesterday. I had forgotten you told us about that. Did I ask you how you went about making that settlement calculation in India, what tests?

A You didn't ask.
Q Good. Can you tell me now?
A No, there were some soil tests there, just bearing tests. There were no other tests.

Q So there was a bearing test and on the basis of that you estimated settlement?

A Settlement, that's right.

SINGH
And because you don't know if the structure was built, th. obviously you don't know how close you were on that predi No, I don't.

MR. PATON: Mr. Singh does have a sta: ment he'd like to add to the prior discussion about Figurt attached to the applicants $9-14-80$ submittal.

MR. 2AMARIN: Is it a statement or cli fication?

MR. PATON: $I$ think it is -- off the record.

MR. ZAMARIN: All right.
(Whereupon there was a short discussic held off the record.)

Q (By Mr. Zamarin, continuing): Just because I am a nice gr I'n going to ask you a question that Mr. Paton and you wor like me to ask and that is what in your opinion would be $t$ very, very, very, very best way to determine the factor of safety with regard to the surcharge of the Diesel Generatr. Building?

A First to get the appropriate shear -- not shear, but stres distribution along the depth below the footing and plot th stress, find out the area of the total stress at the time surcharge and then find on the separate drawing, separate plot total area under full load of the Diesel Generator

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Building and then the factor of safety will be total stress area under surcharg e divided by the total stress area unde: the full load of the Diesel Building.

Q And I understand that in yous opinion that that would be the best way to du it?

A Yes, at that stage, within $10,15=$ inutes of thinking $I$ thin that would be the best.

Q However, by that you are not saying that the method that we described before and went through before on that graph is no an acceptable way of doing it, is it?

A Okay, before that when I say total stress area, from that total stress area the dead load of the earth should be excluced, but dead load due to dewatering should be included Q Okay, so even on your very, very, very, very best method, taking this total area of stress, you would decuct the dead load of the soil but not the dead load of the soil.-
A (Interposing) : Increasing soil --
Q (Interposing) : Strike that, I misspoke.
You would deduct the dead load of the soil but you would not deduct the stress associated with the effect of dewatering, right?

A Yes.
Q Now, again my question was that recognizing what you have just described in your opinion is the best way of detemining

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what the factor of safety is, you would still acyre? thi method that we went through before using that graph on Figure 2 is still an accepted method of determining iac of safety, right?

A We went through two. One we went on the top and one at feet.

Q Right, right.
A In the middle I think is accepted, but the dewatering 1 should not be deducted.

Q I have here what has been marked Exhibit Number 11 of $t$ Kane Deposition a3 of October 15th, 1980, and I want to you if you recall providing any input to Joe Kane so th could prepare a presentation or critique of Consumers. with regard to the Diesel Generator auilding. Do you ri doing that?

A Yes.
Q One of the statements in here is that the state of the $:$ limitations, and he puts that as a Consumer Eower Compar position, using thin samples, meaning the samples that: were taking, the borinçs, the 28 foot samples which you have if you view the Diesel Generator suilding surcharge field test or field experiment, and it gces on and says, however, in some respects we have better control to cupl long term field conditions which did not develop during


preload."
Now, do you agree that there are limit with state of the art because of the use of thin samples a opposed to observing behavior with regard to the full 28 f thick layer?

A I didn't understand the question very well.
Q Okay, let me show you, and there is also some writing in $h$ that you can disregard, but let me show you -- it is Nurise. and Joe Kane said you had some input into that.

A Yes, I had input into that.
Q You did hav input into this remark Number 7?
A Yes.
Q Yes?
A Yes, I gave a graph showing how the sample disturbance sho be minimized.

Q Should be minimized?
A Minimized.
Q Why don't you describe for me how the sample eisturbance should be minimized?

A I gave that, the Schmartmann diagram.
Q I see, so you ara referring to the schmartmann diagram?
A Diagram, right.
C But that really doesn't minimize as much as it corrects foz it, ices it?



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A It is not correction entirely.
Q But it does, it does attempt to correct?
8. But the minimum after that it is acceptable. The state of the art accept that.

The Schmartmann diagram is really nothing more than a typic shape of a curve and then you correct your curve to look sonething like that, right?

A Yes, uh-huh.
Q "Uh-huh doesn't come out neckssarily as a yes. Is your answer to that question a "yes," when you said "uh-huh"? I assume you said "yes"?

A Yes, you take a typical curve or a sample taken -- I assume What you are saying a typical curve meaning the sample has been taken in the nomal procedure taking care of all these things, and the graph has been drawn testing that sample.

Q Yes.

A (Continuing): Then the Schmartmann correction is applied t. make a curve which is very close to which would have been exact curve.

Q So in other words the Schmartmann tells you what shape the curve ought to look like if there are sanple cisturbances, sight?

A Uh-huh, yes.
Q Would you agree that because of the natura of the fill benez the Diesel-Generator Building and the heterogenaity of $t$ ) fill underneath the Diesel-Generator Building that in tak samples and running tests that you would expect to get a scatter of results because of that heterogeneity?

A I believe I saw the boring - there are places you can tz samples and calculate and determine the siear strength or settlement, and using those settlement figures you predic the settlement at various locations. It might be differe in different locations but it can be calculated these thi and that was what would satisfy us.

Q What would isppen if you found that it was different at different locations? That value would you use?

A All different locations?
Q Yes? In other words, let's assume that you took some sam and you had a scatter of results, you had some settlement one place that predicted half an inch of settlement over 1ife of the plant and you had ancther one that precicted sixteench of an inch --

A (Interposing): Okay.
Q (Continuing) : -- you had another one that maybe predicte: thrae inches, what would you do with that information? in would you reconcile those results?

A The structural engineer analyze these things, hew ruch sement you are putting in a place like that, it is not ny p: problem he should take cara of it.
© I see, so what you would do is you would assume that ha to
It is a problem for the construction, somebody has made th the results that you got was a reli>ble prediction of the settlement behavior of that bui'ding, right?

If it has been done carefully, all the prediction there wil be reliable.

9 (How many times have you yourself used that eshractmann procedure to correct for sample disturbanies?

A I have not ust it any, only in the casa of this. I send it
Q (Interposing) : Only in the case you what?
A This Diesel-Generator Building, I draw a graph and send two
Q (Interposing): So that would be the maiden voyage for you in Schmartmann, is that right?

Q Have you done it subsequent to that? After that?
1 No.

Q Did you ever do it before that?

3

That *as the first time?
Yes, I read in books and discuss with my supervisor, especially Jim Simpson and he said that is reliable and he has a lot of experience and based on that $I$ draw the conclu sion that it is okay.

3 Q How many times have you been involved in the selection of appropriate soil properties for use in design?

A No, I have not been involved taking soil property. Excuse me, for that I took help from Ron Ericksen who is our geologist.

Q For what you took help?
A For taking samples and testing regarding those samples.
Q You told us how you would correct for sample disturbance. I would you minimize for sample disturbance?

A That is the minimizing, it is not correct, yeu can't correct 100 percent.

Q You can't what?
A Correct 100 percent.
Q Okay. Is there any way that you minimize the sampling disturbance that you can expect when you are taking the sample?
A I am not expert in that. I take help. I say I take help from Fon Ericksen and he advises me.
Q. What did he tell you ebout that, if anything?

3 Q There is a way he can what?
4 A Take sample with minimum disturbance.
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6 A

## SINGH

He said that there is way he can tell.

But he didn't tell you how he would do that?
A No. I didn't even like because it was too long for me. He for me to do it. It is a waste of my time.

Q Have you ever been involved in selecting locations fror whi borings should be taken?

A I have selected.
Q Where?
A In iuridges.
Q How many?
A On bridges? I have, altogether, I must have designed 50 bridges but, you know, design and review including that, but actually I don't, I am limiting it to review, I am not involved with taking locations of the borings but in design I am involved directly so.

Q About how many, I an soriy?
A That will be approximately 15 to 20 .
Q So with regard to 15 or 20 bridges you have selected the locations of the borings?

A I selectad and give it to them.
Q What tests did you then run? will give me a complete book on that and it is not possible


## SINGH

correcting? In other words, what field test or what $k i$. a situation did you have that you were dealing with? There was a graph given in your FSAR of some test result the graph where they have calculated the sample as being from a certain depth, where that was done from the perce: there, and they took out that graph and then calculated t correct the diagram. The original diagram was from the $F$ and the correction was minor, but the original, the inten that diagram was to show it correct.

Was there any way that you confimed tha fact that your oc tion was accurate or that it worked with regard to this da No. I am saying if that is accurate, you see, the weight corrected the way this graph is done, if I have accurate $d$. from somewhere I can use it.

Okay, so what you have done is you have gone through a corr tion procedure but there is no way that you can know, as yo sit here now, whether this was an accurate correction, righ For this particular input I don't say it. I can't say, but I will do, if I will get the correct information on the same proredure you can tell it.

How do you know though that your correction is accurate? Well, this has been developed by sciontists or engineers so based on that I have been researching that. Q So really all you did was co through a procedure of cerractic

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3 A

40 that Schmartmann -(Interposing): Yes. (Continuing): -- put his name on? Right, yes, and it is tho state of the art and it has been accepted and that's what I did.

I want to show you Note 3 of Table $37-1$, which was an encl sure to the request to Consumers Power Company for additic: borings and sent to them on June 30 th, 1980, and I'd ask $y^{\prime}$ to take a minute and read that, and it starts on November : it starts out saying, "Continuous split spoon sampling" -and if you would just read that.

A I have read it. Did you have any input into Note 3 of Table $37-1$ ?

No, I didn't. I saw this thing and maybe made a few sugges tions in a word here and there but that is not mine. I thi it was -- I think you should talk and this should be clarif. from Ron Ericksen, he is our boring man, investigation expe:
Q I'd like you to read Note 3 and tell me what you thirk that tells Consumers Power Company that they have to do with regard to borings?

A This says, "Continuous isplit spoon sampling should be taken using SPT is required. That means he wants to find cut the different stratum between. That is my interpretation.
Q Is it alsc your interpretation that undisturbed sampling is

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required for each of those areas where the continuous split spoon sampling is done?

A Yes. The continuous samples, sampling should be taken near that, very near or close to that.

Q Not continuous samples, you mean the undisturbed sarples should be taken?

A Undisturbed should be taken.
Q Very near where each of these continuous split spoon samples were taken, right?

A Right. I think that is the intent of that to find out what is the type of layers of these things.
Q In your interpretation of that Note 3 do you think also that undisturbed samples should be continuous?
A No, no.
Q How would you so about determining from what straca they should be taken?
h Let me tell my interpretation.
Q Okay, that's all I am asking you.
A Where the $S P T$ is taken, the penetration there they should have high dense material and then you con't have to take samples there.

Q You just want samples from the worst possible spots?
A 'Shat's the worst case -- no, not worst case, it means very Eim, that is very compressible and likely to lamage the

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structures and then you have to take there to assist, the results of that to assist and so you know the effect of that compressible soil on the stability of the structure.

In other words, don't send us any good news, justsend us the bad news?

If it is good then it is okay.
Yes, okay.
(Continuing) : Let me continue with the intent of my feelinc Suppose you have a very high blow count, the soil is good there so use your judgenent and take the sanple.

Q Right.
A (Continuing) : This has been gone into, this has been discussed before in Fashington, D. C. with the reeting with NRG and Consumers Power, and I was present there.

Q Right, and you listed to what went on there and your understanding of what the intent of note 3 is not only based upo: your uncierstanding of the development of those requests but your reading of it now and the fact that you were at that meeting and you heard this being discussed there?

A There it was discussed, the same thing and we tcld the sare thing, use your judgement.
Q With regard to the pre-lcad program at the Diesel-Generater Duilding, how would you go about estinating piezomater lave pricr to the pre-lcad?

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A Estimate?
3 Uh-huh, would you bother doing that?
4 A I didn't understand what you say.
5 Q 2.11 right. If you are going to do a pre-load like was cone

A During the surcharge I would see piezometer, sure.
Q But you wouldn't estimate what it was zoing to do beforehand, would you?

A I Con't know how.
MR. PATON: I have tried not to interrupt this, but the Witness has obviously misconstrued your questior Your question was not clear that you asked him whether or not he would, prior to surcharge, whather he would do anything $=0$ estimate this, and I was confused by your question, too, and I an sure he is.

MR. Zamarin: He was answering it. ind I understood that he was just going to say that he didn't know how to do it.

MR. PATON: I think he gave an answer because he miscenstrued your question.
A I would like to listen to the question again.
MR. zamarik: Why con't you listen to it again.

A After the surcharge is placed, I don't think I understand your question.

MR. PATON: It could have been more straightforward. : Vuld you read the question back again,


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please.
(Whereupon the Reporter read back the previous question.)

A Beforehand, I assume you were telling me before surcharge and the piezometer I would use only to read the water tab: that's all, the elevation and if there is pravious pressu: There is a lot of tines, but in the Diesal-Generator Buile I don't think there is any previous pressure, but some pla there may be gas or something trapped inside, but in this I will read the piezometer. I will not estinaec, I don't what is reaning of estimate in this particular case, but I will raad the piezometer level and then when the surcharge come then I will like to see those piezometer levels. I w not try to predict. That is compressible soil there, so I will let it go gradually, or I will put half surcharge and let the piezometer disappear and then put another one. Th is the inea of my looking at the piezometer.

MR. ZAMARIN: Could I hear that answer read back, please?
(h)ereupon the Reporter read back the previous answer.)

A (Continuing) : I would like to not see it rise to excessiv heights because thice might be shear failura.
0

I can't predict. You have to test the soil underneath.
In your opinion is there any need to predict the level that the piezometers are going to go to during the pre-load and make that prediction before the pre-load? Is there any need

8 A Yes, I will predict, assume it don't go beyord certain heighOkay, you would predict just to make sure that it dessn't go beyond a certain height because that would indicate a shear failure, right?
possibility of shear failure?


## SINGII

inducing shear failure, right?
A Yes.
Q Nould you predict before pre-load the seftlerent that would :
expected during the pza-lcad?
A Yes, I will precict that.
Q Finy?
A I would calculate this figure first.
Q thy?
A and compare these things.
Q why?
A First I have to know hew nuch sattlerant is coing to place.
Q thy?
A wh?
Q thy?
A Eecause you hava to minimize about a structure that, and I want to eliminate because $I$ have put certain load on that and with that certain load the soils which is underneath, how ruc is going to settle.

Q That do you care about how ruch it is going to asttle as long -- aren't you interested really in the anount of load?

A How I know by the surcharge that the settleant has Lean cem. pleted, how I know that?

Q You wouldn't necessarily knew that on the basis of a prezieti ceforehand, would you?
:io, I will not know if that settlement iz nine inches, or eight incies or 20 inches. I want to eliminate that sett: Right, I understand that.

A That's why I make it.
Q Ch, you just make a prediction to see whether you need to charge it or not?

A Yes, to need means to eliminate that settlement, how much charge I need.

Q I see, so if -- wait a minute, I think I misuncerstood wha you said. You don't predict settlement to detamine the waight of the surcharge load, do you?

A No, I will calculate settlerent only. I will calculate se ment under the structure. I have to know that.

Q Why do you have to know how much settlement you would expe. under the pre-lcad?

A Because pre-load should be under to create that much settl: ment.

Q Okay, but isn't the settlezent pradiction with regard to p: load of compressible material sometimes unreliable?

A Unreliable?
Q Yes?
A Well, that's what I an trying to say.
2 I am saying azen't the preciicticns sometires unraliable ste that it is better to do, for example, what was cone at the

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Diesel-Generator suilding and that is load the structure and watch it and keep the suzcharge on until you get a straight line on a settlament $\log$ time curve?

Fead that question again.
(thereupon the Reporter read back tile previous question.)

A If I do the testing very carefully my settlament will be reasonable, within ten, fifteen percent of misdicted, and kased on that I will put load on, the surcherge load.

Nould it be pessible to get settlament predictions that would be accurats to withint ten or fifteen percent in soils such as we have had underneath the Diesel-Generator Building which are hetercgenous?

A Hetercgenous, okay, go ahead.
Q Iso, go ahead.
A You concluce all the time it is hetarogenous. There are layer: of soil, but you have cifferent types layers and you calcuLate settienent in each and you get accurate results, but you have to do this in every layer, becsuse you have sand, clay, silt, all this and in every layer you calculata the settlemant.

2 You are talking about variation in the vertical distance, and Cosn't it vary uncer the Diesel-Generator Building, in your opirion, in tha herizental distance also?

A Yes, it varies.

30

So if you were to take sample borings in order to calculate this you wouldn't have any dirt left under the building, would you?

Okay, but some of the settlement will be less and sone will be tora, and why not take care of that?

How are you going to find these?
How? You take borings, you will ind it if you have will to do that.

Q If you have what, sir?
A : ilil.
0 : 3111 ?
A liill, yes, to do that.
Q Weren't borings taken under the Diesel-Generator Building already?

A There were borings taken.
Q Did you ever do any settlement calculations based on those Lorfngs?

A You have never given anything to me. You will calculate firs because I an the reviewer. I would like the applicztn to calculate and I will check.

Q Is there a diffaronce between checking and reviewing?
A No, it is saina ihing.
Q It is the same thing is far as you are concerned?

## A Yes.

Q If you had been the reviewer at the tine befora the surcharge program would you have raquested predictions of settlerant beforehand based on borings?

Based upon soil test, accurate soil test I would like to sce them.

Q Ycu would?
A Yes.
Q Okay. To your knowledge nohody evar requested that from Consumers though, did they?

A I don't know. To my knowledge I Con't know.
Q In your opinion if this kind of testing had been done prior . the surcharge and a prediction of settlenent had been ratae what percentage of error would you expect there to have been as a raximum in that prediction?

A If there is a very careful sarple which has carefully been taken and tested, I will predict within ten to fiftean percent.

Q :Jaxinum, right?
A Yes. I might -- I vould consult with ry surervisor regardin these things and based on that, Decause I have not cione very much, that I have already told you, and they advise me that they can pradict wihin ten, fifteen percent.

3 A Soth.
4 Q Simpson, too?
5 A Yes.
8 Q Do you know if anyone has made the calculations of predicted charge?
g a to, I don't.
1: Q Have you reviewed the borilg logs that were furnished with 1: quastion 46 response?

12 A Easponse for 46?
13 Q There were some boring logs furnished, wezen't there?
14 A I have not reviewed the response for question 46 . It has care

17 Q Have you reviewed the response to question 44 ?
15 A No.
12 Q Lave you reviewed the response to question$42 ?$

2: A iny natarial received in my office in last three weeks frem
2: Consumers Power I have not reviewed.
:: $\quad$ In your opinion was the pre-loading of the Eiesel-Gcneratcr Building the best option to deal with the soil fill preblem there?
©: A Yes, I say ao provided it is the proper way, that thay shenta
have the settlament after pre-load and test and satis that it had been achieved, and I agree, and this is I not of the department..

Q I understand.
6 A (Continuing) : But I will add this because the struct

No, I an not involved in that.
In determining when the soil under the Diesel-Gererator
Building is out of primary consolidation, in your opinion will settlenent data provide that information?

Thich settlement data are you talking about? You nean the soil or the building? Are you talking about ti.e settlencat data which I will take from borings or settlenant data you have given to the response of question number 277
Q Settlement data that is actually observed, the real live stuff not the laboratory stuff, would that incicate when the soil is out of primary consolidation?
A I want to know the real settlenent, where is that? Scriebody has taken that.

MR. PATON: He is talking about reasured settlement.

A Wich settlenent, which ioes he mean? I have to have what settlement.

Q Okay, taasurad -- I assura you woulin't have dita unless somebody measured it. I am talking ebout the real live stiff, okay, the settlament data observed, measured, tzken, written down and then done whatever with, plotted and anzlyzed, would that information with regard to the Diesel-Cenerator Building tell wen the soil there is cut of primary consolife=icn? A Tou take zeal data from the settlement, provicad the builcing
is not warping or anything, all the load is doing uniformy and employ this thing it will give you, you can see the curt long enough and that will give this thing, if the building t alreačy been built on a soil which is not consolidated befor and still it is going in primary consolidation and then seccndary consolidation and it will give this.

Q You have seen -- you have sean the settlement versus $\log$ ti: curve for the Diesel-Generator Building that has been subnitted by Consuners Power Company, haven't you?
A Yes, I have seen.
2 Assuning that you don't have this excessive warping tiat you indicate would affact the information on that, would that cu: show a long enough pericd of time to be able to predict whet the soil was out of primary consolication?

A Wo, still I belleve that there is something that should have continued some more because there is not enough data to draw the secondary range at this time.
Q tiny not?
A Vell, I need some more, the scatter is so much thit scmeboly has drawn through that because I can Eraw znother ine on that in a different angie.
Q You are talking about the straight line portion now?
A Yes.
Q So you think that there is too minch of a scatter of Aata?

2 A
$Q$

I want sone more because if you have to draw a straight line if you have mora data then you can predict more accurately. How much more data would you want?

Ckay, beffre answering this thing I would like another question. The soil under the Diesel-Generator Building was not completely saturated, so that is another reason and I con't ecnsicer that represents the consolidation theory of whether it is secondary or primary, and we Eraw conclusions whether is secondary or whether it is prinary, that is what we have ; do so there are two reasons I hzve to essure the soil is just completely saturated, but that is not actually.

Ckay, assuning that the soil was completely saturated except for the top threa feet of the soil underneath the footings would it be your opinion that the curve would predict when tr soil is out of primary consolidation? :io, still be a lot of capillary action and capillary prevents or reciuces the permeability very much due to friction, so sti there will be sone coubt to i.s. Capillary should be complete elininated.

2 The capillary moisture --
(Interposing) : Capillary moisture, you said that three feet is still there, and there is capillary moisture in that so that will pravant your -- that reducas tha permenbility of th soil. Capillary moistura that is not saturated fully, it is

$$
z \varepsilon \because z: \because=s, \therefore \because z=3 . \quad \because \varepsilon
$$

## SI NGiI

partially saturated.
Q ind so is it your opinion that the only tire you can aver predict settlement and preaict when soil is out of primary consolidation from a settlement versus log time curve is whon all of the soil right up to the very botton sice of the load that is applied to it is completaly 3 aturatsd?

A This curve has bsen obtained and is true only for the saturated soil drawn in the laboratory and that is basod on this thin and there is also another thing, the loading, every load incre ment is double the previous load.

Q What do you mean the load increment is double the nrevious load? I con't know what you riazn?

A I mean testing is cone, when you do laboratory testing, and you draw that curve, that is the eriteria.

Q What is the critaria?
A You double the next load. You increase the losd fron one and then two tons, four tons, eight tons, and go on coubling that and on that basis that curve is drawn. That hes soma effect on that primary and secendary consoliciation.

Q Is the shape of the curve, is the general shape of the curve the same for each of these load incrarents that you are talking about?
A So, fcrithole load incra:rents there will be cre curve. This is one curve for the whole thing. There -ight be cna hanczed

## SINGA

lead increments or 20 load increments, it all jepends. To your knowledge does Joe Kane want the Corps of Engineers to be present if borings are taken at the sample tube opening and speciment selection for testing?

No, not to ny knowledge. I am not aware of that because this is handied by Bill Otto and the perscn tho is in charge of borings.

I show you a document which has been marked as Exhibit : Uumsr 6 and ask you to take a look at it and then direct your at.en tion to the last page thereof and to the very last coments writtan on it.
(thereupon the cocument was handed to the Witnass.)

Q (3y Mr. zamerin, continuing): Can you tell me what that dc is ment is, Exhibit Nurber 6?

A That that document is?
Q Yes, is that something from your files?
A. No. Tinis?

2 On the front it says "Singh," co you see what it says there?
A Yes, this top, definitely, this paçe (incicating) I have a cepy of this.

Q Have you seen all of that before?
a Up to this (indicseing), but I con't have copy of it. I isn't have copy of that. This is entirely different.

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When you say that this is different -.. give that to me so that I can cescribe it a little better.

Then you say you don't have a copy of this,
what you are talking abont is the last four pages?
A Okay, Dut I have not seen this thing.
Q All right. On the last nage of this there is sore handwitting. Do you know whose hancwriting that is, like that little subparagraph small letter g. Where it says Jee wants?

A No, I am sorry, I can't recognize who has written it.
Q Do you know whether that is Mr. Otto's hantwriting?
A No, I can't racognize. I Kncw ... I con't think it is Hr. Otto. I know that it is not mine.

Q I know, but does it lock like Nr. Ericksen's handwriting?
A I can't tell that. I am sorry, it is very hard to recognize this but this definitely is not my handwriting.

Q Has Jce Kane ever told you anything generally about Consumers Fower Company?
$A$ No.
Q Never told you what he thinks about them is to whether they are a good utility or a bad utility?

A : *o.
Q Iever told you whether he thinks sechtal is a good or bad engineering outfit?
a tio. Never expressed any kind of opinion --
(Interposing): No.
(Continuing) : -- to you in that regard with resject to Consumers or Bechtel?

A No, sir.
Eid Joe Kane ever talk to you about the fact that your dercsition was going to be taken and the type of thing you raight be asked in your daposition?

A No, I have read his deposition.
Q Okay. What did you think of his deposition? Did you see anything in there that comes to mind as you sit here now about what you disagree?

A I read, but I can't reme-ber all tliese things.
Q All right. Can you remember anything in there on what you thought Joe was wrong or which you disagreed with?

A Ckay, I have read his deposition, not all the portions. Scma of then I haven't received yet.
-hat's right, I haven't either.
A So I have read first day and second day.
Q How about the third day?
A Third day partially I read, but so far I didn't sea that.
Q Ckay. Of what you hava read and that is the first day and the sacond day and part of the third day, did you see anything in thera that you re:inemer that you didn't agrea with

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or where you thought Joe was wrong?
A No, I didn't see it. I didn't disagree anywhere.
So there was nothing in there that you can remember where $j o u$ *ould have answered differently than Joe?

No, at that time I don't remember now what it is, but .(Intarposing): And you naver talked to Joe about what night go on at this deposition here?

A No, sir, he never tells rie. I asked hin and he said, no, he is not supposed to talk.

Q Did you talk to Joe Kane on Wednesday?
A This Vednesday?
Q Yes. Iie was here?
A Yes, I talk to him.
Q What did you talk to him about?
A Oh, he care and we talk and he said that he was going at $1: 30$, and that's all I asked.

Q That's all you communicated as far as Joe kane is concerned?
A Iegarding what?
Q Regarcing any communications with him at all this wesk?
A No.
$0 \quad$ : 0 ?
A Let me see, this weak I didn't talk to him on the telerhone, no, I saw him first time tilis whak et $\epsilon$ ight o'clock in ry office yesterlay, I believe. Is that right? : : o, eay bafore on Wednesday.

Q Did Joe Rane at any time over give you any advice as to what to do in a degosition or what to expect in it?

12 A Yes.
I saw hir yesterday. I saw hin yesterday for the first tina this week, I believe.

Q Ee was here for a few hours yesterday morning and that was the
caly tira you scw him?
9 on hednesday.

A Tell the truth.
Q Anything else?
A : : 0 .
Q Just walked up and said, "Hari, tell the truth," and passed on into the night?

A I'ia telling the truth, that means both of than aivizv re, hin and Jce Kane (indicating Nr. Paten).

Q Dian't Kane tell you anything alse?
A No.
Q Didn't it strike you as odd that that would be the only thing that ha Nould say to you?

A Deg your gazion, I dicn't get the quasticn.

Did it strike you as being odd that that would be the only thing that he would say to you?

A No, he said, the only thing he say is if you want to read my daposition, and I read part of his deposition and I didn't have time to go thzough the complete deposition. I read him and Darl ilood's Ceposition, partly I resd and I like to read Sinpson but I didn't get a copy of this.

Okay. Hava you ever considered whether some kind of a failure of the dike could occur which would affect a category one structure?

A Yes, if it is eatsgory one structure it night be danaged.
Q Well, tell me how that cculd happen?
A I feel that there is pipe, that means discharge pipe going through eategory one -- I am sorry, to the dike and if that dike siides the category one pipe is underneath and it might be damaged.

Q All right, so what you are tellng me is that thare is a catagory one pipe that goes uncerneath the dike? tidarneath, sonewhere under the slope.
Q Okay, and if the aike were to slide it could damage the category ono pipa?
A Yes, yes.
Q In what way? By that I nean what is the mathod by which that Gamage cculd occur? Nsuld it Le crushod? Nould it te brekan,

## SI:!Ga

A
Q Nell, Just estimate whatever you want, make whotever assumption you want and I want you to postulate for ma tha type of failure that would damage in some way a category one pipa?

A Sending.
Q Bending?
A Banding.
Q Okay, tell me how and where the dike would fail and how and where it would bend the categery ene pipa?

A I con't know where it would fail, but if it would fail any. where, say ten feet langth, any langth whera it is loadad with this woight of this slide, suppose that dike slide and the load is on this pipe, I den't know how long, two or three hundred feet long pipe, and it is geing under the dike and the whola dike is sliding and all the load went onto that pipe here, it would bend this way (indicating) and create tromendous pressure on that.

Q This is a discherge pife?
A A discharge pipe.
$Q$ Where is the pige?
A It is under the cooling pond, just noar the ersegancy cooling pone we call it.

Q Euried in the etka?
It depends how zuch land of the dike slide on that.
pene we eall it.

[^0] ripged, moved, clogged or all of the above?

## SING日

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4 A

3 Q

It is under the slope.
So it is buried in the fill that makes up the dike, this pipe? The pige is there somewhere under this.

Dut the pipa is just laying on the botton of the ccoling gond, isn't it?

A It is not the Lotton of tha cooling pond, it is on the edge of it insida the dike.

A11 right, and is that pipe surrounded by fill that the eike is made out of?

A Yes.
Is there any other way in which you can postulata a failure of the dike which would affect a category ona structure or system?

A Any other way I can tell you except other than sliding?
Q Yes, othar than sliding ane damaging that pipe. Is that it?
A Well, that will affect the eategory one structure.
Q And you are not aware of any other way in which the dike could
affect a category one structure, right?
A :To, I can't think of anything eise.
okay. If that pipe were to be moved so that it was down in the natural soil, in the matarial and not in the fill from tha dike would your hypethesized failure of the dike still be able to damaçe it?

A It $5 t 111$ can camağ.
(Intarposing): If it is away from that then definiteiy. In your opinion a sliding failure of the dike couli taka scme of the fill with it, and therefore, bend the pige?

It happens, it happans. If the fill is very good then ot won't happan, but the failure of the slide in the slope it can go to the fill, aad I thinkit will go all the way, very daep.
A. you saying it can or can't?

A It goes, yes, it goes, you have to investigate it and see that I111 is strong enough that it won't develop thera.

MR. 2NMARIN: It is now four o'clock and because of airlino schedules we have to cease the deposition. What I'd like to do is adjourn subject to resumption at some time in the futura as we can agree on and because there has baan soma problea in the past with corplaints about that I think that we ohould note that espacially today I think we have had an actual anount of time çestioning the kitness of not much mere than about Eour houzs, and I don't think va have had four hours bacausa of various matters that tve had to cttend to, but we will adjourn new sine Aie subject to resu:p= tion st a ti:ia znd data to be avraed upen.

SInGH
Do you have any objection to that?
MR. PATON: I, you know, I heard your corments and I have no respense.

MR. zNMARIN: Do you have any objectica
to that?
itR. PATCN: Do I hava ang ebjaction to the continuation of Mr. Singh's dupositicn?

MR. ZNMARIN: Yes.
MR. PAZON: I Con't think it necessary that if I did I wculd have to put it on the racord at this tire. We cbviousiy have scheduling probiens and so I con't have any comm at in response to your question about whether or not I hava an objection at this tine. I an not going to make a statement now that I have no objection or waive any objection that I nay have.

MR. ZANURIN: That I an scying is if you have some objection I will stay hers until initnight and finish it.

UR. PATON: Fina, do what you want. MR. $27 M 2 R I N:$ If you have an objaction soll me now and we will aiter our plans.

MR. FhTCN: I an fust not zesponding to
your statament.
:i2. 2xian土a: You wen't tell te 'hethar

you have an objection or not?

MR. PATON: That's correct.
MR. ZNIARIN: All right, we will adjuuzn and resume at sone futurn date.
(Whereupon the witness was exeused and the Capesition edjournad.)

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STATE OF MICHIGAN ;
COU:TY OF NAYAE ;

## CERTIFICAZE OF 110 GARY PUSLIC

I, Matthew 7 . Sats, of the IIn of BETz AND SUUEERS, INC., a Notary Publie within and Eor the , County of :Hayne, stata of Michizan, Suly eevniasiened and gualified, do heroby certify that the witness whose attached Geposition was taken before me in the before-entitled eausa at the time and place herainbefore set forth, was by mo first duly sworn to testify the truth, the whole truth, and nothing but the truth in the cause aforasaid, that the testimeny ecneained in said deposition was by ne reduced to writing in the presence of said witnesa by means of stenograplyy afterwards transeribed upon a typewriter under my personal supervision and that the said doposition is a true and correct transcript of the whole of the testimony elien given by said witness.

I do further anartify that I aan not cennected by blood or marriage with any of the partien, or their attorneys or agents! that $I$ an not an amployee of elther of then! and shas in mot interested, diractiy or Lndiracty, in the water in aontrovariy, afther as asunsel, aqent, attorney, or otherwiae.

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