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NUCLEAR PRODUCTION

TELEPHONE
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March 15, 1983

Mr. Harold R. Denton, Director
Office of Nuclear Reactor Regulation
U. S. Nuclear Regulatory Commission
Washington, D. C. 20555

Attention: Ms. E. G. Adensam, Chief
Licensing Branch No. 4

Re: Catawba Nuclear Station
Docket Nos. 50-413 and 50-414

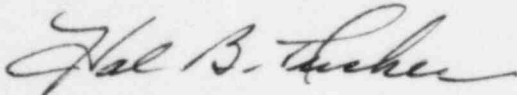
Dear Mr. Denton:

On February 22, 1983 the NRC issued the Safety Evaluation Report (SER) related to the operation of Catawba. Section 1.8 identified a number of Confirmatory Issues which required additional information from Duke.

Attached is a response to the following Confirmatory Issues:

- (6) SSI for buildings not founded on rock (SER Section 3.7.3).
- (8) Vertical seismic response spectra (SER Section 3.9.2.2).

Very truly yours,



Hal B. Tucker

ROS/php
Attachment

cc: Mr. James P. O'Reilly, Regional Administrator
U. S. Nuclear Regulatory Commission
Region II
101 Marietta Street, NW, Suite 2900
Atlanta, Georgia 30303

Mr. P. K. Van Doorn
NRC Resident Inspector
Catawba Nuclear Station

Mr. Robert Guild, Esq.
Attorney-at-Law
P.O. Box 12097
Charleston, South Carolina 29412

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Mr. Harold R. Denton, Director
March 15, 1983
Page 2

cc: Palmetto Alliance
2135½ Devine Street
Columbia, South Carolina 29205

Mr. Jesse L. Riley
Carolina Environmental Study Group
854 Henley Place
Charlotte, North Carolina 28207

Mr. Henry A. Presler, Chairman
Charlotte-Mecklenburg Environmental Coalition
943 Henley Place
Charlotte, North Carolina 28207

Attachment 1

Confirmatory Issue (6)

SSI for buildings not founded on rock (SER Section 3.7.3)

Response:

Attached is an extract from Calculation File CNC-1144.09-01-0010. The lumped mass model used to generate seismic response spectra is shown on sheet 137. Properties of the model are shown on sheet 156. The methodology of using four separate base acceleration time histories is discussed on sheets 157 to 159. Sheets 160 to 167 present typical analysis input data, and sheets 168 to 171 discuss generation of horizontal spectra. Sheets 172 to 189 present the analysis for vertical amplification.

Previous submittals of additional information are discussed and explained on sheets 189.001 to 189.017. Sheets 189.018 to 189.022 address the relative magnitude of vertical and horizontal accelerations. It is shown that the computed acceleration levels are, in fact, in the proper relationship to each other.

Extracts of computer runs "R2TOTSTK", "SPECTRA", and "COMPARE" are included as appropriate to support the calculations presented.

Extract from CNC-1144.09-01-0010

(UNCONTROLLED COPY)
(FOR INFORMATION ONLY)

Dev./Station Catawba Nuclear Station Unit All File No. CNC-114409-01-00
 Subject Seismic Analysis Verification of Published File
CNS-1108.00-00-0002 Using A Stick Model By JMK Date 6-16-82
 Sheet No. 109 of Problem No. Rev 3 Checked By DEB Date 7-1-82

FOR INFORMATION ONLY

10 Overview

A preliminary seismic analysis of the Catawba containment vessel was performed to produce the 'Catawba Containment Vessel Response Acceleration Spectra' for file CNS-1108.00-00-0002, Catawba Nuclear Station Specification For The Response Spectra Displacements For Category I Structures, Volume I. Details of this analysis are shown in calculation file CNC-114409-01-0001. Because the computer print out of the preliminary seismic analysis cannot be located and the 13th ring stiffener has been relocated since the original seismic analysis was performed, a verification of the response spectra used for the Catawba containment vessel is necessary. This calculation investigates the validity of the 'Catawba Containment Vessel Response Acceleration Spectra' in published file CNS-1108.00-00-0002.

The model used for analysis will be analogous to the model used in the preliminary seismic analysis of 1974. The seismic analysis will adhere to NRG 1.92 and Catawba Nuclear Station FCAR section 3.7.

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Dev./Station Catawba N.S.Unit All File No. CNC-1144, 07-01-0010Subject Stick ModelBy JMK Date 6-16-82Sheet No. 110 of _____ Problem No. Rev 3 Checked By DEB Date 7-1-821.1 Nuclear Safety Related

This analysis is a Q.A. Condition 1 item and deemed nuclear safety related.

1.2 Finite Element Model

The seismic analysis model is a one-dimensional linear elastic dynamic model analyzed using 'STRUDL'.

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Dev./Station Catawba N.S.
 Subject Stick Model

Unit All File No. CNC-1144.09-01-001

By JMK Date 6-16-82

Sheet No. 111 of _____ Problem No. 1-13 Checked By DEB Date 7-1-82

1.3 Applicable Codes, Standards, and References

1.3.1 Catawba FSAR, Section 3.7,
 'Seismic Design'.

1.3.2 Nuclear Regulatory Guide, Section 1.92

1.3.3 MEAUTO STRUDL

1. STRUDL Users Manual Appendices - October 1980

2. STRUDL User Manual - April 1981

1.3.4 Calculation File CNC-1144.09-01-0001,
 "Preliminary Analysis"

1.3.5 Calculation File CNC-1144.09-01-0008,
 "Analysis of the Steel Containment
 Vessel for Non-Axisymmetric Localized
 Effects due to the Equipment Hatch
 and Personnel Lock"

1.3.6 Calculation File CNC-1144.09-01-0004,
 "Steel Containment Vessel Analysis
 for Static and Dynamic Loads"

1.3.7 Drawings CN-1042-1 to 5

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Dev./Station Catawba NS
 Subject Stick Model

Unit All File No. CNC-1144.09-01-a

Sheet No. 112f Problem No. Key 3

By JMK Date 6-16-82

Checked By DEB Date 8-24-82

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1.4 Assumptions

All assumptions will be stated in the body of the calculation.

1.5 Computer Programs

Referenced Computer Runs (STRUDL)

<u>Name</u>	<u>Date</u>	<u>Description</u>
R2TOTSTK	7/28/82	Dynamic Modal Analysis (3-Modes) Transient Time History Analysis. Computes and stores Four individual Horizontal and Vertical Total Acceleration VS. Period Response Spectra per Specified Joint
SPECTRA	7/29/82	Restores Horizontal and Vertical Response Spectra from computer program "R2TOTSTK" and computes the total Horizontal and Vertical Response Spectra per Specified Joint
R2RELSTK	7/28/82	Dynamic Modal Analysis (3-Modes) Transient Time History Analysis; Computes and stores Four individual Horizontal and Vertical Relative Acceleration VS. Period Response Spectra per Specified Joint

Dev./Station Catawba N.C.
 Subject Stick Model

Unit A11 File No. CNC-1144,09-01-0010

By JMK Date 6-16-82

Sheet No. 1/3 of 1 Problem No. Rev 3 Checked By DEB Date 8-24-82

1.5 continued

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<u>Name</u>	<u>Date</u>	<u>Description</u>
<u>RELSPEC</u>	<u>7/29/82</u>	<u>Restores Horizontal and Vertical Response Spectra from computer program "R2RELSTK" and compute the relative Horizontal and Vertical Response Spectra per Specified Joint</u>
<u>COMPARE</u>	<u>8/23/82</u>	<u>Determines the acceleration amplification between the generated response at specified joints and the generated ground response. Then calculates the percentage increase. Also determines the acceleration amplification between the adjusted response at specified joints and the FSAR vertical response spectra. Then calculates the percentage increase.</u>

Note: Computer output "R2TOTSTK", "SPECTRA", and "COMPARE" are included in Volume 7.

Computer output "R2RELSTK" and "RELSPEC" are included in Volume 8.

Dev./Station Catawba N.S. Unit All File No. CNC-1144.09-01-001
Subject Stick Model

By JMK Date 6-16-82
Sheet No. 114 of _____ Problem No. Rev 3' Checked By DEB Date 7-1-82

2.0 Model

A stick model of the containment vessel will be used to generate the building response spectra. This model will be based on the original model used in the preliminary analysis calculation file CNC-1144.09-01-001. The relocation of the 13th ring stiffener (which deviates from the original model) is incorporated in the stick model. The mass distribution in the original model implements a smearing technique. To avoid the smearing technique, mass and mass concentrations are applied to mass points with respective elevations so as to reasonably represent the containment vessel. Member properties including moment of inertia, cross-section area, and shear area are calculated in the same manner as the original model.

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Dev./Station Catawba N.S. Unit A11 File No. CNC-1144,09-01-001
Subject Stick Model

By JMK Date 6-17-82
Sheet No. 115 of 1 Problem No. Rev 3 Checked By DEB Date 7-1-82

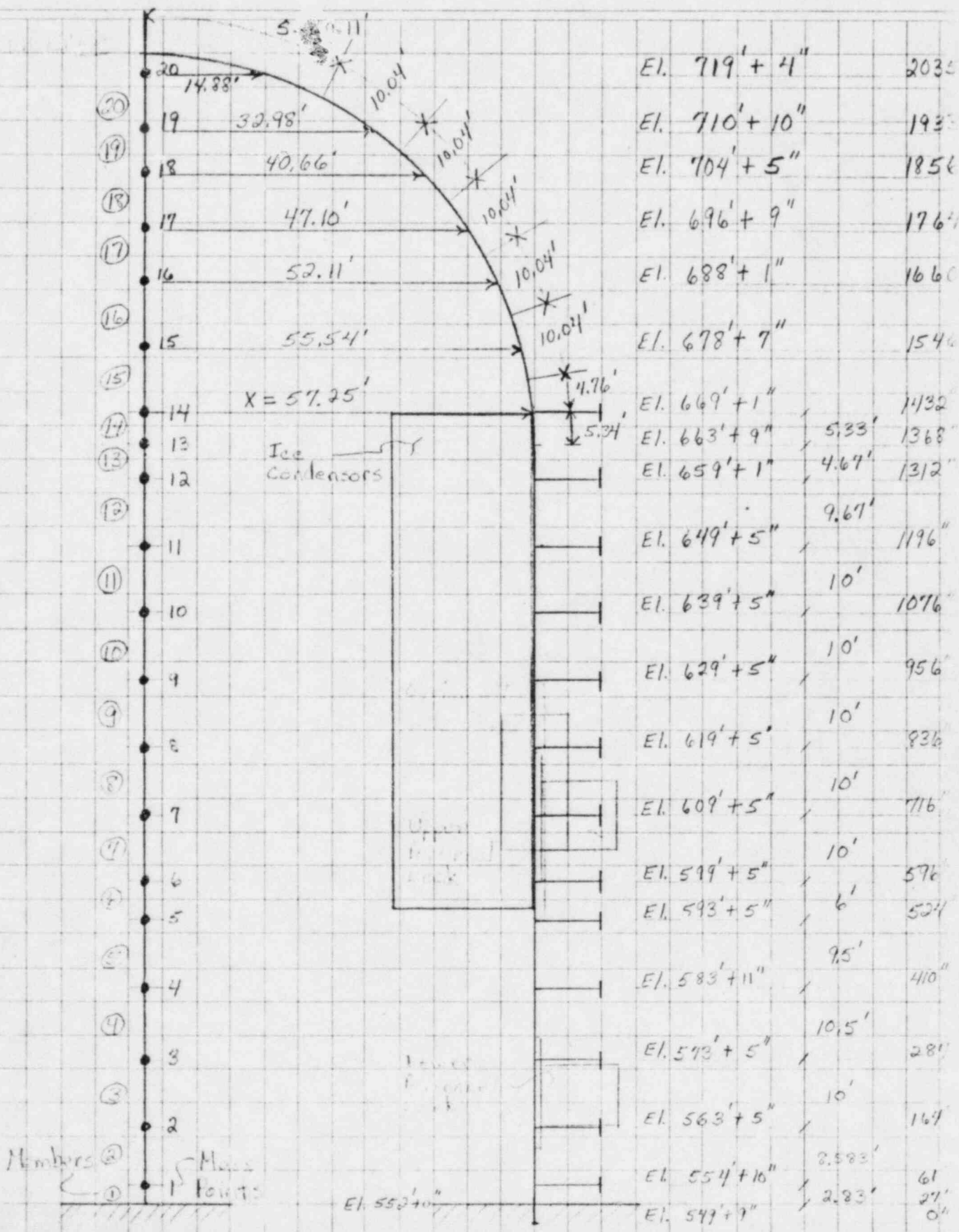
2.1 Containment Weight For Lumped Mass Points

The weight of the containment vessel, penetrations, equipment hatch, personnel locks, ice condensers, ring stiffeners, and vertical stiffeners are calculated and the mass distributed to mass points with respective location.

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Dev./Station Catawba N.S. Unit All File No. CNC-1144.09-01-0010
 Subject Stick Model

By JMK Date 6-23-82
 Sheet No. 137 of Problem No. Rev 3 Checked By DER Date 7-22-82



Dev./Station Catawba N.S.Unit All File No. CNC-114409-21-001Subject Stick ModelBy JMK Date 6-24-82Sheet No. 156 of _____ Problem No. Rev 3Checked By DEB Date 7-23-82**UNCONTROLLED**2.3 Stick Model Data

The input data for computer program "JMKSTK" to generate response spectra for Catawba Nuclear Station Units 1 & 2 is tabulated below:

Joint No.	X Coord. (ft)	Y Coord. (ft)	Member No.	Weight (kips)	Area (ft ²)	Shear Area (ft ²)	Moment of Inertia (ft ⁴)
1	0	0.0					
2	0	2.83	1	160.44412	32.607	11.3035	53945.48
3	0	11.42	2	202.10272	32.607	16.3035	53945.48
4	0	21.42	3	212.57616	25.13	12.57	41585.138
5	0	31.92	4	205.97073	25.13	12.57	41585.138
6	0	41.42	5	163.23697	25.13	12.57	41585.138
7	0	47.42	6	224.45838	25.13	12.57	41585.138
8	0	57.42	7	284.66869	25.13	12.57	41585.138
9	0	67.42	8	275.13342	25.13	12.57	41585.138
10	0	77.42	9	254.52208	25.13	12.57	41585.138
11	0	87.42	10	243.71347	25.13	12.57	41585.138
12	0	97.42	11	243.34579	25.13	12.57	41585.138
13	0	107.08	12	188.02149	25.13	12.57	41585.138
14	0	111.75	13	96.57234	25.13	12.57	41585.138
15	0	117.08	14	138.51967	20.66	10.33	34011.2
16	0	126.63	15	98.35737	20.30	10.15	32286.7
17	0	136.05	16	72.28310	19.38	9.69	28078.7
18	0	144.73	17	84.79279	17.86	8.93	21978.1
19	0	152.42	18	73.38804	15.80	7.90	15211.1
20	0	158.85	19	58.40523	13.26	6.63	8988.4
21	0	167.89	20	73.02222	8.61	4.31	2467.1

$$E = 29 \times 10^6 \text{ PSI} = 29 \times 10^6 \left(\frac{147}{1000} \right) = 4176,000 \text{ ksi}$$

Dev./Station Catawba N.C.Unit #11 File No. CNC-1144.07-01-00Subject Stick ModelBy JMK Date 6-22-82Sheet No. 1571 Problem No. Rev 3Checked By DEB Date 7-23-82

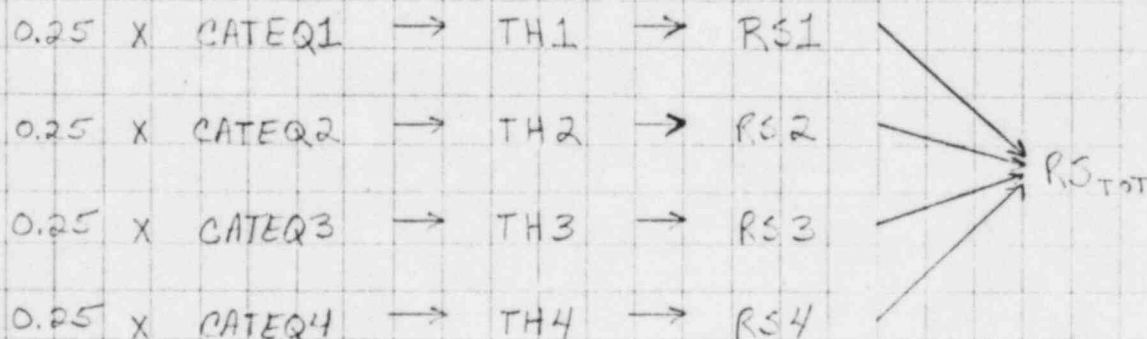
3.0 Seismic Analysis

The intent of the seismic analysis is to generate a response spectra for the Catawba containment vessel. A response spectrum can be defined as the representation of the maximum response of a single mass system for a varying frequency range to a defined base motion. The time history of the mass points is used as the base motion to obtain the response spectrum. The time history of the mass points is generated from transient loads, which are four synthetic earthquakes. The numerical average for the response of the four earthquake time-histories is used to generate the final response spectrum.

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Dev./Station Catawba N.S.Unit A11 File No. C110-1144/17-01-001Subject Stick ModelBy JMK Date 6-22-82Sheet No. 158 ofProblem No. Rev 3Checked By DEBDate 7-23-823.1 Input Data**UNCONTROLLED**

The input transient loads are four synthetic earthquakes noted as CATEQ1, CATEQ2, CATEQ3, and CATEQ4. Because each earthquake is assumed to have an equal contribution, each will be multiplied by a factor of .25. Through the STRUDL/DYNAL analysis the input transient loads are used to generate the corresponding time-histories (TH₁₋₄) and in turn the response spectra (RS₁₋₄). Since each response spectra only represents 25% of the response to the design input transients, the response spectra are summed by absolute value to determine the total response (RS_{TOT}) for each given location. The following figure demonstrates the procedure above:



where: $RS_{TOT} = |RS1| + |RS2| + |RS3| + |RS4|$

Dev./Station Catawba N.S.Unit All File No. CNC-114/09-01-0010Subject Stick ModelBy JMK Date 6-27-82Sheet No. 159 of _____ Problem No. Rev 3Checked By DEB Date 7-23-823.1 continued**UNCONTROLLED**

According to Catawba FSAR 3.7.2.6, an earthquake ground motion (CATEQ1, CATEQ2, CATEQ3, CATEQ4) is to be applied in a N-S and E-W horizontal direction simultaneously with motion applied in the vertical direction. Since the stick model is symmetric, an earthquake ground motion need only be applied in one horizontal direction simultaneously with motion applied in the vertical direction. Thus, 25% of CATEQ1, CATEQ2, CATEQ3, and CATEQ4 will be applied in both the horizontal and vertical directions. Because the horizontal response due to an earthquake ground motion applied in the vertical direction is negligible compared to the horizontal response due to an earthquake ground motion applied in the horizontal direction, the total horizontal response spectra will be calculated as stated on page 158 using only the horizontal response due to an earthquake ground motion applied in the horizontal direction. The same is true for the total vertical response spectra.

The following 8 pages are typical listings of the computer programs used in the STRUDL/DYNAL seismic analysis.

DEB 7-27-82


```

//R2TUTST JOB (6362CATA,D037,,99),2KELLYJK,MSGCLAS A,CLASS=J,
// TIME=60,NOTIFY=JMK8362
//*MESSAGE OPERATOR: PLEASE ALLOW A CPU TIME OF 60 MINUTES
// EXEC STRUCL,CORE=1900K,CLOCK=60
//UD1 DD DSN=TST.R030.C6362.T01.SPECTRA,
// DISP=(NEW,CATLG,DELETE),UNIT=3330V,MSVGP=TEST,
// DCB=(LRECL=80,RECFM=FB,BLKSIZE=6080),
// SPACE=(6144,(100,100),RLSE)
STRUCL 'CATANBA' 'SEISMIC RESPONSE SPECTRA'
UNITS FEET LBM POUNDS CYCLES
TYPE PLANE FRAME

```

```

JOINT COORDINATES
1 0. 0. 0. S
2 0. 2.83
3 0. 11.42
4 0. 21.42
5 0. 31.42
6 0. 41.42
7 0. 47.42
8 0. 57.42
9 0. 67.42
10 0. 77.42
11 0. 87.42
12 0. 97.42
13 0. 107.08
14 0. 111.75
15 0. 117.08
16 0. 126.63
17 0. 136.05
18 0. 144.73
19 0. 152.41
20 0. 158.85
21 0. 167.29

```

```

MEMBER INCIDENCES
1 1
2 2
3 3
4 4
5 5
6 6
7 7
8 8
9 9
10 10
11 11
12 12
13 13
14 14
15 15
16 16
17 17
18 18
19 19
20 20
21 21

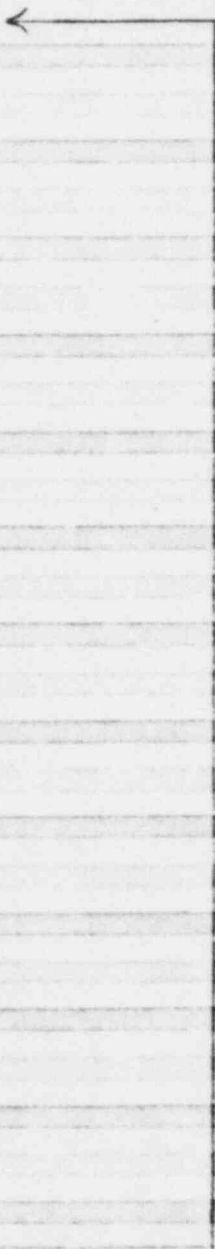
```

```

MEMBER PROPERTIES PRISMATIC
1 2 AX 32.66 AY 16.33 IZ 58280.9
3 TO 13 AX 25.13 AY 12.57 IZ 45837.9
14 AX 20.66 AY 10.33 IZ 34011.2
15 AX 20.30 AY 10.15 IZ 32286.7
16 AX 19.38 AY 9.69 IZ 28078.4
17 AX 17.86 AY 8.93 IZ 21978.1
18 AX 15.80 AY 7.90 IZ 15211.1

```

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Stick Model!

- includes:
1. Joints
 2. Members
 3. Cross-section Area
 4. Shear Area
 5. Moment of Inertia
 6. Mass Points
 7. Lumped Mass

Pg 160
 CME-1144, 09-01-0010
 JMK 6-29-82
 DEB 7-23-82

UNITS INC 3
 CONSTANTS
 E 29000000. ALL
 PUISSON .3 ALL
 INERTIA OF JOINTS ADD
 2 LINEAR ALL 162909.7
 3 LINEAR ALL 203048.9
 4 LINEAR ALL 213707.5
 5 LINEAR ALL 205842.6
 6 LINEAR ALL 162264.2
 7 LINEAR ALL 224514.4
 8 LINEAR ALL 284668.7
 9 LINEAR ALL 275139.3
 10 LINEAR ALL 254630.0
 11 LINEAR ALL 243893.0
 12 LINEAR ALL 243632.2
 13 LINEAR ALL 188218.4
 14 LINEAR ALL 96592.3
 15 LINEAR ALL 138519.7
 16 LINEAR ALL 98357.4
 17 LINEAR ALL 92283.1
 18 LINEAR ALL 84229.6
 19 LINEAR ALL 72824.8
 20 LINEAR ALL 58405.2
 21 LINEAR ALL 79028.0

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Page 101

UNITS FEET
 DAMPING RATIO .01
 TRANSIENT LOAD 1
 SUPPORT ACCELERATIONS
 TRANSLATION X FILE 'CATEQ1' FACTOR .25
 INTEGRATE FROM 0. TO 20. AT DELTA .01
 END
 TRANSIENT LOAD 2
 SUPPORT ACCELERATIONS
 TRANSLATION X FILE 'CATEQ2' FACTOR .25
 INTEGRATE FROM 0. TO 20. AT DELTA .01
 END
 TRANSIENT LOAD 3
 SUPPORT ACCELERATIONS
 TRANSLATION X FILE 'CATEQ3' FACTOR .25
 INTEGRATE FROM 0. TO 20. AT DELTA .01
 END
 TRANSIENT LOAD 4
 SUPPORT ACCELERATIONS
 TRANSLATION X FILE 'CATEQ4' FACTOR .25
 INTEGRATE FROM 0. TO 20. AT DELTA .01
 END
 TRANSIENT LOAD 5
 SUPPORT ACCELERATIONS
 TRANSLATION Y FILE 'CATEQ1' FACTOR .25
 INTEGRATE FROM 0. TO 20. AT DELTA .01
 END
 TRANSIENT LOAD 6
 SUPPORT ACCELERATIONS
 TRANSLATION Y FILE 'CATEQ2' FACTOR .25
 INTEGRATE FROM 0. TO 20. AT DELTA .01
 END
 TRANSIENT LOAD 7
 SUPPORT ACCELERATIONS
 TRANSLATION Y FILE 'CATEQ3' FACTOR .25
 INTEGRATE FROM 0. TO 20. AT DELTA .01
 END

Note: Multiplication factor of one applied as stated on page 159.

← Reference Catwaba FCHP 3.7.1.3
 ← CATEQ1 applied in horizontal X-direction
 ← CATEQ2 " "
 ← CATEQ3 " "
 ← CATEQ4 " "
 ← CATEQ1 applied in the Vertical Direct.
 ← CATEQ2 " "
 ← CATEQ3 " "

DEC 1964
 JMK
 6-29-64
 7-23-64

TRANSLATE Y FILE 'CATEQ4' FACTOR .25
INTEGRATE FROM 0. TO 20. AT DELTA .01
END

ASSEMBLE FOR DYNAMICS
INDEPENDENT DEGREES OF FREEDOM SELECT
CONDENSE DYNAMIC MATRICES
MODAL ANALYSIS HOW MAX 50.
LIST DYNAMIC PARTICIPATION FACTORS
LIST DYNAMIC NORMALIZED MODES
TRANSIENT ANALYSIS LOADS 1 2 3 4 5 6 7 8
STORE TRANSIENT 'HORIZ'
USE 1 2 3 4 FROM TIME 0. TO 20.
JOINT TOTAL ACCELERATIONS
6 9 14 17 20 TRANSLATION X
END

STORE TRANSIENT 'VERT'
USE 5 6 7 8 FROM TIME 0. TO 20.
JOINT TOTAL ACCELERATIONS
6 9 14 17 20 TRANSLATION Y
END

COMPUTE STORE TRANS 'HORIZ' 'VERT'

UNITS RADIANS
STORE SPECTRUM 1
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
OSCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'HORIZ'
USE TRANS 1 2 3 4
JOI TOTAL ACCEL
6 TRANS X
END

STORE SPECTRUM 2
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
OSCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'HORIZ'
USE TRANS 1 2 3 4
JOI TOTAL ACCEL
9 TRANS X
END

STORE SPECTRUM 3
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
OSCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'HORIZ'
USE TRANS 1 2 3 4
JOI TOTAL ACCEL
14 TRANS X
END

UNCONTROLLED

STORE SPECTRUM 4
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
OSCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'HORIZ'
USE TRANS 1 2 3 4
JOI TOTAL ACCEL
17 TRANS X
END

STORE SPECTRUM 5
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
OSCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'HORIZ'
USE TRANS 1 2 3 4
JOI TOTAL ACCEL
20 TRANS X
END

STORE SPECTRUM 6

Modal Analysis required
for upcoming transient analysis

Transient Analysis

Horizontal Time-History Defined
For Specified Joints

Vertical Time-History Defined
For Specified Joints

Computes & Stores Horizontal & Vertical Time-History

Defines input for seismic
response spectra for joint 6
(required for synthetic earthquake).

Reference Catalog
FCAR 27.1.2

" Joint 9 "

" Joint 14 "

" Joint 17 "

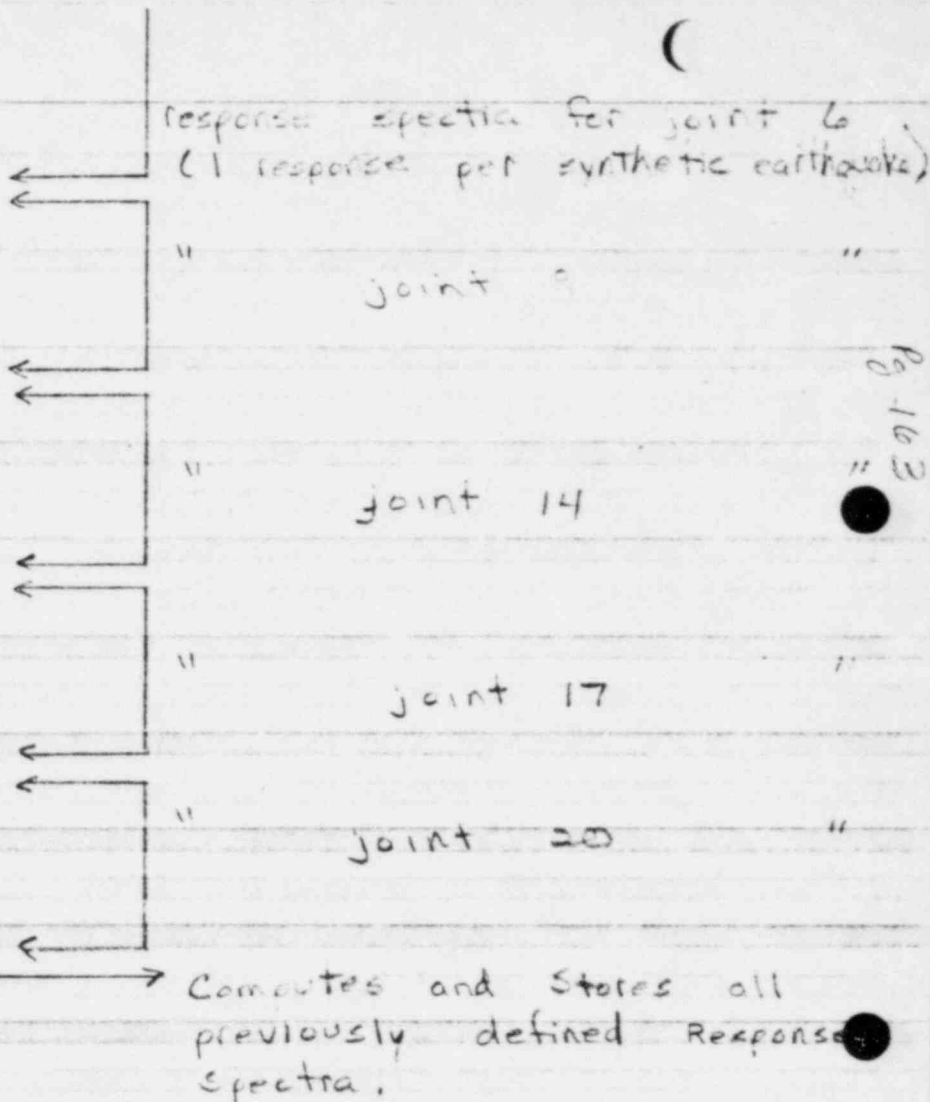
" Joint 20 "

ONE-1144/01-01-0010
JMK 6-29-82
DEB 7-23-84

```

USCIL DAM (RATIO .005 .01 .02 .05
USE STORE 'VERT'
USE TRANS 5 6 7 8
JOI TOTAL ACCEL
6 TRANS Y
END
STORE SPECTRUM 7
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
USCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'VERT'
USE TRANS 5 6 7 8
JOI TOTAL ACCEL
9 TRANS Y
END
STORE SPECTRUM 8
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
USCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'VERT'
USE TRANS 5 6 7 8
JOI TOTAL ACCEL
14 TRANS Y
END
STORE SPECTRUM 9
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
USCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'VERT'
USE TRANS 5 6 7 8
JOI TOTAL ACCEL
17 TRANS Y
END
STORE SPECTRUM 10
RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.
USCIL DAMP RATIO .005 .01 .02 .05
USE STORE 'VERT'
USE TRANS 5 6 7 8
JOI TOTAL ACCEL
20 TRANS Y
END
COMPUTE STORE SPECTRA 1 2 3 4 5 6 7 8 9 10
SAVE 'SPECTRA' SCAN CHECK
FINISH
/*

```



UNCONTROLLED

000-1144,09-01-0010
 JMR 6-29-52
 DEB 7-23-82

```

//R2TOTSP JOB (8362CATA,0037,,99),2KELLYJK,MSGCLASS=,CLASS=F,
// TIME=20,NOTIFY=JMK8362
//*MESSAGE OPERATOR...PLEASE ALLOW A CPU TIME OF 20 MINUTES
//STEPA EXEC STRUDRAW,CURE='400K',CLOCK=20,
// PLOTAPE='CNS.PLOT.T11',PLOTPRO=SPECTEN
//DD1 DD DSN=TS1.R030,C8362.TOT.SPECTRA,DISP=OLD,
// DCM=(LRECL=80,RECFM=FB,BLKSIZE=6080)
STRUDL 'CATAWBA' 'CONTAINMENT RESPONSE SPECTRA'
RESTORE 'SPECTRA'
UNITS CYCLES
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STORE 1
USE TRANS ANAL 1 2 3 4
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCE
6 TRANS X
END SUBSET
END
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STORE 2
USE TRANS ANAL 1 2 3 4
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCE
9 TRANS X
END SUBSET
END
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STORE 3
USE TRANS ANAL 1 2 3 4
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCE
14 TRANS X
END SUBSET
END
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STORE 4
USE TRANS ANAL 1 2 3 4
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCE
17 TRANS X
END SUBSET
END
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STORE 5
USE TRANS ANAL 1 2 3 4
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCEL
20 TRANS X

```

UNCONTROLLED

This program rotates the horizontal and vertical response spectra. Then for each joint (6,9,14,17,20), the total response spectrum per joint is calculated by summing the absolute values of the response spectra calculated for each joint due to synthetic earthquakes CATEQ1, CATEQ2, CATEQ3, & CATEQ4.

pg 164

ENC-114409-01-000
 JMK 6-29-82
 DEB 7-23-82

END
 FINISH
 //STEP2.SYSIN DD *
 CATAKBA CONTAINMENT VESSEL - SEISMIC
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 593.42
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 593.42
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 593.42
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 593.42
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 619.42
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 619.42
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 619.42
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 619.42
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 663.75
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 663.75
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 663.75
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 663.75
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 688.05
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 688.05
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 688.05
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 688.05
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 710.85
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 710.85
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 710.85
 HORIZONTAL RESPONSE TO UBE (.08G) AT EL 710.85
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Pg. 165

ENC-1144,09-01-0010
 JMK 6-29-82
 DFR 7-23-81

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//R2TUTSP JOB (8362CATA,D037,,99),2KELLYJK,MSGCLASS=,CLASS=F,
// TIME=20,NUTIFY=JMK8362,PTY=12
//*MESSAGE OPERATOR...PLEASE ALLOW A CPU TIME OF 20 MINUTES
//*AFTER R2JMKSP1
//STEPA EXEC STRUDRAW,CORE='400K',CLUCK=20,
// PLOTAPE='CNS.PLOT.TT2',PLOTPRU=SPECTEN
//DD1 DD DSN=TS1.R030.C8362.TUT.SPECTRA,DISP=OLD,
// DLB=(LRECL=80,RECFM=FB,BLKSIZE=6080)
STRUDL 'CATAWBA' 'CONTAINMENT RESPONSE SPECTRA'
RESTORE 'SPECTRA'
UNITS CYCLES
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STUKE 6
USE TRANS ANAL 5 6 7 8
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCEL
6 TRANS Y
END SUBSET
END
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STUKE 7
USE TRANS ANAL 5 6 7 8
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCEL
9 TRANS Y
END SUBSET
END
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STUKE 8
USE TRANS ANAL 5 6 7 8
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCEL
14 TRANS Y
END SUBSET
END
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STUKE 9
USE TRANS ANAL 5 6 7 8
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCEL
17 TRANS Y
END SUBSET
END
OUTPUT SPECTRUM
PUNCH ON
OUTPUT G TRUE ACCEL VS PERIOD
USE STUKE 10
USE TRANS ANAL 5 6 7 8
USE DAMP ALL
COMBINE SPECTRA PEAK
JOINT TOTAL ACCEL

```

UNCONTROLLED

This program rectains the horizontal and vertical response spectra. Then for each joint (6,9,14,17,20), the total response spectra for joint is calculated by summing the absolute values of the response spectra's calculated for each joint due to synthetic earthquakes CATEQ1, CATEQ2, CATEQ3, CATEQ4.

Pag-166
 ENCL-1114439-01-00
 JMK 6-29-82
 DER 7-22-82

END SUBSE

END

FINISH

//STEP2.SYSIN DD *

CATANBA CUNTAINMENT VESSEL - SEISMIC	
VERTICAL RESPONSE TO UBE (.08G) AT EL	593.42
VERTICAL RESPONSE TO UBE (.08G) AT EL	593.42
VERTICAL RESPONSE TO UBE (.08G) AT EL	593.42
VERTICAL RESPONSE TO UBE (.08G) AT EL	593.42
VERTICAL RESPONSE TO UBE (.08G) AT EL	619.42
VERTICAL RESPONSE TO UBE (.08G) AT EL	619.42
VERTICAL RESPONSE TO UBE (.08G) AT EL	619.42
VERTICAL RESPONSE TO UBE (.08G) AT EL	619.42
VERTICAL RESPONSE TO UBE (.08G) AT EL	663.75
VERTICAL RESPONSE TO UBE (.08G) AT EL	663.75
VERTICAL RESPONSE TO UBE (.08G) AT EL	663.75
VERTICAL RESPONSE TO UBE (.08G) AT EL	663.75
VERTICAL RESPONSE TO UBE (.08G) AT EL	688.05
VERTICAL RESPONSE TO UBE (.08G) AT EL	688.05
VERTICAL RESPONSE TO UBE (.08G) AT EL	688.05
VERTICAL RESPONSE TO UBE (.08G) AT EL	688.05
VERTICAL RESPONSE TO UBE (.08G) AT EL	710.85
VERTICAL RESPONSE TO UBE (.08G) AT EL	710.85
VERTICAL RESPONSE TO UBE (.08G) AT EL	710.85
VERTICAL RESPONSE TO UBE (.08G) AT EL	710.85

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Page 167

CNC-114409-01-001
JML 6-29-82
DEB 7-23-82

Dev./Station Catawba N.S.
 Subject Stick Model

Unit All File No. CNC-1144.09-01-821

Sheet No. 168 of _____ Problem No. Rev 3

By JMK Date 6-30-82

Checked By DEB Date 8-24-82

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3.1 continued

Listings of the computer programs used in the STRUDL/DYNAL seismic analysis are shown in Appendix A.

The ground response spectra generated due to the four synthetic earthquake time histories used to simulate the horizontal ground motion will be compared to the Catawba FSAR specified ground response spectra shown in FSAR Figure 2.5.2-6. The figures in Appendix B show the generated ground response spectrum for oscillator damping values of .5, 1.0, 2.0, and 5.0 % as well as the Catawba FSAR ground response spectra plotted on these generated spectrum. In reviewing these figures, it is seen that the generated ground response spectrum, with the magnitudes of acceleration being slightly greater, reasonably represents the shape of the Catawba FSAR specified ground response spectrum.

Note: Appendices A thru K are included in Volume 6, "For Information Only".

Note: Section 3.7, Seismic Design, of the Catawba FSAR and FSAR Figure 2.5.2-6 are shown in Appendix K.

Div/Station Catawba N.C.
 Subject Stick Model

Unit All File No. CNS-1144.02-01-001.

Sheet No. 16 of Problem No. Rev 3

By JMK Date 6-30-82

Checked By DEB Date 8-24-82

3.2 Results

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The results of the seismic analysis will include modes with frequencies less than 33 hertz, time histories at specified joints, and horizontal total acceleration vs. period response spectra. In addition, action item number 10 of the NRC Structural Audit concerning the assumption of negligible amplification in the vertical seismic analysis will be addressed.

Three modes had frequencies less than 33 hertz. The first mode is a horizontal flexure mode, the second is a vertical axial mode, the third is the second horizontal flexure mode. These mode shapes are shown in Appendix C.

Time histories were generated for six joints from the transient analysis mentioned in section 3.1. The first joint was chosen to develop a time history at ground elevation. The five remaining joints were chosen so as to develop time histories at the same elevations as the response spectrum in published file CNS-1108.00-00-0002, Volume I, Appendix A "Containment Vessel Seismic

Div./Station Catawba N.S.
 Subject Stick Model

Unit All File No. CNC-1144.09-01-001

Sheet No. 170 of

Problem No.

Rev 3

Checked By

By JMK

Date

6-30-82

Date

8-24-82

UNCONTROLLED

3.2 continued

Response Spectra (ORE)". These time histories incorporated the oscillating damping values stated in Catawba FSAR section 3.1.2 of .5, 1.0, 2.0, and 5.0% damping for ORE. The time history plots for each damping at each elevation are shown in Appendix D.

The horizontal total acceleration vs. period response spectrum, generated by this calculation, were calculated by adding rigid body support motions to those from modal contributions. This resulted in the total acceleration versus time history at each selected elevation. From these time histories, the response spectra for single degree of freedom oscillators were generated for the damping ratios required. These horizontal response spectrum, developed from computer programs "R2TOTSTK" and "SPECTRA" are shown in Appendix E. Data points from these horizontal response spectrum are plotted (shown in red) on the horizontal response spectra plots of

Div./Station Catawba N.S.
 Subject Stick Model

Unit A11 File No. CNE-1114.07-01-001

Sheet No. 176f

Problem No.

Rev 3

Checked By

DEB

Date

9-1-82

By JMK Date 6-30-82

UNCONTROLLED

32 continued

File CNS-1108.00-00-0002, Volume I, Appendix A "containment vessel Seismic Response Spectra (ORS)". These plots are shown in Appendix F. The dashed line represents the horizontal response spectra used for design of Category I structures, systems, and components supported by the containment vessel. The dashed line has incorporated a safety factor in the vicinity of the maximum response acceleration by broadening the peak by 10% and increasing the amplitude of the peak by 10%. To verify the horizontal response spectra of File CNS-1108.00-00-0002 to be satisfactory, the data points (shown in red) from the horizontal response spectra derived in this calculation, must be completely enveloped by the dashed line. The plots in Appendix F show that the data points (shown in red) from the horizontal response spectra developed in this calculation are completely enveloped by the dashed line. This indicates that the horizontal response spectra included in published File CNS-1108.00-00-0002, Volume I, Appendix A seems to adequately represent the containment vessel.

Dev./Station Catawba N.S.
Subject Stick ModelUnit All File No. CNC-1144, 09-01-001Sheet No. 172

Problem No.

Rev 3

Checked By

DEBBy JMK Date 8-6-82Date 9-1-823.2 continued**UNCONTROLLED**

Action item 10 of NRC Structural Audit concerns the assumption of negligible amplification in the vertical seismic analysis. To illustrate negligible amplification in the vertical analysis, a synthetic ground motion will be applied in the vertical direction in a transient analysis to calculate time histories at specified locations. These time histories are used to generate both input time history response spectra and response spectra in the vertical direction at specified elevations. A simulated time history was used due to the lack of a reasonable vertical ground motion time history. For ease of use, the horizontal time history is utilized. It is noted that the vertical response spectra generated in this calculation do not represent actual vertical response spectra. The vertical response spectra generated are for the purpose of illustrating negligible amplification with altitude in the vertical seismic analysis only. The Catawba FSAR section 3.7.2.10 states that "The response spectra used for the design of vertical modes is equal to $\frac{2}{3}$ of the horizontal spectra". The

Dev./Station Catawba N.S.
 Subject Stick Model

Unit All File No. CNC-1144, 09-01-001

Sheet No. 173 of

Problem No.

Part 3

Checked By

By JMK
DEB

Date 8-6-82
Date 9-1-82

3.2 continued **UNCONTROLLED**

horizontal spectra is the horizontal ground response spectra shown in FSAR Figure 2.5.2-6.

The vertical total acceleration vs. period response spectra obtained at each elevation specified in calculation File CNS-1108,00-00-0002, Appendix A can now be compared to the input response spectra to illustrate negligible amplification with altitude. The vertical acceleration vs. period response spectra plots are shown in Appendix G. A FORTRAN program, "COMPARE" was written to determine the acceleration amplification between the generated response spectra at each elevation and the generated ground response spectra. The percentage increase for each period was also calculated. *The figures in Appendix H are vertical response spectra plots showing the region, designated in blue, where the percentage increase is greater than 5%. The maximum percentage increase and its approximate location is noted. The maximum acceleration amplification and maximum percent increase for each damping value per specified joint is tabulated on page 175.

* Reference computer output "COMPARE"

Dev./Station Catawba N.S.Unit #/ File No. CDC-1144,09-01-00Subject Stick ModelBy JMK Date 8-6-82Sheet No. 174 of

Problem No.

Rev 3

Checked By

DEB

Date

9-1-823.2 continued**UNCONTROLLED**

It is noted that the Catawba FSAR section 3.4.1.3, Critical Damping Values states that 'the only system subject to a damping value of .5% for OBE is the 'Primary Coolant Loop System Components'. Since the 'Primary Coolant Loop System Components' are structurally independent of the steel containment vessel, a damping value of .5% will not be considered in the vertical seismic analysis of the steel containment vessel.

Div./Station Catawba N.S.
 Subject Stick Model

Unit A11 File No. CNE-1144/09-01-001

Sheet No 175 of

Problem No. REV 3

By JMK Date 8-6-82
 Checked By DEB Date 9-3-82

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The Maximum Acceleration Amplification and Maximum Percentage Increase Between the Generated Ground Response Spectrum and the Generated Response Spectrum at Specified Points

Elevation	Period	1.0% Damping		2.0% Damping		5.0% Damping	
		Δ Acc.	% Inc	Δ Acc.	% Inc	Δ Acc.	% Inc
593.42	.0795	.0394	17.064	.0322	15.972	.0208	13.065
	.0706	.0348	19.551				
	.0757			.0279	16.053		
	.0776					.0208	13.065
619.42	.0795			.0508	25.198		
	.0706	.0568	31.910				
	.0757			.0440	25.216		
	.0776	.0624	27.791			.0332	20.854
663.75	.0795			.0710	35.218		
	.0706	.0808	45.393				
	.0690			.0560	35.760		
	.0776	.0876	38.985			.0769	38.145
688.05	.0795			.0769	38.145		
	.0706	.0878	49.326				
	.0690			.0608	38.825		
	.0776	.0950	42.279			.0508	31.910
710.25	.0795			.0802	39.782		
	.0706	.0917	51.517				
	.0690			.0635	40.549		
						.0508	31.910

1520 35.001

2077 24/100

Dev./Station Catawba N.S.
 Subject Stick Model

Unit A11 File No. CNC-1144.09-01-01

Sheet No. 176 of

Problem No.

Rev 3

Checked By

By JMK

Date 8-20-82

DEB

Date

7-3-82

UNCONTROLLED

3.2 continued

To better represent the magnitude of the amplification with respect to the Catawba FSAR Vertical Response Spectrum, the generated response spectrum at each specified joint will be ratioed by:

$$\left(\frac{\text{Catawba FSAR Vertical Response Spectra}}{\text{Generated Ground Response Spectra}} \right)$$

This is intended only to get the amplification and the input ground response into the same range of values as the required FSAR Response Spectrum. The Catawba FSAR states that the vertical response spectrum will be $\frac{2}{3}$ the specified horizontal ground response spectrum of FSAR Figure 2.5.2-6. It has been shown that the magnitude of the generated ground response spectrum is larger than the FSAR Ground Response Spectrum Figure 2.5.2-6. Thus, the multiplication factor above, applied to the generated response spectrum, reduces to:

$$\frac{\frac{2}{3} \text{ FSAR Horizontal Ground Response Spectrum of Figure 2.5.2-6}}{\text{Generated Ground Response Spectrum}}$$

for the same period.

Div./Station Catawba N.S.
 Subject Stick Model

Unit A11 File No. CNC-1144.07-01-0010

Sheet No. 177 of

Problem No.

Rev 3

By JMK Date 8-20-82

Checked By DEB Date 9-3-82

3.2 continued

UNCONTROLLED

The adjusted response acceleration that will be compared to $\frac{2}{3}$ of the FSAR horizontal ground response acceleration with respect to the same period will be:

$$\text{Adjusted Acceleration} = (\text{Generated Acceleration}) *$$

$$\left(\frac{\frac{2}{3} \text{ FSAR Figure 2.5.2-6 Acceleration}}{\text{Generated Ground Response Acceleration}} \right)$$

To compare the adjusted response spectrum to $\frac{2}{3}$ of the Catawba FSAR horizontal ground response spectrum, accelerations must be determined with respect to the same periods used to generate the vertical acceleration vs. period response spectrum. The equation of the Catawba FSAR response spectra plots of FSAR figure 2.5.2-6 are required. Since the plots are in a log-log scale, the equation of each respective line is:

$$Y = M X + B$$

where

- Y = Log (acceleration)
- M = multiplication factor
- X = Log (period)
- B = constant

Plant/Station Catawba N.S.
 Project Stick Model

Unit All File No. CNC-114409-01-001

Doc. No. 178.4

Problem No.

Rev 3

By JMK Date 8-20-82

Checked By DEB Date 9-3-82

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3.2 continued

Given the end points of each line, M and B can be determined. The period (P) is known. Therefore, the acceleration (A) can be calculated:

$$\text{Equation: } \text{Log}(A) = M \text{Log}(P) + B$$

$$\text{Given: } P_1, A_1, P_2, A_2$$

$$\text{Log } A_1 = M \text{Log } P_1 + B$$

$$\Rightarrow B = \text{Log } A_1 - M \text{Log } P_1 \quad \textcircled{1}$$

$$\text{Log } A_2 = M \text{Log } P_2 + B$$

$$\Rightarrow B = \text{Log } A_2 - M \text{Log } P_2 \quad \textcircled{2}$$

$$\textcircled{1} = \textcircled{2} \Rightarrow \text{Log } A_1 - M \text{Log } P_1 = \text{Log } A_2 - M \text{Log } P_2$$

$$\text{Log } A_1 - \text{Log } A_2 = M \text{Log } P_1 - M \text{Log } P_2$$

$$\text{Log} (A_1 / A_2) = M \text{Log} (P_1 / P_2)$$

$$\Rightarrow M = \frac{\text{Log} (A_1 / A_2)}{\text{Log} (P_1 / P_2)}$$

$$\Rightarrow B = \text{Log } A_1 - \left[\frac{\text{Log} (A_1 / A_2)}{\text{Log} (P_1 / P_2)} \right] \text{Log } P_1$$

Plant Station: Catawba N.S.
 Subject: Stick Model

Unit All File No. CNE-114409-01-01

Sheet No. 179 of

Problem No.

Rev 3

Checked By

By JMK

Date 8-20-82

DEB

Date 9-3-82

UNCONTROLLED

3.2 continued

Given P_1, A_1, P_2, A_2 and determining M and B ,
 the acceleration (A) now becomes:

$$\text{Log } A = M \text{Log } P + B$$

$$\therefore \underline{\underline{A = 10^{(M \text{Log } P + B)}}}$$

The end points chosen for each of the
 3 lines per damping value .005, .01, .02, .05
 for OBE are tabulated below:

Damping	Line 1				Line 2				Line 3			
	P_1	A_1	P_2	A_2	P_3	A_3	P_4	A_4	P_5	A_5	P_6	A_6
.5%	.05	.140	.166	.40	.166	.40	.50	.40	.50	.40	3.0	.065
1.0%	.05	.132	.166	.340	.166	.340	.50	.340	.50	.340	3.0	.054
2.0%	.05	.125	.166	.280	.166	.280	.50	.280	.50	.280	3.0	.045
5.0%	.05	.110	.166	.20	.166	.20	.50	.20	.50	.20	3.0	.033

Note: the acceleration values are 100% of
 the FSAR horizontal ground response spectra

Station Catawba N.S.
 Subject Stick Model

Unit All File No. CNC-1144,09-01-001

Draw No. 180 of

Problem No.

Rev 3

By JMK Date 8-20-82

Checked By DEB Date 9-3-82

UNCONTROLLED

3.2 continued

The following example problem will be used to verify computer output "COMPARE".

Given: Damping = 1.0% Joint 20 $P = .0706$

Line 1 $\Rightarrow P_1 = .05$; $A_1 = .088g$; $P_2 = .166$; $A_2 = .226g$

$$M = \frac{\text{Log}(.088 / .2266)}{\text{Log}(.05 / .166)}$$

Note: A_1 & A_2 are
 $\frac{2}{3}$ of values
 shown in
 previous tabl

$$M = .78847619$$

$$B = \text{Log}(.088) - \left[\frac{\text{Log}(.088 / .2266)}{\text{Log}(.05 / .166)} \right] \text{Log}(.05)$$

$$B = -.02968615$$

$$\Rightarrow A = 10^{(.78847619 \text{Log}.0706 + (-.02968615))}$$

$$A = 10^{-.93737623}$$

$$\underline{\underline{A = .11551111 = .1155g}}$$

Given: Generated Ground Response = .1780 g
 Generated Response = .2697 g

Location Catawba N.S.
 Subject Stick Model

Unit All File No. CNC-1144,09-01-0010

Order No. 1811

Problem No.

Rev 3

Checked By

DEB

Date

8-20-82
9-3-82

By JMK Date 8-20-82

UNCONTROLLED

3.2 continued

$$\text{Adjusted Acceleration} = (.2697) \left[\frac{(.1155)}{(.1780)} \right] \quad \text{calculated on}$$

$$= \underline{\underline{.1750 \text{ g}}}$$

$$\text{Acceleration Amplification} = .1750 - .1155$$

$$= \underline{\underline{.0595 \text{ g}}}$$

$$\text{Percentage Increase} = \frac{.0595 \text{ g}}{.1155 \text{ g}}$$

$$= \underline{\underline{51.52\%}}$$

Percentages are
 same as previous
 calculated. OK

A comparison of these calculated values to those from output "COMPARE" reveals the same results.

FORTTRAN program "COMPARE" also determines the acceleration amplification between the adjusted response spectra and the Catawba FCAR vertical response spectra given the same period. The percentage increase for each period was also calculated.

*The figures in Appendix I are vertical response spectra plots showing the region, designated in green, where the acceleration

* Reference computer output "COMPARE"

Form 1014-9-71

Location *Catawba N.S.*
Subject *Stick Model*Unit *All* File No. *CNC-114409-01-00*Order No. *ER*

Problem No.

Page 3

Checked By

By *JMK*Date *8-20-82**LEB*Date *9-3-82*

3.2 continued

UNCONTROLLED

amplification is greater than 2%g.
The maximum acceleration amplification
and its approximate location is noted.
The maximum acceleration amplification
and maximum percent increase for each
damping value .01, .02 & .05 per specified
joint is tabulated on the following page.

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Dev./Station *Catawba N.S.*
 Subject *Stick Model*

Unit *All* File No. *CNC-1144.09-01-061*

Sheet No. *183* of

Problem No. *REV 3*

By *JMK* Date *8-20-82*
 Checked By *DEE* Date *9-3-82*

The Maximum Acceleration Amplification and Maximum Percentages Increase Between the Catawba F.S.R. Ground Response Spectrum and the Adjusted Response Spectrum at Specified Joints

Elevation	Period	10% Damping		20% Damping		5.0% Damping	
		Δ Acc	% Inc	Δ Acc	% Inc	Δ Acc	% Inc
593.42	.0795	.0226	19.551	.0182	15.972	.0119	13.065
	.0706			.0177	16.053		
	.0757			.0287	25.1983		
	.0776			.0279	25.317	.0190	20.854
663.75	.0795	.0524	45.393	.0401	35.218	.0268	29.377
	.0706			.0370	35.7598		
	.0690			.0434	38.145		
	.0776			.0402	38.825	.0291	31.910
710.85	.0795	.0595	51.517	.0453	39.782		
	.0706			.0419	40.549		
	.0690						

22.221

Dev./Station Catawba N.S.Unit All File No. CNC-1144/09-01-001Subject Stick ModelBy JMK Date 8-20-82Sheet No. 184 of _____ Problem No. REV 3Checked By DEB Date 9-3-823.2 continued

To show this amplification due to modal contributions, vertical relative acceleration vs. period response spectra was generated, again using 100% horizontal ground motion time history as the time history input, by computer programs "RELSTK" and "RELSPEC". These plots are shown in Appendix J.

In inspecting the figures in Appendix I, it is noted that:

- A. At some points on the vertical response spectrum, it is possible to get out an acceleration 51% greater than that indicated by the input vertical spectrum.
- B. The amplification is noted in a general range of frequency between 5 and 20 hertz. In this range, the generated vertical response is greater than the input vertical excitation.
- C. The magnitude of the maximum acceleration amplification is approximately 1.6g at 14.16 hertz.

Dev./Station Catawba N.S.
Subject Stick Model

Unit All File No. CNC-1144/09-01-001

Sheet No. 185 Problem No. Rev 3

By JMK Date 8-20-81
Checked By DIB Date 9-3-82

3.2 continued

- D. The FEAR ground response spectrum (FEAR Figure 2.5.2) peak is between 2 and 6 hertz. Note:
The amplification is noted at off peak frequencies.

Although vertical amplification is present, the magnitude of the vertical amplification is only approximately 6% g. The mass of all attachments experience a constant acceleration of 1.0 g due to gravity. The amplification of 6% is deemed to be small relative to 1.0 and will be neglected.

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Div./Station Catawba N.E.
 Subject Stick Model

Unit All File No. CNC-1144.09-01-0

By JMK Date 8-24-82

Sheet No. 186 of Problem No. Rev 3 Checked By DEB Date 9-3-82

40 Conclusions

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Calculation File CNC-1144.09-01-0001 includes the preliminary seismic analysis of the Catawba containment vessel which produced the "Catawba Containment Vessel Response Acceleration Spectra" of File CNS-1108.00-00-0002, Catawba Nuclear Station Specification For The Response Spectra Displacements For Category I Structures, Volume I. Because the computer print out of the preliminary seismic analysis could not be located and the 13th ring stiffener had been relocated since the original seismic analysis, a verification of the published response spectra was performed. The stick model used for analysis was analogous to the model used in the preliminary seismic analysis of 1974. The relocation of the 13th ring stiffener was incorporated in the stick model. A more refined smearing technique was utilized. Mass concentrations were applied to mass points with respective elevations. Member properties including moment of inertia, cross-section area, and shear area were calculated in the same manner as the original model. Using the stick

Div./Station Catawba N.S.
 Subject Stick Model

Unit All File No. CNS-1144, 09-01-0016

Sheet No. 187 of _____ Problem No. Rev 3 By JMK Date 8-24-82
 Checked By LEB Date 9-3-82

4.0 continued

UNCONTROLLED

model, a seismic analysis was performed to generate horizontal response spectra. These response spectra were then compared to the response spectra of published File CNS-1108.00-00-0002 to verify that the published response spectra reasonably represented the containment vessel. In compliance with the Catawba FSAR section 3.1.2.6, a simulated horizontal time history was applied in the horizontal and vertical directions. Twenty-five percent of four synthetic earthquakes (CATEQ1, CATEQ2, CATEQ3, CATEQ4) were used to simulate the horizontal ground motion. The generated ground response spectra for oscillator damping values of .5, 1.0, 2.0, and 5.0 % with magnitudes of acceleration being slightly greater, reasonably represented the shape of the Catawba FSAR specified ground response spectra. Horizontal response spectra were generated at 5 joints with the same elevations as those response spectra in published file CNS-1108.00-00-0002. The horizontal total acceleration vs. period response spectra were calculated by adding rigid body support motions to those from modal contributions. This resulted in the total acceleration time history at each selected elevation. From these time histories, the response spectra for single degree of freedom oscillators

Dev./Station Catawba N.S.
 Subject Stick Model

Unit All File No. CNC-1144,09-01-001

Sheet No. 122 of

Problem No.

Rev 3

Checked By

By JMK

Date

8-24-82

Date

9-3-82

UNCONTROLLED

4.0 continued

were generated for the damping ratios required. Data points from these horizontal response spectrum were plotted on the horizontal response spectrum plots of File CNS-1108.00-00-00. The data points were completely enveloped by the dashed line of the published response spectrum. This indicates that the horizontal response spectra included in published File CNS-1108.00-00-0002 Volume I Appendix A seems to adequately represent the containment vessel.

Action item 10 of the NRC Structural Audit concerning the assumption of negligible amplification in the vertical seismic analysis was also addressed. To illustrate negligible amplification in the vertical analysis, a synthetic ground motion was applied in the vertical direction in a transient analysis to calculate time histories and associated response spectra at specified locations. It is noted that the vertical response spectra generated in this calculation do not represent true containment vertical response spectra. The vertical response spectra generated are for the purpose of illustrating negligible amplification with altitude in the vertical seismic analysis only. To represent the magnitude of the amplification with respect to the Catawba FSAR vertical

Div./Station Catawba NS
Subject Stick Model

Unit All File No. CNE-114/09-01-001.

Sheet No. 189 of

Problem No.

rev 3

Checked By

By JMK Date 8-24-82

DEB Date 9-3-82

4.0 continued

UNCONTROLLED

response spectra, the generated response spectra at each specified joint was ratioed (see Results section 2.0) into the same range of values as the FCAR Vertical Response Spectra ($2/3$ of horizontal ground response spectra). Although vertical amplification is present, the magnitude of the vertical amplification is only approximately $6\%g$. The mass of all attachments experience a constant acceleration of $1.0g$ due to gravity. The amplification of $6\%g$ is deemed to be small relative to 1.0 and will be neglected.

Dev./Station **CATAWBA**
 Subject **VERTICAL AMPLIFICATION**

Unit 1 $\frac{1}{2}$ File No. **CNC-1144.09-01-0010**

Sheet No. **189.001**

Problem No. **REV 6**

By **JMM** Date **1-28-83**

Checked By **DEB** Date **1-31-83**

STATEMENT OF THE PROBLEM

FOLLOWING THE ANALYSIS OF THE LUMPED MASS MODEL FOR VERTICAL EXCITATION, AS DESCRIBED ON THE PRECEDING PAGES, A REVISED SUBMITTAL WAS MADE TO THE USNRC S.E.B. DESCRIBING THE ANALYSIS AND EXPLAINING THE CONTENTION THAT AMPLIFICATION IN THE VERTICAL DIRECTION IS NEGLIGIBLE. THIS SUBMITTAL WAS MADE TO SRAL DURING SEPTEMBER 1982, AND A COPY OF THE SUBMITTAL IS INCLUDED ON THE FOLLOWING PAGES.

DURING JANUARY 1983, NRC REPRESENTATIVES REQUESTED CLARIFICATION AND DETAILED DATA SUPPORTING THE CONCLUSIONS OF THE ANALYSIS. A COPY OF THE INFORMATION PROVIDED IS ALSO INCLUDED ON THE FOLLOWING SHEETS.

THE PURPOSE OF THIS REVISION IS TO DOCUMENT THE SOURCES OF THE DATA SUBMITTED.

QA CONDITION 1 APPLIES

STANDARDS, FSAR REFERENCES, AND REFERENCES AS BEFORE.

COMPUTER RUNS USED

- A. RZTOTSTK J 5543 28JUL82 - STICK MODEL AND ACCELERATION TIME HISTORIES FOR BASE AND MASS POINTS (HORIZ & VERT)
- B. SPECTRA J 6199 29JUL82 - Generation of response spectra from acceleration time histories above
- C. COMPARE J 1974 23AUG82 - PERFORMS ANALYSIS DESCRIBED IN SEPTEMBER SUBMITTAL ABOVE
- D. DEB8362D J 4431 17JAN83 } GENERATE PLOTS OF RESULTS
 DEB8362Z J 921 14JAN83 } OF RUN "COMPARE"

Dev./Station CATAWBA

Unit 1^{1/2} File No. CNC-1144-09-01-0010

Subject VERTICAL AMPLIFICATION

By JMM Date 1-28-83

Sheet No. 189.002 of Problem No. Rev 6

Checked By DEB Date 1-31-83

SEPTEMBER SUBMITTAL

THIS SUBMITTAL, SHOWN ON THE FOLLOWING THREE PAGES, IS A SUMMARY OF THE ANALYSIS METHOD FOR DETERMINING VERTICAL AMPLIFICATION WHICH IS DESCRIBED IN THIS CALC, SHEETS 172 TO 185.

THE MAXIMUM AMPLIFICATION OF ACCELERATION, DETERMINED TO BE 0.06g, IS SHOWN ON SHEET 181, AS CALCULATED IN COMPUTER RUN "COMPARE"

10. Provide justification demonstrating negligible amplification assumed in vertical seismic analysis (interior structure, auxiliary building, and steel containment).

Revised Response (Steel Containment only)

During preparation of the design report for the steel containment vessel, a detailed review of all design calculations has been made. In the course of this review, it was found that the spectra submitted in the initial response to this action item had been generated using relative acceleration time histories from the lumped mass stick model. A time history of total accelerations is the appropriate input for generation of these spectra.

The use of spectra generated using relative accelerations results in the conclusion that the vertical ground response spectrum envelopes the vertical spectrum at each point. This, in fact, may not be true when total accelerations are considered. Because of this, the spectra previously submitted are somewhat misleading and should be deleted from Duke's response to this action item.

A reanalysis of the vertical response of the steel containment was made. The containment vessel stick model was analyzed for vertical excitation using an input vertical base acceleration time history identical to the horizontal base acceleration time history used in the horizontal seismic analysis. All containment frequencies up to 33 hz were considered in the analysis. Using total acceleration time histories from this analysis, vertical response spectra were generated at the base and at mass points of interest.

A review of the results of the analysis described above was conducted to quantify the amplification in the vertical direction. Figure 10.1 depicts the methodology used in this review. In this figure, Spectrum "A" represents the required vertical ground response spectrum, defined in the FSAR as two-thirds of the horizontal ground response spectrum. Spectrum "1" represents the vertical ground response spectrum resulting from the input vertical base acceleration time history. It should be noted that this spectrum envelopes the horizontal ground response spectrum shown in the FSAR. Spectrum "n" represents the vertical response spectrum obtained at mass point "n". At a given period t_i , one point on each spectrum is defined.

For a given frequency or period, the magnification factor H was defined to be the ratio of the response acceleration to the excitation acceleration.

$$H = a_n / a_1$$

The digital response spectrum data was reviewed for all frequencies between 0 hz and 33.3 hz and for all applicable damping ratios. In the general frequency range of 5 hz to 20 hz, the magnification factor H was found to be greater than 1.05, indicating a 5% or greater amplification of the acceleration input at the base. At some points, the value of H reached a maximum of 1.51, indicating an amplification of the input acceleration by 51%.

Since this amplification was felt to be potentially significant, the spectrum data was further reviewed to quantify the magnitude of the acceleration increase. For an input vertical base excitation equal to the required vertical ground response (Spectrum "A"), the magnitude of the increase in acceleration is given by

$$\Delta a = (a_n - a_1) a_A / a_1 = (H-1) a_A$$

A pointwise review of spectrum data showed the maximum amplification of acceleration to be approximately 0.06 g. When this increase is compared to the constant gravitational acceleration of 1.0 g experienced by all attachments to the containment vessel, it is felt that the assumption of negligible amplification in the vertical direction is indeed justified.

It should be pointed out that the analysis in question pertains only to attachments to the steel containment, not the vessel itself. In the analysis of the containment, all vertical and horizontal modes below 30 hz were considered by modeling the entire vessel and applying shock spectra at the base in both directions.

sh 189,005

CNC-144.09-01-0010

REV 6

JMM 1-28-83

DEB 1-31-83

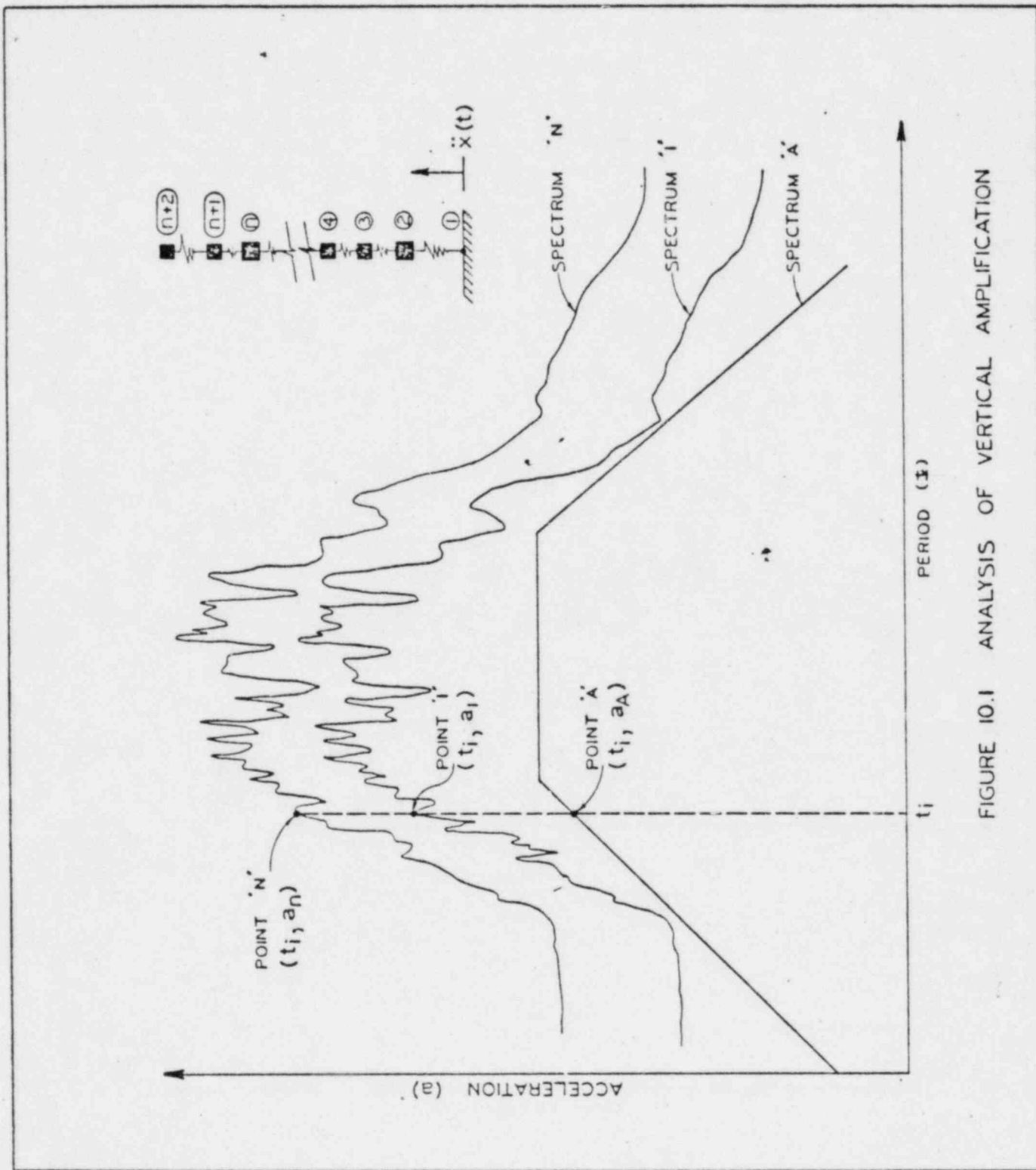


FIGURE 10.1 ANALYSIS OF VERTICAL AMPLIFICATION

Dev./Station **CATAWBA**
Subject **VERTICAL AMPLIFICATION**

Unit **1 1/2** File No. **CNC-1144.09-01-0010**Sheet No. **189.006**

Problem No.

REV 6By **JMM** Date **1-28-83**
Checked By **DEB** Date **1-31-83**

JANUARY SUBMITTAL

THIS SUBMITTAL, SHOWN ON THE FOLLOWING SHEETS, PRESENTS ADDITIONAL DATA FROM THE ANALYSIS FOR VERTICAL AMPLIFICATION.

TABLE 10.1 CONTAINS DATA EXTRACTED FROM RUNS "SPECTRA" AND "COMPARE". SPECTRUM "I" IS THE RESPONSE SPECTRUM AT THE BASE OF THE STRUCTURE RESULTING FROM COMBINING THE SPECTRA GENERATED BY FOUR SYNTHETIC VERTICAL TIME HISTORIES APPLIED AT THE BASE. THIS CAN BE READ FROM RUN "SPECTRA", PAGES 129 TO 133, AS WELL AS IN RUN "COMPARE" WHICH USES THE RESULTS OF "SPECTRA".

SIMILARLY, SPECTRUM "N" IS THE RESPONSE SPECTRUM AT THE TOP OF THE CONTAINMENT DOME RESULTING FROM COMBINING THE SPECTRA GENERATED BY FOUR VERTICAL TIME HISTORIES AT THE BASE. THIS LOCATION WAS CHOSEN BECAUSE IT EXHIBITED THE GREATEST PERCENTAGE AMPLIFICATION AND THE GREATEST INCREASE IN "g" LEVEL. SPECTRUM "N" CAN BE FOUND IN RUN "SPECTRA", PAGES 229 TO 233, AS WELL AS IN THE POSTPROCESSOR "COMPARE".

THE AMPLIFICATION FACTOR "H" IS CALCULATED BY RUN "COMPARE", AS IS THE FSAR VERTICAL G.R.S. (SEE ALSO SH. 177-179). THE AMPLIFIED SPECTRUM, AND THE AMPLIFICATION.

COMPUTER RUNS DEB836ZD AND DEB836ZZ ARE USED TO GENERATE PLOTS OF THE AMPLIFICATION AND AMPLIFIED SPECTRUM FOR COMPARISON PURPOSES. FOR PLOTS, SEE SUBMITTAL AND THE FOLLOWING PAGES.

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CNC-1144.09-01-0010

REV 6
JMMI-28-83

DEB 1-31-83

ADDITIONAL INFORMATION FOR ACTION ITEM 10

10. Provide justification demonstrating negligible amplification assumed in vertical seismic analysis.

Steel Containment

Previous submittals have described the methodology used in assessing the amplification of input vertical seismic excitation for the containment vessel. The purpose of this additional information is to provide more detailed data at the request of NRC reviewers.

A review of the results of the analysis described in previous submittals showed the maximum amplification of input vertical excitation to occur at the top of the containment vessel. The stick model data point at elevation 710'+11" (Approximately 10' below the top of the containment dome) shows the greatest amplification of input excitation.

Table 10.1 presents digital data used in assessing the amplification in the vertical direction. Columns (1) and (2) present representative periods/frequencies on the spectra. Column (3) shows the input base vertical spectrum resulting from four synthetic time histories applied to the stick model (Spectrum "I" from previous submittals). Column (4) shows the calculated spectrum at elevation 710'+11" resulting from these time histories (Spectrum "N" from previous submittals). The structural amplification factor for a given frequency is the ratio of these two values and is shown in column (5). Column (6) shows the vertical ground response spectrum from the FSAR defined as 2/3 the horizontal ground response spectrum. The amplified spectrum at elevation 710'+11" is the product of the structural amplification factor and the FSAR vertical ground response spectrum and is shown in column (7). The magnitude of the amplification is shown in column (8).

Figure 10.2 graphically depicts the results of the analysis, showing a comparison of the FSAR vertical ground response spectrum, the amplified vertical spectrum at elevation 710'+11", and the amplification of the input vertical spectrum by the containment vessel. Figures 10.3 and 10.4 display the same information in tripartite format.

The frequency range of measurable vertical structural amplification is seen to be very narrow, and the maximum increase in acceleration of 0.06 g is negligible compared to the constant gravitational acceleration of 1.0 g experienced by all attachments to the containment vessel.

As a result of this analysis, it is concluded that the assumption of negligible amplification in the vertical direction is justified for all attachments to the containment vessel.

TABLE 10.1

CATAWBA NUCLEAR STATION
 STEEL CONTAINMENT VESSEL
 OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION
 ELEVATION 710'+11" 1% DAMPING

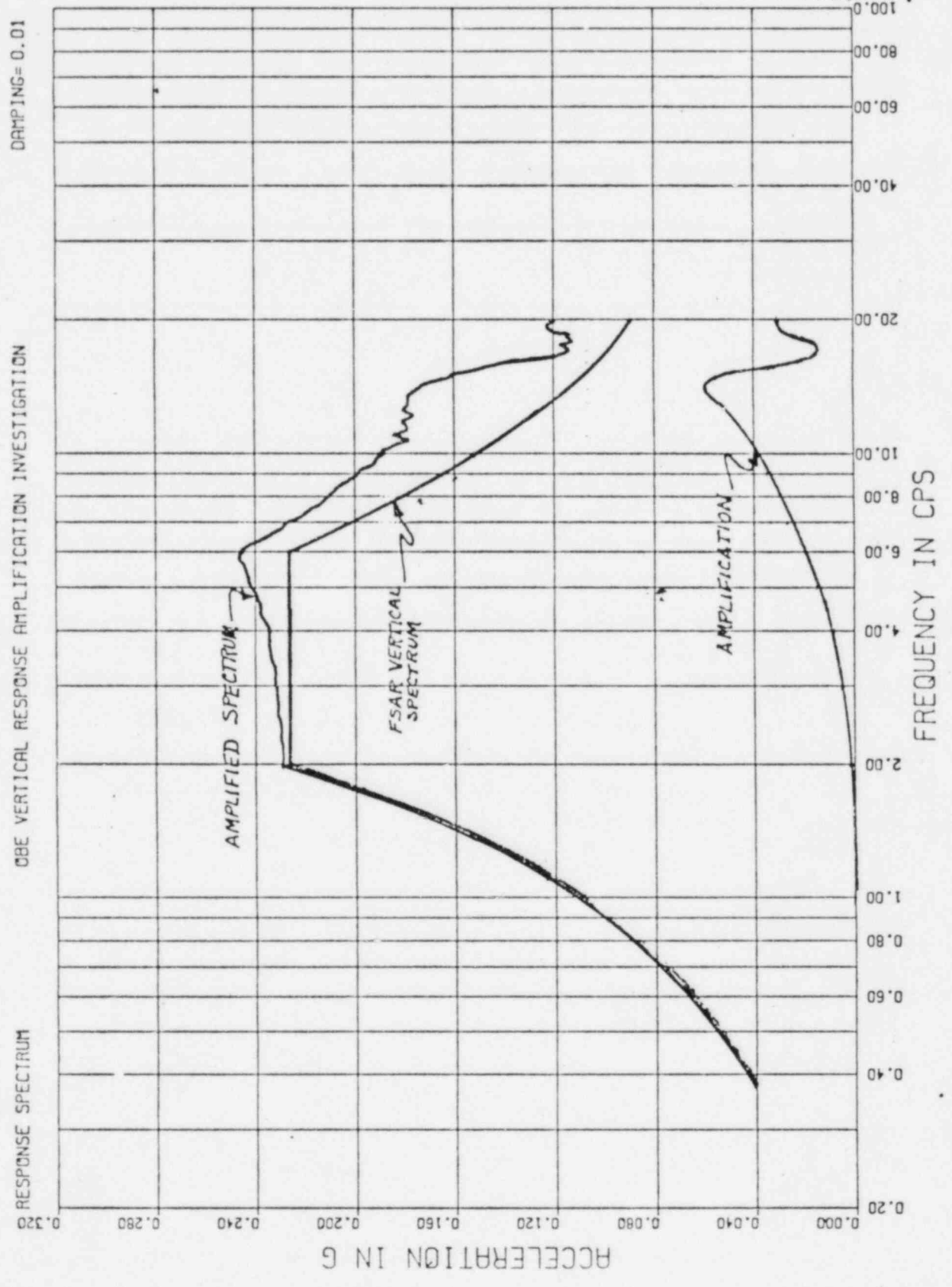
(1) PERIOD t_i (Sec)	(2) FREQUENCY f_i (hz)	(3) SPECTRUM "I" a_I (g)	(4) SPECTRUM "N" a_N (g)	(5) AMPLIFICATION FACTOR "H" (4)/(3)	(6) FSAR VERT. SPECTRUM (g) a_A (g)	(7) AMPLIFIED SPECTRUM (g) (5)·(6)	(8) AMPLIFICATION (g) (7)-(6)
1.0300	0.9709	0.1916	0.1920	1.00209	0.1079	0.108	0.000
0.4947	2.0214	0.4796	0.4846	1.01042	0.2266	0.229	0.002
0.3307	3.0239	0.4157	0.4250	1.02237	0.2266	0.232	0.005
0.2513	3.9793	0.3872	0.4006	1.03461	0.2266	0.234	0.008
0.2027	4.9334	0.4457	0.4743	1.06417	0.2266	0.241	0.015
0.1653	6.0496	0.4371	0.4752	1.08717	0.2266	0.246	0.020
0.1428	7.0028	0.4068	0.4593	1.12906	0.2012	0.227	0.026
0.1257	7.9554	0.3686	0.4265	1.15708	0.1820	0.211	0.029
0.1102	9.0744	0.3661	0.4437	1.21196	0.1641	0.199	0.035
0.0997	10.0301	0.2939	0.3703	1.25995	0.1516	0.191	0.039
0.0911	10.9769	0.2864	0.3744	1.30726	0.1412	0.185	0.043
0.0838	11.9332	0.2444	0.3348	1.36988	0.1322	0.181	0.049
0.0776	12.8866	0.2247	0.3238	1.44103	0.1244	0.179	0.055
0.0706	14.1643	0.1780	0.2697	1.51517	0.1155	0.175	0.060
0.0661	15.1286	0.1418	0.2087	1.47179	0.1097	0.161	0.052
0.0622	16.0772	0.1257	0.1534	1.22037	0.1045	0.128	0.023
0.0587	17.0358	0.1187	0.1349	1.13648	0.0999	0.114	0.014
0.0556	17.9856	0.1196	0.1401	1.17141	0.0957	0.112	0.016
0.0528	18.9394	0.1194	0.1583	1.32579	0.0919	0.122	0.030
0.0503	19.8807	0.1171	0.1580	1.34927	0.0884	0.119	0.031

CNF-144,00-01-0010
 Rev 6
 JMM 1-28-83
 DEB 1-31-83
 SH 189,008

CXC-1144.09-01-0010
REV 6
JMM 1-28-83
DEB 1-31-83
Sh 189.009

FIGURE 10.2
CATAWBA NUCLEAR STATION
CONTAINMENT VESSEL
OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION
DAMPING = 0.01

ELEVATION 710+11



ELEVATION 710+11

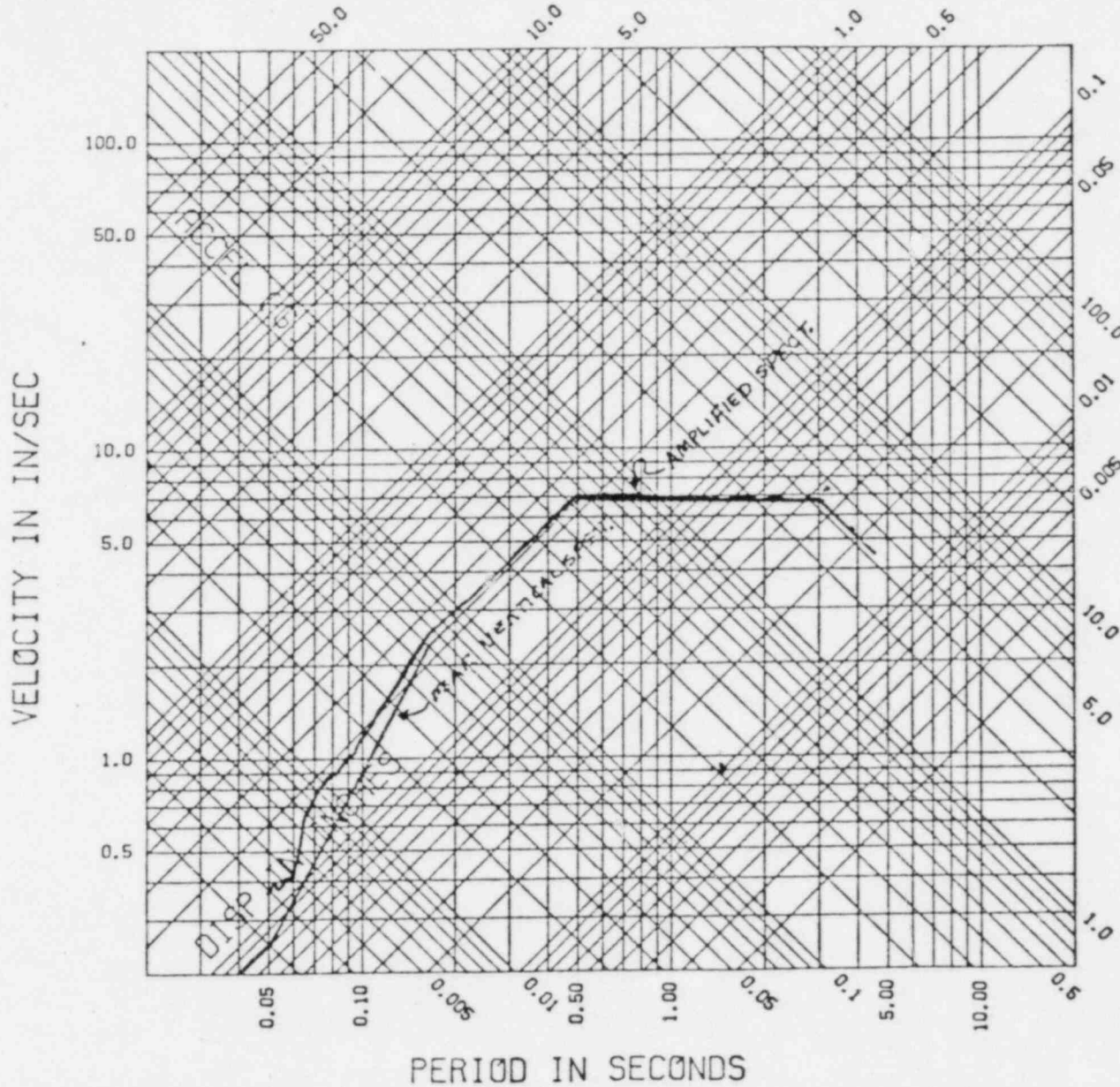
FIGURE ((
CATAWBA NUCLEAR STATION
CONTAINMENT VESSEL

1-17-83

RESPONSE SPECTRUM

OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION

DAMPING= 0.01



CNC-1144.09-01-0010
 Rev 6
 JMM 1-28-83
 DEB 1-31-83
 Sh 189.610

CNC-1144.09-01-0010

REV 6

JMM 1-28-83

DEB 1-31-83

sh 189.011

1-17-83

DAMPING = 0.01

FIGURE 10.4

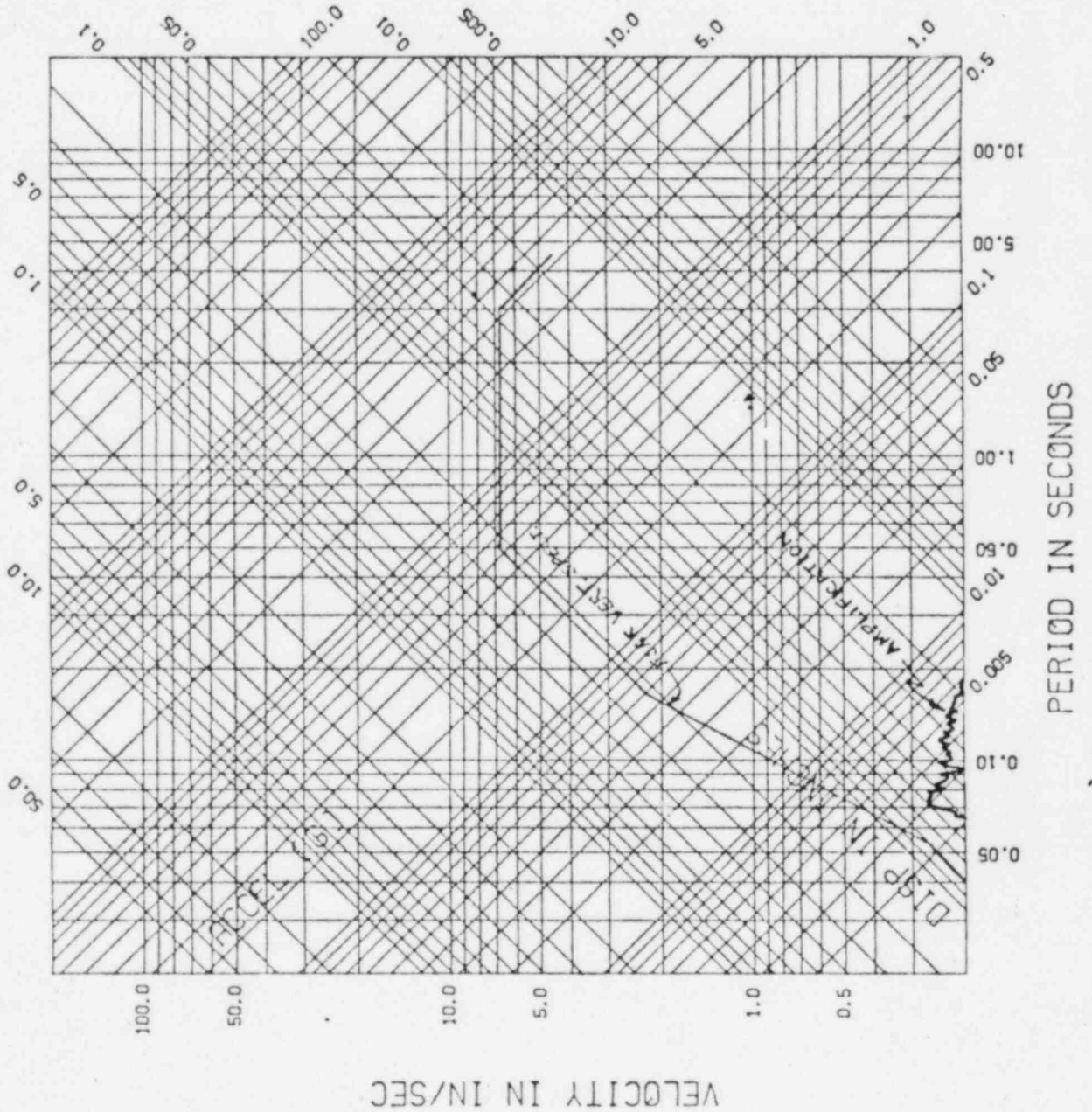
CATAWBA NUCLEAR STATION

CONTAINMENT VESSEL

OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION

ELEVATION 710+11

RESPONSE SPECTRUM



ELEVATION 710+11

CATAWBA NUCLEAR STATION

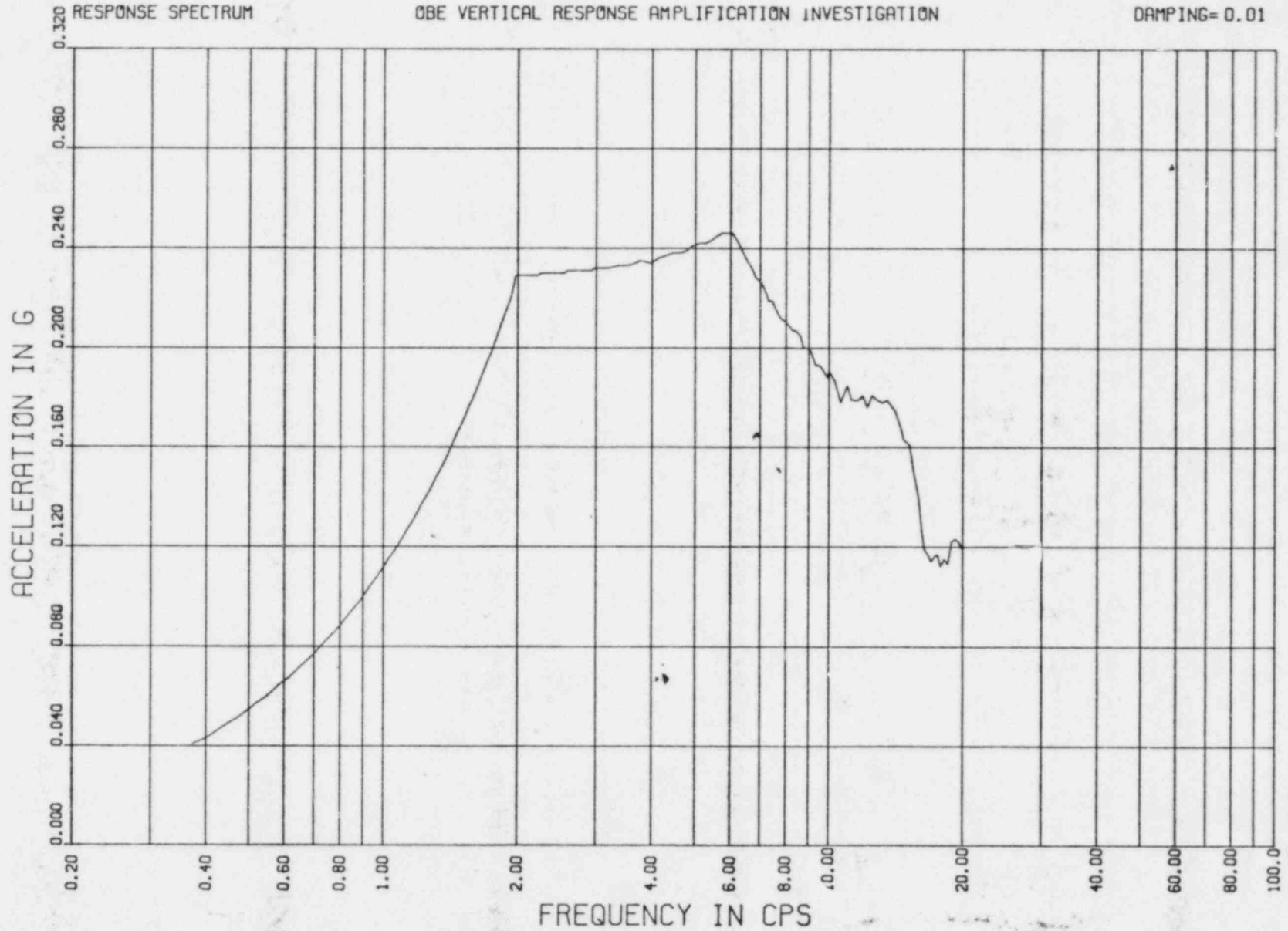
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CONTAINMENT VESSEL

RESPONSE SPECTRUM

OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION

DAMPING=0.01



CNC-1144.09-01-0010
REV C
JMM 1-28-83
DEB 1-31-83
SH 189,012

CNC-1144.09-01-0010
REV 6
JMM 1-28-83
DEB 1-31-83
SH 189.013

1-17-83

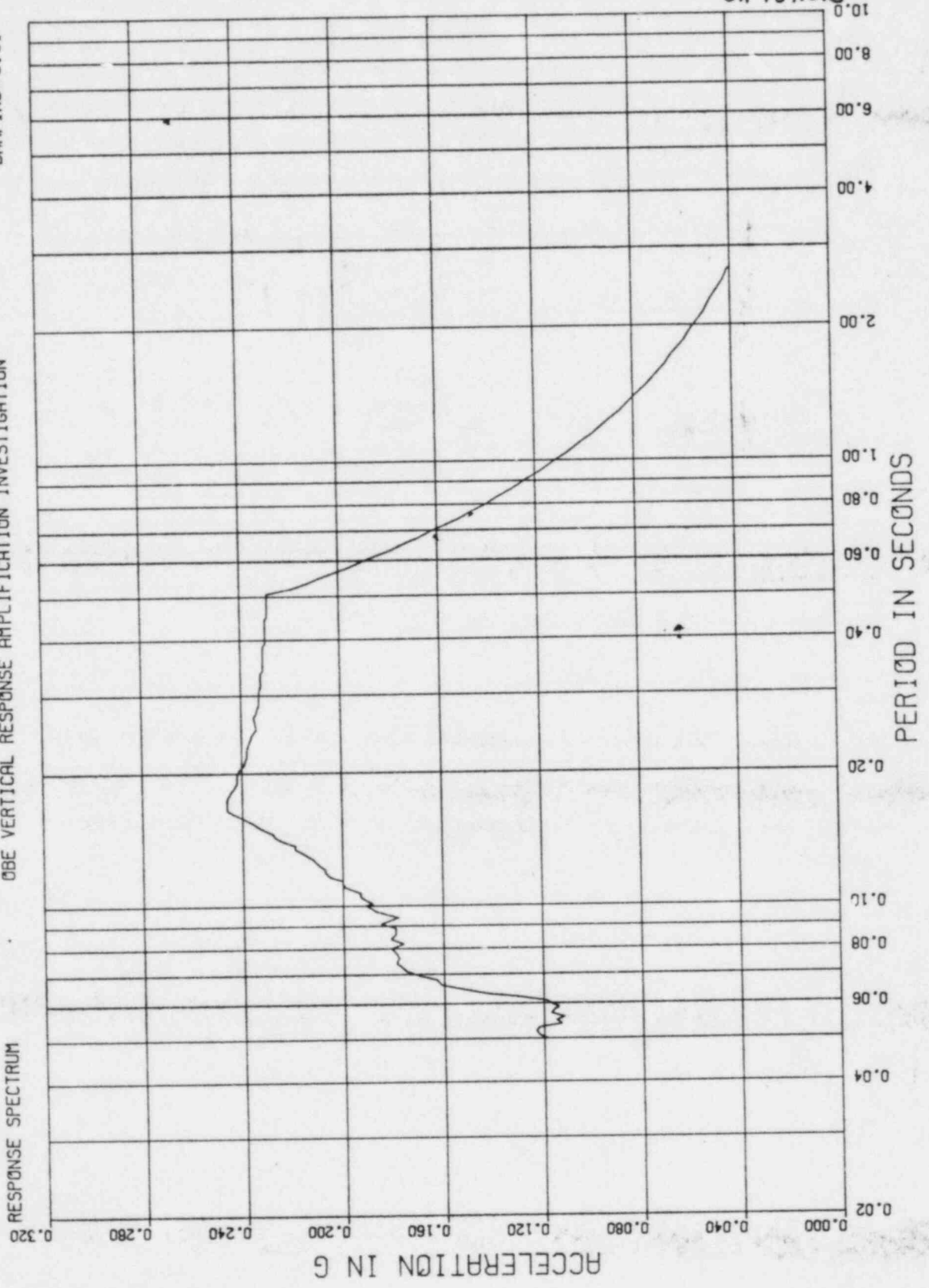
CATAWBA NUCLEAR STATION

CONTAINMENT VESSEL

OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION

DAMPING = 0.01

ELEVATION 710+11



CNC-1144.09-01-0010

REV 6

JMM-28-83

DEB 1-31-83

Sh 189.014

1-17-83

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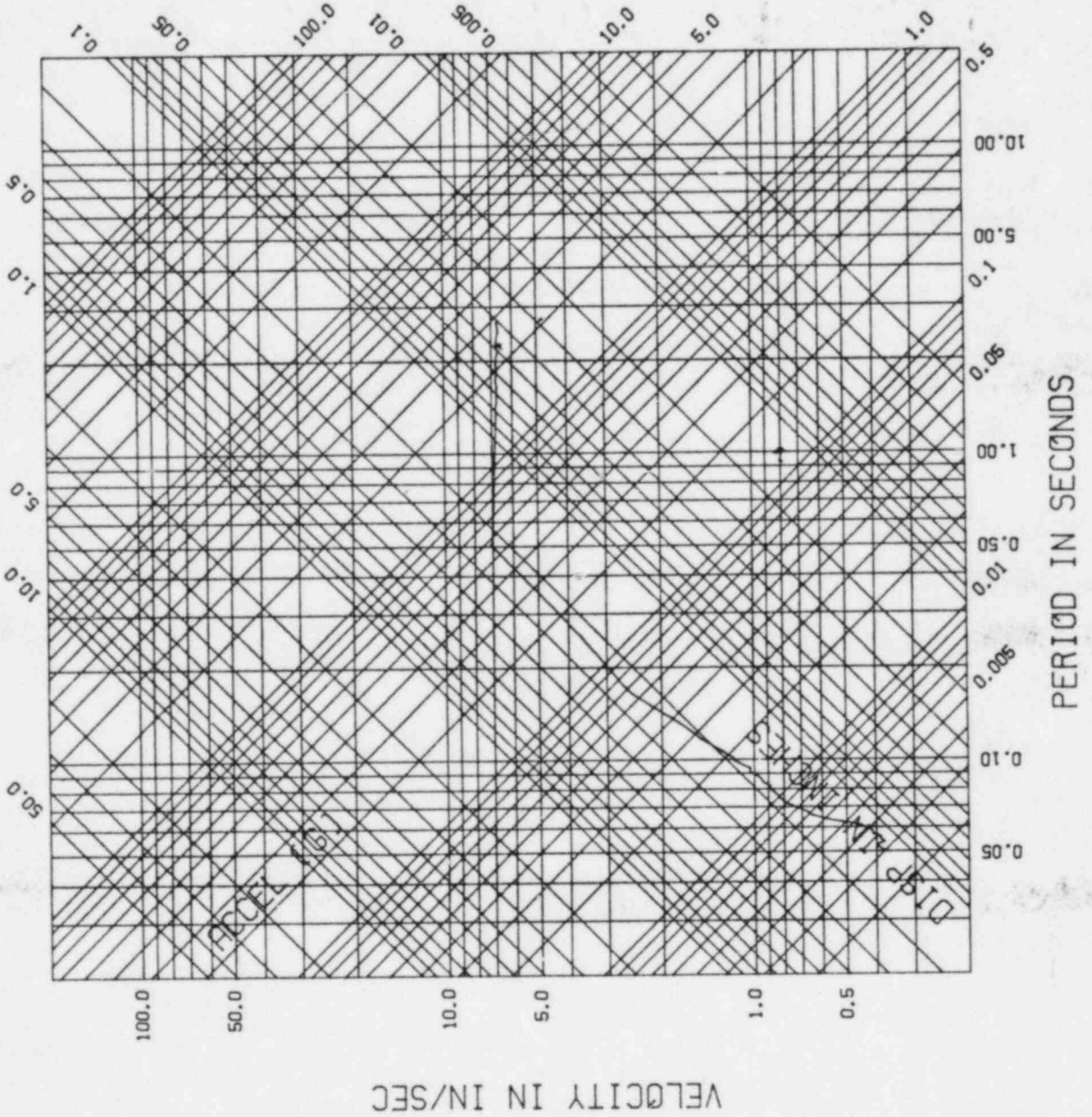
CONTAINMENT VESSEL

OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION

DAMPING = 0.01

ELEVATION 710+11

RESPONSE SPECTRUM



ELEVATION 710+11

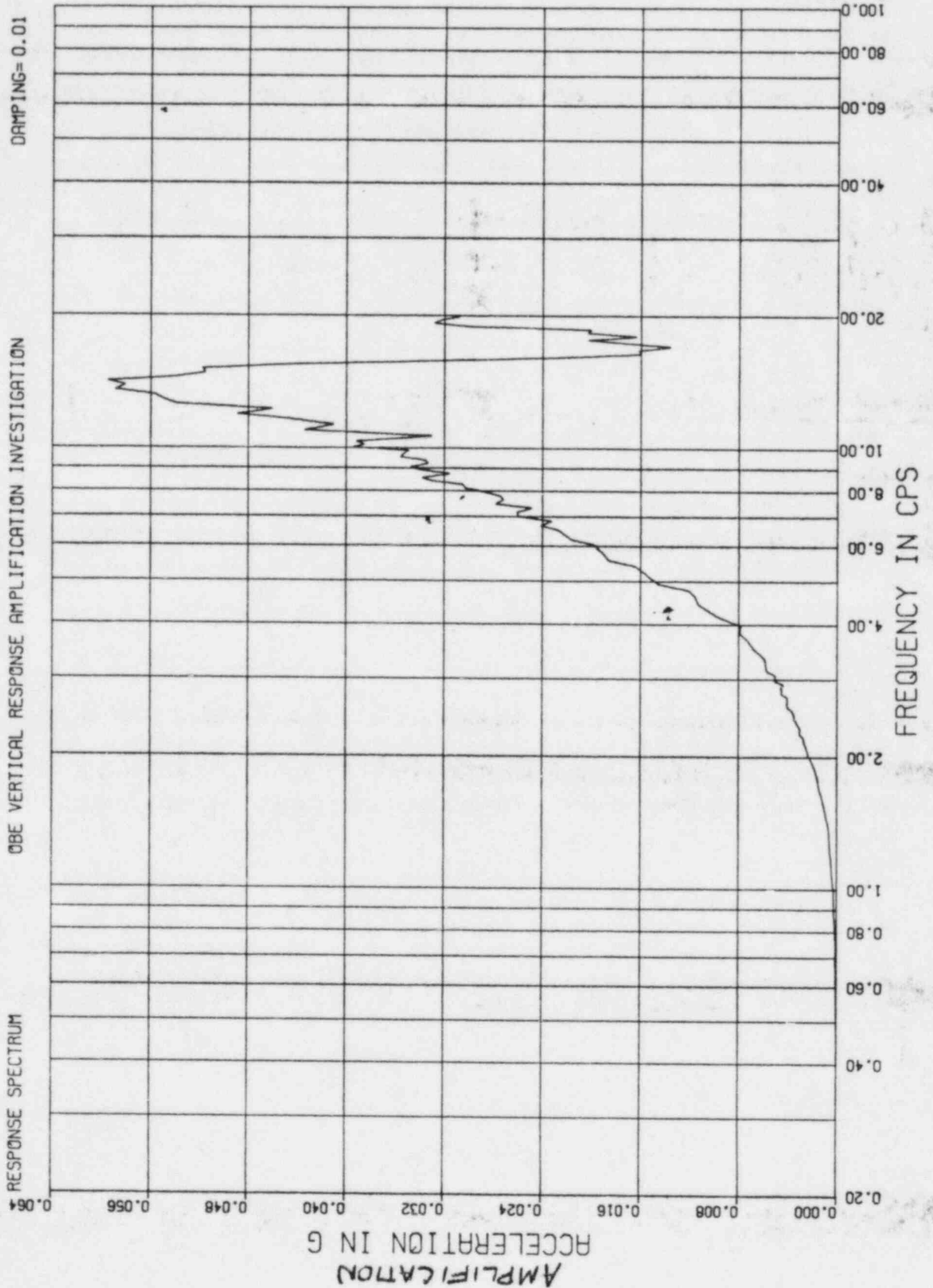
CATAWBA NUCLEAR STATION
CONTAINMENT VESSEL

1-17-83

RESPONSE SPECTRUM

OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION

DAMPING= 0.01



CNC-1144.09-01-0010
Rev 6
JMM 1-28-83
DEB 1-31-83
SH189.015

CNC-1144.09-01-0010
REV 6
JMM 1-28-83
DEB 1-31-83
Sh 189.016

1-17-83

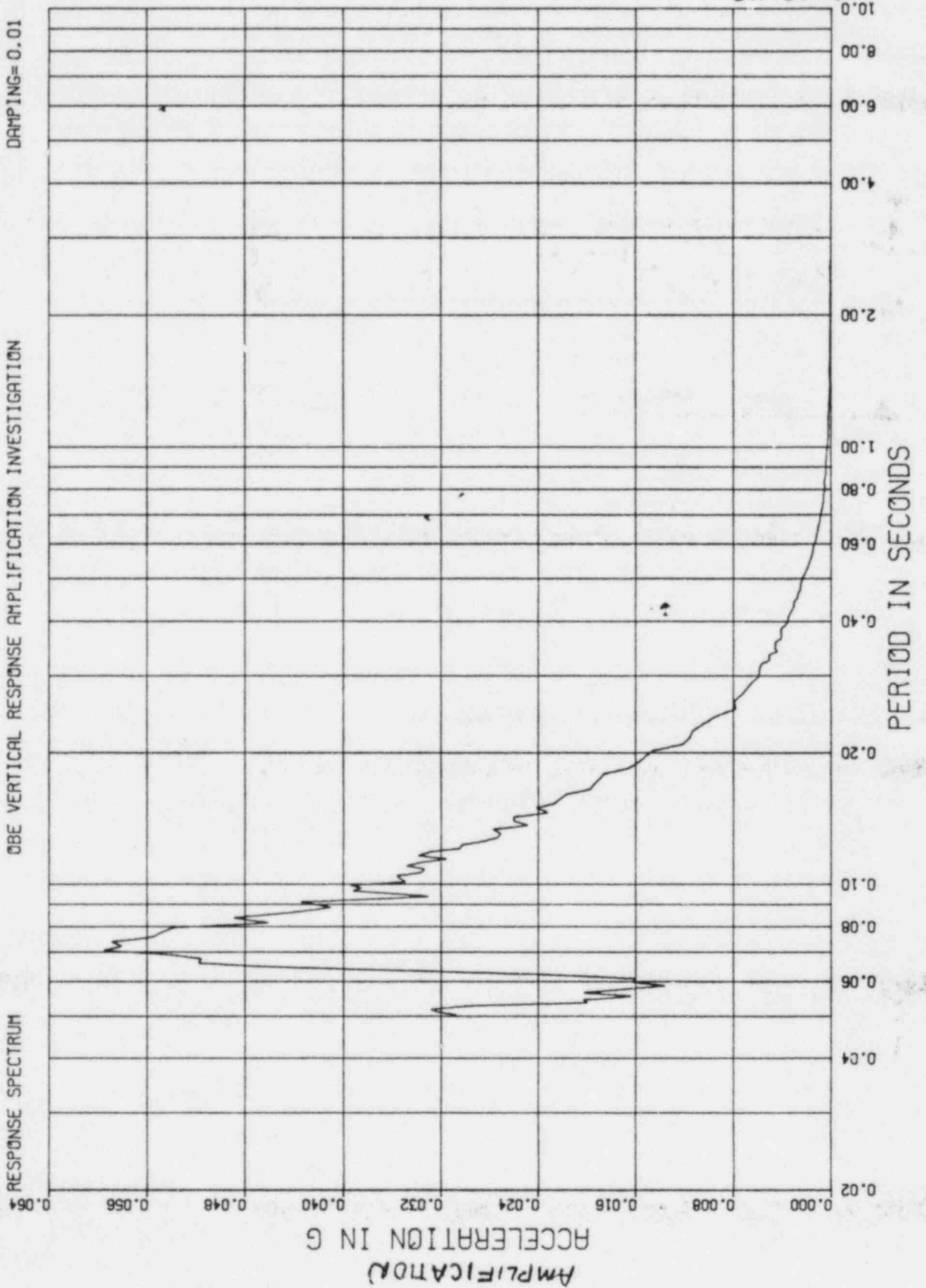
CATAWBA NUCLEAR STATION
CONTAINMENT VESSEL

08E VERTICAL RESPONSE AMPLIFICATION INVESTIGATION

DAMPING = 0.01

ELEVATION 710+11

RESPONSE SPECTRUM



ELEVATION 710+11

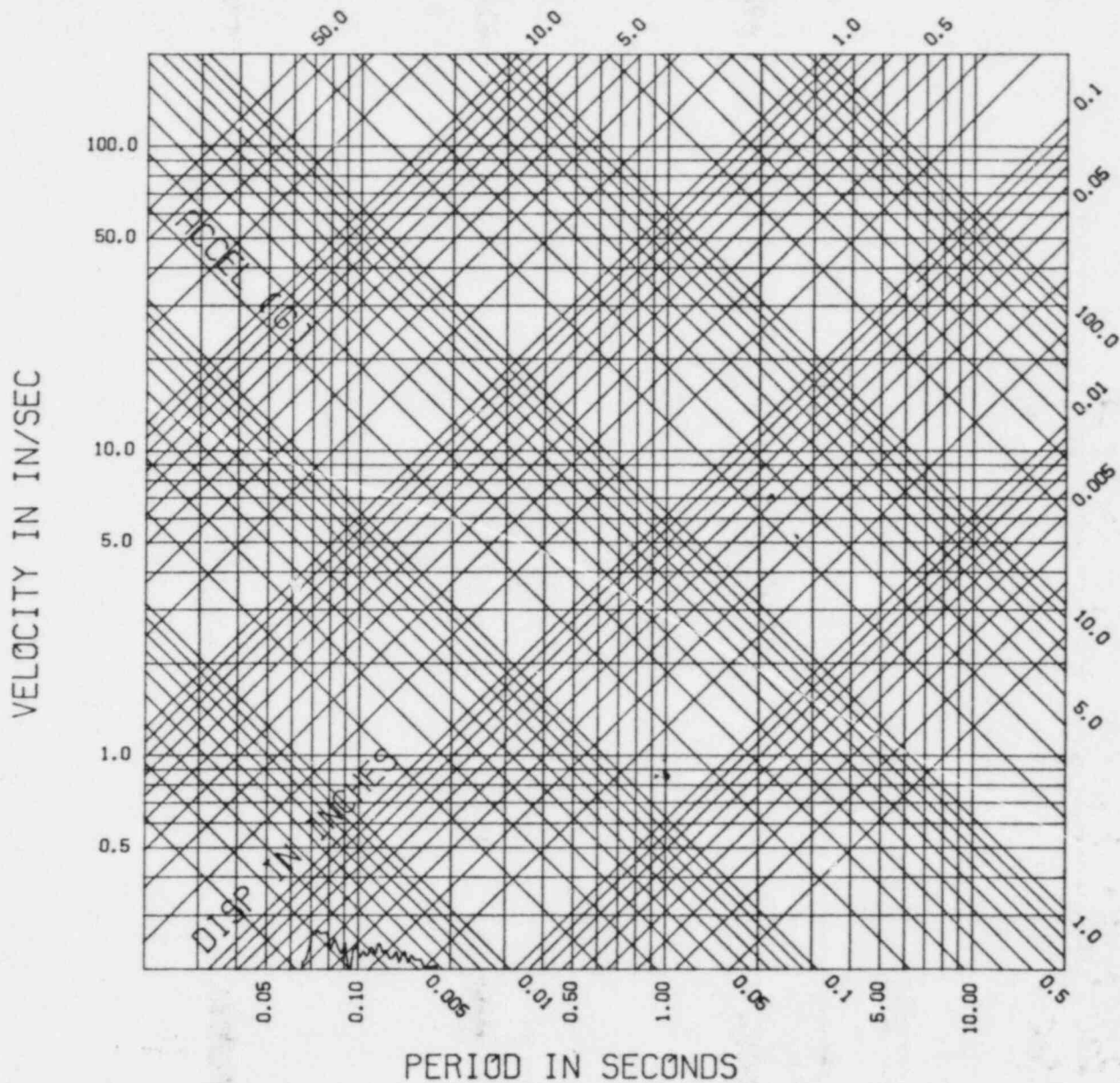
CATAWBA NUCLEAR STATION

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RESPONSE SPECTRUM

CONTAINMENT VESSEL
(AMPLIFICATION ONLY)
OBE VERTICAL RESPONSE AMPLIFICATION INVESTIGATION

DAMPING= 0.01



ENC-1144-09-01-0010
Rev 6
JMM 1-28-83
DEB 1-31-83
SH 189,017

Dev./Station CATAWBA Unit 1st Z File No. CNC-1144.09-01-0010
 Subject VERTICAL & HORIZ. SPECTRA
 Sheet No. 189.018 or Problem No. REV 6 By JMM Date 1-28-83
 Checked By DEB Date 1-31-83

RELATIVE MAGNITUDE OF VERTICAL AND HORIZONTAL SPECTRA

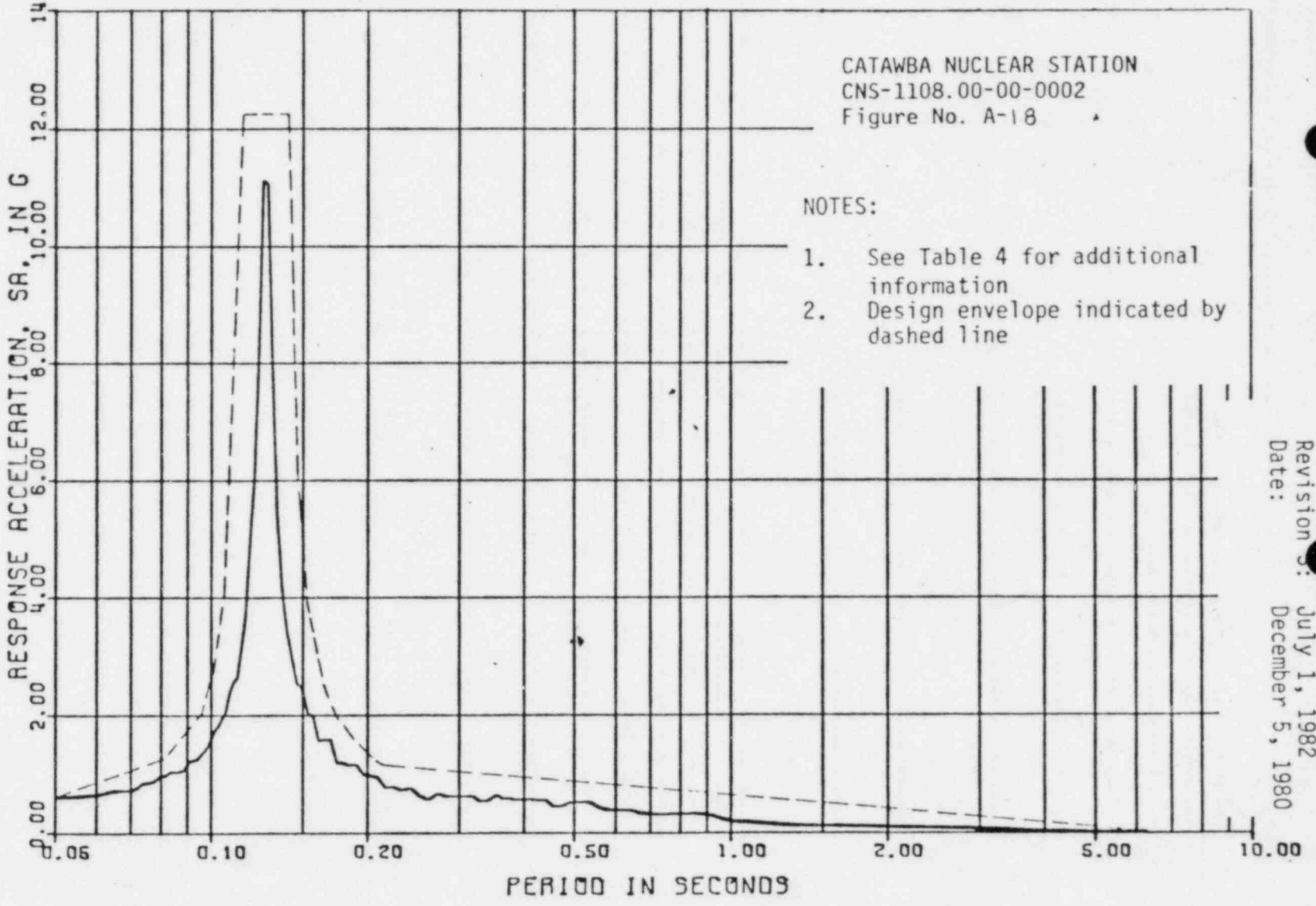
During the discussion of vertical amplification of seismic input, some discussion was raised over the relative magnitude of horizontal and vertical response spectra at one point.

For example, at the top of the dome, Figure A-18 of Specification CNS-1108.00-00-0002 (see next sheet) shows a peak acceleration level on the design envelope (10% above calculated spectrum) of approximately $12.2g$. At the same point, use of $\frac{2}{3}$ of the horizontal ground response spectrum applied in the vertical direction gives a design vertical acceleration of $0.23g$.

The purpose of this section is to address this apparent disparity and to show that it is in fact well founded.

As a starting point, it should be pointed out that the analysis of the lumped mass model shows very good comparison to the overall analysis of the steel containment model. In the table below, the results of the overall analysis are taken from Calc CNC-1144.09-01-0013, while the results of the lumped mass model are from computer run "RZTOTSTK". EXCELLENT CORRELATION IS noted, and the stick model may be taken to accurately reflect overall SCV behavior for these purposes.

CATAWBA CONTAINMENT VESSEL - SEISMIC ANA
 RESPONSE ACCELERATION SPECTRA, DAMPING= 0.010
 CONTAINMENT VESSEL RESPONSE TO OBE (.08G) AT EL 710.85



Revision: July 1, 1982
 Date: December 5, 1980

SH 189.019
 CJC - 1144.09-01-0010
 Rev 6
 JMM 128-83
 DEF 1-31-83

Div./Station CATAWBA
 Subject HORIZ / VERT SPECTRA

Unit 112 File No. CNC-1144.04-01-0010

Sheet No. 189.020
 Problem No. REV G

By JMM Date 1-28-83

Checked By DEB Date 1-31-83

OVERALL SCV ANALYSIS	STICK MODEL ANALYSIS
Mode 1 6.73 Hz overall cantilever mode	Mode 1 7.71 Hz $\Gamma = 80.385$ horizontal cantilever mode
Mode 2 17.76 Hz second cantilever mode	Mode 2 21.09 Hz $\Gamma = 84.091$ vertical vibration
Mode 3 18.76 Hz vertical dome vibration	Mode 3 21.96 Hz $\Gamma = 34.187$ second cantilever mode
	NOTE: FOR ALL THREE MODES, MAXIMUM MODAL DISPLACEMENT OCCURS AT TOP OF DOME

TO PREDICT THE RELATIVE MAGNITUDES OF THE HORIZONTAL AND VERTICAL RESPONSES AT THE TOP OF THE DOME (WHERE ALL MODE SHAPES ARE A MAXIMUM), THE FOLLOWING APPROACH WILL BE FOLLOWED:

USING BIGGS, INTRODUCTION TO STRUCTURAL DYNAMICS, Chapter 6, The Maximum Modal amplitude is given by

$$C_n = \frac{\Gamma_n S_{an}}{\omega_n^2}$$

where Γ_n = participation factor, mode n
 S_{an} = modal acceleration, mode n
 ω_n = frequency, mode n

Dev./Station CATAWBA
 Subject HORIZ / VERT SPECTRA

Unit 172 File No. CXC-1144.09-01-0010

By JMM Date 1-28-83

Sheet No. 189.021 of _____ Problem No. REV 6

Checked By DEB Date 1-21-83

AS INPUT, USE THE GROUND RESPONSE SPECTRUM FOR OBE \neq 1% DAMPING WHICH RESULTS FROM THE FOUR SYNTHETIC TIME HISTORIES (SEE FSAR FIGURE 2.5.2-5 (2 of 4))

for horizontal motion $f_h = 7.71 \text{ Hz} \rightarrow t = .13 \text{ sec} \rightarrow S_{ah} \approx .38g$

for vertical motion $f_v = 21.09 \text{ Hz} \rightarrow t = .047 \text{ sec} \rightarrow S_{av} = .13g$

Therefore the ratio of the horizontal response maximum to the vertical response maximum is given by

$$\frac{\Gamma_h \frac{S_{ah}}{\omega_h^2}}{\Gamma_v \frac{S_{av}}{\omega_v^2}} = \frac{(80.385)(.38)}{(7.71)^2 (2\pi)^2} = \frac{(84.091)(.13)}{(21.09)^2 (2\pi)^2} = 20.91$$

WHEN THE STICK MODEL WAS ANALYZED IN THE VERTICAL DIRECTION FOR A VERTICAL ACCELERATION TIME HISTORY CORRESPONDING TO 100% OF THE HORIZONTAL GROUND RESPONSE SPECTRUM, THE MAXIMUM RESPONSE SPECTRUM ACCELERATION NOTED (FROM SPECTRUM "N", RUN "COMPARE" OR RUN "SPECTRA"), WAS 0.53g

THEREFORE, PREDICT A MAXIMUM IN THE HORIZONTAL DIRECTION OF

$$20.91 (0.53g) = 11.08g$$

TAKING THE 10% ENVELOPE OFF OF THE HORIZONTAL DESIGN SPECTRUM, GET A PEAK OF

$$\frac{120.2g}{1.1} = 11.09g$$

VERY GOOD COMPARISON IS NOTED FOR THIS PREDICTION.

Div./Station

CATAWBA

Unit 112

File No. CNC-114409-01-0010

Subject

HORIZ / VERT SPECTRA

By

JMM

Date

1-28-83

Sheet No.

189-022

Problem No.

REV 6

Checked By

DEB

Date

1-31-83

CONCLUSION:

THE RELATIVE MAGNITUDES OF HORIZONTAL AND VERTICAL RESPONSES ARE IN THE CORRECT PROPORTION TO ONE ANOTHER.

JES2 JOB LOG -- SYSTEM 3034 -- NODE CHURCH

```

2.01 JOB 5543 BHASP173 R2T0T5TK STARTED - INIT 0 - CLASS L - SYS 3034
2.02 JOB 5543 BHASP173 R2T0T5TK - STARTED - TIME=19.32.02
4.05 JOB 5543 +MCC0011 -----M=051 S=00000 E=00066
6.04 JOB 5543 +MUL0011 -----M=102 S=00000 E=00122
8.09 JIB 5543 -----
8.09 JOB 5543 -JOBNAME STEPNAM PRUGRAM KC EXCP CPU SMO CLUCK SERV PG PAGE SWAP VIU SWAPS (APPROX.)
8.09 JOB 5543 -R2T0T5TK STEP1 DFINT 00 R450 30.42 .76 166.1 6150K 1 4480 0 0 0 $ C U S 1
8.09 JOB 5543 BHASP173 R2T0T5TK - ENDED - TIME=17.18.09 TOTAL CPU TIME= 30.49 TOTAL ELAPSED TIME= 166.1 JOB COST=$ 199.77
8.09 JOB 5543 -R2T0T5TK ENDED. NAME=2KELLYJK
8.10 JOB 5543 BHASP195 R2T0T5TK ENDED
    
```

-- JES2 JOB STATISTICS -----

JUL 82 JOB EXECUTION RATE

201 CARDS READ

28,977 SYSOUT PRINT RECORDS

0 SYSOUT PUNCH RECORDS

166.13 MINUTES EXECUTION TIME

THESE DESIGN CALCULATIONS COVER ITEMS RELATING TO NUCLEAR SAFETY IN ACCORDANCE WITH ESTABLISHED PROCEDURES, THE QUALITY HAS BEEN ASSURED.

Jeffrey M. Kelly 8/24/82
 ORIGINATED BY DATE
Daryl E. Byrd 9/1/82
 CHECKED BY DATE

APPROVED BY

DATE

FOR INFORMATION ONLY

.....
MCAUTO ICES EXECUTIVE SYSTEM
.....

RELEASE 3.5 - 1st AUG 1983

LINE 1413213 DATE JUL 28, 1982

MODEL 33 VS2 REL 3.8F
.....

↑ | |
FOR INFORMATION ONLY | |
↑ | |

STRUDL 'CATAWBA' 'TRANSIENT ANALYSIS OF CATAWBA STICK MODEL'

* MCAUTO STRUDL DYNAL RELEASE 4.5 APR 1981 *
* MCAUTO STRUDL PLUTS RELEASE 6.5 *
* MCAUTO STRUDL PLUTS RELEASE 3.5 *
*
* TIME 14.33.23, 7/28/82 *
* DATA POOL SIZE 30040 BYTES *

UNITS FEET LBM POUNDS CYCLES

TYPE PLANE FRAME

JOINT COORDINATES

- 1 0. 0.0 S
- 2 0. 2.83
- 3 0. 11.42
- 4 0. 21.42
- 5 0. 31.92
- 6 0. 41.42
- 7 0. 47.42
- 8 0. 57.42
- 9 0. 67.42
- 10 0. 77.42
- 11 0. 87.42
- 12 0. 97.42
- 13 0. 107.08
- 14 0. 111.75

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15 0. 117.08
16 0. 126.63
17 0. 136.05
18 0. 144.73
19 0. 152.42
20 0. 158.85
21 0. 167.29

MEMBER INCIDENCES

1 1 2
2 2 3
3 3 4
4 4 5
5 5 6
6 6 7
7 7 8
8 8 9
9 9 10
10 10 11
11 11 12
12 12 13
13 13 14
14 14 15
15 15 16
16 16 17
17 17 18
18 18 19

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19 19 20

20 20 21

MEMBER PROPERTIES PRISMATIC

1 2 AX 32.607 AY 16.3035 IZ 53945.5

3 10 13 AX 25.13 AY 12.57 IZ 41585.1

14 AX 20.66 AY 10.33 IZ 34011.2

15 AX 20.30 AY 10.15 IZ 32280.7

16 AX 19.38 AY 9.69 IZ 28078.4

17 AX 17.86 AY 8.93 IZ 21978.1

18 AX 15.80 AY 7.90 IZ 15211.1

19 AX 13.26 AY 6.63 IZ 8988.4

20 AX 8.61 AY 4.31 IZ 2468.1

UNITS INCHES

CONSTANTS

E 29000000. ALL

POISSON .3 ALL

INERTIA OF JOINTS 400

2 LINEAR ALL 160944.1

3 LINEAR ALL 202102.7

4 LINEAR ALL 212576.2

5 LINEAR ALL 205970.7

6 LINEAR ALL 163237.0

7 LINEAR ALL 224458.4

8 LINEAR ALL 284608.7

9 LINEAR ALL 275133.0

10 LINEAR ALL 254522.1

FOR INFORMATION ONLY

11 LINEAR ALL 243713.5
 12 LINEAR ALL 243345.8
 13 LINEAR ALL 188021.5
 14 LINEAR ALL 96592.3
 15 LINEAR ALL 138519.7
 16 LINEAR ALL 98357.9
 17 LINEAR ALL 92283.1
 18 LINEAR ALL 84792.8
 19 LINEAR ALL 73368.0
 20 LINEAR ALL 58705.2
 21 LINEAR ALL 79028.0

FOR INFORMATION ONLY

UNITS FEET

ASSEMBLE FOR DYNAMICS

**** STRIPL MESSAGE - BANDWIDTH STATISTICS ARE AS FOLLOWS :

THE MAXIMUM BANDWIDTH IS 1 AND OCCURS AT JOINT 3
 THE AVERAGE BANDWIDTH IS 0.95
 THE STANDARD DEVIATION IS 0.22

CORRESPONDENCE TABLE

JOINT ID	XI	YI	ZI	XK	YK	ZK	JOINT ID	XI	YI	ZI	XK	YK	ZK
1							2	1	2				3
3	4	5				6	7	8					9
4	10	11				12	13	14					15
7	16	17				18	19	20					21
11	22	23				24	25	26					27
12	28	29				30	31	32					33
13	34	35				36	37	38					39
14	40	41				42	43	44					45
17	46	47				48	49	50					51
19	52	53				54	55	56					57
21	58	59				60							

INDEPENDENT DEGREES OF FREEDOM SELECT
CONDENSE DYNAMIC MATRICES

CONDENSATION CORRESPONDENCE TABLE
INDEPENDENT COORDINATES

JOINT ID	XT	YT	ZT	XR	YR	ZR	JOINT ID	XT	YT	ZT	XR	YR	ZR
2	1	2					3	3	4				
3	5	10					5	7	8				
4	13	14					4	11	12				
5	17	18					11	13	14				
6	21	22					13	15	16				
7	25	26					15	17	18				
8	29	30					17	19	20				
9	33	34					19	21	22				
10	37	38					21	23	24				

CONDENSATION CORRESPONDENCE TABLE
DEPENDENT COORDINATES

JOINT ID	XT	YT	ZT	XR	YR	ZR	JOINT ID	XT	YT	ZT	XR	YR	ZR
2						1	3						2
3						5	5						4
4						7	7						6
5						9	9						8
10						11	11						10
12						13	13						12
14						15	15						14
16						17	17						16
18						19	19						18
20							21						20

**** STRUCL MESSAGE - THE ICES SYSTEM WILL TEMPORARILY BE
ROLLED OUT, WHILE CONDENSATION IS PERFORMED.

ROLL-OUT OF THE ICES SYSTEM COMPLETED ; CONDENSATION INITIATED

FOR INFORMATION ONLY

MCAUTION: ICES EXECUTIVE SYSTEM

RELEASE 1.5 - 14 AUG 1980

LINE 19154125 DATE JUL 28, 1982

MUDEL 33 VS2 REL 3.0P

FOR INFORMATION ONLY

STRUDL

```
*****  
* MCAUTO STRUDL          RELEASE 4.5  APR 1981 *  
* MCAUTO STRUDL DYNAL   RELEASE 6.5 *  
* MCAUTO STRUDL PLUTS   RELEASE 3.5 *  
*  
* TIME 14.34.34. 7/28/82 *  
* DATA POOL SIZE 30640 BYTES *  
*****
```

ROLL-IN OF THE ICES SYSTEM COMPLETED ; CONDENSATION ACCOMPLISHED

FOR INFORMATION ONLY

MODAL ANALYSIS HGM, MAX 33.

FOR INFORMATION ONLY

RESULTS OF LATEST ANALYSIS

PROBLEM - CATANHA TITLE - TRANSIENT ANALYSIS OF CATANHA STICK MODEL
ACTIVE UNITS FEET LR CYC. FAHR SEC LRM

EIGENVALUES ζ

MODE	EIGENVALUE	FREQUENCY	PERIOD
1	0.5947660+02	0.7712110+01	0.1296660+00
2	0.3449030+03	0.2109270+02	0.4740970-01
3	0.4020590+03	0.2195570+02	0.4554620-01

LIST DYNAMIC PARTICIPATION FACTORS

FOR INFORMATION ONLY

RESULTS OF LATEST ANALYSIS

PROBLEM - CATARA TITLE TRANSIENT ANALYSIS OF CATARA STICK MODEL
PARTICIPATION FACTORS (OUTPUT IN INTERNAL UNITS)

MODE	X DISP.	Y DISP.	Z DISP.	X ROT.	ROTATION ROT.	Z ROT.
1	-80.389987	0.000000	0.0	0.0	0.0	0.0
2	0.000000	89.090650	0.0	0.0	0.0	0.0
3	38.186950	-0.000000	0.0	0.0	0.0	0.0

LIST DYNAMIC NORMALIZED MODES

FOR INFORMATION ONLY

RESULTS OF LATEST ANALYSIS

PROBLEM = CATANBA TITLE TRANSIENT ANALYSIS OF CATANBA STICK MODEL
ACTIVE UNITS FEET LB CYC. FAHR SEC LRM

NORMALIZED EIGENVECTORS

MODE 1 MAXIMUM VALUE (IN INTERNAL UNITS) IS -1.972451E-02 AT JOINT 21 IN DIRECTION DISP X

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
21	GLOBAL	0.0	0.0				0.0
	GLOBAL	0.0121339	-0.0000000				-0.0000225
	GLOBAL	0.0512381	-0.0000000				-0.0000869
	GLOBAL	0.1136106	0.0000000				-0.0001744
	GLOBAL	0.1841283	0.0000000				-0.0002550
	GLOBAL	0.2507845	0.0000000				-0.0003164
	GLOBAL	0.2938735	0.0000000				-0.0003535
	GLOBAL	0.3261537	0.0000000				-0.0004052
	GLOBAL	0.3377564	-0.0000000				-0.0004477
	GLOBAL	0.5075781	-0.0000000				-0.0004819
	GLOBAL	0.5748246	-0.0000000				-0.0005088
	GLOBAL	0.5380810	-0.0000000				-0.0005292
	GLOBAL	0.6462001	-0.0000000				-0.0005437
	GLOBAL	0.7221050	-0.0000000				-0.0005491
	GLOBAL	0.7528420	-0.0000000				-0.0005555
	GLOBAL	0.8044424	-0.0000000				-0.0005645
	GLOBAL	0.8533472	-0.0000000				-0.0005713
	GLOBAL	0.8965245	-0.0000000				-0.0005760
	GLOBAL	0.9331077	-0.0000000				-0.0005793
	GLOBAL	0.9622912	-0.0000000				-0.0005815
GLOBAL	1.0000000	-0.0000000				-0.0005846	

MODE 2 MAXIMUM VALUE (IN INTERNAL UNITS) IS 1.603218E-02 AT JOINT 21 IN DIRECTION DISP Y

FOR INFORMATION ONLY

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
17	GLOBAL	0.536153	0.000000				-0.0012088
18	GLOBAL	0.5361945	0.000000				-0.0012444
19	GLOBAL	0.7796087	0.000000				-0.0012697
20	GLOBAL	0.8760223	0.000000				-0.0012872
21	GLOBAL	1.0000000	0.000000				-0.0013129

PLOT DEVICE PLOTTER

PLOT FORMAT NORMAL

TITLE 'CATAMBA STICK MODEL - MODE SHAPES'

TITLE BLOCK

JOB 'CATAMBA NUCLEAR STATION'

FOR 'STICK TRANSIENT ANALYSIS'

BY 'DOYLE E. BYRD'

DISPLAY MODE SHAPE THREE DIMENSIONAL

PLOT FINISH

DAMPING RATIO .01 10

TRANSIENT LOAD 1 'SUPPORT ACCEL. IN HORIZ. X-DIRECTION FOR EARTHQUAKE 1'

SUPPORT ACCELERATIONS

TRANSLATION X FILE 'CATEQ1' FACTOR .25

INTEGRATE FROM 0. TO 20. AT DELTA .01

END

TRANSIENT LOAD 2 'SUPPORT ACCEL. IN HORIZ. X-DIRECTION FOR EARTHQUAKE 2'

SUPPORT ACCELERATIONS

TRANSLATION X FILE 'CATEQ2' FACTOR .25

INTEGRATE FROM 0. TO 20. AT DELTA .01

END

TRANSIENT LOAD 3 'SUPPORT ACCEL. IN HORIZ. X-DIRECTION FOR EARTHQUAKE 3'

FOR INFORMATION ONLY

SUPPORT ACCELERATIONS

TRANSLATION X FILE 'CATEQ3' FACTOR .25

INTEGRATE FROM 0. TO 20. AT DELTA .01

END

TRANSIENT LOAD 4 'SUPPORT ACCEL. IN HORIZ. X-DIRECTION FOR EARTHQUAKE 4'

SUPPORT ACCELERATIONS

TRANSLATION X FILE 'CATEQ4' FACTOR .25

INTEGRATE FROM 0. TO 20. AT DELTA .01

END

TRANSIENT LOAD 5 'SUPPORT ACCEL. IN VERT. Y-DIRECTION FOR EARTHQUAKE 1'

SUPPORT ACCELERATIONS

TRANSLATION Y FILE 'CATEQ1' FACTOR .25

INTEGRATE FROM 0. TO 20. AT DELTA .01

END

TRANSIENT LOAD 6 'SUPPORT ACCEL. IN VERT. Y-DIRECTION FOR EARTHQUAKE 2'

SUPPORT ACCELERATIONS

TRANSLATION Y FILE 'CATEQ2' FACTOR .25

INTEGRATE FROM 0. TO 20. AT DELTA .01

END

TRANSIENT LOAD 7 'SUPPORT ACCEL. IN VERT. Y-DIRECTION FOR EARTHQUAKE 3'

SUPPORT ACCELERATIONS

TRANSLATION Y FILE 'CATEQ3' FACTOR .25

INTEGRATE FROM 0. TO 20. AT DELTA .01

END

TRANSIENT LOAD 8 'SUPPORT ACCEL. IN VERT. Y-DIRECTION FOR EARTHQUAKE 4'

SUPPORT ACCELERATIONS

FOR INFORMATION ONLY

TRANSLATION Y FILE 'CATEQ4' FACTOR .25

INTEGRATE FROM 0. TO 20. AT DELTA .01

END

TRANSIENT ANALYSIS LOADS 1 2 3 4 5 6 7 8

**** STRUCL WARNING - NO INITIAL CONDITIONS SPECIFIED FOR TRANSIENT LOAD 1
ZEROS ASSUMED.

**** STRUCL WARNING - NO INITIAL CONDITIONS SPECIFIED FOR TRANSIENT LOAD 2
ZEROS ASSUMED.

**** STRUCL WARNING - NO INITIAL CONDITIONS SPECIFIED FOR TRANSIENT LOAD 3
ZEROS ASSUMED.

**** STRUCL WARNING - NO INITIAL CONDITIONS SPECIFIED FOR TRANSIENT LOAD 4
ZEROS ASSUMED.

**** STRUCL WARNING - NO INITIAL CONDITIONS SPECIFIED FOR TRANSIENT LOAD 5
ZEROS ASSUMED.

**** STRUCL WARNING - NO INITIAL CONDITIONS SPECIFIED FOR TRANSIENT LOAD 6
ZEROS ASSUMED.

**** STRUCL WARNING - NO INITIAL CONDITIONS SPECIFIED FOR TRANSIENT LOAD 7
ZEROS ASSUMED.

FOR INFORMATION ONLY

**** STRUDL WARNING - NO INITIAL CONDITIONS SPECIFIED FOR TRANSIENT LOAD 8
ZEROS ASSUMED.

STORE TRANSIENT 'TRANHORZ'
USE MODAL RESULTS OF TRANSIENT ANALYSIS 1 2 3 4 FROM TIME 0. 10 20.
JOINT TOTAL ACCELERATIONS

1 6 9 14 17 20 TRANSLATION X

END

STORE TRANSIENT 'TRANVERT'
USE MODAL RESULTS OF TRANSIENT ANALYSIS 5 6 7 8 FROM TIME 0. 10 20.
JOINT TOTAL ACCELERATIONS

1 6 9 14 17 20 TRANSLATION Y

END

COMPUTE STORE TRANS 'TRANHORZ' 'TRANVERT'

UNITS RADIAN'S

STORE SPECTRUM 'HORZEQ'

RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.

OSCIL DAMP RATIO .005 .01 .02 .05

USE STORE 'TRANHORZ'

USE TRANSIENT 1 2 3 4 FROM TIME 0. 10 20.

JOINT TOTAL ACCELERATIONS

1 6 9 14 17 20 TRANSLATION X

END

STORE SPECTRUM 'VERTEQ'

RANGE FREQ LIN 1. 4. .15 4. 20. .3 20. 65. 1. 65. 210. 2.

OSCIL DAMP RATIO .005 .01 .02 .05

FOR INFORMATION ONLY

USE STORE 'TRANVERT'

USE TRANSIENT 5 6 7 8

JOINT TOTAL ACCELERATION

1 6 9 14 17 20 TRANSLATION Y

END

COMPUTE AND STORE SPECTRA RESULTS 'HORZER' 'VERTED'

SAVE 'SPECTRA' SCAN CHECK

DD1 DSN=STST.R030.C8362.JMK.PER.STICK

VOLSER=MSST02

*****NON-DESTRUCTIVE SAVE SUCCESSFUL FOR FILE SPECTRA USING MAP SAVESTRU
*****ON JUL 26, 1982 AT 17.09.08 * THIS FILE HAS BEEN SAVED 1 TIME(S)

OUTPUT STORED TRANSIENT RESULTS

PLUT ON

ABSCISSA SCALE ON INDIVIDUAL BASIS

ORDINATE SCALE ON INDIVIDUAL BASIS

USE STORE TRANSIENT SET IDENT 'TRANMURZ'

USE TRANSIENT ANALYSIS 1 2 3 4

JOINT TOTAL ACCELERATION

1 6 9 14 17 20 TRANSLATION X

END SUBSET

FOR INFORMATION ONLY

 RESULTS OF LATEST ANALYSIS

PROBLEM = CATAHBA TITLE | TRANSIENT ANALYSIS OF CATAHBA STICK MODEL

ACTIVE UNITS FEET LB RAD. FAHR SEC LHM

TRANSIENT LOAD 1

TIME HISTORY FOR JOINT 1 XT TOTAL ACCELERATION

TIME	VALUE	TIME	VALUE	TIME	VALUE	TIME	VALUE
0.0	0.0	4.44444E-03	-6.74444E-03	2.00000E-02	-8.47444E-03	3.00000E-02	-1.08250E-02
4.00000E-02	-1.20750E-02	5.00000E-02	-1.23750E-02	6.00000E-02	-1.16744E-02	6.44444E-02	-9.47444E-03
7.44444E-02	-7.02444E-03	8.44444E-02	-3.74444E-03	9.44444E-02	1.45000E-03	1.10000E-01	-5.82500E-03
1.19999E-01	7.22444E-03	1.30000E-01	1.70000E-01	1.40000E-01	1.89994E-03	1.50000E-01	-2.65304E-03
1.60000E-01	-6.57500E-03	1.70000E-01	-9.00000E-03	1.79994E-01	-4.54444E-03	1.90000E-01	-8.34444E-03
2.00000E-01	-5.47444E-03	2.10000E-01	-2.62444E-03	2.20000E-01	1.15000E-03	2.30000E-01	5.35000E-03
2.44444E-01	0.40000E-03	2.44444E-01	2.44444E-01	2.60000E-01	-5.34444E-03	2.70000E-01	-1.05000E-02
2.80000E-01	-1.45000E-02	2.90000E-01	-2.13944E-02	3.00000E-01	-2.10244E-02	3.09994E-01	-1.38444E-02
3.20000E-01	-3.72444E-03	3.30000E-01	3.30000E-01	3.44444E-01	7.44444E-03	3.50000E-01	5.44444E-03
3.60000E-01	1.70444E-03	3.69994E-01	4.10000E-01	3.74444E-01	2.97550E-03	3.90000E-01	1.71750E-02
4.00000E-01	3.71500E-02	4.10000E-01	4.50000E-01	4.19744E-02	0.19744E-02	4.24444E-01	3.00740E-02
4.40000E-01	4.49944E-02	4.50000E-01	4.50000E-01	4.71000E-02	3.71000E-02	4.70000E-01	3.86000E-02
4.80000E-01	4.20250E-02	4.94444E-01	4.94444E-01	5.17250E-02	5.17250E-02	5.10000E-01	7.32740E-02
5.20000E-01	4.98944E-02	5.30000E-01	5.30000E-01	5.12444E-02	5.50000E-01	5.50000E-01	7.32500E-02
5.64444E-01	-2.17500E-02	5.70000E-01	5.70000E-01	5.89744E-02	5.90000E-01	6.30000E-01	6.73250E-02
6.00000E-01	-6.42444E-02	6.10000E-01	6.10000E-01	-6.83254E-02	6.30000E-01	6.70500E-01	6.70500E-02
6.40000E-01	-4.44014E-01	6.50000E-01	6.50000E-01	-1.50051E-01	6.70000E-01	6.70000E-01	-1.81820E-01
6.74444E-01	-2.08675E-01	6.90000E-01	6.90000E-01	-2.07147E-01	7.10000E-01	7.10000E-01	-1.70646E-01
7.20000E-01	-1.22444E-01	7.30000E-01	7.30000E-01	-4.75735E-02	7.44444E-01	7.44444E-01	-2.62444E-01
7.60000E-01	-5.42314E-03	7.70000E-01	7.70000E-01	4.94518E-02	7.90000E-01	7.90000E-01	-7.60014E-02
8.00000E-01	4.00000E-02	8.04444E-01	8.04444E-01	1.05144E-01	8.30000E-01	8.30000E-01	6.58470E-02
8.40000E-01	4.85463E-02	8.50000E-01	8.50000E-01	-4.22773E-02	8.69994E-01	8.69994E-01	-7.27534E-02
8.80000E-01	-4.00000E-02	8.94444E-01	8.94444E-01	-1.15275E-01	9.10000E-01	9.10000E-01	-1.24497E-01
9.20000E-01	-1.27444E-01	9.30000E-01	9.30000E-01	-1.16224E-01	9.50000E-01	9.50000E-01	-1.12244E-01
9.60000E-01	-1.04225E-01	9.70000E-01	9.70000E-01	-5.77401E-02	9.84444E-01	9.84444E-01	-1.50716E-01
9.94444E-01	-2.20765E-02	1.00000E+00	1.00000E+00	3.20444E-02	1.03000E+00	1.03000E+00	-2.61764E-01
1.04000E+00	-3.43050E-02	1.04444E+00	1.04444E+00	-1.03471E-01	1.07000E+00	1.07000E+00	-4.61764E-01
1.08000E+00	-5.51357E-02	1.08000E+00	1.08000E+00	5.26407E-02	1.11000E+00	1.11000E+00	-7.32934E-01
1.12000E+00	6.57474E-02	1.12444E+00	1.12444E+00	3.40440E-02	1.15000E+00	1.15000E+00	4.63484E-01
1.16000E+00	6.74510E-02	1.16944E+00	1.16944E+00	1.11141E-01	1.19000E+00	1.19000E+00	1.42437E-01
1.20000E+00	1.00474E-01	1.20444E+00	1.20444E+00	5.67443E-02	1.23000E+00	1.23000E+00	-6.63762E-01
1.24000E+00	-2.12564E-01	1.24000E+00	1.24000E+00	-2.37161E-02	1.27000E+00	1.27000E+00	-5.95002E-02
1.28000E+00	-1.09444E-01	1.28000E+00	1.28000E+00	-1.42474E-01	1.30994E+00	1.30994E+00	-1.65410E-01
1.32000E+00	-2.02744E-01	1.32000E+00	1.32000E+00	-2.44502E-01	1.34994E+00	1.34994E+00	-2.90861E-01
1.36000E+00	-2.47174E-01	1.36000E+00	1.36000E+00	-3.08423E-01	1.38994E+00	1.38994E+00	-5.74441E-01

REMAINDER OF
 RUN CONSISTS OF
 ACCELERATION
 TIME HISTORIES
 FOR VARIOUS
 JOINTS

FOR INFORMATION ONLY

MCAUTO ICES EXECUTIVE SYSTEM
RELEASE 3.5 - 14 AUG 1980
TIME - 17:35
MODEL 33
V52 REL 3.8F

FOR INFORMATION ONLY

STRUDL *CATAMBA* *RESPONSE SPECTRA FROM STICK TRANSIENT ANALYSIS*

MCAUTO STRUDL
MCAUTO STRUDL DYNAL
MCAUTO STRUDL PL0TS

RELEASE 4.4 OCT 1980
RELEASE 6.4
RELEASE 3.4

TIME 17.35.14, 7/28/82

DATA P00L SIZE 30640 BYTES

RESTORE *SPECTRA*

DD1 DSNAME=TSY.R030.C0362.JMK.OEB.STICK

V0LSER=MSST02

*****NON-DESTRUCTIVE RESTORE SUCCESSFUL FOR FILE SPECTRA USING MAP SAVESTRU
 ****THIS FILE HAS BEEN SAVED 1 TIME(S), THE MOST RECENT SAVE ON JUL 28, 1982 AT 17.09.08 USING MAP SAVESTRU
 ****THIS FILE HAS BEEN RESTORED 4 TIME(S) SINCE THEN
 * JOB STATUS AT TIME OF SAVE:
 ACTIVE UNITS ARE: FEET LB RAD. FAHR SEC LBM
 IN ADDITIONS MODE

UNITS CYCLES

OUTPUT SPECTRUM

PUNCH ON

PL0T ON

ABSCISSA SCALE LOG ON INDIVIDUAL BASIS

ORDINATE SCALE ON INDIVIDUAL BASIS

DAMPING CURVES PER PL0T 1

OUTPUT G TRUE ACCEL VS PERIOD

USE STORE *HORZEQ*

USE TRANSIENT ANALYSIS 1 2 3 4

USE DAMP ALL

FOR INFORMATION ONLY

COMBINE SPECTRA BY PEAK

JOINT TOTAL ACCELERATION

1 6 7 14 17 20 TRANSLATION X

END SUBSET

FOR INFORMATION ONLY

 RESULTS OF LATEST ANALYSIS

PROBLEM - CATANBA TITLE TRANSIENT ANALYSIS OF CATANBA STICK MODEL
 ACTIVE UNITS FEET LB CYC. FAHR SEC LBM

INPUT HORIZONTAL SPECTRUM
 AT BASE DUE TO 4-TIME HIST.

SPECTRUM IDENT HØRZ EQ XT TØT
 SPECTRUM FROM JOINT 1
 COMBINED TRANSIENTS PEAK
 DAMPING = 0.010

FREQUENCY	PSEUDO DISPLACEMENT	PSEUDO VELOCITY	TRUE ACCELERATION	TRUE G	PERIOD
0.159155E+00	0.281422E+00	0.281422E+00	0.281422E+00	0.873793E-02	0.628337E+01
0.183028E+00	0.306674E+00	0.335767E+00	0.405602E+00	0.125963E-01	0.546364E+01
0.206701E+00	0.371102E+00	0.482302E+00	0.126792E+00	0.174710E-01	0.483322E+01
0.230775E+00	0.431834E+00	0.626746E+00	0.407932E+00	0.281966E-01	0.433323E+01
0.254648E+00	0.449438E+00	0.719100E+00	0.115056E+01	0.357317E-01	0.392679E+01
0.278521E+00	0.470967E+00	0.824192E+00	0.144234E+01	0.447930E-01	0.359039E+01
0.302394E+00	0.557722E+00	0.105976E+01	0.201338E+01	0.625272E-01	0.330674E+01
0.326267E+00	0.67118E+00	0.138104E+01	0.242113E+01	0.751905E-01	0.306477E+01
0.350141E+00	0.847028E+00	0.182378E+01	0.265727E+01	0.825247E-01	0.285594E+01
0.374014E+00	0.478884E+00	0.117238E+01	0.275508E+01	0.855616E-01	0.267370E+01
0.397887E+00	0.778652E+00	0.119663E+01	0.279157E+01	0.729060E-01	0.251327E+01
0.421760E+00	0.477885E+00	0.118828E+01	0.306793E+01	0.453240E-01	0.237101E+01
0.445634E+00	0.412815E+00	0.112788E+01	0.315807E+01	0.480768E-01	0.224394E+01
0.469507E+00	0.313660E+00	0.113160E+01	0.333280E+01	0.103689E+00	0.212498E+01
0.493380E+00	0.314745E+00	0.114621E+01	0.355325E+01	0.110350E+00	0.202183E+01
0.517253E+00	0.351832E+00	0.114345E+01	0.371623E+01	0.115411E+00	0.193329E+01
0.541127E+00	0.325342E+00	0.113616E+01	0.376096E+01	0.116800E+00	0.184800E+01
0.565000E+00	0.303104E+00	0.107602E+01	0.381986E+01	0.118629E+00	0.176491E+01
0.588873E+00	0.281322E+00	0.104089E+01	0.385130E+01	0.119606E+00	0.169811E+01
0.612746E+00	0.273084E+00	0.105137E+01	0.404779E+01	0.125702E+00	0.163200E+01
0.636620E+00	0.264179E+00	0.105672E+01	0.422687E+01	0.132679E+00	0.157008E+01
0.660493E+00	0.222662E+00	0.105745E+00	0.411701E+01	0.137858E+00	0.146121E+01
0.684366E+00	0.187852E+00	0.873314E+00	0.401727E+01	0.142478E+00	0.136571E+01
0.708240E+00	0.168232E+00	0.822337E+00	0.451545E+01	0.140356E+00	0.128228E+01
0.732113E+00	0.149550E+00	0.793859E+00	0.515247E+01	0.140014E+00	0.120830E+01
0.755987E+00	0.180640E+00	0.793522E+00	0.541437E+01	0.147201E+00	0.114240E+01
0.779860E+00	0.178780E+00	0.103673E+01	0.546437E+01	0.146776E+00	0.108331E+01
0.803734E+00	0.165806E+00	0.101141E+01	0.611763E+01	0.141603E+00	0.103003E+01
0.827607E+00	0.173302E+00	0.113091E+01	0.614984E+01	0.220449E+00	0.981748E+00
0.851481E+00	0.146859E+00	0.113676E+01	0.883701E+01	0.274441E+00	0.937784E+00
0.875354E+00	0.149131E+00	0.113191E+01	0.932422E+01	0.291125E+00	0.897548E+00
0.899228E+00	0.144793E+00	0.113799E+01	0.103605E+02	0.322376E+00	0.860710E+00
0.923101E+00	0.18379E+00	0.137622E+01	0.101113E+02	0.329544E+00	0.826735E+00
0.946975E+00	0.1699E+00	0.134296E+01	0.106794E+02	0.329444E+00	0.795340E+00

FOR INFORMATION ONLY

PERIOD
↑
TRUE ACCELERATION
TRUE VELOCITY
TRUE DISPLACEMENT
FREQUENCY

FOR INFORMATION ONLY

 RESULTS OF LATEST ANALYSIS

PROBLEM - CATAMBA TITLE - TRANSIENT ANALYSIS OF CATAMBA STICK MODEL
 ACTIVE, UNITS FEET LB CYC. FAHR SEC LBS

HORIZONTAL SPECTRUM AT
 TOP OF DOME DUE TO
 A-TIME HISTORIES

SPECTRUM FRB4 SPECTRUM IDENT HORZ EQ XT TBT
 JOINT 20
 COMBINED TRANSIENTS PEAK
 DAMPING = 0.010

FREQUENCY	PSEUDO DISPLACEMENT	PSEUDO VELOCITY	TRUE ACCELERATION	TRUE G	PERIOD
0.159355E+00	0.285308E+00	0.285308E+00	0.285308E+00	0.886097E-02	0.628317E+01
0.183028E+00	0.311413E+00	0.311413E+00	0.411843E+00	0.127901E-01	0.546364E+01
0.206401E+00	0.374127E+00	0.486368E+00	0.632278E+00	0.196360E-01	0.483322E+01
0.230775E+00	0.433992E+00	0.486368E+00	0.912467E+00	0.283375E-01	0.433323E+01
0.254648E+00	0.453451E+00	0.725222E+00	0.116083E+01	0.360508E-01	0.392697E+01
0.278521E+00	0.473712E+00	0.825996E+00	0.145074E+01	0.450541E-01	0.359037E+01
0.302394E+00	0.560602E+00	0.101514E+01	0.202377E+01	0.628501E-01	0.330697E+01
0.326267E+00	0.581033E+00	0.121999E+01	0.244177E+01	0.758317E-01	0.306497E+01
0.350141E+00	0.554543E+00	0.121999E+01	0.268399E+01	0.833536E-01	0.285597E+01
0.374014E+00	0.503911E+00	0.115419E+01	0.278285E+01	0.864238E-01	0.267370E+01
0.397887E+00	0.481787E+00	0.123447E+01	0.301118E+01	0.935149E-01	0.251327E+01
0.421760E+00	0.439931E+00	0.115882E+01	0.308941E+01	0.959446E-01	0.237101E+01
0.445634E+00	0.404982E+00	0.113955E+01	0.317506E+01	0.986044E-01	0.224399E+01
0.469507E+00	0.386902E+00	0.114136E+01	0.336701E+01	0.104566E+00	0.212989E+01
0.493380E+00	0.373732E+00	0.115857E+01	0.359156E+01	0.111539E+00	0.202683E+01
0.517253E+00	0.356523E+00	0.115870E+01	0.376577E+01	0.116949E+00	0.193329E+01
0.541127E+00	0.330119E+00	0.112411E+01	0.382196E+01	0.118694E+00	0.184800E+01
0.565000E+00	0.306473E+00	0.107988E+01	0.386233E+01	0.119948E+00	0.176991E+01
0.588873E+00	0.285097E+00	0.105486E+01	0.390298E+01	0.121211E+00	0.169816E+01
0.612746E+00	0.267355E+00	0.101781E+01	0.411109E+01	0.127674E+00	0.163200E+01
0.636620E+00	0.252750E+00	0.107036E+01	0.428194E+01	0.132964E+00	0.157080E+01
0.660493E+00	0.227524E+00	0.978352E+00	0.426691E+01	0.130649E+00	0.146121E+01
0.684366E+00	0.193237E+00	0.888892E+00	0.408890E+01	0.126985E+00	0.136591E+01
0.708240E+00	0.142288E+00	0.944368E+00	0.462740E+01	0.143708E+00	0.128228E+01
0.732113E+00	0.119520E+00	0.101504E+01	0.527623E+01	0.163920E+00	0.120830E+01
0.755987E+00	0.165179E+00	0.101849E+01	0.560367E+01	0.173965E+00	0.114240E+01
0.779860E+00	0.184713E+00	0.107133E+01	0.621373E+01	0.192773E+00	0.108331E+01
0.803734E+00	0.170672E+00	0.109110E+01	0.635069E+01	0.197226E+00	0.103003E+01
0.827607E+00	0.179133E+00	0.114645E+01	0.733279E+01	0.227866E+00	0.951748E+00
0.851481E+00	0.204553E+00	0.137051E+01	0.978240E+01	0.285168E+00	0.737787E+00
0.875354E+00	0.180409E+00	0.138886E+01	0.972203E+01	0.301926E+00	0.877598E+00
0.899228E+00	0.203294E+00	0.149405E+01	0.108335E+02	0.336446E+00	0.860710E+00
0.923101E+00	0.192417E+00	0.146617E+01	0.111429E+02	0.346052E+00	0.826735E+00
0.946975E+00	0.177687E+00	0.143733E+01	0.110895E+02	0.344393E+00	0.795340E+00

FOR INFORMATION ONLY

PERIOD	TRUE G	TRUE ACCELERATION	PSEUDO VELOCITY	PSEUDO DISPLACEMENT	FREQUENCY
00	00	00	00	00	00
01	00	00	00	00	00
02	00	00	00	00	00
03	00	00	00	00	00
04	00	00	00	00	00
05	00	00	00	00	00
06	00	00	00	00	00
07	00	00	00	00	00
08	00	00	00	00	00
09	00	00	00	00	00
10	00	00	00	00	00
11	00	00	00	00	00
12	00	00	00	00	00
13	00	00	00	00	00
14	00	00	00	00	00
15	00	00	00	00	00
16	00	00	00	00	00
17	00	00	00	00	00
18	00	00	00	00	00
19	00	00	00	00	00
20	00	00	00	00	00
21	00	00	00	00	00
22	00	00	00	00	00
23	00	00	00	00	00
24	00	00	00	00	00
25	00	00	00	00	00
26	00	00	00	00	00
27	00	00	00	00	00
28	00	00	00	00	00
29	00	00	00	00	00
30	00	00	00	00	00
31	00	00	00	00	00
32	00	00	00	00	00
33	00	00	00	00	00
34	00	00	00	00	00
35	00	00	00	00	00
36	00	00	00	00	00
37	00	00	00	00	00
38	00	00	00	00	00
39	00	00	00	00	00
40	00	00	00	00	00
41	00	00	00	00	00
42	00	00	00	00	00
43	00	00	00	00	00
44	00	00	00	00	00
45	00	00	00	00	00
46	00	00	00	00	00
47	00	00	00	00	00
48	00	00	00	00	00
49	00	00	00	00	00
50	00	00	00	00	00
51	00	00	00	00	00
52	00	00	00	00	00
53	00	00	00	00	00
54	00	00	00	00	00
55	00	00	00	00	00
56	00	00	00	00	00
57	00	00	00	00	00
58	00	00	00	00	00
59	00	00	00	00	00
60	00	00	00	00	00
61	00	00	00	00	00
62	00	00	00	00	00
63	00	00	00	00	00
64	00	00	00	00	00
65	00	00	00	00	00
66	00	00	00	00	00
67	00	00	00	00	00
68	00	00	00	00	00
69	00	00	00	00	00
70	00	00	00	00	00
71	00	00	00	00	00
72	00	00	00	00	00
73	00	00	00	00	00
74	00	00	00	00	00
75	00	00	00	00	00
76	00	00	00	00	00
77	00	00	00	00	00
78	00	00	00	00	00
79	00	00	00	00	00
80	00	00	00	00	00
81	00	00	00	00	00
82	00	00	00	00	00
83	00	00	00	00	00
84	00	00	00	00	00
85	00	00	00	00	00
86	00	00	00	00	00
87	00	00	00	00	00
88	00	00	00	00	00
89	00	00	00	00	00
90	00	00	00	00	00
91	00	00	00	00	00
92	00	00	00	00	00
93	00	00	00	00	00
94	00	00	00	00	00
95	00	00	00	00	00
96	00	00	00	00	00
97	00	00	00	00	00
98	00	00	00	00	00
99	00	00	00	00	00
00	00	00	00	00	00

FOR INFORMATION ONLY

PERIOD
TRUE VELOCITY
TRUE ACCELERATION
PSEUDO VELOCITY
PSEUDO DISPLACEMENT
FREQUENCY

FOR INFORMATION ONLY

 RESULTS OF LATEST ANALYSIS

PROBLEM - CATANBA TITLE - TRANSIENT ANALYSIS OF CATANBA STICK MODEL
 ACTIVE UNITS FEET LB CYC. FAHR SEC LBM

VERTICAL SPECTRUM AT BASE
 DUE TO 4-TIME HISTORIES
 " SPECTRUM I "

SPECTRUM IDENT VERTEQ
 FROM JOINT 1 YR TOT
 COMBINED TRANSIENTS PEAK
 DAMPING = 0.010

FREQUENCY	PSEUDO DISPLACEMENT	PSEUDO VELOCITY	TRUE ACCELERATION	TRUE G	PERIOD
0.159155E+00	0.281422E+00	0.281422E+00	0.281422E+00	0.873783E-02	0.628319E+01
0.183028E+00	0.306649E+00	0.355647E+00	0.405602E+00	0.125463E-01	0.546364E+01
0.206401E+00	0.371002E+00	0.482302E+00	0.626472E+00	0.174718E-01	0.483322E+01
0.230775E+00	0.431834E+00	0.626160E+00	0.707932E+00	0.281766E-01	0.433323E+01
0.254648E+00	0.447438E+00	0.717100E+00	0.115056E+01	0.357317E-01	0.372679E+01
0.278521E+00	0.470467E+00	0.524142E+00	0.144234E+01	0.447930E-01	0.359037E+01
0.302394E+00	0.557722E+00	0.105467E+01	0.201338E+01	0.625272E-01	0.330674E+01
0.326267E+00	0.576118E+00	0.115104E+01	0.242113E+01	0.751905E-01	0.306497E+01
0.350141E+00	0.547028E+00	0.120786E+01	0.265709E+01	0.825247E-01	0.285549E+01
0.374014E+00	0.498884E+00	0.117238E+01	0.275508E+01	0.855616E-01	0.267370E+01
0.397887E+00	0.478652E+00	0.117663E+01	0.277157E+01	0.929060E-01	0.251327E+01
0.421760E+00	0.437085E+00	0.115828E+01	0.306493E+01	0.953240E-01	0.237101E+01
0.445634E+00	0.402255E+00	0.112788E+01	0.315807E+01	0.980768E-01	0.224399E+01
0.469507E+00	0.383660E+00	0.113160E+01	0.333880E+01	0.103689E+00	0.212489E+01
0.493380E+00	0.364745E+00	0.114621E+01	0.355325E+01	0.110350E+00	0.202683E+01
0.517253E+00	0.351832E+00	0.114345E+01	0.371123E+01	0.115411E+00	0.193329E+01
0.541127E+00	0.325342E+00	0.113166E+01	0.376046E+01	0.116800E+00	0.184800E+01
0.565000E+00	0.303104E+00	0.107602E+01	0.381486E+01	0.118629E+00	0.176491E+01
0.588873E+00	0.281322E+00	0.104089E+01	0.385130E+01	0.119606E+00	0.169816E+01
0.612746E+00	0.273084E+00	0.105137E+01	0.404779E+01	0.125708E+00	0.163200E+01
0.636620E+00	0.264179E+00	0.105672E+01	0.422687E+01	0.131267E+00	0.157008E+01
0.660493E+00	0.222662E+00	0.957445E+00	0.411201E+01	0.127858E+00	0.146121E+01
0.732113E+00	0.187852E+00	0.873349E+00	0.401727E+01	0.124760E+00	0.136591E+01
0.779854E+00	0.168823E+00	0.922337E+00	0.451745E+01	0.140356E+00	0.128228E+01
0.827605E+00	0.190550E+00	0.990859E+00	0.515247E+01	0.160014E+00	0.120830E+01
0.875352E+00	0.180640E+00	0.993522E+00	0.546437E+01	0.164701E+00	0.114240E+01
0.923099E+00	0.178780E+00	0.103673E+01	0.601447E+01	0.186776E+00	0.108331E+01
0.970845E+00	0.165806E+00	0.101141E+01	0.616463E+01	0.191603E+00	0.103003E+01
0.101854E+01	0.173302E+00	0.110413E+01	0.704845E+01	0.220449E+00	0.981748E+00
0.106634E+01	0.146859E+00	0.131896E+01	0.883701E+01	0.274441E+00	0.737789E+00
0.111408E+01	0.149331E+00	0.133717E+01	0.937422E+01	0.291125E+00	0.877588E+00
0.116183E+01	0.144743E+00	0.142149E+01	0.103805E+02	0.322376E+00	0.860710E+00
0.120958E+01	0.183714E+00	0.139622E+01	0.106113E+02	0.324544E+00	0.826735E+00
0.125732E+01	0.164445E+00	0.134296E+01	0.106094E+02	0.329484E+00	0.795340E+00

FOR INFORMATION ONLY

FREQUENCY	PSEUDO DISPLACEMENT	PSEUDO VELOCITY	TRUE ACCELERATION	TRUE G	PERIOD
0.281704E+02	0.114159E-03	0.202062E-01	0.357650E+01	0.111072E+00	0.354982E-01
0.284887E+02	0.111591E-03	0.199709E-01	0.357479E+01	0.111018E+00	0.355106E-01
0.288070E+02	0.109011E-03	0.197400E-01	0.357294E+01	0.110961E+00	0.354738E-01
0.291254E+02	0.106632E-03	0.195137E-01	0.357100E+01	0.110901E+00	0.354373E-01
0.294437E+02	0.104262E-03	0.192922E-01	0.356905E+01	0.110840E+00	0.354011E-01
0.297620E+02	0.102010E-03	0.190758E-01	0.356708E+01	0.110782E+00	0.353654E-01
0.300803E+02	0.998407E-04	0.188649E-01	0.356511E+01	0.110728E+00	0.353304E-01
0.303986E+02	0.977320E-04	0.186630E-01	0.356313E+01	0.110673E+00	0.352959E-01
0.307169E+02	0.956556E-04	0.184615E-01	0.356108E+01	0.110615E+00	0.352619E-01
0.310352E+02	0.936645E-04	0.182646E-01	0.355905E+01	0.110558E+00	0.352285E-01
0.313535E+02	0.917359E-04	0.180720E-01	0.355701E+01	0.110501E+00	0.351957E-01
0.316718E+02	0.898698E-04	0.178841E-01	0.355497E+01	0.110443E+00	0.351635E-01
0.319901E+02	0.880608E-04	0.177002E-01	0.355291E+01	0.110384E+00	0.351319E-01
0.323084E+02	0.862985E-04	0.175206E-01	0.355083E+01	0.110324E+00	0.351009E-01
0.326268E+02	0.845885E-04	0.173406E-01	0.354873E+01	0.110263E+00	0.350704E-01
0.329451E+02	0.829334E-04	0.171668E-01	0.354661E+01	0.110201E+00	0.350404E-01
0.332634E+02	0.813233E-04	0.169966E-01	0.354448E+01	0.110138E+00	0.350109E-01

FOR INFORMATION ONLY

FREQUENCY	PSEUDO DISPLACEMENT	PSEUDO VELOCITY	TRUE ACCELERATION	TRUE G	PERIOD
0.130507E+01	0.144709E+00	0.1119661E+01	0.973024E+01	0.302181E+00	0.766242E+00
0.135282E+01	0.133839E+00	0.113763E+01	0.966488E+01	0.300307E+00	0.739198E+00
0.140056E+01	0.121879E+00	0.1106725E+01	0.939384E+01	0.291672E+00	0.713194E+00
0.144831E+01	0.120857E+00	0.109780E+01	0.100082E+02	0.310813E+00	0.690460E+00
0.149606E+01	0.120373E+00	0.113150E+01	0.106361E+02	0.330315E+00	0.668424E+00
0.154380E+01	0.125724E+00	0.121452E+01	0.118244E+02	0.367371E+00	0.647751E+00
0.159155E+01	0.117419E+00	0.117419E+01	0.117419E+02	0.364656E+00	0.628319E+00
0.163930E+01	0.111213E+00	0.111549E+01	0.117986E+02	0.366416E+00	0.610018E+00
0.168704E+01	0.104713E+00	0.111995E+01	0.117655E+02	0.365389E+00	0.592753E+00
0.173479E+01	0.992377E-01	0.109169E+01	0.117904E+02	0.366163E+00	0.576439E+00
0.178253E+01	0.104958E+00	0.112598E+01	0.131710E+02	0.409037E+00	0.560999E+00
0.183028E+01	0.107715E+00	0.123872E+01	0.142453E+02	0.442400E+00	0.546364E+00
0.187803E+01	0.113154E+00	0.133521E+01	0.157555E+02	0.489302E+00	0.532473E+00
0.192577E+01	0.108607E+00	0.136414E+01	0.159011E+02	0.493824E+00	0.519272E+00
0.197352E+01	0.103081E+00	0.127820E+01	0.158497E+02	0.492228E+00	0.506709E+00
0.202127E+01	0.967493E-01	0.122872E+01	0.156079E+02	0.484618E+00	0.494731E+00
0.206901E+01	0.868752E-01	0.112938E+01	0.146619E+02	0.455960E+00	0.483322E+00
0.211676E+01	0.740129E-01	0.984372E+00	0.130422E+02	0.406589E+00	0.472420E+00
0.216451E+01	0.683780E-01	0.929941E+00	0.126472E+02	0.392770E+00	0.461499E+00
0.221225E+01	0.647785E-01	0.973199E+00	0.134858E+02	0.418813E+00	0.452028E+00
0.226000E+01	0.600636E-01	0.113610E+01	0.161443E+02	0.501368E+00	0.442478E+00
0.230775E+01	0.766550E-01	0.111150E+01	0.161167E+02	0.500519E+00	0.433323E+00
0.235549E+01	0.706664E-01	0.109586E+01	0.154788E+02	0.480707E+00	0.424504E+00
0.240324E+01	0.647633E-01	0.105343E+01	0.159067E+02	0.493998E+00	0.416105E+00
0.245098E+01	0.680391E-01	0.104780E+01	0.161362E+02	0.501123E+00	0.407999E+00
0.249873E+01	0.639824E-01	0.109452E+01	0.157710E+02	0.484784E+00	0.400203E+00
0.254648E+01	0.606260E-01	0.977016E+00	0.155203E+02	0.481996E+00	0.392694E+00
0.259422E+01	0.574757E-01	0.936653E+00	0.152707E+02	0.474295E+00	0.385472E+00
0.264197E+01	0.570400E-01	0.947693E+00	0.157317E+02	0.488562E+00	0.378505E+00
0.268972E+01	0.581844E-01	0.983316E+00	0.166180E+02	0.516088E+00	0.371786E+00
0.273746E+01	0.555212E-01	0.921565E+00	0.158337E+02	0.491730E+00	0.365301E+00
0.278521E+01	0.551204E-01	0.969605E+00	0.168806E+02	0.524241E+00	0.359039E+00
0.283296E+01	0.540122E-01	0.962110E+00	0.171255E+02	0.531849E+00	0.352468E+00
0.288070E+01	0.488112E-01	0.883483E+00	0.159910E+02	0.496166E+00	0.347137E+00
0.292845E+01	0.435279E-01	0.803913E+00	0.147368E+02	0.457664E+00	0.341478E+00
0.297620E+01	0.392213E-01	0.733438E+00	0.137353E+02	0.425941E+00	0.335999E+00
0.302394E+01	0.379088E-01	0.723266E+00	0.136853E+02	0.425002E+00	0.330694E+00
0.307169E+01	0.380086E-01	0.733566E+00	0.141578E+02	0.439684E+00	0.325554E+00
0.311944E+01	0.388421E-01	0.7661303E+00	0.149215E+02	0.463402E+00	0.320571E+00
0.316718E+01	0.395906E-01	0.787852E+00	0.156783E+02	0.486402E+00	0.315738E+00
0.321493E+01	0.386456E-01	0.773912E+00	0.154783E+02	0.480691E+00	0.311594E+00
0.326267E+01	0.341220E-01	0.717192E+00	0.150610E+02	0.467734E+00	0.299199E+00
0.331041E+01	0.294898E-01	0.659775E+00	0.145351E+02	0.450778E+00	0.285599E+00
0.335816E+01	0.290926E-01	0.666129E+00	0.153909E+02	0.477949E+00	0.273182E+00
0.340590E+01	0.212268E-01	0.501443E+00	0.122266E+02	0.374779E+00	0.261799E+00
0.345364E+01	0.206405E-01	0.516011E+00	0.129103E+02	0.400630E+00	0.251327E+00
0.413803E+01	0.230820E-01	0.599613E+00	0.155899E+02	0.484359E+00	0.241661E+00

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PERIOD

TRUE
G

TRUE
ACCELERATION

PSEUDO
VELOCITY

PSEUDO
DISPLACEMENT

FREQUENCY

FREQUENCY	PSEUDO DISPLACEMENT	PSEUDO VELOCITY	TRUE ACCELERATION	TRUE G	PERIOD
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000
000000	000000	000000	000000	000000	000000

FOR INFORMATION ONLY

FREQUENCY	PSEUDO DISPLACEMENT	PSEUDO VELOCITY	TRUE ACCELERATION	TRUE G	PERIOD
0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
0.0001	0.0000	0.0000	0.0000	0.0000	0.0000
0.0002	0.0000	0.0000	0.0000	0.0000	0.0000
0.0003	0.0000	0.0000	0.0000	0.0000	0.0000
0.0004	0.0000	0.0000	0.0000	0.0000	0.0000
0.0005	0.0000	0.0000	0.0000	0.0000	0.0000
0.0006	0.0000	0.0000	0.0000	0.0000	0.0000
0.0007	0.0000	0.0000	0.0000	0.0000	0.0000
0.0008	0.0000	0.0000	0.0000	0.0000	0.0000
0.0009	0.0000	0.0000	0.0000	0.0000	0.0000
0.0010	0.0000	0.0000	0.0000	0.0000	0.0000
0.0011	0.0000	0.0000	0.0000	0.0000	0.0000
0.0012	0.0000	0.0000	0.0000	0.0000	0.0000
0.0013	0.0000	0.0000	0.0000	0.0000	0.0000
0.0014	0.0000	0.0000	0.0000	0.0000	0.0000
0.0015	0.0000	0.0000	0.0000	0.0000	0.0000
0.0016	0.0000	0.0000	0.0000	0.0000	0.0000
0.0017	0.0000	0.0000	0.0000	0.0000	0.0000
0.0018	0.0000	0.0000	0.0000	0.0000	0.0000
0.0019	0.0000	0.0000	0.0000	0.0000	0.0000
0.0020	0.0000	0.0000	0.0000	0.0000	0.0000
0.0021	0.0000	0.0000	0.0000	0.0000	0.0000
0.0022	0.0000	0.0000	0.0000	0.0000	0.0000
0.0023	0.0000	0.0000	0.0000	0.0000	0.0000
0.0024	0.0000	0.0000	0.0000	0.0000	0.0000
0.0025	0.0000	0.0000	0.0000	0.0000	0.0000
0.0026	0.0000	0.0000	0.0000	0.0000	0.0000
0.0027	0.0000	0.0000	0.0000	0.0000	0.0000
0.0028	0.0000	0.0000	0.0000	0.0000	0.0000
0.0029	0.0000	0.0000	0.0000	0.0000	0.0000
0.0030	0.0000	0.0000	0.0000	0.0000	0.0000

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JES2 JOB LOG -- SYSTEM 3034 -- NODE CHURCH

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7.09.10 JOB 1974 IEF6771 WARNING MESSAGE(S) FOR JOB COMPARE ISSUED
7.09.10 JOB 1974 $HASP373 COMPARE STARTED - INIT 15 - CLASS L - SYS 3034
7.09.10 JOB 1974 IEF4031 COMPARE - STARTED - TIME=17.09.10
7.09.16 JOB 1974 -
7.09.16 JOB 1974 --JOBNAME  STEPNAME  PROGRAM  RC  EXCP  CPU  SRB  CLOCK  SERV  PG  PAGE  SWAP  VIO  SWAPS  (APPROX.)
7.09.16 JOB 1974 -COMPARE  FURT      IGFURT      00  348  .02  .00  .1  5076  1  0  0  7  0  $  0.17
7.09.16 JOB 1974 -COMPARE  LAED      IEWL       00  214  .00  .00  .3  2587  1  0  72  68  5  $  0.10
7.12.07 JOB 1974 -COMPARE  GI       PGM=*.DD   00  57   .08  .00  2.4  24245  1  22  176  24  5  $  0.38
7.12.24 JOB 1974 IEF4041 COMPARE - ENDED - TIME=17.12.24
7.12.24 JOB 1974 -COMPARE  ENDED  NAME=2AFLLYJK
7.12.24 JOB 1974 $HASP395 COMPARE  ENDED
TOTAL CPU TIME= .11 TOTAL ELAPSED TIME= 3.2 JOB COST=$ 0.65
    
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----- JES2 JOB STATISTICS -----

23 AUG 82 JOB EXECUTION DATE

375 CARDS READ

12,374 SYSOUT PRINT RECORDS

0 SYSOUT PUNCH RECORDS

3.23 MINUTES EXECUTION TIME

THESE DESIGN CALCULATIONS COVER ITEMS RELATING TO NUCLEAR SAFETY. IN ACCORDANCE WITH ESTABLISHED PROCEDURES, THE QUALITY HAS BEEN ASSURED.

Jeffrey M. Kelly 8/24/82
 ORIGINATED BY _____ DATE _____
Doyle E. Boyd 9/1/82
 CHECKED BY _____ DATE _____

APPROVED BY _____ DATE _____

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0037      18 CONTINUE                                00000540
0038      19 CONTINUE                                00000550
0039      20 FORMAT(6F13.4)                          00000560
0040      21 FORMAT(3X,6F13.4)                       00000570
0041      22 FORMAT(/)                                00000580
0042      99 DO 1 J=1,4                               00000590
0043      WRITE(6,40)                                  00000600
0044      IF(J.EQ.1)DAMP=0.005                        00000610
0045      IF(J.EQ.2)DAMP=0.010                        00000620
0046      IF(J.EQ.3)DAMP=0.020                        00000630
0047      IF(J.EQ.4)DAMP=0.050                        00000640
0048      WRITE(6,51)JOINT,ELEV,DAMP                  00000650
0049      51 FORMAT(/,27X,'RESPONSE ACCELERATION DATA FOR JOINT ',I2,/,27X,
1'ELEVATION ',F6.2,/,27X,'OSCILLATING DAMPING VALUE OF ',F5.3) 00000660
0050      WRITE(6,82)                                  00000670
0050      READS AND PRINTS GENERATED RESPONSE SPECTUM DATA FOR SPECIFIED JOINTS 00000680
0051      DO 13 I=1,64                                00000690
0052      JR=J+I                                       00000700
0053      READ(JR,20)A2(J,I),AP2(J,I),B2(J,I),BP2(J,I),C2(J,I),CP2(J,I) 00000710
0054      IF(I.EQ.1)WRITE(6,22)                         00000720
0055      WRITE(6,21)A2(J,I),AP2(J,I),B2(J,I),BP2(J,I),C2(J,I),CP2(J,I) 00000730
0056      13 CONTINUE                                  00000740
0057      12 CONTINUE                                  00000750
0057      THIS SECTION CALCULATES THE ACCELERATION SHOWN IN CATAWBA FSAR 00000760
0057      FIGURE 2.5.2-0 WHICH CORRESPONDS TO A GIVEN PERIOD (P) OF ANWAY AAP1 00000770
0057      THE PERIOD RANGE IS .05 TO 3.0 SECONDS. THE DATA READ IN ARE THE END 00000780
0057      POINTS OF EACH LINEAR LINE SHOWN IN FSAR FIGURE 2.5.2-0. THE 00000790
0057      EQUATION USED TO CALCULATE THE ACCELERATION GIVEN THE PERIOD (P) IS : 00000800
0057      LOG(A) = M * LOG(P) + B                      00000810
0057      GIVEN THE TWO END POINTS OF THE LINEAR LINE, M AND B ARE KNOWN. THE 00000820
0057      PERIOD (P) IS GIVEN. THUS, ACCELERATION (A) EQUALS 10 RAISED TO THE 00000830
0057      POWER OF (M*LOG(P)+B).                        00000840
0057      A = 10 ** (M * LOG(P) + B)                  00000850
0058      DO 14 J=1,4                                  00000860
0059      IF(J.EQ.1)DAMP=0.005                        00000870
0060      IF(J.EQ.2)DAMP=0.010                        00000880
0061      IF(J.EQ.3)DAMP=0.020                        00000890
0062      IF(J.EQ.4)DAMP=0.050                        00000900
0063      READ(5,500)SAMP1,SARA1,SAMP2,SARA2,SAMP3,SARA3,SAMP4,SARA4 00000910
0064      500 FORMAT(8F5.4)                            00000920
0065      DO 15 I=10,48                                00000930
0065      15 CONTINUE                                  00000940
0065      14 CONTINUE                                  00000950
0065      13 CONTINUE                                  00000960
0065      12 CONTINUE                                  00000970
0065      11 CONTINUE                                  00000980
0065      10 CONTINUE                                  00000990
0065      9 CONTINUE                                   00010000
0065      8 CONTINUE                                   00010010

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0066      X=SARA1/SARA2      00001020
0067      Y=SARP1/SARP2      00001030
0068      XX=ALOG10(X)      00001040
0069      YY=ALOG10(Y)      00001050
0070      XY=XX/YY      00001060
0071      CONST=ALOG10(SARA1)-(XY*ALOG10(SARP1))      00001070
0072      Z=XY*ALOG10(AAP1(J,I))+CONST      00001080
0073      A3(J,I)=10.**Z      00001090
0074      15 CONTINUE      00001100
0075      DO 16 I=99,93      00001110
0076      X=SARA3/SARA1      00001120
0077      Y=SARP3/SARP1      00001130
0078      XX=ALOG10(X)      00001140
0079      YY=ALOG10(Y)      00001150
0080      XY=XX/YY      00001160
0081      CONST=ALOG10(SARA3)-(XY*ALOG10(SARP3))      00001170
0082      Z=XY*ALOG10(AAP1(J,I))+CONST      00001180
0083      A3(J,I)=10.**Z      00001190
0084      16 CONTINUE      00001200
0085      DO 17 I=94,150      00001210
0086      X=SARA4/SARA3      00001220
0087      Y=SARP4/SARP3      00001230
0088      XX=ALOG10(X)      00001240
0089      YY=ALOG10(Y)      00001250
0090      XY=XX/YY      00001260
0091      CONST=ALOG10(SARA4)-(XY*ALOG10(SARP4))      00001270
0092      Z=XY*ALOG10(AAP1(J,I))+CONST      00001280
0093      A3(J,I)=10.**Z      00001290
0094      17 CONTINUE      00001300
0095      WRITE(6,40)      00001310
0096      52 FORMAT(//,27X,'CATAWBA FSAR RESPONSE ACCELERATION DATA FOR JOINT',      00001320
0097      1',12,/,27X,'ELEVATION ',F6.2,/,27X,'OSCILLATING DAMPING VALUE OF',      00001330
0098      1',F5.3)      00001350
0098      WRITE(6,83)      00001360
0099      83 FORMAT(//,10X,'CATAWBA',19X,'CATAWHA',19X,'CATAWBA',/,11X,      00001370
0099      1'FSAR',22X,'FSAR',22X,'FSAR',/,      00001380
0099      19X,'RESPONSE',14X,'RESPONSE',14X,'RESPONSE',/,7X,      00001390
0099      1'ACCELERATION',4X,'PERIOD',4X,'ACCELERATION',4X,'PERIOD',      00001400
0099      14X,'ACCELERATION',4X,'PERIOD')      00001410
0100      II=10      00001420
0100      PRINTS THE CALCULATED CATAWBA FSAR RESPONSE ACCELERATION      00001430
0101      DO 19 I=1,47      00001450
0102      IF(I.EQ.1)WRITE(6,22)      00001460
0103      III=III+1      00001470
0104      II=II+1      00001480
0104      III=III+1      00001490

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0105      WRITE(6,21)A3(J,II),AAP1(J,II),A3(J,III),AAP1(J,III),A3(J,IIII),
          JAAP1(J,IIII)
0106      II=II+3
0107      19 CONTINUE
0108      14 CONTINUE
C
C      THIS SECTION DETERMINES THE ACCELERATION AMPLIFICATION BETWEEN THE
C      GENERATED GROUND RESPONSE ACCELERATION AND THE GENERATED RESPONSE
C      ACCELERATIONS OF SPECIFIED JOINTS. THE PERCENTAGE INCREASE IS
C      ALSO CALCULATED.
C
0109      DO 30 J=1,4
0110      AMAX=0.0
0111      PERMAX=0.0
0112      WRITE(6,40)
0113      IF(J.EQ.1)DAMP=0.005
0114      IF(J.EQ.2)DAMP=0.010
0115      IF(J.EQ.3)DAMP=0.020
0116      IF(J.EQ.4)DAMP=0.050
0117      WRITE(6,51)JOINT,FLEV,DAMP
0118      DO 31 I=1,64
0119      ADELTA=A2(J,I)-A1(J,I)
0120      BDELTA=B2(J,I)-B1(J,I)
0121      CDELTA=C2(J,I)-C1(J,I)
0122      ADELTA(J,I)=A2(J,I)-A1(J,I)
0123      HDELTA(J,I)=H2(J,I)-H1(J,I)
0124      CDELTA(J,I)=C2(J,I)-C1(J,I)
0125      APER(J,I)=(ADELTA(J,I)/A1(J,I))*100.
0126      BPER(J,I)=(HDELTA(J,I)/B1(J,I))*100.
0127      CPER(J,I)=(CDELTA(J,I)/C1(J,I))*100.
0128      IF(I.EQ.1)WRITE(6,60)
0129      60 FORMAT(//,4X,'GENERATED',/10X,14X,'GENERATED',9X,'GROUND',/10X,
115X,'RESPONSE',9X,'RESPONSE',6X,'ACCELERATION',9X,'PERCENT',/10X,
110X,'PERIOD',7X,'ACCELERATION',4X,'ACCELERATION',4X,'AMPLIFICATION',
11X,'INCREASE',//)
0130      WRITE(6,71)A1(J,I),A2(J,I),A1(J,I),ADELTA(J,I),APER(J,I)
0131      WRITE(6,71)B1(J,I),B2(J,I),B1(J,I),HDELTA(J,I),BPER(J,I)
0132      WRITE(6,71)C1(J,I),C2(J,I),C1(J,I),CDELTA(J,I),CPER(J,I)
0133      71 FORMAT(10X,4(F6.4,10X),F10.4)
0134      IF(ADELTA.GT.AMAX)IAD=1
0135      IF(ADELTA.GT.AMAX)AMAX=ADELTA
0136      IF(BDELTA.GT.AMAX)IAD=1
0137      IF(BDELTA.GT.AMAX)IAD=1
0138      IF(BDELTA.GT.AMAX)IAD=1
0139      IF(BDELTA.GT.AMAX)IAD=1
0140      IF(CDELTA.GT.AMAX)IAD=1
0141      IF(CDELTA.GT.AMAX)IAD=1
0142      IF(CDELTA.GT.AMAX)IAD=1
          IF(CDELTA.GT.AMAX)AMAX=CDELTA

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00001500
00001510
00001520
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00001955

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0143 IF (APER(J,I).GT.PERMAX) IPID=1 00001970
0144 IF (APER(J,I).GT.PERMAX) PERMAX=APER(J,I) 00001971
0145 IF (APER(J,I).GT.PERMAX) PERMAX=APER(J,I) 00001990
0146 IF (APER(J,I).GT.PERMAX) IPID=1 00001990
0147 IF (APER(J,I).GT.PERMAX) NUM=2 00002000
0148 IF (APER(J,I).GT.PERMAX) PERMAX=APER(J,I) 00002001
0149 IF (CPER(J,I).GT.PERMAX) IPID=1 00002010
0150 IF (CPER(J,I).GT.PERMAX) NUM=3 00002020
0151 IF (CPER(J,I).GT.PERMAX) PERMAX=CPER(J,I) 00002021
0152 31 CONTINUE 00002030
0153 WRITE(6,40) 00002040
0154 IF (J.EQ.1) DAMP=0.005 00002050
0155 IF (J.EQ.2) DAMP=0.010 00002060
0156 IF (J.EQ.3) DAMP=0.020 00002070
0157 IF (J.EQ.4) DAMP=0.050 00002080
0158 WRITE(6,51) JOINT,ELEV,DAMP 00002090
0159 WRITE(6,72) 00002100
0160 72 FORMAT(//,10X,'THE FOLLOWING IS A LIST OF THOSE RESPONSE ACCELERATION GREATER THAN 5X',7,10X,'OF THE GROUND RESPONSE ACCELERATION',1) 00002110
0161 WRITE(6,60) 00002130
0162 DU 52 I=1,64 00002140
0163 IF (APER(J,I).GT.5.0) WRITE(6,71) A1(J,I),A2(J,I),A1(J,I), 00002150
0164 1ADELT(J,I),APER(J,I) 00002160
0164 IF (BPER(J,I).GT.5.0) WRITE(6,71) B1(J,I),B2(J,I),B1(J,I), 00002170
0164 1BDELT(J,I),BPER(J,I) 00002180
0165 IF (CPER(J,I).GT.5.0) WRITE(6,71) C1(J,I),C2(J,I),C1(J,I), 00002190
0165 1CDELT(J,I),CPER(J,I) 00002200
0166 32 CONTINUE 00002210
0167 WRITE(6,40) 00002220
0168 WRITE(6,51) JOINT,ELEV,DAMP 00002230
0169 WRITE(6,60) 00002240
0170 601 FORMAT(//,10X,'THE FOLLOWING IS A LIST OF THOSE RESPONSE ACCELERATION GREATER THAN 5X',7,10X,'2/3 OF THE CATAMBA FSAR GROUND RESPONSE ACCELERATION',1) 00002250
0171 WRITE(6,600) 00002260
0172 600 FORMAT(///,10X,14X,'GENERATED',8X,'ADJUSTED',9X,'CATAMBA',/,10X, 00002270
0172 115X,'RESPONSE',8X,'RESPONSE',10X,'FSAR',8X,'ACCELERATION',10X,'PERCENT',/,10X,'PERIOD',7X,'ACCELERATION',4X,'ACCELERATION',4X, 00002280
0172 1'ACCELERATION',4X,'AMPLIFICATION',9X,'INCREASE',//) 00002290
0173 II=9 00002300
00002310
00002320
00002330
00002340
00002350
00002360
00002370
00002380
00002390
00002400
00002410

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C
C
C
C
C
C

THIS SECTION ADJUSTS THE GENERATED RESPONSE ACCELERATIONS AT SPECIFIED JOINTS TO COMPARE TO TWO-THIRDS THE CATAMBA FSAR GROUND RESPONSE SPECTRUM. THE CATAMBA FSAR STATES THAT 2/3 OF THE HORIZONTAL GROUND RESPONSE SPECTRUM WILL BE USED AS THE VERTICAL RESPONSE SPECTRUM. SINCE THE RESPONSE SPECTRUM GENERATED USED 100% OF THE GROUND MOTION, A FACTOR OF 2/3 WILL BE APPLIED TO THE

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C      GENERATED RESPONSE SPECTRUM. IT IS SHOWN THAT THE GENERATED          00002420
C      GROUND RESPONSE SPECTRUM IS LARGER THAN THE CATANBA FSAR GROUND        00002421
C      RESPONSE SPECTRUM.  THUS, A REDUCTION FACTOR OF 1.                     00002422
C      (CATANBA FSAR GROUND RESPONSE / (2./3.)*(GENERATED GROUND RESPONSE)) 00002423
C      WILL ALSO BE APPLIED TO THE GENERATED RESPONSE SPECTRUM AT SPECIFIED 00002424
C      JOINTS.  THE ADJUSTED RESPONSE SPECTRUM AT SPECIFIED JOINTS IS :      00002425
C      ADJ RES SPEC = 2/3*(GEN_RES_SPEC)*(FSAR_GKND_RES / GEN_GRND_RES)      00002426
C      THE ACCELERATION AMPLIFICATION BETWEEN THE ADJUSTED RESPONSE SPECTRUM 00002427
C      AND FSAR RESPONSE SPECTRUM WILL BE DETERMINED.  THE PERCENTAGE        00002428
C      INCREASE WILL ALSO BE CALCULATED.                                     00002429
C      DD 510 I=4,50                                                         00002430
C      II=II+1                                                                00002431
C      Z=(2./3.)*A2(J,I)                                                      00002432
C      Y=A3(J,II)/((2./3.)*(41(J,I)))                                         00002433
C      ADJACC=Z*Y                                                              00002434
C      DDELT=ADJACC-A3(J,II)                                                  00002435
C      DPRCNT=(DDELT/A3(J,II))*100.                                           00002436
C      IF(DPRCNT.GT.5.0)WRITE(6,602)API(J,I),A2(J,I),ADJACC,A3(J,II),        00002437
C      DDELT,DPRCNT                                                           00002438
C      602 FORMAT(10X,5(F6.4,10X),F10.4)                                       00002439
C      II=II+1                                                                00002440
C      Z=(2./3.)*B2(J,I)                                                      00002441
C      Y=A3(J,II)/((2./3.)*(81(J,I)))                                         00002442
C      ADJACC=Z*Y                                                              00002443
C      DDELT=ADJACC-A3(J,II)                                                  00002444
C      DPRCNT=(DDELT/A3(J,II))*100.                                           00002445
C      IF(DPRCNT.GT.5.0)WRITE(6,602)BPI(J,I),B2(J,I),ADJACC,A3(J,II),        00002446
C      DDELT,DPRCNT                                                           00002447
C      II=II+1                                                                00002448
C      Z=(2./3.)*C2(J,I)                                                      00002449
C      Y=A3(J,II)/((2./3.)*(C1(J,I)))                                         00002450
C      ADJACC=Z*Y                                                              00002451
C      DDELT=ADJACC-A3(J,II)                                                  00002452
C      DPRCNT=(DDELT/A3(J,II))*100.                                           00002453
C      IF(DPRCNT.GT.5.0)WRITE(6,602)CPI(J,I),C2(J,I),ADJACC,A3(J,II),        00002454
C      DDELT,DPRCNT                                                           00002455
C      510 CONTINUE                                                           00002456
C      THIS SECTION DETERMINES THE MAXIMUM ACCELERATION AMPLIFICATION AND    00002457
C      MAXIMUM PERCENTAGE INCREASE BETWEEN THE GENERATED RESPONSE SPECTRUM  00002458
C      AT SPECIFIED JOINTS AND THE GENERATED GROUND RESPONSE SPECTRUM.      00002459
C      WRITE(6,40)                                                             00002460
C      40 FORMAT(10X,5(F6.4,10X),F10.4)                                       00002461
C      1
    
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0199      40 FORMAT(1H1)
0200      IF(J.EQ.1)DAMP=0.005
0201      IF(J.EQ.2)DAMP=0.010
0202      IF(J.EQ.3)DAMP=0.020
0203      IF(J.EQ.4)DAMP=0.050
0204      IF(NUMBER.EQ.1)PERIOD=AP1(J,IAD)
0205      IF(NUMBER.EQ.1)ACC=A2(J,IAD)
0206      IF(NUMBER.EQ.1)PERCENT=APER(J,IAD)
0207      IF(NUMBER.EQ.1)GRNDAC=A1(J,IAD)
0208      IF(NUMBER.EQ.2)PERIOD=BP1(J,IAD)
0209      IF(NUMBER.EQ.2)ACC=B2(J,IAD)
0210      IF(NUMBER.EQ.2)PERCENT=BPER(J,IAD)
0211      IF(NUMBER.EQ.2)GRNDAC=B1(J,IAD)
0212      IF(NUMBER.EQ.3)PERIOD=CP1(J,IAD)
0213      IF(NUMBER.EQ.3)ACC=C2(J,IAD)
0214      IF(NUMBER.EQ.3)PERCENT=CPER(J,IAD)
0215      IF(NUMBER.EQ.3)GRNDAC=C1(J,IAD)
0216      WRITE(6,41)JOINT,ELEV,DAMP,PERIOD,ACC,GRNDAC,AMAX,PERCENT
0217      41 FORMAT(////,10X,'JOINT',I2,' ELEVATION',F6.2,' OSCILLATION AMPLIFICATION BETWEEN THE',F5.3,'',//,10X,'THE MAXIMUM RESPONSE ACCELERATION SPECTRUM AND THE GENERATED RESPONSE SPECTRUM AT SPECIFIED JOINTS OCCURS AT PERIOD',F6.4,'',//,10X,'WHICH HAS A RESPONSE ACCELERATION OF',F6.4,'',//,10X,'THE GROUND RESPONSE ACCELERATION IS',F6.4,'',//,10X,'THE PERCENTAGE INCREASE IS',F6.3,'%')
0218      THIS SECTION DETERMINES THE MAXIMUM ACCELERATION AMPLIFICATION AND MAXIMUM PERCENTAGE INCREASE BETWEEN THE GENERATED RESPONSE SPECTRUM AT SPECIFIED JOINTS AND THE CATANHA FSRM RESPONSE SPECTRUM.
0219      IF(NUM.EQ.1)PERIOD=AP1(J,IPID)
0220      IF(NUM.EQ.1)GRNDAC=A1(J,IPID)
0221      IF(NUM.EQ.1)ACC=A2(J,IPID)
0222      IF(NUM.EQ.2)PERIOD=BP1(J,IPID)
0223      IF(NUM.EQ.2)GRNDAC=B1(J,IPID)
0224      IF(NUM.EQ.2)ACC=B2(J,IPID)
0225      IF(NUM.EQ.3)PERIOD=CP1(J,IPID)
0226      IF(NUM.EQ.3)GRNDAC=C1(J,IPID)
0227      IF(NUM.EQ.3)ACC=C2(J,IPID)
0228      WRITE(6,41)JOINT,ELEV,DAMP,PERIOD,ACC,GRNDAC,PERMAX
0229      41 FORMAT(////,10X,'JOINT',I2,' ELEVATION',F6.2,' OSCILLATION AMPLIFICATION BETWEEN THE',F5.3,'',//,10X,'THE MAXIMUM RESPONSE ACCELERATION SPECTRUM AND THE GENERATED RESPONSE SPECTRUM AT SPECIFIED JOINTS OCCURS AT PERIOD',F6.4,'',//,10X,'WHICH HAS A RESPONSE ACCELERATION OF',F6.4,'',//,10X,'THE GROUND RESPONSE ACCELERATION IS',F6.4,'',//,10X,'THE PERCENTAGE INCREASE IS',F6.3,'%')

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0229
0230
0231
0232

1ASE IS ('F6.3,'X.')

30 CONTINUE
9 CONTINUE
STOP
END

00003200
00003210
00003220
00003230
00003240

FOR INFORMATION ONLY

RESPONSE ACCELERATION DATA FOR JOINT 1

GROUND ELEVATION

OSCILLATING DAMPING VALUE OF 0.010

SPECTRUM "I" 1

RESPONSE ACCELERATION	PERIOD	RESPONSE ACCELERATION	PERIOD	RESPONSE ACCELERATION	PERIOD
0.0087	6.2832	0.0126	5.4636	0.0195	4.8332
0.0282	4.3332	0.0357	3.9270	0.0448	3.5904
0.0625	3.3069	0.0752	3.0650	0.0825	2.8500
0.0856	2.6737	0.0929	2.5133	0.0953	2.3710
0.0981	2.2440	0.1037	2.1299	0.1103	2.0268
0.1154	1.9333	0.1168	1.8480	0.1186	1.7699
0.1196	1.6982	0.1257	1.6320	0.1313	1.5708
0.1279	1.4612	0.1248	1.3659	0.1404	1.2823
0.1600	1.2083	0.1697	1.1424	0.1608	1.0833
0.1916	1.0300	0.2204	0.9817	0.2744	0.9378
0.2911	0.8976	0.3224	0.8607	0.3295	0.8267
0.3295	0.7953	0.3009	0.7662	0.2990	0.7392
0.5903	0.7140	0.3091	0.6705	0.3284	0.6684
0.3651	0.6476	0.3623	0.6283	0.3639	0.6100
0.3626	0.5928	0.3654	0.5764	0.4058	0.5610
0.4387	0.5464	0.4851	0.5325	0.4893	0.5193
0.4875	0.5067	0.4796	0.4947	0.4511	0.4833
0.4022	0.4724	0.3882	0.4620	0.4138	0.4520
0.4947	0.4425	0.4939	0.4333	0.4741	0.4245
0.4868	0.4161	0.4934	0.4080	0.4823	0.4005
0.4743	0.3927	0.4659	0.3856	0.4759	0.3785
0.5070	0.3718	0.4830	0.3653	0.5138	0.3590
0.5714	0.3530	0.4867	0.3471	0.4490	0.3415
0.4173	0.3360	0.4157	0.3307	0.4311	0.3256
0.4521	0.3206	0.4747	0.3157	0.4607	0.3142
0.4558	0.2992	0.4378	0.2956	0.4631	0.2732
0.3668	0.2618	0.3872	0.2513	0.4648	0.2417
0.4030	0.2327	0.4303	0.2244	0.4027	0.2167
0.5038	0.2094	0.4457	0.2027	0.4346	0.1933
0.4920	0.1904	0.4500	0.1848	0.4165	0.1795
0.4343	0.1745	0.4909	0.1698	0.4371	0.1653
0.3991	0.1611	0.4580	0.1571	0.4152	0.1532
0.4200	0.1496	0.4077	0.1461	0.4068	0.1428
0.4125	0.1396	0.4099	0.1366	0.4206	0.1337
0.3498	0.1309	0.3342	0.1282	0.3686	0.1257
0.3920	0.1232	0.3999	0.1208	0.3861	0.1186
0.3594	0.1164	0.3257	0.1142	0.3460	0.1122
0.3661	0.1102	0.3517	0.1083	0.2983	0.1065
0.2905	0.1047	0.2775	0.1030	0.2883	0.1013
0.2939	0.0997	0.2685	0.0982	0.2765	0.0967
0.2357	0.0938	0.2864	0.0911	0.2545	0.0885
0.2435	0.0861	0.2444	0.0838	0.2326	0.0816
0.2304	0.0795	0.2247	0.0776	0.2082	0.0757
0.1943	0.0739	0.1826	0.0722	0.1780	0.0706
0.1419	0.0690	0.1569	0.0676	0.1418	0.0661
0.1394	0.0648	0.1301	0.0635	0.1257	0.0622
0.1213	0.0610	0.1292	0.0594	0.1177	0.0587

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RESPONSE ACCELERATION DATA FOR JOINT 20

ELEVATION 710.45

OSCILLATING DAMPING VALUE OF 0.010

SPECTRUM "N"

RESPONSE ACCELERATION	PERIOD	RESPONSE ACCELERATION	PERIOD	RESPONSE ACCELERATION	PERIOD
0.0087	0.2832	0.0126	0.4836	0.0195	0.4833
0.0282	0.3332	0.0359	0.4976	0.0448	0.5004
0.0625	0.3064	0.0752	0.5050	0.0826	0.5060
0.0856	0.2737	0.0930	0.5133	0.0924	0.5100
0.0981	0.2440	0.1038	0.5209	0.1004	0.5128
0.1155	0.2333	0.1169	0.5280	0.1107	0.5199
0.1197	1.6482	0.1258	0.5320	0.1214	0.5208
0.1281	1.4612	0.1249	0.5354	0.1286	0.5223
0.1603	1.2683	0.1701	0.5424	0.1472	0.5233
0.1920	1.0300	0.2210	0.5487	0.2753	0.5278
0.2921	0.8407	0.3235	0.5507	0.3309	0.5267
0.3308	0.7953	0.3022	0.7662	0.3003	0.7392
0.2917	0.7140	0.3108	0.6905	0.3303	0.6684
0.3674	0.6478	0.3647	0.6283	0.3664	0.6100
0.3654	0.5928	0.3662	0.5764	0.4090	0.5610
0.4424	0.5464	0.4893	0.5325	0.4938	0.5193
0.4922	0.5067	0.4846	0.4947	0.4560	0.4833
0.4066	0.4724	0.3928	0.4620	0.4188	0.4520
0.5014	0.4425	0.5005	0.4333	0.4807	0.4245
0.4940	0.4161	0.5011	0.4080	0.4898	0.4002
0.4820	0.3927	0.4742	0.3855	0.4886	0.3785
0.5161	0.3718	0.4917	0.3653	0.5242	0.3590
0.5318	0.3530	0.4966	0.3471	0.4577	0.3415
0.4259	0.3360	0.4250	0.3307	0.4397	0.3256
0.4634	0.3206	0.4860	0.3157	0.4807	0.3142
0.4677	0.2992	0.4508	0.2856	0.4779	0.2933
0.3797	0.2618	0.4006	0.2513	0.4842	0.2417
0.4216	0.2327	0.4518	0.2244	0.4232	0.2167
0.5307	0.2094	0.4743	0.2027	0.4638	0.1963
0.5263	0.1904	0.4827	0.1848	0.4442	0.1795
0.4786	0.1745	0.5330	0.1698	0.4752	0.1653
0.4381	0.1611	0.5048	0.1571	0.4595	0.1532
0.4696	0.1496	0.4985	0.1461	0.4593	0.1478
0.4668	0.1396	0.4624	0.1366	0.4817	0.1337
0.4003	0.1309	0.3837	0.1282	0.4265	0.1257
0.4583	0.1232	0.4690	0.1208	0.4592	0.1186
0.4304	0.1164	0.3867	0.1142	0.4160	0.1122
0.4437	0.1102	0.4242	0.1083	0.3610	0.1065
0.3561	0.1047	0.3805	0.1030	0.3539	0.1013
0.3703	0.0997	0.3376	0.0982	0.3497	0.0967
0.2872	0.0938	0.3744	0.0911	0.3302	0.0885
0.3235	0.0861	0.3308	0.0838	0.3154	0.0816
0.3289	0.0795	0.3238	0.0776	0.3032	0.0757
0.2899	0.0739	0.2728	0.0722	0.2697	0.0706
0.2887	0.0680	0.2244	0.0676	0.2067	0.0661
0.1987	0.0628	0.1725	0.0635	0.1534	0.0622

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CATAWA FSAR RESPONSE ACCELERATION DATA FOR JOINT 20

ELEVATION 710.85

OSCILLATING DAMPING VALUE OF 0.010

FSAR VERTICAL SPECTRUM

2A

(2/3 X HORIZ GROUND
RESP. SPECTRUM)

CATAWA FSAR RESPONSE ACCELERATION	PERIOD	CATAWA FSAR RESPONSE ACCELERATION	PERIOD	CATAWA FSAR RESPONSE ACCELERATION	PERIOD
0.0405	2.5737	0.0432	2.5133	0.0458	2.3710
0.0485	2.2440	0.0512	2.1299	0.0538	2.0268
0.0565	1.9333	0.0592	1.8480	0.0619	1.7699
0.0646	1.6982	0.0673	1.6320	0.0700	1.5708
0.0753	1.4612	0.0753	1.3659	0.0782	1.2623
0.0916	1.2083	0.0970	1.1424	0.1024	1.0833
0.1079	1.0300	0.1133	0.9817	0.1158	0.9378
0.1243	0.8976	0.1297	0.8607	0.1352	0.8267
0.1407	0.7953	0.1462	0.7662	0.1517	0.7392
0.1572	0.7140	0.1627	0.6905	0.1642	0.6684
0.1737	0.6478	0.1792	0.6283	0.1844	0.6100
0.1903	0.5928	0.1958	0.5764	0.2013	0.5640
0.2069	0.5464	0.2124	0.5325	0.2160	0.5193
0.2266	0.5067	0.2266	0.4947	0.2266	0.4833
0.2266	0.4724	0.2266	0.4620	0.2266	0.4520
0.2266	0.4425	0.2266	0.4333	0.2266	0.4245
0.2266	0.4161	0.2266	0.4080	0.2266	0.4022
0.2266	0.3927	0.2266	0.3855	0.2266	0.3785
0.2266	0.3716	0.2266	0.3653	0.2266	0.3590
0.2266	0.3530	0.2266	0.3471	0.2266	0.3415
0.2266	0.3360	0.2266	0.3307	0.2266	0.3265
0.2266	0.3206	0.2266	0.3157	0.2266	0.3142
0.2266	0.2992	0.2266	0.2952	0.2266	0.2732
0.2266	0.2618	0.2266	0.2613	0.2266	0.2417
0.2266	0.2327	0.2266	0.2244	0.2266	0.2167
0.2266	0.2094	0.2266	0.2027	0.2266	0.1963
0.2266	0.1904	0.2266	0.1848	0.2266	0.1745
0.2266	0.1745	0.2266	0.1698	0.2266	0.1653
0.2213	0.1611	0.2170	0.1571	0.2127	0.1532
0.2088	0.1496	0.2049	0.1461	0.2012	0.1428
0.1977	0.1396	0.1943	0.1366	0.1911	0.1337
0.1879	0.1309	0.1848	0.1282	0.1820	0.1257
0.1791	0.1232	0.1769	0.1208	0.1738	0.1186
0.1713	0.1164	0.1687	0.1142	0.1664	0.1122
0.1641	0.1102	0.1618	0.1083	0.1597	0.1065
0.1576	0.1047	0.1556	0.1030	0.1535	0.1013
0.1516	0.0997	0.1499	0.0982	0.1480	0.0967
0.1445	0.0934	0.1412	0.0911	0.1360	0.0885
0.1351	0.0861	0.1322	0.0848	0.1295	0.0816
0.1268	0.0795	0.1248	0.0776	0.1220	0.0757
0.1197	0.0734	0.1178	0.0722	0.1155	0.0706
0.1134	0.0690	0.1116	0.0676	0.1097	0.0661
0.1060	0.0648	0.1062	0.0635	0.1045	0.0622
0.1029	0.0610	0.1013	0.0598	0.0999	0.0587
0.0924	0.0576	0.0970	0.0566	0.0947	0.0556

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RESPONSE ACCELERATION DATA FOR JOINT 20
ELEVATION 710.45

OSCILLATING DAMPING VALUE OF 0.010

PERIOD	SPECT. "N" GENERATED RESPONSE ACCELERATION	SPECT. "I" GENERATED GROUND RESPONSE ACCELERATION	"N"-"I" ACCELERATION AMPLIFICATION	(H-1)*100 PERCENT INCREASE
0.2832	0.0087	0.0087	0.0	0.0
0.4636	0.0126	0.0126	0.0	0.0
0.8332	0.0195	0.0195	0.0	0.0
0.3332	0.0282	0.0282	0.0	0.0
0.9270	0.0357	0.0357	0.0	0.0
0.5904	0.0448	0.0448	0.0	0.0
0.3069	0.0625	0.0625	0.0	0.0
0.0650	0.0752	0.0752	0.0	0.0
0.8560	0.0826	0.0825	0.0001	0.1212
0.6737	0.0856	0.0856	0.0	0.0
0.5133	0.0930	0.0929	0.0001	0.1077
0.3710	0.0954	0.0953	0.0001	0.1049
0.2440	0.0981	0.0981	0.0	0.0
0.1299	0.1038	0.1037	0.0001	0.0964
0.0268	0.1104	0.1103	0.0001	0.0907
1.0533	0.1155	0.1154	0.0001	0.0866
1.8480	0.1169	0.1168	0.0001	0.0856
1.7699	0.1187	0.1186	0.0001	0.0843
1.6982	0.1197	0.1196	0.0001	0.0836
1.6320	0.1258	0.1257	0.0001	0.0796
1.5708	0.1314	0.1313	0.0001	0.0762
1.4612	0.1281	0.1279	0.0002	0.1564
1.3659	0.1249	0.1248	0.0001	0.0801
1.2823	0.1406	0.1404	0.0002	0.1425
1.2083	0.1603	0.1600	0.0003	0.1875
1.1424	0.1701	0.1697	0.0004	0.2357
1.0833	0.1872	0.1868	0.0004	0.2141
1.0300	0.1920	0.1918	0.0004	0.2087
0.9817	0.2210	0.2204	0.0006	0.2723
0.9374	0.2753	0.2744	0.0009	0.3280
0.8976	0.2921	0.2911	0.0010	0.3435
0.8607	0.3235	0.3224	0.0011	0.3412
0.8267	0.3309	0.3295	0.0014	0.4249
0.7953	0.3304	0.3295	0.0013	0.3945
0.7662	0.3022	0.3009	0.0013	0.4320
0.7392	0.3003	0.2990	0.0013	0.4348
0.7140	0.2917	0.2903	0.0014	0.4823
0.6905	0.3108	0.3091	0.0017	0.5500
0.6684	0.3308	0.3284	0.0019	0.5785
0.6478	0.3673	0.3651	0.0023	0.6300
0.6283	0.3647	0.3623	0.0024	0.6624
0.6100	0.3664	0.3639	0.0025	0.6870
0.5928	0.3654	0.3626	0.0028	0.7722
0.5764	0.3662	0.3634	0.0028	0.7765
0.5610	0.4000	0.4058	0.0052	0.7846

FOR INFORMATION ONLY

0.0351	0.1179	0.1110	0.0069	6.2162
0.0347	0.1178	0.1110	0.0068	6.1261
0.0343	0.1177	0.1109	0.0068	6.1316
0.0340	0.1176	0.1108	0.0067	6.1373
0.0336	0.1175	0.1108	0.0067	6.0469
0.0332	0.1174	0.1107	0.0067	6.0524
0.0329	0.1173	0.1107	0.0066	6.0620
0.0326	0.1173	0.1106	0.0067	6.0578
0.0322	0.1172	0.1106	0.0066	6.0675
0.0319	0.1171	0.1105	0.0066	6.0729
0.0316	0.1170	0.1105	0.0065	6.0824
0.0313	0.1169	0.1104	0.0066	6.0782
0.0310	0.1169	0.1104	0.0065	6.0877
0.0306	0.1168	0.1104	0.0065	6.0877
0.0304	0.1168	0.1103	0.0065	6.0930
0.0301	0.1168	0.1103	0.0065	

FOR INFORMATION ONLY

RESPONSE ACCELERATION DATA FOR JOINT 20
 ELEVATION 710.85
 OSCILLATING DAMPING VALUE OF 0.010

FOR INFORMATION ONLY

THE FOLLOWING IS A LIST OF THOSE RESPONSE ACCELERATION GREATER THAN SIX
 2/3 OF THE CATANBA FSAH GROUND RESPONSE ACCELERATION :

PERIOD	SPECTRUM #N ^o GENERATED RESPONSE ACCELERATION	(AMPLIFIED SPECTRUM) ADJUSTED RESPONSE ACCELERATION	2A CATANBA FSAH ACCELERATION	AMPLIFICATION ACCELERATION AMPLIFICATION	(H-1)*100 PERCENT INCREASE
4kz	0.2167	0.4232	0.2381	0.2266	5.0906
	0.2094	0.5307	0.2387	0.2266	2.5350
	0.2027	0.4743	0.2411	0.2266	6.5169
	0.1963	0.4634	0.2414	0.2266	6.7188
	0.1904	0.5263	0.2424	0.2266	6.9715
	0.1844	0.4827	0.2431	0.2266	7.2266
	0.1795	0.4442	0.2452	0.2266	8.2266
	0.1745	0.4704	0.2455	0.2266	8.2266
	0.1698	0.5330	0.2460	0.2266	8.2266
	0.1653	0.4752	0.2464	0.2266	8.2266
6kz	0.1611	0.4381	0.2429	0.2213	9.7720
	0.1571	0.5044	0.2391	0.2170	10.2266
	0.1532	0.4595	0.2354	0.2127	10.2266
	0.1496	0.4696	0.2329	0.2084	11.3469
	0.1461	0.4985	0.2282	0.2049	12.9056
	0.1424	0.4593	0.2272	0.2012	13.1634
	0.1396	0.4668	0.2237	0.1977	15.2266
	0.1366	0.4624	0.2192	0.1943	16.2266
	0.1337	0.4817	0.2188	0.1911	18.2266
	0.1309	0.4903	0.2159	0.1874	18.2266
	0.1282	0.3837	0.2122	0.1848	18.2266
	0.1257	0.4265	0.2106	0.1820	19.7080
8kz	0.1232	0.4583	0.2099	0.1791	19.0132
	0.1206	0.4690	0.2099	0.1764	17.2793
	0.1186	0.4592	0.2068	0.1738	18.2266
	0.1164	0.4304	0.2051	0.1713	19.2266
	0.1142	0.3867	0.2003	0.1687	18.2266
9kz	0.1122	0.4160	0.2001	0.1664	20.2266
	0.1102	0.4437	0.1988	0.1641	21.1963
	0.1083	0.4242	0.1952	0.1618	20.2266
	0.1065	0.3610	0.1933	0.1597	21.2266
	0.1047	0.3561	0.1932	0.1576	22.5817
	0.1	0.3405	0.1909	0.1556	22.7027
	0.1044	0.3539	0.1885	0.1535	22.7541
	0.0982	0.3703	0.1910	0.1516	25.0050
	0.0969	0.3376	0.1884	0.1494	25.7355
	0.0949	0.3447	0.1872	0.1480	26.4738
11kz	0.0934	0.3472	0.1776	0.1445	27.1026
	0.0911	0.3744	0.1846	0.1412	30.7262
	0.0885	0.3502	0.1791	0.1380	29.7886
	0.0861	0.3235	0.1794	0.1351	32.8541
	0.0854	0.3544	0.1711	0.1322	30.0845

Attachment 2

Confirmatory Issue (8)

Vertical seismic response spectra (SER Section 3.9.2.2)

Response:

Attached is a copy of the stress report for the Refueling Water Storage Tanks.

SEISMIC - STRESS ANALYSIS

OF

ASME SECTION III, CLASS 2 TANKS

FOR

DUKE POWER COMPANY CATAWBA NUCLEAR STATION UNITS 1 and 2

MILL POWER P. O. NO. C - 39969

TANK

SPECIFICATION

DUKE DRAWING

Refueling Water Storage

CNS - 1148.00-03

CN - 1325 - 02

MANUFACTURED BY

RICHMOND ENGINEERING COMPANY, INC.

Analysis By

McDONALD ENGINEERING ANALYSIS COMPANY

BIRMINGHAM, ALABAMA

ME-311

CERTIFICATION STATEMENT

This tank and foundation has been analyzed in accordance with Duke Power Company Specification CNS-1148.00-03, the ASME Boiler and Pressure Vessel Code, Section III, Class 2, ACI-318-71 and accepted good practice in seismic stress analysis.

The tank and foundation meets all requirements of the specification and is structurally adequate to withstand Normal, Upset, Faulted, and Tornado Loading conditions.

Prepared by: J. H. Appleton
 J. H. Appleton (Foundation, App. E) P.E.

5/10/76
 Date

CK McDonald
 C. K. McDonald (Tank), P.E.

1/14/77 R1
 Date

Checked by: T. K. Hicks
 T. K. Hicks, P.E.

1/14/77 R1
 Date

Approved by: CK McDonald
 C. K. McDonald
 McDonald Engineering Analysis Co.

1/14/77 R1
 Date

Approved by: Grayson A. Harding
 Grayson A. Harding, P. E.
 Richmond Engineering Company

6-1-77 R1
 Date

2. SUMMARY OF RESULTS

2.1 Normal

<u>Component</u>	<u>Actual</u>	<u>Allowable</u>
Anchor Bolts Stress, PSI - Tensile	276	40,000
- Shear	376	12,320
Concrete Compressive Stress, PSI	342	1,250
Support Ring Stress, PSI	16,104	27,900
Anchor Chair Stress, PSI		
Top Plate	192	21,750
Shell	111	27,900
Vertical Plates	176	21,750
Bearing in Bottom Plate	472	27,000
Roof Pressure, PSIG	.21	.443
Shell Compressive Stress, PSI		
Ring #1	322	4,688
Ring #2	243	4,219
Ring #3	226	3,281
Ring #4	223	2,344
Ring #5	196	1,875
Shell Tensile Stress, PSI	7,085	18,600
Nozzle Stresses, PSI		
Mark A (4" Nozzle)	19,564	55,800
Mark B (24" Nozzle)	27,694	55,800
Mark C (6" Nozzle)	20,872	55,800
Mark D (4" Nozzle)	19,564	55,800
Mark E (3" Nozzle)	13,042	55,800
Mark G (8" Nozzle)	27,685	55,800
Mark H and J (1" Nozzle)	8,575	55,800

2.2 Upset

<u>Component</u>	<u>Actual</u>	<u>Allowable</u>
Anchor Bolts Stress, PSI - Tensile	15,718	40,000
- Shear	5,736	12,320
Concrete Compressive Stress, PSI	342	1,250
Support Ring Stress, PSI	16,104	27,900
Anchor Chair Stress, PSI		
Top Plate	10,933	21,750
Shell	6,321	27,900
Vertical Plates	10,004	21,750
Bearing in Bottom Plate	7,208	27,000
Roof Pressure, PSIG	.23	.443
Shell Compressive Stress, PSI		
Ring #1	2,818	4,688
Ring #2	2,186	4,219
Ring #3	1,786	3,281
Ring #4	1,319	2,344
Ring #5	617	1,875
Shell Tensile Stress, PSI	10,856	18,600
Nozzle Stresses, PSI		
Mark A	44,124	55,800
Mark B	52,075	55,800
Mark C	41,744	55,800
Mark D	44,124	55,800
Mark E	31,077	55,800
Mark G	52,041	55,800
Mark H and J	13,842	55,800

2.3 Faulted

<u>Component</u>	<u>Actual</u>	<u>Allowable</u>
Anchor Bolts Stress, PSI - Tensile	31,008	38,749
- Shear	11,068	14,784
Concrete Compressive Stress, PSI	642	1,666
Support Ring Stress, PSI	30,177	33,480
Anchor Chair Stress, PSI		
Top Plate	21,568	26,100
Shell	12,469	33,480
Vertical Plates	19,736	26,100
Bearing in Bottom Plate	13,908	32,400
Roof Pressure,	.25	.53
Shell Compressive Stress, PSI		
Ring #1	5,290	5,625
Ring #2	4,119	5,063
Ring #3	3,335	3,937
Ring #4	2,401	2,813
Ring #5	1,023	2,250
Shell Tensile Stress, PSI	14,630	22,320
Nozzle Stresses, PSI		
Mark A	58,902	66,960
Mark B	66,154	66,960
Mark C	52,180	66,960
Mark D	58,902	66,960
Mark E	42,594	66,960
Mark G	66,134	66,960
Mark H and J	18,361	66,960

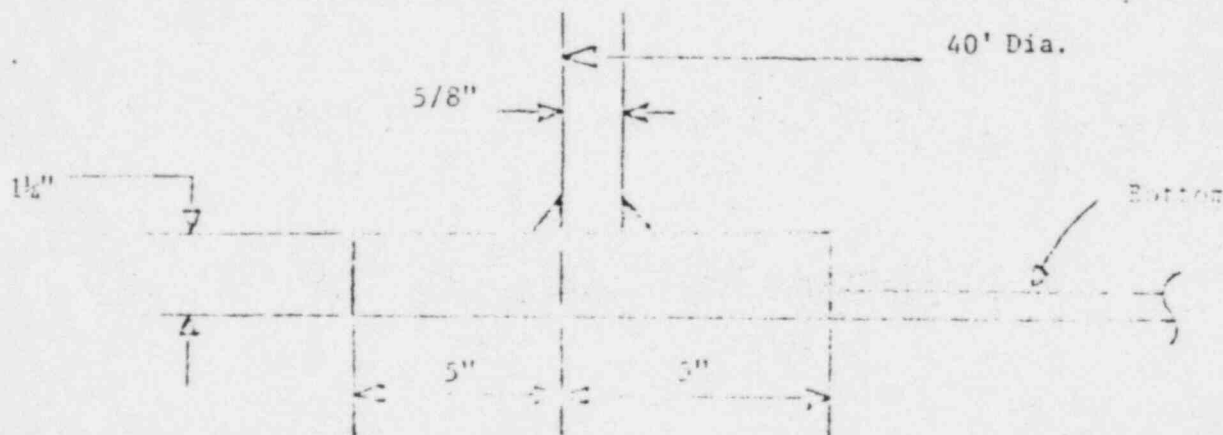
$$S = \frac{4(574305326)}{48(485)(3.1416)} - \frac{60598}{48(3.1416)} = 31,008 \text{ psi tensile}$$

$$V = \frac{1669072}{48(3.1416)} = 11,068 \text{ psi shear}$$

The allowable stresses are 38,749 psi tensile and 14,784 psi shear.

5.2 Concrete Compressive Stress

The tank has a support ring as shown below which transfers the overturning moment into the foundation.



The loads are the same as in Section 5.1 except the nozzle loads and seismic act downward and snow is added.

The concrete compressive stresses are obtained by the method of Brownell and Young, "Process Equipment Design", John Wiley & Sons, First Corrected Printing, Chapter 10. The anchor bolt stresses are also obtained, but since the method of Section 5.1 is more conservative for the anchor bolts, it is recommended that the stresses of Section 5.1 be used for the bolts.

The nomenclature is that of the Reference. Some of the parameters are:

$$n = 6 \quad t_3 = 10" \quad t_1 = \frac{48(3.1416)}{3.1416(485)} = .099"$$

The normal case is not given since it is trivial.

(a) Upset

$$K = .15; \quad C_c = 1.049; \quad C_t = 2.772; \quad z = .469; \quad j = .771$$

$$P_t = \frac{297046998 - 180427(.469)(485)}{(.771)(485)} = 664,627 \text{ lbs.}$$

$$f_s = \frac{684627}{(.099)(242.5)(2.772)} = 10,288 \text{ psi}$$

$$F_c = 684627 + 180427 = 865,054 \text{ lbs.}$$

$$f_c = \frac{865054}{(10 + 6(.099))(242.5)(1.049)} = 321 \text{ psi}$$

$$\text{Check: } k = \frac{1}{1 + \frac{10288}{6(321)}} = .157 \text{ O.K.} \quad f_{cmax} = 321(162.29/152.29) = 342 \text{ psi}$$

(b) Faulted

$$k = .15; \quad C_c = 1.049; \quad C_t = 2.772; \quad Z = .469; \quad j = .771$$

$$F_t = \frac{574305326 - 199455(.469)(485)}{(.771)(485)} = 1,414,514 \text{ lbs.}$$

$$f_s = \frac{1414514}{(.099)(242.5)(2.772)} = 21,255 \text{ psi}$$

$$F_c = 1414514 + 199455 = 1,613,969 \text{ lbs.}$$

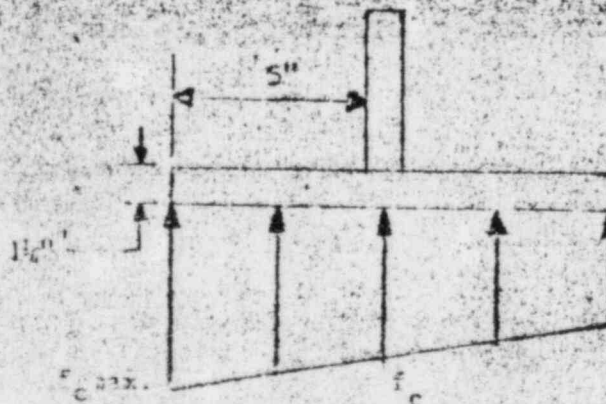
$$f_c = \frac{1613969}{(10 + 6(.099))(242.5)(1.049)} = 599 \text{ psi}$$

$$\text{Check: } k = \frac{1}{1 + \frac{21255}{6(599)}} = .145 \text{ O.K.} \quad f_{cmax} = 599(150.65/140.65) = 642 \text{ psi}$$

The allowable concrete compressive stresses for 5,000 psi concrete are 1,250 psi Normal and Upset and 1,666 psi Faulted per the AISC Code.

5.3 Support Ring Stress

The ring of Section 5.2 is subjected to bending stress due to the concrete compression. The stresses act as shown below.



Thus, the bending moment in the plate per inch of width is:

$M = 5(f_c)(2.5) + (f_{cmax} - f_c)(2.5)(5)(2/3)$ where f_c and f_{cmax} are the stresses from Section 5.2. Thus the moments are:

$$M = 5(321)(2.5) + (342 - 321)(2.5)(5)(2/3) = 4,187 \text{ in-lbs.}$$

$$M = 5(599)(2.5) + (642 - 599)(2.5)(5)(2/3) = 7,846 \text{ in-lbs.}$$

The section modulus of the plate is:

$$Z = \frac{1.25^2}{6} = .260 \text{ in.}^3/\text{in.}$$

and the stress is:

$$S = \frac{4187}{.26} = 16,104 \text{ psi Upset}$$
$$.26 = 30,177 \text{ psi Faulted}$$

The allowable stresses are $1.5(18,600) = 27,900$ psi Normal and Upset and 33,480 Faulted.

5.4 Anchor Bolt Chairs

The anchor bolt loads can be obtained by multiplying the stress in Section 5.1 by the bolt area. Thus the loads are:

Thus, the stress is:

$$\begin{aligned}
 S &= \frac{2.5(867)}{12.34} &= & 176 \text{ psi Normal} \\
 & &= & 10,004 \text{ psi Upset} \\
 & &= & 19,736 \text{ psi Faulted}
 \end{aligned}$$

The allowable stress is $1.5(14500) = 21,750$ psi Normal and Upset and 26,100 psi Faulted

(d) Bearing in Bottom Plate

The shearing load in the bolt imposes bearing stress in the $1\frac{1}{4}$ " bottom plate. This stress is:

$$V = \frac{R}{1.25(2.0)} = \frac{R}{2.5}$$

$$\begin{aligned}
 \text{Thus } V &= 472 \text{ psi Normal} \\
 &= 7,208 \text{ psi Upset} \\
 &= 13,908 \text{ psi Faulted}
 \end{aligned}$$

The allowable bearing stress per Appendix XVII - 2216 is $.9(30,000) = 27,000$ psi Normal and Upset and 32,400 psi Faulted.

5.5 Roof Stresses

The roof must be checked for the compressive stresses due to snow, deadweight, and seismic, and the tensile stress due to the 3 psi tornado induced vacuum.

The allowable compressive stress is obtained from ASME Code Section NC-3133.4.

$$\frac{R}{T} = \frac{480}{1.25} = 1,920$$

From Figure VII-1102-4:

$$B = 850$$

$$\begin{aligned}
 \text{Thus } P_a &= \frac{850}{1920} &= & .443 \text{ psig Normal \& Upset} \\
 & &= & .53 \text{ psig Faulted}
 \end{aligned}$$

(e) Normal

ME-311

CERTIFICATION STATEMENT

This tank and foundation has been analyzed in accordance with Duke Power Company Specification CNS-1148.00-03, the ASME Boiler and Pressure Vessel Code, Section III, Class 2, ACI-318-71 and accepted good practice in seismic stress analysis.

The tank and foundation meets all requirements of the specification and is structurally adequate to withstand Normal, Upset, Faulted, and Tornado Loading conditions.

Prepared by:	<u>J. H. Appleton</u> J. H. Appleton (Foundation, App. E) P.E.	<u>5/10/76</u> Date
	<u>C. K. McDonald</u> C. K. McDonald (Tank), P.E.	<u>6/24/76</u> Date
Checked by:	<u>T. K. Hicks</u> T. K. Hicks, P.E.	<u>6/24/76</u> Date
Approved by:	<u>C. K. McDonald</u> C. K. McDonald McDonald Engineering Analysis Co.	<u>6/24/76</u> Date
Approved by:	<u>J. L. Dawson</u> J. L. Dawson, V. P. Engineering Richmond Engineering Company	<u>6/25/76</u> Date

TABLE OF CONTENTS

1.	INTRODUCTION	1
2.	SUMMARY OF RESULTS	3
2.1	Normal	3
2.2	Upset	4
2.3	Faulted	5
3.	FREQUENCY ANALYSIS	6
3.1	Lateral Frequencies	6
3.2	Vertical Frequencies	7
4.	LOADING CRITERIA	10
4.1	Seismic Loading	10
4.2	Individual Nozzle Loadings	10
4.3	Nozzle Loads on Tank and Support	11
4.4	Tornado Loads	12
4.5	Other Normal Loads	12
5.	DETAILED STRESS ANALYSIS	14
5.1	Anchor Bolts	14
5.2	Concrete Compressive Stress	15
5.3	Support Ring Stress	16
5.4	Anchor Bolt Chairs	16
5.5	Roof Stresses	19
5.6	Shell Compressive Stresses	20
5.7	Tensile Stress in Shell	24
5.8	Stresses in Nozzles	25
	APPENDIX A - Computer Input	
	APPENDIX B - Seismic and Deadweight Loads	
	APPENDIX C- References	
	APPENDIX D - Frequencies and Mode Shapes	

1. INTRODUCTION

This report covers the structural and seismic analysis of the Refueling Water Storage Tank and its foundation. The tank is shown on Duke Power Company Drawing CN-1325-02, Revision 2. A dimensional sketch of the tank and foundation is shown on page 2 of this report.

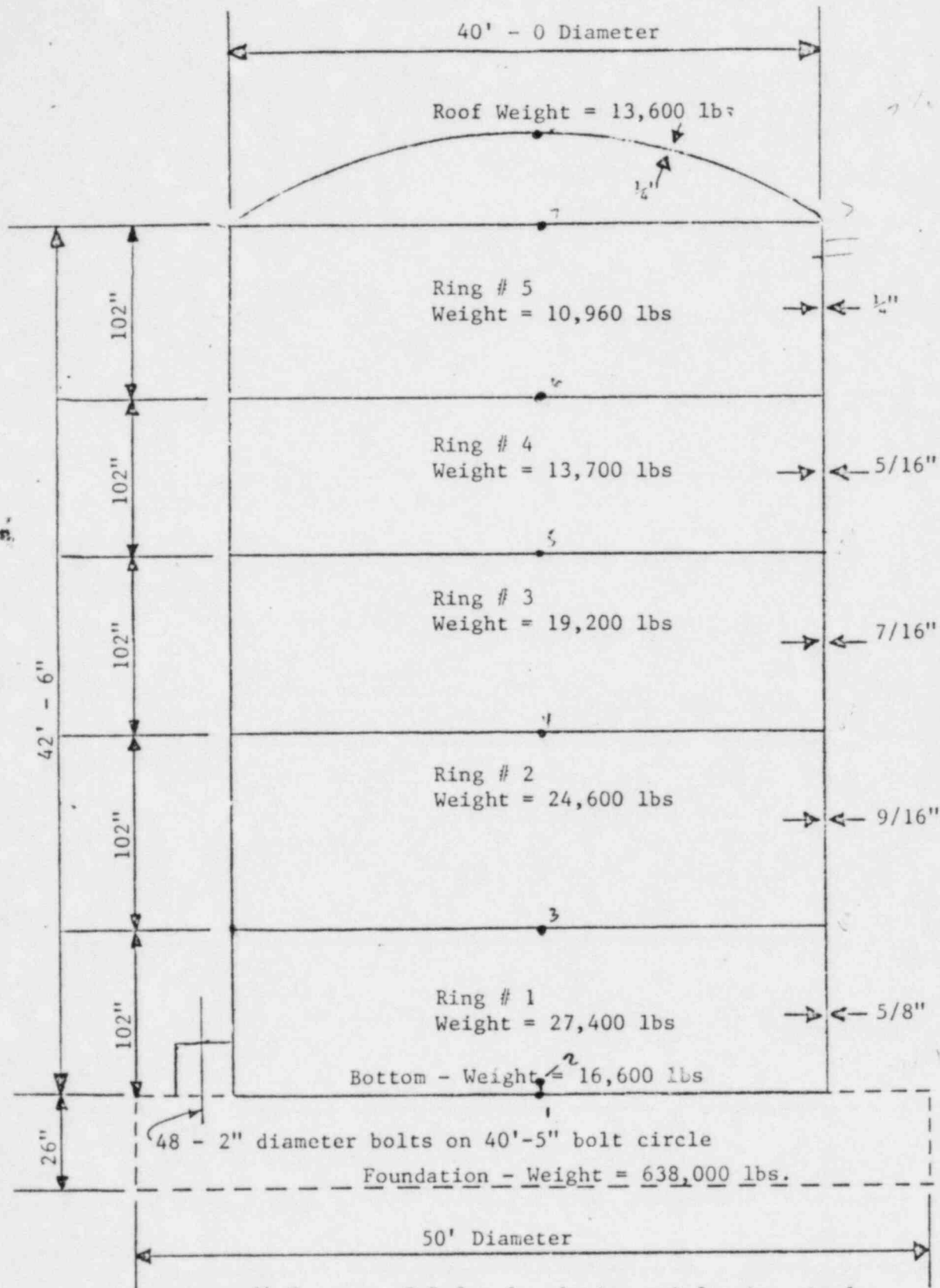
The lateral frequency of the tank was checked for both the sloshing and no sloshing conditions. The no sloshing condition was the worst case.

The localized nozzle stresses are treated as nozzles in a flat plate by use of Roark's Formulas for Stress and Strain (Ref. 1). This was done because the tank parameters are not within the range of Welding Research Council Bulletin 107, the usual technique used. The flat plate approach is believed to be very conservative.

The soil structure interaction was included by the use of soil springs. The soil springs were calculated by use of equations given by Richart, Hall, and Woods (Ref. 5), page 350, Table 10-13. The Poissons ratio of the soil (.33) and damping of the combined soil/shell first mode (5%), not given in the specification, was obtained by direct communication with Duke Power.

There are two Faulted conditions for this tank. These are the Normal + SSE and Normal + Tornado conditions. From the loadings given in Section 4, it is obvious that the Normal + SSE case governs for most of the stress calculations. For those conditions where it is not obvious that the Normal + SSE case governs, the Normal + Tornado case is also calculated.

FIGURE 1 - TANK OUTLINE SKETCH



- 1) See page E-9 for foundation reinforcing steel
- 2) See pages 25-30 for reinforcing pad sizes required

2. SUMMARY OF RESULTS

2.1 Normal

<u>Component</u>	<u>Actual</u>	<u>Allowable</u>
Anchor Bolts Stress, PSI - Tensile	276	40,000
- Shear	376	12,320
Concrete Compressive Stress, PSI	20	750
Support Ring Stress, PSI	1,500	27,900
Anchor Chair Stress, PSI		
Top Plate	192	21,750
Shell	111	27,900
Vertical Plates	176	21,750
Bearing in Bottom Plate	591	27,000
Roof Pressure, PSIG	.21	.443
Shell Compressive Stress, PSI		
Ring #1	322	4,688
Ring #2	243	4,219
Ring #3	226	3,281
Ring #4	223	2,344
Ring #5	196	1,875
Shell Tensile Stress, PSI	7,085	18,600
Nozzle Stresses, PSI		
Mark A (4" Nozzle)	19, 4	55,800
Mark B (24" Nozzle)	27,694	55,800
Mark C (6" Nozzle)	20,872	55,800
Mark D (4" Nozzle)	19,564	55,800
Mark E (3" Nozzle)	13,042	55,800
Mark G (8" Nozzle)	27,685	55,800
Mark H and J (1" Nozzle)	8,575	55,800

2.2 Upset

<u>Component</u>	<u>Actual</u>	<u>Allowable</u>
Anchor Bolts Stress, PSI - Tensile	15,718	40,000
- Shear	5,736	12,320
Concrete Compressive Stress, PSI	179	750
Support Ring Stress, PSI	13,428	27,900
Anchor Chair Stress, PSI		
Top Plate	10,933	21,750
Shell	6,321	27,900
Vertical Plates	10,004	21,750
Bearing in Bottom Plate	9,010	27,000
Roof Pressure, PSIG	.23	.443
Shell Compressive Stress, PSI		
Ring #1	2,818	4,688
Ring #2	2,186	4,219
Ring #3	1,786	3,281
Ring #4	1,319	2,344
Ring #5	617	1,875
Shell Tensile Stress, PSI	10,856	18,600
Nozzle Stresses, PSI		
Mark A	44,124	55,800
Mark B	52,075	55,800
Mark C	41,744	55,800
Mark D	44,124	55,800
Mark E	31,077	55,800
Mark G	52,041	55,800
Mark H and J	13,842	55,800

2.3 Faulted

<u>Component</u>	<u>Actual</u>	<u>Allowable</u>
Anchor Bolts Stress, PSI - Tensile	31,008	38,749
- Shear	11,068	14,784
Concrete Compressive Stress, PSI	337	1,000
Support Ring Stress, PSI	25,278	33,480
Anchor Chair Stress, PSI		
Top Plate	21,568	26,100
Shell	12,469	33,480
Vertical Plates	19,736	26,100
Bearing in Bottom Plate	17,386	32,400
Roof Pressure ,	.25	.53
Shell Compressive Stress, PSI		
Ring #1	5,290	5,625
Ring #2	4,119	5,063
Ring #3	3,335	3,937
Ring #4	2,401	2,813
Ring #5	1,023	2,250
Shell Tensile Stress, PSI	14,630	22,320
Nozzle Stresses, PSI		
Mark A	58,902	66,960
Mark B	66,154	66,960
Mark C	52,180	66,960
Mark D	58,902	66,960
Mark E	42,594	66,960
Mark G	66,134	66,960
Mark H and J	18,361	66,960

3. FREQUENCY ANALYSIS

3.1 Lateral Frequencies

(a) Completely Filled Condition

The completely filled condition is analyzed by use of the stick model shown on page 8 and the ICES-STRU DL code. The soil spring constants are calculated by use of Table 10-13, page 350, of Reference 5. These are:

$$G = 70,000 \text{ psi} \quad \nu = .33 \quad r_o = 25' = 300''$$

Vertical

$$K_y = \frac{4(70000)(300)}{1 - .33} = 125,373,134 \text{ lb/in.}$$

Lateral

$$K_x = \frac{32(1 - .33)(70000)(300)}{7 - 8(.33)} = 103,266,055 \text{ lb/in.}$$

Rocking

$$K_{MZ} = \frac{8(70000)(300)^3}{3(1 - .33)} = 7,522,388,059,701 \text{ in-lbs./radians}$$

The weights are based upon the tank being filled to the overflow nozzle, see page 2 for dimensions. Additional weight was included for nozzles, chairs, etc.

No soil weight was included at Joint 1 for lateral motion but since it is believed that some soil will move with the foundation vertically, a conservative amount of soil was included at Joint 1 for vertical motion.

The computer input data is included on pages A-1 & A-2. The frequencies are given below. The lowest frequency is seen to be 5.99 cps.

MODE	EIGENVALUE	FREQUENCY	PERIOD
1	3.5344720 C1	5.987040D C0	1.670273E-01
2	4.073741D C2	2.018351D C1	4.954533E-02
3	1.152221D C3	3.394580D C1	2.945867E-02
4	1.945027D C3	4.413040D C1	2.265701E-02
5	2.300908E C3	5.348740D C1	1.869597E-02
6	4.227077D C3	6.001090D C1	1.666000E-02
7	1.030398E C3		

The mode shapes are given in Appendix D.

(b) Sloshing Condition

Sloshing is checked per A.E.C. T.I.D. 7024 (Ref. 2). The nomenclature is per T.I.D. 7024.

$$\frac{1.84(h)}{R} = \frac{(1.84)42.5}{20} = 3.91$$

$$\text{Tanh}(3.91) = .992$$

$$\omega^2 = \frac{1.84(32.2)(.992)}{20} = 2.94$$

$$f = .27 \text{ cps}$$

The spectra curve shows that this low frequency gives a much lower seismic load than is obtained by use of the no sloshing case. Thus the no sloshing case is the worst and is used henceforth in the analysis.

3.2 Vertical Frequencies

The model, page 8, used for the lateral frequency analysis is also used for the vertical analysis. However, the roof must be checked for its frequency. The equation for the lowest mode of the roof is given by Kraus, Ref. 6, pages 316-319. Thus:

$$R/a = 2$$

$$\omega^2 = \frac{.97121E}{\rho R^2}$$

$$E = 29 \times 10^6 \text{ psi}$$

$$\rho = .285/386 = .0007383$$

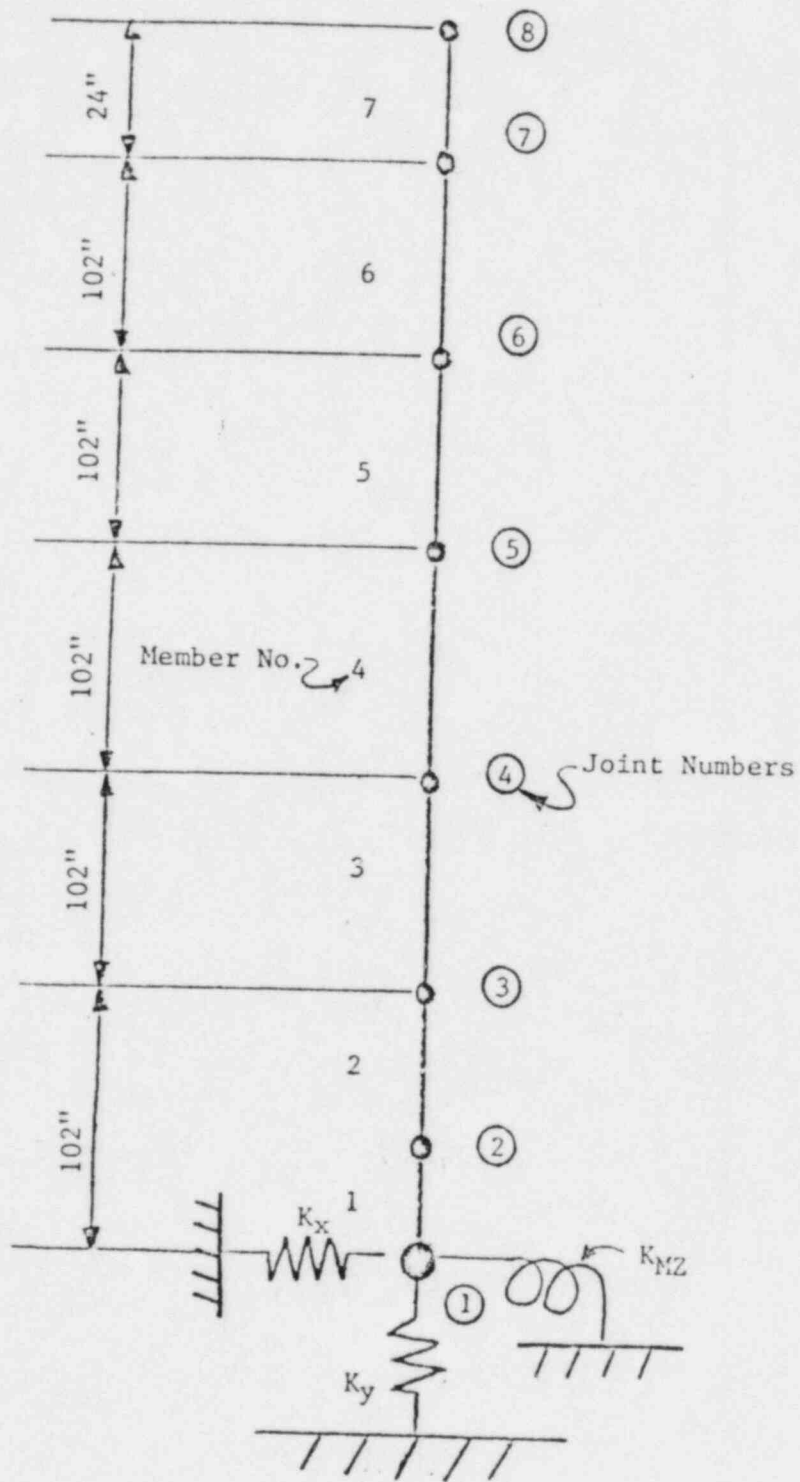
$$R = 40' = 480''$$

Thus:

$$f = \frac{\omega}{2\pi} = 64.76 \text{ cps}$$

and the roof is rigid and can be included in the stick model.

As mentioned previously, an amount of soil is included at Joint 1 for



the vertical analysis. The weight of soil was selected conservatively since some authors do not include any soil weight in the model and other authors include various amounts.

The computer input is given on pages A-3 and A-4 and the frequencies are given below:

EIGENVALUES

MODE	EIGENVALUE	FREQUENCY	PERIOD
1	2.535439D 02	1.592306D C1	6.280200D-C2
2	7.617490D 03	3.727324D C1	1.145761D-U2
3	4.704019D C4	2.108875D C2	4.610655D-C3
4	1.368962D 05	3.69949D 02	2.702740D-C3
5	2.353752D C5	4.884410D 02	2.047327D-U3
6	3.393807D C5	5.825639D C2	1.710550D-C3
7	1.102370D 06	1.052792D C2	9.458550D-C4
8	1.385110D C6	1.176907D 03	8.496948D-C4

The mode shapes are given in Appendix D.

4. LOADING CRITERIA

4.1 Seismic Loading

The seismic loadings obtained from the spectra curves are:

Lateral OBE

<u>Mode</u>	<u>Frequency</u>	<u>Loading</u>
1	5.99	.3g
2	20.2	.13g
3	33.95	.13g
4	44.13	.13g
5	53.5	.13g

Vertical OBE

1	15.9	.12g
2	87.3	.09g

The SSE loads are twice the OBE loads.

The computer input for the ICES-STRUDL analysis (IBM 370/158 computer) is given in Appendix A and the output in Appendix B.

4.2 Individual Nozzle Loadings

The nozzle loads are calculated in accordance with the specification. The attached piping is assumed to be of the same schedule and size as the nozzle pipe. The loads are:

<u>Nozzle</u>	<u>Normal</u>	<u>Upset</u>	<u>Faulted</u>
<u>1" Size</u>			
F _x , F _y , F _z , lbs.	128	256	320
M _x , M _y , M _z , in-lbs.	322	644	805
<u>3" Size</u>			
F _x , F _y , F _z , lbs.	446	892	1115
M _x , M _y , M _z , lbs.	3448	6896	8620

<u>Nozzle</u>	<u>Normal</u>	<u>Upset</u>	<u>Faulted</u>
<u>4" Size</u>			
F _x , F _y , F _z , lbs.	634	1268	1585
M _x , M _y , M _z , in-lbs.	6440	12880	16100
<u>6" Size</u>			
F _x , F _y , F _z , lbs.	1116	2232	2790
M _x , M _y , M _z , in-lbs.	17000	34000	42500
<u>8" Size</u>			
F _x , F _y , F _z , lbs.	1680	3360	4200
M _x , M _y , M _z , in-lbs.	33620	67240	84050
<u>24" Size</u>			
F _x , F _y , F _z , lbs.	5560	11120	13900
M _x , M _y , M _z , in-lbs.	323800	647600	809500

4.3 Nozzle Loads on Tank and Support

Since Unit 1 and Unit 2 has different nozzle orientations, the worst case orientations for resultant overturning of the tank are assumed. The resultant shear on the nozzle is assumed to act upward (downward) for the worst case.

(a) Loads on the Support

The Normal loads are tabulated below:

<u>Mark</u>	<u>Resultant Shear, Lbs.</u>	<u>Axial Load, Lbs.</u>	<u>Result. Bending Mom., in-lbs</u>
A	634	897	546,921
B	5560	7862	2,627,028
C	1578	1116	1,095,795
D	634	897	230,397
E	446	631	382,753
G	6720	9502	2,757,006
H	384	543	139,673
J	384	543	143,513
SRSS	8867	12537	4,030,128

Upset and Faulted are 2 and 2.5 times the Normal loads.

(b) Loads on the Upper Portion of Tank

Nozzles A, C, and E impose loads in the shell above the first shell ring. All other nozzles are located in the first ring. The bending moment is as follows:

$$M = 674,860 + 2658 X \text{ in-lbs.}$$

$$P = 2644 \text{ lbs. axial load}$$

X is distance from roof/shell intersection, inches.

4.4 Tornado Loads

The tornado loads are 60 mph translational and 300 mph rotational for a total wind load of 360 mph. This is equal to 324 pounds per square foot of projected area (using a .60 shape factor). Thus the tornado wind loads are as follows:

(a) Loads at the Support

$$V = 324(40)(47.86) = 620,266 \text{ lbs. Shear}$$

$$M = \frac{620266(47.86)(12)}{2} = 178,115,585 \text{ in-lbs.}$$

(b) Loads on Upper Portion of Tank

$$M = 2,234,014 + 540 X^2 + 69466 X \text{ in-lbs.}$$

X = distance in inches from shell/roof intersection.

In addition to the above loads, the tornado induces a 3 psi vacuum.

4.5 Other Normal Loads

(a) Atmospheric pressure at 120° F.

(b) Snow load of 20 PSF on projected roof area.

$$\text{Total snow load} = 20(.785)(40)^2 = 25,120 \text{ lbs.}$$

(c) Wind load of 95 mph at 30 ft. above ground. For simplicity a conservative 25 PSF is used for the length of the tank. Thus the loads at the base are: $V = 25(40)(47.86) = 47,860 \text{ lbs. shear}$

$$M = \frac{47,860(47.86)(12)}{2} = 13,743,478 \text{ in-lbs.}$$

$$M = 172378 + 41.67 X^2 + 5360 X \quad (X \text{ as in Section 4.4})$$

5. DETAILED STRESS ANALYSIS

5.1 Anchor Bolts

The anchor bolts are checked by Equation 43, page 26, of Reference 3. The loads on the support are given in Section 4 and Appendix B. There are 48 - 2" diameter A-325 bolts on a 40' - 5" bolt circle.

(a) Normal (Wind + Nozzles + Deadweight)

$$M = 13,743,478 + 4,030,128 = 17,773,606 \text{ in-lbs.}$$

$$R = 8867 + 47860 = 56,727 \text{ lbs. shear}$$

$$P = 117,460 - 12,537 = 104,923 \text{ lbs. Down}$$

$$S = \frac{4(17773606)}{48(485)(3.1416)} - \frac{104923}{48(3.1416)} = 276 \text{ psi tensile}$$

$$V = \frac{56727}{48(3.1416)} = 376 \text{ psi shear}$$

The allowable stresses are 40,000 psi tensile and 12,320 psi shear per Appendix XVII.

(b) Upset (Normal + OBE)

$$M = 2(4030128) + 13743478 + 275243264 = 297,046,998 \text{ in-lbs.}$$

$$R = 2(8867) + 47860 + 799522 = 865,116 \text{ lbs.}$$

$$P = 117,460 - 2(12537) - 12760 = 79,626 \text{ lbs.}$$

$$S = \frac{4(297046998)}{48(485)(3.1416)} - \frac{79626}{48(3.1416)} = 15,718 \text{ psi tensile}$$

$$V = \frac{865116}{48(3.1416)} = 5,736 \text{ psi shear}$$

The allowable stresses are 40,000 psi tensile and 12,320 psi shear.

(c) Faulted (Normal + SSE)

$$M = 13743478 + 2.5(4030128) + 2(275243264) = 574,305,326 \text{ in-lbs.}$$

$$R = 2.5(8867) + 47860 + 2(799522) = 1,669,072 \text{ lbs.}$$

$$P = 117,460 - 2.5(12537) - 2(12760) = 60,598 \text{ lbs.}$$

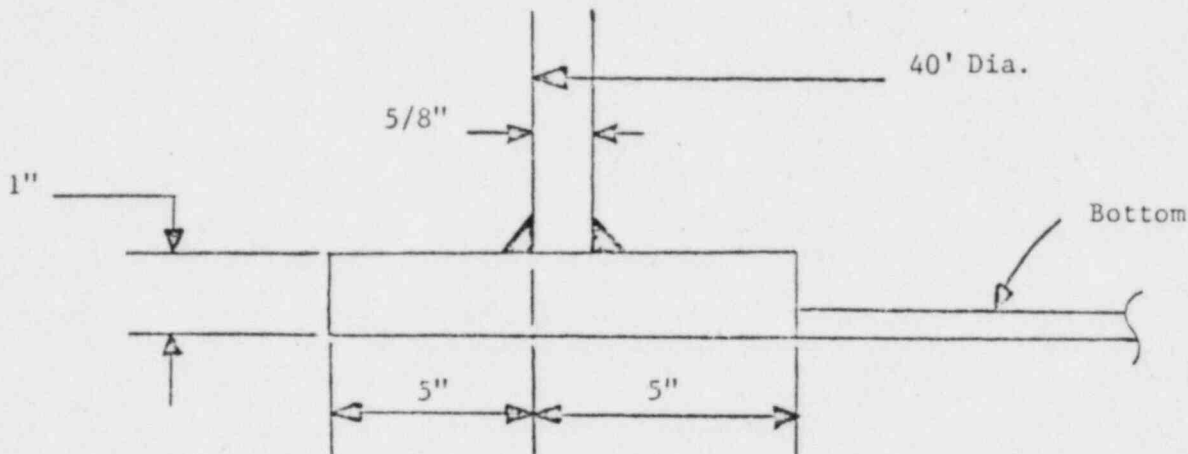
$$S = \frac{4(574305326)}{48(485)(3.1416)} - \frac{60598}{48(3.1416)} = 31,008 \text{ psi tensile}$$

$$V = \frac{1669072}{48(3.1416)} = 11,068 \text{ psi shear}$$

The allowable stresses are 38,749 psi tensile and 14,784 psi shear.

5.2 Concrete Compressive Stress

The tank has a support ring as shown below which transfers the overturning moment into the foundation.



The properties of the ring which imposes stress into the concrete are:

$$A = .785(490^2 - 470^2) = 15,072 \text{ in.}^2$$

$$Z = \frac{3.1416(490^4 - 470^4)}{64(245)} = 1,773,397 \text{ in.}^3$$

The loads are the same as in Section 5.1 except the nozzle loads and seismic act downward and snow is added. Thus the compressive stresses are:

$$S = \frac{17773606}{1773397} + \frac{155130}{15072} = 20 \text{ psi Normal}$$

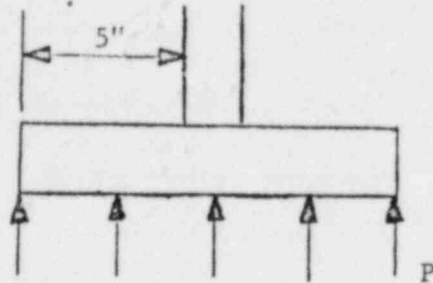
$$S = \frac{297046998}{1773397} + \frac{180427}{15072} = 179 \text{ psi Upset (Normal + OBE)}$$

$$S = \frac{574305326}{1773397} + \frac{199455}{15072} = 337 \text{ psi Faulted (Normal + SSE)}$$

The allowable stresses for 3,000 psi concrete are 750 psi Normal and Upset and 1,000 psi Faulted per the A.I.S.C. Code.

5.3 Support Ring Stress

The ring of section 5.2 is subjected to bending stress due to the concrete compression. The stresses act as shown below:



Thus the bending moment in the plate per inch of width is:

$$M = 5(P)(2.5) = 12.5 P \text{ where } P \text{ is the stress from Section 5.2.}$$

Thus the moments are:

$$\begin{aligned} M &= 12.5(20) = 250 \text{ in-lbs. Normal} \\ &= 12.5(179) = 2,238 \text{ in-lbs. Upset} \\ &= 12.5(337) = 4,213 \text{ in-lbs. Faulted (Normal plus SSE)} \end{aligned}$$

The section modulus of the plate is:

$$Z = \frac{(1)^2}{6} \text{ in.}^3/\text{in.}$$

and the stress is:

$$\begin{aligned} S &= 250(6) = 1,500 \text{ psi Normal} \\ &= 2238(6) = 13,428 \text{ psi Upset (Normal + OBE)} \\ &= 4213(6) = 25,278 \text{ psi Faulted (Normal + SSE)} \end{aligned}$$

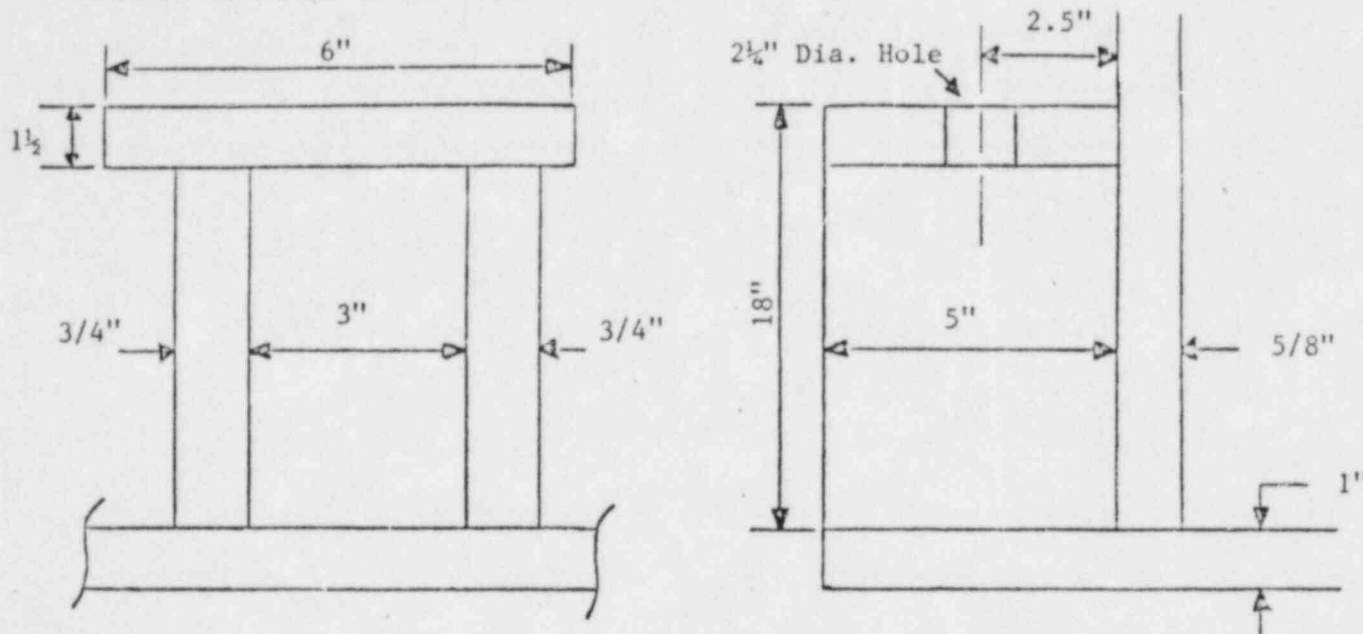
The allowable stresses are $1.5(18600) = 27,900$ psi Normal and Upset and 33,480 psi Faulted.

5.4 Anchor Bolt Chairs

The anchor bolt loads can be obtained by multiplying the stress in Section 5.1 by the bolt area. Thus the loads are:

	<u>Normal</u>	<u>Upset</u>	<u>Faulted</u> (Normal plus SSE)
P (uplift), Lbs.	867	49,380	97,415
R (shear), Lbs.	1,181	18,020	34,771

The chair is as shown below:



(a) Top Plate

The stress in the top plate is given by:

$$S = \frac{P((.375)(3) - .22(2))}{1.375(1.5)^2} = .2214 P$$

Thus, the stress is:

$$\begin{aligned} S &= 192 \text{ psi Normal} \\ &= 10,933 \text{ psi Upset} \\ &= 21,568 \text{ psi Faulted} \end{aligned}$$

The allowable stress is $1.5(14500) = 21,750$ psi Normal and Upset and 26,100 psi Faulted.

(b) Shell

The stress in the shell is given Equation 46 of Reference 3.

$$S = \frac{.9P(2.5) \sqrt[4]{240(.625)}}{18 \sqrt{6} (1^2 + .625)^2} = .128 P$$

Thus, the stress is:

$$S = 111 \text{ psi Normal; } 6,321 \text{ psi Upset; } 12,469 \text{ psi Faulted}$$

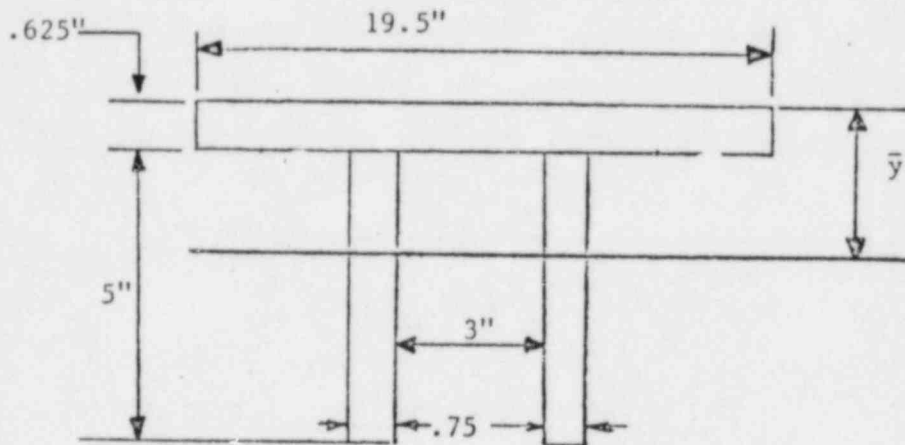
The allowable stress is $1.5(18600) = 27,900$ psi Normal and Upset and
 33,480 psi Faulted.

(c) Bending in Vertical Plate

The bending moment in the plates is:

$$M = 2.5 P$$

The section modulus of the plates/shell composite is:



$$\bar{y} = \frac{19.5(.625)(.3125) + 2(.75)(5)(3.125)}{12.1875 + 7.5} = 1.384''$$

$$I = \frac{2(.75)(5)^3}{12} + 7.5(3.125 - 1.384)^2 + 12.1875(1.384 - .312)^2$$

$$= 52.36 \text{ in.}^4$$

$$Z = \frac{52.36}{4.241} = 12.34 \text{ in.}^3$$

Thus, the stress is:

$$\begin{aligned}
 S &= \frac{2.5(867)}{12.34} &= & 176 \text{ psi Normal} \\
 & &= & 10,004 \text{ psi Upset} \\
 & &= & 19,736 \text{ psi Faulted}
 \end{aligned}$$

The allowable stress is $1.5(14500) = 21,750$ psi Normal and Upset and 26,100 psi Faulted

(d) Bearing in Bottom Plate

The shearing load in the bolt imposes bearing stress in the 1" bottom plate. This stress is:

$$V = \frac{R}{1(2.0)} = \frac{R}{2.0}$$

$$\begin{aligned}
 \text{Thus } V &= 591 \text{ psi Normal} \\
 &= 9,010 \text{ psi Upset} \\
 &= 17,386 \text{ psi Faulted}
 \end{aligned}$$

The allowable bearing stress per Appendix XVII - 2216 is $.9(30,000) = 27,000$ psi Normal and Upset and 32,400 psi Faulted.

5.5 Roof Stresses

The roof must be checked for the compressive stresses due to snow, deadweight, and seismic, and the tensile stress due to the 3 psi tornado induced vacuum.

The allowable compressive stress is obtained from ASME Code Section NC-3133.4.

$$\frac{R}{T} = \frac{480}{.25} = 1,920$$

From Figure VII-1102-4:

$$B = 850$$

$$\begin{aligned}
 \text{Thus } P_a &= \frac{850}{1920} &= & .443 \text{ psig Normal \& Upset} \\
 & &= & .53 \text{ psig Faulted}
 \end{aligned}$$

(a) Normal

$$\text{Snow} = \frac{20}{144} = .14 \text{ psig}$$

Deadweight = .07 psi
 Total = .21 psi

(b) Upset

Seismic (.25 g) .02 psi
 Normal .21 psi
 .23 psi

(c) Faulted

Seismic (.5 g) .04 psi
 Normal .21 psi
 .25 psi

In addition, the tensile stress due to the 3 psi negative tornado pressure must be checked.

$$S = \frac{PR}{2t} = \frac{3(480)}{2(.25)} = 2,880 \text{ psi}$$

The allowable tensile stress is $18,600(1.2) = 22,320 \text{ psi}$.

5.6 Shell Compressive Stresses

The seismic, nozzles, and wind overturning moments produce compressive buckling stress in the shell. This stress must be computed for each shell section since the thickness is not the same.

The overturning moments due to seismic are obtained from Appendix B, those due to wind from Section 4.5, and those due to nozzles from Section 4.3. The seismic axial loads are obtained from Appendix B and the deadweight is obtained from data given on page 2.

The shell ring numbers are given on page 2.

Ring No. 1

(a) Normal

$$M = 13,743,478 + 4,030,128 = 17,773,606 \text{ in-lbs.}$$

$$P = 117460 + 12537 + 25120 = 155,117 \text{ lbs.}$$

$$S = \frac{17773606}{113097} + \frac{155117}{942} = 322 \text{ psi}$$

The allowable compressive stress is $S_a = \frac{1800000 \times (.625)}{240} = 4,688 \text{ psi}$

from NC-3922.3.

(b) Upset

$$M = 2(4030128) + 13743478 + 275243264 = 297,046,998 \text{ in-lbs.}$$

$$P = 117460 + 2(12537) + 25120 + 12760 = 180,414 \text{ lbs.}$$

$$S = \frac{297046998}{113097} + \frac{180414}{942} = 2,818 \text{ psi}$$

The allowable is 4,688 psi.

(c) Faulted (Normal plus SSE)

$$M = 2.5(4030128) + 13743478 + 2(275243264) = 574,305,326 \text{ in-lbs.}$$

$$P = 117460 + 2.5(12537) + 25120 + 2(12760) = 199,443 \text{ lbs.}$$

$$S = \frac{574305326}{113097} + \frac{199443}{942} = 5,290 \text{ psi}$$

The allowable is $1.2(4688) = 5,625 \text{ psi}$.

Ring No. 2

(a) Normal

$$M = 674860 + 2658(408) + 172378 + 41.67(408)^2 + 5360(408) = 11,055,137 \text{ in-lbs.}$$

$$P = 86060 + 2644 + 25120 = 113,824 \text{ lbs.}$$

$$S = \frac{11055137}{101788} + \frac{113824}{848} = 243 \text{ psi}$$

The allowable is 4,219 psi (Note: 1/2" plate was used in computer analysis but had to be increased to 9/16" here.)

(b) Upset

$$M = 11055137 + (674860 + (2658)(408)) + 194535680 = 207,350,141 \text{ in-lbs.}$$

$$P = 86060 + 2644(2) + 25120 + 9766 = 126,234 \text{ lbs.}$$

$$S = \frac{207350141}{101788} + \frac{126234}{848} = 2,186 \text{ psi}$$

The allowable stress is 4,219 psi.

(c) Faulted (Normal plus SSE)

$$M = 11055137 + 1.5(674860) + 1.5(2658)(408) + 2(194535680)$$

$$= 402,765,483 \text{ in-lbs.}$$

$$P = 86060 + 2644(2.5) + 25120 + 9766(2) = 137,322 \text{ lbs.}$$

$$S = \frac{402765483}{101788} + \frac{137322}{848} = 4,119 \text{ psi}$$

The allowable stress is 5,063 psi.

Ring No. 3

(a) Normal

$$M = 674860 + 2658(306) + 172378 + 41.67(306)^2 + 5360(306)$$

$$= 7,202,558 \text{ in-lbs.}$$

$$P = 61460 + 2644 + 25120 = 89,224 \text{ lbs.}$$

$$S = \frac{7202558}{79168} + \frac{89224}{660} = 226 \text{ psi}$$

The allowable stress is 3,281 psi.

(b) Upset

$$M = 7202558 + (2658)(306) + 674860 + 120837184 = 129,527,950 \text{ in-lbs.}$$

$$P = 61460 + 2(2644) + 25120 + 7215 = 99,083 \text{ lbs.}$$

$$S = \frac{129527950}{79168} + \frac{99083}{660} = 1,786 \text{ psi}$$

The allowable stress is 3,281 psi.

(c) Faulted (Normal plus SSE)

$$M = 7202558 + 1.5(2658)(306) + 1.5(674860) + 2(120837184)$$

$$= 251,109,238 \text{ in-lbs.}$$

$$P = 61460 + 2.5(2644) + 25120 + 2(7215) = 107,620 \text{ lbs.}$$

$$S = \frac{251109238}{79168} + \frac{107620}{660} = 3,335 \text{ psi}$$

The allowable stress is 3,937 psi.

27
612

Ring No. 4

(a) Normal

$$M = 674860 + 2658(204) + 172378 + 41.67(204)^2 + 5360(204)$$

$$= 4,217,049 \text{ in-lbs.}$$

$$P = 42260 + 2644 + 25120 = 70,024 \text{ lbs.}$$

$$S = \frac{4217049}{56549} + \frac{70024}{471} = 223 \text{ psi}$$

The allowable stress is 2,344 psi.

(b) Upset

$$M = 4217049 + 2658(204) + 674860 + 59807776 = 65,241,917 \text{ in-lbs.}$$

$$P = 42260 + 2(2644) + 25120 + 5166 = 77,834 \text{ lbs.}$$

$$S = \frac{65241917}{56549} + \frac{77834}{471} = 1,319 \text{ psi}$$

The allowable stress is 2,344 psi.

(c) Faulted (Normal plus SSE)

$$M = 4217049 + 1.5(2658)(204) + 1.5(674860) + 2(59807776)$$

$$= 125,658,239 \text{ in-lbs.}$$

$$P = 42260 + 2.5(2644) + 25120 + 2(5166) = 84,322 \text{ lbs.}$$

$$S = \frac{125658239}{56549} + \frac{84322}{471} = 2,401 \text{ psi}$$

The allowable stress is 2,813 psi.

Ring No. 5

(a) Normal

$$M = 674860 + 2658(102) + 172378 + 41.67(102)^2 + 5360(102)$$

$$= 2,098,609 \text{ in-lbs.}$$

$$P = 28560 + 2644 + 25120 = 56,324 \text{ lbs.}$$

$$S = \frac{2098609}{45239} + \frac{56324}{377} = 196 \text{ psi}$$

The allowable stress is 1,875 psi.

(b) Upset

$$M = 2098609 + 2658(102) + 674860 + 17352448 = 20,397,033 \text{ in-lbs.}$$

$$P = 28560 + 2(2644) + 25120 + 3485 = 62453 \text{ lbs.}$$

$$S = \frac{20397033}{45239} + \frac{62453}{377} = 617 \text{ psi}$$

The allowable stress is 1,875 psi.

(c) Faulted (Normal plus SSE)

$$M = 2098609 + 1.5(2658)(102) + 1.5(674860) + 2(17352448) \\ = 38,222,469 \text{ in-lbs.}$$

$$P = 28560 + 2.5(2644) + 25120 + 2(3485) = 67,260 \text{ lbs.}$$

$$S = \frac{38222469}{45239} + \frac{67260}{377} = 1,023 \text{ psi}$$

The allowable stress is 2,250 psi.

5.7 Tensile Stress in Shell

The maximum tensile stress in the shell (Ring no. 1) is checked here.

(a) Normal

$$P = \frac{42.5(62.5)}{144} = 18.45 \text{ psig}$$

$$S = \frac{PR}{t} = \frac{18.45(240)}{.625} = 7,085 \text{ psi}$$

The allowable stress is 18,600 psi.

(b) Upset

$$P = 18.45(1.25) + \frac{40(62.5)(.3)}{144} = 28.27 \text{ psig}$$

$$S = \frac{28.27(240)}{.625} = 10,856 \text{ psi}$$

The allowable stress is 18,600 psi.

(c) Faulted (Normal + SSE)

$$P = 18.45(1.5) + \frac{40(62.5)(.6)}{144} = 38.1 \text{ psig}$$

$$S = \frac{38.1(240)}{.625} = 14,630 \text{ psi}$$

The allowable stress is 22,320 psi.

5.8 Stresses in Nozzles

The nozzles in the tank are treated by Cases 5 and 4, page 217 of Reference 1. The nomenclature is that of Reference 1.

In this case $a \gg r$. Thus the equations reduce to:

Case 5

$$S_3 = \frac{.454M}{t^2r}$$

Case 4

$$S_4 = \frac{.1434W}{t^2} (3.33 + 4.33L_n \frac{d}{r}) \qquad S_1 = S_3 + S_4$$

$S_b = S_1 + S_2$;	$W =$ Axial Force	$M =$ Resultant Bending Moment
	$t =$ plate thickness	$b = r =$ outside radius of nozzle
$S_2 =$ Internal pressure stress		

$d =$ distance from edge of shell/head or shell/bottom junction.

The shearing stresses are:

$$V = \frac{M_t}{2\pi r^2 t} + \frac{R}{\pi r t}$$

$M_t =$ torsional moment $R =$ Resultant shearing force

The stresses are combined by the maximum shearing stress theory.

$$S = \sqrt{(2V)^2 + S_b^2}$$

The nozzle loads are given in Section 4.2.

The nozzles in the shell should be analyzed by Welding Research Council Bulletin 107. However, the shell parameter exceeds those covered by WRC 107. Therefore, since it is known that the flat plate is more severe than the curved shell (for stresses) the nozzles in the shell are treated as being in a flat plate. This is believed to be conservative. Note that the bending stress is independent of a for $a \gg r$.

1" Nozzle

$$d = 30'' \quad r = .875'' \quad t = .625''$$

$$S_1 = \frac{.454 \sqrt{322^2 + 322^2}}{(.625)^2 (.875)} + \frac{.1434(128)(3.33 + 4.33 \ln \frac{30}{.875})}{(.625)^2}$$

$$= 1,480 \text{ psi Normal}$$

$$= 2,960 \text{ psi Upset}$$

$$= 3,700 \text{ psi Faulted}$$

$$S_2 = 7,085 \text{ psi Normal; } 10,856 \text{ psi Upset, and } 14,630 \text{ psi Faulted due to internal pressure.}$$

Thus:

$$S_b = 1480 + 7085 = 8,565 \text{ psi Normal}$$

$$= 13,816 \text{ psi Upset}$$

$$= 18,330 \text{ psi Faulted}$$

$$V = \frac{322}{2(3.142)(.875)^2(.625)} + \frac{\sqrt{128^2 + 128^2}}{3.142(.875)(.625)} = 212 \text{ psi Normal}$$

$$= 424 \text{ psi Upset}$$

$$= 530 \text{ psi Faulted}$$

$$S = \sqrt{(2(212))^2 + (8565)^2} = 8,575 \text{ psi Normal}$$

$$= 13,842 \text{ psi Upset}$$

$$= 18,361 \text{ psi Faulted}$$

The allowable stress per Appendix XIII is $3(18600) = 55,800$ psi for the total localized stress for Normal and Upset and 66,960 psi Faulted.

3" Nozzle (in shell)

An 8" O.D. x $\frac{1}{4}$ " reinforcing pad is used.

$$r_o = 1.75'' \quad d = 4.75'' \quad t = .5''$$

$$S_1 = \frac{.454 \sqrt{3448^2 + 3448^2}}{(.5)^2 (1.75)} + \frac{.1434(446)(3.33 + 4.33 \ln \frac{4.75}{.5})}{(.5)^2}$$

$$= 7018 \text{ psi Normal; } 14,036 \text{ psi Upset; and } 17,545 \text{ psi Faulted}$$

The pressure stress is 0 psi Normal, 5000 psi Upset, and 10,000 psi Faulted

for this nozzle. Thus:

$$S_b = 7018 \text{ psi Normal; } 19,036 \text{ psi Upset, and } 27,545 \text{ Faulted}$$

$$V = \frac{3448}{2(3.142)(1.75)^2(.5)} + \frac{\sqrt{446^2 + 446^2}}{3.142(1.75)(.5)} = \begin{array}{l} 588 \text{ psi Normal} \\ 1176 \text{ psi Upset} \\ 1470 \text{ psi Faulted} \end{array}$$

$$S = \sqrt{(2(588))^2 + (7018)^2} = \begin{array}{l} 7,116 \text{ psi Normal} \\ 19,181 \text{ psi Upset} \\ 27,701 \text{ psi Faulted} \end{array}$$

At edge of Pad

$$S_1 = \frac{.454 \sqrt{3448^2 + 3448^2}}{(.25)^2 (4)} + \frac{.1434(446)(3.33 + 4.33 \text{ Ln } \frac{4.75}{4})}{(.25)^2}$$

= 13,024 psi Normal; 26,048 psi Upset; and 32,560 psi Faulted

$$S_b = \begin{array}{l} 13,024 \text{ psi Normal} \\ 31,048 \text{ psi Upset} \\ 42,560 \text{ psi Faulted} \end{array}$$

$$V = \frac{3448}{2(3.142)(4)^2(.25)} + \frac{\sqrt{446^2 + 446^2}}{3.142(4)(.25)} = 338 \text{ psi Normal}$$

$$S = \sqrt{(2(338))^2 + (13024)^2} = \begin{array}{l} 13,042 \text{ psi Normal} \\ 31,077 \text{ psi Upset} \\ 42,594 \text{ psi Faulted} \end{array}$$

4" Nozzle (Mark A - Mark D does not require a pad)

This nozzle has a 9" O.D. x 1/4" reinforcing pad.

$$r_o = 2.25" \quad d = 4.5" \quad t = .5"$$

$$S_1 = \frac{.454 \sqrt{6440^2 + 6440^2}}{(.5)^2 (2.25)} + \frac{.1434(634)(3.33 + 4.33 \text{ Ln } \frac{4.5}{2.25})}{(.5)^2} = 9,652 \text{ psi Normal}$$

$$S_p = 0 \text{ psi Normal, } 5,000 \text{ psi Upset, and } 10,000 \text{ psi Faulted.}$$

$$S_b = 9,652 \text{ psi Normal; } 24,304 \text{ psi Upset; } 34,130 \text{ psi Faulted}$$

$$V = \frac{6440}{2(3.142)(2.25)^2(.5)} + \frac{\sqrt{634^2 + 634^2}}{3.142(2.25)(.5)} = \begin{array}{l} 659 \text{ psi Normal; } 1,318 \text{ psi} \\ \text{Upset; } 1,648 \text{ psi Faulted} \end{array}$$

$$S = \sqrt{(2(659))^2 + (9652)^2} = 9,742 \text{ psi Normal; } 24,446 \text{ Upset; and } 34,289 \text{ Faulted}$$

At Edge of Pad

$$r = 4.5" \quad t = .25"$$

$$S_1 = \frac{.454 \sqrt{6440^2 + 6440^2}}{(.25)^2 (4.5)} + \frac{.1434(634)(3.33 + 4.33 \text{ Ln } \frac{4.5}{4.5})}{4.5} = 19,543 \text{ psi Normal}$$

$$\begin{aligned}
S_b &= 19,543 \quad \text{Normal} \\
&= 44,086 \quad \text{Upset} \\
&= 58,858 \quad \text{Faulted}
\end{aligned}$$

$$V = \frac{6440}{2(3.142)(4.5)^2(.25)} + \frac{\sqrt{634^2 + 634^2}}{3.142(4.5)(.25)} = 456 \text{ psi Normal,}$$

$$\begin{aligned}
S &= \sqrt{(2(456))^2 + (19543)^2} = 19,564 \text{ psi Normal} \\
&= 44,124 \text{ psi Upset} \\
&= 58,902 \text{ psi Faulted}
\end{aligned}$$

6" Nozzle

This nozzle has a 40" O.D. x 1/2" pad.

$$r_o = 3.31" \qquad d = 24" \qquad t = .5"$$

$$S_1 = \frac{.454 \sqrt{17000^2 + 17000^2}}{(.5)^2 (3.31)} + \frac{.1434(1116)(3.33 + 4.33 \ln \frac{24}{3.31})}{(.5)^2}$$

= 20,811 psi Normal; 41,622 psi Upset; 52,028 psi Faulted

The pressure stress is zero. Thus:

$$\begin{aligned}
S_b &= 20,811 \text{ psi Normal} \\
&= 41,622 \text{ psi Upset} \\
&= 52,028 \text{ psi Faulted}
\end{aligned}$$

$$V = \frac{17000}{2(3.142)(3.31)^2(.5)} + \frac{\sqrt{1116^2 + 1116^2}}{3.142(3.31)(.5)} < 797 \text{ psi Normal}$$

$$\begin{aligned}
S &= \sqrt{(2(797))^2 + (20811)^2} = 20,872 \text{ psi Normal} \\
&= 41,744 \text{ psi Upset} \\
&= 52,180 \text{ psi Faulted}
\end{aligned}$$

At edge of pad

$$r_o = 20" \qquad t = .25"$$

$$S_1 = \frac{.454 \sqrt{17000^2 + 17000^2}}{(.25)^2 (20)} + \frac{.1434(1116)(3.33 + 4.33 \ln \frac{24}{20})}{(.25)^2}$$

= 19,279 psi Normal; 38,558 psi Upset; and 48,198 psi Faulted

The pressure stress is zero, thus $S_b = 19,279$ psi Normal.

$$V = \frac{17000}{2(3.142)(20)^2(.25)} + \frac{\sqrt{1116^2 + 1116^2}}{3.142(20)(.25)} = \begin{matrix} 128 \text{ psi Normal} \\ 356 \text{ psi Upset} \\ 320 \text{ psi Faulted} \end{matrix}$$

$$\begin{aligned}
S &= \sqrt{(2(128))^2 + (19279)^2} = 19,281 \text{ psi Normal} \\
&= 38,561 \text{ psi Upset} \\
&= 48,203 \text{ psi Faulted}
\end{aligned}$$

8" Nozzle

$$d = 36.25 \qquad r_o = 4.31 \qquad t = .625''$$

$$S_1 = \frac{.454 \sqrt{33620^2 + 33620^2}}{(.625)^2 (4.31)} + \frac{.1434(1680)(3.33 + 4.33 \text{ Ln } \frac{36.25}{4.31})}{(.625)^2}$$

= 20,560 psi Normal ; 41,120 psi Upset; and 51,400 psi Faulted

The pressure stress is 7,085 psi Normal, 10,856 psi Upset, and 14,630 psi Faulted, Thus:

$$S_b = 27,645 \text{ psi Normal}$$

$$= 51,956 \text{ psi Upset}$$

$$= 66,030 \text{ psi Faulted}$$

$$V = \frac{33620}{2(3.142)(4.31)^2(.625)} + \frac{\sqrt{1680^2 + 1680^2}}{3.142(4.31)(.625)} = 742 \text{ psi Normal}$$

$$= 1484 \text{ psi Upset}$$

$$= 1855 \text{ psi Faulted}$$

$$S = \sqrt{(2(742))^2 + (27645)^2} = 27,685 \text{ psi Normal}$$

$$= 52,041 \text{ psi Upset}$$

$$= 66,134 \text{ psi Faulted}$$

24" Nozzle 72" O.D. pad 1/2" thick

$$d = 32.25'' \qquad r = 12'' \qquad t = 1.125''$$

$$S_1 = \frac{.454 \sqrt{323800^2 + 323800^2}}{(1.125)^2 (12)} + \frac{.1434(5560)(3.33 + 4.33 \text{ Ln } \frac{32.25}{12})}{(1.125)^2}$$

= 18,481 psi Normal; 36,962 psi Upset; and 46,203 psi Faulted

The pressure stress is 7,085 psi Normal; 10,856 psi Upset; and 14,630 psi Faulted Thus:

$$S_b = 25,566 \text{ psi Normal}$$

$$= 47,818 \text{ psi Upset}$$

$$= 60,833 \text{ psi Faulted}$$

$$V = \frac{323800}{2(3.142)(12)^2(1.125)} + \frac{\sqrt{5560^2 + 5560^2}}{3.142(12)(1.125)} = 503 \text{ psi Normal}$$

$$S = \sqrt{(2(503))^2 + (25566)^2} = 25,586 \text{ psi Normal}$$

$$= 47,860 \text{ psi Upset}$$

$$= 60,885 \text{ psi Faulted}$$

At edge of pad

$$r = 36'' \qquad t = .625''$$

$$S_1 = \frac{.454 \sqrt{323800^2 + 323800^2}}{(.625)^2 (36)} + \frac{.1434(5560)(3.33 + 4.33 \text{ Ln } \frac{32.25}{36})}{(.625)^2}$$

= 20,607 psi Normal; 41,214 psi Upset; and 51,518 psi Faulted

$S_b = 27,692$ psi Normal
 $= 52,070$ psi Upset
 $= 66,148$ psi Faulted

$$V = \frac{323800}{2(3.142)(36)^2(.625)} + \frac{\sqrt{5560^2 + 5560^2}}{3.142(36)(.625)} = 175 \text{ psi Normal}$$

$$S = \sqrt{(2(175))^2 + (27692)^2} = 27,694 \text{ psi Normal}$$

$$= 52,075 \text{ psi Upset}$$

$$= 66,154 \text{ psi Faulted}$$

STRUDL 'MPC-111' 'REFUELING WATER STORAGE TANK'

LATERAL

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.....
      ICES STRUDL-11
      THE STRUCTURAL DESIGN LANGUAGE
.....
      CIVIL ENGINEERING SYSTEMS LABORATORY
      MASSACHUSETTS INSTITUTE OF TECHNOLOGY
      CAMBRIDGE, MASSACHUSETTS
      VZ MC      JUNE, 1972
      20:27:22      4/23/72
.....

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UNITS INCHES POUNDS SECONDS

TYPE PLANE PRIME

JOINT COORDINATES

1	O.	O.	SUPPORT
2	O.	O.2	
3	C.	107.	
4	C.	204.	
5	C.	309.	
6	O.	403.	
7	C.	510.	
8	O.	534.	

JOINT RELEASE

1 FORCE KY 125377134.
 1 FORCE KY 133266055.
 1 MOMENT KYZ 1522191079701.

MEMBER INCIDENCES

1	1	2
2	2	3
3	3	4
4	4	5
5	5	6
6	6	7
7	7	8

CONSTANTS

R 24000000. ALL

MEMBER PROPERTIES

1 2 PRIS AX 942. AY 942. IZ 27148361. SZ 113057.
 3 PRIS AX 734. AY 734. IZ 21714053. SZ 93743.
 4 PRIS AX 600. AY 600. IZ 19000552. SZ 77169.
 5 7 PRIS AX 471. AY 471. IZ 13571640. SZ 26549.
 6 PRIS AX 377. AY 377. IZ 10397344. SZ 45237.

DYNAMIC DEGREES OF FREEDOM

JOINTS 1 TO 8 FREE

1.0 DEGREE OF FREEDOM

APPENDIX A - COMPUTER INPUT

INERTIA OF JOINTS 2 LINEAR ALL 956.
INERTIA OF JOINTS 3 LINEAR ALL 1793.
INERTIA OF JOINTS 4 LINEAR ALL 1783.
INERTIA OF JOINTS 5 LINEAR ALL 1772.
INERTIA OF JOINTS 6 LINEAR ALL 1761.
INERTIA OF JOINTS 7 LINEAR ALL 879.
INERTIA OF JOINTS 8 LINEAR ALL 92.
DAMPING .01 5
UNITS CYCLES
STEADY RESPONSE SPECTRA ACCELERATION VS FREQUENCY *G3E*
DAMPING .01 FACTOR 336.
.4 5. .2 5.79 .13 20. .13 33.9 .13 44.1 .13 53.5 .13 70.
DYNAMIC LOADING 1 *08E LATERAL*
SUPPORT ACCELERATION
G10N X FILE *08E*
DYNAMIC ANALYSIS MODAL
OUTPUT DECIMAL 5
LIST DYNAMIC EIGENVALUES EIGENVECTORS

STRUCL 146-3114 'REFUELING WATER STORAGE TANK'

VERTICAL

1615 STRUCL-11
THE STRUCTURAL DESIGN LANGUAGE
CIVIL ENGINEERING SYSTEMS LABORATORY
MASSACHUSETTS INSTITUTE OF TECHNOLOGY
CAMBRIDGE, MASSACHUSETTS
M2 M2 JUN 1972
2014457 4773776

UNITS INCHES POUNDS SECONDS

TYPE PLANE FRAME

JOINT COORDINATES

1 C. C. SUPPORT
2 D. C.2
3 C. 102.
4 C. 204.
5 C. 306.
6 D. 408.
7 C. 510.
8 C. 514.

JOINT RELEASE

1 FIFCE KFY 125373134.
1 FIFCE KFX 102266055.
1 FIFCE KMZ 1223800597014.

MEMBER INCIDENCES

1 1 2
2 2 3
3 3 4
4 4 5
5 5 6
6 6 7
7 7 8

CONSTANTS

E 29000000. ALL

MEMBER PROPERTIES

1 2 PDIS AX 942. AY 942. IZ 27163361. SZ 113097.
3 PDIS AX 754. AY 754. IZ 21714075. SZ 53749.
4 PDIS AX 660. AY 660. IZ 19000302. SZ 79163.
5 7 PDIS AX 471. AY 471. IZ 13071610. SZ 30549.
6 PDIS AX 377. AY 377. IZ 10907304. SZ 46224.

DYNAMIC EFFECTS OF PREPROM

JOINTS 1 TO 8 DISM Y

INITIAL OF JOINTS 1 TO 8 DISM Y

8500

INERTIA OF JOINTS 2 LINEAR ALL 3744.

INERTIA OF JOINTS 3 LINEAR ALL 64.

INERTIA OF JOINTS 4 LINEAR ALL 24.

INERTIA OF JOINTS 5 LINEAR ALL 43.

INERTIA OF JOINTS 6 LINEAR ALL 39.

INERTIA OF JOINTS 7 LINEAR ALL 20.

INERTIA OF JOINTS 8 LINEAR ALL 52.

DAMPING .01 8

UNITS CYCLES

STEADY RESPONSE SPECTRA ACCELERATION VS FREQUENCY *DBE*

DAMPING .01 FACTOR 300.

.12 15. .12 15.92 .05 87.3 .09 216.5 .09 500.

DYNAMIC LOADING 2 *DBE VERTICAL*

SUPPORT ACCELERATION

DISP Y FILE *C1E*

DYNAMIC ANALYSIS MODAL

OUTPUT DECIMAL 5

LIST DYNAMIC EIGENVALUES EIGENVECTORS

LOADING 2 *DEAD WEIGHT*
JOINT LOADS
2 FOR Y 3335144.
3 FOR Y 24705.
4 FOR Y 20844.
5 FOR Y 14400.
6 FOR Y 13600.
7 FOR Y 7750.
8 FOR Y 20100.
LOADING LIST ALL
STIFFNESS ANALYSIS
OUTPUT DECIMAL 5
LIST FORCES DISPLACEMENTS

DEAD Weight

GEOMETRIC DATA SAME
AS PAGE A-1

LOADING - 1 ONE LATERAL

MEMBER FORCES

MEMBER	JOINT	RESPONSE TYPE	FORCE			MOMENT	
			AXIAL	SHEAR-Y	SHEAR-Z	TORSIONAL	BENDING-Y
1	1	RMS	0.0	612217.12500			275246552.00000
		ABS SUM	0.0	623405.31250			279559552.00000
1	2	RMS	0.0	612217.12500			275246552.00000
		ABS SUM	0.0	623405.31250			279559552.00000
2	2	RMS	0.0	196521.50000			275243264.00000
		ABS SUM	0.0	196649.37500			277156800.00000
2	3	RMS	0.0	779521.50000			154535520.00000
		ABS SUM	0.0	794049.37500			160246780.00000
3	3	RMS	2.0	727216.43750			154735680.00000
		ABS SUM	0.0	733216.31250			205749120.00000
3	4	RMS	0.0	127616.51750			130837408.00000
		ABS SUM	0.0	128326.41250			135347920.00000
4	4	RMS	0.0	601372.43750			173537184.00000
		ABS SUM	0.0	607523.56250			173743730.00000
4	5	RMS	0.0	601372.43750			55407542.00000
		ABS SUM	0.0	607523.56250			72374726.00000
5	5	RMS	0.0	417250.43750			55407716.00000
		ABS SUM	0.0	423411.25000			72374516.00000
5	6	RMS	0.0	417250.43750			17352416.00000
		ABS SUM	0.0	423411.25000			23695324.00000
6	6	RMS	0.0	167225.43750			17352448.00000
		ABS SUM	0.0	167824.50000			23695356.00000
6	7	RMS	0.0	167225.43750			223227.43750
		ABS SUM	0.0	167824.50000			311315.62500
7	7	RMS	2.0	9714.70312			223226.87500
		ABS SUM	0.0	13323.02375			311408.68750
7	3	RMS	0.0	9714.70312			223.24918
		ABS SUM	0.0	13323.02375			221.77106

RESULTANT JOINT DISPLACEMENTS- SUPPORTS

JOINT	RESPONSE TYPE	DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL RMS	0.00010	0.0				0.00002
	ABS SUM	0.00012	0.0				0.00002

RESULTANT JOINT DISPLACEMENTS- FREE JOINTS

JOINT	RESPONSE TYPE	DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
2	GLOBAL RMS	0.00716	0.0				0.00002
	ABS SUM	0.00576	0.0				0.00002
3	GLOBAL RMS	0.00716	0.0				0.00002
	ABS SUM	0.00576	0.0				0.00002
4	GLOBAL RMS	0.00013	0.0				0.00002
	ABS SUM	0.00020	0.0				0.00002
5	GLOBAL RMS	0.00716	0.0				0.00002
	ABS SUM	0.00576	0.0				0.00002
6	GLOBAL RMS	0.00716	0.0				0.00002
	ABS SUM	0.00576	0.0				0.00002
7	GLOBAL RMS	0.00716	0.0				0.00002
	ABS SUM	0.00576	0.0				0.00002
8	GLOBAL RMS	0.00716	0.0				0.00002
	ABS SUM	0.00576	0.0				0.00002

LOADING - 2

JOINT VELOCITY

MEMBER FORCES

MEMBER	JOINT	RESPONSE TYPE	FORCE			MOMENT		
			AXIAL	SHEAR-Y	SHEAR-Z	TORSIONAL	BENDING-Y	BENDING-Z
1	1	RMS	417355.56753	C.0				0.0
		ABS SUM	417323.50000	C.0				0.0
1	2	RMS	417355.56753	C.0				C.0
		ABS SUM	417323.50000	C.0				C.0
2	2	RMS	12760.33534	C.0				C.0
		ABS SUM	13016.17117	C.0				0.0
2	3	RMS	12760.33534	C.0				C.0
		ABS SUM	13016.17117	C.0				0.0
2	3	RMS	9765.61719	C.0				0.0
		ABS SUM	9999.77734	C.0				0.0
3	4	RMS	9765.61719	C.0				0.0
		ABS SUM	9999.77734	C.0				C.0
4	4	RMS	7214.75391	C.0				C.0
		ABS SUM	7413.51406	C.0				C.0
4	5	RMS	7214.75391	C.0				C.0
		ABS SUM	7413.51406	C.0				0.0
5	5	RMS	5166.30703	C.0				C.0
		ABS SUM	5320.33203	C.0				0.0
5	6	RMS	5166.30703	C.0				C.0
		ABS SUM	5320.33203	C.0				C.0
5	6	RMS	3635.05786	C.0				C.0
		ABS SUM	3607.92293	C.0				0.0
6	7	RMS	3635.05786	C.0				C.0
		ABS SUM	3607.92293	C.0				C.0
7	7	RMS	2607.13574	C.0				C.0
		ABS SUM	2607.13574	C.0				0.0
7	8	RMS	2607.13574	C.0				C.0
		ABS SUM	2607.13574	C.0				C.0

RESULTANT JOINT DISPLACEMENTS- SUPPORTS

JOINT	RESPONSE TYPE	DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL RMS	0.0	C.00662				C.0
	ABS SUM	0.0	C.00662				C.0

RESULTANT JOINT DISPLACEMENTS- FREE JOINTS

JOINT	RESPONSE TYPE	DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
2	GLOBAL RMS	0.0	C.00662				C.0
	ABS SUM	0.0	C.00662				C.0
3	GLOBAL RMS	0.0	C.00662				C.0
	ABS SUM	0.0	C.00662				C.0
4	GLOBAL RMS	0.0	C.00662				C.0
	ABS SUM	0.0	C.00662				C.0
5	GLOBAL RMS	0.0	C.00662				C.0
	ABS SUM	0.0	C.00662				C.0
6	GLOBAL RMS	0.0	C.00662				C.0
	ABS SUM	0.0	C.00662				C.0
7	GLOBAL RMS	0.0	C.00662				C.0
	ABS SUM	0.0	C.00662				C.0
8	GLOBAL RMS	0.0	C.00662				C.0
	ABS SUM	0.0	C.00662				C.0

LOADING - 3

DEAD WEIGHT

MEMBER FORCES

MEMBER	JOINT	MEMBER FORCES						
		AXIAL	FORCE SHEAR Y	SHEAR Z	TORSIONAL	MOMENT BENDING Y	BENDING Z	
1	1	-3473793.00000	0.0	0.0	0.0	0.0	0.0	
1	2	3473793.00000	0.0	0.0	0.0	0.0	0.0	
2	2	-1033579.00000	0.0	0.0	0.0	0.0	0.0	
2	3	1033579.00000	0.0	0.0	0.0	0.0	0.0	
3	3	-73373.93750	0.0	0.0	0.0	0.0	0.0	
3	4	73373.93750	0.0	0.0	0.0	0.0	0.0	
4	4	-51450.00000	0.0	0.0	0.0	0.0	0.0	
4	5	51450.00000	0.0	0.0	0.0	0.0	0.0	
5	5	-41450.00000	0.0	0.0	0.0	0.0	0.0	
5	6	41450.00000	0.0	0.0	0.0	0.0	0.0	
6	6	-27850.00000	0.0	0.0	0.0	0.0	0.0	
6	7	27850.00000	0.0	0.0	0.0	0.0	0.0	
7	7	-20097.99000	0.0	0.0	0.0	0.0	0.0	
7	8	20097.99000	0.0	0.0	0.0	0.0	0.0	

RESULTANT JOINT DISPLACEMENTS - SUPPORTS

JOINT	GLOBAL	DISPLACEMENT					
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0	0.02775	0.0	0.0	0.0	0.0

RESULTANT JOINT DISPLACEMENTS - FREE JOINTS

JOINT	GLOBAL	DISPLACEMENT					
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
2	GLOBAL	0.0	0.02777	0.0	0.0	0.0	0.0
3	GLOBAL	0.0	0.02777	0.0	0.0	0.0	0.0
4	GLOBAL	0.0	0.02777	0.0	0.0	0.0	0.0
5	GLOBAL	0.0	0.02777	0.0	0.0	0.0	0.0
6	GLOBAL	0.0	0.02777	0.0	0.0	0.0	0.0
7	GLOBAL	0.0	0.02777	0.0	0.0	0.0	0.0
8	GLOBAL	0.0	0.02777	0.0	0.0	0.0	0.0

 RESULTS OF LATEST ANALYSIS

PROBLEM - ME-311 TITLE - REFUELING WATER STORAGE TANK

LATERAL

ACTIVE UNITS INCH LB CYC DEGF SEC

ACTIVE STRUCTURE TYPE PLANE FRAME

ACTIVE COORDINATE AXES X Y

EIGENVALUES

MODE	EIGENVALUE	FREQUENCY	PERIOD
1	2.53447E3 C1	5.58726E0 C0	1.670277E-01
2	4.073741E0 C2	2.018351E0 C1	4.954535E-02
3	1.152321E0 C3	3.39453E0 C1	2.945867E-02
4	1.245027E0 C3	4.412646E0 C1	2.265701E-02
5	2.465014E0 C3	5.348746E0 C1	1.869597E-02
6	4.727077E0 C3	6.51293E0 C1	1.534032E-02
7	1.050374E0 C3	3.23674E0 C2	3.042342E-02
8	2.265955E0 C3	1.512231E0 C3	6.612527E-02

EIGENVECTORS

MODE 1

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.00273	0.0				-0.00077
2	GLOBAL	0.00300	0.0				-0.00077
3	GLOBAL	0.21274	0.0				-0.00101
4	GLOBAL	0.37547	0.0				-0.00121
5	GLOBAL	0.53723	0.0				-0.00144
6	GLOBAL	0.73724	0.0				-0.00162
7	GLOBAL	0.99911	0.0				-0.00184
8	GLOBAL	1.00000	0.0				-0.00194

MODE 2

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	-0.48120	0.0				-0.00314
2	GLOBAL	-0.48131	0.0				-0.00334
3	GLOBAL	-0.70993	0.0				-0.00555
4	GLOBAL	-0.71023	0.0				-0.00587
5	GLOBAL	-0.87792	0.0				-0.00716
6	GLOBAL	-0.89442	0.0				-0.00783
7	GLOBAL	-0.92004	0.0				-0.00835
8	GLOBAL	-1.00000	0.0				-0.00906

MODE 3

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.70727	0.0				0.00194
2	GLOBAL	0.70744	0.0				0.00194
3	GLOBAL	0.49050	0.0				0.00126
4	GLOBAL	-0.28411	0.0				0.00110
5	GLOBAL	-0.71447	0.0				0.00114
6	GLOBAL	-0.71603	0.0				0.00128
7	GLOBAL	0.70265	0.0				-0.00156
8	GLOBAL	1.00000	0.0				-0.00177

 RESULTS OF LATEST ANALYSIS

PROBLEM - ME-311 TITLE - REFUELING WATER STORAGE TANK

VERTICAL

ACTIVE UNITS INCH LB CYL DECF SEC

ACTIVE STRUCTURE TYPE PLANE FRAME

ACTIVE COORDINATE AXES X Y

EIGENVALUES

MODE	EIGENVALUE	FREQUENCY	PERIOD
1	1.535439E-02	1.597306D-01	6.280200D-02
2	1.617490E-03	3.727324E-01	1.655101D-02
3	4.130401E-03	2.145575D-01	4.610455E-02
4	1.363550E-02	3.644440E-01	2.707375E-02
5	1.353172E-02	4.344410E-01	2.047333E-02
6	1.353172E-02	5.125617E-01	1.716650E-02
7	1.152277E-02	1.052772D-01	4.456550E-02
8	1.335113E-02	1.176507D-01	8.449574E-02

EIGENVECTORS

MODE 1

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0	0.93655				
2	GLOBAL	0.0	0.75710				0.0
3	GLOBAL	0.0	0.56702				0.0
4	GLOBAL	0.0	0.37644				0.0
5	GLOBAL	0.0	0.18434				0.0
6	GLOBAL	0.0	0.00000				0.0
7	GLOBAL	0.0	0.00000				0.0
8	GLOBAL	0.0	1.00000				0.0

MODE 2

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0	-0.01286				0.0
2	GLOBAL	0.0	-0.11279				0.0
3	GLOBAL	0.0	0.12047				0.0
4	GLOBAL	0.0	0.15314				0.0
5	GLOBAL	0.0	0.00000				0.0
6	GLOBAL	0.0	0.77704				0.0
7	GLOBAL	0.0	0.77702				0.0
8	GLOBAL	0.0	1.00000				0.0

MODE 3

JOINT		DISPLACEMENT			ROTATION		
		X DISP.	Y DISP.	Z DISP.	X ROT.	Y ROT.	Z ROT.
1	GLOBAL	0.0	-0.00726				0.0
2	GLOBAL	0.0	-0.00000				0.0
3	GLOBAL	0.0	0.00000				0.0
4	GLOBAL	0.0	1.00000				0.0
5	GLOBAL	0.0	0.00000				0.0
6	GLOBAL	0.0	0.00000				0.0
7	GLOBAL	0.0	-0.00774				0.0
8	GLOBAL	0.0	-0.00000				0.0

APPENDIX C - REFERENCES

1. Roark, Raymond J., "Formulas for Stress and Strain", McGraw-Hill.
2. U. S. Atomic Energy Commission, T. I. D. 7024, Nuclear Reactors and Earthquakes.
3. A. I. S. C. Steel Plate Engineering Data - Volume 2, "Design of Plate Structures".
4. Blodgett, Omer W., "Design of Welded Structures", The James F. Lincoln ARC Welding Foundation
5. Richart, Hall, and Woods, "Vibration of Soils and Foundations", Prentice Hall, New York
6. Kraus, Harry, "Thin Elastic Shells", John Wiley & Sons, Inc.

375-65-7

APPENDIX E - FOUNDATION ANALYSIS

BY

J.H. APPLETON
PHD., P.E.

5/13/70

ANALYSIS & DESIGN OF FOUNDATION
REFUELING WATER STORAGE TANK
 Normal capacity 400,000 gal

Tank 40'-0" OD, 42'-6" high, Roof Domed 5.36'
 Foundation Allowable Bearing Pressure = 15,000 psf
 (service loads)

Reinforced Concrete Design - ACI 318-71
 $f'_c = 3000$ psi, Grade 60 reinf. for concrete

Tank Weights & Loads

Tank (not inc. bottom)	= 117.5 ^k ↓
Tank vertical seismic	= 12.8 ^k ↑ or ↓
Tank Bottom	= 16.6 ^k ↓
Tank bottom vertical seismic	= 1.8 ^k ↑ or ↓
Nozzle horizontal	= 8.9 ^k ← or →
Wind horizontal	= 47.9 ^k ← or →
Nozzle overturning	= 335.8 ^{ik}
Wind overturning	= 1145.3 ^{ik}
Tank horizontal seismic	= 799.5 ^k ← or →
Seismic overturning	= 22,937 ^{ik}
Normal water wt	= 3337 ^k ↓ (400,000 gal)
Min water wt	= 2920 ^k ↓ (350,000 gal)
Foundation wf	= 638 ^k ↓
Water vert seismic	= 300.3 ^k ↑ or ↓
Foundation vert seismic	= 57.4 ^k
Snow	= 25.1 ^k ↓
Nozzle vertical	= 12.5 ^k ↑ or ↓

5/31

(a) Normal Foundation Pressures

(Wind + nozzles + Dead weight + snow)

$$\frac{P}{A} = \frac{(117.5 + 16.6 + 3337 + 638 + 25.1 + 12.5)}{\pi(50)^2/4} = \frac{4146.7}{1963.5} = 2.11 \text{ k/ft}^2 = 2110 \text{ psf}$$

$$\frac{M_c}{I} = \frac{(335.8 + 1145.3)(25)}{\pi(50)^4/64} = \frac{(1481.1)(25)}{306,796 \text{ ft}^4} = 0.12 \text{ k/ft} = 120 \text{ psf}$$

No uplift

Max pressure = $2110 + 120 = 2230 \text{ psf} < 15,000$

(b) Upset (Normal + OBE)

Max down

$$\frac{P}{A} = \frac{(117.5 + 12.8 + 16.6 + 1.8 + 3337 + 300.3 + 638 + 57.4 + 25.1 + 2 \times 12.5)}{1963.5} = \frac{4531.5}{1963.5} = 2.31 \text{ k/ft}^2 = 2310 \text{ psf}$$

$$\frac{M_c}{I} = \frac{(2 \times 335.8 + 1145.3 + 22,937)(25)}{306,796} = \frac{2,4753.9(25)}{306,796} = 2.02 \text{ k/ft} = 2020 \text{ psf}$$

Max down = $2310 + 2020 = 4330 < 15,000$

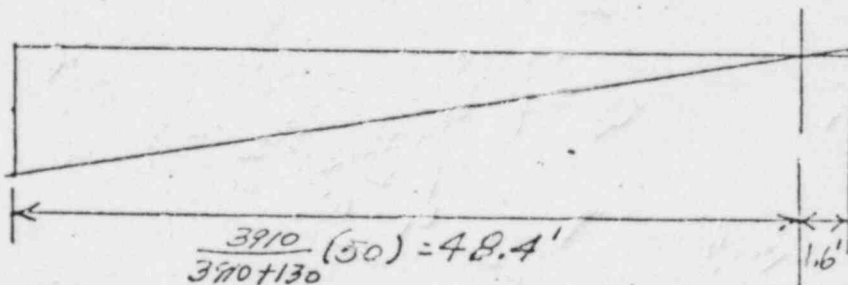
Seismic up & no snow

$$\frac{P}{A} = \frac{(117.5 - 12.8 + 16.6 - 1.8 + 3337 - 300.3 + 638 - 57.4 - 2 \times 12.5)}{1963.5} = \frac{3711.8}{1963.5} = 1.89 \text{ k/ft}^2 = 1890 \text{ psf}$$

$$\frac{M_c}{I} = 2020 \text{ psf} \uparrow$$

1890	1890
2020	2020
3910	130

Uplift area negligible with stresses for below ($3910 < 15,000$) allowable



Make check for next loading condition

10
5/3/1

(c) Faulted (Normal + SSE)

Max down

$$\frac{P}{A} = \frac{(117.5 + 2 \times 12.8 + 16.6 + 2 \times 1.8 + 3337 + 2 \times 300.3 + 638 + 2 \times 57.4 + 25.1 + 2.5 \times 12.5)}{1963.5}$$

$$= \frac{4910}{1963.5} = 2.50 \text{ k/ft} = 2500 \text{ psf}$$

$$\frac{M_c}{I} = \frac{(335.8 \times 2.5 + 1145.3 + 2 \times 22,937)(2.5)}{306,796} = 3.90 \text{ k/ft} = 3900 \text{ psf}$$

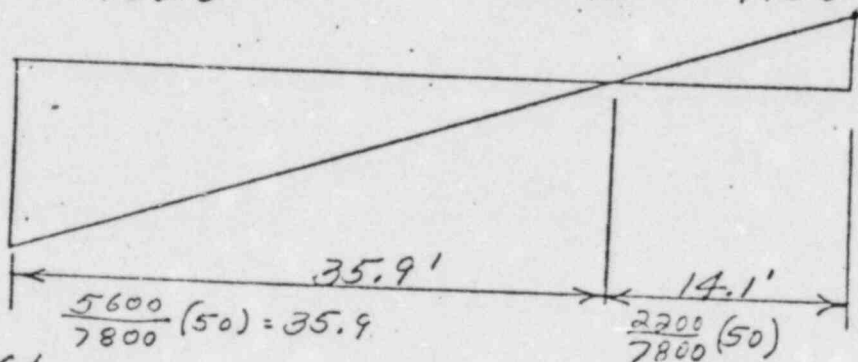
Max down = 2500 + 3900 = 6400 psf < 15,000 psf

Seismic up with no snow

$$\frac{P}{A} = \frac{(117.5 - 2 \times 12.8 + 16.6 - 2 \times 1.8 + 3337 - 2 \times 300.3 + 638 - 2 \times 57.4 - 2.5 \times 12.5)}{1963.5}$$

$$= \frac{3333.2}{1963.5} = 1.70 \text{ k/ft} = 1700 \text{ psf}$$

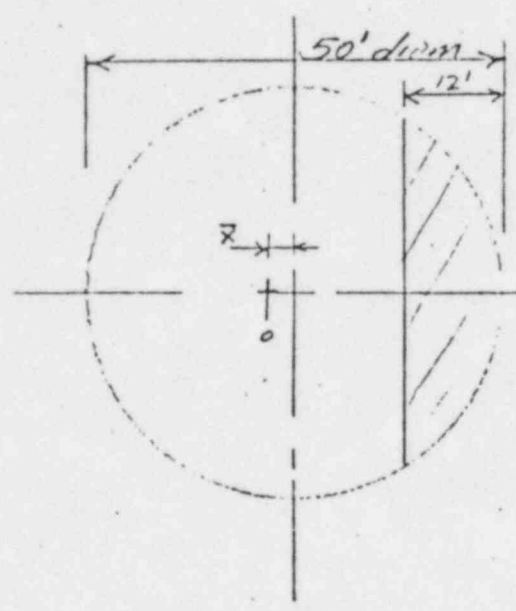
1700
3900
5600



1700
- 3900
2200

uplift occurs on 14.1' + distance from one side
check for uplift on 12'

$$\frac{38}{50} = 0.74$$



$$\bar{x} = 0.193(25) = 4.32'$$

$$A = 0.622(50)^2 = 1555 \text{ ft}^2$$

$$I_o = 0.378(25)^4 = 147,656$$

(coefficients from
chart, p. 703,
Reinforced Concrete
Fundamentals, Ferguson
Second Edition, Wiley)

For seismic up & no snow

$$\frac{P}{A} = \frac{3333.2}{1555} = 2.14 \frac{\text{K}}{\text{ft}^2} = 2140 \text{ psf}$$

$$\frac{M_c}{I} = \frac{(47859 - 3333.2 \times 4.82)(25 - 4.82)}{147,656} = 4.35 \text{ K/ft}^3$$

$$\text{or } \frac{(31,793)(25 + 4.82 - 12)}{147,656} = 3.84 \text{ K/ft}^3$$

Try uplift over 15'
 $\frac{35}{50} = 0.7$

$$\bar{x} = 0.22 \times 25 = 5.5$$

$$A = 0.59(50)^2 = 1475$$

$$I_o = 0.32(25)^4 = 125,000$$

$$\frac{P}{A} = \frac{3333.2}{1475} = 2.26 \text{ K/ft}^2$$

$$\frac{M_c}{I} = \frac{(47859 - 3333.2 \times 5.5)(25 + 5.5 - 15)}{125,000}$$

$$= 3.66 \text{ K/ft}^3$$

12
JA
5/3/72

Try uplift over 20' $\bar{x} = 0.31(25) = 7.75$
 $30/50 = 0.6$ $A = 0.49(50)^2 = 1225$
 $I = 0.2(25)^4 = 78,125$

$\frac{P}{A} = \frac{3333.2}{1225} = 2.72 \text{ k/ft}$
 $\frac{M_c}{I} = \frac{(47,959 - 3333.2 \times 7.75)(25 + 7.75 - 20)}{78,125}$
 $= 3.59 \text{ k/ft}$

Try uplift over 25' $\bar{x} = 0.42(25) = 10.5$
 $A = 0.395(50)^2 = 997.5$
 $I = 0.105(25)^4 = 41,016$

$\frac{P}{A} = \frac{3333.2}{997.5} = 3.34 \text{ k/ft}$
 $\frac{M_c}{I} = \frac{(47,959 - 3333.2 \times 10.5)(25 + 10.5 - 25)}{41,016}$
 $= 3.29 \text{ k}$

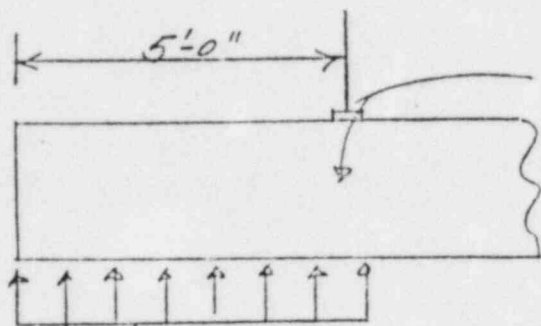
} uplift occur over 25' of slab

& $\frac{M_c}{I} = \frac{(47,959 - 3333.2 \times 10.5)(25 - 10.5)}{41,016}$
 $= 4.55 \text{ k}$

Max pressure = 4550 + 3380 = 7930 psf < 15,000

Max slab bending outside tank wall

13
JA
5/3/76



$$M = \frac{7.67(5)^2}{2} = 95.9 \text{ k}$$

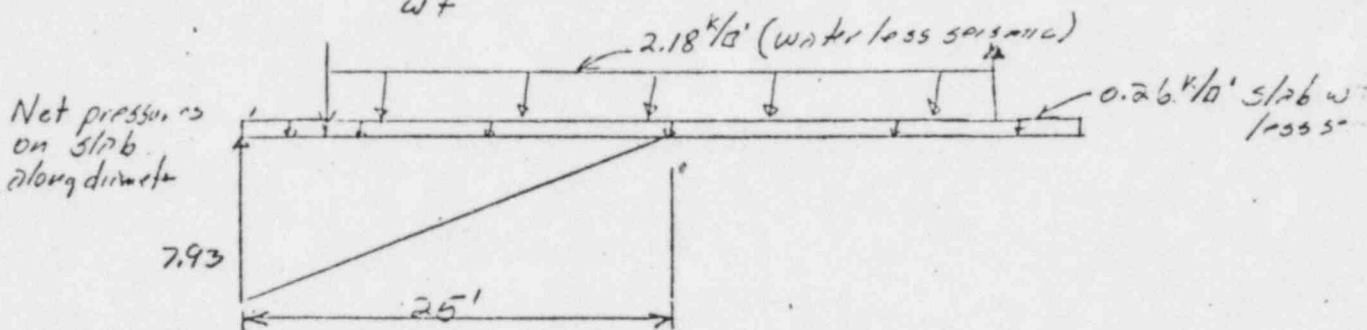
$$V \approx 7.67(5 - \frac{22}{12}) = 24.3 \text{ k}$$

see below for controlling moment

design pressure

$$7.93 \text{ k/ft} - \frac{26(15)(1-0.18)}{12} = 7.67 \text{ k/ft}$$

slab wt



$$\text{ULT } M_{\text{across}} = 47,859 \left(\frac{1}{2}\right) - 2.18 \left(\frac{1}{2}\right) (\pi) (20)^2 (6.42) (20) - 0.26 \left(\frac{1}{2}\right) (\pi) (25)^2 (6.42) (25)$$

$$\text{center diameter} = 9744 \text{ k}$$

$$M/I = \frac{9744}{50} = 194.9 \text{ k/ft}$$

Design bottom reinforcement for 194.9 k service

$$\text{bott } A_s = \frac{194.9 (1.7) (0.75) (12)}{0.9 (60) (6.9) (26 - 4.4)}$$

reduction factor when seismic loads are included
ACI 318-71
Sec 9.3

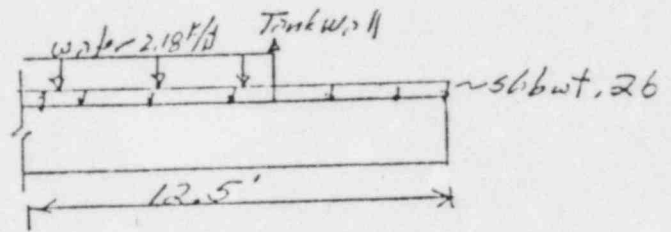
$$= 2.84 \text{ in}^2/\text{ft} \text{ use } \#11 @ 6\frac{1}{2} \text{ each way}$$

do not splice, use full length bars

$$\text{top } A_s = \#8 @ 6\frac{1}{2} \text{ each way}$$

14
JH
5/2/10

M across chord
12.5' from diametric
uplift side

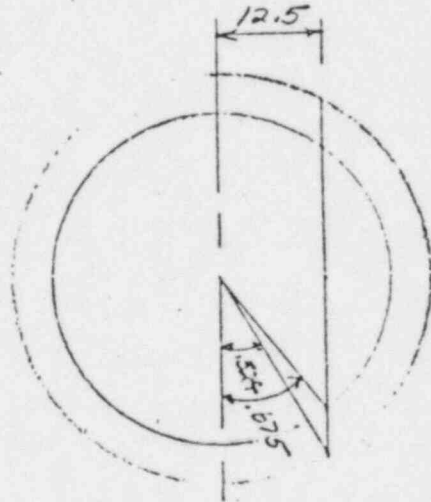


Tank wall forces

$$F = \frac{(1175 - 2 \times 12.5 - 2.5 \times 12.5)}{\pi(40)} + \frac{(335.2 \times 2.5 + 1195.3 + 2 \times 22,937)y}{\pi(40)^3/16}$$

(y measured from diametric)

$$= 0.48 \text{ k/ft } \downarrow + 3.91 y$$



$$M_{12.5} = -0.26 \int_{12.5}^{25} (625 - y^2)^{1/2} (2)(y - 12.5) dy - 2.18 \int_{12.5}^{20} (400 - y^2)^{1/2} 2(y - 12.5) dy$$

$$+ 2 \int_{0.675}^{1.571} [0.48 + 3.91(20 \sin \theta)] 20 \sin \theta d\theta$$

$$= -0.26 \left[-2 \frac{(625 - y^2)^{3/2}}{3} \right]_{12.5}^{25} - 0.26(-12.5)(2) \left[\frac{y \sqrt{625 - y^2}}{2} + \frac{625}{2} \sin^{-1} \frac{y}{25} \right]_{12.5}^{25}$$

$$+ 2 \left[0.48(20) \cos \theta \right]_{0.675}^{1.571} + (3.91(20)(20) \left(-\frac{1}{2} \sin \theta \cos \theta + \frac{1}{2} \theta \right) \right]_{0.675}^{1.571}$$

$$- 2.18(2) \left[-\frac{(400 - y^2)^{3/2}}{3} \right]_{12.5}^{20} - 2.18(2)(-12.5) \left[\frac{y \sqrt{400 - y^2}}{2} + \frac{400}{2} \sin^{-1} \frac{y}{20} \right]_{12.5}^{20}$$

15
JA
5/4/

$$M_{12.5 \text{ uplift end}} = -1759 + 1248 - 5531 + 4445 + 15 + 372 + 1197 - 514 = -527 < 9744 \text{ at center diameter}$$

$$M_{12.5 \text{ pressure end}} = -1759 + 1248 - 5531 + 4445 - 15 - 372 + 1197 + 514 + 2 \int_{0.524}^{1.571} [3.38 + 0.314(2.5 \sin \theta - 10.5)] [2.5 \sin \theta - 10.5] [2(25)(\cos \theta)] d\theta$$

$$= -2667 + 2 \int_{0.524}^{1.571} [4225 \sin \theta \cos \theta - 1774.5 \cos \theta + 9812.5 \sin^2 \theta \cos \theta - 4121.2 \sin \theta \cos \theta + 4121.2 \sin \theta \cos \theta + 1731 \cos \theta] d\theta$$

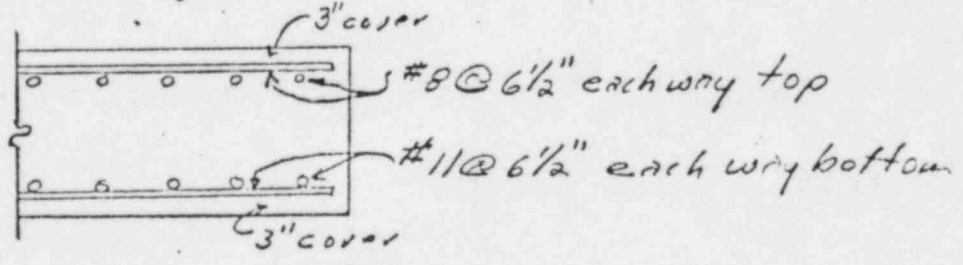
$$= -2667 + \left[2 \left[\frac{4225 \sin^2 \theta}{2} \right]_{0.524}^{1.571} + 2 \left[1774.5 \sin \theta \right]_{0.524}^{1.571} + \left[2(9812.5) \frac{\sin^3 \theta}{3} \right]_{0.524}^{1.571} - \left[4121.2(4) \frac{\sin^2 \theta}{2} \right]_{0.524}^{1.571} + \left[2(1731) \sin \theta \right]_{0.524}^{1.571} \right]$$

$$= -2667 + 3167 + 1773 + 5722 - 6179 + 1730$$

$$= 3546 \text{ K} \div 2(25 \cos .524) = 81.9 \text{ ft}^6/\text{ft}$$

9744 at center diameter

M at center diameter governs



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ADDENDUM NO. 1 to REPORT ME-311

July 11, 1977

This addendum is issued to cover the change in anchor bolt material from SA-325 to Specification SA-320 with SA-193 Grade B7 bolts.

The SA-320 Spec with SA-193 Grade B7 bolts is acceptable based on either/or the following two criteria:

- 1) The SA-320 with SA-193 Grade B7 bolts is a higher grade material than the SA-325 and it has higher yield and ultimate strengths. The mechanical properties are at least equivalent to the SA-325 bolts.
- 2) The SA-320 with SA-193 Grade B7 bolts has a higher allowable stress than the SA-325 bolts when the criteria of ASME Code Case 1644-5 is applied to both bolts. The SA-320 with SA-193 Grade B7 bolts has allowable stresses (for both shear and tensile stresses) given by ASME Code Case 1644-5 which exceeds the actual stresses in the bolts.

Prepared by:

C. K. McDonald

C. K. McDonald, P.E.

7/11/77

Date