

Calculation Cover Sheet



Contract No. 5057-06

Discipline MKES

Calc. No. 12-624-02-02

No. of Sheets 39 + App. A + B + C

Project

UMTRA - LOWMAN

Feature

EMBANKMENT DESIGN

Item

SLOPE STABILITY ANALYSIS

Sources of Data

1. MKE, "UMTRA Design Procedures Manual", July 1983. Revised Jan. 1988 (Rev. 6). MKE Doc. No. 4005-GEN-CR-01-00571-06
2. Siegel, Ronald A., "STABL User Manual"
Carpenter, James R., "STABIL/PC STABLES User Manual" Purdue University, School of Civil Engineering

Sources of Formulae & References

3. U.S. Department of Energy, "Conceptual Design Package for Stabilization of the Inactive Uranium Mill Tailings Site at Lowman, Idaho", May 1989. MKE Doc. No. 5057-LOW-R-13-00183-00
4. MKE, "UMTRA-Lowman, Embankment Design, Material Properties Calc.". MKE Calc. No. 12-624-01-00
5. Duncan, J. H., and Bushnoori, A. L., "An Engineering Manual for Slope Stability Studies". UC Berkeley, Department of Civil Engineering, March 1975.

9102080294 910205
PDR WASTE
WM-43 PDR

Preliminary Calc.

Final Calc.

Supersedes Calc. No. _____

Rev No	Revision	Calculation By	Date	Checked By	Date	Approved By	Date
02	App C added	P. Y. LI	11/27/90	W. Y. LI	11/27/90	[Signature]	11/27/90
01	4-39 App E	P. Y. LI	4/17/90	[Signature]	2/13/90	[Signature]	2/15/90
00	—	P. Y. LI	2/10/89	W. Y. LI	2/10/89	[Signature]	2/10/89

Project UMTRA - LOW
 Feature Embankment Design
 Item Slope Stability Analysis Rev. Δ

Contract No. 5057-06 Sheet 1
 File No. _____
 Designed PTL Date 12/15/89
 Checked WYL Date 2/5/90
PTL Date 2/17/90
KJee Date 2/3/90

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REV
 Δ

Project UMTRA - LOW
 Feature Embankment Design
 Item Slope Stability Analysis

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Sheet 2
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 Date 2/5/90

SUMMARY & CONCLUSION

Results of slope stability analysis are as follows: -

LOADING & CONDITION	MIN. COMPUTED * FACTOR OF SAFETY	MIN. REQUIRED (a) FACTOR OF SAFETY
Short-term, Static	2.069 (3.9) ^(b)	1.3
Short-term, Seismic	1.103 (2.04)	1.1
Long-term, Static	4.098 (3.9)	1.5
Long-term, Seismic	1.599 (1.73)	1.1

The minimum calculated F.S. for each case exceeds the minimum required F.S. for the typical slope analysed herein. Therefore failure is not expected to occur.

* Failure Surfaces for all cases occur in columnar foundation soil.

(a) From UMTRA Design Manual (Ref 1)

(b) Infinite Slope Analysis

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Item Slope Stability AnalysisSheet 3
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PURPOSE:

The purpose of this calculation is to analyze the stability of the proposed tailings embankment. Short-term static, seismic (pseudo-static); and long-term static and seismic stability was evaluated.

METHODOLOGY & ASSUMPTIONS

1. The analysis was performed using the Modified Janbu method for sliding surfaces. The computer program 'STABLES' (Ref. 1) was used to determine the factors of safety. For description, coding procedures, and assumptions associated with the program, see Ref. 2.
2. Potential sliding mechanism and locations were identified as follows:
 - i) Sliding block failure surface in radioactive sand
 - ii) Sliding block failure surface in colluvium soil
 - iii) Sliding block failure surface in Alluvium I soil (silty sand to clayey sand with gravel.)
 - iv) Sliding block failure surface in Alluvium II Soil (gravelly sand with cobbles and boulders)For (i-iii) 'Block 2' method was used.
- v) Irregular shaped failure surface through all soils and





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originating in the neighborhood of the embankment toe.
The 'RANDOM' searching routine was used.

3. Loading conditions analyzed were : i) short-term, static ;
ii) short-term, seismic ; iii) long-term, static ; iv) long-term, seismic
4. It was assumed that ground-water table remained stable
for all loading cases.
5. For simplicity in coding and conservatism, it was assumed
that the erosion protection layer and radon barrier cover (each
18" thick) were one material layer with soil parameters
as those for radon barrier.
6. Shallow surface failure was analyzed using infinite slope
method for cohesionless erosion protection layer.



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CROSS-SECTION FOR ANALYSIS

A typical cross-section of the proposed embankment with longest 5:1 slope (most critical) was analysed. The section were developed from the embankment plan (sh 7) and boring data (see Ref. 3).

The groundwater elevations were also taken from Ref 3 (boring logs). It is assumed that groundwater elevations stay the same for all cases analysed.

The cross-section is presented on sh. 6.

Coordinat. system used in the analysis is also shown on sh. 6.

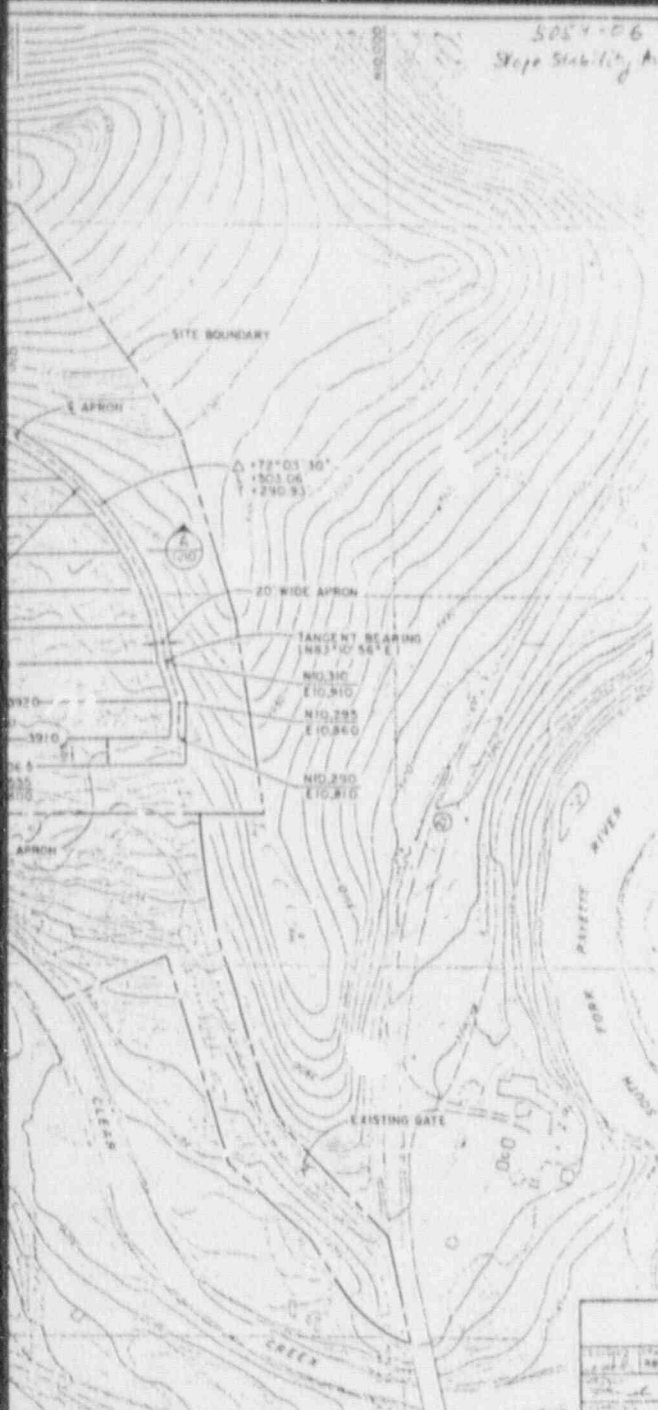
5054-P6
Slope Stability Analysis

PYL 12/14/89
WYL 3/13/90

Sh. 7

NOTES

1. THE SUBCONTRACTOR SHALL INCLUDE IN HIS WORK PLACING OF 30,000 CUBIC YARDS OF CONTAMINATED MATERIAL FROM VICINITY PROPERTIES DELIVERED TO THE SITE BY OTHERS.



SI
APERTURE
CARD

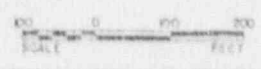
Also Available On
Aperture Card

REFERENCE DRAWINGS:

- LOW-PS-10-1203 SITE PLAN
- LOW-PS-10-1204 CONSTRUCTION FACILITIES AND SITE DRAINAGE
- LOW-PS-10-1208 CONTAMINATED MATERIAL EXCAVATION PLAN AND DEMOLITION
- LOW-PS-10-1210 TAILINGS EMBANKMENT SECTIONS

LEGEND:

- EXISTING SITE FEATURES AND CONTOURS
- FINAL CONTOURS
- CONSTRUCTION GRID COORDINATE
- STATE HIGHWAY
- EXISTING SURVEY MONUMENT (BRASS CAP)
- CONSTRUCTION SITE BOUNDARY



U. S. DEPARTMENT OF ENERGY
ALBUQUERQUE, NEW MEXICO

LOWMAN SITE
LOWMAN, IDAHO

FINAL GRADING PLAN AND RESTORATION

MOHRISON-KLUEDEN ENGINEERS, INC.
PARTNER OFFICE ENGINEERS
ULTRA PROJECT

PROJECT NO.
DE-AC04-B3AL18796

DATE: _____
PROJECT ENGINEER: _____
SITE: _____

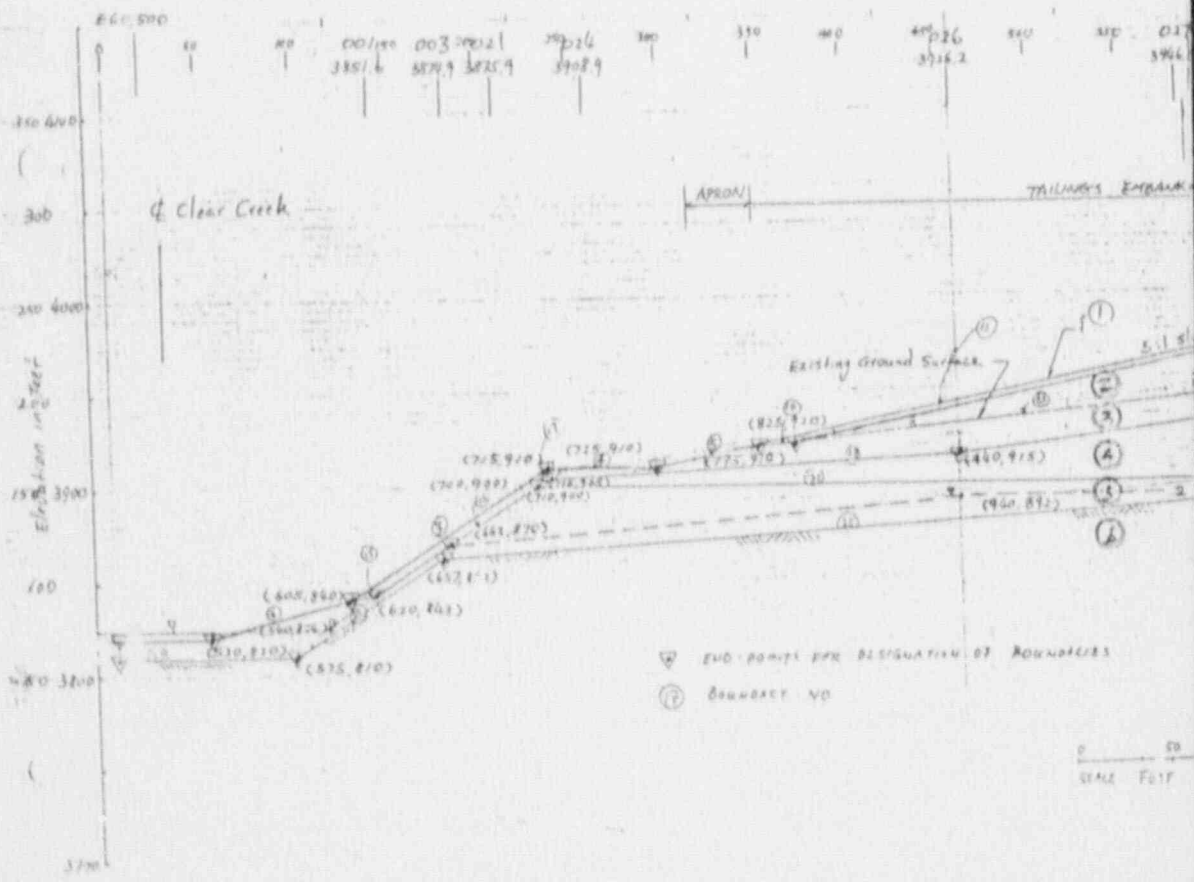
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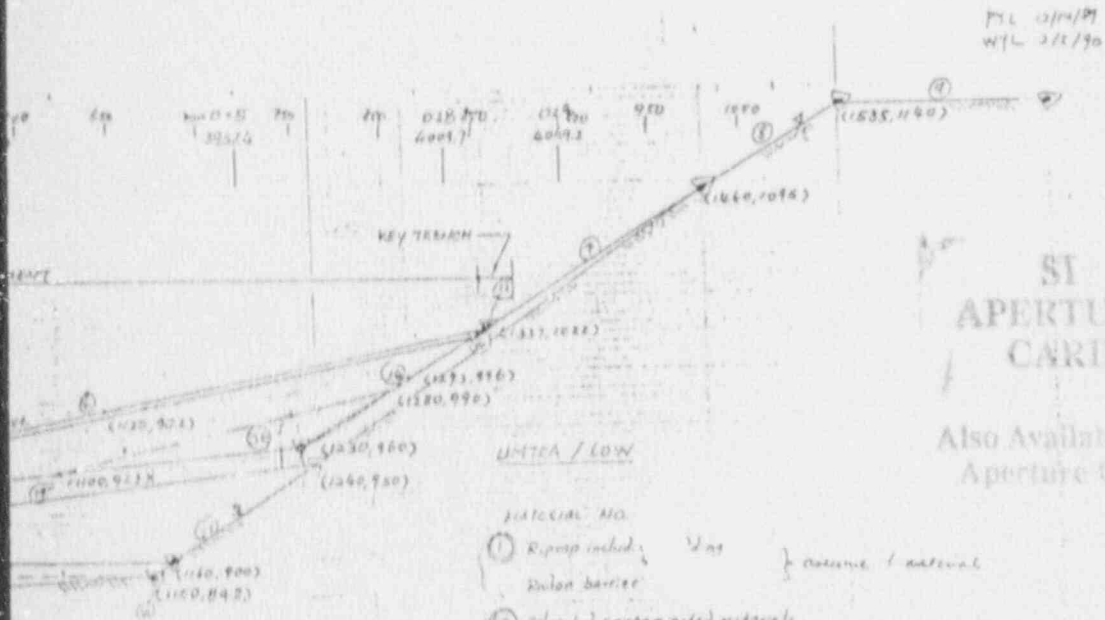
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ISSUED FOR PRELIMINARY REVIEW

REVISIONS

NO.	DATE	BY	DESCRIPTION





PL 12/1/79
W/L 2/1/90

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UNITA / LOW

MATERIAL NO.

- ① R. prop. incl. Vms } concrete / natural
Dulon barrier
- ② selected concrete/brick materials
- ③ Existing in large p.h.
- ④ Foundation soil - silty sand to clayey sand (colluvium)
- ⑤ Foundation soil - silty sand to clayey sand w/ gravel (colluvium)
- ⑥ Foundation soil - gravelly sand with cobbles and boulders (colluvium)
- ⑦ Washed bed rock

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Project VNTRA - LDW
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SOIL PARAMETERS

Soil parameters used in analysis were taken from Ref. 4 (Material Properties Calc.). A reasonable degree of conservatism had also been incorporated. The input soil parameters required in 'STABL5' are short term static and pseudo-static strengths, long term static strengths, long term pseudo-static strengths and unit weights (both total and saturated). The parameters are presented in table on sh 9.



Project UMTRAILLOW
Feature Embankment Design
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SOIL PARAMETERS FOR SLOPE STABILITY ANALYSIS

LAYER NO.	I. SHORT TERM - STATIC			II. SHORT TERM - PSEUDO-STATIC			III. LONG TERM - STATIC			IV. LONG TERM - PSEUDO-STATIC		
	C (PSF)	ϕ	$\frac{\gamma_c}{\gamma_{sat}}$	C (PSF)	ϕ	$\frac{\gamma_c}{\gamma_{sat}}$	C (PSF)	ϕ	$\frac{\gamma_c}{\gamma_{sat}}$	C (PSF)	ϕ	$\frac{\gamma_c}{\gamma_{sat}}$
① COVER LAYER	850	27°	130 / 135	850	27°	130 / 135	98	36°	130 / 135	250	21°	130 / 135
② RADIOACTIVE SAND	-	40°	144 / 155	-	40°	144 / 155	-	40°	144 / 155	-	40°	144 / 155
③ COLLUVIUM	350	17.5°	122.5 / 130.5	350	17.5°	122.5 / 130.5	245	37°	122.5 / 130.5	750	27°	122.5 / 130.5
④ ALLUVIUM I	1460	32°	124.5 / 131	1460	32°	124.5 / 131	885	37°	124.5 / 131	1460	32°	124.5 / 131
⑤ ALLUVIUM II	1460	32°	124.5 / 131	1460	32°	124.5 / 131	885	37°	124.5 / 131	1460	32°	124.5 / 131
⑥ WEATHERED BEDROCK	-	45°	137 / 143.5	-	45°	137 / 143.5	-	45°	137 / 143.5	-	45°	137 / 143.5



Project VITRA-LOW
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Date 12/14/89Date 2/5/90SEISMIC COEFFICIENTS

Horizontal seismic coefficients used in the analysis

are:

Long term conditions : $K_h = 0.23 g$

Short term conditions : $K_h = 0.17 g$

These values are from Ref 3 p 6





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SHALLOW SURFACE FAILURE ANALYSIS

Factors of safety for linear failure surfaces occurring in the cohesionless erosion protection layer were computed for each loading condition using the infinite slope method as follows:

A. SHORT-TERM & LONG-TERM, STATIC CONDITION

Erosion Protection Layer

$$F.S. = \frac{\tan \phi}{\tan \beta} \quad (\text{Ref } 5)$$



$$\tan \beta = 1/5$$

$$\phi = 38^\circ \quad (\text{Ref } 4)$$

$$\therefore F.S. = \frac{\tan 38^\circ}{0.2} = \underline{\underline{3.9}}$$

B. SHORT-TERM, SEISMIC

$$F.S. = \frac{(1 - \frac{k_h}{g} \tan \beta) \tan \phi}{\tan \beta + \frac{k_h}{g}} \quad (\text{see derivation on sl } 12)$$

$$k_h = 0.17g \quad (\text{Ref } 3)$$

$$\phi = 38^\circ$$

$$\therefore F.S. = \frac{(1 - (0.17)(0.2)) \tan 38^\circ}{0.2 + 0.17} = \underline{\underline{2.04}}$$





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C. LONG-TERM, SEISMIC

$$F.S. = \frac{(1 - \frac{k_a}{g} \tan \beta) \tan \phi'}{\tan \beta + \frac{k_a}{g}}$$

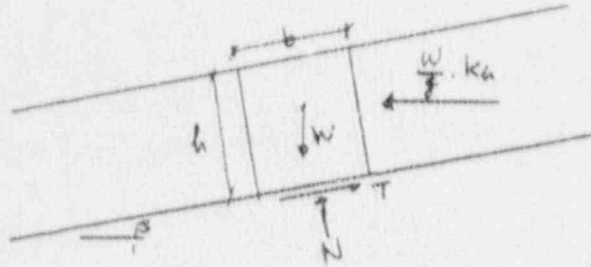
$$k_a = 0.23g \text{ (Ref. 3)}$$

$$\phi' = 38'$$

$$\therefore F.S. = \frac{[1 - (0.23)(0.2)] \tan 38}{0.2 + 0.23}$$

$$= \underline{1.73}$$

Derivation



$$\Sigma F_x = 0 \quad W \cos \beta + \frac{k_a}{g} W \sin \beta - N = 0$$

$$\Sigma F_y = 0 \quad T - W \sin \beta - \frac{k_a}{g} W \cos \beta = 0$$

$$T = \frac{N \cdot \tan \phi}{F.S.}$$

$$\therefore F.S. = \frac{(W \cos \beta + \frac{k_a}{g} W \sin \beta) \tan \phi}{W \sin \beta + \frac{k_a}{g} W \cos \beta}$$

$$= \frac{(\cos \beta + \frac{k_a}{g} \sin \beta) \tan \phi}{\sin \beta + \frac{k_a}{g} \cos \beta}$$

$$= \frac{(1 - \frac{k_a}{g} \tan \beta) \tan \phi}{\tan \beta + \frac{k_a}{g}}$$

RESULTS OF "STABLES" ANALYSIS

LOADING CONDITION	FAILURE NO.: SAND (ft)	COMPUTED FACTOR OF SAFETY				MIN. REQUIRED FACTOR OF SAFETY	REMARKS
		COLLUMPTION (a)	ALLOWIUM I (a)	ALLOWIUM II (a)	WHOLE SLOPE (b)		
SHORT TERM, STATIC	4658	2.069	4.804	4.815	4.032	1.3	SAFE
SHORT TERM, DYNAMIC	2435	1.103	2.574	2.559	2.153	1.1	SAFE
LONG TERM, STATIC	4504	4.098	5.154	5.231	4.322	1.5	SAFE
LONG TERM, DYNAMIC	2009	1.599	2.708	2.717	1.990	1.1	SAFE

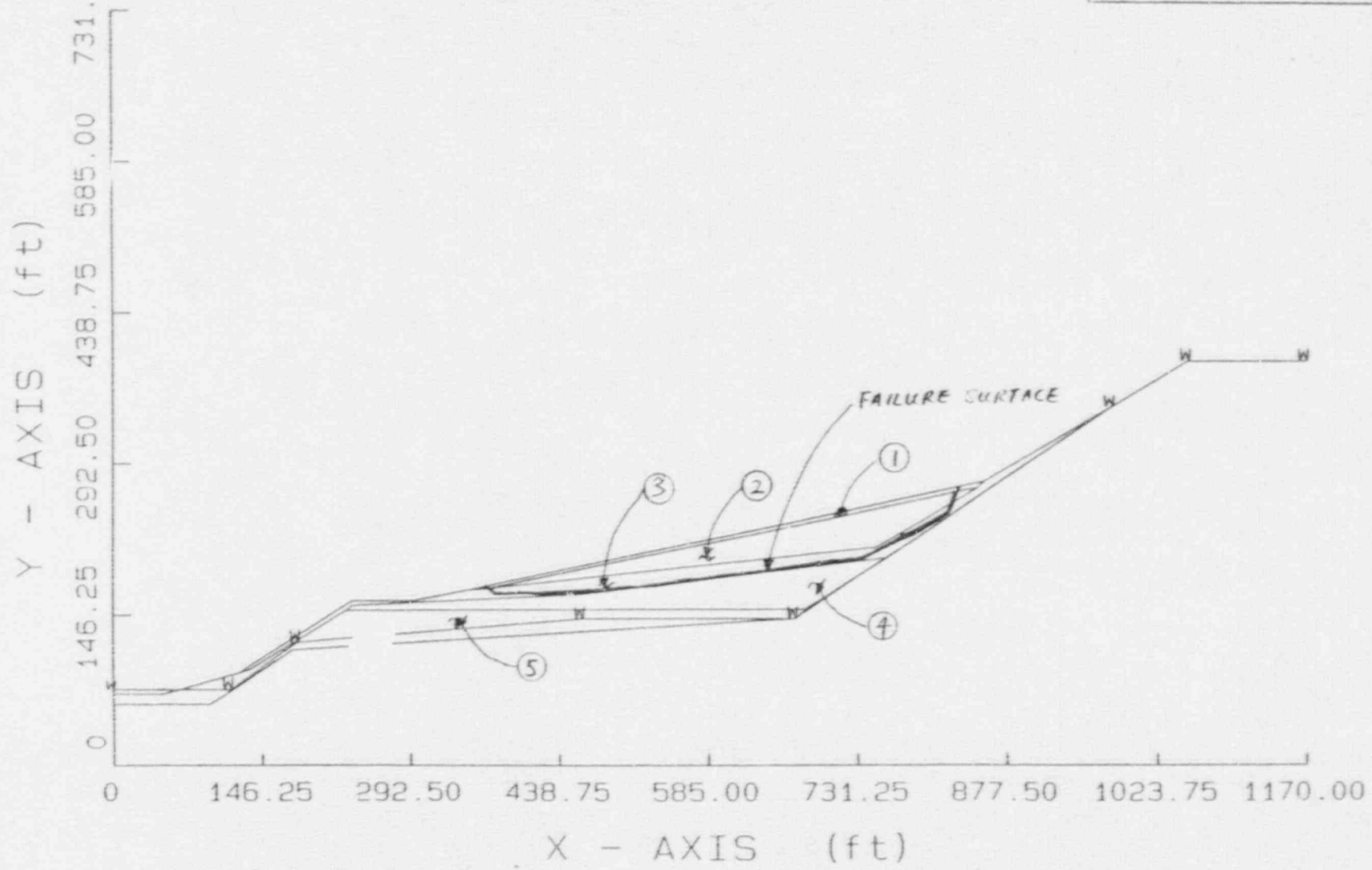
Copies of "STABLES" output with critical failure surface description and F.S. are attached as Appendix A. Failure surfaces for all of the above cases were shown on SL 2E - 33.

- (a) Sliding Block failure surface analysis
- (b) Irregular s.s. - 1 fact. s surface analysis
- (c) Distinction between Allowium I and Allowium II is described in Ref. 9 (Material Properties Calc)

MINIMUM

F.S. = 2.069

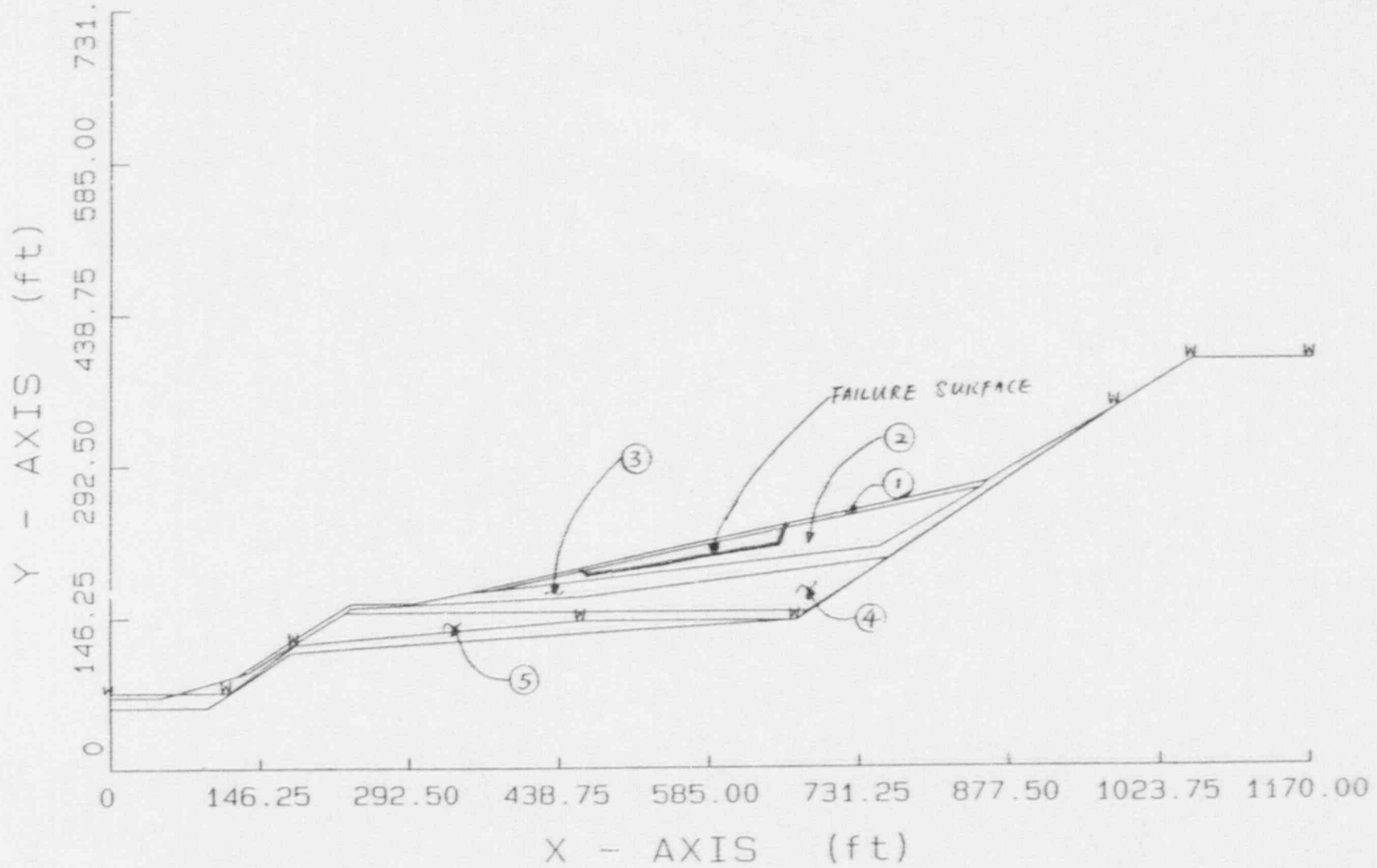
SHORT-TERM NON-SEISMIC THRU COLLUVIUM



see sh. 9 for layer description

UMTRA-LOW
SOS 7-06
PYL 12/13/87
W 2-1-193

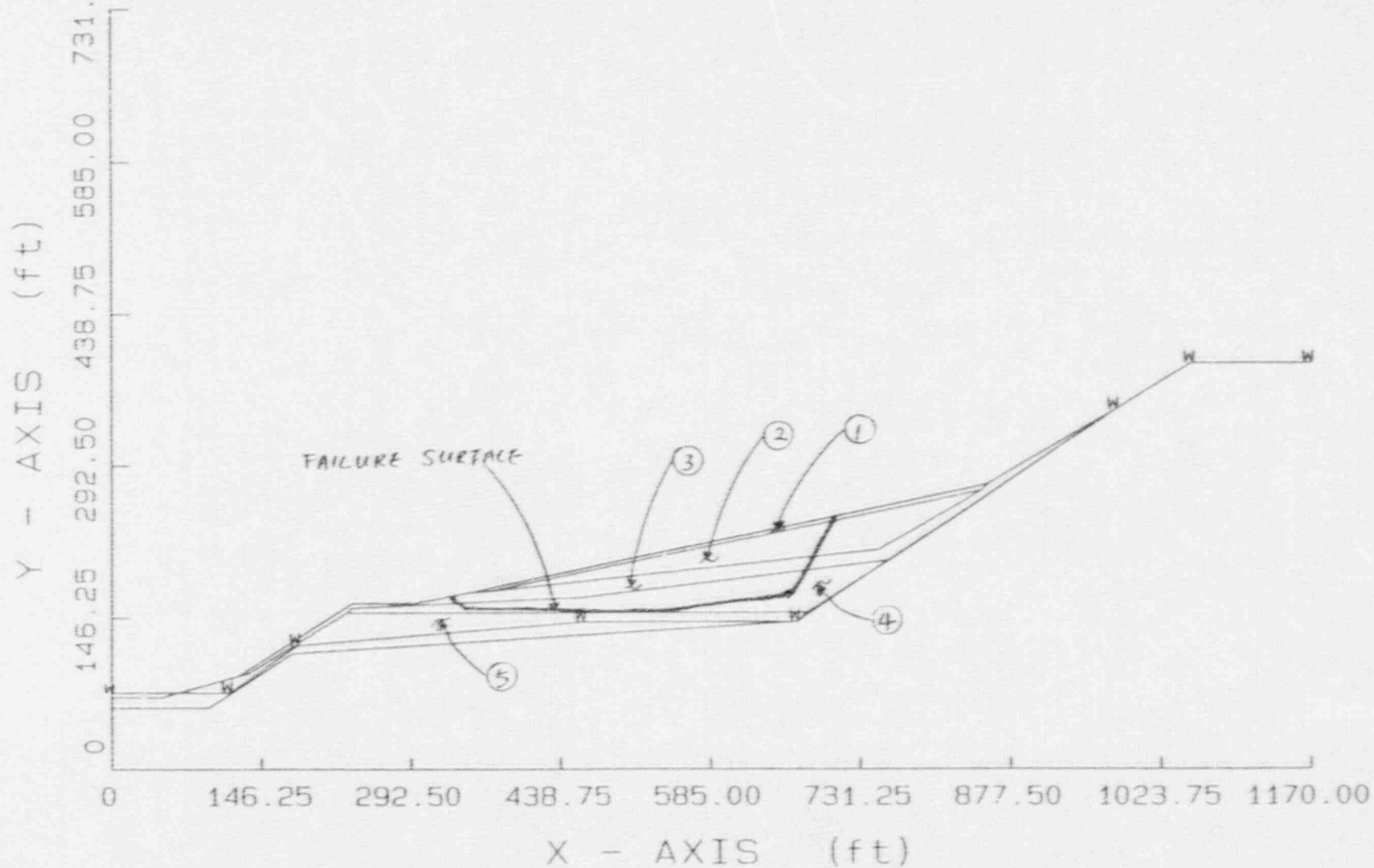
SHORT-TERM NON-SEISMIC THRU SAND F.S. = 4.658



UMTRA-LOW
5057-66
P.L. 12/13/87
W/L - 2/12/87

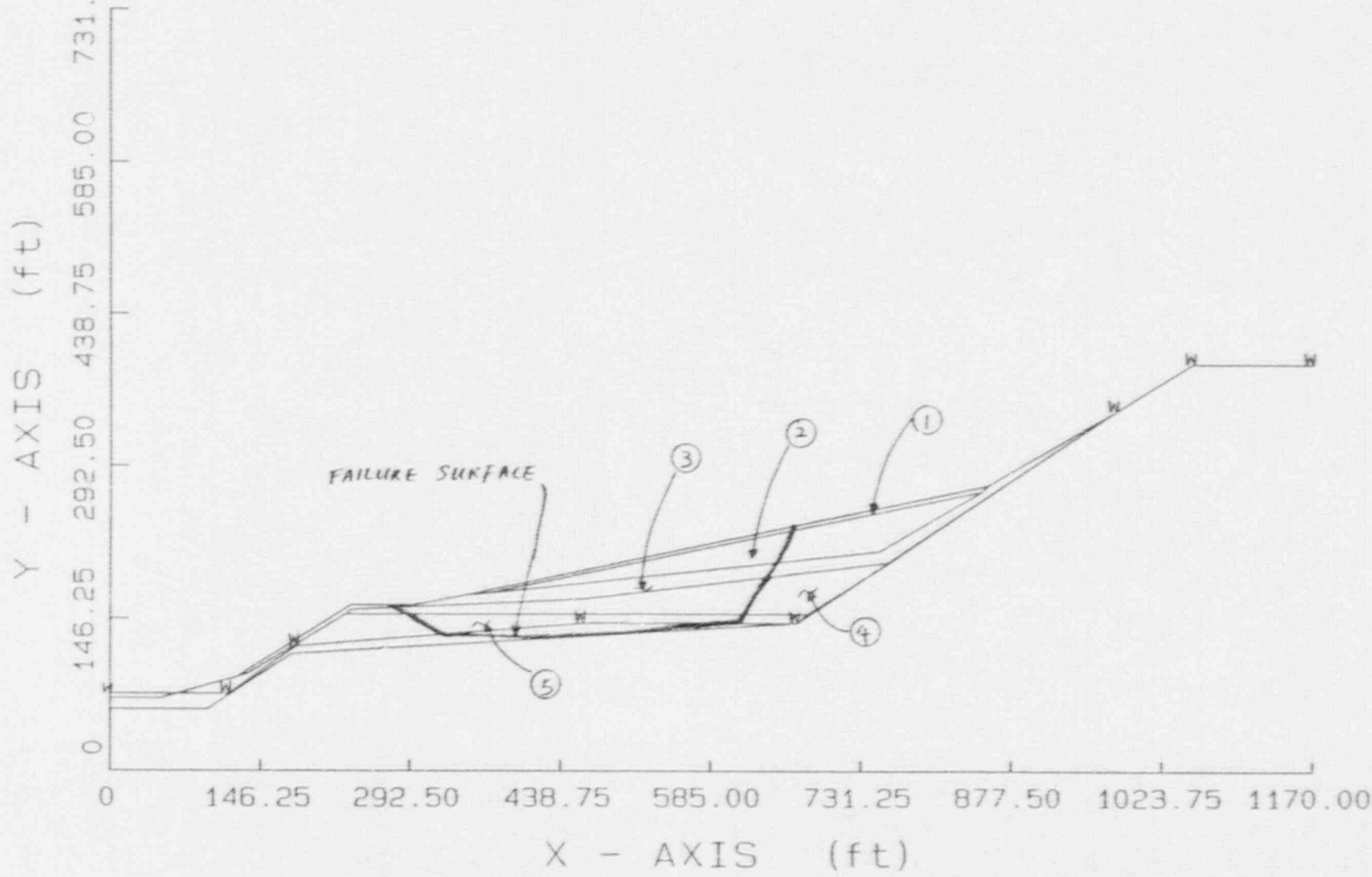
SL 15

SHORT-TERM NON-SEISMIC THRU ALLUVIUM I F.S.=4.804



66/2/8 7/M
 68/21/01 7 x d
 90-6505
 007-48617

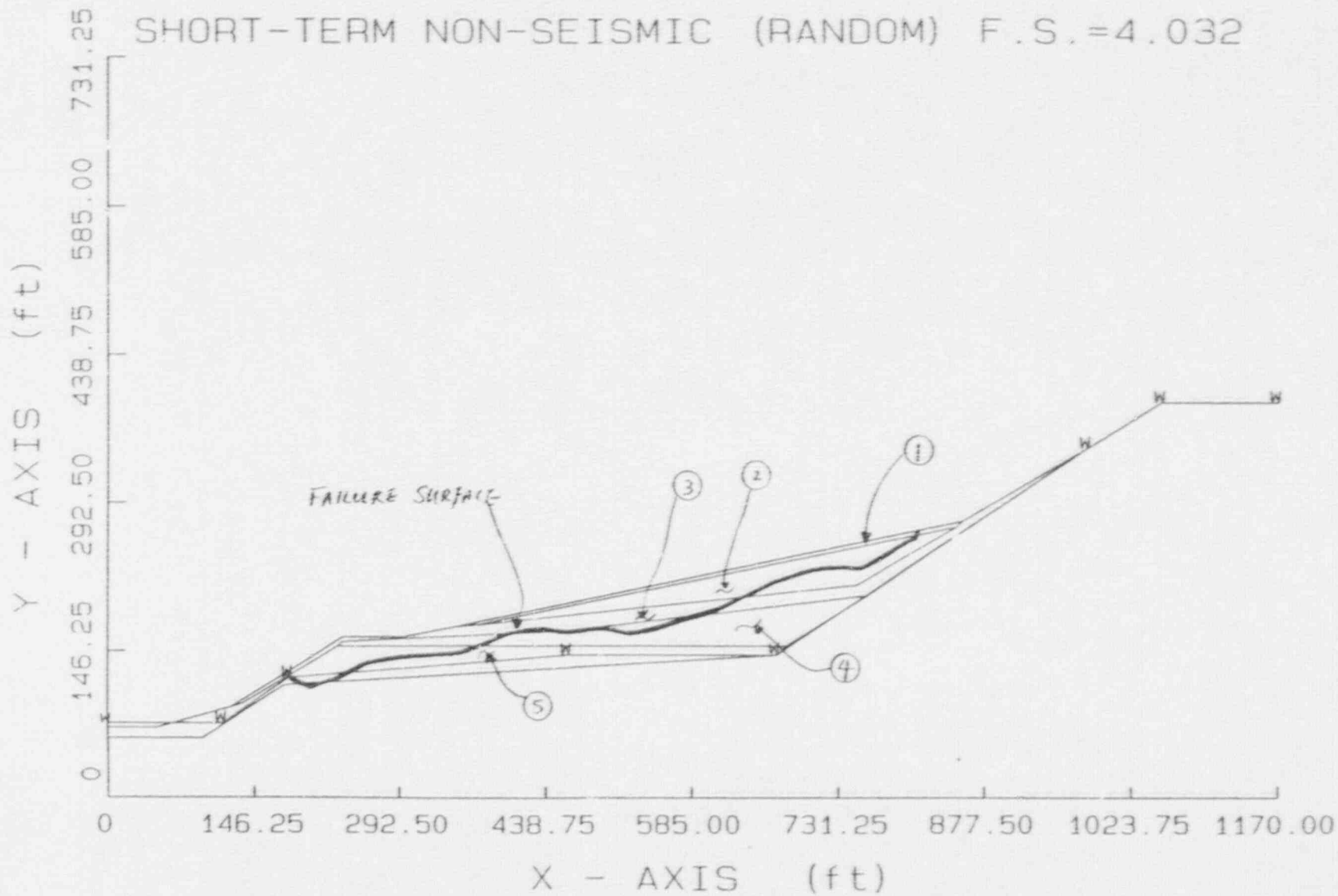
SHORT-TERM NON-SEISMIC THRU ALLUVIUM II F.S.=4.813



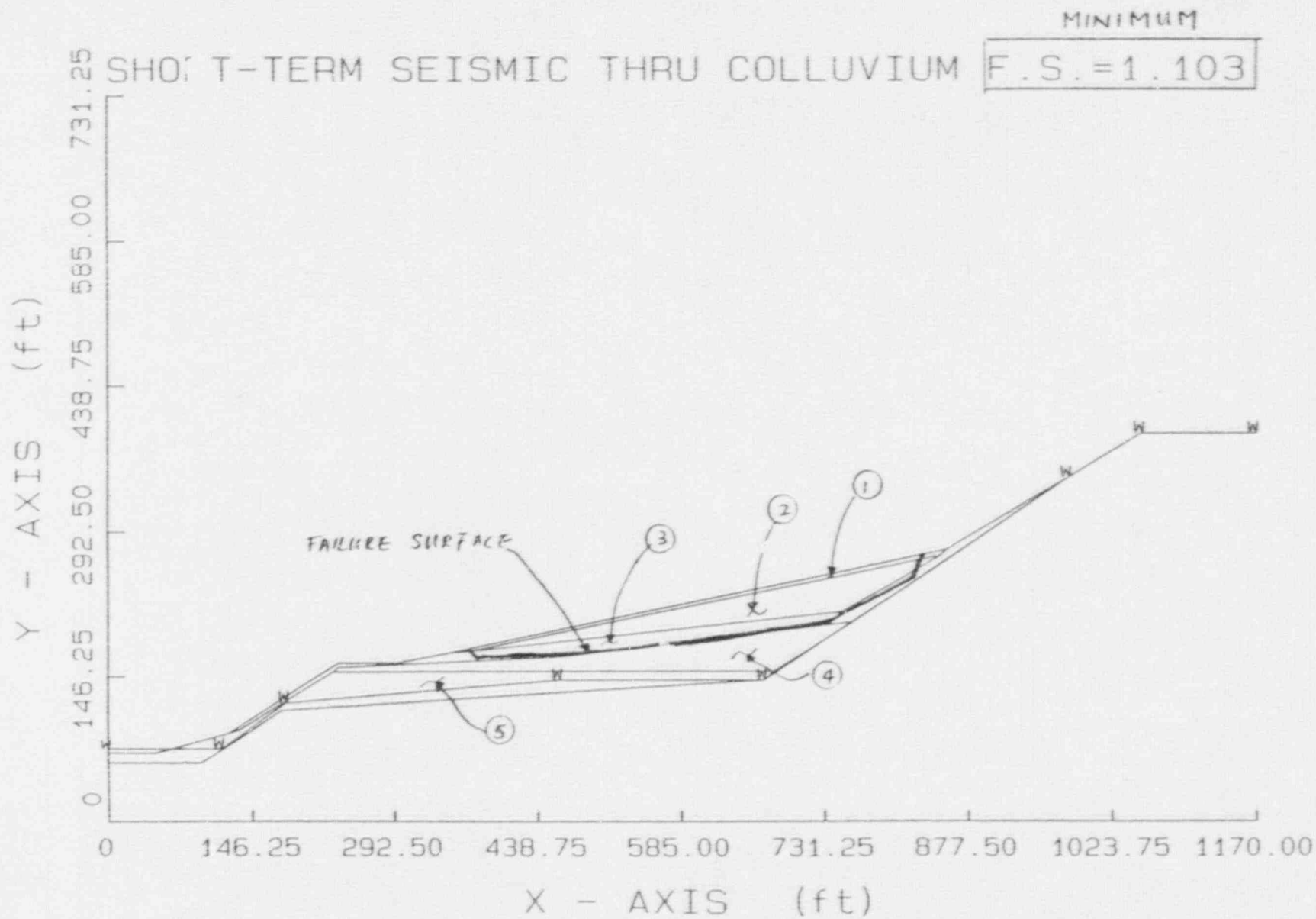
DATA - CIV
 5057-06
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 RYL 12/12/89
 W/L 2/12/90

5/1/7

SHORT-TERM NON-SEISMIC (RANDOM) F.S. = 4.032

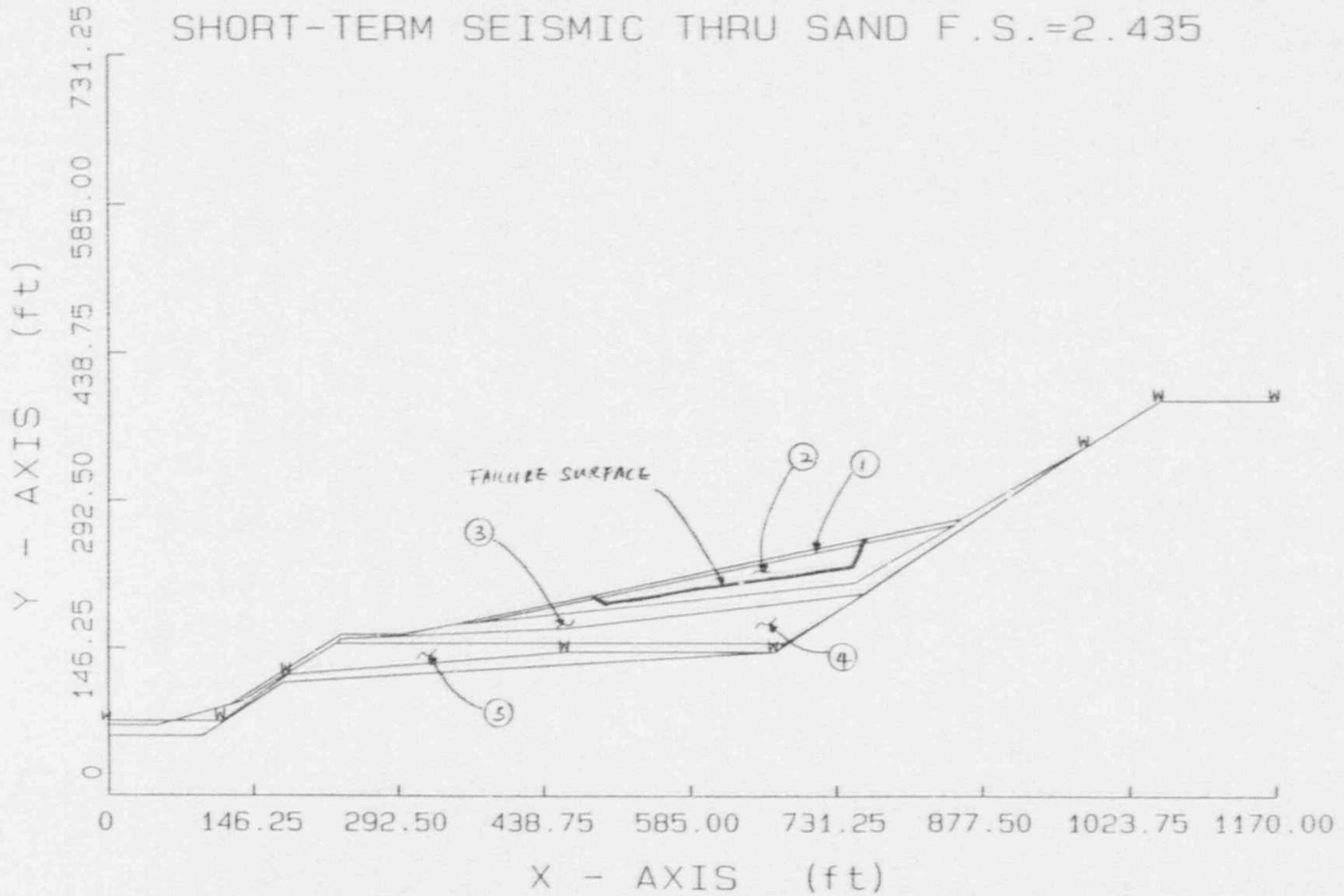


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PRC 12/3/89
W/L 2/1/90



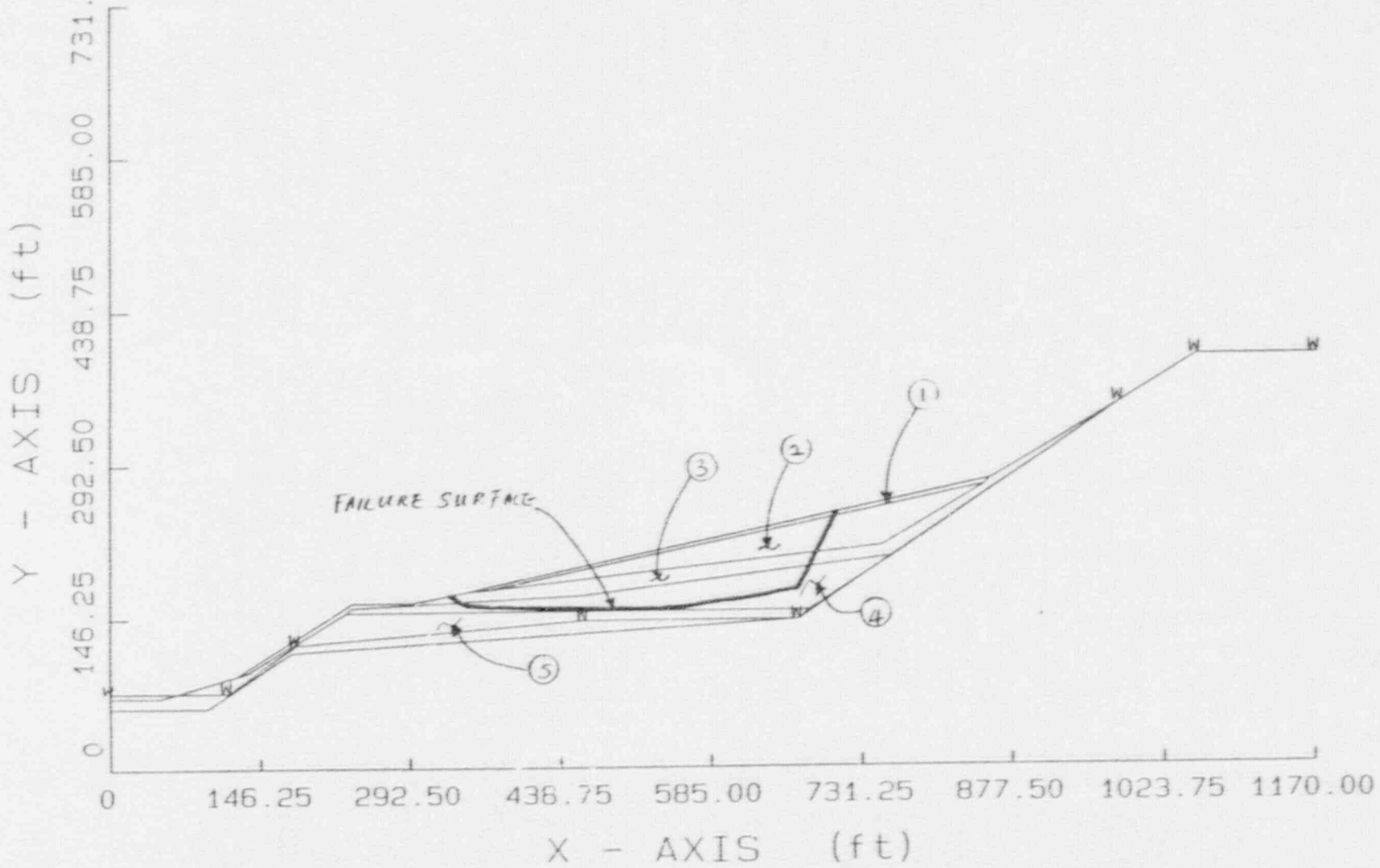
UHTRA-LOW
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 PVL 12/3/89
 WYL 2/2/90
 5h.19

SHORT-TERM SEISMIC THRU SAND F.S.=2.435



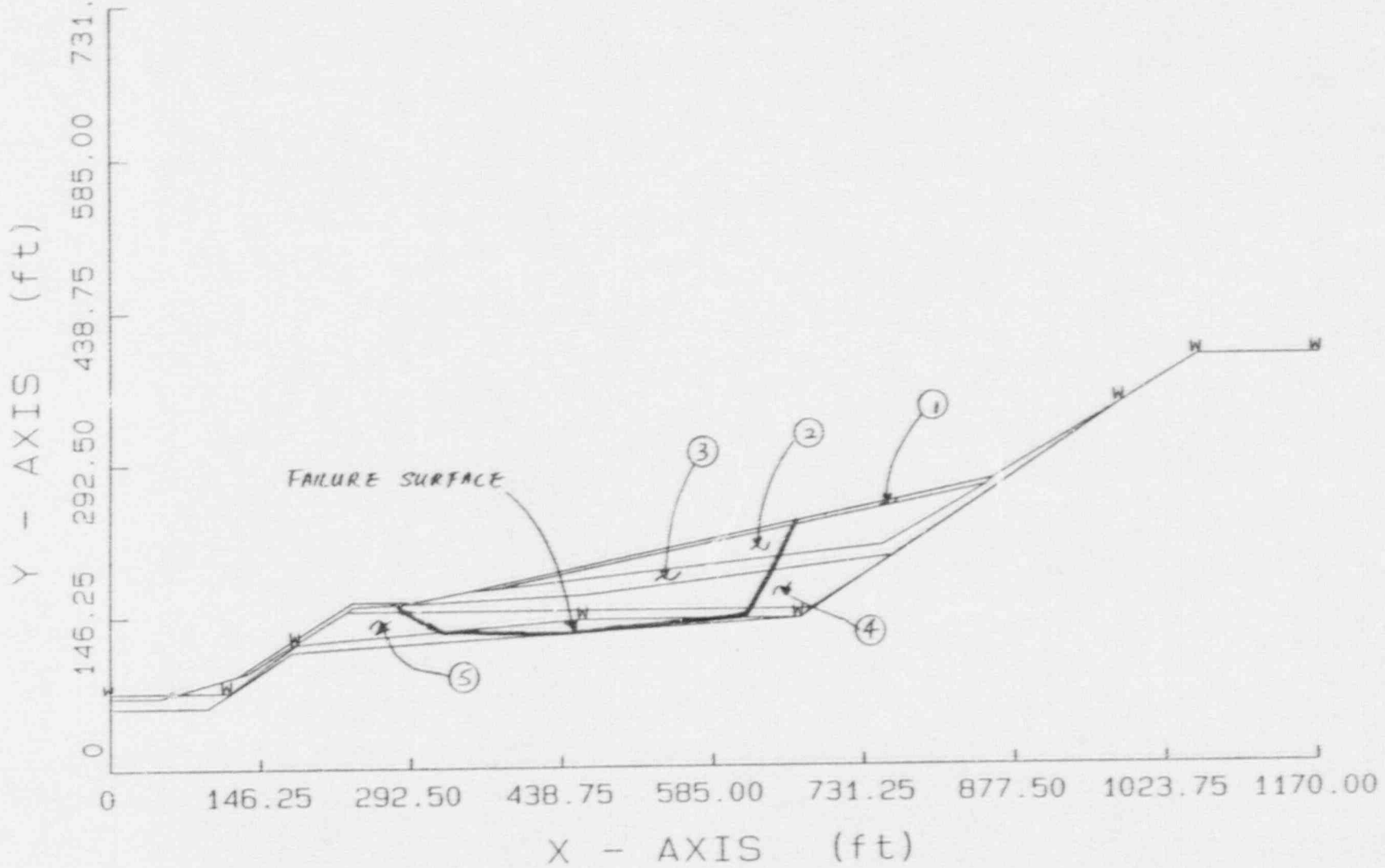
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P/L 12/13/89
W/L 2/5/90

SHORT-TERM SEISMIC THRU ALLUVIUM I F.S.=2.574



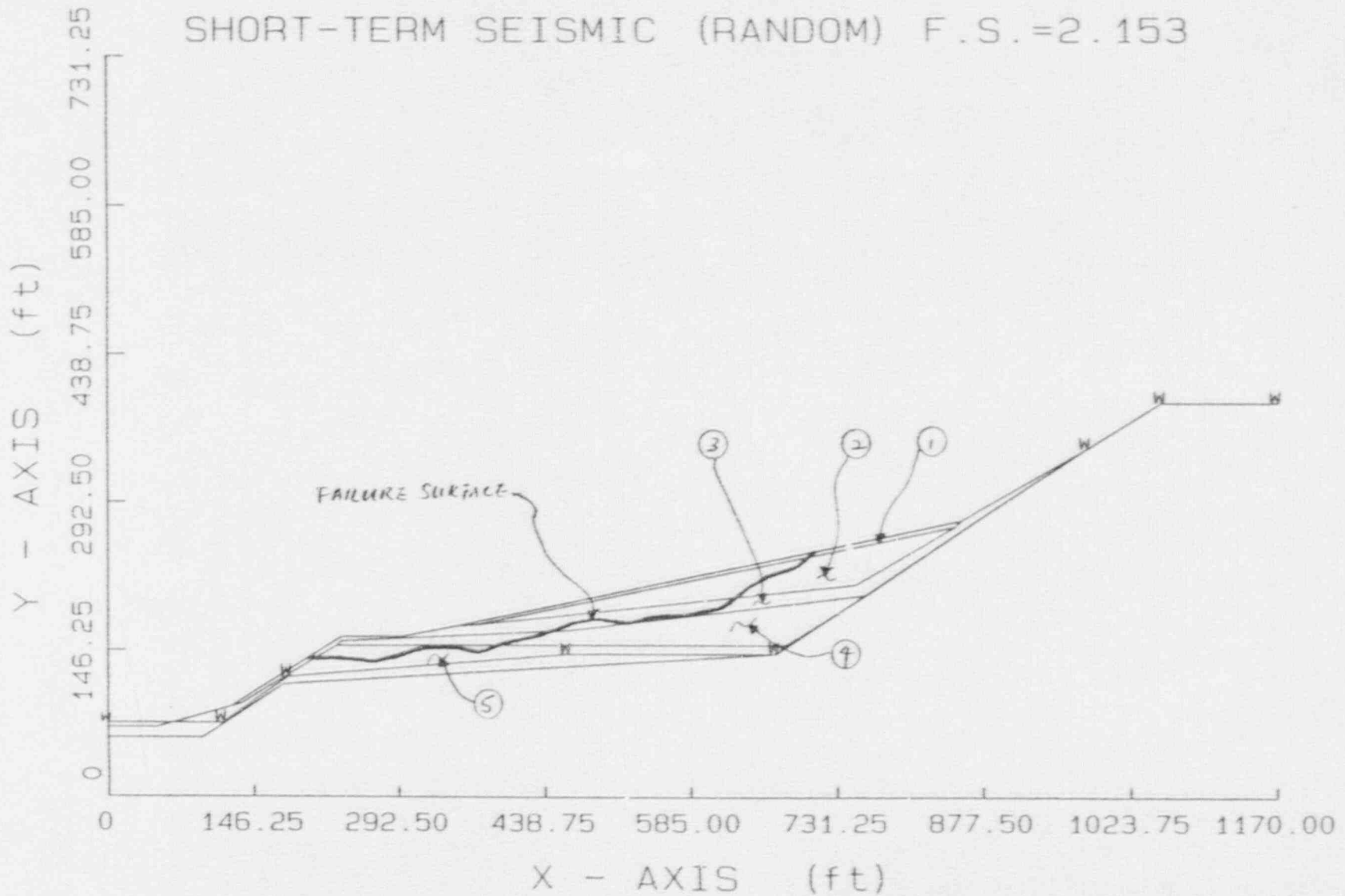
UHTRA-LOW
 5059-06
 PXL 12/13/89
 NYL 2/5/90
 Sh. 21

SHORT-TERM SEISMIC THRU ALLUVIUM II F.S.=2.559



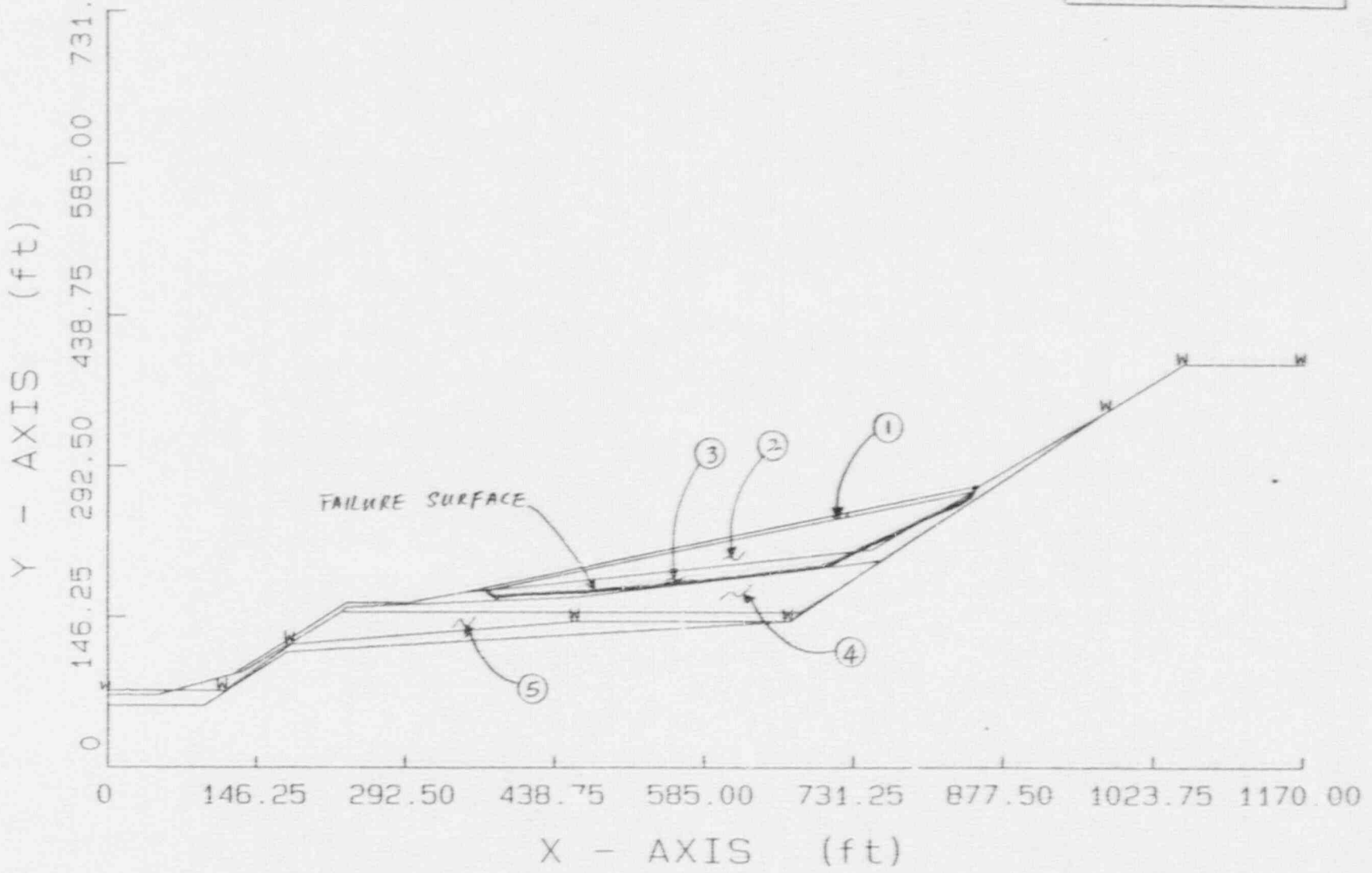
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 PTL 10/13/84
 6/8/10/1 7:20
 90-4505
 U1179A-10W
 107-V262W

SHORT-TERM SEISMIC (RANDOM) F.S.=2.153



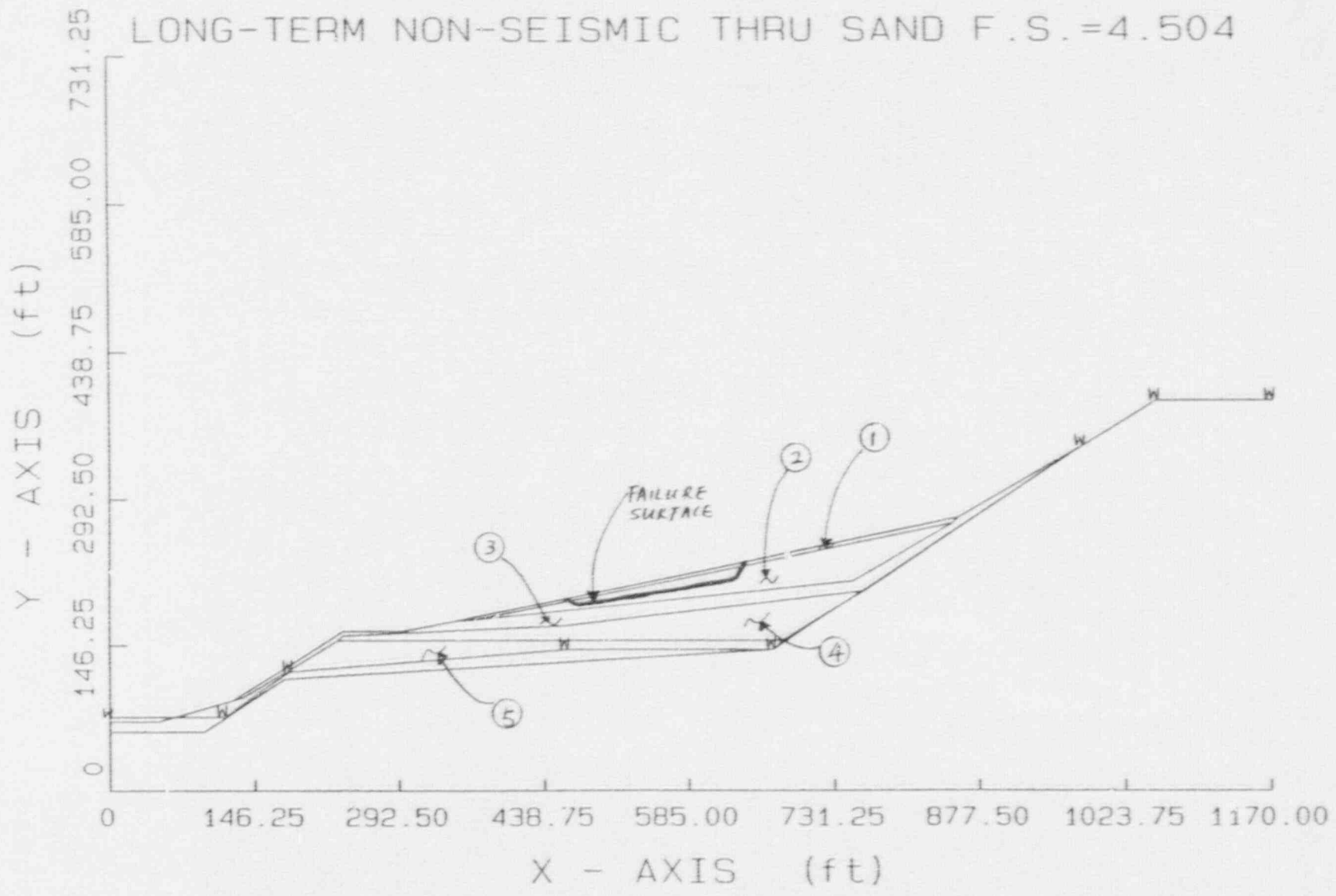
UNTR-LOW
6057-05
PRC 12/19/84
WYL 2/15/85

LONG-TERM NON-SEISMIC THRU COLLUVIUM MINIMUM F.S. = 4.098



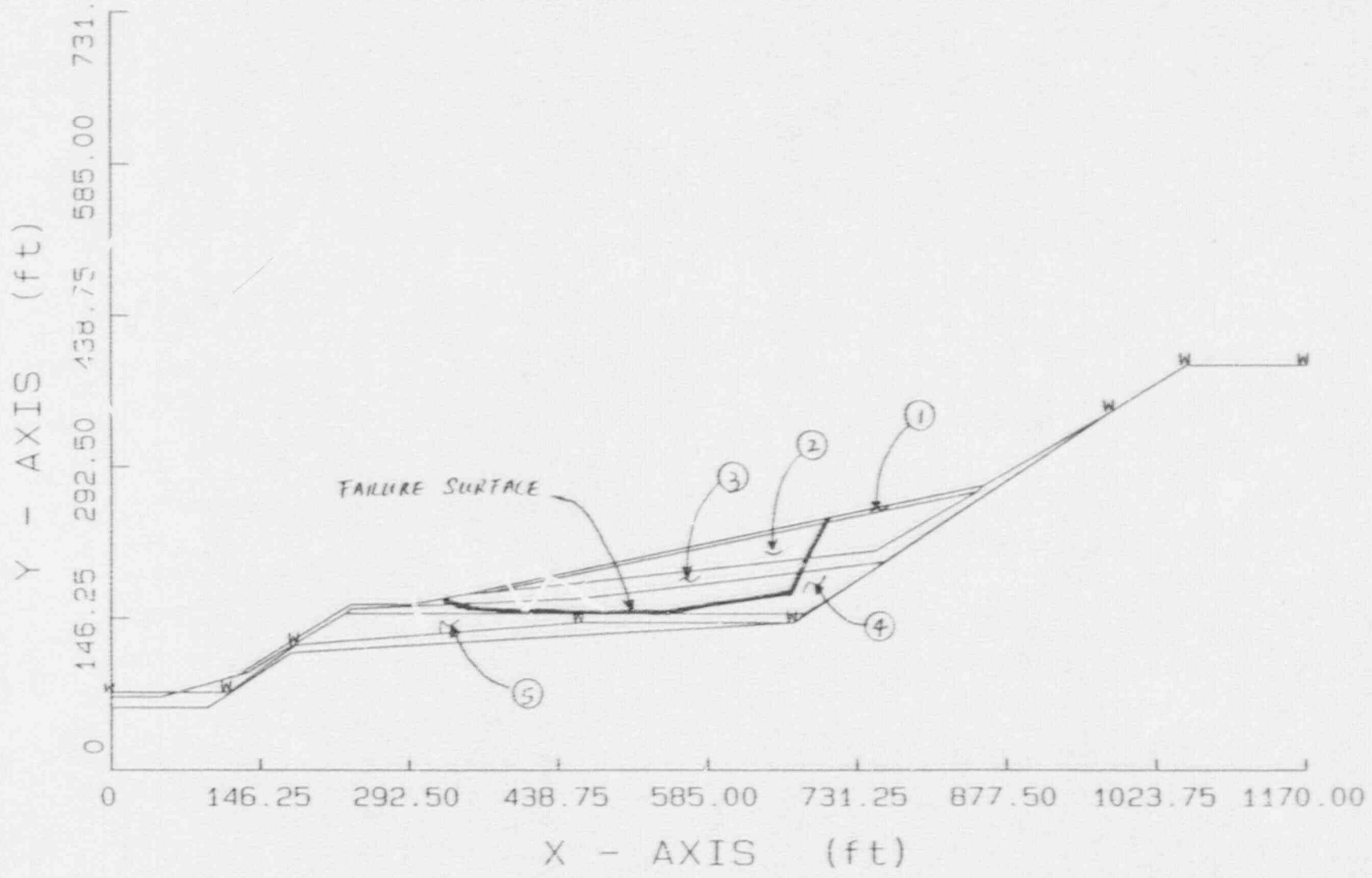
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 P.L. 2/1/84
 S. 24
 UTR-60W
 S. 24

LONG-TERM NON-SEISMIC THRU SAND F.S.=4.504



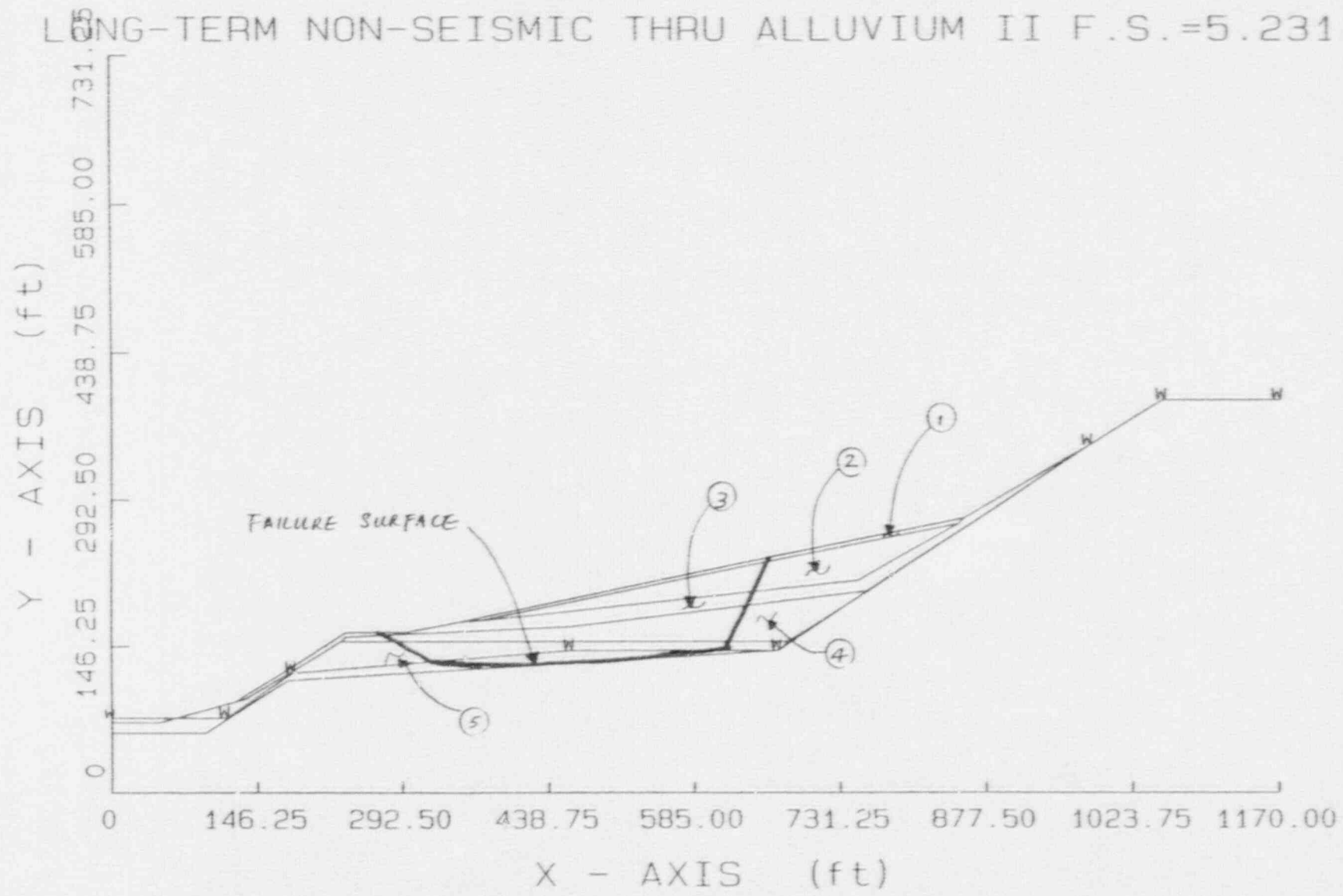
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 90-659
 5657-06
 UHTRA-LOW

LONG-TERM NON-SEISMIC THRU ALLUVIUM I F.S.=5.154



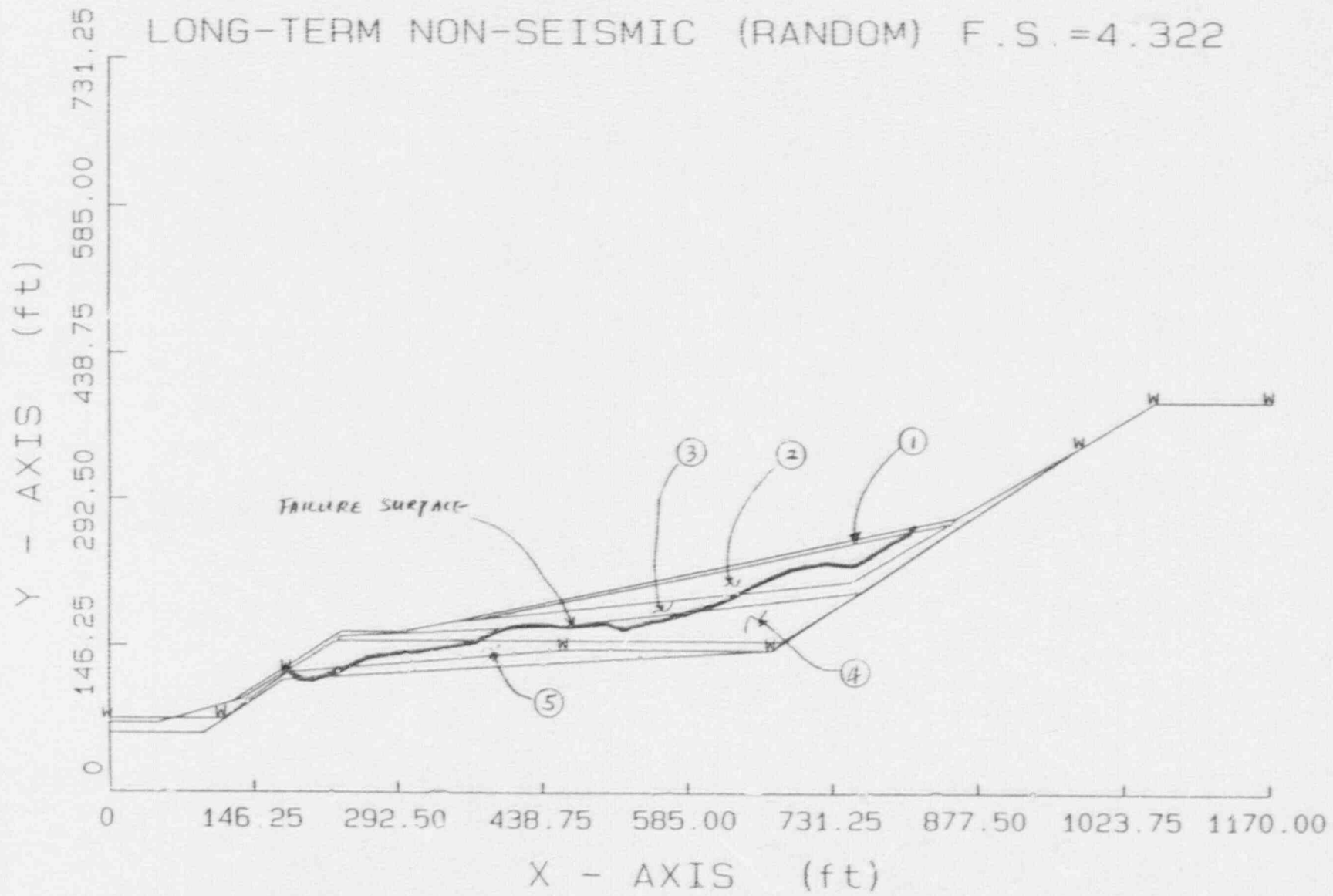
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 W/L 2/2/33
 92 15

LONG-TERM NON-SEISMIC THRU ALLUVIUM II F.S.=5.231

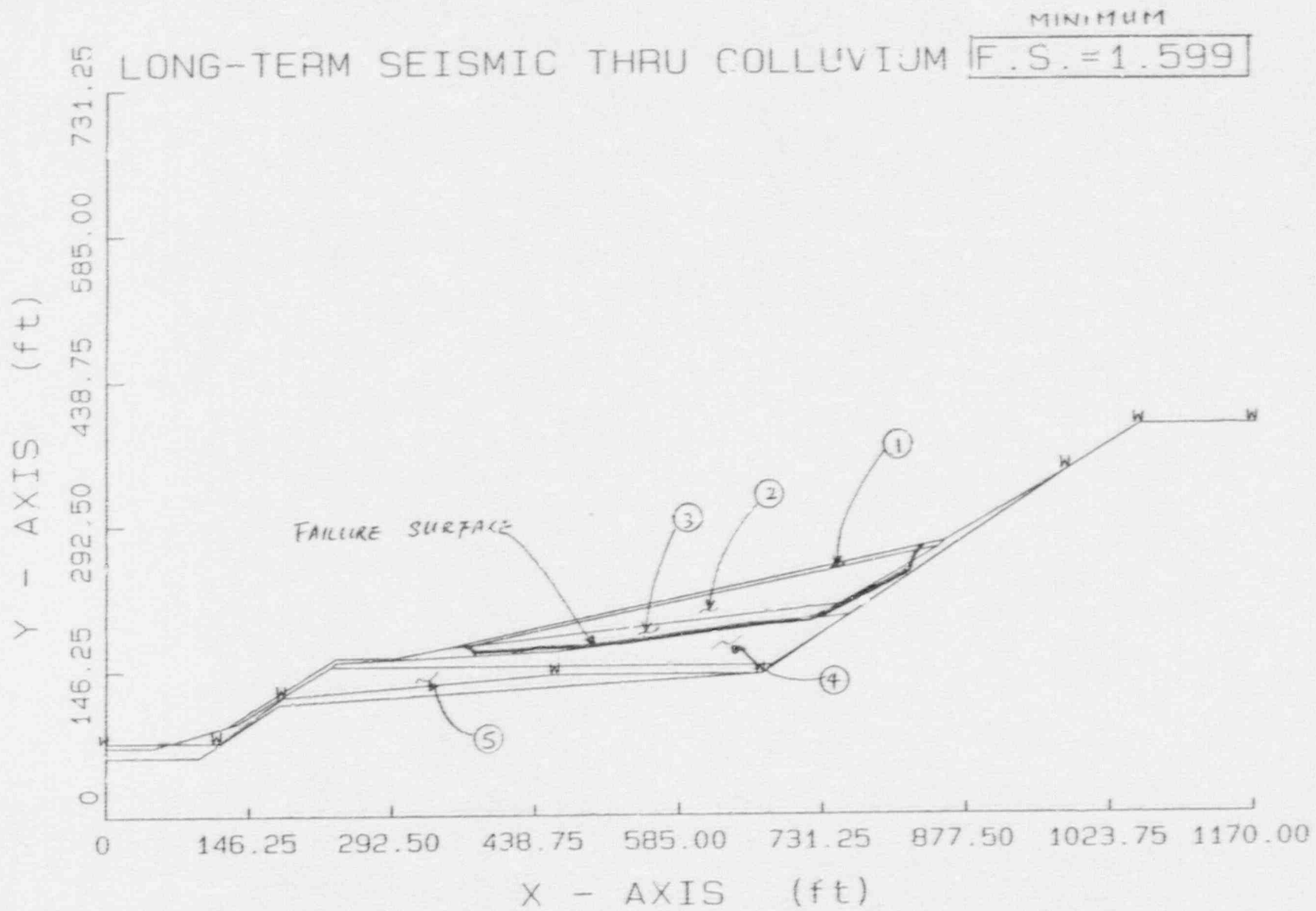


UMTBA-106
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 6/2/01
 7/20
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 45

LONG-TERM NON-SEISMIC (RANDOM) F.S. = 4.322



66.5.18 7/W
68/2.11.72
5054.00
MOT. 1/28/61

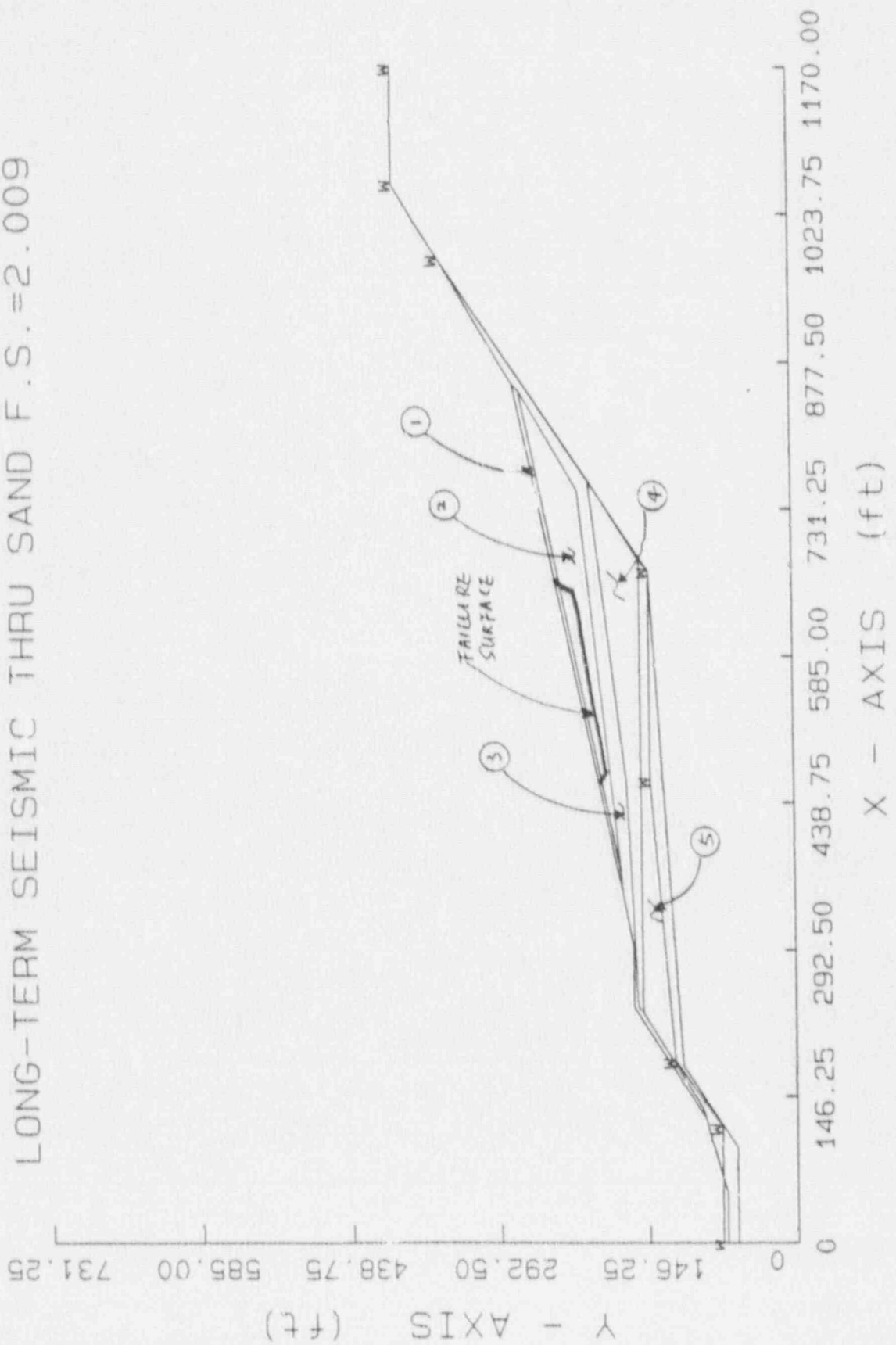


UHTRA-LOW
 COST-06
 90-12/3/29
 W/L 2/1/30

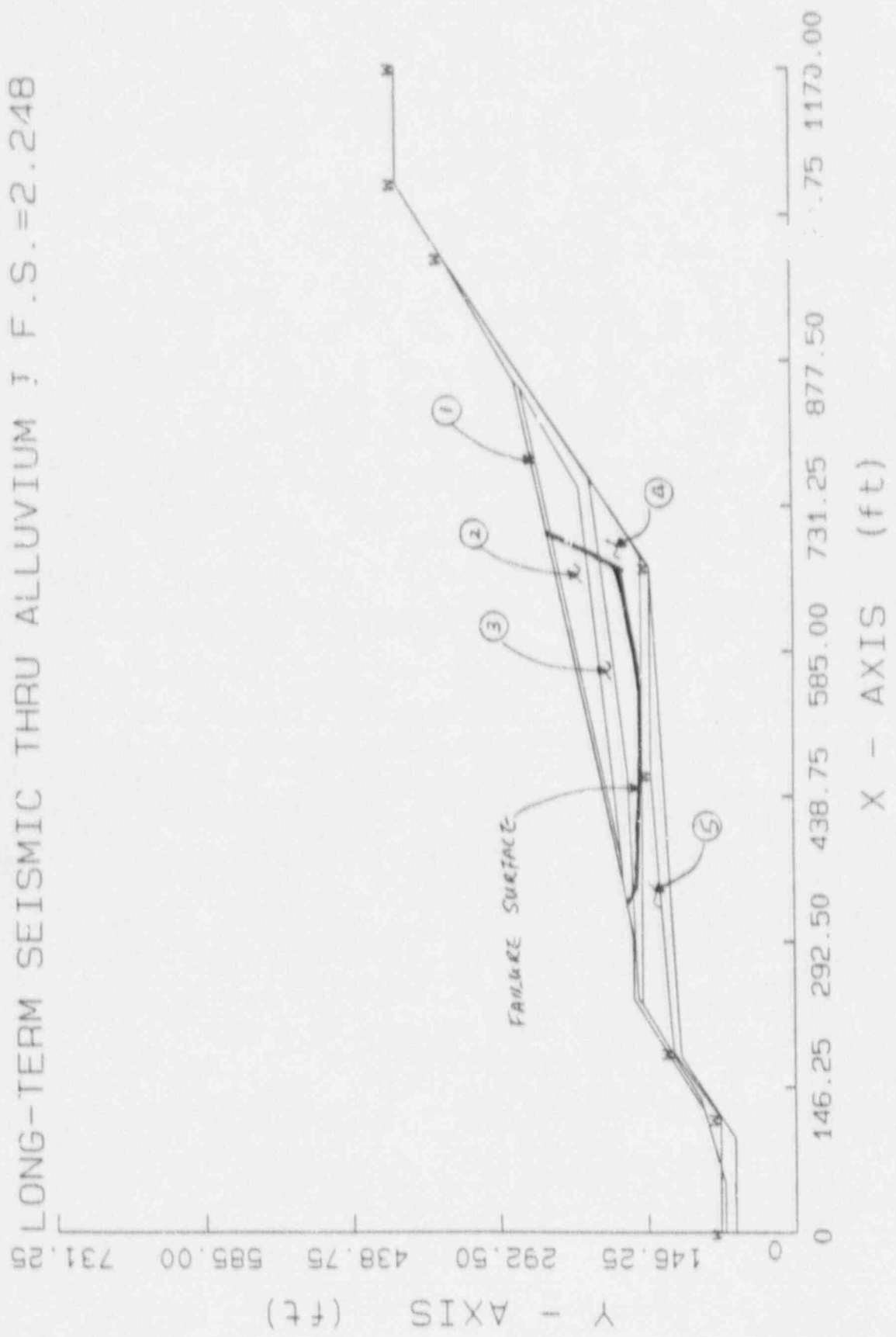
51.29

UMTRA-LOW Sh. 30
5054-06
PXL 12/13/89
WYL 2/5/90

LONG-TERM SEISMIC THRU SAND F.S. = 2.009



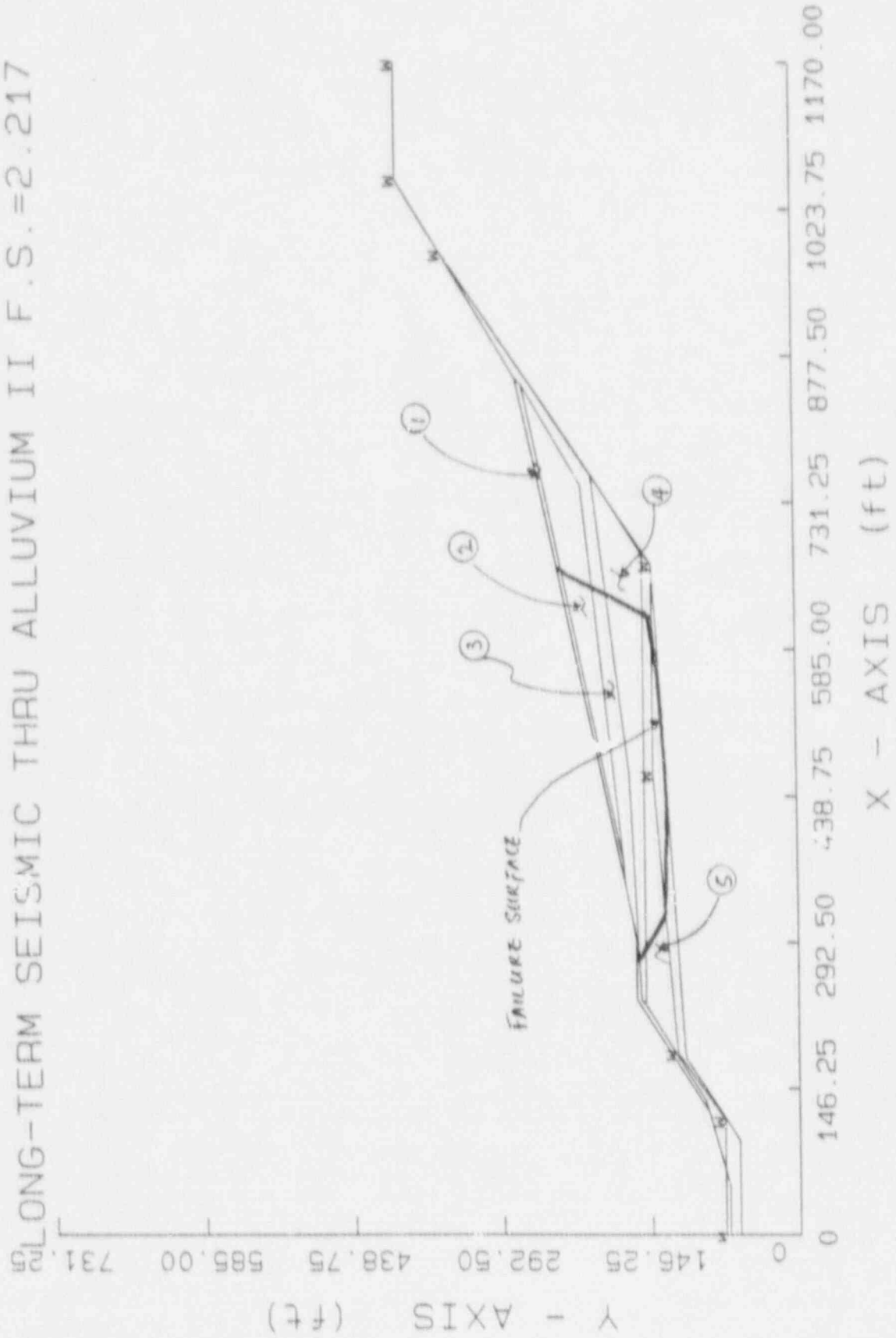
LONG-TERM SEISMIC THRU ALLUVIUM ; F.S. = 2.248



UMTRA - LOW
5057-06
PFL 12/13/89
NYL 2/1/90

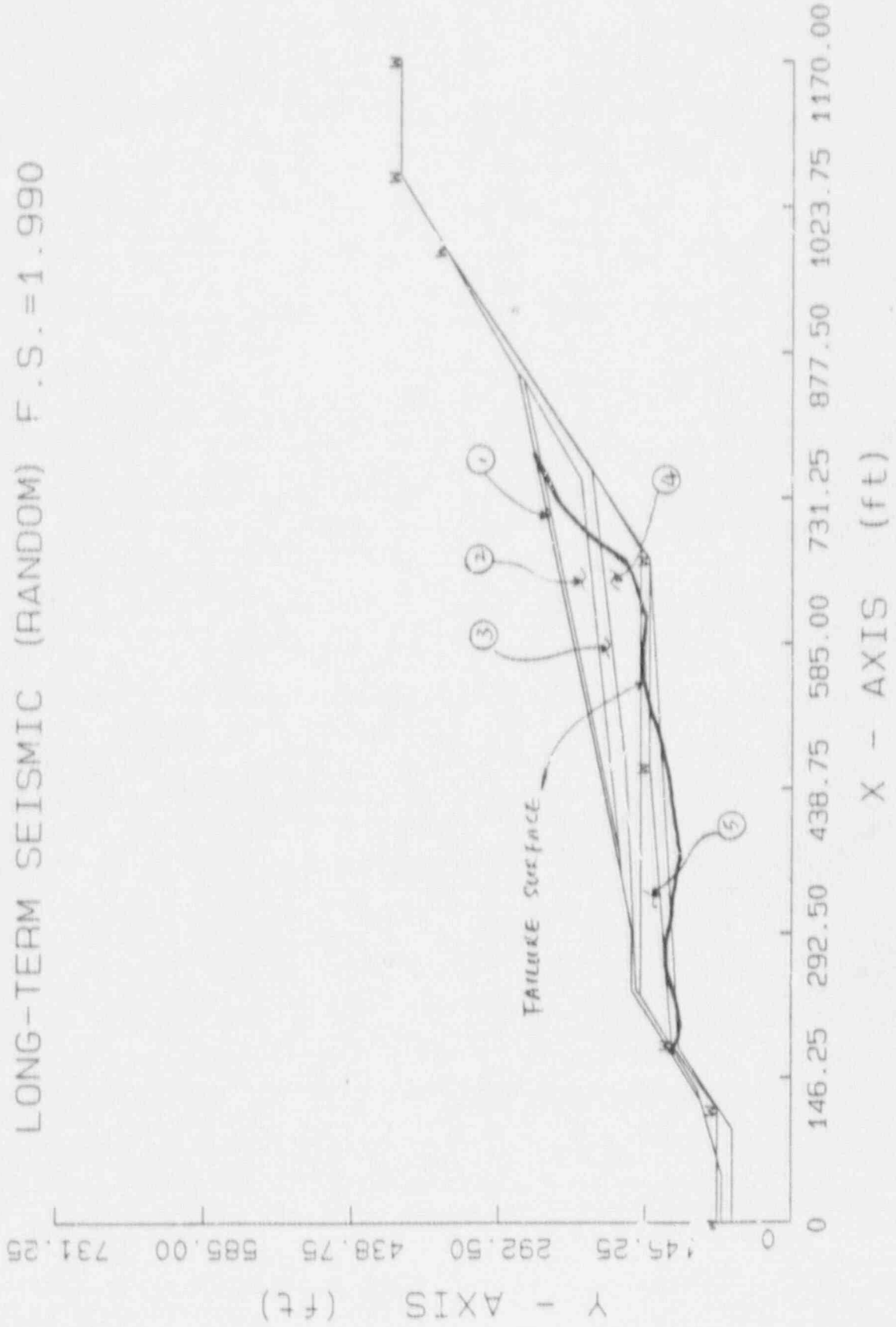
Sh. 31

LONG-TERM SEISMIC THRU ALLUVIUM II F.S. = 2.217



UMTRA-LOU
 5057-06
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 WYL 2/5/90

LONG-TERM SEISMIC (RANDOM) F.S. = 1.990



UMTRA-LOW
5057-06
PYL 12/17/89
WYL 2/12/90

SH, 23

Project UMTRA / LOWContract No. 3885-48Sheet 34Feature Embankment DesignDesigned PVL

File No. _____

Item Slope Stability Analysis Rev ΔChecked R. JoeDate 7/12/90Date 2/3/90REVISION Δ

Purpose: Two potential sliding surfaces were analyzed here in response to NRC's informal comments dated June, 1990.

1. Shallow Surface Failure in Colluvium Layer on Steep Slope above Disposal Cell:

A thin, partial layer of colluvium on the steep slope above the disposal cell was included in the model for the slope stability analysis (see sh 6). It was based on the geotechnical profile, section A, developed from available borholes in Ref 4. The F.S. against shallow surface failure for the 4 loading conditions were determined using 'STABLES'. The results were summarized below:-

i) <u>Short Term, Static</u>	F.S. = <u>2.513</u>	> 1.3	SAFE
ii) <u>Short Term, Seismic</u>	F.S. = <u>1.902</u>	> 1.1	SAFE
iii) <u>Long Term, Static</u>	F.S. = <u>2.741</u>	> 1.5	SAFE
iv) <u>Long Term, Seismic</u>	F.S. = <u>3.500</u>	> 1.1	SAFE

Failure Surfaces were shown on sh 36-39.

'STABLES' output are attached as App. B.

Results of analysis show that this layer of colluvium is safe against shallow surface failure. Moreover, the effect of tree roots due to presence of trees helps to prevent raveling of this layer.





Project UTTERA - LOW Contract No. 3205-42 Sheet 35
 Feature Embankment Design File No. _____
 Item Slope Stability Analysis Rev D Designed PRC Date 3/12/92
 Checked K. Lee Date 2/3/92

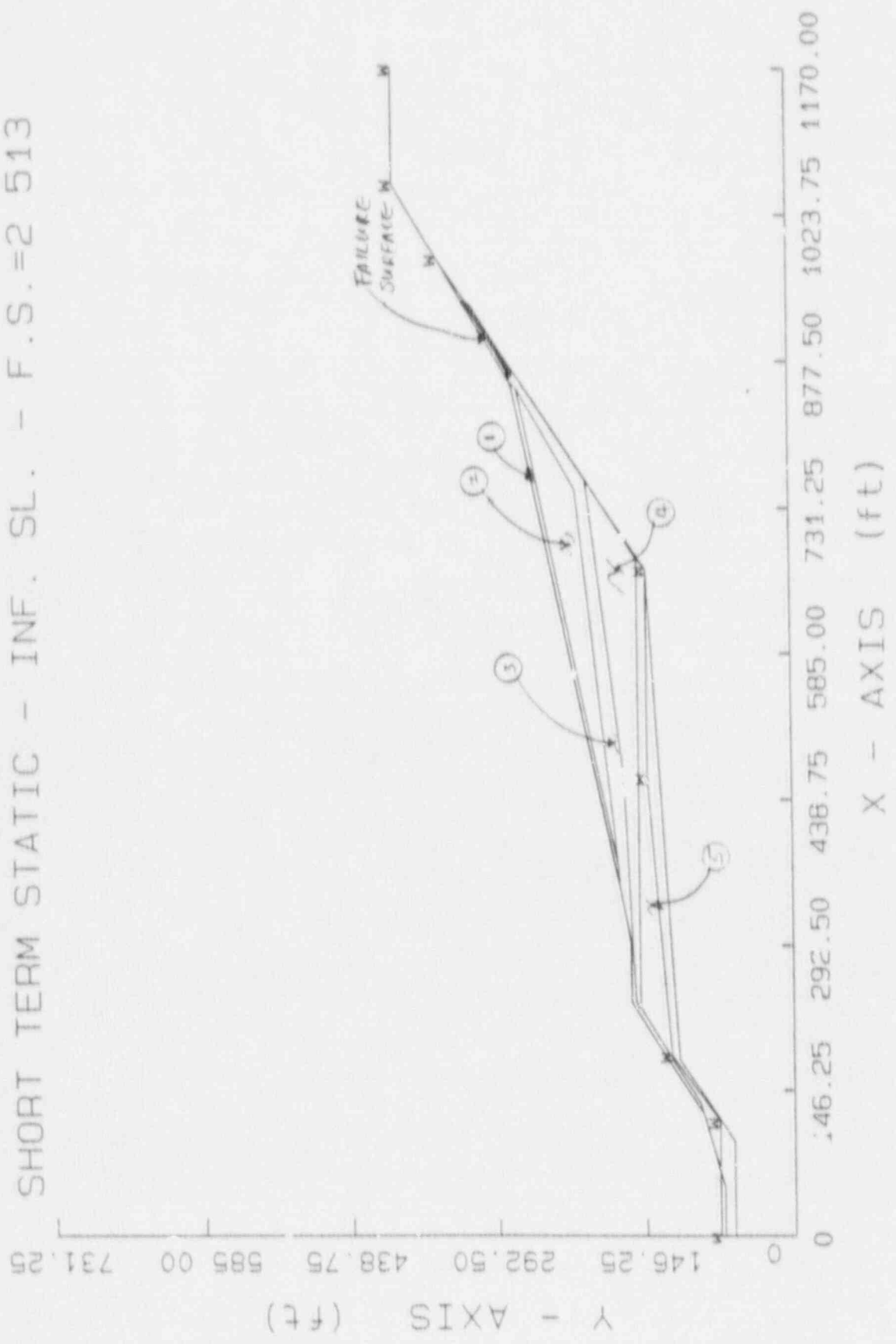
2. Potential Failure Surface through Alluvium on the Clear Creek Embankment at the Toe of the Tailings Cell.

This potential failure surface for the 4 loading conditions had been analyzed in the earlier calculation (Rev. C) (see sh. 3, pt. 2 (v)). The 'RANDOM' searching routine was used to search for critical surfaces terminating in the neighborhood of the toe (including through alluvium layer). The results had been presented on sh. 13. F.S. for all 4 conditions exceed the required. Failure Surfaces had been presented on sh. 18, 23, 28 & 33.

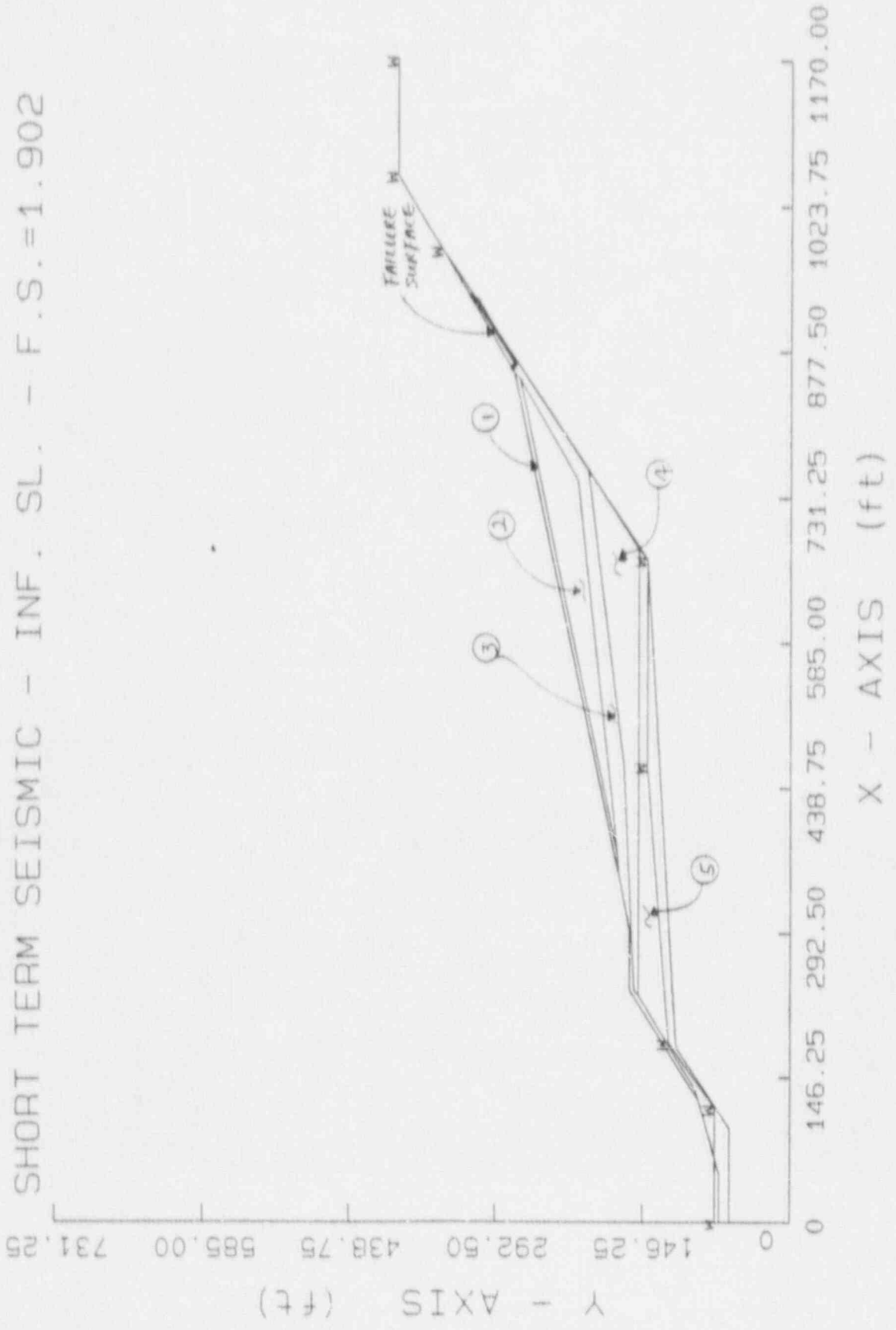


SHORT TERM STATIC - INF. SL. - F.S. = 2.513

UMTRA - LOW
 Rev Δ Slope Stability
 PPL 7/10/90
 Checked 2/22/93, 40

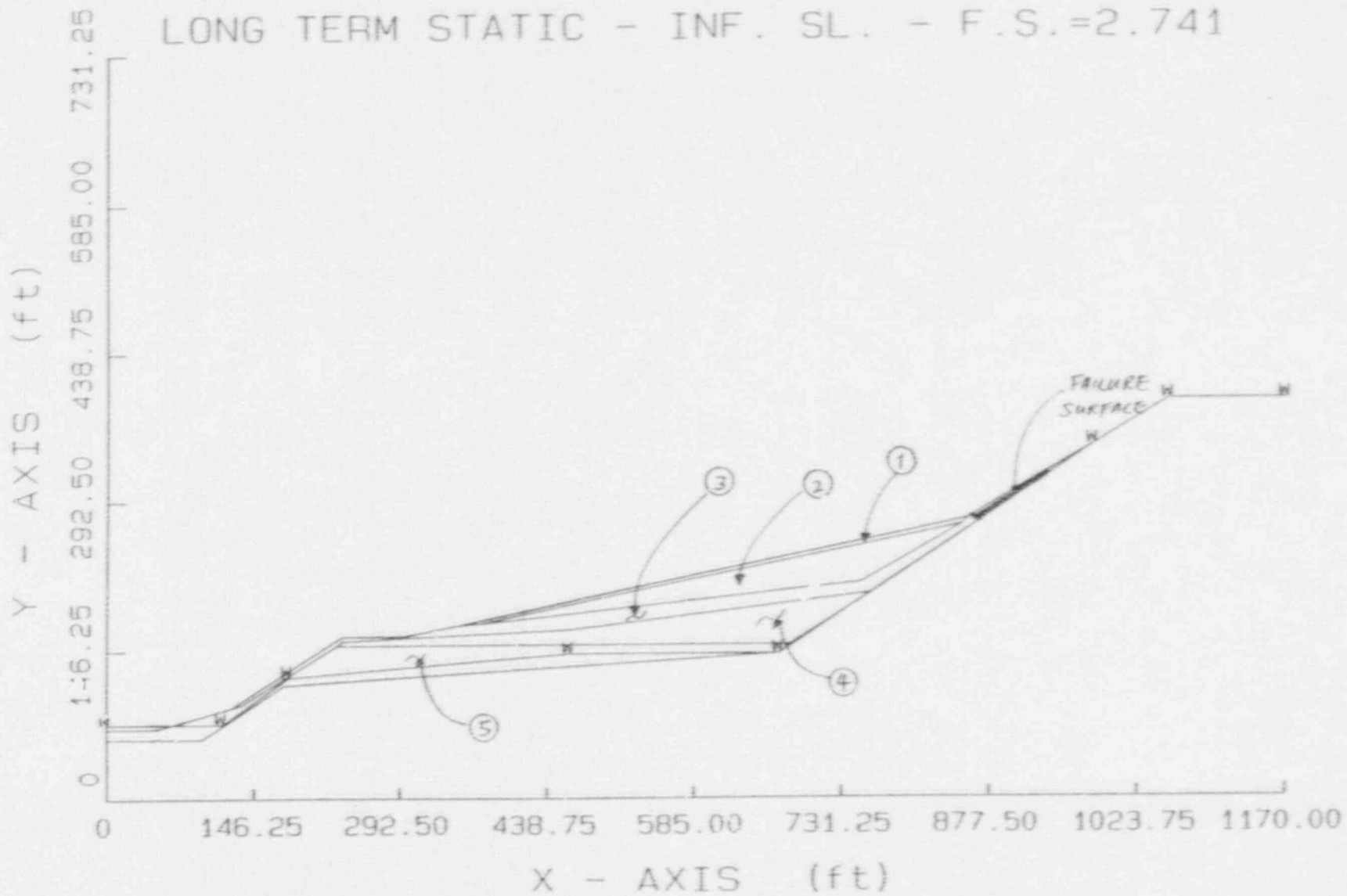


SHORT TERM SEISMIC - INF. SL. - F.S. = 1.902



UPPER LOW SL 37
REVISED BYC 01/12/90
PTL 9/19/90
checked KDC d/3/90

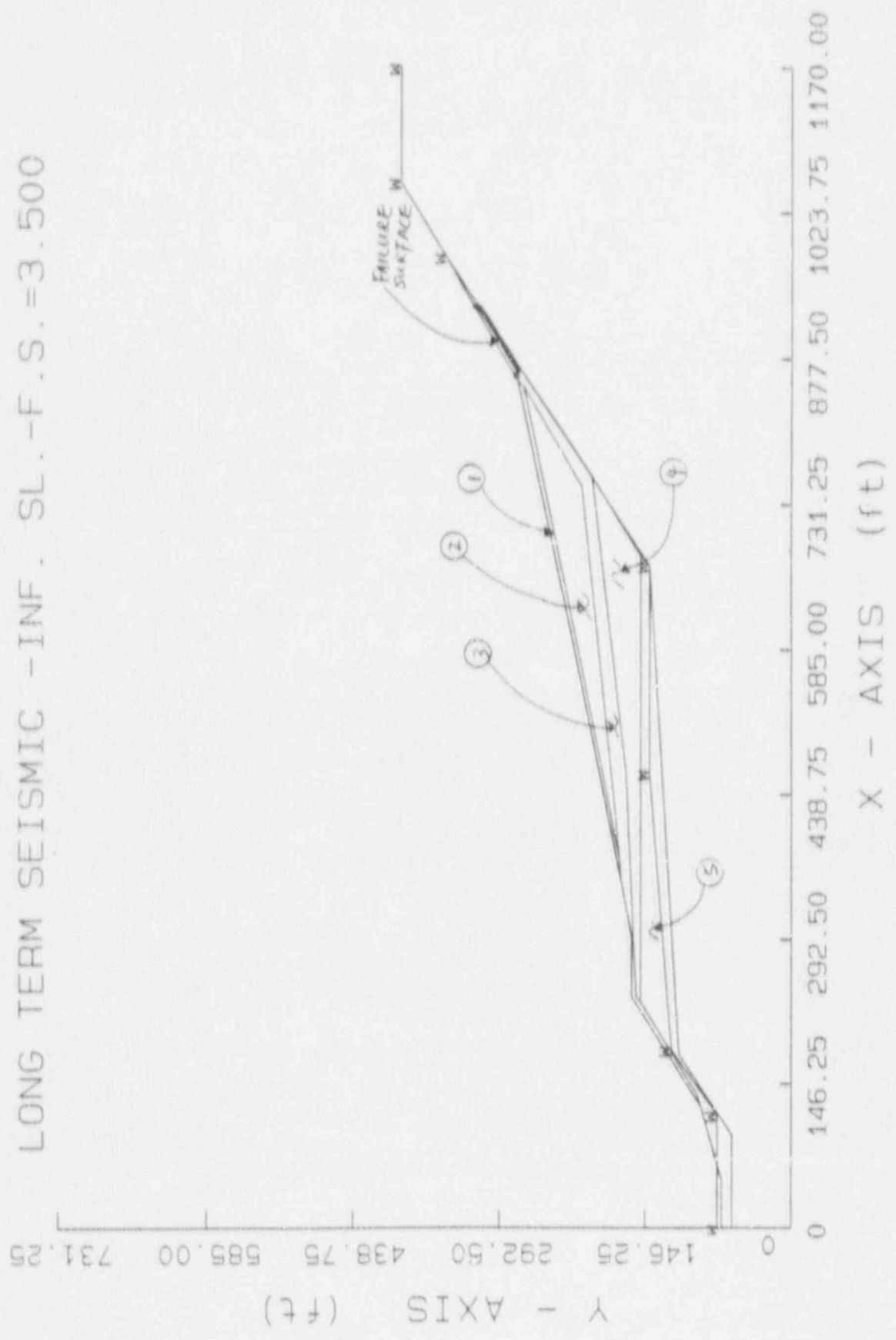
LONG TERM STATIC - INF. SL. - F.S.=2.741



UN712-LOW
Eav Δ SURF 5/16/18
DVL 7/18/18
Carter RDC 2/3/90

LONG TERM SEISMIC - INF. - F.S. = 3.500

UMTRA-LD14 sh. 39
 REV Δ Slope ...
 PTL 9/12/90
 Checked R. Rice 9/30/90





MORRISON-KNUDSEN ENGINEERS, INC.

A MORRISON-KNUDSEN COMPANY

Project UNITA-LOW
Feature Embankment Design
Item Slope Stability Analysis

Contract No. 5057-06
Designed PTL
Checked WTL

Sheet A-1
File No. _____
Date 12/5/89
Date 2/15/92

APPENDIX A

'STABLES' OUTPUT



** PCSTABL5 **

PYL 12/11/89

71 - 21.00

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STNSC.DAT
Output Filename: A:STNSD.OUT
Plotted Output Filename: A:STNSD.PLT

PROBLEM DESCRIPTION ULTRA-LOWMAN #1 SHORT TERM NON-SEISMIC-TM
RU SAND

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Brd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

*** 5.324 ***

PYL 12/2/89

WYL 2/1/90

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	272.50	160.00
2	278.64	156.94
3	292.56	150.00
4	307.61	142.50
5	447.19	130.10
6	589.67	140.73
7	594.30	150.00
8	607.68	176.85
9	610.53	182.56
10	617.43	196.39
11	630.10	223.56
12	632.13	227.54

*** 5.362 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	210.21	144.22
2	215.62	141.52
3	230.80	133.96
4	470.04	132.87
5	610.89	142.62
6	614.57	150.00
7	627.96	176.85
8	632.06	185.07
9	638.79	198.59
10	651.47	225.78
11	652.56	228.10
12	654.61	232.13

*** 5.390 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
-----------	-------------	-------------

2 Boxes Specified For Generation Of Central Block Base

PYL 12/11/89
411 - 212/22

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	460.00	186.00	520.00	193.00	5.00
2	590.00	205.00	750.00	220.00	10.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 6 Coordinate Points *STNSC Det*

Point No.	X-Surf (ft)	Y-Surf (ft)
1	457.37	191.90
2	461.35	189.46
3	468.39	186.18
4	650.42	214.81
5	657.04	229.01
6	659.57	233.14

*** 4.658 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	483.62	197.25
2	487.66	194.78
3	499.85	189.09
4	720.58	221.21
5	731.20	243.99
6	733.83	248.29

*** 4.671 ***

Failure Surface Specified By 6 Coordinate Points

3 Boxes Specified For Generation Of Central Block Base

PXL 12/12/29
W^v = 2/1.30

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	230.00	130.00	330.00	135.00	25.00
2	420.00	135.00	475.00	139.00	15.00
3	510.00	138.00	620.00	145.00	10.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 14 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	265.21	160.00
2	271.04	156.64
3	285.26	150.00
4	296.87	144.21
5	323.72	130.83
6	435.85	128.94
7	615.99	142.57
8	619.70	150.00
9	633.08	176.85
10	637.50	185.71
11	644.20	199.14
12	656.88	226.33
13	658.24	229.25
14	660.30	233.29

*** 5.231 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	249.96	160.00
2	257.94	156.02
3	270.02	150.00
4	292.62	138.73
5	430.56	129.25

1	487.44	198.03
2	491.49	195.55
3	500.73	191.24
4	746.67	224.54
5	758.29	249.47
6	760.96	253.82

PTL 12/11/89
WYL 2/5/90

*** 4.674 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	458.39	192.11
2	462.37	189.67
3	471.14	185.57
4	721.51	218.93
5	733.41	244.44
6	736.04	248.74

*** 4.677 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	462.38	192.92
2	466.37	190.47
3	473.78	187.02
4	639.79	213.94
5	645.76	226.73
6	648.28	230.84

*** 4.678 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	458.27	192.08
2	462.25	189.64
3	469.97	186.04
4	633.39	212.82

** PCSTABL5 **

by
Purdue University

PYL 12/12/89
WV 2/15/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: B:LTNSALL2.DAT
Output Filename: B:LTNSALL2.OUT
Plotted Output Filename: B:LTNSALL2.PLT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 LONG TERM NON-SEISMIC-THR
U ALLUVIUM II

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Brd
1	.60	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

3	509.05	190.91
4	716.36	221.45
5	726.42	243.03
6	729.05	247.31

PYL 12/11/89

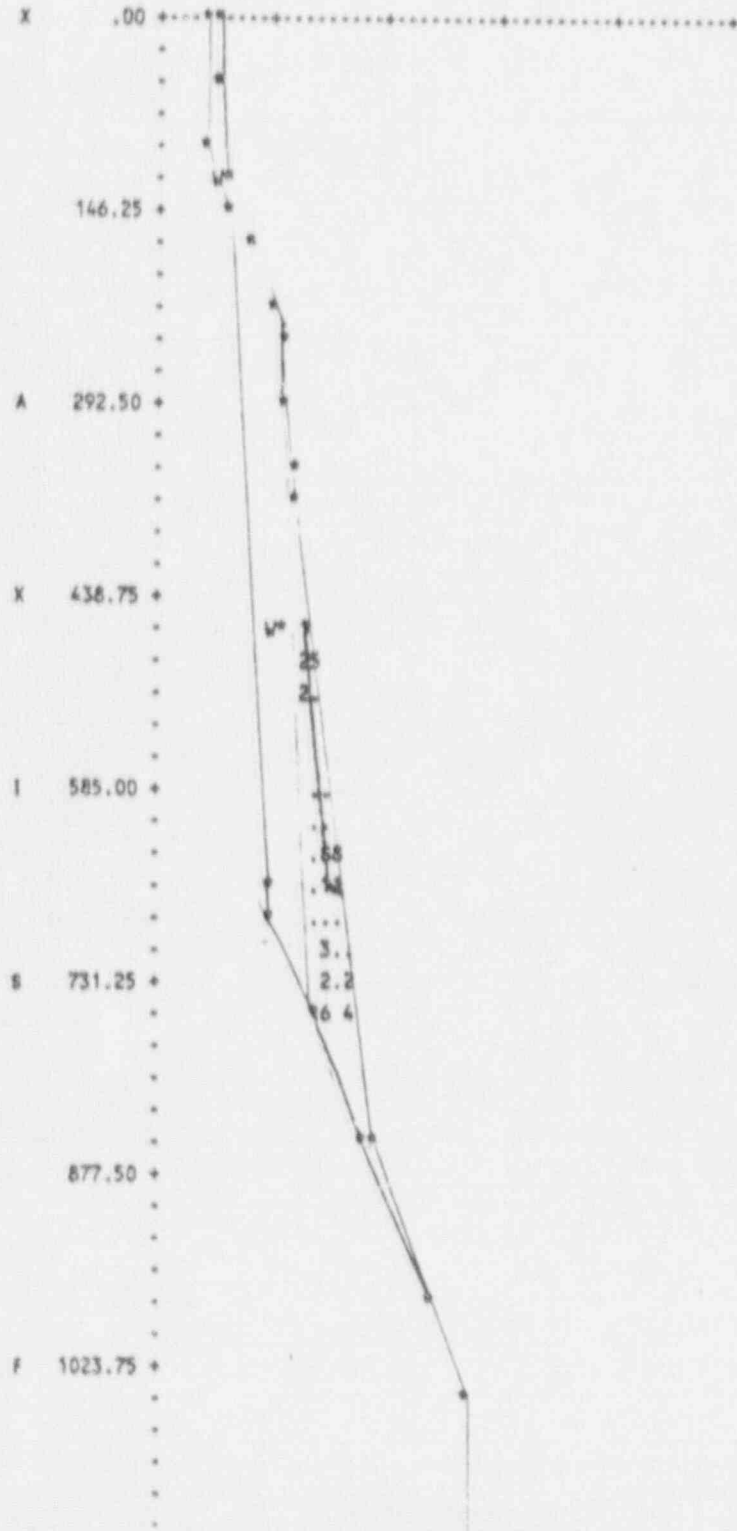
W - 2.2 - 3.3

*** 4.683 ***

Y A X I S F T

146.25 292.50 438.75 585.00 731.25

STASC.



Failure Surface Specified By 10 Coordinate Points

PYL 12/12/89
WYL 2/2/90

Point No.	X-Surf (ft)	Y-Surf (ft)
1	321.48	164.81
2	332.47	159.33
3	346.60	152.29
4	469.86	152.27
5	654.43	169.96
6	663.82	188.78
7	670.32	201.82
8	683.00	229.61
9	685.70	234.80
10	687.79	238.89

*** 5.212 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	323.05	165.10
2	334.45	159.42
3	352.19	150.58
4	530.90	153.72
5	635.49	168.25
6	642.48	186.29
7	649.14	199.65
8	661.82	226.84
9	663.43	230.30
10	665.50	234.35

*** 5.213 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	318.95	164.35
2	329.30	159.19
3	346.38	150.68
4	546.46	154.00
5	664.95	165.22
6	677.49	190.37
7	683.89	203.22
8	696.57	230.40
9	699.97	237.68

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	667.00	150.00	6

PYL 12/11/89

WYL 2/15/93

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

10 690.24 239.40

PXL 12/12/89

WVL 2/5/91

*** 5.160 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	327.62	165.93
2	340.17	159.67
3	351.31	154.12
4	477.29	150.65
5	675.56	169.31
6	686.59	191.44
7	692.93	204.14
8	705.61	231.33
9	709.46	239.60
10	711.57	243.75

*** 5.171 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	321.04	164.74
2	331.93	159.31
3	349.04	150.78
4	548.79	151.90
5	644.24	167.03
6	654.54	187.70
7	661.12	200.88
8	673.79	228.07
9	676.02	232.84
10	678.10	236.92

*** 5.189 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	312.93	163.26

PXL 12/11/89

NVL 2/2/90

*** 2.085 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	393.85	178.94
2	397.68	176.59
3	402.52	174.34
4	411.52	167.74
5	525.24	179.29
6	728.80	198.54
7	833.66	255.05
8	838.08	261.09
9	840.40	266.06
10	843.17	270.59

*** 2.090 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	391.91	178.55
2	395.74	176.20
3	400.24	174.10
4	411.72	165.68
5	519.85	173.63
6	731.13	198.78
7	795.47	226.29
8	808.34	243.84
9	816.44	261.22
10	819.19	265.70

*** 2.109 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	395.05	179.19
2	398.89	176.84

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 12/12/89
W... 2/2/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param. (pcf)	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	98.0	36.0	.00	.0	1
2	146.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	245.0	37.0	.00	.0	1
4	124.5	131.0	885.0	37.0	.00	.0	1
5	124.5	131.0	885.0	37.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

PYL 12/11/89
WYL 2/2/90

Point No.	X-Surf (ft)	Y-Surf (ft)
1	332.19	166.76
2	335.16	164.59
3	472.97	173.94
4	730.03	201.61
5	813.44	238.65
6	823.83	252.82
7	828.93	263.74
8	831.68	268.24

*** 2.167 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	381.19	176.36
2	385.00	174.03
3	387.62	172.81
4	400.00	163.73
5	465.28	173.47
6	731.67	202.37
7	836.49	252.11
8	846.76	266.12
9	847.39	267.47
10	850.18	272.02

*** 2.177 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	357.09	171.45
2	358.74	170.44
3	365.04	165.82
4	488.21	177.34
5	735.64	203.37
6	821.35	246.81
7	827.19	254.77
8	831.63	264.29
9	834.39	268.80

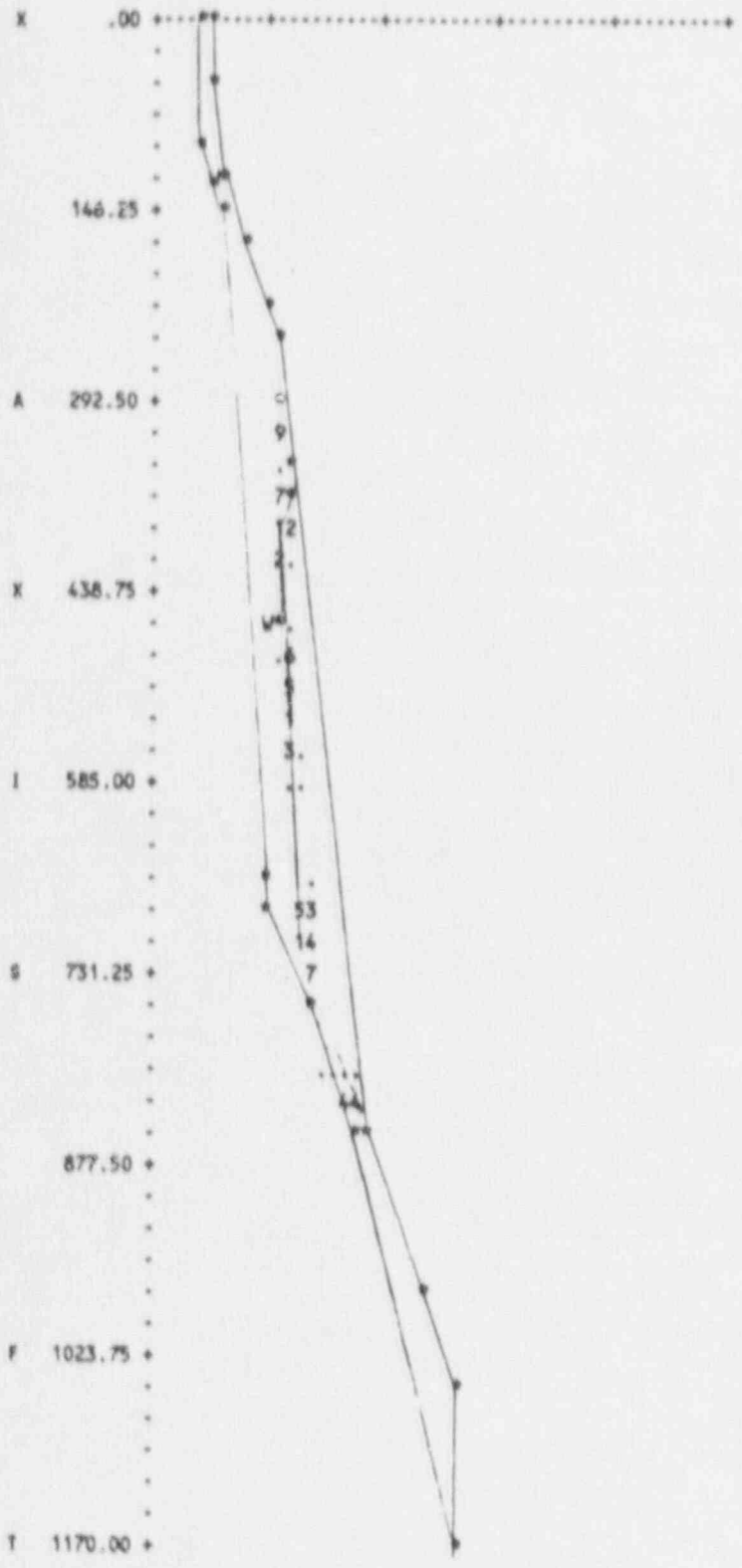
*** 2.206 ***

PYL 12/12/89

Y/L 2/5/93

Y A X I S F T

.00 146.25 292.50 438.75 585.00 731.25



** PCSTABL5 **

by
Purdue University

PYL 12/11/89
WYL 2/5/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STNSALL1.DAT
Output Filename: A:STNSALLB.OUT
Plotted Output Filename: A:STNSALLB.PLT

PROBLEM DESCRIPTION ULTRA-LOWMAN #1 SHORT TERM NON-SEISMIC-TERR
RU ALLUVIUM 1

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

3	395.64	173.63
4	407.23	167.85
5	523.83	173.90
6	677.67	193.26
7	823.70	248.86
8	826.42	254.33
9	831.01	264.16
10	833.25	268.56

PTL 12/12/29

WVL 2/12/30

*** 4.115 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	384.92	177.12
2	389.28	174.90
3	392.65	173.33
4	403.91	167.72
5	495.47	170.19
6	708.16	198.17
7	822.82	246.84
8	826.60	254.43
9	831.15	264.19
10	833.40	268.59

*** 4.119 ***

1
Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	361.53	172.35
2	364.71	170.74
3	376.56	164.83
4	530.16	173.16
5	732.77	199.54
6	818.52	244.75
7	822.02	251.77
8	827.46	263.45
9	829.70	267.84

*** 4.120 ***

Failure Surface Specified By 10 Coordinate Points

3 Boxes Specified For Generation Of Central Block Base

PXL 12/11/89

WYL 2/5/90

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	153.00	370.00	153.00	5.00
2	460.00	153.00	550.00	155.00	10.00
3	620.00	160.00	680.00	160.00	20.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical To Least.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 10 Coordinate Points

STNS ALL 1.00

Point No.	X-Surf (ft)	Y-Surf (ft)
1	329.62	166.29
2	338.74	159.61
3	347.18	154.93
4	535.60	150.98
5	666.39	169.26
6	678.14	190.45
7	687.79	203.62
8	700.47	230.80
9	704.06	238.51
10	706.46	242.74

*** 4.504 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	331.72	166.68
2	341.21	159.72
3	351.31	154.12
4	477.29	150.65
5	675.56	169.31
6	687.91	191.59
7	697.46	206.61
8	710.13	231.80
9	714.22	240.56

4 Boxes Specified For Generation Of Central Block Base

PYL 12/12/89

W/L 2/1/89

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	162.00	420.00	167.00	5.00
2	460.00	170.00	580.00	180.00	10.00
3	670.00	195.00	740.00	204.00	10.00
4	795.00	228.00	845.00	260.00	5.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	365.66	173.19
2	369.97	171.00
3	380.27	160.86
4	547.74	175.54
5	706.93	195.10
6	843.03	258.24
7	847.04	266.28
8	847.62	267.52
9	849.88	271.96

LTN5COLL.DAT

4.098

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	388.11	177.77
2	392.48	175.54
3	396.41	173.71
4	409.18	167.34
5	469.92	167.26
6	700.91	195.66
7	841.86	257.22
8	846.11	265.74
9	846.87	267.37

2	335.17	159.45
3	349.23	151.66
4	540.70	151.47
5	648.22	166.80
6	560.17	188.35
7	670.03	201.79
8	682.71	228.98
9	685.39	234.73
10	687.96	238.93

PYL 12/11/89
WYL 2/2/90

*** 4.846 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	322.95	165.08
2	330.89	159.26
3	346.38	150.67
4	546.46	154.00
5	664.95	165.22
6	678.99	190.55
7	688.63	203.70
8	701.31	230.89
9	704.95	238.69
10	707.55	242.93

*** 4.879

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	316.22	163.86
2	322.97	158.91
3	334.80	152.35
4	530.55	151.96
5	638.88	169.11
6	648.81	187.03
7	658.79	200.64
8	671.47	227.83
9	673.58	232.35
10	676.13	236.52

*** 4.881 ***

** PCSTABL5 **

by
Purdue University

PYL 12/12/89

W 2/1/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:LTMSCOLL.DAT
Output Filename: A:LTMSCOLL.OUT
Plotted Output Filename: A:LTMSCOLL.PLT

PROBLEM DESCRIPTION UNTRA-LOWMAN #1 LONG TERM NON-SEIMIC THR
U COLLUVIUM

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1176.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	1
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

5	602.00	217.88
6	603.99	221.80

PYL 12/12/89
WYL 2/5/90

*** 4.562 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	491.54	198.87
2	496.19	196.50
3	510.40	189.87
4	713.70	221.23
5	723.60	242.46
6	725.73	246.63

*** 4.581 ***

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 12/11/89
W/L 2/2/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

PYL 12/12/89
1171 2/5/92

Point No.	X-Surf (ft)	Y-Surf (ft)
1	456.79	191.78
2	461.35	189.46
3	468.39	186.18
4	650.42	214.81
5	657.04	229.01
6	659.09	233.04

*** 4.506 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	471.53	196.79
2	476.13	192.45
3	481.63	189.88
4	638.23	214.39
5	643.80	226.33
6	645.84	230.54

*** 4.517 ***

1
Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	476.22	195.74
2	480.82	193.40
3	491.30	190.51
4	605.26	211.08
5	609.11	219.32
6	611.11	223.26

*** 4.517 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
-----------	-------------	-------------

6	611.86	149.13
7	612.34	150.00
8	626.88	176.24
9	631.76	185.04
10	641.93	198.91
11	654.61	226.10
12	655.85	228.77
13	655.38	232.90

PYL 12/11/89
WYL 2/5/90

*** 4.899 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	283.90	160.00
2	287.54	157.33
3	300.77	150.00
4	311.61	143.00
5	424.04	133.06
6	609.55	145.79
7	611.88	150.00
8	626.42	176.24
9	631.27	184.98
10	641.45	198.86
11	654.13	226.05
12	655.34	228.66
13	657.88	232.79

*** 4.916 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	277.32	160.00
2	281.33	157.06
3	294.07	150.00
4	307.61	142.50
5	447.19	130.10
6	589.67	140.73
7	594.81	150.00
8	609.36	176.24
9	613.02	182.85
10	623.40	197.01
11	636.08	224.20
12	636.38	224.83
13	638.88	228.92

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 12/12/09
 WY = 2/2/00

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param. (pcf)	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	98.0	36.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	245.0	37.0	.00	.0	1
4	124.5	131.0	885.0	37.0	.00	.0	1
5	124.5	131.0	885.0	37.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1035.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

PYL 12/11/89

V/L 2/2/90

1	211.85	145.27
2	216.36	141.96
3	230.80	133.96
4	470.04	132.87
5	610.89	142.62
6	614.98	150.00
7	629.53	176.24
8	634.59	185.37
9	644.72	199.20
10	657.40	226.38
11	658.79	229.36
12	661.33	233.50

*** 5.015 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	254.26	160.00
2	259.59	156.09
3	270.58	150.00
4	284.36	142.36
5	466.20	130.00
6	595.04	115.00
7	596.13	150.00
8	610.66	176.24
9	614.43	183.02
10	624.80	197.15
11	637.47	224.34
12	637.84	225.00
13	640.35	229.22

*** 5.034 ***

Failure Surface Specified By 12 Coordinate Points

Point no.	X-Surf (ft)	Y-Surf (ft)
1	132.30	160.00
2	186.02	157.27
3	299.13	150.00
4	319.18	138.89
5	424.53	129.98
6	558.57	137.78
7	565.35	150.00
8	579.90	176.24
9	581.52	179.18

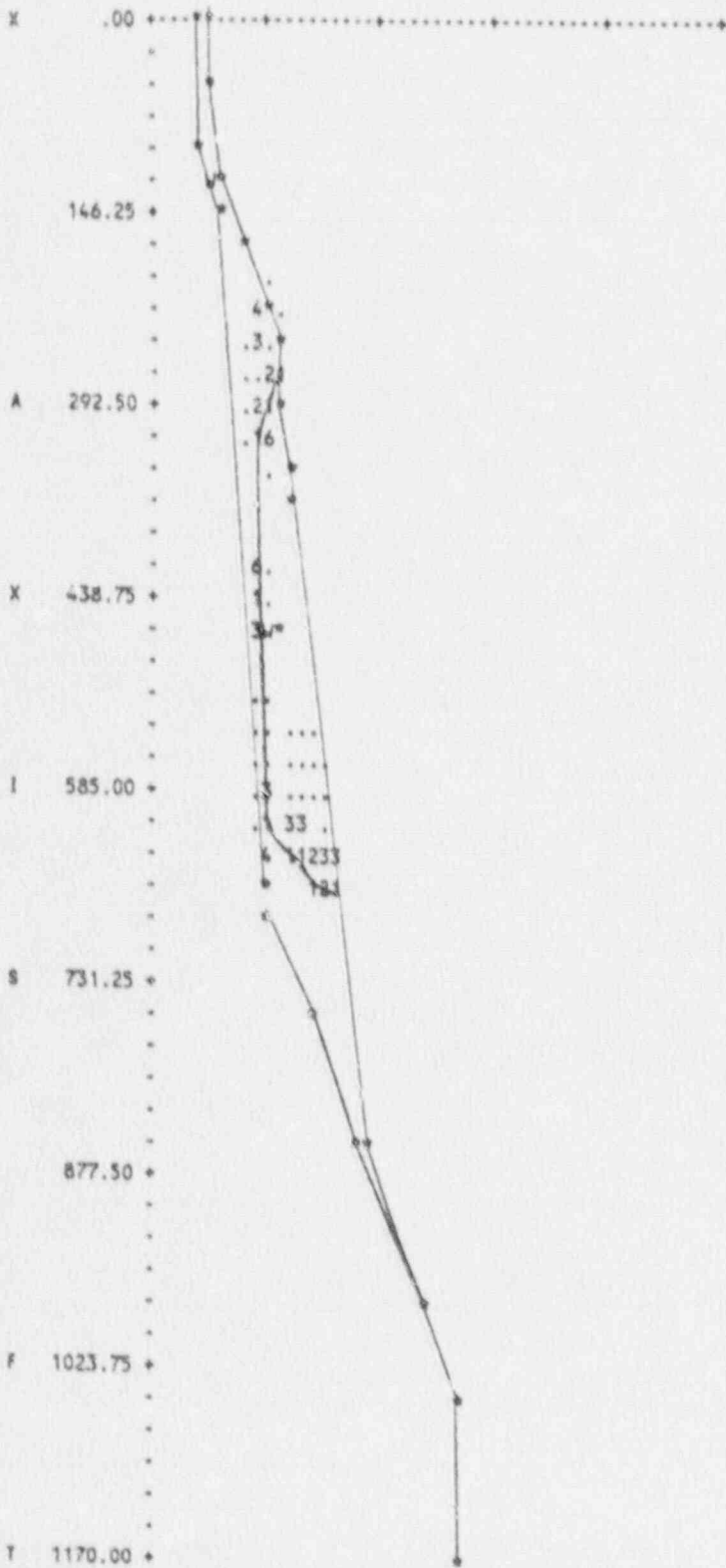
PYL 12/12/59
WVC 2/5/90

*** 2.687 ***

Y A X I S F Y

.00 146.25 292.50 438.75 585.00 731.25

STGALL 2.



10	598.75	194.48
11	610.46	219.59
12	612.94	223.63

PYL 12/11/89

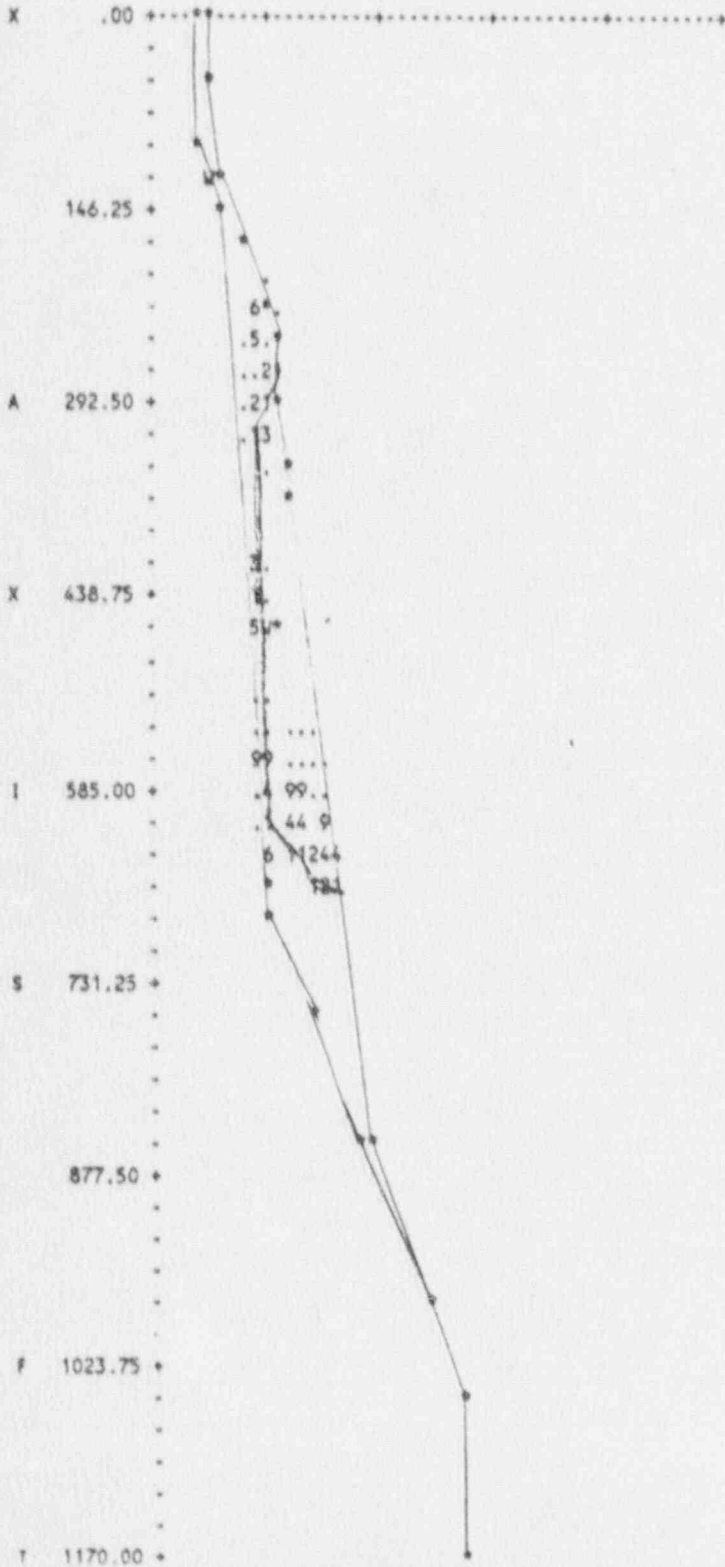
WYL 2/2/90

*** 5.093 ***

Y A X I S F T

STNSALL II.

.00 146.25 292.50 438.75 585.00 731.25



PYL 12/12/89
11' - 2/10' 2)

1

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	277.32	160.00
2	281.33	157.06
3	294.07	150.00
4	307.61	142.50
5	447.19	130.10
6	589.67	140.73
7	594.81	150.00
8	609.36	176.24
9	613.02	182.85
10	623.40	197.01
11	636.08	224.20
12	636.38	224.83
13	638.88	228.92

*** 2.635 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	254.26	160.00
2	259.59	156.09
3	270.58	150.00
4	284.36	142.36
5	466.20	132.33
6	595.04	148.04
7	596.13	150.00
8	610.68	176.24
9	614.43	183.02
10	624.80	197.15
11	637.47	224.34
12	637.84	225.13
13	640.35	229.22

*** 2.662 ***

1

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
-----------	-------------	-------------

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 12/2/89
W/L 2/5/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.30	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient of .170 Has Been Assigned

A Vertical Earthquake Loading Coefficient of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been

PYL 12/12/89
W/L 2/5/90

Point No.	X-Surf (ft)	Y-Surf (ft)
1	255.94	160.00
2	261.17	156.16
3	272.29	150.00
4	292.62	138.73
5	430.56	129.25
6	611.86	149.13
7	612.34	150.00
8	626.88	176.24
9	631.76	185.04
10	641.93	198.91
11	654.61	226.10
12	655.85	228.77
13	658.38	230.90

*** 2.592 ***

1
Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	207.96	142.79
2	212.53	139.58
3	238.32	125.18
4	457.42	130.79
5	586.50	143.81
6	589.94	150.00
7	604.48	176.24
8	607.81	182.24
9	618.25	196.48
10	630.92	223.67
11	630.96	223.71
12	633.45	227.31

*** 2.593 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	215.76	147.76
2	220.41	144.35
3	231.19	138.37
4	465.10	131.86
5	618.19	142.97

5	745.30	246.84
6	747.95	251.17

PYL 12/12/89
W'L 2/5/90

*** 2.435 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	458.39	192.11
2	462.37	189.67
3	471.14	185.57
4	721.51	218.93
5	733.41	244.44
6	736.04	248.74

*** 2.436 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	457.37	191.90
2	461.35	189.46
3	468.39	186.18
4	650.42	214.81
5	657.04	229.01
6	659.57	233.14

*** 2.436 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	483.62	197.25
2	487.66	194.78
3	499.85	189.09
4	720.58	221.21
5	731.20	243.99
6	733.83	248.29

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PTL 12/12/89
WVL 2/2/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .170 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been

PYL 12/12/89
W/L 2/5/90

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	492.13	198.99
2	496.19	196.50
3	510.40	189.87
4	713.70	221.23
5	723.60	242.46
6	726.22	246.73

*** 2.441 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	492.74	199.11
2	496.80	196.62
3	509.05	190.91
4	716.36	221.45
5	726.42	243.03
6	729.05	247.31

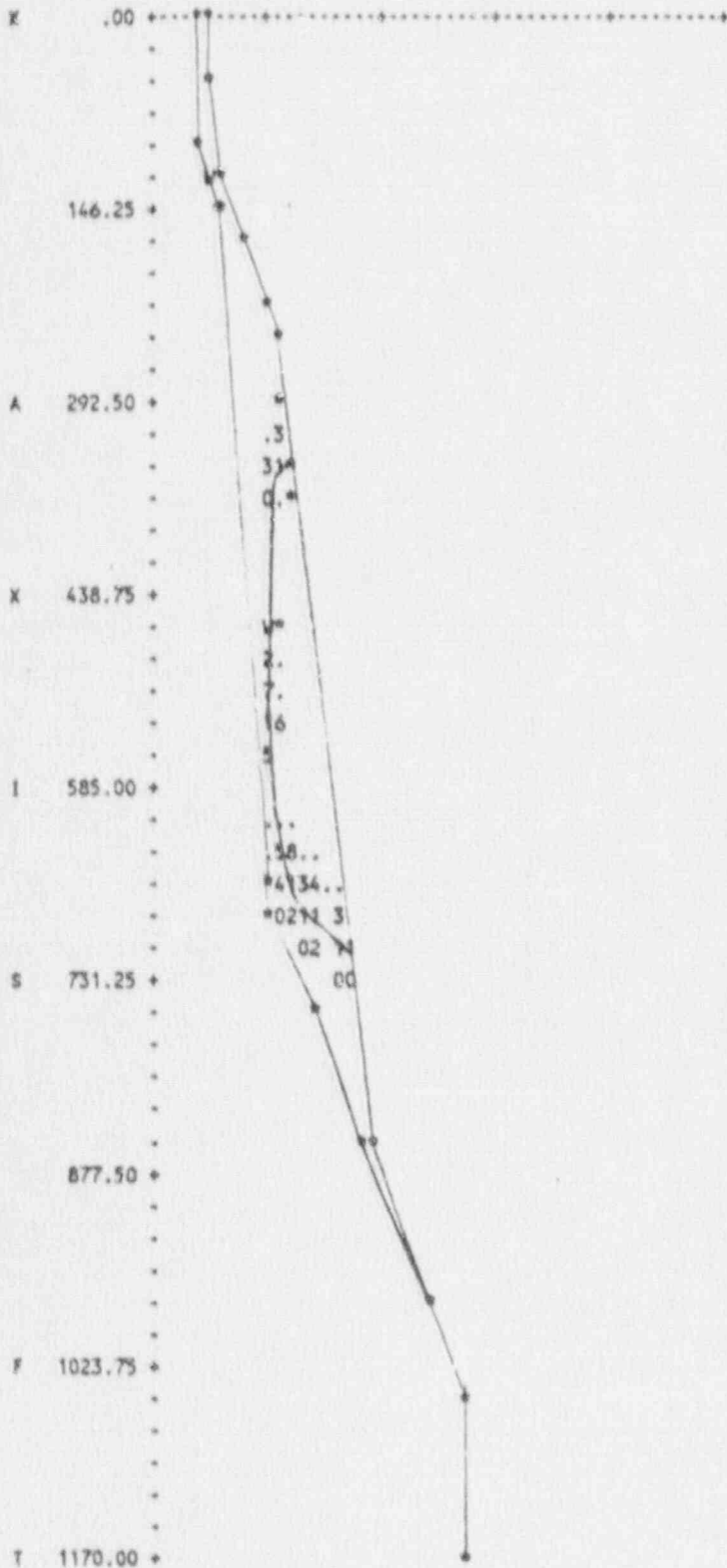
*** 2.444 ***

PYL 12/12/89
IV - : : 03

Y A X I S F T

.00 146.25 292.50 438.75 585.00 731.25

STSALL 1.



** POSTABLES **

by
Purdue University

PYL 12/12/89
WYL 2/5/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STSCOLL.DAT
Output Filename: A:STSCOLLB.OUT
Plotted Output Filename: A:STSCOLLB.PLT

PROBLEM DESCRIPTION ULTRA-LOWMAN #1 SHORT TERM SEISMIC-THRU C
OLLUVIUM

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

7	669.66	201.76
8	682.34	228.94
9	685.01	234.66
10	687.58	238.85

PYL 12/12/89

W/L 2/5/90

*** 2.620 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	316.22	163.86
2	322.97	158.91
3	334.80	152.35
4	530.55	151.96
5	638.88	169.11
6	648.81	187.03
7	658.79	200.64
8	671.47	227.83
9	673.58	232.35
10	676.13	236.52

*** 2.621 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	325.42	165.53
2	333.79	159.39
3	342.23	152.29
4	469.86	152.27
5	654.63	169.96
6	664.94	188.91
7	674.74	202.28
8	687.42	229.47
9	690.34	235.74
10	692.92	239.94

*** 2.622 ***

Failure Surface Specified By 11 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
-----------	-------------	-------------

Specified.

PYL 12/12/89

W/L 2/2/90

The Active And Passive Portions Of The Sliding Surfaces
Are Generated According To The Rankine Theory.

100 Trial surfaces Have Been Generated.

4 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of
Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	162.00	420.00	167.00	5.00
2	460.00	170.00	580.00	180.00	10.00
3	670.00	195.00	740.00	204.00	10.00
4	795.00	228.00	845.00	260.00	5.00

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated . The Modified Janbu Method * *

Failure Surface Specified By 9 Coordinate Points

STSCOLL.DAT

Point No.	X-Surf (ft)	Y-Surf (ft)
1	364.82	173.02
2	368.26	170.91
3	376.56	164.83
4	530.16	173.16
5	732.77	199.54
6	818.52	244.75
7	824.89	253.43
8	829.77	263.91
9	832.53	268.42

*** 1.103 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
-----------	-------------	-------------

PYL 12/12/89
W/L 2/5/90

1	331.72	166.68
2	341.21	159.72
3	351.31	154.12
4	477.29	150.65
5	675.56	169.31
6	687.91	191.59
7	697.46	204.61
8	710.13	231.80
9	714.22	240.56
10	716.83	244.82

*** 2.579 ***

1
Failure Surface Specified by 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	321.13	164.75
2	328.74	159.17
3	339.32	153.30
4	546.06	150.78
5	656.02	168.69
6	667.38	189.19
7	677.16	202.52
8	689.83	229.71
9	692.88	236.25
10	695.46	240.46

*** 2.587 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	326.59	165.74
2	335.17	159.45
3	349.23	151.66
4	540.70	151.47
5	648.22	166.80
6	660.17	188.33
7	670.03	201.79
8	682.71	228.98
9	685.39	234.73
10	687.96	238.93

*** 2.600 ***

PYL 12/12/29
W/L - 2/1/20

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	395.05	179.19
2	398.89	176.84
3	403.94	174.48
4	411.91	168.63
5	514.46	177.97
6	737.45	187.02
7	807.29	234.53
8	818.39	249.67
9	824.54	262.85
10	827.29	267.35

*** 1.125 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	353.89	170.79
2	354.79	170.24
3	364.84	162.87
4	493.60	173.00
5	733.31	203.13
6	809.54	232.57
7	817.14	248.94
8	823.53	262.65
9	826.28	267.14

*** 1.144 ***

Failure Surface Specified By 8 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	332.19	166.76
2	335.16	164.59
3	472.77	173.94
4	730.03	201.61
5	813.44	238.65
6	823.83	252.82
7	828.93	263.74
8	831.68	268.24

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 12/12/89
 PYL 2/11/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	112.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient
 of .170 Has Been Assigned

. Vertical Earthquake Loading Coefficient
 of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random
 Technique For Generating Sliding Block Surfaces, Has Been

5	735.64	203.37
6	821.35	246.81
7	827.19	254.77
8	831.63	264.29
9	834.39	258.80

PYL 12/12/89
WIL 2 10/20

*** 1.165 ***

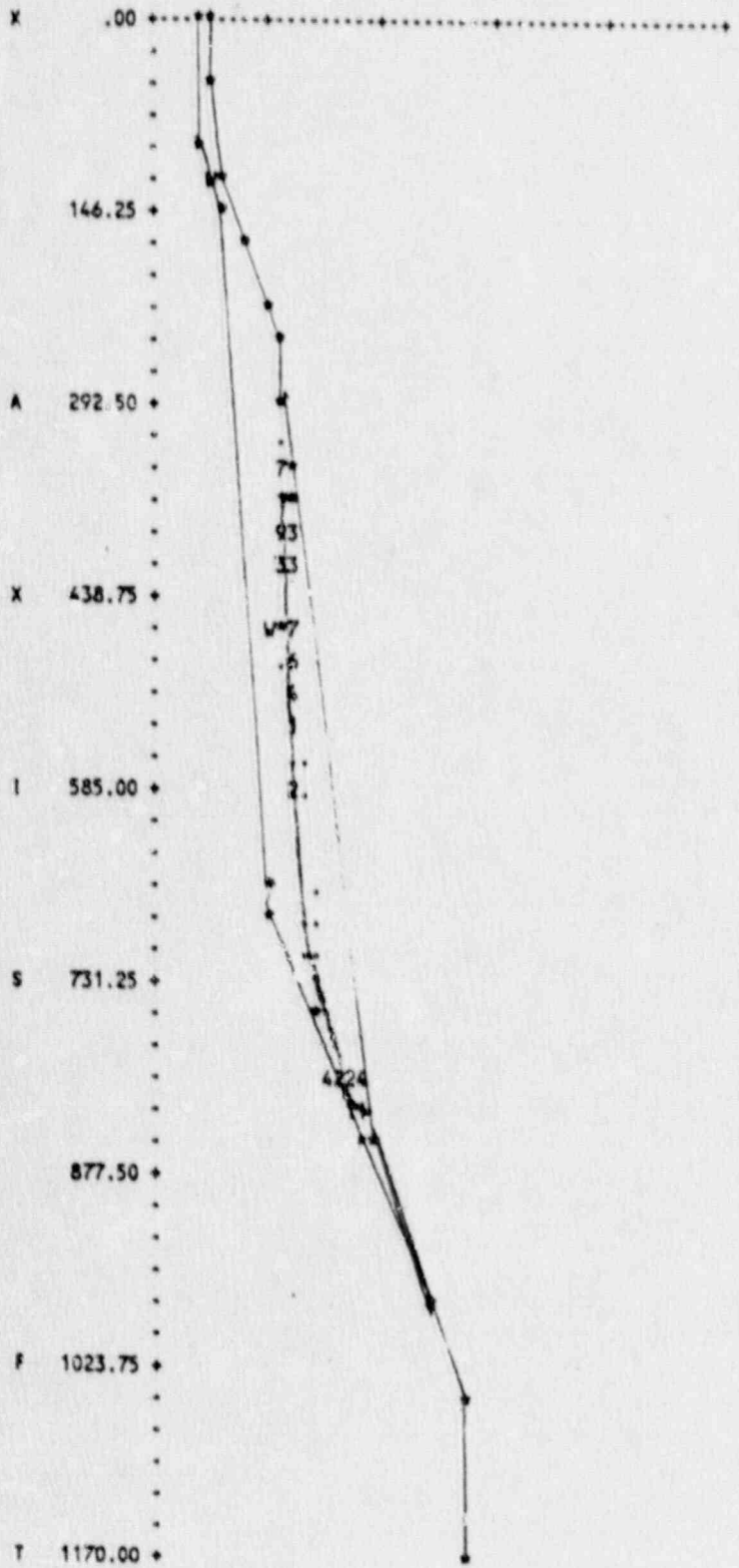
*** 1.165 ***

PYL 12/12/89

WYL 2/5/93

Y A X I S F T STSCULL

.00 146.25 292.50 438.75 585.00 731.25



** PCSTABL5 **

by
Purdue University

PYL 12/12/89
1172 2 13 133

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STSALL1.DAT
Output Filename: A:STSALL1B.OUT
Plotted Output Filename: A:STSALL1B.PLT

PROBLEM DESCRIPTION ULTRA-LOWMAH #1 SHORT TERM SEIMIC-THRU A
LLUVIUM I

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

PYL 12/12/89
WYL 2/2/90

*** 1.145 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	365.57	173.17
2	369.18	170.96
3	375.98	165.98
4	546.33	180.15
5	725.79	198.82
6	833.07	252.04
7	840.90	262.72
8	842.67	266.52
9	845.45	271.05

*** 1.148 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	381.19	176.36
2	385.00	174.03
3	387.62	172.81
4	400.00	163.73
5	483.28	173.47
6	731.67	202.37
7	836.49	252.11
8	846.76	266.12
9	847.39	267.47
10	850.18	272.02

*** 1.155 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	357.09	171.45
2	358.74	170.44
3	365.04	165.82
4	488.21	177.34

Specified.

PYL 12/12/89
WYL 2/12/90

The Active And Passive Portions Of The Sliding Surfaces
Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

3 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of
Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	153.00	370.00	153.00	5.00
2	460.00	155.00	550.00	155.00	10.00
3	620.00	160.00	680.00	160.00	20.00

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	329.62	166.29
2	338.74	159.61
3	347.18	154.93
4	535.60	150.98
5	666.39	169.26
6	678.14	190.45
7	687.79	203.62
8	700.47	230.80
9	704.06	238.51
10	706.66	242.74

ST = ALLI.DAT

*** 2.574 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
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1	358.74	171.78
2	360.77	170.54
3	370.51	163.40
4	577.27	178.75
5	730.72	201.09
6	802.21	233.43
7	810.96	245.36
8	818.56	261.65
9	821.30	266.13

PYL 12/12/89

WVL 2/5/90

*** 1.105 ***

1
Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	393.85	178.94
2	397.68	176.59
3	402.52	174.34
4	411.52	167.74
5	525.24	179.29
6	728.80	198.54
7	833.66	255.05
8	838.08	261.09
9	840.40	266.06
10	843.17	270.59

*** 1.116 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	391.91	178.55
2	395.74	176.20
3	400.24	174.10
4	411.72	165.68
5	519.85	173.63
6	731.13	198.78
7	795.47	226.29
8	808.34	243.84
9	816.44	261.22
10	819.19	265.70

*** 1.117 ***

PTL 12/12/89
11 - 2 1/2 100

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	325.19	165.49
2	333.52	159.38
3	349.04	150.78
4	348.79	151.90
5	644.24	167.03
6	655.77	187.84
7	665.68	201.35
8	678.36	228.54
9	680.82	233.81
10	683.38	238.00

*** 2.601 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	322.95	165.08
2	330.89	159.26
3	346.38	150.68
4	346.46	154.00
5	664.95	165.22
6	678.99	190.55
7	688.63	203.70
8	701.31	230.89
9	704.95	238.69
10	707.55	242.93

*** 2.612 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	329.16	166.21
2	338.20	159.59
3	352.06	151.90
4	513.45	150.24
5	646.00	163.40
6	659.81	188.31

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PXL 12/12/89

NY - 2/2/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param. (pcf)	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .170 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 pcf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been

PYL 12/12/89

2 1 7 3

1	341.27	168.41
2	352.44	160.22
3	366.86	152.23
4	485.94	150.19
5	677.81	155.64
6	692.75	181.88
7	698.42	192.82
8	707.85	205.67
9	720.53	232.86
10	725.14	242.77
11	727.77	247.05

*** 2.625 ***

5	726.42	243.03
6	729.05	247.31

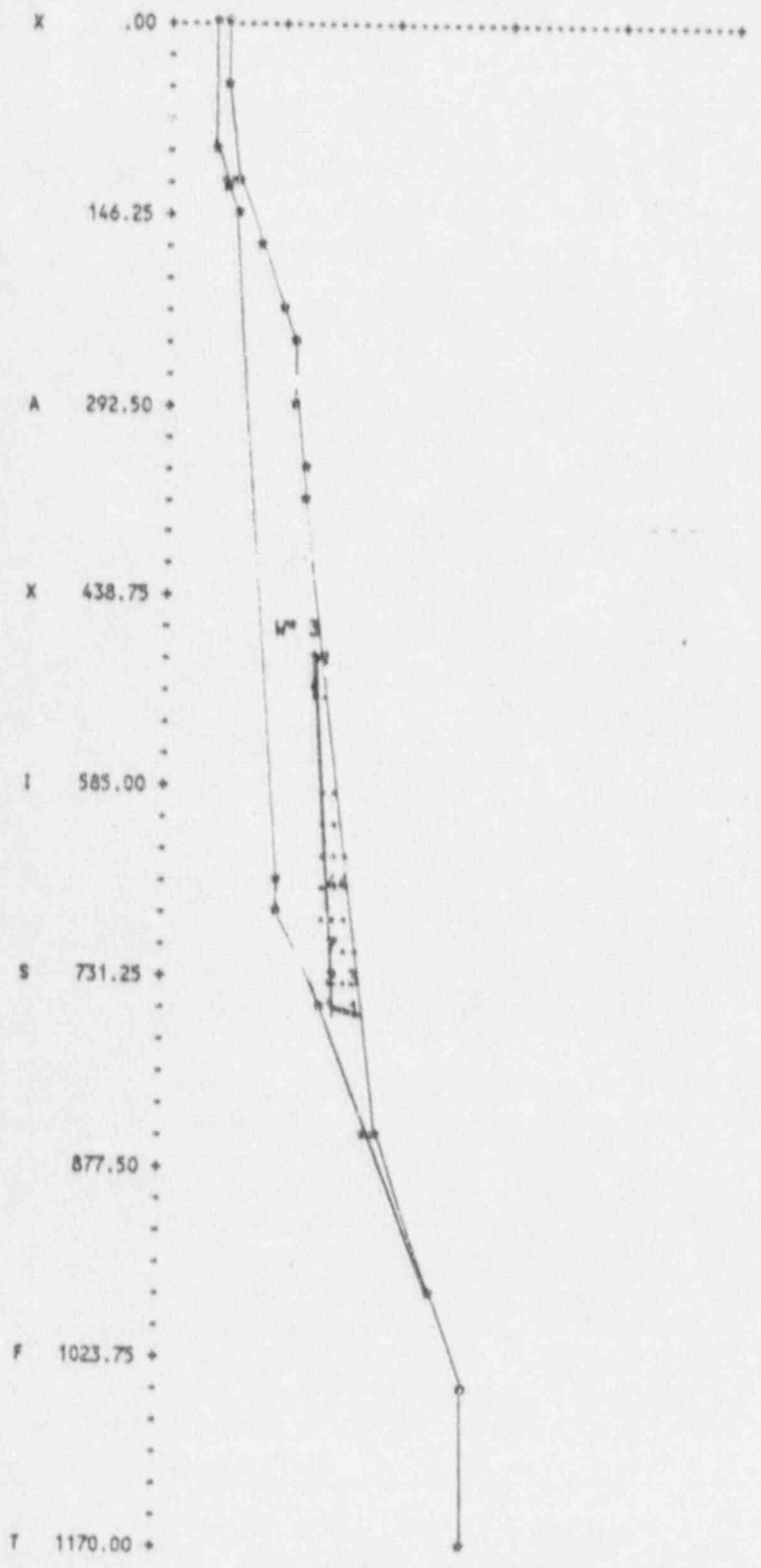
PYL 12/12/89
 W/L 2/2/90

*** 2.444 ***

Y A X I S F T

STSSA.

00 146.25 292.50 438.75 585.00 731.25



** PCSTABL5 **

by
Purdue University

PYL 12/12/89
WYL 2/15/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: B:STSALL2.DAT
Output Filename: B:STSALL2.OUT
Plotted Output Filename: B:STSALL2.PLT

PROBLEM DESCRIPTION ULTRA-LOWMAN #1 SHORT TERM SEISMIC-THEU A
LLUVIUM II

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	355.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

*** 2.437 ***

PYL 12/12/89
W/L 2/15/90

Failure Surface Specified By 7 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	453.99	191.21
2	457.96	188.78
3	464.41	185.77
4	734.79	220.03
5	747.47	247.22
6	747.51	247.29
7	750.16	251.62

*** 2.437 ***

1
Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	479.51	196.41
2	483.54	193.94
3	496.05	188.11
4	714.17	220.09
5	724.70	242.68
6	727.33	246.96

*** 2.438 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	479.83	196.48
2	483.86	194.01
3	491.77	190.32
4	736.51	222.52
5	748.12	247.41
6	750.77	251.74

*** 2.441 ***

Specified.

PYL 12/12/89
WYL 2/2/90

The Active And Passive Portions Of The Sliding Surfaces
Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

3 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of
Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	230.00	130.00	330.00	135.00	25.00
2	420.00	135.00	475.00	139.00	15.00
3	510.00	138.00	620.00	145.00	10.00

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 14 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	272.47	160.00
2	276.76	156.86
3	289.13	150.00
4	297.48	145.37
5	323.72	130.83
6	435.85	128.94
7	615.99	142.57
8	620.11	150.00
9	634.66	176.24
10	640.07	186.01
11	650.15	199.75
12	662.83	226.94
13	664.49	230.51
14	667.04	234.66

STS ALL 2 . DAT

*** 2.559 ***

Failure Surface Specified By 13 Coordinate Points

Specified.

PYL 12/12/89

WYL 2/5/90

The Active And Passive Portions Of The Sliding Surfaces
Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

2 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of
Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	460.00	186.00	520.00	193.00	5.00
2	590.00	205.00	750.00	220.00	10.00

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 6 Coordinate Points

STSC.DAT

Point No.	X-Surf (ft)	Y-Surf (ft)
1	487.44	198.03
2	491.49	195.55
3	500.73	191.24
4	746.67	224.54
5	758.29	249.47
6	760.96	253.82

*** 2.433 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	479.22	196.36
2	483.25	193.89
3	493.12	189.28
4	733.86	222.30

6	622.09	150.00
7	636.64	176.24
8	642.19	186.26
9	652.24	199.97
10	664.92	227.16
11	666.69	230.96
12	669.24	235.11

PYL 12/12/89

W/L 2'0"0"

*** 2.623 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	211.85	145.27
2	216.36	141.96
3	230.80	133.96
4	470.04	132.87
5	610.89	142.62
6	614.98	150.00
7	629.53	176.24
8	634.59	185.37
9	644.72	199.20
10	657.40	226.38
11	658.79	229.36
12	661.33	233.50

*** 2.624 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	283.90	160.00
2	287.54	157.33
3	300.77	150.00
4	311.61	143.99
5	424.04	133.06
6	609.55	145.79
7	611.88	150.00
8	626.42	176.24
9	631.27	184.98
10	641.45	198.86
11	654.13	226.05
12	655.34	228.66
13	657.88	232.79

*** 2.624 ***

** PCSTABL5 **

by
Purdue University

PYL 12/12/89
W/L 2/5/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STSC.DAT
Output Filename: A:STSD.OUT
Plotted Output Filename: A:STSD.PLT

PROBLEM DESCRIPTION: ULTRA-LOWMAN #1 SHORT TERM SEISMIC-THRU S
AND

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	5
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

PYL 12/12/89

11/12 2/12/89

1	218.93	149.77
2	223.69	146.28
3	224.78	145.68
4	251.01	131.13
5	461.82	132.66
6	603.91	141.11
7	608.84	150.00
8	623.38	176.24
9	628.02	184.60
10	638.23	198.53
11	650.91	225.72
12	651.96	227.98
13	654.49	232.10

*** 2.671 ***

Failure Surface Specified By 11 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	214.21	146.77
2	218.81	143.40
3	239.74	131.80
4	453.26	133.78
5	585.48	145.82
6	587.80	150.00
7	602.35	176.24
8	605.53	181.98
9	615.99	196.25
10	628.58	223.26
11	631.08	227.33

*** 2.687 ***

10	592.25	193.81
11	603.63	218.21
12	606.10	222.23

PXL 12/11/89
W/L 2/5/90

*** 5.058 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	267.45	160.00
2	272.02	156.65
3	284.01	150.00
4	298.93	141.71
5	432.77	130.19
6	565.56	139.28
7	571.50	150.00
8	586.04	176.24
9	588.10	179.94
10	598.75	194.48
11	610.46	219.59
12	612.94	223.63

*** 5.093 ***

** PCSTABL5 **

by
Purdue University

PYL 12/12/89
V1 - 2/2/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:LTN55A.DAT
Output Filename: A:LTN55A.OUT
Plotted Output Filename: A:LTN55A.PLT

PROBLEM DESCRIPTION ULTRA-LOWMAN #1 LONG TERM NON-SEISMIC-THR
U SAND

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

*** 4.951 ***

PYL 12/11/89
V/L 2 10/90

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	207.96	142.79
2	212.33	139.58
3	238.32	125.18
4	457.42	130.79
5	586.50	143.81
6	589.94	150.00
7	604.48	176.24
8	607.81	182.24
9	618.25	196.48
10	630.92	223.67
11	630.96	223.73
12	633.45	227.81

*** 4.984 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	215.76	147.76
2	220.41	144.35
3	231.19	138.37
4	465.10	131.86
5	618.19	142.97
6	622.09	150.00
7	636.64	176.24
8	642.19	186.26
9	652.24	199.97
10	664.92	227.16
11	666.69	230.96
12	669.24	235.11

*** 5.010 ***

Failure Surface specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
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2 Boxes Specified For Generation Of Central Block Base

PYL 12/12/87
WYL 2/2/90

Length Of Line Segments For Active And Passive Portions Of Sliding Block is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	460.00	186.00	520.00	193.00	5.00
2	590.00	205.00	750.00	220.00	10.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	457.69	191.97
2	462.25	189.64
3	469.97	186.04
4	633.39	212.82
5	639.26	225.41
6	641.30	229.41

L_WN SCA OPT

*** 4.504 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	461.80	192.80
2	466.37	190.47
3	473.78	187.02
4	639.79	213.94
5	645.76	226.73
6	647.80	230.74

*** 4.505 ***

Failure Surface Specified By 6 Coordinate Points

3 Boxes Specified For Generation Of Central Block Base

PRL 12/1/89
WYL 2/5/90

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	230.00	130.00	330.00	135.00	25.00
2	420.00	135.00	475.00	139.00	15.00
3	510.00	138.00	620.00	145.00	10.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 14 Coordinate Points

STASALL 2 OUT

Point No.	X-Surf (ft)	Y-Surf (ft)
1	272.47	160.00
2	276.76	156.86
3	289.13	150.00
4	297.48	145.37
5	323.72	130.83
6	435.85	128.94
7	615.99	142.57
8	620.11	150.00
9	634.66	176.24
10	640.07	186.01
11	650.15	199.75
12	662.83	226.94
13	664.49	230.51
14	667.04	234.66

*** 4.813 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	255.94	160.00
2	261.17	156.16
3	272.29	150.00
4	292.62	138.73
5	430.56	129.25

1	463.03	193.05
2	467.60	190.72
3	471.09	189.10
4	645.11	214.37
5	651.40	227.87
6	653.45	231.89

PYL 12/12/89
W/L 2/5/90

*** 4.534 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	461.34	192.71
2	465.90	190.38
3	469.09	188.89
4	607.29	209.48
5	612.17	219.94
6	614.10	223.88

*** 4.544 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	456.33	191.69
2	460.88	189.37
3	463.70	188.05
4	617.92	210.36
5	623.45	222.22
6	625.47	226.19

*** 4.545 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	453.73	191.16
2	458.28	188.84
3	460.13	187.98
4	597.14	207.47

** PCSTABL5 **

by
Purdue University

PYL 12/11/89
WYL 2 4 30

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: B:STNSALL2.DAT
Output Filename: B:STNSALL2.OUT
Plotted Output Filename: B:STNSALL2.PLT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 SHORT TERM NON-SEISMIC-TN
RU ALLUVIUM II

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	0
19	460.00	165.00	760.00	200.00	0
20	760.00	200.00	980.00	345.00	0
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

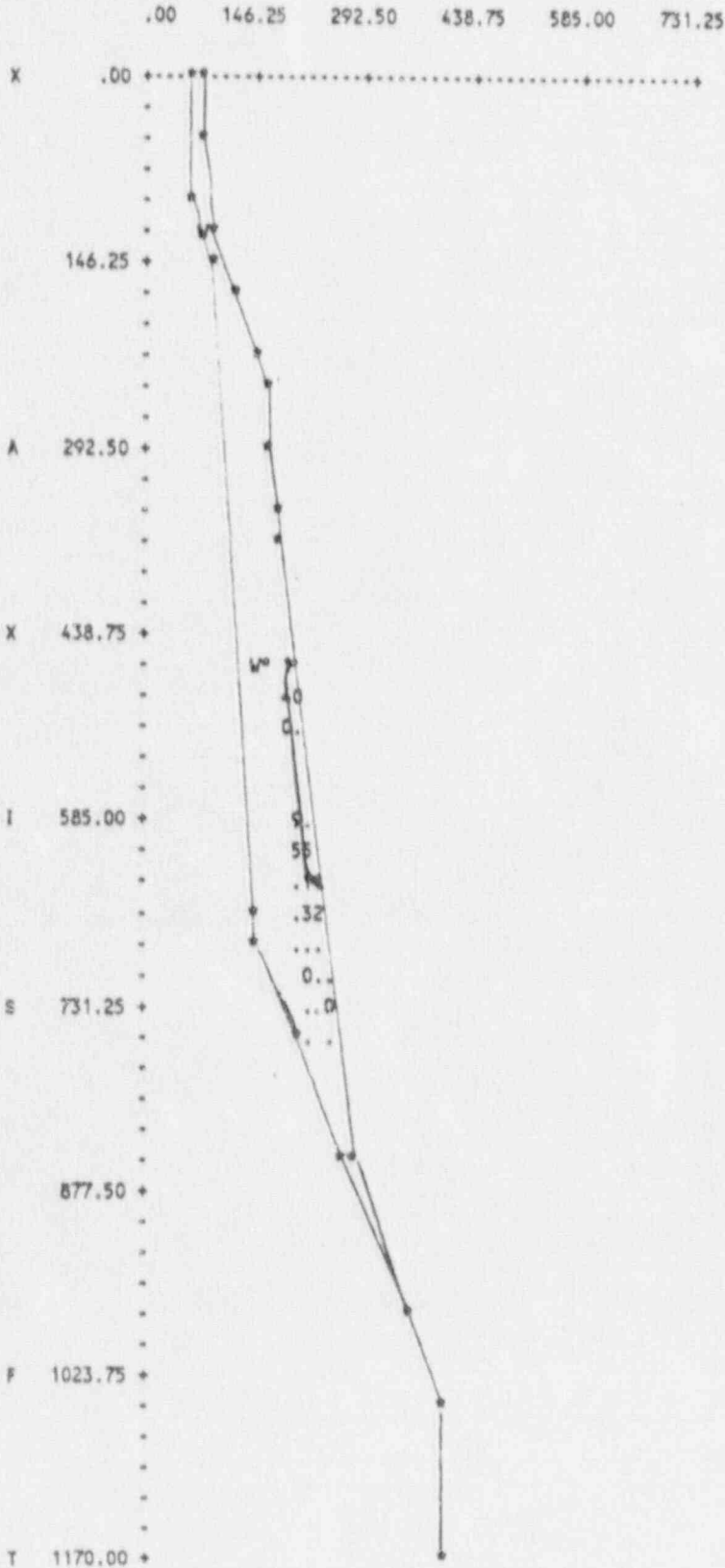
*** 4.581 ***

PYL 12/12/89

WYL 2/1/90

Y A X I S F T

LTNS 54



Failure Surface Specified By 10 Coordinate Points

PYL 12/11/89
WYL 2/12/90

Point No.	X-Surf (ft)	Y-Surf (ft)
1	327.38	165.89
2	336.10	159.49
3	352.19	150.58
4	530.90	153.72
5	633.49	168.25
6	643.56	186.41
7	653.59	200.11
8	666.27	227.30
9	668.11	231.24
10	670.66	235.40

*** 4.887 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	329.16	166.21
2	338.20	159.59
3	352.06	151.90
4	513.45	150.24
5	646.00	163.40
6	659.81	188.31
7	669.66	201.76
8	682.34	228.94
9	685.01	234.66
10	687.58	238.85

*** 4.888 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	325.42	165.53
2	333.79	159.39
3	346.60	152.29
4	469.86	152.27
5	654.43	169.96
6	664.94	188.91
7	674.74	202.28
8	687.42	229.47
9	690.34	235.74

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 12/12/89

W.L. 2/2/32

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	98.0	36.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	245.0	37.0	.00	.0	1
4	124.5	131.0	885.0	37.0	.00	.0	1
5	124.5	131.0	885.0	37.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

10 716.83 244.82

PYL 12/11/89

W/L 2/2/90

*** 4.817 ***

1
Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	321.13	164.75
2	328.74	159.17
3	339.32	153.30
4	546.06	150.78
5	656.02	168.69
6	667.38	189.19
7	677.16	202.52
8	689.83	229.71
9	692.88	236.25
10	695.46	240.46

*** 4.823 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	325.19	165.49
2	333.52	159.38
3	349.04	150.78
4	548.79	151.90
5	644.24	167.03
6	655.77	187.84
7	665.68	201.35
8	678.36	228.54
9	680.82	233.81
10	683.38	238.00

*** 4.846 ***

1
Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	326.59	165.74

10 849.13 271.60

PTL 12/12/89

WYL 2/5/90

*** 4.099 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	364.20	172.90
2	368.11	170.91
3	384.37	162.79
4	554.53	176.60
5	685.74	201.15
6	841.18	256.20
7	845.86	265.60
8	846.67	267.53
9	848.92	271.76

*** 4.105 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	380.19	176.16
2	384.54	173.94
3	387.08	172.75
4	398.24	167.19
5	531.80	174.45
6	710.76	200.21
7	830.14	251.98
8	833.30	258.31
9	836.55	265.28
10	838.79	269.69

*** 4.108 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	387.45	177.64
2	391.82	175.41

25	177.00	113.00	70.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 12/11/89
WYL 2/5/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param. (pcf)	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	130.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

PYL 12/12/89
11/12 21:30

Point No.	X-Surf (ft)	Y-Surf (ft)
1	377.65	175.64
2	382.00	173.43
3	384.10	172.45
4	394.92	167.05
5	503.44	170.74
6	671.24	196.12
7	829.26	249.97
8	833.47	258.42
9	836.69	265.31
10	838.94	269.72

*** 4.120 ***

Failure Surface Specified By 8 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	321.08	164.74
2	327.34	161.62
3	568.60	180.50
4	707.06	201.00
5	840.79	259.20
6	843.21	264.06
7	844.53	266.90
8	846.79	271.33

*** 4.122 ***

Failure Surface Specified By 9 Coordinate Points

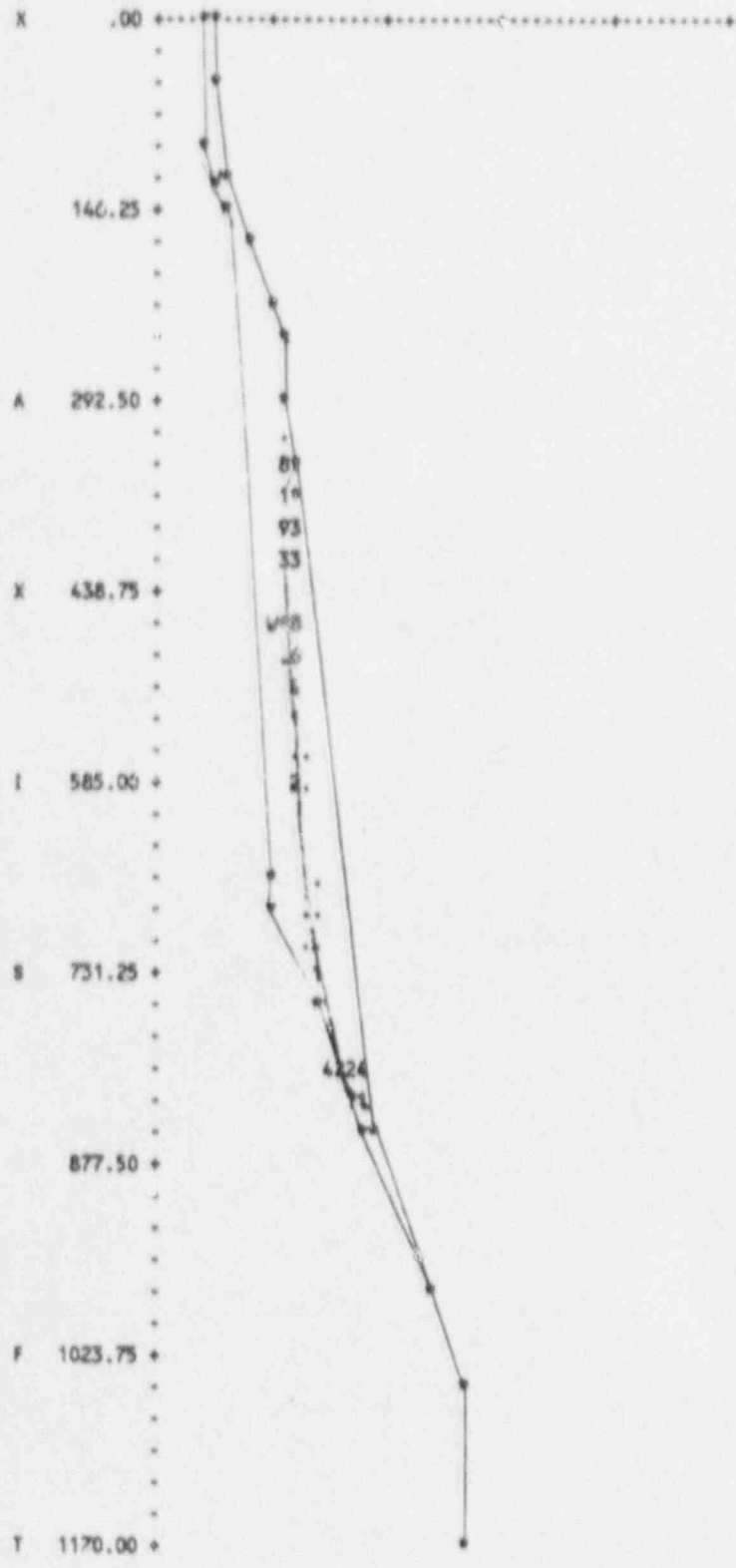
Point No.	X-Surf (ft)	Y-Surf (ft)
1	352.85	170.58
2	353.63	170.18
3	363.67	165.18
4	525.94	176.99
5	719.37	201.68
6	838.64	258.15
7	840.92	262.73
8	842.69	266.52
9	844.94	270.95

*** 4.122 ***

PXL 12/11/89
WYL 2/2 '90

Y A X I S F T

.00 146.25 292.50 438.75 585.00 731.25



***** - illegal Command *****

** PCSTABL5 **

by
Purdue University

PYL 12/12/89

11/1 2/1/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By:
Input Data Filename: A:LTNSALL1.DAT
Output Filename: A:LTNSALL1.OUT
Plotted Output Filename: A:LTNSALL1.PLT

PROBLEM DESCRIPTION ULTRA-LOWMAN #1 LONG TERM NON-SEISMIC-THR
U ALLUVIUM 1

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Brd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

3	403.94	174.48
4	411.91	168.63
5	514.46	177.97
6	733.45	199.02
7	807.29	234.53
8	818.39	249.67
9	824.54	262.85
10	827.29	267.35

PYL 12/11/89

PYL 2/2/90

*** 2.123 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	353.89	170.79
2	354.79	170.24
3	364.84	162.87
4	493.60	173.00
5	733.31	203.13
6	809.54	238.57
7	817.14	248.94
8	823.53	262.65
9	826.28	267.14

*** 2.165 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	365.57	173.17
2	369.18	170.96
3	379.98	165.98
4	546.33	180.15
5	725.79	198.82
6	833.07	252.04
7	840.90	262.72
8	842.67	266.52
9	845.45	271.05

*** 2.166 ***

Failure Surface Specified By 8 Coordinate Points

PYL 12/12/89

W/L 2/15/90

Segments For Active And Passive Portions Of
 Base is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	153.00	370.00	153.00	5.00
2	460.00	155.00	550.00	155.00	10.00
3	620.00	160.00	680.00	160.00	20.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 10 Coordinate Points

LINE ALL 1.0 FT

Point No.	X-Surf (ft)	Y-Surf (ft)
1	325.78	165.60
2	337.87	159.57
3	347.18	154.93
4	535.60	150.98
5	666.39	169.26
6	676.88	190.30
7	683.29	203.15
8	695.97	230.34
9	699.33	237.55
10	701.43	241.68

*** 5.154 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	317.63	164.12
2	327.66	159.12
3	339.32	153.30
4	546.06	150.78
5	656.02	168.69
6	666.17	189.05
7	672.66	202.06
8	685.33	229.25
9	688.15	235.29

4 Boxes Specified For Generation Of Central Block Base

PYL 12/11/89

11/1 - 2/1 1990

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	162.00	420.00	167.00	5.00
2	460.00	170.00	580.00	180.00	10.00
3	670.00	195.00	740.00	204.00	10.00
4	795.00	228.00	845.00	260.00	5.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 9 Coordinate Points

STNS COLL DAT

Point No.	X-Surf (ft)	Y-Surf (ft)
1	364.82	173.02
2	368.26	170.91
3	376.56	164.83
4	530.16	173.16
5	732.77	199.54
6	818.52	244.75
7	824.89	253.43
8	829.77	263.91
9	832.53	268.42

*** 2.069 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	358.74	171.78
2	360.77	170.54
3	370.51	163.40
4	577.27	178.75
5	730.72	201.09
6	802.21	233.43
7	810.96	245.36
8	818.56	261.65
9	821.30	266.13

2	321.76	158.86
3	334.80	152.35
4	530.55	151.96
5	638.88	169.11
6	647.75	186.90
7	654.37	200.19
8	667.05	227.37
9	668.93	231.41
10	671.00	235.47

PYL 12/12/89
 V1 - 2/1/20

*** 5.193 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	322.48	165.00
2	333.73	159.39
3	349.23	151.66
4	540.70	151.47
5	648.22	166.80
6	658.90	188.20
7	665.44	201.32
8	678.12	228.51
9	680.56	233.76
10	682.65	237.85

*** 5.194 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	309.08	162.56
2	316.94	158.64
3	327.53	153.36
4	516.75	150.47
5	640.46	167.84
6	650.10	187.18
7	656.71	200.43
8	669.38	227.61
9	671.39	231.90
10	673.46	235.97

*** 5.212 ***

** PCSTABL5 **

by
Purdue University

PYL 12/11/29

W" = 2/2/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STNSCOLL.DAT
Output Filename: A:STNSCOLB.OUT
Plotted Output Filename: A:STNSCOLB.PLT

PROBLEM DESCRIPTION ULTRA-LOWMAK #1 SHORT TERM NON-SEISMIC-TH
RU COLLUVIUM

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

10 702.07 241.81

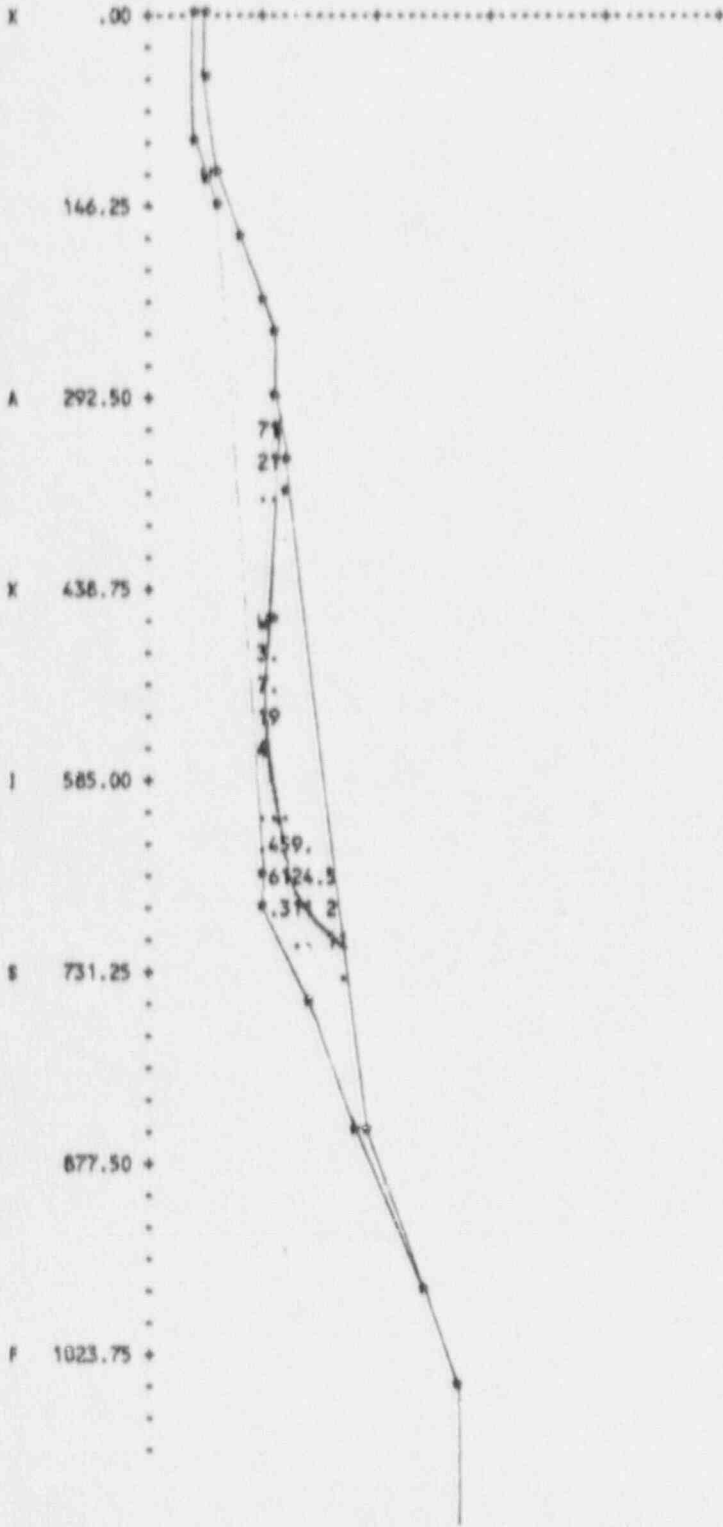
PYL 12/12/89

WYL 2/6/90

*** 5.237 ***

Y A X I S F T

.00 146.25 292.50 438.75 585.00 731.25



5	639.26	225.41
6	641.77	229.51

PYL 12/11/89
W/L 2/2/90

*** 4.679 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	492.74	199.11
2	496.80	196.62
3	509.05	190.91
4	716.36	221.45
5	726.42	243.03
6	729.05	247.31

*** 4.683 ***

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 14/12/29
W/L 2/1/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	96.0	36.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	245.0	37.0	.00	.0	1
4	124.5	131.0	885.0	37.0	.00	.0	1
5	124.5	131.0	885.0	37.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

PYL 12/11/89

NY - 2/5/90

Point No.	X-Surf (ft)	Y-Surf (ft)
1	479.51	196.41
2	483.54	193.94
3	496.05	188.11
4	714.17	220.09
5	724.70	242.68
6	727.33	246.96

*** 4.672 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	479.22	196.36
2	483.25	193.89
3	493.12	189.28
4	733.86	222.30
5	745.30	246.84
6	747.95	251.17

*** 4.673 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	492.13	198.99
2	496.19	196.50
3	510.40	189.87
4	713.70	221.23
5	723.60	242.46
6	726.22	246.73

*** 4.674 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
-----------	-------------	-------------

6	611.86	149.13
7	612.29	150.00
8	625.67	176.85
9	629.63	184.79
10	636.39	198.34
11	649.07	225.53
12	650.03	227.59
13	652.08	231.61

PTL 12/12/89
WYL 2/2/90

*** 5.282 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	279.50	160.00
2	285.07	157.23
3	299.56	150.00
4	311.61	143.99
5	424.04	133.06
6	609.55	145.79
7	611.65	150.00
8	625.03	176.85
9	628.95	184.71
10	635.71	198.27
11	648.39	225.46
12	649.32	227.45
13	651.37	231.47

*** 5.321 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	205.78	141.40
2	211.00	138.80
3	211.47	138.56
4	238.52	125.18
5	457.42	130.79
6	586.50	143.81
7	589.59	150.00
8	602.98	176.85
9	605.53	181.98
10	612.47	195.88
11	624.88	222.51
12	626.91	226.48

25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

PYL 12/11/89

WV 2/2/90

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	930.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

1	214.27	146.81
2	219.86	144.02
3	231.19	138.37
4	465.10	131.86
5	618.19	142.97
6	621.70	150.00
7	635.09	176.85
8	639.63	185.96
9	646.31	199.36
10	658.99	226.55
11	660.46	229.70
12	662.52	233.74

PXL 12/2/89
 11/2 2'2'9"

*** 5.392 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	248.98	160.00
2	257.05	155.98
3	269.04	150.00
4	284.36	142.36
5	466.20	132.33
6	595.04	148.04
7	596.02	150.00
8	609.41	176.85
9	612.36	182.78
10	619.25	196.58
11	631.92	223.77
12	632.01	223.95
13	634.04	227.93

*** 5.408 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	276.84	160.00
2	282.62	157.12
3	296.89	150.00
4	317.18	138.89
5	424.55	129.98
6	558.57	137.78
7	564.67	150.00
8	578.06	176.85
9	579.07	178.89
10	586.20	193.19

UMTRA - LOW B-26
 RC Δ Slope Stability
 PYL 7/17/90

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	862.36	277.24
2	867.19	274.28
3	949.13	326.21
4	950.10	327.78

*** 3.852 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	855.65	273.37
2	860.32	270.51
3	948.20	325.53
4	949.28	327.31

*** 3.861 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	861.45	276.71
2	866.02	273.91
3	942.61	322.11
4	943.89	324.20

*** 3.866 ***

Y A X I S F Y

.00 146.25 292.50 438.75 585.00 731.25

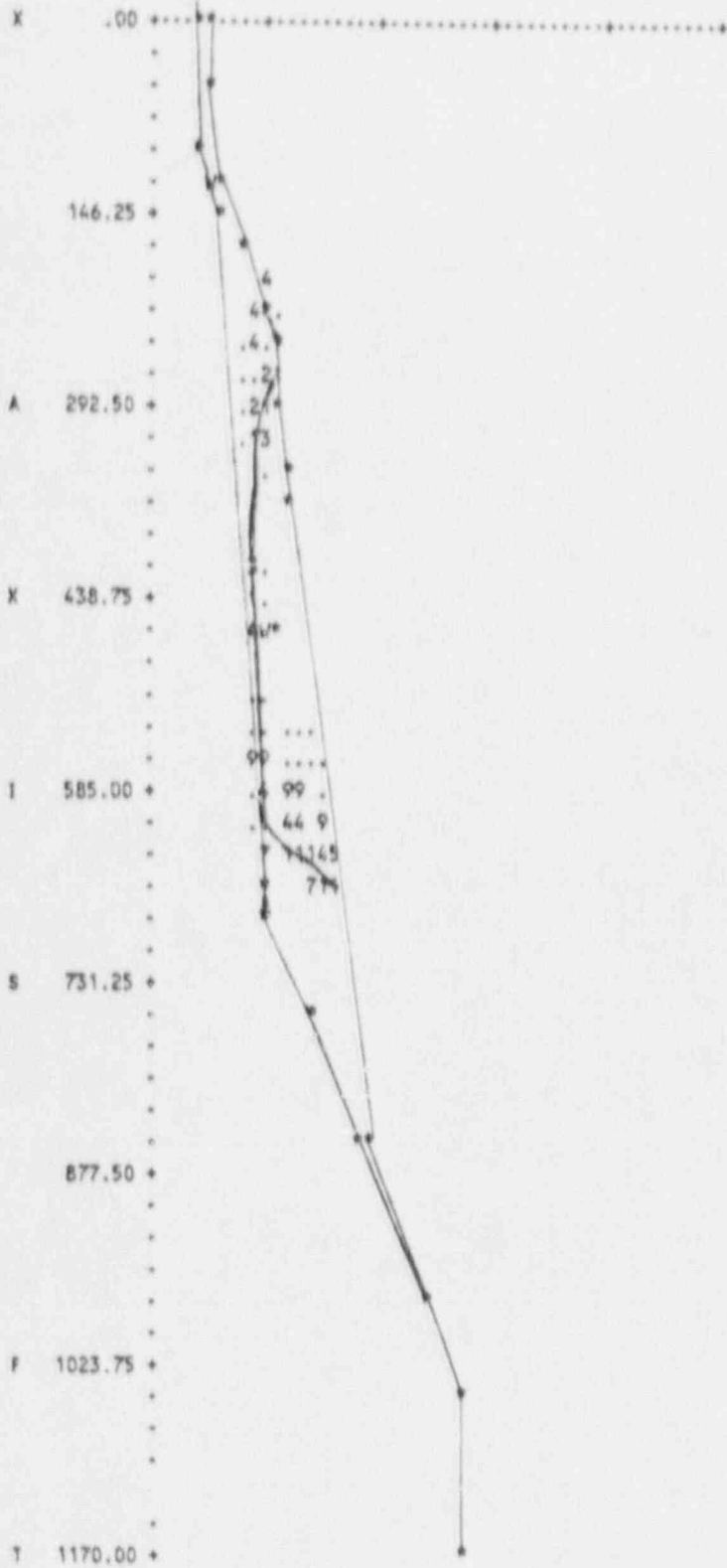


PYL 12/10/79
WVL 2/5/80

Y A X I S F T

LTNEALL2.

.00 146.25 292.50 438.75 585.00 731.25



Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 20.0

ULTIMATE - LOW
 Rev D Slope Stability
 P.L. 2.17.92
 checked K. Joe 8/2/96

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	860.00	270.00	900.00	295.00	2.00
2	930.00	315.00	950.00	327.00	1.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	859.08	275.35
2	864.50	272.03
3	930.13	314.79
4	931.53	317.08

*** 3.500 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	864.19	278.29
2	869.54	275.02
3	942.18	322.39
4	942.97	323.67

*** 3.713 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	863.28	277.77
2	868.37	274.65
3	935.66	318.30
4	936.76	320.09

*** 3.735 ***

26 670.00 142.00 680.00 150.00

PYL 12/14/89
6
WYL 2/1-133

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	250.0	21.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	750.0	27.0	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .230 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

** PCSTABL5 **

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

UMTRA-LOW ^{B-2}
Rev Δ Slope Stability
PYL 7/17/90
checked K. Yee 8/3/90

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: B:LTSI.DAT
Output Filename: B:LTSI.OUT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 LONG TERM SEISMIC-I.S. ✓

BOUNDARY COORDINATES

B Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	115.00	6
25	177.00	115.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

ISOTROPIC SOIL PARAMETERS

6 648.63 230.91

PXL 12/12/89

W = 2.2 2;

*** 2.011 ***

1

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	458.60	192.15
2	462.25	189.64
3	469.97	186.04
4	633.39	212.82
5	679.26	225.41
6	642.13	229.58

*** 2.011 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	472.45	194.97
2	476.13	192.45
3	481.63	189.88
4	638.23	214.39
5	643.80	226.33
6	646.67	230.51

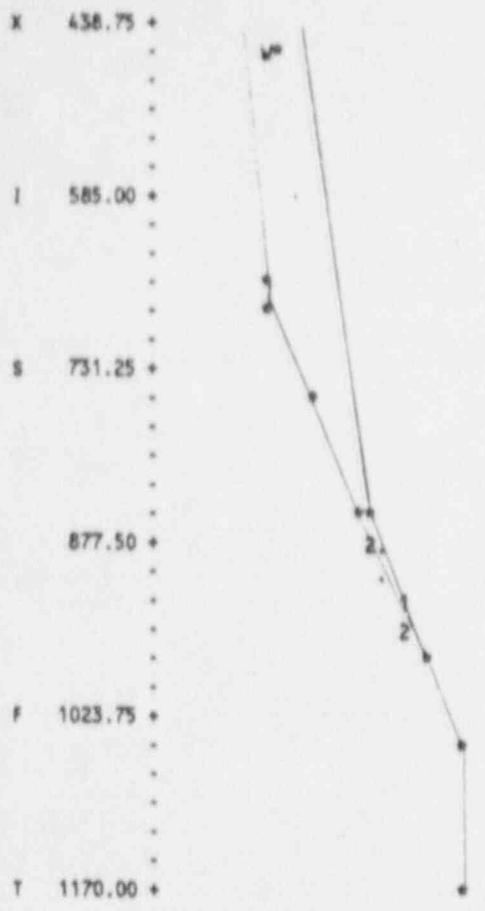
*** 2.016 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	463.94	193.24
2	467.60	190.72
3	471.09	189.10
4	645.11	214.37
5	651.40	227.87
6	654.28	232.06

*** 2.018 ***

UNITED LOW ²⁻¹¹
REV Δ Slope Stability
PYL 2/17/90



***** - Illegal Command *****

- Legal Commands -
- PROFIL
 - LOADS
 - TIES
 - WATER
 - SURFAC
 - EXECUT
 - EQUAKE
 - SOIL
 - RANDOM
 - CIRCLE
 - CIRCL2
 - BLOCK
 - BLOCK2
 - LIMITS
 - ANISO
 - SURBIS
 - SPENCR

PXL 12/12/89
W/L 2.035

Point No.	X-Surf (ft)	Y-Surf (ft)
1	477.14	195.93
2	480.82	193.40
3	491.30	188.51
4	605.26	211.03
5	609.11	219.32
6	611.93	223.42

*** 2.035 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	479.85	196.48
2	483.54	193.94
3	496.05	188.11
4	714.17	220.09
5	724.70	242.68
6	727.70	247.04

*** 2.036 ***

LIMITA-LOW B-

Rev Δ Slope Stability

PYL 7/7/90

checked K. Lee 2/3/92

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	858.50	275.02
2	864.50	272.03
3	930.13	314.79
4	931.17	316.87

*** 2.741 *** ✓

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	863.62	277.97
2	869.54	275.02
3	942.18	322.39
4	942.76	323.55

*** 2.834 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	862.74	277.46
2	868.37	274.65
3	935.66	318.30
4	936.47	319.93

*** 2.842 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	856.14	273.66
2	861.37	271.05

** PCSTABL5 **

PYL 11/12/89
WYL 2/2/90

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:LTSCOLL.DAT
Output Filename: A:LTSCOLL2.OUT
Plotted Output Filename: NOHE

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 LONG TERM SEIMIC-THRU CO
LLUVIUM

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6

** PCSTABL5 **

by
Purdue University

UMTRA-LOW 5d B-11
REV Δ Slope Stability
PYL 7/17/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: B:LTNSI.DAT
Output Filename: B:LTNSI.OUT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 LONG TERM NON-SEISMIC-I.S ✓

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	267.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

ISOTROPIC SOIL PARAMETERS

Specified.

PYL 12/12/89
WYL 2/1/90

The Active And Passive Portions Of The Sliding Surfaces
Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

4 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of
Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	162.00	420.00	167.00	5.00
2	460.00	170.00	580.00	180.00	10.00
3	670.00	195.00	740.00	204.00	10.00
4	795.00	228.00	845.00	260.00	5.00

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	363.86	172.83
2	366.75	170.84
3	376.56	164.83
4	530.16	173.16
5	732.77	199.54
6	818.52	244.75
7	823.26	252.49
8	828.47	263.65
9	831.61	268.23

*** 1.599 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
--------------	----------------	----------------

UMTRA-LOW 34.13-17
 Row Δ Slope Stability
 PTL 7/17/90

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	862.81	277.50
2	867.19	274.28
3	949.13	326.21
4	950.42	327.96

*** 2.072 ***

1
 Failure Surface Specified By 4 Coordinate Points

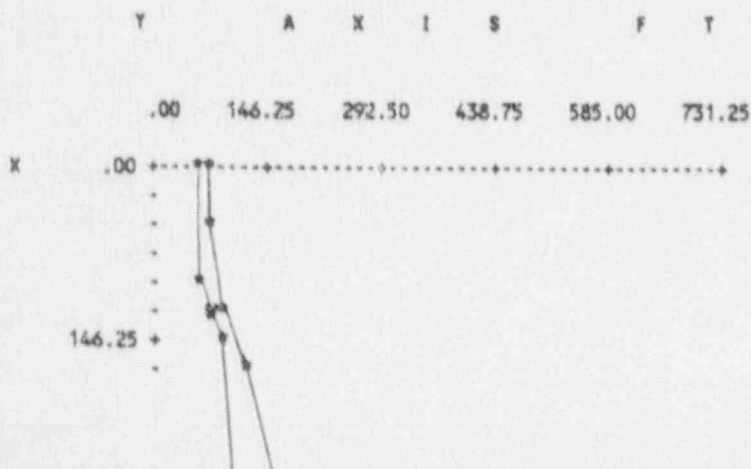
Point No.	X-Surf (ft)	Y-Surf (ft)
1	856.08	273.62
2	860.32	270.51
3	948.20	325.53
4	949.65	327.52

*** 2.074 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	861.87	276.96
2	866.02	273.91
3	942.61	322.11
4	944.33	324.65

*** 2.078 ***



PYL 12/12/87
WYL 2/5/90

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	369.27	173.93
2	372.73	171.55
3	373.21	171.33
4	380.66	166.76
5	536.96	174.22
6	711.58	195.58
7	816.66	242.71
8	822.32	251.95
9	827.71	263.49
10	830.85	268.08

*** 1.622 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	392.88	178.75
2	396.39	176.33
3	401.00	174.18
4	411.2	167.74
5	525.24	179.29
6	728.80	198.54
7	833.66	255.05
8	836.95	260.43
9	839.49	265.88
10	842.66	270.48

*** 1.623 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	394.23	179.02
2	397.74	176.61
3	402.59	174.35
4	411.91	168.63
5	514.46	177.97
6	733.45	199.02
7	807.29	234.53

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 20.0

UMTIRA - LOW Sh B. 3
 Rev Δ Slope Stability
 P.Y.L. 7/17/90
 checked K. Roe 8/3/90

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	860.00	270.00	900.00	295.00	2.00
2	930.00	315.00	950.00	327.00	1.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	859.58	275.64
2	864.50	272.03
3	930.13	314.79
4	932.01	317.36

*** 1.902 *** ✓

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	864.68	278.58
2	869.34	275.02
3	942.18	322.39
4	943.23	323.82

*** 2.008 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	863.74	278.04
2	868.37	274.65
3	935.66	318.30
4	937.13	320.31

*** 2.015 ***

PYL 12/12/89
WV - 2/2/90

1	389.77	178.11
2	393.27	175.70
3	397.34	173.81
4	409.76	166.19
5	573.75	180.27
6	707.88	196.38
7	827.31	249.93
8	831.98	257.55
9	835.48	265.07
10	838.64	269.66

*** 1.642 ***

LTSCOLL.

** PCSTABL5 **

by
Purdue University

UMTRA-LOW

sh B-

PC-Δ Slope Stability

PYL 7/1/90

checked K. Lee 8/3/90

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: B:STSI.DAT
Output Filename: B:STSI.OUT



PROBLEM DESCRIPTION UMTRA-LOWMAN #1 SHORT TERM SEISMIC-I.S. ✓

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Brd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

ISOTROPIC SOIL PARAMETERS

** PCSTABL5 **

PYL 12/12/89

WYL 2/8/90

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:LTSALL1.DAT
Output Filename: A:LTSALL1.OUT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 LONG TERM SEIMIC-THRU AL
LUVIUM I

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6

ULTRA-LOW
REV Δ Slope Stability
PYL 7/17/90

3	949.13	326.21
4	950.42	327.96

*** 2.735 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	856.08	273.62
2	860.32	270.51
3	948.20	325.53
4	949.65	327.52

*** 2.738 ***

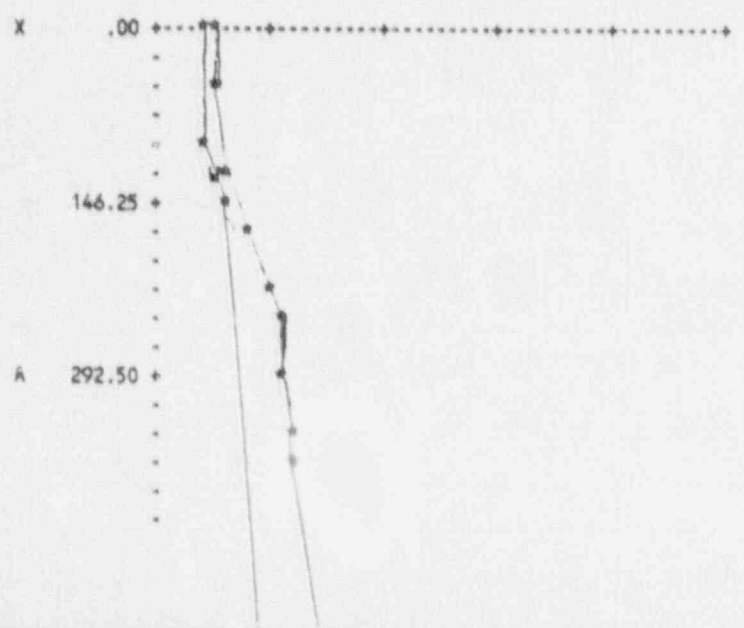
Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	861.87	276.96
2	866.02	273.91
3	942.61	322.11
4	944.33	324.45

*** 2.743 ***

Y A X I S F T

.00 146.25 292.50 438.75 585.00 731.25



The Active And Passive Portions Of The Sliding Surfaces
Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

3 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of
Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	153.00	370.00	153.00	5.00
2	460.00	155.00	550.00	155.00	10.00
3	620.00	160.00	680.00	160.00	20.00

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	328.24	166.04
2	338.74	159.61
3	347.18	154.93
4	535.60	150.98
5	666.39	169.26
6	678.14	190.45
7	686.10	203.44
8	698.78	230.63
9	702.28	238.15
10	705.24	242.46

*** 2.248 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
-----------	-------------	-------------

UMTRA-LOW
Rev Δ Slope Stability
PYL 2/17/90
Checked K. Kee 2/3/90

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	859.58	275.64
2	864.50	272.03
3	930.13	314.79
4	932.01	317.36

*** 2.513 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	864.68	278.58
2	869.54	275.02
3	942.18	322.39
4	943.23	323.82

*** 2.651 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	863.74	278.04
2	868.37	274.65
3	935.66	318.50
4	937.13	320.31

*** 2.665 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	869.53	281.37
2	874.07	278.04

PYL 12/12/29
HVL 2/2/30

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	323.93	165.26
2	333.52	159.38
3	349.04	150.78
4	548.79	151.90
5	644.24	167.03
6	655.77	187.84
7	663.94	201.17
8	676.62	228.36
9	678.99	233.44
10	681.92	237.70

*** 2.274 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	321.75	164.86
2	330.89	159.26
3	346.38	150.68
4	546.46	154.00
5	664.95	165.22
6	678.99	190.55
7	686.94	203.53
8	699.62	230.72
9	703.17	238.33
10	706.13	242.64

*** 2.277 ***

Failure Surface Specified By 11 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	339.58	168.10
2	352.44	160.22
3	366.86	152.23
4	485.94	150.19
5	677.81	155.64
6	692.36	181.88
7	698.42	192.82

** PCSTABL5 **

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

UMTRA-LOW sh B-
3885-48

Rev. Δ Slope stability Analysis

PYL 7/17/90

checked K. Joe 8/3/90

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: B:STNSI.DAT
Output Filename: B:STNSI.OUT



PROBLEM DESCRIPTION UMTRA-LOWMAN #1 SHORT TERM NON-SEISMIC-1,
S.

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

ISOTROPIC SOIL PARAMETERS

PXL 12/12/89

Y1 - 2/5 '00

No.	(ft)	(ft)
1	315.20	163.67
2	322.97	158.91
3	334.80	152.35
4	530.55	151.96
5	638.88	169.11
6	648.81	187.03
7	657.04	200.46
8	669.72	227.65
9	671.74	231.98
10	674.65	236.22

*** 2.291 ***

1756111

** PCSTABL5 **

PYL 12/12/89
WYL 2/5/90

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:LTSALL2.DAT
Output Filename: A:LTSALL2.OUT

PROBLEM DESCRIPTION UMTA-LOWMAN #1 LONG TERM SEIMIC-THRU AL
LUVIUM II

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6

PYL 12/13/27
WVL 2/5/27

2	228.83	138.88
3	258.56	134.87
4	288.39	138.09
5	316.75	147.86
6	346.72	149.24
7	376.39	144.81
8	404.85	154.31
9	434.37	159.64
10	461.50	172.45
11	491.25	176.32
12	521.09	173.23
13	550.72	177.91
14	580.67	179.69
15	609.93	186.31
16	635.41	202.14
17	660.17	219.08
18	688.56	228.77
19	709.42	243.31

*** 2.176 ***

Failure Surface Specified By 24 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	213.33	146.21
2	236.92	127.68
3	261.82	110.93
4	291.53	115.10
5	321.52	115.62
6	351.52	115.76
7	378.46	128.97
8	408.16	133.19
9	436.91	124.62
10	466.86	122.92
11	494.35	134.95
12	523.61	141.54
13	550.46	154.92
14	580.44	156.19
15	610.34	158.57
16	637.16	172.02
17	667.15	172.81
18	694.97	184.04
19	709.63	210.21
20	733.39	228.52
21	762.61	235.30
22	788.82	249.91
23	818.73	252.18
24	839.37	269.81

*** 2.206 ***

PYL 12/2/89
Y/L - 2/5/90

The Active And Passive Portions Of The Sliding Surfaces
Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

3 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of
Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	230.00	130.00	330.00	135.00	25.00
2	420.00	135.00	475.00	139.00	15.00
3	510.00	138.00	620.00	145.00	10.00

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 14 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	271.63	160.00
2	276.76	156.86
3	289.13	150.00
4	297.48	145.37
5	323.72	130.83
6	435.85	128.94
7	615.99	142.57
8	620.11	150.00
9	634.66	176.24
10	640.07	186.01
11	648.38	199.57
12	661.06	226.76
13	662.64	230.14
14	665.54	234.36

*** 2.217 ***

Failure Surface Specified By 11 Coordinate Points

PYL 12/13/89
WY - 2/5/90

16	611.83	188.12
17	639.40	199.95
18	666.77	212.23
19	694.43	223.84
20	724.18	227.73
21	754.18	228.07
22	780.52	242.42
23	807.56	255.42
24	811.54	264.14

*** 2.054 ***

Failure Surface Specified By 22 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	191.67	132.42
2	216.40	115.44
3	246.34	113.52
4	275.57	120.25
5	304.98	114.34
6	334.10	121.55
7	363.94	124.72
8	389.51	140.40
9	418.40	148.49
10	447.12	139.84
11	476.35	133.08
12	505.98	137.82
13	535.94	139.37
14	557.18	160.56
15	585.01	171.74
16	610.15	188.12
17	639.73	193.13
18	661.71	213.55
19	684.07	233.55
20	714.05	234.60
21	740.17	249.35
22	740.27	249.60

*** 2.137 ***

1

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	184.44	127.83
2	210.02	112.15
3	239.86	115.26
4	266.78	128.52

P.L. 12/12/89
W.L. 2/5/92

8	642.19	186.26
9	650.48	199.79
10	663.16	226.98
11	664.84	230.58
12	667.75	234.81

*** 2.264 ***

1
Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	211.41	144.99
2	216.36	141.96
3	230.80	133.96
4	470.04	132.87
5	610.89	142.62
6	614.98	150.00
7	629.53	176.24
8	634.59	185.37
9	642.95	199.01
10	655.63	226.20
11	656.92	228.98
12	659.81	233.19

*** 2.265 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	283.19	160.00
2	287.54	157.33
3	300.77	150.00
4	311.61	143.99
5	424.04	133.06
6	609.55	145.79
7	611.88	150.00
8	626.42	176.24
9	631.27	184.98
10	639.66	198.68
11	652.34	225.87
12	653.47	228.28
13	656.36	232.48

*** 2.278 ***

PYL 12/13/29
PYL 2-1-29

100 Trial Surfaces Have Been Generated.

10 Surfaces Initiate From Each Of 10 Points Equally Spaced
Along The Ground Surface Between X = 170.00 ft.
and X = 235.00 ft.

Each Surface Terminates Between X = 550.00 ft.
and X = 850.00 ft.

Unless Further Limitations Were Imposed, The Minimum Elevation
At Which A Surface Extends Is Y = 110.00 ft.

30.00 ft. Line Segments Define Each Trial Failure Surface.

1

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 23 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	170.00	118.64
2	198.80	110.22
3	228.30	115.63
4	257.04	124.25
5	287.04	124.38
6	316.52	118.84
7	346.49	117.44
8	375.87	111.38
9	405.53	115.91
10	435.05	121.23
11	464.81	125.07
12	493.67	133.23
13	522.44	141.76
14	551.43	149.45
15	581.43	149.04
16	611.15	144.93
17	639.35	155.17
18	668.06	163.86
19	684.60	188.89
20	703.75	211.98
21	726.55	231.47
22	754.00	243.59
23	778.71	257.44

PXL 12/12/89
WY 2/2/90

2	259.59	156.09
3	270.58	150.00
4	284.36	142.36
5	466.20	132.33
6	595.04	148.04
7	596.13	150.00
8	610.68	176.24
9	614.43	183.02
10	622.98	196.96
11	635.66	224.15
12	635.93	224.74
13	638.79	228.90

*** 2.311 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	209.90	144.03
2	214.79	141.04
3	227.61	133.93
4	253.85	119.39
5	453.50	138.64
6	601.87	145.13
7	604.57	150.00
8	619.11	176.24
9	623.45	184.07
10	631.91	197.88
11	644.59	225.07
12	645.32	226.64
13	648.20	230.82

*** 2.322 ***

LTSALL2

PYL 12/13/79
W/L 2/1/79

** PCSTABL5 **

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:LTSRAN2.DAT
Output Filename: A:LTSRAN2.OUT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 LONG TERM SEIMIC-RANDOM
2

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6

PYL 12/12/89
1.1 - 2.2.3

** PCSTABL5 **

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STNSRAN2.DAT
Output Filename: STNSRAN2.OUT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 SHORT TERM NON-SEIMIC-RA
NDOM 2

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6

PYL 12/13/87
 WYL 2/1/88

6	3	118.11
7	3	136.33
8	3	150.31
9	4	159.64
10	4	164.11
11	4	165.72
12	5	162.56
13	5	169.70
14	5	172.75
15	5	180.28
16	6	193.95
17	6	205.33
18	6	220.91
19	6	227.73

*** 2.7 ***

Failure Surface Defined By 19 Coordinate Points

Point No.	Y-Coord	Y-Surf (ft)
1	1	107.02
2	2	138.88
3	2	134.87
4	2	138.09
5	3	147.86
6	3	149.24
7	3	114.81
8	4	154.31
9	4	159.64
10	4	172.45
11	4	176.32
12	5	173.23
13	5	177.91
14	5	170.69
15	6	196.31
16	6	202.14
17	6	210.08
18	6	223.77
19	7	243.31

*** 2.7 ***

and X = 850.00 ft.

PYL 12/2/79

WYL 2/15/80

Unless Further Limitations Were Imposed, The Minimum Elevation At Which A Surface Extends Is Y = 110.00 ft.

30.00 ft. Line Segments Define Each Trial Failure Surface.

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 24 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	177.22	123.23
2	204.36	110.44
3	233.07	119.12
4	259.86	132.62
5	289.48	137.43
6	319.04	142.52
7	348.99	144.20
8	377.88	152.30
9	405.66	163.63
10	435.49	166.81
11	465.39	164.44
12	495.33	166.36
13	525.22	163.72
14	554.13	171.70
15	582.80	180.56
16	611.83	188.12
17	639.40	199.95
18	666.77	212.23
19	694.43	223.84
20	724.18	227.73
21	754.18	228.07
22	780.52	242.42
23	807.56	255.42
24	811.54	264.14

STNSK1122 OK

*** 4.032 ***

Failure Surface Specified By 19 Coordinate Points

Point	X-Surf	Y-Surf
-------	--------	--------

18	610.00	203.16
19	610.00	203.18
20	610.00	204.48

PTL 12/13/87
W/L 2/5/90

*** 4.727 ***

Failure Surface Specified By 23 Coordinate Points

Point No.	X-Surf	Y-Surf (ft)
1	177.00	118.64
2	199.52	113.26
3	220.53	120.68
4	251.52	136.10
5	281.12	139.56
6	310.00	135.92
7	327.00	117.05
8	327.00	113.69
9	303.00	123.65
10	410.00	115.48
11	410.00	110.98
12	410.00	114.05
13	510.00	113.91
14	510.00	113.87
15	510.00	113.47
16	510.00	118.41
17	610.00	111.75
18	610.00	110.90
19	610.00	119.62
20	610.00	113.98
21	710.00	112.24
22	710.00	119.58
23	710.00	119.14

*** 4.700 ***

1

Failure Surface Specified By 22 Coordinate Points

Point No.	X-Surf	Y-Surf (ft)
1	100.00	102.42
2	200.00	115.44
3	200.00	113.52
4	270.00	120.25
5	300.00	114.34
6	300.00	121.55
7	300.00	114.72
8	300.00	110.40
9	400.00	119.49

PYL 12/2/89
WYL 2/5/90

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	206.11	141.62
2	236.05	143.48
3	265.78	139.46
4	295.61	142.68
5	323.97	152.46
6	353.94	153.83
7	383.61	149.41
8	412.07	158.90
9	441.59	164.24
10	468.72	177.05
11	498.47	180.92
12	528.31	177.82
13	557.94	182.51
14	587.89	184.29
15	617.15	190.90
16	642.63	206.74
17	667.39	223.68
18	695.78	233.37
19	710.31	243.69

*** 4.273 ***

1
Failure Surface Specified By 21 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	184.44	127.83
2	210.46	112.89
3	240.40	110.95
4	269.53	118.10
5	299.53	117.67
6	328.90	111.53
7	358.88	110.50
8	388.71	113.70
9	415.72	126.75
10	442.70	139.87
11	472.30	144.72
12	500.70	154.39
13	529.41	163.10
14	559.05	167.76
15	588.90	164.77
16	615.63	178.38
17	643.48	189.55
18	673.30	192.81
19	699.96	206.57

No.	(ft)	(ft)
1	17	118.64
2	17	110.22
3	223	115.63
4	237	124.25
5	207	124.38
6	316	118.84
7	317	117.44
8	379	111.38
9	405	115.91
10	405	121.23
11	47	125.07
12	47	133.23
13	57	141.76
14	57	149.45
15	57	149.04
16	67	144.93
17	67	155.17
18	67	163.86
19	67	188.89
20	77	211.98
21	77	231.47
22	77	243.59
23	77	257.44

PXL 12/13/89
 11 - 2/2/89

*** 2 1/2 ***

Failure Surface Specified By 21 Coordinate Points

Point No.	X-Coord	Y-Surf (ft)
1	17	127.83
2	21	112.89
3	21	110.95
4	27	118.10
5	207	117.67
6	316	111.53
7	317	110.50
8	379	113.70
9	405	126.75
10	405	139.87
11	47	144.72
12	57	154.39
13	57	163.10
14	57	167.76
15	57	164.77
16	67	178.38
17	67	169.55
18	67	192.81
19	67	206.57
20	77	203.79
21	77	214.05

15	585.01	171.74
16	610.15	188.12
17	639.73	193.13
18	661.71	213.55
19	681.07	233.55
20	714.05	234.60
21	740.17	249.35
22	740.27	249.60

PK - 12/2/89
WYL 2/2/90

*** 4.546 ***

Failure Surface Specified By 23 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	170.00	118.64
2	199.52	113.26
3	228.58	120.68
4	254.32	136.10
5	284.12	139.56
6	313.89	135.92
7	337.22	117.05
8	367.03	113.69
9	395.33	123.65
10	425.27	125.48
11	451.54	110.98
12	481.38	114.05
13	511.38	113.91
14	533.78	133.87
15	562.20	143.47
16	588.22	158.41
17	618.03	161.75
18	641.12	180.90
19	664.57	199.62
20	694.56	198.98
21	721.48	212.24
22	733.81	239.58
23	738.03	249.14

*** 4.570 ***

Failure Surface Specified By 20 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	177.22	123.23
2	204.91	111.69
3	234.87	110.08
4	262.50	121.76

PYL 12/13/77

26 670.00 142.00 680.00 150.00 6

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	100.0	98.0	36.0	.00	.0	1
2	144.0	100.0	.0	40.0	.00	.0	1
3	122.5	100.5	245.0	37.0	.00	.0	1
4	124.5	101.0	895.0	37.0	.00	.0	1
5	124.5	101.0	885.0	37.0	.00	.0	1
6	137.0	100.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	0.00	75.00
2	115.00	75.00
3	180.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Irregular Surfaces, Has Been Specified.

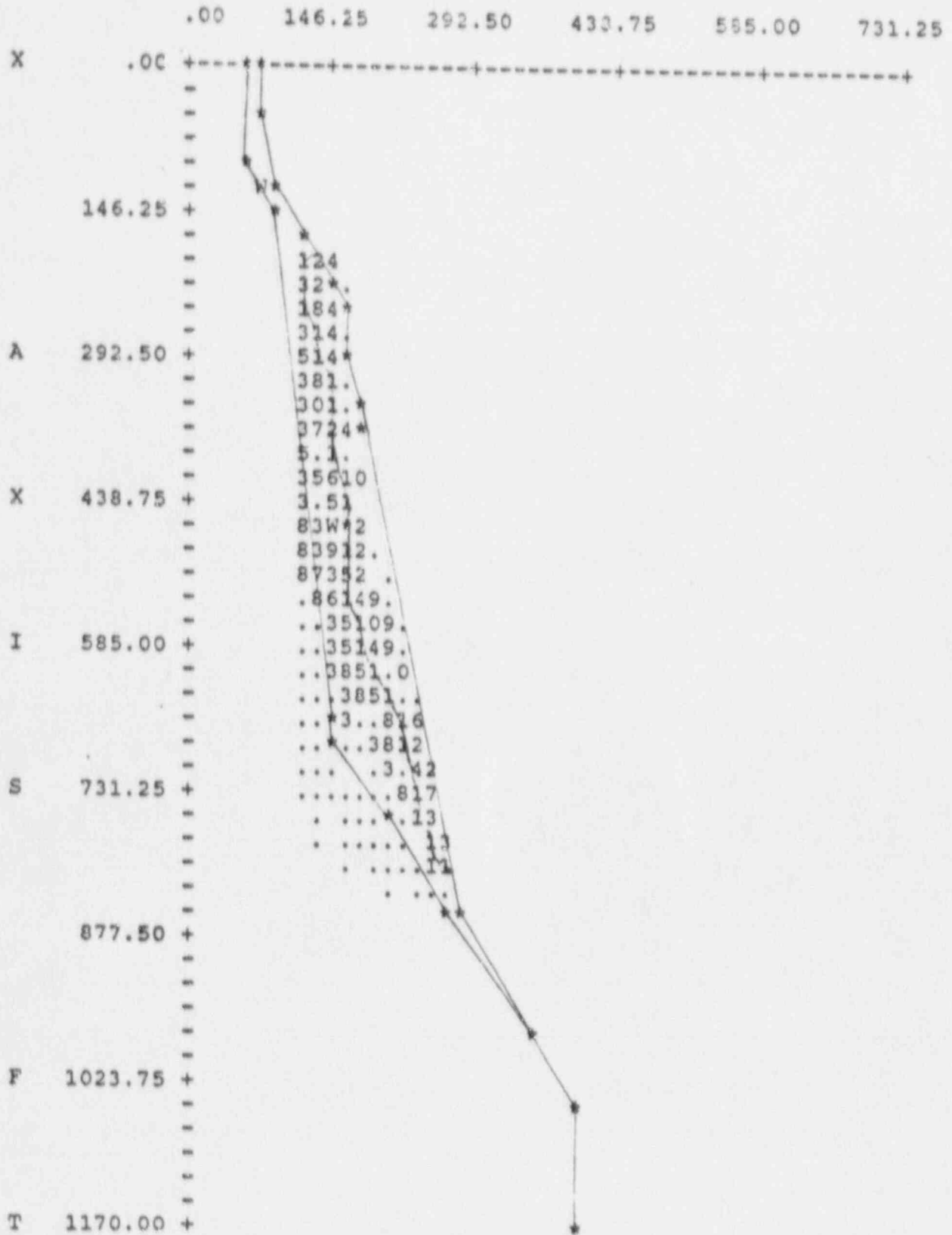
100 Trial Surfaces Have Been Generated.

10 Surfaces Initiate From Each Of 10 Points Equally Spaced Along The Ground Surface Between X = 170.00 ft. and X = 235.00 ft.

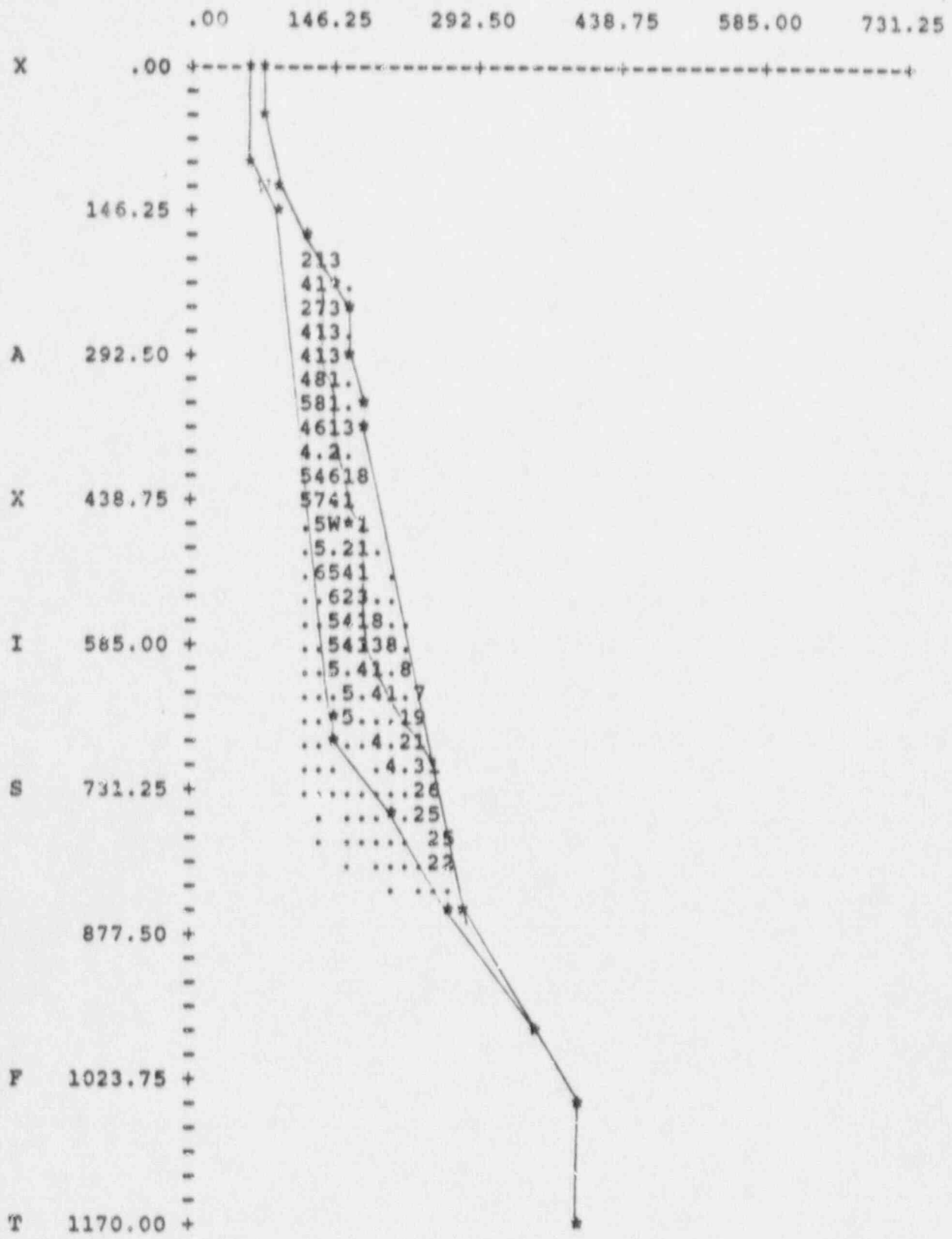
Each Surface Terminates Between X = 550.00 ft.

1

Y A X I S F T



PYL 17/13/39
 W 27.1/33



1

 ***** - Illegal Command *****

26 670.00 142.00 680.00 150.00

PTL 12/13/29

6

W/L 2/2/20

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .170 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Irregular Surfaces, Has Been Specified.

PYL 12/13/59
 211 - 2/2/59

2	206.87	127.15
3	227.73	128.64
4	264.73	117.87
5	293.09	110.42
6	323.99	110.06
7	352.71	116.40
8	383.30	115.79
9	412.70	120.70
10	437.35	138.09
11	464.33	150.11
12	494.61	153.78
13	523.60	161.52
14	552.71	165.63
15	577.81	180.25
16	600.70	182.82
17	624.21	199.53
18	640.39	228.95
19	640.71	229.21

*** 2.000 ***

Failure Surface Specified By 18 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	170.00	118.64
2	199.71	122.80
3	220.73	124.20
4	257.23	112.33
5	286.32	118.84
6	313.41	132.14
7	340.79	133.26
8	372.72	140.43
9	399.06	152.54
10	420.75	170.56
11	440.78	179.19
12	463.78	179.71
13	510.75	175.28
14	540.25	171.87
15	560.26	189.85
16	594.26	198.91
17	616.73	219.56
18	617.78	224.62

*** 2.178 ***

1

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
-----------	-------------	-------------

PTL 12/13/87
11/2/87

Failure Surface Specified By 24 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	177.22	123.23
2	204.36	110.44
3	233.07	119.12
4	259.86	132.62
5	280.48	137.43
6	310.04	142.52
7	348.99	144.20
8	377.88	152.30
9	405.66	163.63
10	435.49	166.81
11	465.09	164.44
12	495.33	166.36
13	525.22	163.72
14	554.13	171.76
15	582.00	180.56
16	611.73	188.12
17	639.40	199.95
18	677.77	212.23
19	691.43	223.84
20	727.73	227.73
21	751.28	228.07
22	779.52	242.42
23	807.56	255.42
24	811.04	264.14

*** 2.228 ***

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	206.11	141.62
2	236.05	143.48
3	265.78	139.46
4	295.61	142.68
5	320.07	152.46
6	353.04	153.83
7	387.71	149.41
8	417.07	158.90
9	442.09	164.24
10	467.72	177.05
11	491.47	180.92
12	522.31	177.82
13	557.04	182.51
14	587.79	184.29
15	617.25	190.90
16	645.13	206.74

PYL 12/13/89
71/2 2/5/90

17	667.39	223.68
18	609.78	233.37
19	711.1	243.49

*** 2.231 ***

Failure Surface Specified By 21 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	184.44	127.83
2	210.46	112.89
3	240.40	110.95
4	260.53	118.10
5	299.53	117.67
6	328.90	111.53
7	350.78	110.50
8	380.71	113.70
9	415.72	126.75
10	440.70	139.87
11	470.70	144.72
12	500.70	154.39
13	520.71	163.10
14	530.75	167.76
15	530.70	164.77
16	610.73	178.38
17	640.78	189.55
18	670.70	192.61
19	699.76	206.57
20	710.77	233.78
21	710.76	244.05

*** 2.206 ***

1 Failure Surface Specified By 23 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	170.00	118.64
2	190.00	110.22
3	220.00	115.63
4	250.04	124.25
5	280.04	124.38
6	310.52	118.84
7	340.09	117.44
8	375.07	111.38
9	405.53	115.91
10	435.75	121.23
11	460.01	125.07

PYL 12/13/59

11/2 210 30

12	493.67	133.23
13	577.4	141.76
14	551.3	149.45
15	501.3	149.04
16	611.15	144.93
17	639.25	155.17
18	668.06	163.86
19	684.60	188.89
20	703.75	211.98
21	726.75	231.47
22	754.00	243.59
23	778.71	257.44

*** 2.207 ***

Failure Surface Specified By 22 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	191.67	132.42
2	216.70	115.44
3	246.4	113.52
4	275.07	120.25
5	304.08	114.34
6	334.00	121.55
7	363.74	124.72
8	389.31	140.40
9	418.10	148.49
10	447.72	139.84
11	477.5	133.08
12	505.8	137.82
13	535.4	139.37
14	557.8	160.56
15	582.1	171.74
16	610.5	188.12
17	639.3	193.13
18	661.1	213.55
19	684.7	233.55
20	714.5	234.60
21	740.7	249.35
22	740.7	249.60

*** 2.422 ***

1

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	177.02	123.23

PC 12/13/89
V = 2.153

100 Trial Surfaces Have Been Generated.

10 Surfaces Initiate From Each Of 10 Points Equally Spaced
Along The Ground Surface Between X = 170.00 ft.
and X = 235.00 ft.

Each Surface Terminates Between X = 550.00 ft.
and X = 850.00 ft.

Unless Further Limitations Were Imposed, The Minimum Elevation
At Which A Surface Extends Is Y = 110.00 ft.

30.00 ft. Line Segments Define Each Trial Failure Surface.

1

Following Are Displayed The Ten Most Critical Of The Trial
Failure Surfaces Examined. They Are Ordered - Most Critical
First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	198.89	137.02
2	228.83	138.88
3	258.56	134.87
4	288.39	138.09
5	316.75	147.86
6	346.72	149.24
7	376.39	144.81
8	404.85	154.31
9	434.37	159.64
10	461.50	172.45
11	491.25	176.32
12	521.09	173.23
13	550.72	177.91
14	580.67	179.69
15	600.03	186.31
16	635.41	202.14
17	660.17	219.08
18	688.56	228.77
19	700.42	243.31

*** 2.153 ***

PYL 12/13/89
 11 - 2 - 25

1	184.44	127.83
2	212.77	112.15
3	240.79	115.26
4	266.78	128.52
5	293.78	135.37
6	325.50	140.75
7	351.79	143.30
8	380.21	140.00
9	411.75	141.42
10	444.72	148.26
11	474.77	146.89
12	501.81	146.56
13	522.74	146.64
14	540.79	155.77
15	557.73	159.42
16	610.74	177.23
17	640.79	199.41
18	660.73	226.23
19	680.76	232.61

*** 2.376 ***

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	184.44	127.83
2	212.77	121.51
3	240.79	113.17
4	270.79	113.66
5	300.78	113.14
6	322.77	118.11
7	350.79	136.33
8	380.74	150.31
9	411.75	159.64
10	440.72	164.11
11	470.77	165.72
12	500.81	162.56
13	520.74	169.70
14	540.79	172.75
15	560.73	180.28
16	610.74	193.95
17	640.79	205.33
18	660.73	220.91
19	680.76	237.73

*** 2.504 ***

Y A X I S Z T

** PCSTABL5 **

PYL 12/13/79
W/L - 2/7/80

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STSRAN2.DAT
Output Filename: STSRAN2.OUT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 SHORT TERM SEIMIC-RANDOM
-2-

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6

** PCSTABL5 **

PYL 12/13/79
W L 2.5000

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:LTNSRAN2.DAT
Output Filename: LTNSRAN2.OUT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 LONG TERM NON-SEIMIC-RAN
DOM 2

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6

PYL 12/12/89
NYL 2/15/90

5	289.27	135.31
6	319.16	132.82
7	343.68	115.53
8	373.54	112.62
9	403.52	113.76
10	425.48	134.20
11	455.06	139.19
12	483.40	149.03
13	510.33	162.26
14	530.87	184.12
15	557.45	198.04
16	587.45	197.71
17	617.44	197.14
18	646.83	203.16
19	663.38	228.18
20	666.15	234.48

*** 4.662 ***

Failure Surface Specified By 18 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	170.00	118.64
2	199.71	122.80
3	229.68	124.20
4	257.23	112.33
5	286.52	118.84
6	313.41	132.14
7	343.39	133.26
8	372.52	140.43
9	399.96	152.54
10	423.95	170.56
11	452.68	179.19
12	482.68	179.71
13	512.35	175.28
14	542.15	171.87
15	566.16	189.85
16	594.76	198.91
17	616.53	219.56
18	617.78	224.62

*** 4.678 ***

and X = 850.00 ft.

PYL 12/13/87

WYL 2/4/90

Unless Further Limitations Were Imposed, The Minimum Elevation At Which A Surface Extends Is Y = 110.00 ft.

30.00 ft. Line Segments Define Each Trial Failure Surface.

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 24 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	177.23	123.23
2	201.24	110.44
3	230.27	119.12
4	255.25	132.62
5	280.43	137.43
6	310.04	142.52
7	335.23	144.20
8	377.23	152.30
9	405.65	163.63
10	425.20	166.81
11	461.20	164.44
12	495.23	166.36
13	525.23	163.72
14	555.23	171.70
15	585.23	180.56
16	615.23	188.12
17	645.23	199.95
18	675.23	212.23
19	705.23	223.84
20	735.23	227.73
21	765.23	228.07
22	795.23	242.42
23	825.23	255.42
24	850.00	264.14

*** 4.000 ***

Failure Surface Specified By 23 Coordinate Points

Point	X-Surf	Y-Surf
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20	712.57	233.78
21	713.06	244.05

PXL 12/12/27
WYL 2/5/72

*** 4.316 ***

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	184.44	127.83
2	210.02	112.15
3	239.86	115.26
4	266.78	128.52
5	295.98	135.37
6	325.50	140.75
7	355.39	143.30
8	385.21	140.00
9	415.17	141.42
10	444.38	148.26
11	474.35	146.89
12	504.35	146.56
13	534.35	146.64
14	562.92	155.77
15	592.70	159.42
16	616.84	177.23
17	637.04	199.41
18	650.47	226.23
19	656.98	232.61

*** 4.532 ***

1

Failure Surface Specified By 22 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	191.67	132.42
2	216.40	115.44
3	246.34	113.52
4	275.57	120.25
5	304.98	114.34
6	334.10	121.55
7	363.94	124.72
8	389.51	140.40
9	418.40	148.49
10	447.12	139.84
11	476.35	133.08
12	505.98	137.82
13	535.94	139.37
14	557.18	160.56

PYL 12/13/89

*** 4.000 ***

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	191.44	127.83
2	211.22	112.15
3	211.22	115.26
4	211.22	128.52
5	211.22	135.37
6	323.50	140.75
7	375.37	143.30
8	385.21	140.00
9	425.17	141.42
10	425.17	148.26
11	425.17	146.89
12	521.35	146.56
13	521.35	146.64
14	521.35	145.77
15	521.35	149.42
16	611.22	147.23
17	611.22	149.41
18	611.22	226.23
19	611.22	222.61

*** 4.000 ***

Failure Surface Specified By 20 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	177.22	123.23
2	201.01	111.69
3	201.07	110.08
4	263.50	121.76
5	211.27	135.31
6	311.15	132.82
7	311.13	135.53
8	311.14	112.62
9	411.22	113.76
10	411.18	124.20
11	411.16	139.19
12	411.40	149.03
13	511.13	142.26
14	511.17	124.12
15	511.15	138.04
16	511.15	137.71
17	611.14	137.14

PSL 1/2/77
WYL 2.5/70

No.	(ft)	(ft)
1	198.89	137.02
2	228.83	138.88
3	258.56	134.87
4	288.39	138.09
5	316.75	147.86
6	346.72	149.24
7	376.39	144.81
8	404.85	154.31
9	434.37	159.64
10	461.50	172.45
11	491.25	176.32
12	521.09	173.23
13	550.72	177.91
14	580.67	179.69
15	609.93	186.31
16	635.41	202.14
17	660.17	219.08
18	688.56	228.77
19	709.42	243.31

*** 4.105 ***

1
Failure Surface Specified By 23 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	170.00	118.64
2	198.80	110.22
3	228.30	115.63
4	257.04	124.25
5	287.04	124.38
6	316.52	118.84
7	346.49	117.44
8	375.87	111.38
9	405.53	115.91
10	435.05	121.23
11	464.81	125.07
12	493.67	133.23
13	522.44	141.76
14	551.43	149.45
15	581.43	149.04
16	611.15	144.93
17	639.35	155.17
18	668.06	163.86
19	684.60	188.89
20	703.75	211.98
21	726.55	231.47
22	754.00	243.59
23	778.71	257.44

*** 4.246 ***

10	417.13	139.84
11	417.13	133.08
12	517.13	127.82
13	517.13	139.37
14	517.13	160.56
15	517.13	171.74
16	617.13	188.12
17	617.13	193.13
18	617.13	213.55
19	617.13	223.55
20	717.13	234.60
21	717.13	249.35
22	717.13	249.60

PYL 12/13/87
WVL 2/15/88

*** 4.073 ***

Failure Surface Specified By 17 Coordinate Points

Point No.	X-Coord	Y-Surf (ft)
1	117.13	118.64
2	117.13	110.15
3	217.13	113.12
4	217.13	112.45
5	217.13	125.75
6	317.13	132.05
7	317.13	126.67
8	317.13	126.35
9	417.13	128.47
10	417.13	115.81
11	417.13	120.63
12	417.13	126.35
13	517.13	113.96
14	517.13	117.48
15	517.13	116.76
16	517.13	212.46
17	517.13	214.29

*** 4.073 ***

1

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Coord	Y-Surf (ft)
1	117.13	127.83
2	217.13	121.51
3	217.13	113.17
4	217.13	113.66
5	317.13	113.14

1 26 670.00 142.00 680.00 150.00

PYL 12/2/89
Y/L - 2:00' 30"
6

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

1 A Critical Failure Surface Searching Method, Using A Random Technique For Generating Irregular Surfaces, Has Been Specified.

100 Trial Surfaces Have Been Generated.

10 Surfaces Initiate From Each Of 10 Points Equally Spaced Along The Ground Surface Between X = 170.00 ft. and X = 235.00 ft.

Each Surface Terminates Between X = 550.00 ft.

*** 2.322 ***

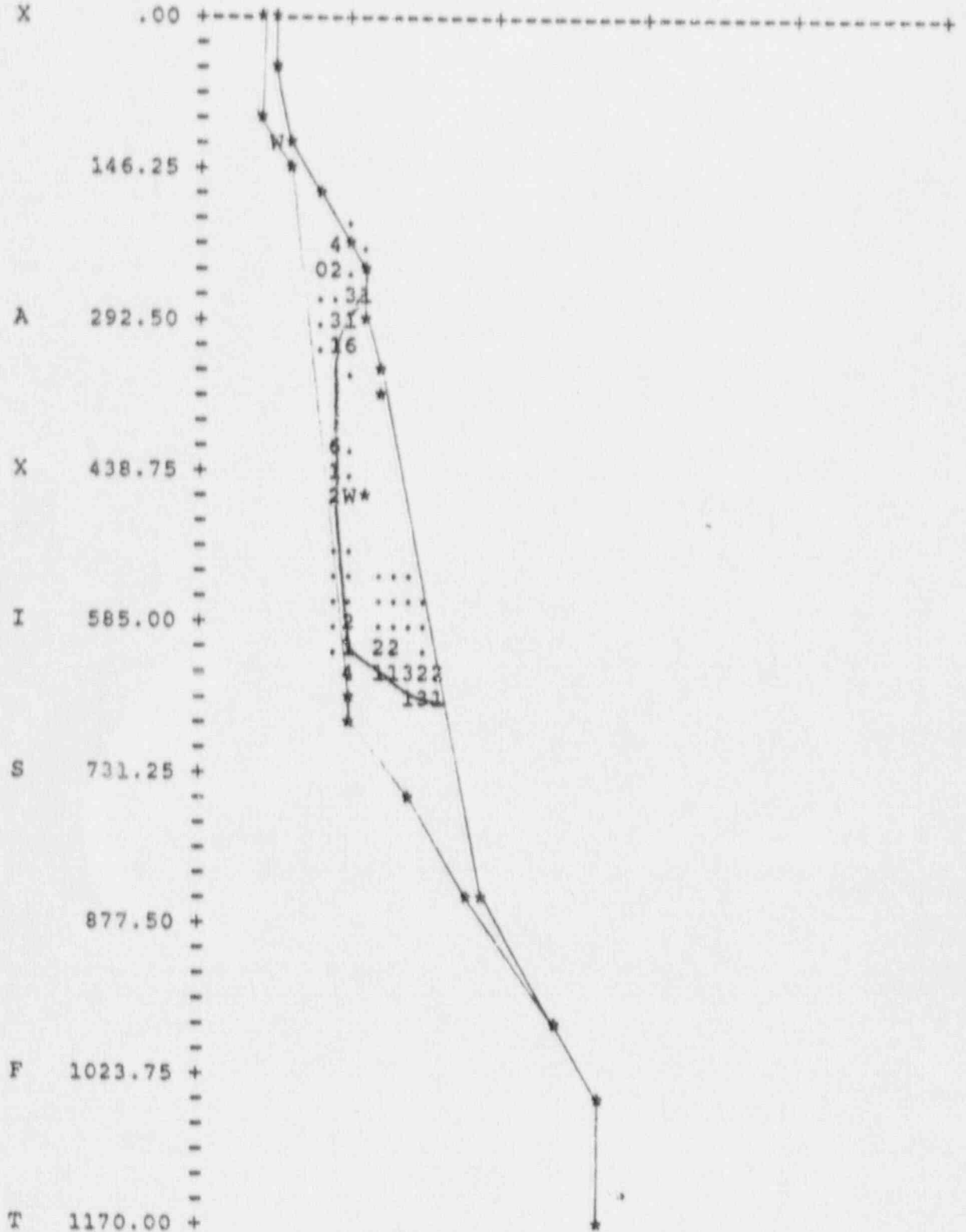
PYL 12/12/59
W - 2/8/50

LTSALL2

1

Y A X I S F T

.00 146.25 292.50 438.75 585.00 731.25



1

26 670.00 142.00 680.00 150.00

PYL 12/13/89
W¹ - 2/10/90
6

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	250.0	21.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	750.0	27.0	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .230 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Irregular Surfaces, Has Been Specified.

PYL 12/12/89
W/VL 2/2/90

1

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	276.54	160.00
2	281.33	157.06
3	294.07	150.00
4	307.61	142.50
5	447.19	130.10
6	589.67	140.73
7	594.81	150.00
8	609.36	176.24
9	613.02	182.85
10	621.58	196.82
11	634.26	224.01
12	634.47	224.44
13	637.32	228.60

*** 2.287 ***

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	218.47	149.48
2	223.69	146.28
3	224.78	145.68
4	251.01	131.13
5	461.82	132.66
6	603.91	141.11
7	608.84	150.00
8	623.38	176.24
9	628.02	184.60
10	636.44	198.34
11	649.12	225.53
12	650.08	227.60
13	652.96	231.79

*** 2.303 ***

1

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	253.21	160.00

PYL 12/13/89
WV- 2/10/90

*** 1.990 ***

Failure Surface Specified By 21 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	184.44	127.83
2	210.46	112.89
3	240.40	110.95
4	269.53	118.10
5	299.53	117.67
6	328.90	111.53
7	358.88	110.50
8	388.71	113.70
9	415.72	126.75
10	442.70	139.87
11	472.30	144.72
12	500.70	154.39
13	529.41	163.10
14	559.05	167.76
15	588.90	164.77
16	615.63	178.38
17	643.48	189.55
18	673.30	192.81
19	699.96	206.57
20	712.57	233.78
21	713.06	244.05

*** 2.033 ***

1

Failure Surface Specified By 24 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	177.22	123.23
2	204.36	110.44
3	233.07	119.12
4	259.86	132.62
5	289.48	137.43
6	319.04	142.52
7	348.99	144.20
8	377.88	152.30
9	405.66	163.63
10	435.49	166.81
11	465.39	164.44
12	495.33	166.36
13	525.22	163.72
14	554.13	171.70
15	582.80	180.56

PYL 12/12/89
N^o - 2 1/2

Point No.	X-Surf (ft)	Y-Surf (ft)
1	207.53	142.52
2	212.33	139.58
3	238.32	125.18
4	457.42	130.79
5	586.50	143.81
6	589.94	150.00
7	604.48	176.24
8	607.81	182.24
9	616.42	196.29
10	629.03	223.35
11	631.88	227.49

*** 2.237 ***

1

Failure Surface Specified By 13 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	254.91	160.00
2	261.17	156.16
3	272.29	150.00
4	292.62	138.73
5	430.56	129.25
6	611.86	149.13
7	612.34	150.00
8	626.88	176.24
9	631.76	185.04
10	640.15	198.73
11	657.82	225.91
12	653.98	228.39
13	656.86	232.59

*** 2.247 ***

Failure Surface Specified By 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	215.31	147.47
2	220.41	144.35
3	231.19	138.37
4	465.10	131.86
5	618.19	142.97
6	622.09	150.00
7	636.64	176.24

PYL 12/13/59
 WYL 2/5/60

5	295.98	135.37
6	325.50	140.75
7	355.39	143.30
8	325.21	140.00
9	415.17	141.42
10	444.38	148.26
11	474.35	146.89
12	504.35	146.56
13	534.35	146.64
14	562.92	155.77
15	592.70	159.42
16	616.84	177.23
17	637.04	199.41
18	650.47	226.23
19	656.98	232.61

*** 2.162 ***

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	177.22	123.23
2	206.97	127.15
3	236.93	128.64
4	264.93	117.87
5	293.99	110.42
6	323.99	110.06
7	353.31	116.40
8	383.30	115.79
9	412.90	120.70
10	437.33	138.09
11	464.83	150.11
12	494.61	153.78
13	523.60	161.52
14	553.31	165.63
15	579.51	180.25
16	609.40	182.82
17	634.31	199.53
18	640.19	228.95
19	640.31	229.21

*** 2.172 ***

1 Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	198.89	137.02

1 26 670.00 142.00 680.00 150.00

PYL 12/12/89

6

V/L 2 1 1

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	250.0	21.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	750.0	27.0	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .230 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 psf

1 A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

PXL 12/13/89
W/L 2/2/90

Failure Surface Specified By 19 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	184.44	127.83
2	213.77	121.51
3	242.59	113.17
4	272.59	113.66
5	302.58	113.14
6	332.17	118.11
7	356.00	136.33
8	382.54	150.31
9	411.05	159.64
10	440.72	164.11
11	470.67	165.72
12	500.51	162.56
13	529.64	169.70
14	559.49	172.75
15	588.53	180.28
16	615.24	193.95
17	642.99	205.33
18	668.63	220.91
19	682.06	237.73

*** 2.215 ***

Failure Surface Specified By 20 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	220.56	150.81
2	248.39	139.61
3	278.16	135.91
4	306.08	124.94
5	335.18	117.65
6	362.70	129.58
7	392.16	135.26
8	419.36	147.92
9	448.92	153.03
10	478.87	154.69
11	508.15	148.12
12	536.29	137.74
13	564.67	147.46
14	593.58	155.48
15	621.55	166.34
16	646.79	182.55
17	673.00	197.15
18	696.68	215.57
19	721.88	231.85
20	725.94	246.68

PYL 12/2/89

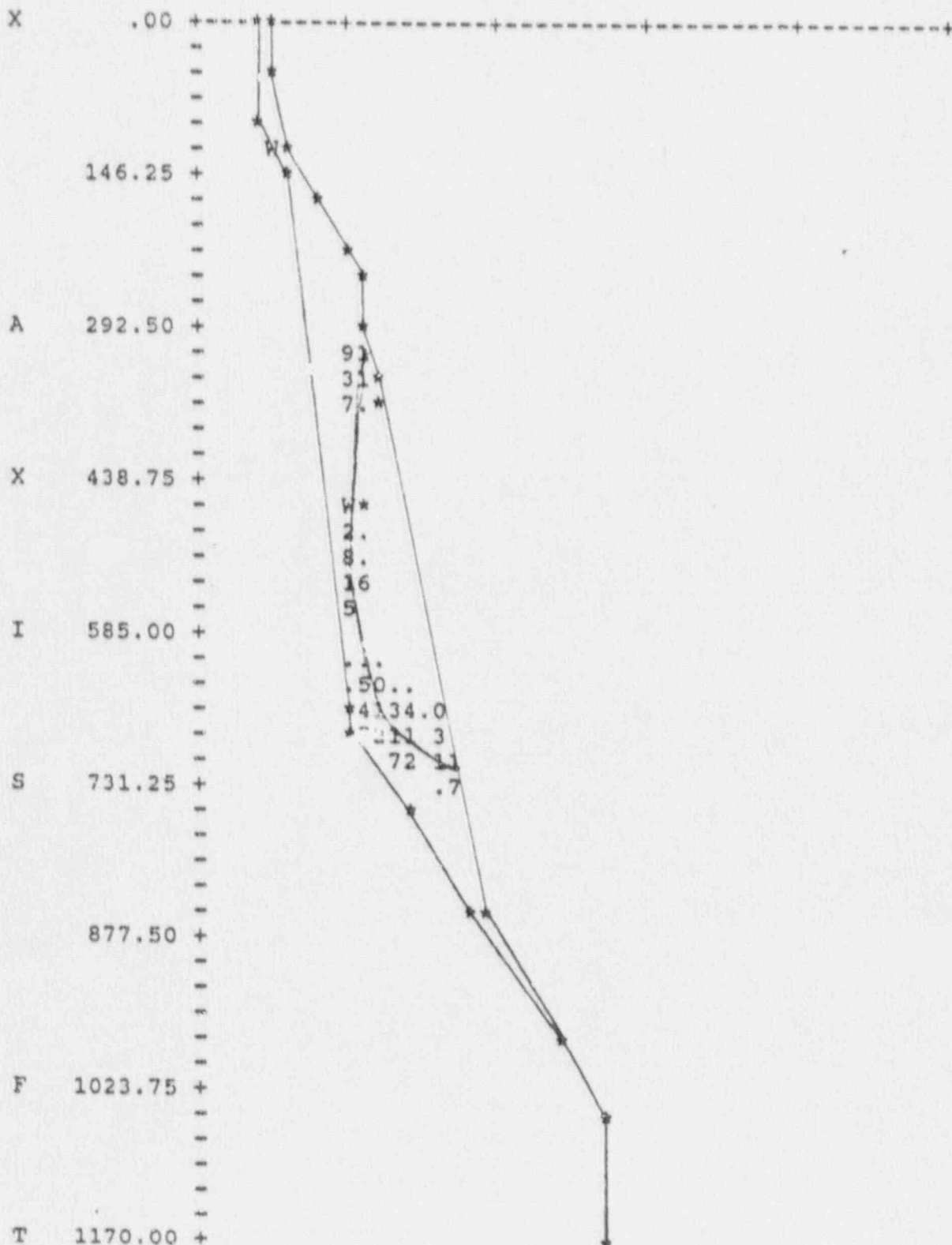
W - 2:00 30

*** 2.291 ***

LTSALLI

Y A X I S F T

.00 146.25 292.50 438.75 585.00 731.25





MORRISON-KNUDSEN ENGINEERS, INC.
A MORRISON KNUDSEN COMPANY

Project UNTRA - LOW
Feature Embankment Design
Item Slope Stability Analysis Rev D

Contract No. 3885-48
Designed PTL
Checked K. K. K.

Sheet B-1
File No. _____
Date 7/17/90
Date 8/3/90

Rev. D

APPENDIX B

'STABLES' OUTPUT

SHALLOW FAILURE SURFACE



PXL 12/7/27
W = 2/2/20

8	706.19	205.50
9	718.87	232.69
10	723.41	242.42
11	726.40	246.77

*** 2.285 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	327.79	165.96
2	338.20	159.59
3	352.06	151.90
4	513.45	150.24
5	646.00	163.40
6	659.81	188.31
7	667.94	201.58
8	680.62	228.77
9	683.19	234.29
10	686.12	238.56

*** 2.289 ***

1

Failure Surface Specified By 11 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	306.50	162.09
2	312.45	158.44
3	324.16	151.95
4	521.23	50.38
5	661.73	161.88
6	676.27	188.12
7	677.52	190.38
8	685.49	203.38
9	698.17	230.57
10	701.64	238.02
11	704.60	242.33

*** 2.290 ***

Failure Surface Specified By 10 Coordinate Points

Point	X-Surf	Y-Surf
-------	--------	--------

Rev. Δ Slope stability 1.1
 P.L. 7/12/90

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.9	130.5	350.0	17.5	.00	.0	1 ✓
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

check: K. Lee 8/3/90

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

2 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of Sliding Block is 20.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	860.00	270.00	900.00	295.00	2.00
2	930.00	315.00	950.00	327.00	1.00

PYL 12/12/89
W' - 2.2/00

1	330.28	166.41
2	341.21	159.72
3	351.31	154.12
4	477.29	150.65
5	675.56	169.31
6	687.91	191.59
7	695.78	204.44
8	708.46	231.62
9	712.46	240.21
10	715.44	244.51

*** 2.251 ***

1

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	319.97	164.54
2	328.74	159.17
3	339.32	153.30
4	546.06	150.78
5	656.02	168.69
6	667.38	189.19
7	675.44	202.35
8	688.12	229.54
9	691.08	235.89
10	694.02	240.17

*** 2.258 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	325.29	165.51
2	335.17	159.45
3	349.23	151.66
4	540.70	151.47
5	648.22	166.80
6	660.17	188.35
7	668.30	201.62
8	680.98	228.80
9	683.57	234.37
10	686.50	238.63

*** 2.272 ***

3	930.07	314.88
4	931.80	317.24

*** 2.699 ***

Rev Δ

ULTRA-LOW SLIP-5
Slope Stability
PCL 7/17/90

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	857.08	274.20
2	861.37	271.05
3	946.07	324.22
4	947.64	326.36

*** 2.699 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	865.17	278.86
2	869.58	275.63
3	931.73	315.86
4	933.43	318.18

*** 2.715 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	862.69	277.43
2	867.43	273.95
3	946.44	325.02
4	947.26	326.14

*** 2.716 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	862.81	277.50
2	867.19	274.28

PYL 12/12/89
W' = 2/5 22
6

26 670.00 142.00 680.00 150.00 6

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	250.0	21.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	750.0	27.0	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

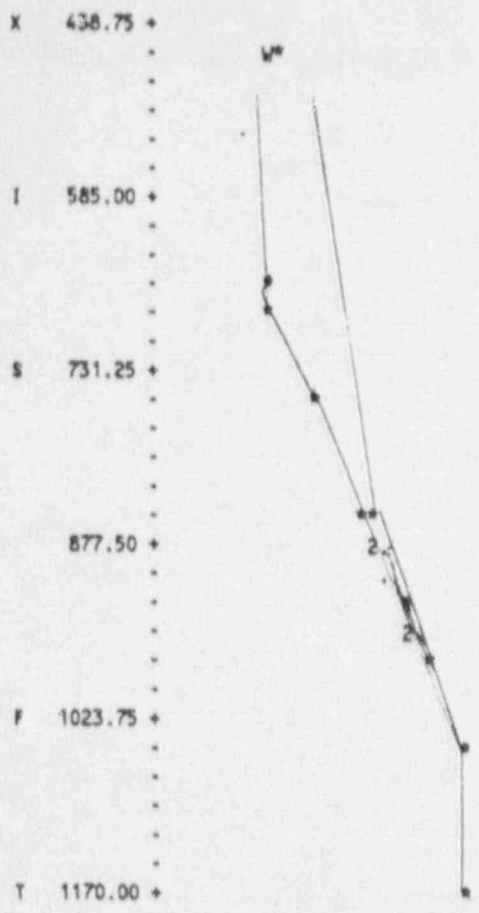
A Horizontal Earthquake Loading Coefficient Of .230 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

ULTRA-LOW SB 13-7
 Rev Δ Slope Stability
 PXL 7/17/90



1

 ***** - Illegal Command *****

- Legal Commands -
- PROFIL
 - LOADS
 - TIES
 - WATER
 - SURFAC
 - EXECUT
 - EQUAKE
 - SOIL
 - RANDOM
 - CIRCLE
 - CIRCL2
 - BLOCK
 - BLOCK2
 - LIMITS
 - ANISO
 - SURBIS
 - SPENCR

LM78A-LLW Sh B-9
 Rev A Slp Stability
 PTL 7/17/90

Checked: K. Lee 8/3/90

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	350.0	17.5	.00	.0	1 ✓
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	342.00
7	1055.00	190.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .170 Has Been Assigned ✓

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 pcf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

2 Boxes Specified For Generation Of Central Block Base

8	815.56	248.03
9	822.26	262.39
10	825.40	266.96

PYL 12/12/19
 W - 2'5"2)

*** 1.627 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	387.31	177.61
2	390.81	175.21
3	394.45	173.51
4	403.91	167.72
5	495.47	170.19
6	708.16	198.17
7	822.82	246.84
8	827.95	255.21
9	832.24	264.41
10	835.39	269.00

*** 1.639 ***

1
 Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	385.96	177.34
2	389.46	174.93
3	392.86	173.35
4	403.51	166.82
5	506.25	171.52
6	703.51	197.69
7	799.18	230.36
8	806.94	243.03
9	815.32	260.99
10	818.45	265.55

*** 1.639 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
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UMTRA-LCW Sh B-11
Rev A Slope Stability
PYL 7/12/90

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	857.08	274.20
2	861.37	271.05
3	946.07	324.22
4	947.64	326.36

*** 2.044 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	869.53	281.37
2	874.07	278.04
3	930.07	314.88
4	931.80	317.24

*** 2.045 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	865.17	278.86
2	869.58	275.63
3	931.73	315.86
4	933.43	318.18

*** 2.057 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	862.69	277.43
2	867.43	273.95
3	946.44	325.02
4	947.26	326.14

*** 2.058 ***

PYL 12/2/89
WYL 2/5/90

1	390.59	178.28
2	394.09	175.87
3	398.30	173.90
4	411.72	165.68
5	519.85	173.63
6	731.13	198.78
7	795.47	226.29
8	805.06	241.93
9	813.80	260.69
10	816.93	265.24

*** 1.610 ***

1

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	357.45	171.52
2	359.00	170.45
3	370.51	163.40
4	577.27	178.75
5	730.72	201.09
6	802.21	233.43
7	808.73	244.06
8	816.76	261.28
9	819.89	265.84

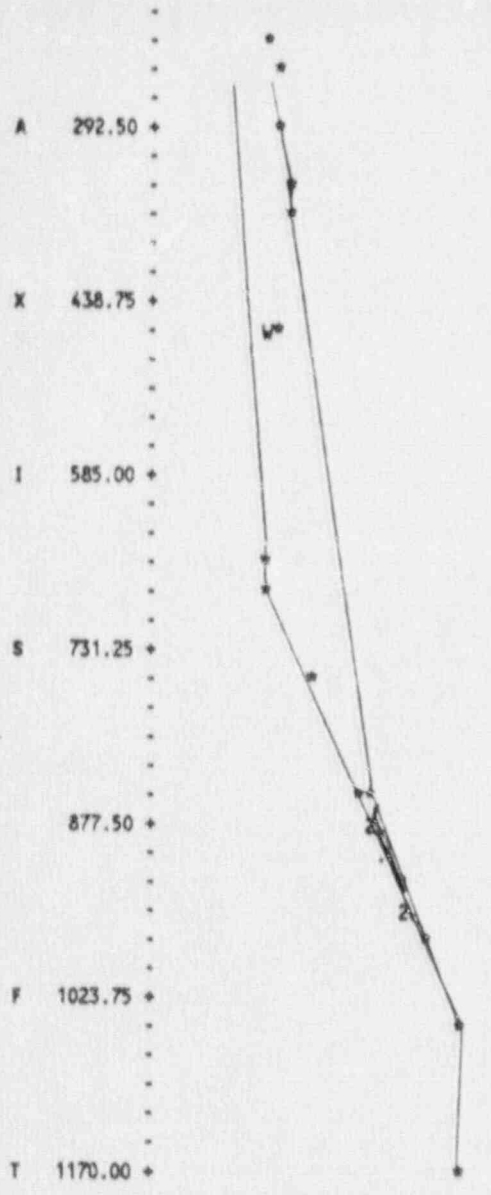
*** 1.612 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	392.47	178.66
2	395.98	176.25
3	400.52	174.13
4	410.55	167.99
5	552.19	177.61
6	717.18	197.34
7	824.57	245.87
8	831.59	257.32
9	835.17	265.00
10	838.33	269.60

*** 1.621 ***

UMTKA-LOW Sh. B-13
 ECI Δ Slope Stability
 PYL 7/17/90



 ***** - Illegal Command *****

- Legal Commands -
- PROFIL
 - LOADS
 - TIEB
 - WATER
 - SURFAC
 - EXECIT
 - EQUAKE
 - SOIL
 - RANDOM
 - CIRCLE
 - CIRCL2
 - BLOCK
 - BLOCK2
 - LIMITS
 - ANISO
 - SURBIS
 - SPENCR

25	177.00	113.00	670.00	142.00
26	670.00	142.00	680.00	150.00

PTL 12/12/89
WYL 2/5/90
6
6

ISOTROPIC SOIL PARAMETERS

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	250.0	21.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	750.0	27.0	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .230 Has Been Assigned

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been

UMTRA-LOW B-15

REV Δ Slope Stability

PYL 2/17/90

Checked K. Goe 8/3/90

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (psf)	Friction Angle (deg)	Pore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	98.0	36.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	245.0	37.0	.00	.0	1 ✓
4	124.5	131.0	885.0	37.0	.00	.0	1
5	124.5	131.0	885.0	37.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

2 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 20.0

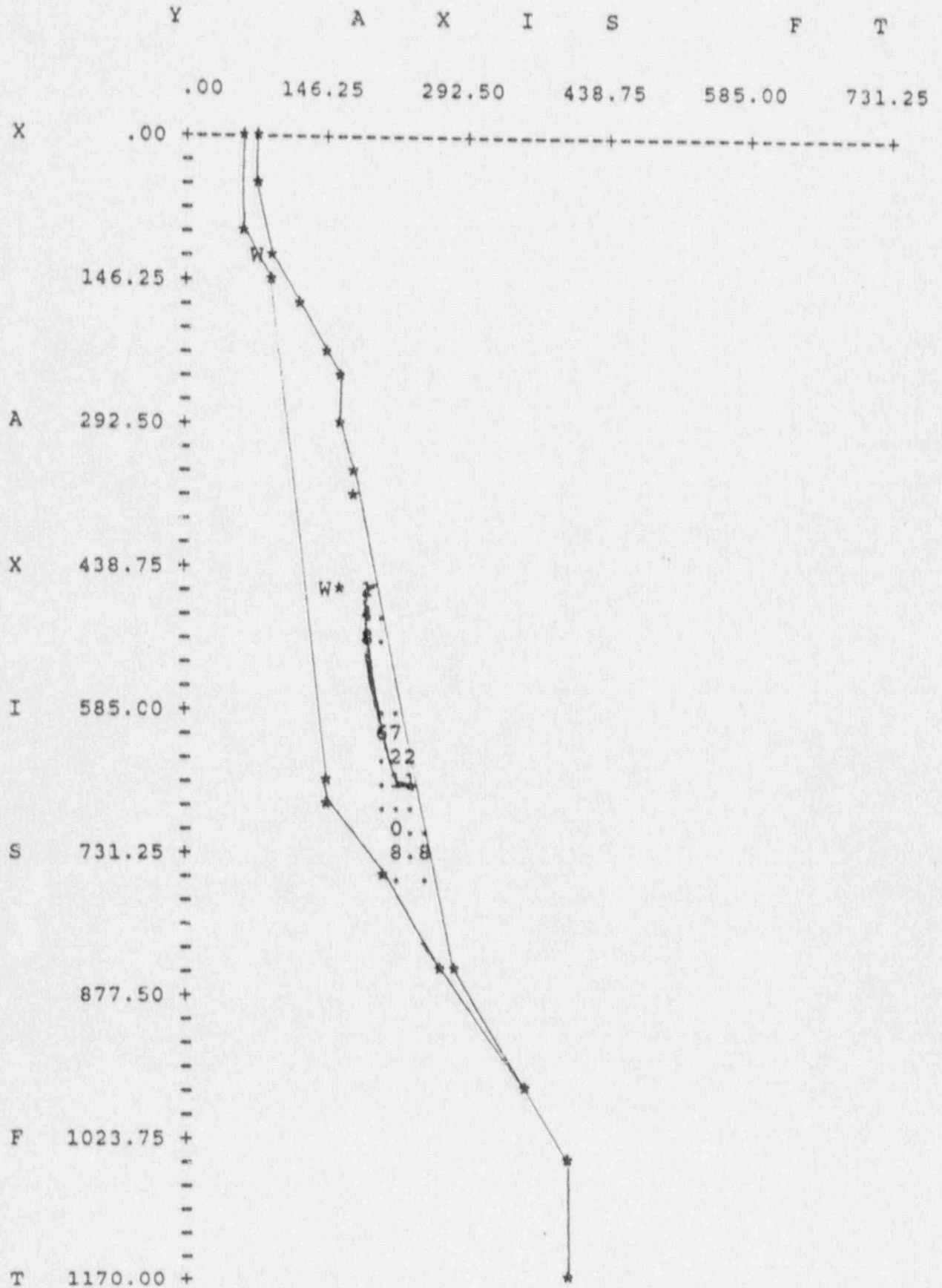
Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	860.00	270.00	900.00	295.00	2.00
2	930.00	315.00	950.00	327.00	1.00

5 724.70 242.68
 6 727.70 247.04

PYL 12/12/89
 WYL 2/5/90

*** 2.036 ***

LTSSA.



UMTRA-LOW B77
RCV Δ Slope Stability
PYL = 1/12/90

3	946.07	324.22
4	946.93	325.95

*** 2.851 ***

1
Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	868.54	280.80
2	874.07	278.04
3	930.07	314.88
4	931.03	316.79

*** 2.872 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	861.65	276.83
2	867.43	273.95
3	946.44	325.02
4	946.89	325.93

*** 2.875 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	855.15	273.09
2	860.32	270.51
3	948.20	325.53
4	949.00	327.14

*** 2.876 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	864.21	278.31
2	869.58	275.63

PYL 12/12/89
WV - 2/5/90

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	457.24	191.87
2	460.88	189.37
3	463.70	188.05
4	617.92	210.36
5	623.45	222.22
6	626.29	226.35

*** 2.026 ***

1

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	462.25	192.89
2	465.90	190.38
3	469.09	188.89
4	607.29	209.48
5	612.17	219.94
6	615.00	224.05

*** 2.029 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	483.96	197.32
2	487.66	194.78
3	499.85	189.09
4	720.58	221.21
5	731.20	243.99
6	734.21	248.36

*** 2.035 ***

1

Failure Surface Specified By 6 Coordinate Points

UMTRA - LOW B-19
 Rev. Δ Slope Stability
 PYL 7/17/90

3 931.73 315.86
 4 932.67 317.74

*** 2.876 ***

Failure Surface Specified By 4 Coordinate Points

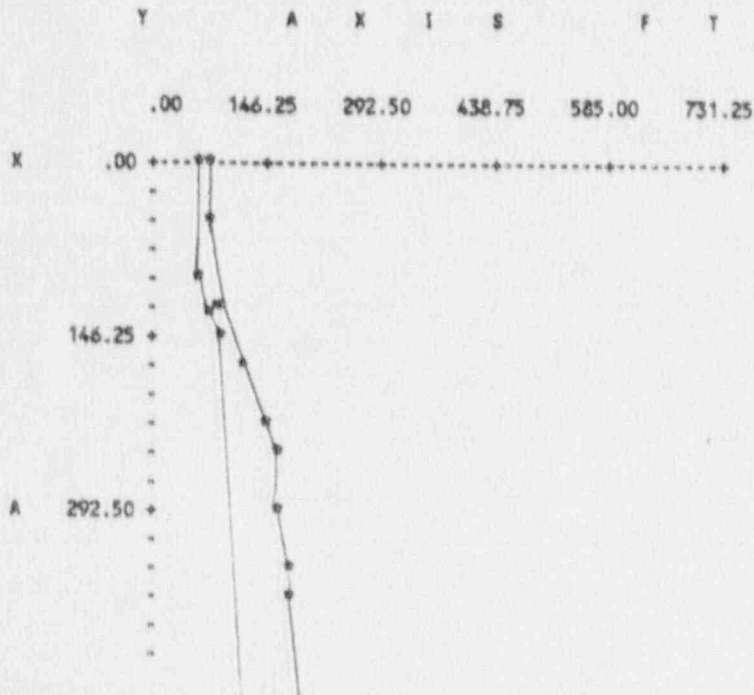
Point No.	X-Surf (ft)	Y-Surf (ft)
1	861.85	276.94
2	867.19	274.28
3	949.13	326.21
4	949.85	327.63

*** 2.879 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	860.96	276.43
2	865.02	273.91
3	942.61	322.11
4	943.56	324.01

*** 2.883 ***



PYL 12/12/87
WYL 2/2/92

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

2 Boxes Specified For Generation Of Central Block Base

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 30.0

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	460.00	186.00	520.00	193.00	5.00
2	590.00	205.00	750.00	220.00	10.00

1

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	457.70	191.97
2	461.35	189.46
3	468.39	186.18
4	650.42	214.81
5	657.04	229.01
6	659.93	233.21

*** 2.009 ***

Failure Surface Specified By 6 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	462.71	192.99
2	466.37	190.47
3	473.78	187.02
4	639.79	213.94
5	645.76	226.73

UNTEA - LOW

RCU Δ Slope Stability

PYL 2/12/90

Checked K. Lee 8/3/90

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Fore Pressure Param.	Pressure Constant (psf)	Piez. Surface No.
1	130.0	135.0	250.0	21.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	750.0	27.0	.00	.0	1 ✓
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient Of .230 Has Been Assigned ✓

A Vertical Earthquake Loading Coefficient Of .000 Has Been Assigned

Cavitation Pressure = .0 psf

A Critical Failure Surface Searching Method, Using A Random Technique For Generating Sliding Block Surfaces, Has Been Specified.

The Active And Passive Portions Of The Sliding Surfaces Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

2 Boxes Specified For Generation Of Central Block Base

** PCSTABLES **

PYL 12/12/89
NYL 2/5/90

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:LTSSA.DAT
Output Filename: A:LTSSA2.OUT

PROBLEM DESCRIPTION UMTRA-LOWMAN #1 LONG TERM SEIMIC-THRU SAND

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6

LIMITRA-LOW B.23
Rev Δ Slips Stability
PYL 7/12/02

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	869.07	281.11
2	874.07	278.04
3	930.07	314.88
4	931.36	316.99

*** 3.778 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	856.64	273.95
2	861.37	271.05
3	946.07	324.22
4	947.24	326.13

*** 3.800 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	864.73	278.60
2	869.58	275.63
3	931.73	315.86
4	933.00	317.93

*** 3.810 ***

Failure Surface Specified By 4 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	862.21	277.15
2	867.43	273.95
3	946.44	325.02
4	947.05	326.02

*** 3.817 ***

11	597.27	216.93
12	599.27	220.84

PYL 12/12/29
WV - 2 7/29

*** 5.467 ***

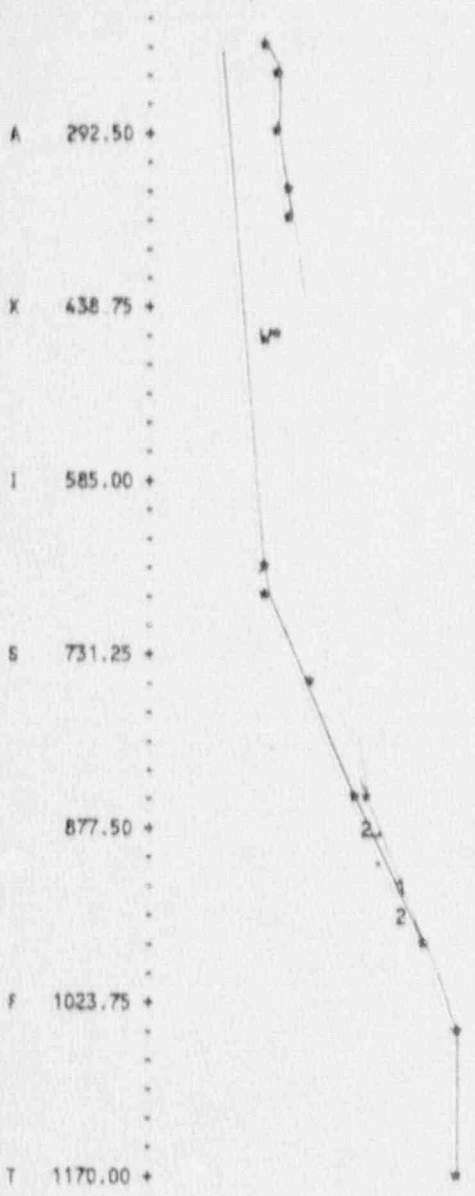
Failure Surface Specified by 12 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	262.28	160.00
2	269.26	156.52
3	282.34	150.00
4	298.96	141.71
5	432.77	130.19
6	565.56	139.28
7	570.90	150.00
8	584.29	176.85
9	585.69	179.66
10	592.77	193.86
11	604.18	218.32
12	606.18	222.25

*** 5.496 ***

8-23

LIMTRA-LOW
Flow Δ Slope stab. etc.
P.L.C. 2/12/90





MORRISON-KNUDSEN ENGINEERS, INC.
A MORRISON KNUDSEN COMPANY

Project UMTRA-LCW
Feature Embankment Design
Item Rev A Slope Stability - App C

Contract No. 3885-4d
Designed PTL
Checked WYI

Sheet C 1
File No. _____
Date 12/4/90
Date 12/5/90

REV A

APPENDIX C



Project UMTRA - LOW
Feature Embankment Design
Item Slope Stability Analysis App. C

Contract No. 3885-48 Sheet C-1
Designed PYL File No. _____
Checked WYL Date 12/4/90
Date 12/5/90

I. PURPOSE & INTRODUCTION

This appendix C is added in response to NRC's informal comments on the validity of the strength parameters used for colluvium foundation material in slope stability analyses. (See pl. 3 on attached NRC comments). Average of 2 UU test data ($C = 350$ psf, $\phi = 17^\circ$) have been used by the RAC for short-term seismic strength parameters for colluvium. (For sample 022-4 $C = 100$ psf $\phi = 17^\circ$, for sample 025-6 $C = 100$ psf $\phi = 17.5^\circ$, Ref 4). Results of the slope stability analyses show that for short-term seismic case, the critical failure occurs in the colluvium layer and the computed Factor of Safety is 1.10, equal to the minimum required based on the RAC's criteria. The TAC's minimum required FS is 1.0 (Ref. Technical Approach Document). To achieve that C must be at least 200 psf, with ϕ unchanged. (See POSTABLE printout attached). Thus C value is higher than the lower of the 2 test values (100 psf).

Nevertheless, the RAC believe that it is still valid and reasonable to use the previous average parameters for the material, rather than the lower value ^(for sample 025-6) which are considered unreasonably low. In fact, the average parameters used have already some degree of conservatism. The next section provides the justification for using the average parameters and their validity.



II JUSTIFICATION FOR USING THE AVERAGE PARAMETERS & THEIR VALUES

1. Undrained shear strength can often be correlated with the SPT N values:

For sample 023-4 depth 6'-9' N = 11 (Ref. 3
burlap)
 For sample 025-6, depth 12'-15' N = 12

Based on the $N = q_u$ (undrained compressive strength) correlation as shown in sl. C 8.

$$\begin{aligned} \text{For } N = 11, \quad q_u &= 0.8 \text{ tsf} & S_u &= \frac{q_u}{2} = 800 \text{ psf} \\ & & & \\ & 12, \quad q_u &= 0.9 \text{ tsf} & S_u &= \frac{q_u}{2} = 900 \text{ psf} \end{aligned}$$

Coulomb - Mohr criterion: $\tau = c + \sigma \tan \phi$

In total stress analysis, τ is also defined as $\tau = S_u$

$$\therefore S_u = c + \sigma \tan \phi$$

To back-figure c using the above equation for the 2 samples, using ϕ obtained from UU tests:-

$$\begin{aligned} \text{Sample } 023-4 \quad \phi = 17^\circ \quad \sigma = 919 \text{ psf}^* & \Rightarrow \underline{c = 520 \text{ psf}} \\ 025-6 \quad \phi = 19.5^\circ, \quad \sigma = 1654 \text{ psf} & \Rightarrow \underline{c = 380 \text{ psf}} \end{aligned}$$

Therefore, c should probably be in the range of 380 - 520 psf with a ϕ value of about 17°. A c -value of 100 psf thus is unreasonably low. Using the average UU test value of $c = 350$ psf is considered reasonable and conservative.

(* based on $\sigma_v = 122.5$ psf for cohesion material. (Ref 4)





Project WMTCA - LOW
Feature Embankment Design
Item App. C

Contract No. 385-41
Designed PTL
Checked WYL
File No. _____
Date 12/4/90
Date 12/5/90

2. Failure criterion used in UU tests

A conservative 4% strain failure criterion has been used, especially for sample C25-6 (see sk. C-11). Thus, the actual strength could be higher than that used in the analyses (based on stress paths of CU tests).

3. Effect of degree of saturation on undrained shear strength

Average degree of saturation for colluvium layer is 68.5% (Ref. Material Properties table p. 10). Samples for UU tests were basic saturated. Strength for partially saturated soil is often higher than that for fully saturated (Ref. C-1). Thus, using strength parameter from saturated sample for partially saturated colluvium soil is conservative.

4. Conservative construction of Mohr's envelope

Mohr's envelope for UU test on sample C25-06 were conservatively constructed using the largest circle at highest conf. pressure and smallest circle at lowest conf. pressure but ignoring the middle circle (see sk. C-10). This ending up with a very low cohesion intercept. Should the second and third circle be used instead, strength parameter would be much higher. Thus, C of 100 psf is very conservative. And using just the parameter from the sample above in analysis would be too unreasonably conservative.





Project Highway - LOW Contract No. 2025-42 Sheet C-4
 Feature Embankment Design Designed PYL File No. _____
 Item App C Checked WYL Date 12/6/90
 Date 12/5/90

5. Overconsolidation of Alluvium foundation material

Results of consolidation tests on alluvium samples indicate that the alluvium layer is overconsolidated (Ref. & Historical Properties). For sample 627-05 at depth 6-9', OCR is about 14. The undrained strength S_u of the overconsolidated sample can be estimated using the correlation shown on sl. C-9.

$$\text{At } OCR = 14 > 7 \quad \therefore B/A = OCR^{0.25} \\ = 14^{0.25} = 7$$

$$7 = B/A = \frac{(S_u/p_o)_{OCR}}{(S_u/p_o)_{NC}}$$

$$\Rightarrow (S_u)_{OCR} = 7 \cdot p_o \cdot (S_u/p_o)_{NC}$$

$$\text{avg } (S_u/p_o)_{NC} \approx 0.23 \quad (\text{sl. C-9, C-10}) \text{ for } p_o = 7$$

$$\therefore (S_u)_{OCR} = 7 \times 122.5 \text{ psf} \times 9.5' \times 0.23 \\ = \underline{\underline{1479 \text{ psf}}}$$

Assume this consolidation test sample also represents the alluvium material in other locations within the embankment area. Then compared with this value the lower value set of parameters for (111) & (101) (at 100' & 2-19.5') appears to be reasonably low. $S_u = 654 \text{ psf}$ at 9' deep. Using the average values $(S_u/p_o)_{NC} = 0.23$ & $p_o = 122.5$ it will be more reasonable.





Project LA 77 EP - LHW
 Feature Embankment Design
 Item App G

Contract No. 2825-60
 Designed PYL
 Checked WYL

Sheet C-5
 File No. _____
 Date 12/4/90
 Date 12/5/90

6. Sample Disturbance

Based on previous experience with UU tests, due to often sampling disturbances, the strength parameters obtained from the lab are often lower than actual in the field. Actual factor of safety against failure in coliseum is expected to be higher than 1.10.

III CONCLUSIONS

Based on engineering judgement and experience, considering the above points, the RAC believe that the average parameters from 2 UU test data are reasonable, realistic values, with reasonable degree of conservatism, and are valid to use in slope stability analyses.

References

- C-1. Bishop, A.W. & Bjerrum, L., 'The Relevance of the Triaxial Test to the Solution of Stability Problems' Proceedings of the ASCE Research Conference on the Shear Strength of Cohesive Soils, Boulder, CO, 1960
- C-2. Jamiolkowski, M et al., 'New Developments in Field and Laboratory Testing of Soils' Proceedings of the Eleventh ICSMFE, San Francisco, Vol 1 August 1972.



C.6
PYL 11/4/90
WYL 11/5/90

ATTACHMENT 2

IMPORTANT NOTICE

The attached comments or questions have resulted from the NRC's review of the Lowman draft Remedial Action Selection Report (RAS). Under streamlining agreement 3, the NRC is to provide verbal comments on the draft RAS at a site visit meeting. These informal comments are being provided to the DOE as a convenience, but should not be considered formally transmitted comments, nor shall they be considered as all encompassing. The NRC staff does not expect DOE to provide formal responses to the individual comments/questions, but to take these comments into account in the preparation of the final RAS.

PYL 12/4/90
WYL 12/5/90

Informal Review of Lowman, Idaho Draft RAP
Geotechnical Engineering

1. Section 3.2.2 of the Remedial Action Selection Report indicates that Figure 1 in Volume I of the Information for Bidders shows the location of test pits (21) and boreholes (9). The same section indicates that there are an additional 64 boreholes drilled to depths from 10 to 150 feet. Although these borings are associated with the contamination investigation, they provide important information (material descriptions, blow counts) and should be included on the borehole location plan.
2. All of the samples of the radioactive sands were collected from the top 2.5 feet of the piles. Evidence should be provided to demonstrate that these surface samples are representative of the entire depth of the piles.
- * 3. The slope stability analysis (short-term with earthquake loading case) resulted in a factor of safety equal to the minimum allowable. The failure surface occurs in the foundation colluvium. The colluvium layer strength parameters used in the analysis are based on an average of only two (00) tests. In order to obtain a better basis for the strength parameters for this critical layer, consideration should be given to performance of additional laboratory strength tests.
4. The model for the slope stability analysis included a thin, partial layer of colluvium on the steep slope above the disposal cell. What is the basis for the characterization of the thickness and extent of this layer? What are the results of a shallow surface failure analysis applied to this layer? If the results indicate potential for failure, what would be the impact to the disposal cell, i.e., ditch clogging, cover disruption?
5. The slope stability analysis does not appear to have considered a potential failure surface through the alluvium on the Clear Creek embankment at the toe of the tailings cell. This case should be analyzed.
6. The Remedial Action Plan documentation contains no discussion relative to the protection of the radon barrier from the effects of frost degradation. Discussion on the potential for damage due to frost should be provided.
7. Typos:
 - Remedial Action Selection Report, Table 3.1; Data for the in situ tailings, relocated tailings and radon barrier are all under the wrong columns and the unit weight data is missing.
 - Remedial Action Selection Report, Section 3.3.5; Paragraph 1 reference to Section 3.2.2 should be 3.3.3. Paragraph 5 reference to Detail 1 should be Detail 3.
 - Calculations, Volume II, 12-624-01-00 Material Properties, Summary of Colluvial Test Results; Sample 027-5 classification is wrong - GC should be SC.

PYL 12/4/90
WV- 12/2/93

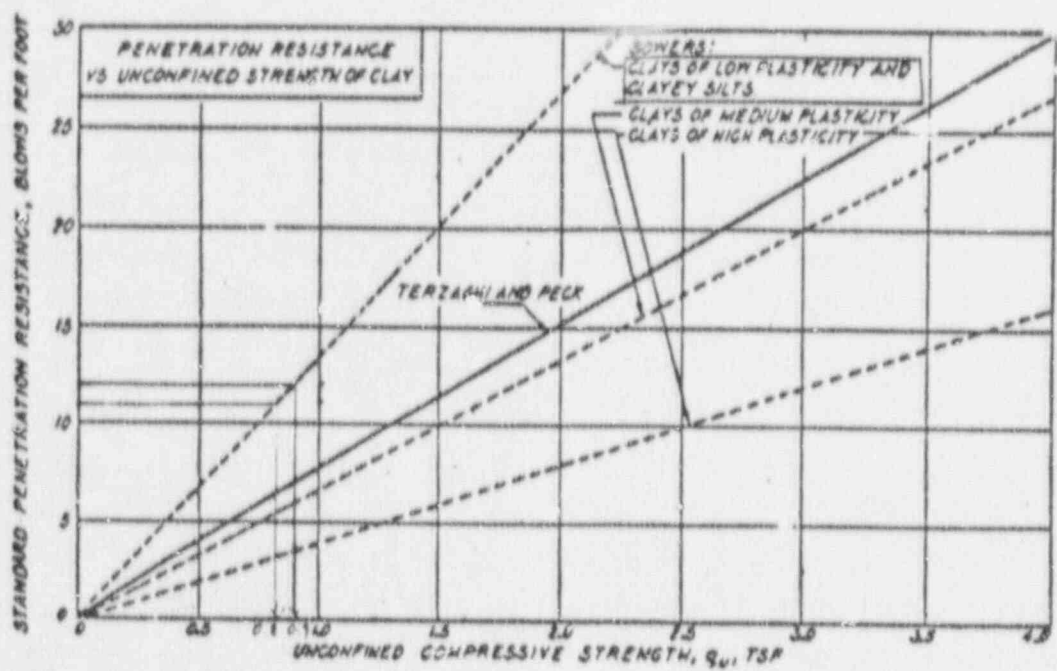


Fig. 47. Correlation Between Standard Penetration Resistance and Unconfined Compressive Strength for Clay (After DM-7).

Project DMTR - LOW
Feature Embankment Design
Item App C

Contract No. 3015-41 File No. _____
Designed PTL Date 12/4/90
Checked WJL Date 12/5/90

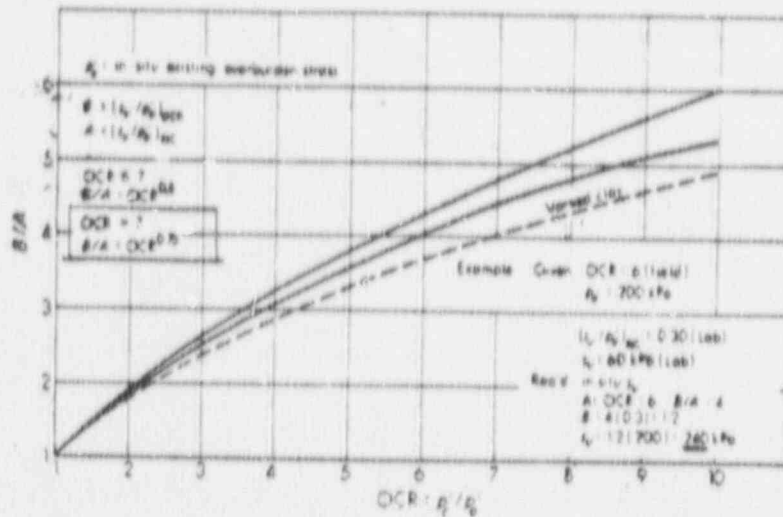


Figure 2-18 Normalized plot to obtain the ratio of undrained shear strength at some pre-consolidation pressure to a normally consolidated value for several OCR [After Ladd et al. (1977)]

From Ref [C-3] Bowles J. E., "Foundation Analysis and Design, Third Edition, McGraw-Hill, Inc."

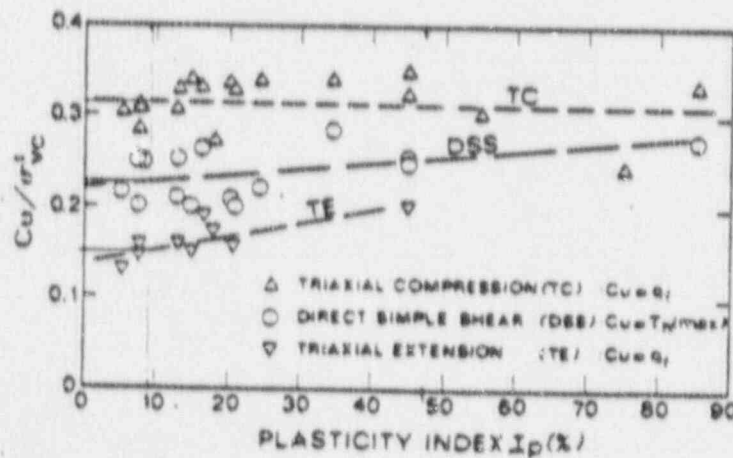


Fig. 21: Undrained Strength Anisotropy from CK_0U , Tests Performed on Normally Consolidated Clays [Data from Lefebvre et al. (1983), Vaid and Campanella (1974) and Various Publications by MIT and NGI].

From Ref [C-2]

SOIL TEST RESULT SUMMARY

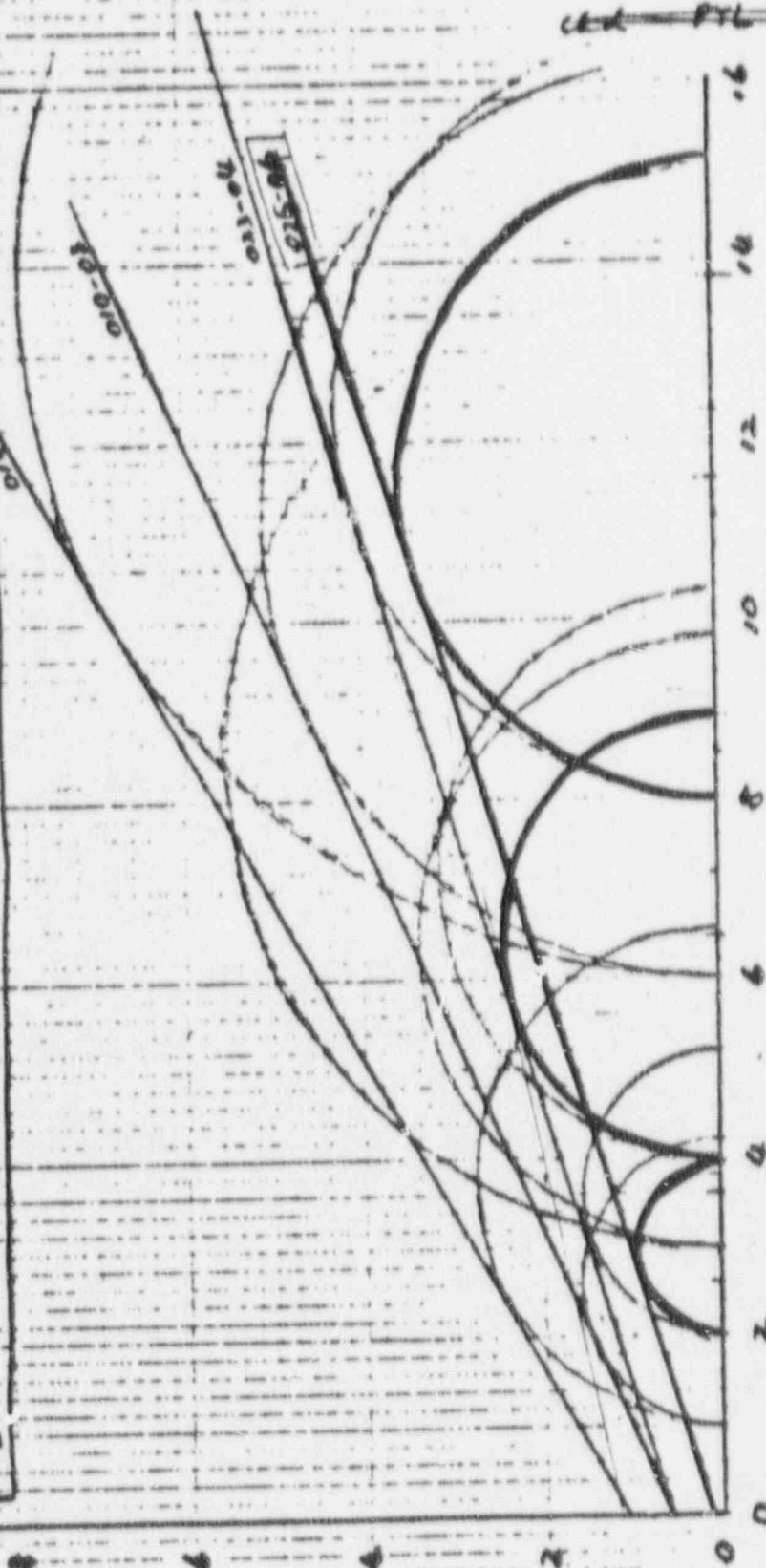
At 4% strain

Location	Sample Depth	soil	w (%)	f _c (pcf)	C (pcf)	φ	
01P	03	25'-80"	SM	15.2	110	600	24.5
01B	03	20'-25"	SC	12.5	121.0	1100	30.0
02B	06	50'-90"	SC	12.0	101.4	800	19.0
02F	06	120'-150"	SM	14.7	110.6	100	17.5

remolded
RADON BREKER

IN-SITU
COLUMN

Shear Stress (ksf)



Normal Stress (ksf)

W/L 12/4/90
W/L 12/5/90
C-10

W/L 12/2/90 C-11

PRC 11/6/90

PRC 12/4/90

W/L 12/2/90

TRIAxIAL 'Q' TEST

6/17/88 LOMAN

	TEST/STAGE 1	TEST/STAGE 2	TEST/STAGE 3
LOCATION ID:	025	025	025
SAMPLE ID:	05	05	05
DEPTH INTERVAL (FT):	12.0 - 18.0	12.0 - 18.0	12.0 - 18.0
INITIAL DRY DENSITY (PCF):	110.8	114.2	114.1
INITIAL MOISTURE CONTENT (%):	18.7	14.2	14.8
CELL CONFINING PRESSURE (PSF):	1850.	4000.	8000.
UNIFIED SOIL CLASSIFICATION:	SM	SM	SM
IN-SITU COLLUVIUM			

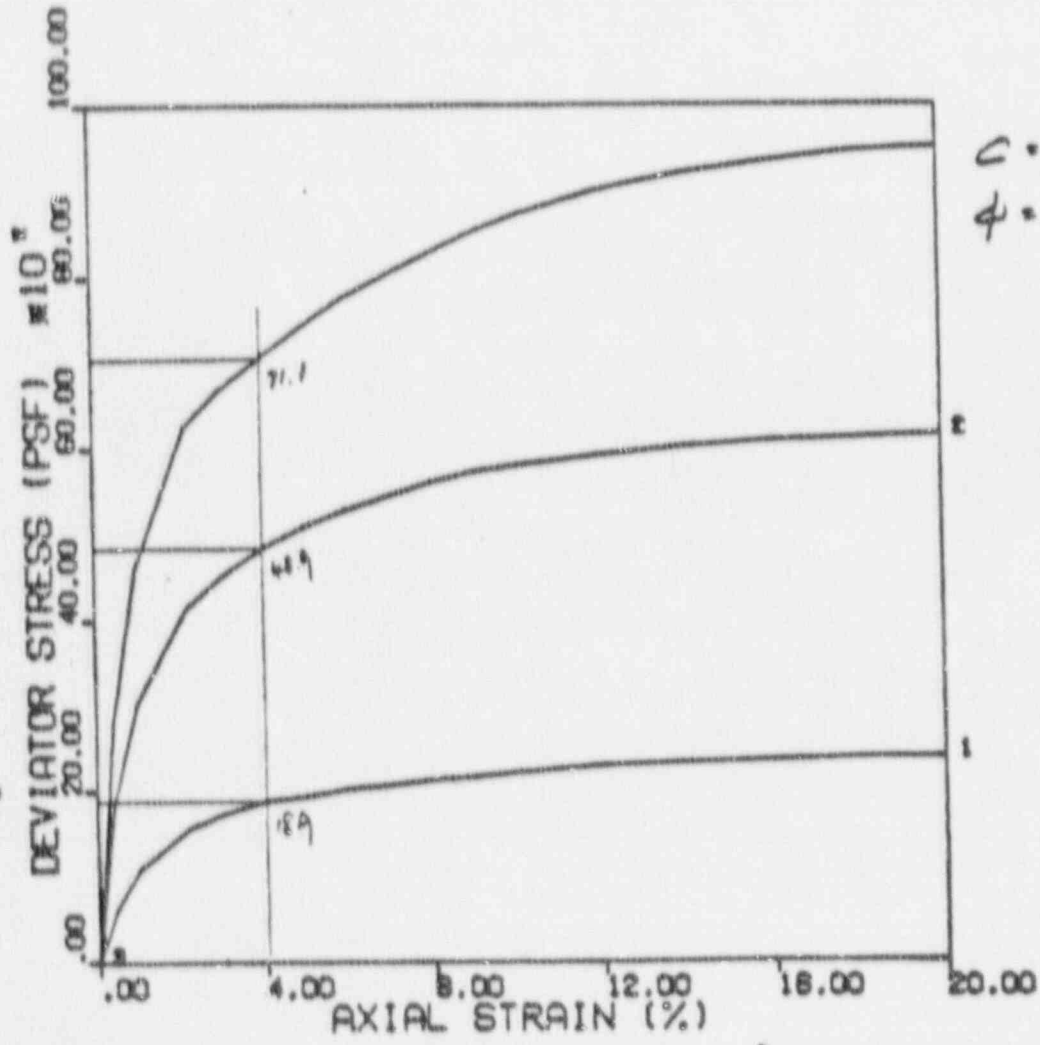


FIGURE D.4.3.D
TRIAxIAL 'Q' TEST

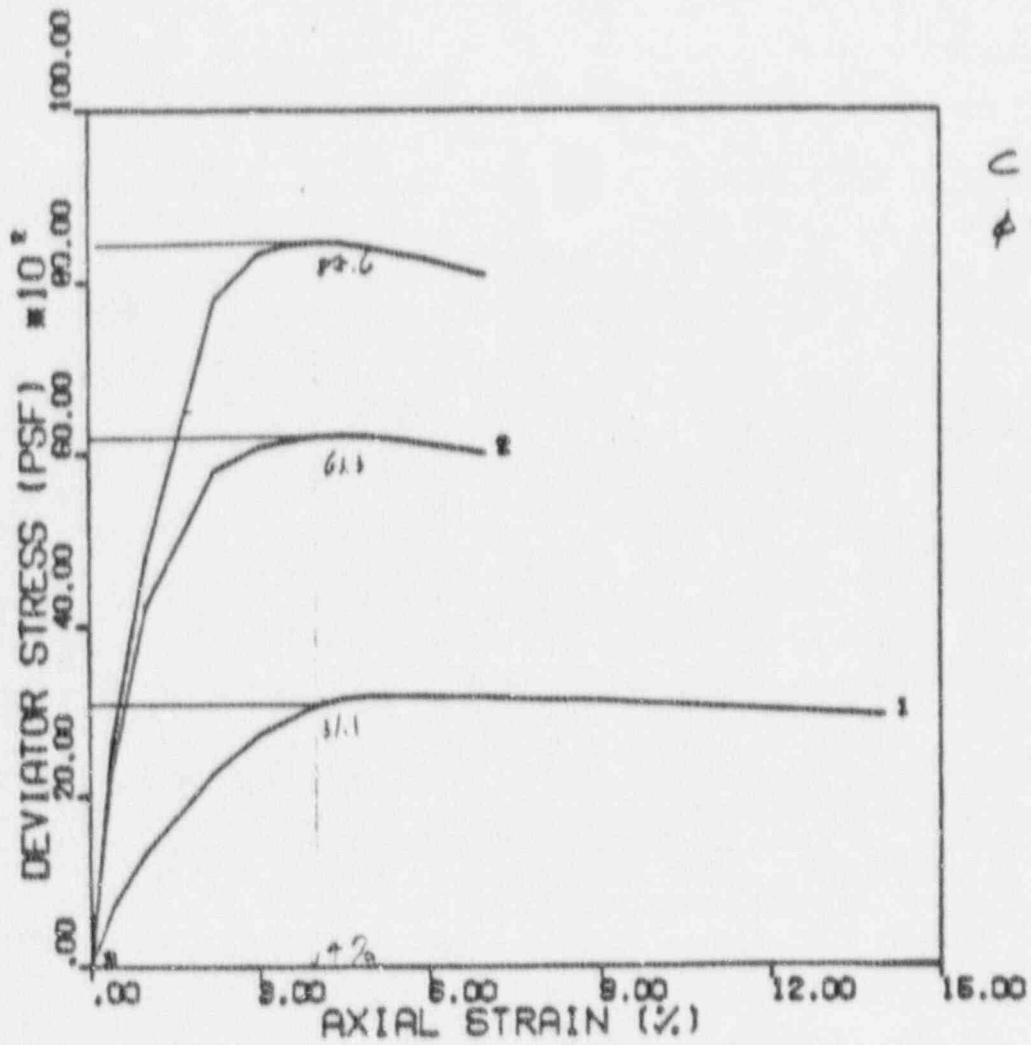
From Ref L

W.L.L. 12/4/90
 W.L.L. 12/5/90
 C-12

TRIAXIAL 'Q' TEST

5/17/88 LOWMAN

	TEST/STAGE 1	TEST/STAGE 2	TEST/STAGE 3
LOCATION ID:	02B	02B	02B
SAMPLE ID:	04	04	04
DEPTH INTERVAL (FT):	8.0 - 8.0	8.0 - 8.0	8.0 - 8.0
INITIAL DRY DENSITY (PCF):	101.4	108.8	118.0
INITIAL MOISTURE CONTENT (%):	12.0	12.2	12.8
CELL CONFINING PRESSURE (PSF):	2002.	4008.	8008.
UNIFIED SOIL CLASSIFICATION:	SC	SC	SC
IN-SITU COLLUVIUM			



$c = 600 \text{ psf}$
 $\phi = 17.^\circ$

FIGURE D.4.3.C
 TRIAXIAL 'Q' TEST

From Ref. 1

** PCSTABL5 **

by
Purdue University

--Slope Stability Analysis--
Simplified Janbu, Simplified Bishop
or Spencer's Method of Slices

C.13

PYL 12/4/90
WVL 12/5/90

C
200 12 3
= 2 6 1 1

Run Date:
Time of Run:
Run By: PYL
Input Data Filename: A:STSCOLL.DAT
Output Filename: A:STSCOLLB.DAT

PROBLEM DESCRIPTION UNTRA-LOWMAN #1 SHORT TERM SEIMIC-THRU C
OLLUVIUM

BOUNDARY COORDINATES

8 Top Boundaries
26 Total Boundaries

Boundary No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Soil Type Below Bnd
1	.00	70.00	50.00	70.00	5
2	50.00	70.00	125.00	90.00	5
3	125.00	90.00	235.00	160.00	3
4	235.00	160.00	295.00	160.00	3
5	295.00	160.00	350.00	170.00	3
6	350.00	170.00	855.00	273.00	1
7	855.00	273.00	980.00	345.00	3
8	980.00	345.00	1055.00	390.00	6
9	1055.00	390.00	1170.00	390.00	6
10	350.00	170.00	370.00	171.00	3
11	370.00	171.00	850.00	268.00	2
12	850.00	268.00	855.00	273.00	3
13	370.00	171.00	750.00	210.00	3
14	750.00	210.00	850.00	268.00	3
15	125.00	90.00	135.00	94.00	5
16	135.00	94.00	230.00	150.00	5
17	230.00	150.00	235.00	155.00	4
18	235.00	155.00	460.00	165.00	4
19	460.00	165.00	760.00	200.00	4
20	760.00	200.00	980.00	345.00	6
21	230.00	150.00	680.00	150.00	5
22	680.00	150.00	760.00	200.00	6
23	.00	60.00	95.00	60.00	6
24	95.00	60.00	177.00	113.00	6
25	177.00	113.00	670.00	142.00	6
26	670.00	142.00	680.00	150.00	6

ISOTROPIC SOIL PARAMETERS

PYL 12/4/90

WYL 12/5/90

6 Type(s) of Soil

Soil Type No.	Total Unit Wt. (pcf)	Saturated Unit Wt. (pcf)	Cohesion Intercept (pcf)	Friction Angle (deg)	Pore Pressure Param. (pcf)	Pressure Constant (pcf)	Piez. Surface No.
1	130.0	135.0	850.0	27.0	.00	.0	1
2	144.0	155.0	.0	40.0	.00	.0	1
3	122.5	130.5	200.0	17.5	.00	.0	1
4	124.5	131.0	1460.0	32.0	.00	.0	1
5	124.5	131.0	1460.0	32.0	.00	.0	1
6	137.0	143.5	.0	45.0	.00	.0	1

1 PIEZOMETRIC SURFACE(S) HAVE BEEN SPECIFIED

Unit Weight of Water = 62.40

Piezometric Surface No. 1 Specified by 8 Coordinate Points

Point No.	X-Water (ft)	Y-Water (ft)
1	.00	75.00
2	115.00	75.00
3	182.00	120.00
4	460.00	142.00
5	670.00	142.00
6	980.00	345.00
7	1055.00	390.00
8	1170.00	390.00

A Horizontal Earthquake Loading Coefficient
Of .170 Has Been AssignedA Vertical Earthquake Loading Coefficient
Of .000 Has Been Assigned

Cavitation Pressure = .0 pcf

A Critical Failure Surface Searching Method, Using A Random
Technique For Generating Sliding Block Surfaces, Has Been
Specified.The Active And Passive Portions Of The Sliding Surfaces
Are Generated According To The Rankine Theory.

100 Trial Surfaces Have Been Generated.

4 Boxes Specified For Generation Of Central Block Base

C-15

Length Of Line Segments For Active And Passive Portions Of Sliding Block Is 30.0

RYL 12/6/90
W L 12/5/90

Box No.	X-Left (ft)	Y-Left (ft)	X-Right (ft)	Y-Right (ft)	Height (ft)
1	320.00	162.00	420.00	167.00	5.00
2	460.00	170.00	580.00	180.00	10.00
3	670.00	195.00	740.00	204.00	10.00
4	795.00	228.00	845.00	260.00	5.00

Following Are Displayed The Ten Most Critical Of The Trial Failure Surfaces Examined. They Are Ordered - Most Critical First.

* * Safety Factors Are Calculated By The Modified Janbu Method * *

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	364.82	173.02
2	368.26	170.91
3	376.56	164.83
4	530.16	173.16
5	732.77	199.54
6	818.52	244.75
7	824.89	253.43
8	829.77	263.91
9	832.53	268.42

*** 1.004 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	358.74	171.78
2	360.77	170.54
3	370.51	163.40
4	577.27	178.75
5	730.72	201.09
6	802.21	233.43
7	810.96	245.36
8	818.56	261.65
9	821.30	266.13

*** 1.005 ***

Failure Surface Specified By 10 Coordinate Points

PYL 12/4/90

WYL 12/2/90

Point No.	X-Surf (ft)	Y-Surf (ft)
1	393.85	178.96
2	397.58	176.59
3	402.52	174.34
4	411.52	167.74
5	525.24	179.29
6	728.80	198.54
7	833.66	255.05
8	838.08	261.09
9	840.40	260.06
10	843.17	270.59

*** 1.013 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	391.91	178.55
2	395.74	176.20
3	400.24	174.10
4	411.72	165.69
5	519.85	173.63
6	731.13	198.78
7	795.47	226.29
8	808.34	243.84
9	816.44	261.22
10	819.19	265.70

*** 1.023 ***

Failure Surface Specified By 10 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	395.05	179.19
2	398.89	176.84
3	403.94	174.48
4	411.91	168.63
5	514.46	177.97
6	733.45	199.02
7	807.29	234.53
8	818.39	249.67
9	824.54	262.85
10	827.29	267.35

*** 1.026 ***

Failure Surface Specified By 8 Coordinate Points

C-17

DYL 12/4/90
WYL 12/5/90

Point No.	X-Surf (ft)	Y-Surf (ft)
1	332.19	166.76
2	335.16	164.59
3	472.97	173.94
4	730.03	201.61
5	813.44	238.65
6	823.83	252.82
7	828.93	263.74
8	831.68	268.24

*** 1.030 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	353.89	170.79
2	354.79	170.24
3	364.84	162.87
4	493.60	173.00
5	733.31	203.13
6	809.54	238.57
7	817.14	248.94
8	823.53	262.65
9	826.28	267.14

*** 1.038 ***

Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	365.57	173.17
2	369.18	170.96
3	375.98	165.98
4	546.33	180.15
5	725.79	198.82
6	833.07	252.04
7	840.90	262.72
8	842.67	266.52
9	845.45	271.05

*** 1.042 ***

Failure Surface Specified By 10 Coordinate Points

C-18

PYL 12/4/90

WYL 12/5/90

Point No.	X-Surf (ft)	Y-Surf (ft)
1	381.19	176.36
2	385.00	174.03
3	387.62	172.81
4	400.00	163.73
5	483.28	173.47
6	731.67	202.37
7	836.49	252.11
8	846.76	266.12
9	847.39	267.47
10	850.18	272.02

*** 1.049 ***

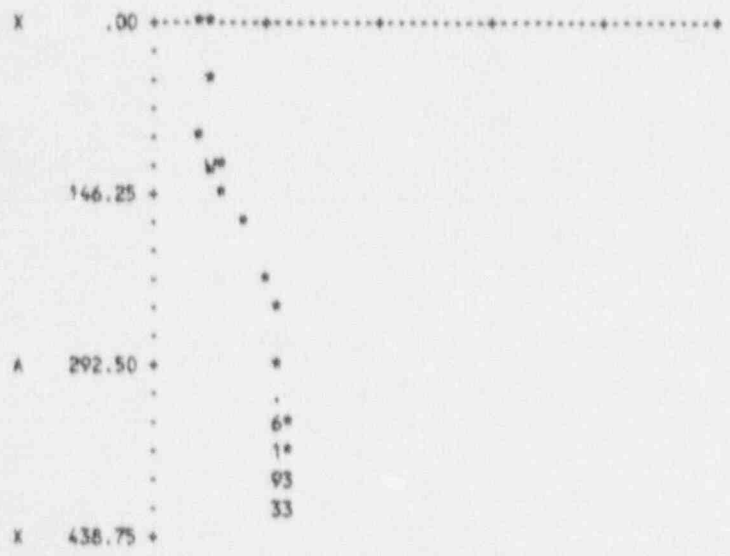
Failure Surface Specified By 9 Coordinate Points

Point No.	X-Surf (ft)	Y-Surf (ft)
1	357.09	171.45
2	358.74	170.44
3	365.04	165.82
4	488.21	177.34
5	735.64	203.37
6	821.35	246.81
7	827.19	254.77
8	831.63	264.29
9	834.39	268.80

*** 1.051 ***

Y A X I B F T

.00 146.25 292.50 438.75 585.00 731.25



ENGINEERS
AND
CONSTRUCTORS



MK-FERGUSON COMPANY
A MORRISON-KNUDSEN COMPANY

HEADQUARTERS OFFICE
ONE EIGHT SEVEN AND A
SIXTY SEVEN ONE EAST L STREET
PHOENIX, ARIZONA 85016

REGIONAL OFFICE
CENTRAL-SOUTHWEST REGION
P.O. BOX 9146
ALBUQUERQUE, NEW MEXICO 87106
91-3050-044

January 17, 1991

Mr. Mark L. Matthews
Project Manager
U.S. Department of Energy
Uranium Mill Tailings Remedial Action Project Office
First National Bank Building
5301 Central Avenue N.E.
Suite 1700
Albuquerque, New Mexico 87108

SUBJECT: Lowman Soil Test Data

REFERENCE: Contract No. DE-AC04-83AL18796

Dear Mr. Matthews:

Enclosed please find a copy of the PS1 Geotechnical Investigation Report for radon barrier material testing. Also included are soil test result summary sheets, which list results of the recently performed testing and data from earlier testing by the TAC. The latest results show the material to have a higher gravel content and a lower fines content than the previous data indicated. Therefore, we propose to change the specifications for radon barrier material to require soils with a minimum of 15% by weight passing the No. 200 sieve and a maximum of 35% by weight retained on the No. 4 sieve.

These enclosures are being transmitted to Ray Bennett at the TAC, so we can obtain concurrence on this specification change as soon as possible.

If you have any questions, please contact Charles Spencer or Rob Cooney of my staff at 766-3093.

Sincerely,

MK-Ferguson Company

J. G. Oldham
J. G. Oldham
Project Director

JOO/REC/CRS/sls
cc (R. Bennett - TAC/UMTRA (w/enc.))
P. Mann - DOE/UMTRA (w/o enc.)

39608

REV. Δ RADON BARRIER ADDITIONAL DATA
(REMOLDED COLLUVIUM)

SUMMARY OF

Job No 3885-48

Project Name UMTRA - LOWMAN

Hole or Trench Number	Sample Number	Depth		Laboratory Classification	Mechanical Analysis			Atterberg Limits		Specific Gravity G	Natural		Test
		From	To		Gravel	Sand	Fines	LL	PI		w %	Y _r	
TP-1	A	3.0'	5.0'	SM-SC	24	56	20	24	5		3.7		D698
	B	7.0'	9.0'	SM	30	53	17	NP	NP	2.66	10.0		D698
TP-2		3.0'	5.0'	SM-SC	30	36	34	25	6	2.64	11.1		D698
TP-3		5.0'	7.0'	SM	32	39	29	NP	NP		9.4		D698
TP-4	A	3.0'	5.0'	SM	29	53	18	NP	NP		3.0		D698
	B	7.0'	9.0'	SM	37	46	17	NP	NP	2.64	4.8		D698

FOR INFORMATION ONLY

* Visual Classification

SP • Standard Proctor
MP • Modified Proctor
S • Special - See Test

TC • Triaxial Compression
UC • Unconfined Compression
DS • Direct Shear

UU • Unconsolidated Undrained
CU • Consolidated Undrained
CD • Consolidated Drained

C = Capillary Moisture

SOIL TEST RESULTS

Feature Embankment Design
Material Properties

Date 1/14/91

Compaction Level	Optimum W _p	Shear Strength 12.5%			C	* (See 5h. --)	Permeability k		Consolidation			Notes	ORGANIC CONTENT	
		Comp. (%)	W _p	W _L			W _p /W _L	W _p /W _L	(cm/sec)	C	C			E
1.3	122.3													
2.0	121.1	95%	14.5	11.5		*	14	11.5	4.54 x 10 ⁻⁶				C (95% Compaction)	
1.5	121.0	95%	12.5	114.9			12.5	114.9	1.93 x 10 ⁻⁶				C (95% Compaction)	0.32%
1.5	121.5	95%	12.5	115.4		"	12.5	115.4	2.06 x 10 ⁻⁶				C (95% Compaction)	0
3	120.5													
5	119.5													

SI
APERTURE
CARD

Also Available On
Aperture Card

% strain was used as failure criteria

9102080294-03

(B)

FOUNDATION SOIL - COLLECTIVITY

SUNYMA

Job No 5057-06

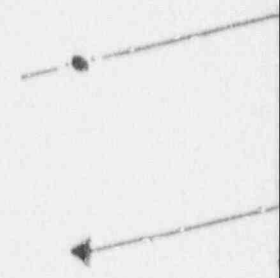
Project Name UMTRA

Hole or Trench Number	Sample Number	Depth		Laboratory Classification	Mechanical Analysis			Atterberg Limits		Specific Gravity G	Nat W %	
		From	To		Gravel	Sand	Fines	LL	PI			
001	2	3.5	4.0	SM/SC	9	65	26				10.5	
004	1	1.0	1.5	SC							13.8	
005	1	2.0	2.5	SM							13.5	
010	1	1.0	1.5	SM							18.1	
	3	7.5	8.0	ML/CL	4	43	53	28	6	2.66	15.2	
011	1	2.0	2.5	SM							9.1	
	5	11.5	12.0	SM/SC	10	69	21				11.6	
016	3	8.0	8.5	SC	0	78	22	30	10	2.63	12.5	
023	2	3.0	4.5	SM/SC				29	7		14.3	
	4	6.0	9.0	SC	5	66	29	30	10	2.70	12.0	
025 _u	6	12.0	15.0	SM/SC	4	66	30	25	5	2.67	15.7	
	8	16.5	19.5	SM/SC	5	72	23	30	10	2.65	11.3	
	11	24.0	25.5	SM/SC							11.6	
	15	36.0	37.5	SM/SC							11.7	
	17	44.0	45.5	SM/SC	27	57	16				13.6	
026	4	4.5	6.0	SM	0	57	43				14.8	
	6	8.5	10.5	SM	4	67	29	NP	NP	2.68	17.9	
027	4	4.5	6.0								15.1	
	5	6.0	9.0	GC	2	51	47	36	16	2.68	14.6	
028	4	4.5	6.0	SM/SC	0	86	14				4.6	
029	2	1.5	3.0	SM/SC	3	77	20				3.9	
				NO. OF TESTS		13		For plastic soil only		7	21	
				AVG.	SM/SC	5	66	29	30	9	2.67	12.6

EV. Δ ADDITIONAL CAPILLARY MOISTURE TEST RESULTS FOR RADON BARRIER MATERIAL

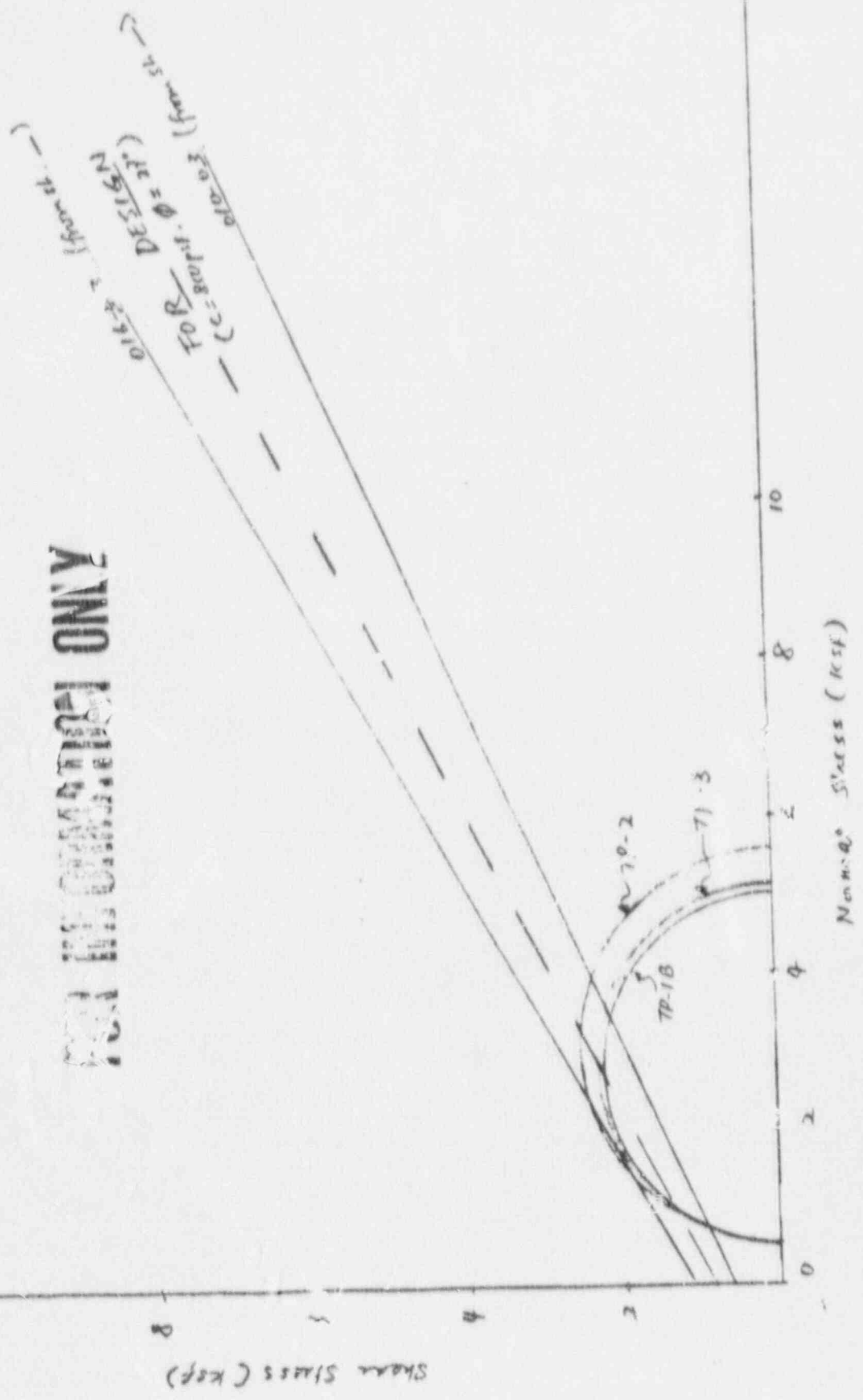
I.D.	Depth	γ_{cp} (pcf)	Comp.	Fine (#200)
TP-1B	7'-9"	115	95%	17%
TP-2	3'-5"	114.9	95%	34%
TP-3	5'-7"	115.4	95%	29%

DATA FROM REF. —



W.U. TEST RESULTS
FOR RADON BARRIER MATERIAL

FOR INFORMATION ONLY



Mkes Doc. No.
3885-LOW-R-09-00594-00

GEOTECHNICAL INVESTIGATION
UMTRA PROJECT - LOWMAN

performed for:
MK - FERGUSON COMPANY
POST OFFICE BOX 9136
ALBUQUERQUE, NEW MEXICO 87119

ATTENTION: MR. JOHN SINGLETON

PST FILE NUMBER
355-00118



Professional Service Industries, Inc.

230 SOUTH COLE ROAD
BOISE, IDAHO 83709
(208) 376-4748



Professional Service Industries, Inc.

TESTED FOR: MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT: LOWMAN, IDAHO

Attention: Mr. John Singleton

DATE: December 21, 1990

OUR REPORT NO: 355-00118-001 Pg. 1 of 46

REMARKS: Attached is our inspectors report, the test pit logs and all requested laboratory test results. It is noted that locations were described and staked by our client. The capillary moisture, triaxial shear and triaxial permeability were performed by Dames and Moore Inc., Salt Lake City, Ut. All other tests were performed by Professional Service Industries, Inc.

We appreciate this opportunity to be of service. If you have any questions please give our office a call.

Very truly yours,
PROFESSIONAL SERVICE INDUSTRIES, INC.

A handwritten signature in black ink, appearing to read 'Bob J. Arnold', is written over the typed name.

Bob J. Arnold, P.E.
Division Manager



Professional Service Industries, Inc.

TESTED FOR: MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT: LOWMAN, IDAHO

Attention: Mr. John Singleton

DATE: December 21, 1990

OUR REPORT NO: 355-00118-001 Pg. 2 of 46

REMARKS: Kevin Sagez-Project Inspector

I left the lab at 6:10 a.m. for Lowman, Idaho and arrived at the meeting point at South Fork Lodge at 7:50 a.m. Upon arrival the backhoe operator Gerry Rail from Gillingham Construction was already there. The operator was in having coffee. At 8:05 a.m. I met with John Singleton of MK-Ferguson to discuss the job and to review radiation safety procedures. At 8:30 a.m. the operator started reviewing safety procedures.

At 10:00 a.m. John and I examined the site and talked about how to handle the soil. A representative from Chen-Northern was with us at the time to check for contamination. At approx. 11:00 a.m. we started the first hole examination depth 3'-5'. Contaminated soil was removed first and placed to the right of the hole, when soil was found to be safe we proceed placing good soil to the left onto plastic sheets. Over all four holes were tested and examined. Four of the test pits were to depths 3'-5' and two pits TP1 and TP4 were extended to a depth of 7'-9'.

Soils were the result of the weathering soft granite producing a granitic sand:soil.

We returned to the lodge to check and discuss what tests needed to be performed. At 2:00 p.m. I left the lodge to return to the Boise lab. I returned with 12, five gallon buckets for further testing. On the following day six buckets were prepared and mailed to Rogers and Associates for further testing.
Nothing follows

Respectfully submitted,
PROESSIONAL SERVICE INDUSTRIES, INC.

TEST FIT LOGS

TP-1
Depth: 3'-5' & 7'-9'
Location: N11812, E10969

0	Surface organic soil, dark brown.
2"	Brown, weak fine granular, and weak subangular blocky sandy loam.
16"	Weathering of granite into sand.
42"	Sample: color value is 4, chroma is 3, 10 YR brown.
90"	Sample: color value is 5, chroma is 4, 10 YR yellow brown. Increasing larger rounded weather granite with depth.
108"	End of Excavation

TP-2
Depth: 3'-5'
Location: N11450, E1076

0"	Surface Black Uranium contamination and organic soil.
2"	Dark brown, weak fine granular and weak subangular blocky sandy loam.
6"	Blocky sandy loam.
50"	Sample: color value is 4, chroma is 2, hue 7.5 YR brown.
	Brown to light brown, massive and single grain gravelly sand 30 to 35 % fine gravel. Evidence of larger weathering granite below.
60"	End of Excavation

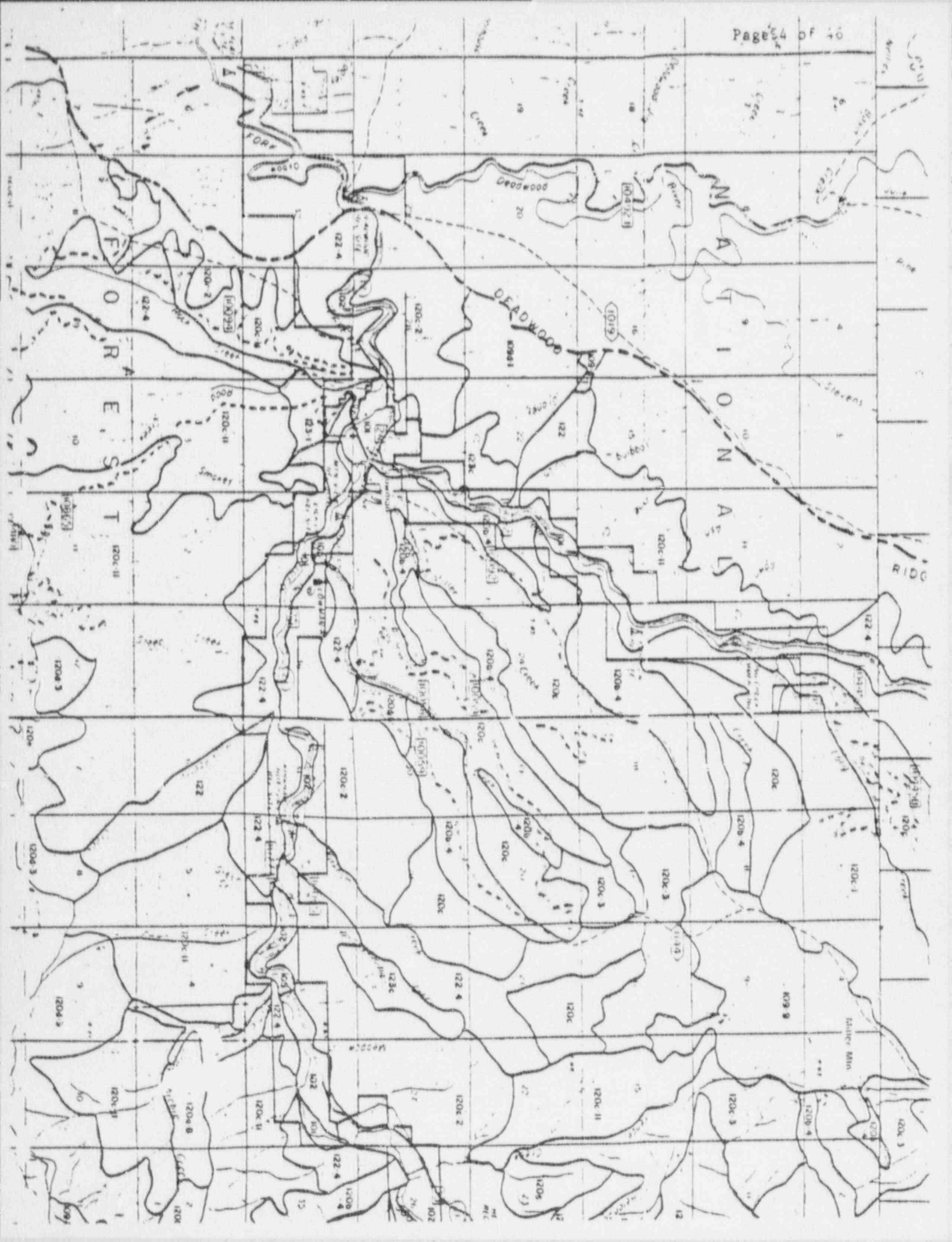
TP-3
Depth: 5'-7'
Location: N11149, E10800

0"	Surface Clean contaminated quartz sand.
3"	Original organic zone.
5"	Dark brown silty loam.
7"	Brown sandy silt increasing in sand and red coloration with depth.
62"	Sample Increase in the size of rounded weathering granite and red iron staining with depth.
84"	End of Excavation

TP-4
Depth: 3'-7' & 7'-9'
Location: N11312, E11058

0"	Surface Organic soil, dark brown.
2"	SAME AS TP-1
55"	Sample: color value is 5, chroma is 6, 10 YR yellow brown.
100"	Sample: color value is 5, chroma is 4, 10 YR yellow brown.
108"	End of Excavation

Note: Color values are from Munsell's Soil Color Charts.





Professional Service Industries, Inc.

TESTED FOR:
MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT:
LOWMAN, IDAHO

Attention: Mr. John Singleton

December 21, 1990

PSI Project #:355-00118-001

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REMARKS

TP-1A @ Depth 3'-5'

I. Particle Size Analysis with Hydrometer (ASTM D-422).

Sieve Size	Percent Passing
2.0'	100
1.5"	95
1.0"	92
3/4"	90
1/2"	87
3/8"	85
#4	76
#8	66
#16	53
#30	40
#50	30
#100	24
#200	20
1 Min.	20
2 Min.	17
5 Min.	15
15 Min.	12
30 Min.	11
60 Min.	10
250 Min.	8

II. Atterberg Limits (ASTM D-4318)

Liquid Limit	24
Plastic Limit	19
Plastic Index	5

III. Natural Moisture Content (ASTM D-2216)

Moisture %	3.7
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Respectfully submitted;
PROFESSIONAL SERVICE INDUSTRIES, INC.



Professional Service Industries, Inc.

TESTED FOR:
MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT:
LOWMAN, IDAHO

Attention: Mr. John Singleton

December 21, 1990

PSI Project #:355-00118-001

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REMARKS

TP-1B @ Depth 7'-9'

I. Particle Size Analysis with Hydrometer (ASTM D-422).

Sieve Size	Percent Passing
2.0"	100
1.5"	93
1.0"	88
3/4"	85
1/2"	81
3/8"	79
#4	70
#8	59
#16	45
#30	37
#50	30
#100	22
#200	17
1 Min.	17
2 Min.	15
5 Min.	10
15 Min.	9
30 Min.	8
60 Min.	6
250 Min.	5

II. Atterberg Limits (ASTM D-4318)

Liquid Limit	NP
Plastic Limit	NP
Plastic Index	NP

III. Natural Moisture Content (ASTM D-2216)

Moisture % 10.0

Respectfully submitted;
PROFESSIONAL SERVICE INDUSTRIES, INC.



REPORT OF MOISTURE DENSITY RELATIONSHIP OF SOIL

TESTED FOR: MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT: LOWMAN, IDAHO

Attention: Mr. John Singleton

DATE: December 21, 1990

OUR REPORT NO: 355-00118-001 Pg. 8 of 46

TEST DATA

Visual Classification: Sand with gravel and silt

Sample Source: TP-1B 7-9'

Method of Test: ASTM D698 Method D

Test Results

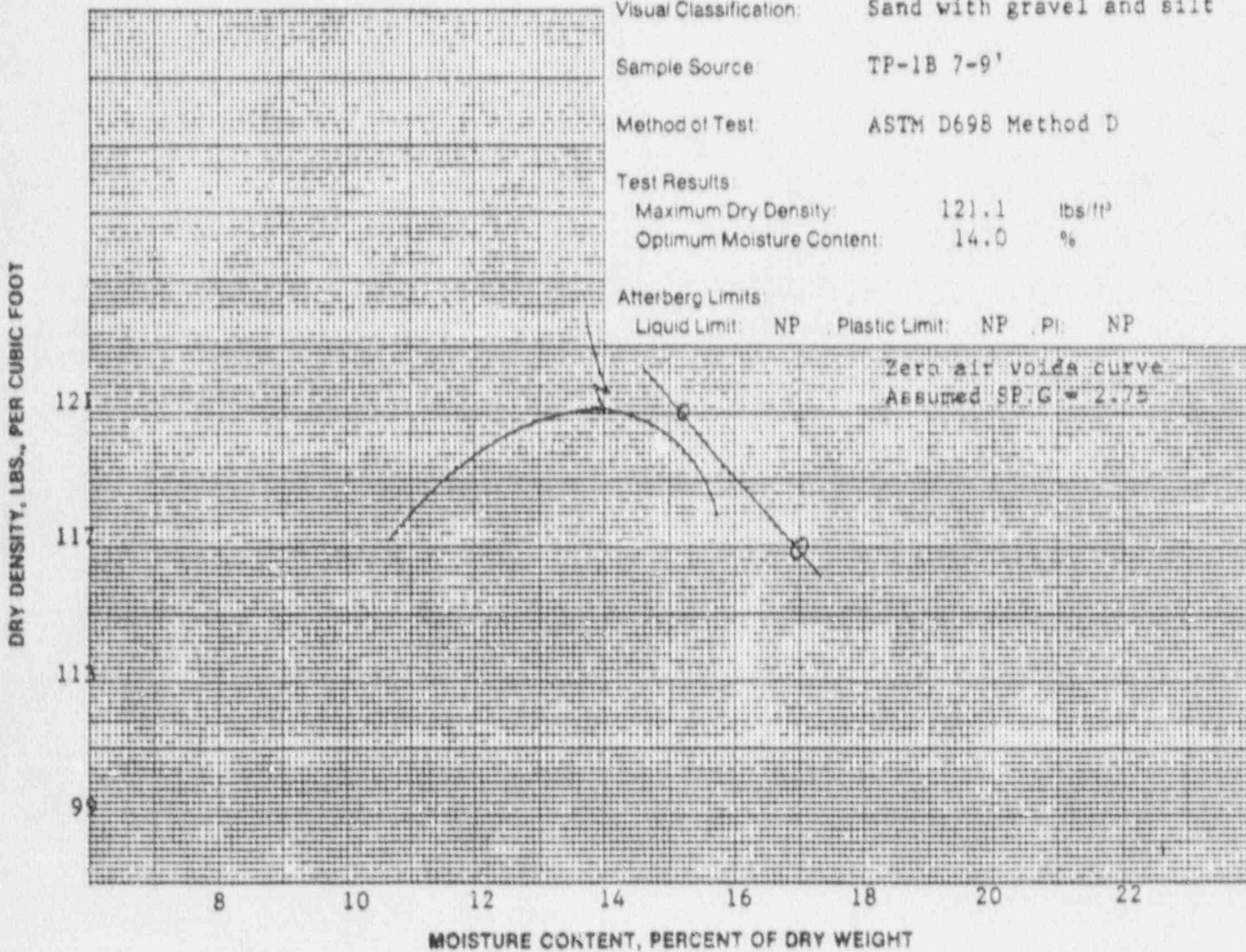
Maximum Dry Density: 121.1 lbs/ft³

Optimum Moisture Content: 14.0 %

Atterberg Limits

Liquid Limit: NP Plastic Limit: NP PI: NP

Zero air voids curve
Assumed SP.G = 2.75



SPECIFIC GRAVITY OF SOIL (ASTM D-854)

G = 2.66

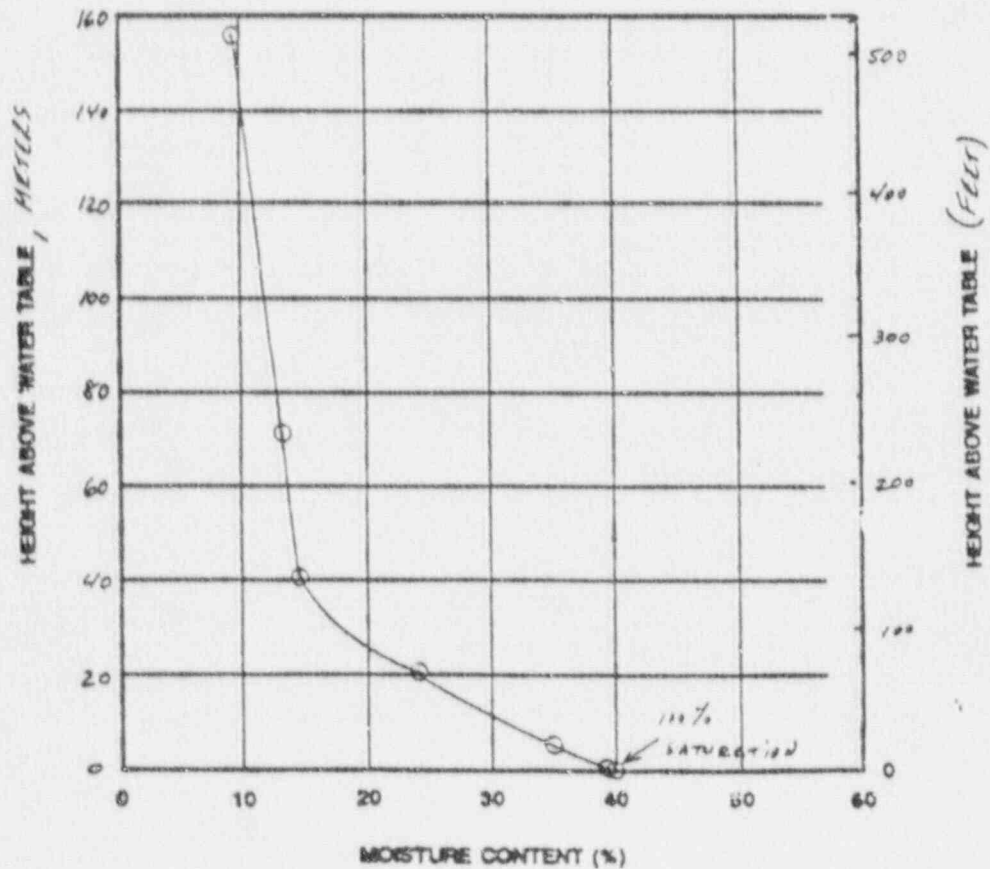
Respectfully submitted,
Professional Service Industries, Inc.

CAPILLARY - MOISTURE RELATIONS ASTM - D 3157

Location: UNKNOWN Sample No: TP-18
 Depth: 7-9' Date: START 12/3 - END 12/10
 Identification: 95% MPO of OMC Soil type: S: F-G SANDY CLAY
 Initial moisture content: 14.0% Sample type: _____
 Initial dry unit weight: 1.842 g/cm³ 115.0 lb/ft³ Porosity: _____
 Specific gravity: _____ Remarks: MPO = 12.1 (W) 14.0% OMC
A = 3.142 in
IT = 0.40 in

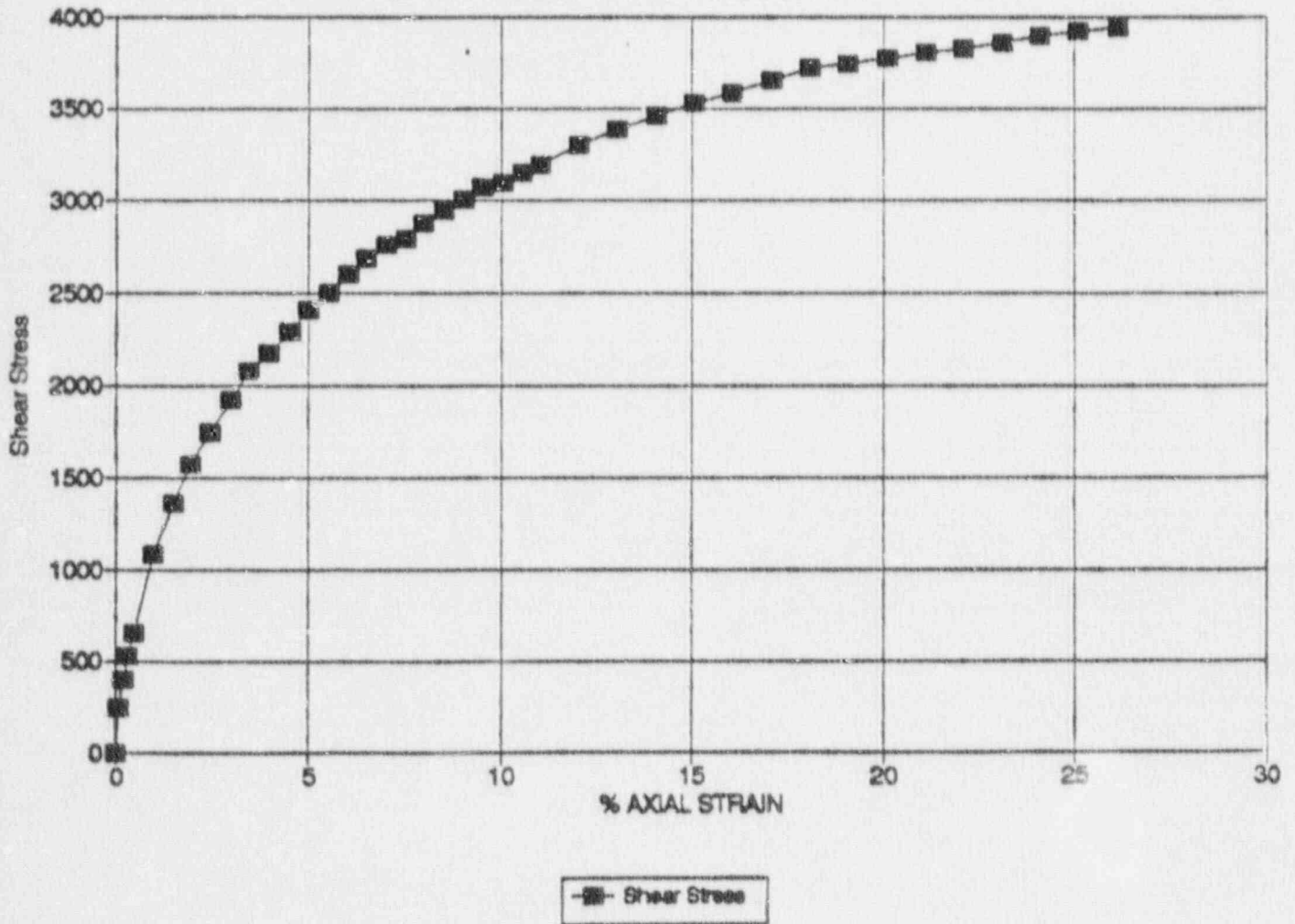
(1) Tension, <u>BAR</u>	*	**	0.1	0.5	2.0	4.0	7.0	15.0
(2) Container number <u>METRES</u>	-	-	1	5.2	20.7	41.3	72	155
(3) Wt of container and ring + wet sample, g (W _{wet})	477	505	50.4	49.5	47.3	45.3	45.0	44.2
(4) Wt of container and ring + dry sample, g (W _{dry})	42.3	=	=	=	=	=	=	=
(5) Wt of moisture, g (W _w), (3 - 4)	54	8.2	8.1	7.2	5.0	3.0	2.7	1.9
(6) Wt of container and ring g (W _c)	4.3	=	=	=	=	=	=	=
(7) Wt of dry sample, g (W _s), (4 - 6)	37.9	=	=	=	=	=	=	=
(8) Moisture content, % (w) (5 + 7) x 100	14.2	21.6	21.4	18.9	13.2	7.9	7.1	5.0
(9) Unit wt of dry sample (γ _s)	114.6	=	=	=	=	=	=	=
(10) Moisture content, volume percent (w _v) (8 x 9)	26.2	39.8	39.4	34.8	24.3	14.6	12.1	9.2

* INITIAL
 ** 24 HR SOAK



FILE ORIGINAL BY JLO DATE 9/28/59 CHECKED BY ESK DATE 12/10

TRIAxIAL UNCONSOLIDATED UNDRAINED-Sat
Boring TB-1 Sample Remold 7-9 feet



Triaxial UnConsolidated Undrained - Saturated

Height I 4.971 Boring TP-1B
 Area I 4.449 REMOLD 95 % MDD at OMC
 7-9 feet

Confining Pressure 504

LOAD RANGE 0.000 %

	Initial	Final
Percent Moisture	14.0	15.6
Wet Density pcf	131.1	137.7
Dry Density pcf	115.0	119.1

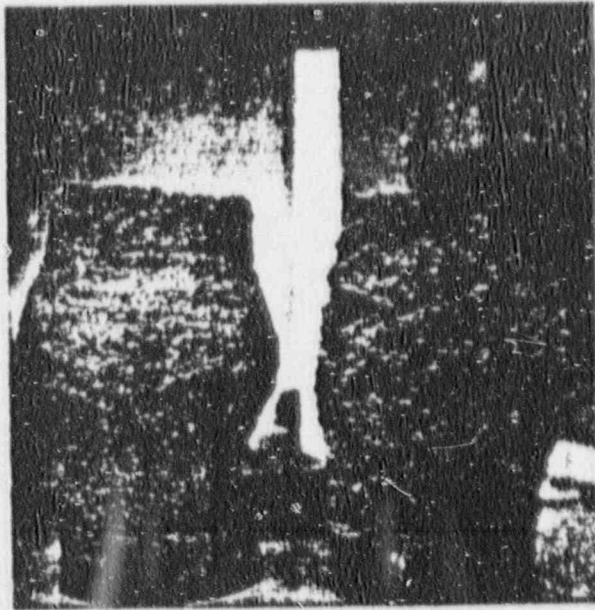
% LOAD	AXIAL DEFLECT HEIGHT		E %	S1-S3 psf	S1-S3/2 Psf
	dial in.	change in.			
0.0	0.000	0.000	0.00	0	0
490	0.005	0.005	0.10	490	245
790	0.010	0.010	0.20	788	394
1050	0.015	0.015	0.30	1047	523
1300	0.025	0.025	0.50	1293	647
2180	0.050	0.050	1.01	2158	1079
2750	0.075	0.075	1.51	2709	1354
3210	0.100	0.100	2.01	3145	1573
3580	0.125	0.125	2.51	3490	1745
3960	0.150	0.150	3.02	3841	1920
4300	0.175	0.175	3.52	4149	2074
4530	0.200	0.200	4.02	4348	2174
4800	0.225	0.225	4.53	4583	2291
5070	0.250	0.250	5.03	4815	2408
5290	0.275	0.275	5.53	4997	2499
5530	0.300	0.300	6.04	5196	2598
5730	0.325	0.325	6.54	5355	2678
5930	0.350	0.350	7.04	5512	2756
6040	0.375	0.375	7.54	5584	2792
6250	0.400	0.400	8.05	5747	2874
6440	0.425	0.425	8.55	5889	2945
6600	0.450	0.450	9.05	6003	3001
6770	0.475	0.475	9.56	6123	3062
6890	0.500	0.500	10.06	6197	3098
7040	0.525	0.525	10.56	6296	3148
7170	0.550	0.550	11.06	6377	3188
7500	0.600	0.600	12.07	6595	3297
7770	0.650	0.650	13.08	6754	3377
8040	0.700	0.700	14.08	6908	3454
8310	0.750	0.750	15.09	7056	3528
8540	0.800	0.800	16.09	7166	3583
8800	0.850	0.850	17.10	7295	3648
9080	0.900	0.900	18.11	7436	3718
9250	0.950	0.950	19.11	7482	3741

9440	1.000	1.000	20.12	7541	3770
9650	1.050	1.050	21.12	7612	3806
9830	1.100	1.100	22.13	7655	3827
10040	1.150	1.150	23.13	7717	3859
10270	1.200	1.200	24.14	7791	3895
10470	1.250	1.250	25.15	7837	3919
10670	1.300	1.300	26.15	7880	3940

WU'S 47
 TRIAXIAL TEST DATA SHEET
 LOW PRESSURE-ENGLISH

Owner PSI
 Job # 5077
 Location VNK
 Boring # TP-1B
 Sample # Remold 95% H₂O at OMC
 Depth 7-9'

Deflecting Speed 0.10 in/Hr
 Lateral Pressure 3.5 ^{PSF} ~~504~~ _(PS)
 Saturated Field Moisture
 Set-Up 11/28/90 Tested FEL (162 Office)
 Soil Type OR si F-C SANDY CLAY
H₂O = 121.1 at 14.0%

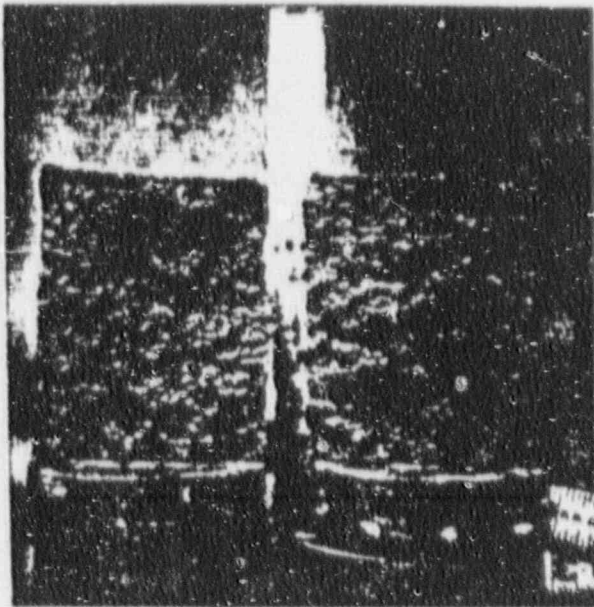


	Initial	Final
Weight soil & dish no.		
Dry weight soil & dish		
Net loss of moisture		
Weight of dish only		
Net weight of dry soil		
Moisture, % of dry weight	<u>14.0</u>	<u>15.6</u>
.....		
Wt. solids + moisture	<u>W₀ 789.1</u>	<u>799.9</u> gms.
W ₀ + 454	<u>W₀</u>	<u></u> lbs.
Weight solids	<u>W_s 692.2</u>	<u></u> gms.
Wet density W ₀ ' - V ₀ '	<u>131.1</u>	<u>137.7</u> pcf
Dry density	<u>115.0</u>	<u>119.1</u> pcf
.....		
Net diameter	<u>D₀ 2.416</u>	<u></u> in.
Area (0.785 D ₀ ²)	<u>A₀ 4582</u>	<u>4449</u> sq. in.
Height	<u>H₀ 5.00</u>	<u>4.971</u> in.
Volume (A ₀ H ₀) = 1728	<u>V₀</u>	<u></u> cu. ft.
Volume (A ₀ H ₀) x 16.4	<u>V₀ 375.73</u>	<u>362.70</u> cc
Specific gravity of solids	<u>G_s</u>	<u></u>
Volume of solids W _s ÷ G _s	<u>V_s</u>	<u></u> cc
(V ₀ - V _s) ÷ V _s	<u>e_i</u>	<u></u>
I _v burette reading	<u></u>	<u></u> cc
Burette reading under pressure	<u></u>	<u></u> cc
(V _p - V _s) - V _s	<u>e_p</u>	<u></u>

BACK - PRESSURE
PERMEABILITY TEST DATA

Owner PSI
 Job # 5461-025-2077
 Location UNK
 Boring # TP-1B
 Sample # 96% MOO REMOLD
 Depth 7-9'

Deflecting Speed _____ $\frac{\text{in}}{\text{hr}}$
 Lateral Pressure 3.5 $\frac{\text{PSF}}{\text{Min}}$
 Saturated Field Moisture
 Set-Up SPC Tested 12/1/90 (1620 Office)
 Soil Type SC SILTY F-C SANDY CLAY



	Initial	Final
Weight soil & dish no		
Dry weight soil & dish		
Net loss of moisture		
Weight of dish only		
Net weight of dry soil		
Moisture, % of dry weight	<u>14.0</u>	<u>14.2</u>
.....		
Wt. solids + moisture	<u>473.5</u>	<u>474.5</u> gms
$W_0 - 454$		<u> </u> lbs
Weight solids	<u>415.4</u>	<u> </u> gms
Wet density W_0' / V_0'	<u>131.1</u>	<u>140.0</u> pct
Dry density	<u>115.0</u>	<u>122.6</u> pct
.....		
Net diameter	<u>2.416</u>	<u> </u> in.
Area $(0.785 D_0^2)$	<u>4.582</u>	<u>4.308</u> sq in
Height	<u>3.00</u>	<u>2.994</u> in.
Volume $(A_0 H_0) - 1728$		<u> </u> cu. ft
Volume $(A_0 H_0) \times 16.4$	<u>225.44</u>	<u>211.54</u> cc
Specific gravity of solids	<u> </u>	<u> </u>
Volume of solids W_s / G_s	<u> </u>	<u> </u> cc
$(V_0 - V_s) - V_s$	<u> </u>	<u> </u>
Initial burette reading	<u> </u>	<u> </u> cc
Burette reading under pressure	<u> </u>	<u> </u> cc
$(V_0 - V_s) - V_s$	<u> </u>	<u> </u>

$$K_{AV} = \underline{4.54 \times 10^{-6} \text{ cm/s}}$$

PERMEABILITY TEST BY BACK PRESSURE CONSTANT-HEAD

Professional Service Industries 5461-025-5077

Sample Number TP-1B

Depth 7-9 feet Remold 95% Mdd at emc

		Initial	Final
Wet Density	pcf	131.1	140.0
Dry density	pcf	115.0	122.6
% Moisture		14.0	14.2

Height Initial	3.000	573.5	Wet soil and dish
Diameter Initial	2.416	515.4	Dry soil and dish
Area Initial	4.582		100 dish only
Volume Initial	225.44	473.5	Ws Initial
Initial dial	1.084	474.5	Final Ws
Final dial	1.09	415.4	Weight solids
Initial cc/in res	0		
Final cc/in res.	23.1		

Height Final	2.994	7.605	cm
Diameter Final	2.343		
Area Final	4.308	27.817	cm ²
Volume Final	211.54		

Height change	-0.006		
cc/in reser.	0.008		
Volume change	-23.100		
Cell Change	9.2		
Net Volume Change	-13.900		
h= T/B PRESS. diff	9	630.90	cm

Standard Water .005 N CaSO4

Hydraulic Gradient
82.96

Elapsed
Time
minutes

cc's

K
cm/sec

17.0	11.60	4.59E-06
4.0	2.70	4.54E-06
10.0	6.80	4.57E-06
13.0	8.60	4.45E-06

K Average = 4.54E-06 cm/s



Professional Service Industries, Inc.

TESTED FOR:
MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT:
LOWMAN, IDAHO

Attention: Mr. John Singleton

December 21, 1990

PSI Project #:355-00118-001

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REMARKS

TP-2 @ Depth 3'-5'

I. Particle Size Analysis with Hydrometer (ASTM D-422).

Table with 2 columns: Sieve Size and Percent Passing. Lists sieve sizes from 2.0 inch down to 250 microns and corresponding percent passing values.

II. Atterberg Limits (ASTM D-4318)

Table with 2 columns: Test Name and Value. Lists Liquid Limit (25), Plastic Limit (19), and Plastic Index (6).

III. Natural Moisture Content (ASTM D-2216)

Table with 2 columns: Test Name and Value. Lists Moisture % (11.1).

Respectfully submitted;

PROFESSIONAL SERVICE INDUSTRIES, INC.



Professional Service Industries, Inc.

REPORT OF MOISTURE DENSITY RELATIONSHIP OF SOIL

TESTED FOR: MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT: LOWMAN, IDAHO

Attention: Mr. John Singleton

DATE: December 21, 1990

OUR REPORT NO: 355-00118-001 Pg. 20 of 46

TEST DATA

Visual Classification: Sand with silt and gravel

Sample Source: TP-2 3-5'

Method of Test: ASTM D698 Method D

Test Results:

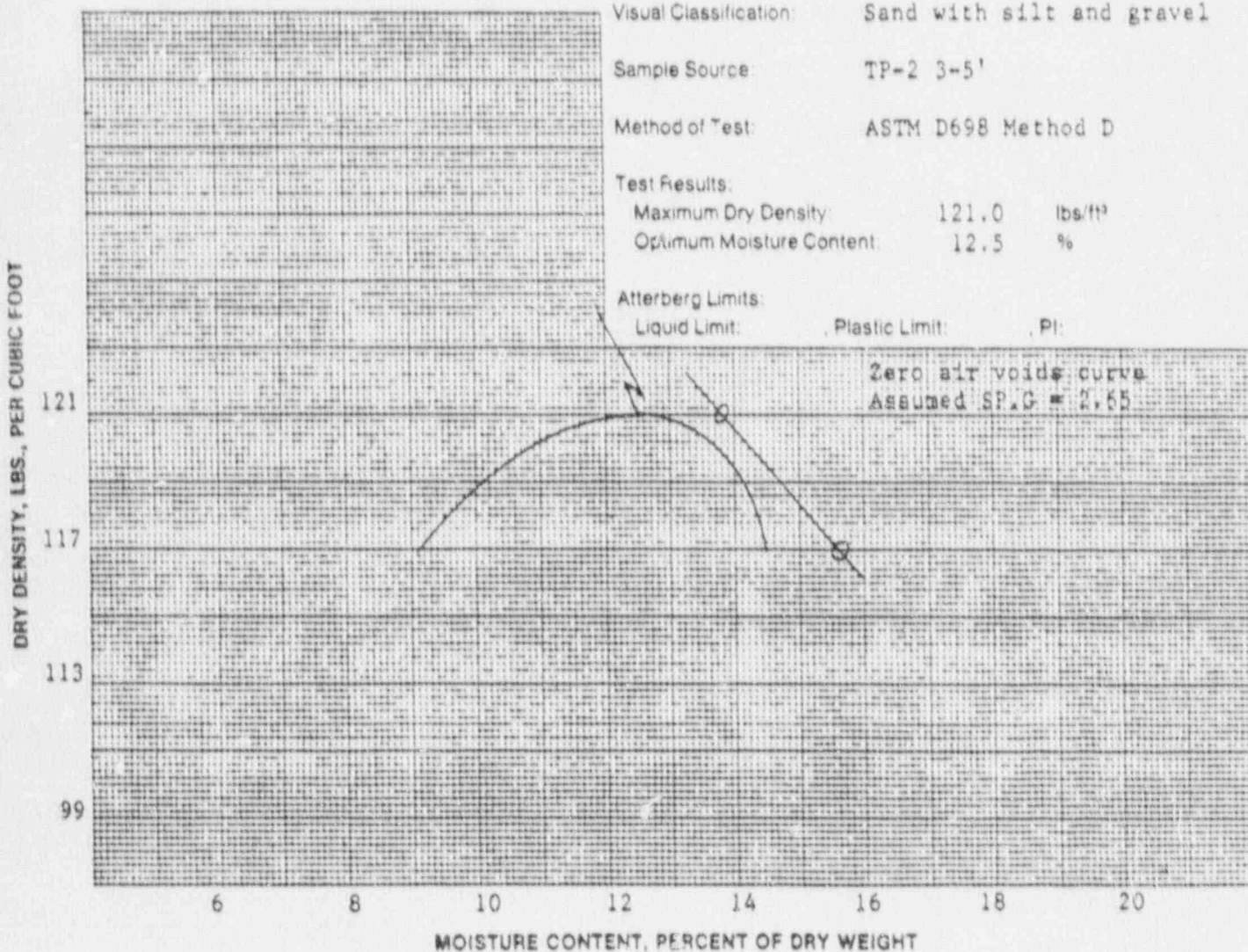
Maximum Dry Density: 121.0 lbs/ft³

Optimum Moisture Content: 12.5 %

Atterberg Limits:

Liquid Limit: Plastic Limit: PI:

Zero air voids curve
Assumed $SP.G = 2.65$



SPECIFIC GRAVITY OF SOIL (ASTM D-854)

G = 2.64

Respectfully submitted,
Professional Service Industries, Inc.


Professional Service Industries, Inc.
ANALYTICAL REPORT

Tested For: PROFESSIONAL SERVICE
INDUSTRIES, INC.
254 S. Cole Road
Boise, Idaho 83709

Project: Chemical Analysis
Lab # 12093 and 12094

ATTN: Bob Arnold

Date: December 11, 1990

Report #214-03154-01

Date Received: December 5, 1990

Sample Identification: Two soil samples labeled below.

Methodology Employed: EPA Methods for Chemical Analysis of Water
and Waste

Analyst: CM 12/11/90, 1233

	<u>Results</u>	<u>Performed by</u>
Sample ID: TP2; lab #12093 Walkley Black TOC, %	0.32	CM 12/11/90, 1223
Sample ID: TP3; lab #12094 Walkley Black TOC, %	0.29	CM 12/11/90, 1223

Q.A. DATA

	<u>Mdl</u>	<u>Blank</u>	<u>Orig</u>	<u>Dup</u>	<u>Rpd %</u>	<u>Amount Spiked</u>	<u>Spike Rec. %</u>
Walkley Black TOC	-	--	0.32	0.23	20.7	--	---

DATA REVIEW AND CERTIFICATION

The information given in this analytical report has been reviewed and is certified to have been prepared according to the methods cited herein.

Anita Abbott

12/11/90

Respectfully submitted,
PROFESSIONAL SERVICE INDUSTRIES, INC.

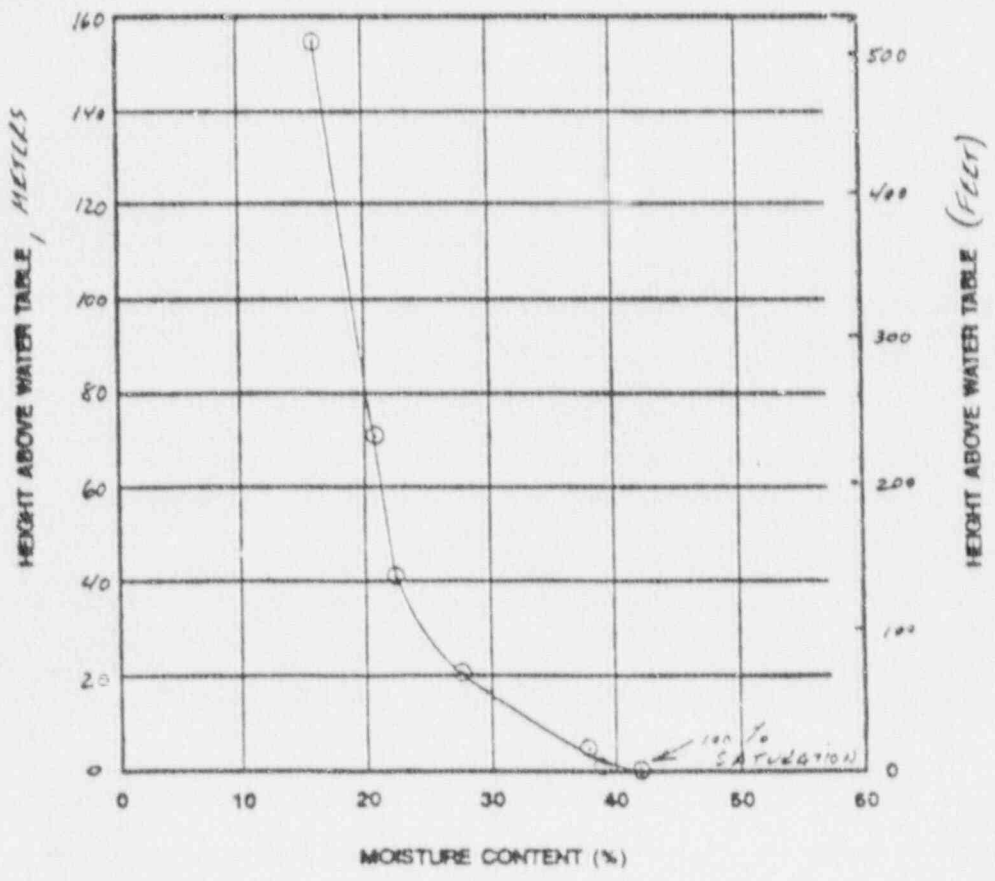
CAPILLARY - MOISTURE RELATIONS

ASTM - D 3152

Location: UNKNOWN Sample No: TP-2
 Depth: 3-5' Date: START 12/3 - END 12/10
 Identification: 95% MOO at PUC Soil type: SANDY GRAY SILT
 Initial moisture content: _____ % Sample type: BULK
 Initial dry unit weight: 1.841 g/cm³ 114.9 lb/ft³ Porosity: _____ %
 Specific gravity: _____ Remarks: MOO = 121.0 at 12.5%
A = 3.142 cm²
H = 0.40 cm

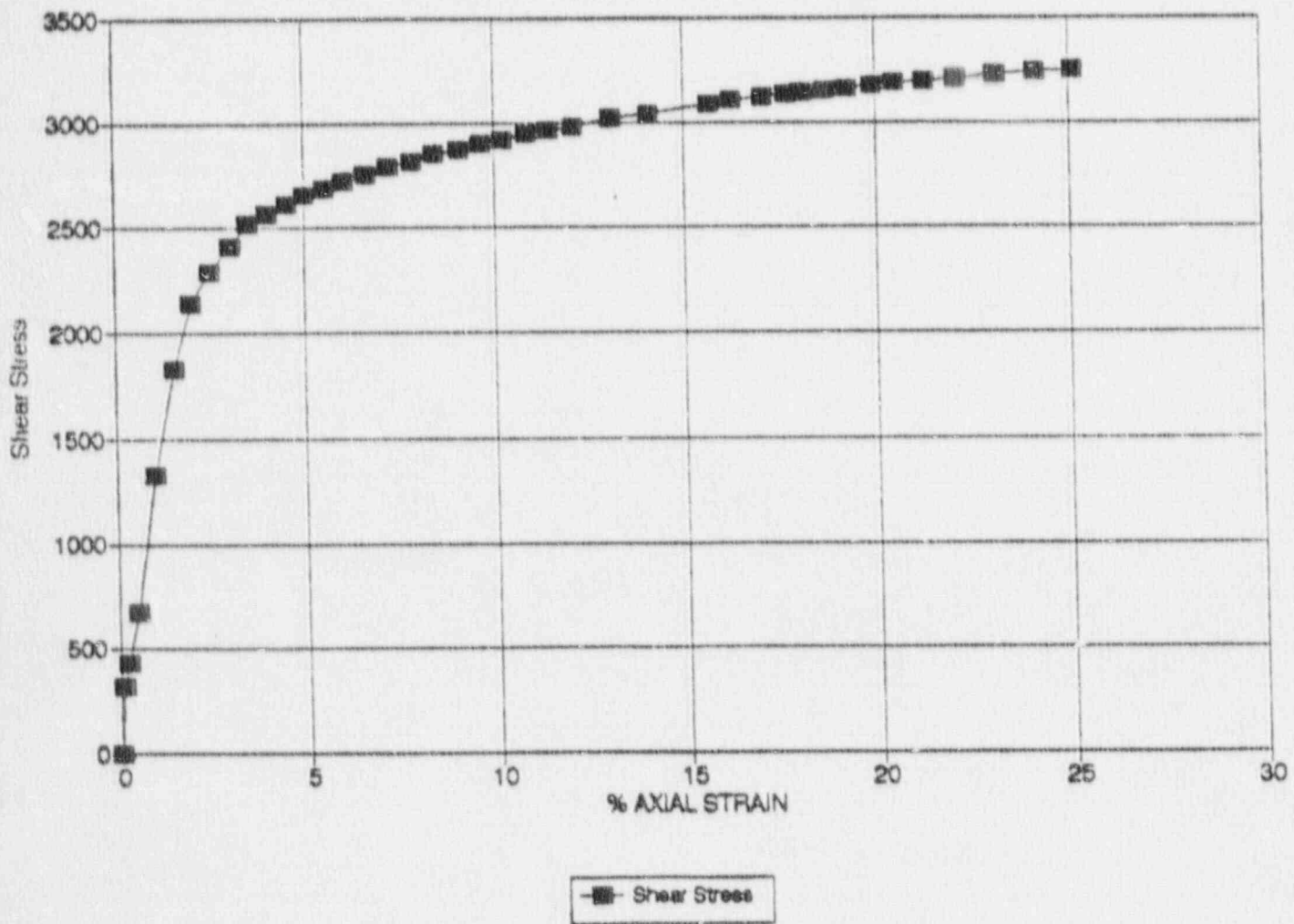
(1) Tension, <u>BAR</u>	*	**	0.1	0.5	2.0	4.0	7.0	15.0
(2) Container number <u>NETLES</u>	-	-	1	5.2	20.7	41.3	72	155
(3) Wt of container and ring + wet sample, g (W _w)	46.7	50.5	49.9	49.7	47.6	46.5	46.2	45.2
(4) Wt of container and ring + dry sample, g (W _s)	41.9	"	"	"	"	"	"	"
(5) Wt of moisture, g (W _w), (3 - 4)	4.8	8.6	8.0	7.8	5.7	4.6	4.3	3.3
(6) Wt of container and ring g (W _c)	4.0	"	"	"	"	"	"	"
(7) Wt of dry sample, g (W _d) (4 - 6)	37.9	"	"	"	"	"	"	"
(8) Moisture content, % (w) (5 + 7) x 100	12.7	12.7	21.1	20.6	15.0	12.1	11.3	9.7
(9) Unit wt of dry sample (γ _s)	1.84	"	"	"	"	"	"	"
(10) Moisture content, volume percent (w _v) (8 x 9)	23.4	41.8	38.8	37.9	27.6	22.3	20.8	16.0

* INITIAL
 ** 24 HR SOAK



DATE: 12/10
 TIME: 09:00
 BY: B
 SHEET: 23/46

TRIAXIAL UNCONSOLIDATED UNDRAINED-Sat
Boring TP-2 3-5 feet Remold 95% MDD



Triaxial UnConsolidated Undrained - Saturated

Height 14.9745 Boring TP-2
 Area I 4.553 REMOLD 95 % MDD at OMC
 3-5 feet

Confining Pressure 504

LOAD RANGE 0.000 %

	Initial	Final
Percent Moisture	12.5	16.5
Wet Density pcf	129.3	135.5
Dry Density pcf	114.9	116.3

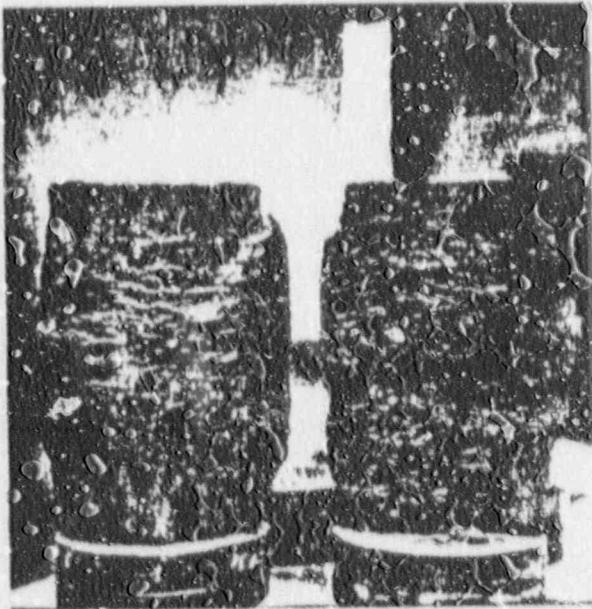
% LOAD	AXIAL DEFLECT HEIGHT		E %	S1-S3 psf	S1-S3/2 Psf
	dial in.	change in.			
0.0	0.000	0.000	0.00	0	0
640	0.005	0.005	0.10	639	320
870	0.010	0.010	0.20	868	434
1360	0.025	0.025	0.50	1353	677
2680	0.050	0.050	1.01	2653	1327
3710	0.075	0.075	1.51	3654	1827
4370	0.100	0.100	2.01	4282	2141
4700	0.125	0.125	2.51	4582	2291
4980	0.150	0.150	3.02	4830	2415
5230	0.175	0.175	3.52	5046	2523
5350	0.200	0.200	4.02	5135	2567
5480	0.225	0.225	4.52	5232	2616
5590	0.250	0.250	5.03	5309	2655
5690	0.275	0.275	5.53	5375	2688
5790	0.300	0.300	6.03	5441	2720
5910	0.330	0.330	6.63	5518	2759
6010	0.360	0.360	7.24	5575	2788
6110	0.390	0.390	7.84	5631	2815
6220	0.420	0.420	8.44	5695	2847
6310	0.450	0.450	9.05	5739	2870
6410	0.480	0.480	9.65	5791	2896
6500	0.510	0.510	10.25	5834	2917
6610	0.540	0.540	10.86	5892	2946
6700	0.570	0.570	11.46	5932	2966
6780	0.600	0.600	12.06	5962	2981
6940	0.650	0.650	13.07	6033	3017
7080	0.700	0.700	14.07	6084	3042
7320	0.780	0.780	15.68	6172	3086
7420	0.810	0.810	16.28	6212	3106
7520	0.850	0.850	17.09	6235	3118
7610	0.880	0.880	17.69	6264	3132
7660	0.900	0.900	18.09	6274	3137
7750	0.930	0.930	18.70	6301	3151
7830	0.960	0.960	19.30	6319	3159

7920	0.990	0.990	19.90	6344	3172
8020	1.020	1.020	20.50	6376	3188
8110	1.060	1.060	21.31	6382	3191
8240	1.100	1.100	22.11	6418	3209
8390	1.150	1.150	23.12	6450	3225
8540	1.200	1.200	24.12	6480	3240
8680	1.250	1.250	25.13	6499	3249

DU/CAI
TRIAxIAL TEST DATA SHEET
LOW PRESSURE-ENGLISH

Owner PSI
 Job # 5461-025-5277
 Location UNK
 Boring # TP-2
 Sample # REHOLD 95% H100
 Depth 3-5'

Deflecting Speed 0.10 N/Hi Min
 Lateral Pressure 3.5 PSF-504 PSD
 Saturated Field Moisture
 Set-Up 12/2/90 Tested FBT (1620 Hicc)
 Soil Type BR F-10 SANDY CLAYEY SILT



	Initial	Final
Weight soil & dish no.		
Dry weight soil & dish		
Net loss of moisture		
Weight of dish only		
Net weight of dry soil		
Moisture, % of dry weight	<u>12.5</u>	<u>16.5</u>

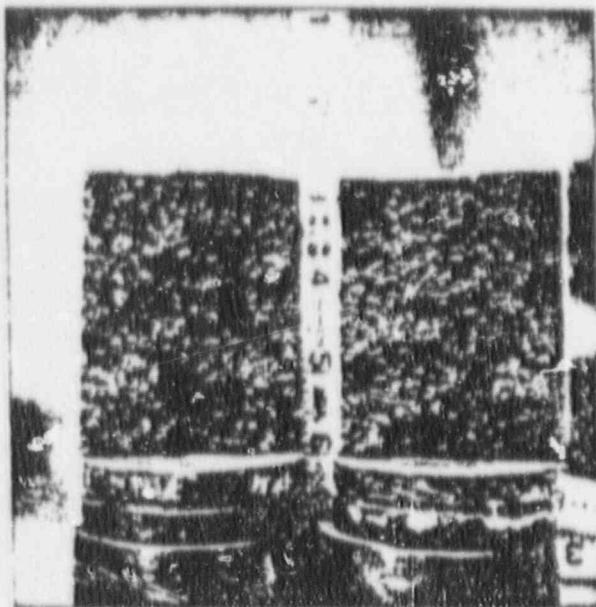
Wt. solids + moisture	<u>729.3</u>	<u>806.1</u>	gms.
$W_0 = 454$			lbs.
Weight solids	<u>691.5</u>		gms
Wet density $W_0' - V_0'$	<u>129.3</u>	<u>125.5</u>	pcf
Dry density	<u>114.9</u>	<u>114.3</u>	pcf

Net diameter	<u>2.416</u>		in.
Area ($0.785 D_0^2$)	<u>4.562</u>	<u>4.563</u>	sq. in
Height	<u>5.00</u>	<u>4.975</u>	in.
Volume ($A_0 H_0$) = 1728			cu. in.
Volume ($A_0 H_0$) x 16.4	<u>27503</u>	<u>27143</u>	cc
Specific gravity of solids	<u>G_s</u>		
Volume of solids $W_s - G_s$	<u>V_s</u>		cc
$(V_0 - V_s) = V_v$	<u>e_i</u>		
Initial burette reading			cc
Burette reading under pressure			cc
$(V_p - V_v) = V_s$	<u>e_p</u>		

PERMEABILITY TEST DATA

Owner PEI
 Job # 5461-025-5077
 Location UNKNOWN
 Boring # TP-2
 Sample # REMOLO - 95% H₂O at 2.12
 Depth 3.5'

Deflecting Speed 5 in./hr
 Lateral Pressure 3.5 PSF
 Saturated Field Moisture
 Set-Up 12/1/90 Tested PEI (165 Office)
 Soil Type RF-C SANDY CLAYEY SILT
H₂O = 121.0 at 13.5'



	Initial	Final
Weight soil & dish no.		
Dry weight soil & dish		
Net loss of moisture		
Weight of dish only		
Net weight of dry soil		
Moisture, % of dry weight	125	121
.....		
Wt. solids + moisture	W ₀ 466.9	455.9 gms
W ₀ - 454	W ₀	lbs.
Weight solids	W _s 415.0	gms.
Wet density W ₀ ' - V ₀ '	129.3	121.7 pcf
Dry density	114.5	112.5 pcf
.....		
Net diameter	D ₀ 2.416	in.
Area (0.785 D ₀ ²)	A ₀ 4.582	4.697 sq. in.
Height	H ₀ 3.00	2.989 in.
Volume (A ₀ H ₀) - 1728	V ₀ ' 225.44	230.26 cu. in.
Volume (A ₀ H ₀) x 16.4	V _e	cc
Specific gravity of solids	G _s	
Volume of solids W _s - G _s	V _s	cc
(V ₀ ' - V _s) - V _s	e _i	
Initial burette reading		cc
Burette reading under pressure		cc
(V ₀ ' - V _s) - V _s	e _p	

K₁₁ = 173 x 10⁻⁶ in./s

PERMEABILITY TEST BY BACK PRESSURE
CONSTANT HEAD (Pbp)

PROJECT: P. S. I NO.: 5461-025-5077 LOCATION: UNK.

Boring No.: TP-2 Sample No.: PC-012 Depth: 3-5' (ft.)(m.) Tested by: PH 121 5 1.90
95%

DATE	TIME	ELAPSED TIME (MIN)	CHAMBER PRESSURE (PSI)	h_1 BACK PRESS 1 (PSI)	h_2 BACK PRESS 2 (PSI)	EXTERNAL BURETTE OR DIAL RDG. (CC)/(IN.)	INTERNAL BURETTE (CC)	Q (cc)	ΔQ (cc)
12/5/90	0606	0	72.5	65	60	323	21.3		
	0638					327	9.7		
	0731					336	+0.0		
	0738	0	72.5	69	60	336	23.7		
	0755						19.2		
	0810						16.3		
	0841						10.6		
	0847						9.2		
	0912						+0.0		
	0923	0	72.5	69	60	337	22.9		
	0926						21.9		
	1025	59					4.8		
	1030	5					3.4		
	1035	5					2.1		
	1056	0	72.5	69	60	337	23.0		
	1154	58					6.0		

$$\Delta p = 9 \text{ psi}$$

$$\bar{\sigma}_3 = 35 \text{ psi}$$

$$K_{20^\circ C} = \frac{QL}{\tau h A} \times \frac{U_r}{U_{r \cdot C}}$$

$$Q = \text{cc}$$

$$h = (70.1) = \text{cm}$$

$$L = \text{cm}$$

$$U_r = .931 (23^\circ C)$$

$$\tau = \text{min. (60)} = \text{sec}$$

$$A = \text{cm}^2$$

$$K_{20^\circ C} = \text{---} \times .931 =$$

SEE TOP 4.

PERMEABILITY TEST BY BACK PRESSURE CONSTANT-HEAD

Professional Service Industries 5461-025-5077

Sample Number TP-2

Depth 3-5 feet Remold 95% Mdd at omc

	Initial	Final
Wet Density pcf	119.3	131.7
Dry density pcf	114.9	112.5
% Moisture	12.5	17.1

Height Initial	3.000	566.9 Wet soil and dish
Diameter Initial	2.416	515 Dry soil and dish
Area Initial	4.582	100 dish only
Volume Initial	225.44	466.9 Ws Initial
Initial dial	0.227	485.9 Final Ws
Final dial	0.238	415.0 Weight solids
Initial cc/in res	-0.396	
Final cc/in res.	-0.337	

Height Final	2.989	7.592 cm
Diameter Final	2.446	
Area Final	4.697	30.330 cm ²
Volume Final	230.26	

Height change	-0.011	
cc/in reser.	0.008	
Volume change	-7.375	
Cell Change	12.2	
Net Volume Change	4.825	
h = T/B PRESS. diff	9	630.90 cm

Standard Water .005 N CaSO₄Hydraulic Gradient
83.10Elapsed
Time
minutes

cc's

K
cm/sec

59.0	17.10	1.78E-06
5.0	1.40	1.72E-06
5.0	1.30	1.60E-06
58.0	17.00	1.80E-06

K Average = 1.73E-06 cm/s



Professional Service Industries, Inc.

TESTED FOR:
MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT:
LOWMAN, IDAHO

Attention: Mr. John Singleton

December 21, 1990

PSI Project #:355-00118-001

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REMARKS

TP-3 @ Depth 5'-7'

I. Particle Size Analysis with Hydrometer (ASTM D-422).

Sieve Size	Percent Passing
2.0"	100
1.5"	93
1.0"	89
3/4"	85
1/2"	85
3/8"	82
#4	68
#8	66
#16	64
#30	53
#50	42
#100	35
#200	29
1 Min.	26
2 Min.	20
5 Min.	18
15 Min.	15
30 Min.	13
60 Min.	11
250 Min.	8

II. Atterberg Limits (ASTM D-4318)

Liquid Limit	NP
Plastic Limit	NP
Plastic Index	NP

III. Natural Moisture Content (ASTM D-2216)

Moisture %	9.4
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Respectfully submitted;
PROFESSIONAL SERVICE INDUSTRIES, INC.



Professional Service Industries, Inc.

REPORT OF MOISTURE DENSITY RELATIONSHIP OF SOIL

TESTED FOR: MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT: LOWMAN, IDAHO

Attention: Mr. John Singleton

DATE: December 21, 1990

OUR REPORT NO: 355-00118-001 Pg. 33 of 46

TEST DATA

Visual Classification: Sand with gravel and silt

Sample Source: TP-3 5-7'

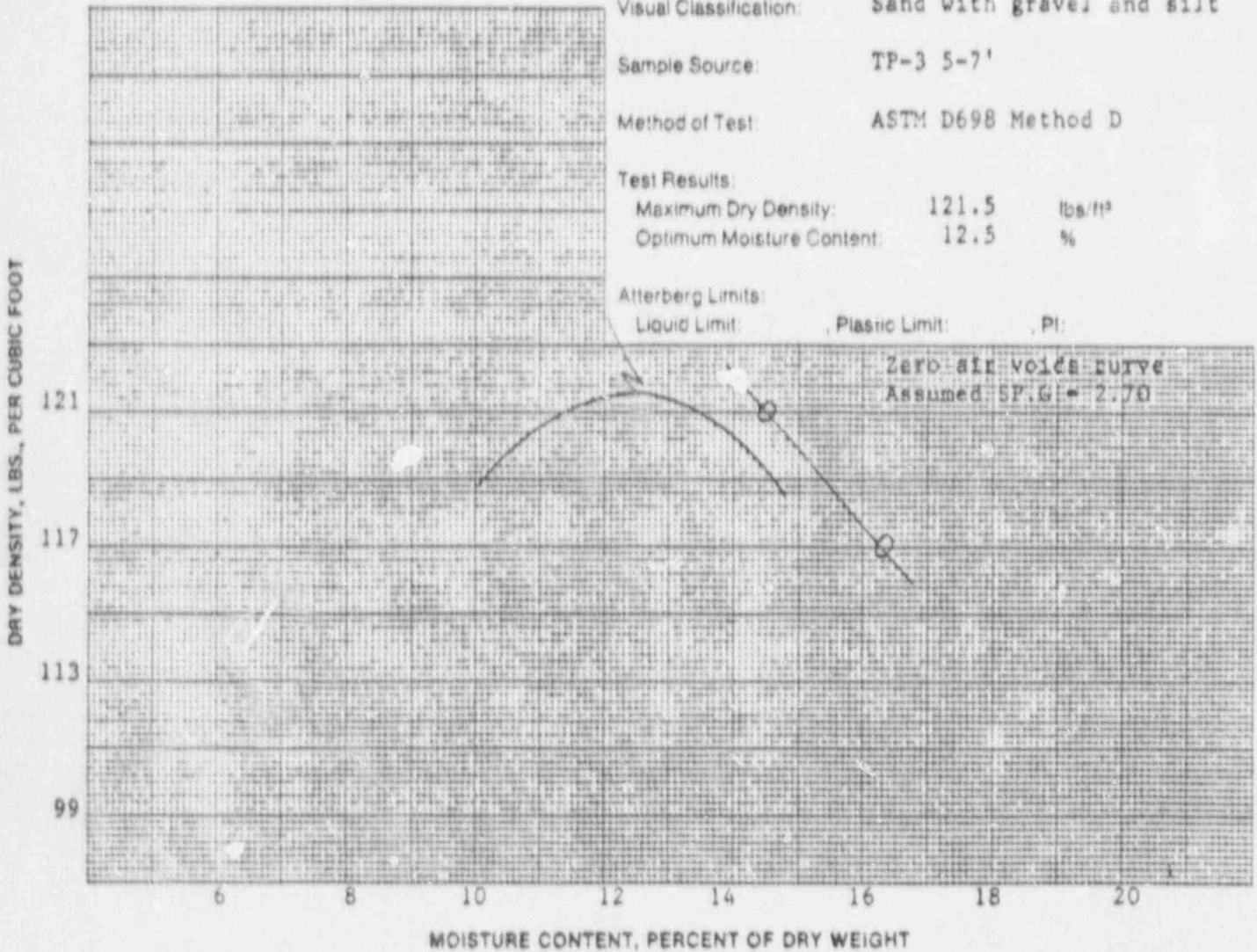
Method of Test: ASTM D698 Method D

Test Results:

Maximum Dry Density: 121.5 lbs/ft³
Optimum Moisture Content: 12.5 %

Atterberg Limits:

Liquid Limit: Plastic Limit: PI:



Respectfully submitted,
Professional Service Industries, Inc.

CAPILLARY - MOISTURE RELATIONS

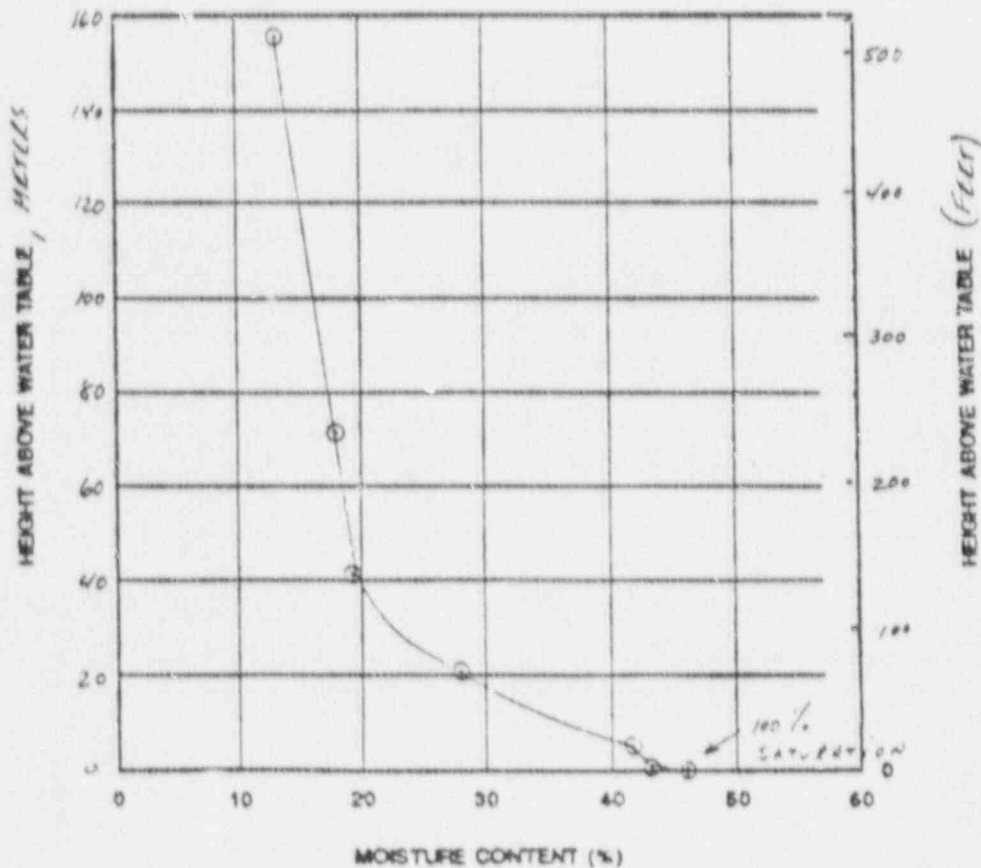
ASTM - D 9152

Location: UNKNOWN Sample No: TP-1
 Depth: 5-7' Date: START 12/10 - END 12/10
 Identification: 95% MDD at DMC Soil type: CLAYEY C SANDY SILT
 Initial moisture content: 12.5 % Sample type: _____
 Initial dry unit weight: _____ g/cm³: 1.875 lb/ft³: Porosity: _____ %
 Specific gravity: _____ Remarks: MDD = 121.5 at 12.5%
A = 2.142 in²
H = 0.4 in

(1) Tension, <u>BAR</u>	*	**	0.1	0.5	2.0	4.0	7.0	15.0
(2) Container number <u>NETLES</u>	-	-	1	5.2	20.7	41.3	72	155
(3) Wt of container and ring + wet sample, g (W _{wet})	47.1	51.8	51.2	50.8	48.0	46.3	46.0	45.0
(4) Wt of container and ring + dry sample, g (W _d)	42.3							
(5) Wt of moisture, g (W _w), (3 - 4)	4.8	4.5	8.9	8.5	5.7	4.0	3.7	2.7
(6) Wt of container and ring, g (W _c)	4.3	"	"	"	"	"	"	"
(7) Wt of dry sample, g (W _s), (4 - 6)	38.0	"	"	"	"	"	"	"
(8) Moisture content, % (w), (5 + 7) x 100	12.6	25.4	22.4	22.4	15.0	10.5	9.7	7.1
(9) Unit wt of dry sample (γ _s)	1.87							
(10) Moisture content, volume percent (w _v), (8 x 9)	2.7	46.	43.2	41.4	27.7	19.4	12.9	13.1

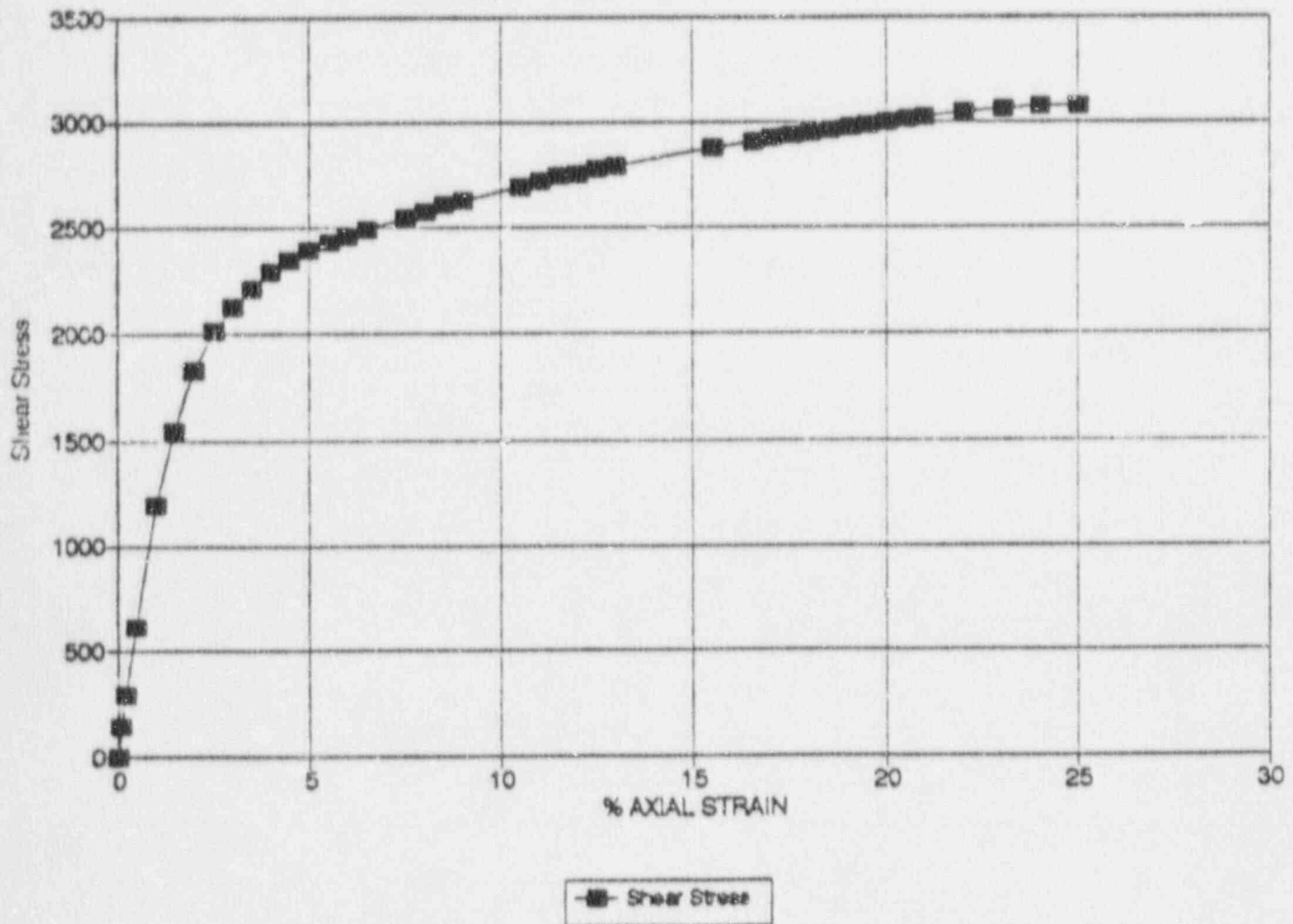
* INITIAL

** 24 HR SOAK



DATE 3-11-59 CHECKED BY RL DATE 12/10

TRIAxIAL UNCONSOLIDATED UNDRAINED-Sat
Boring TP-3 Remold 95% MDD 5-7 feet



Triaxial UnConsolidated Undrained - Saturated

Height I 4.986 Boring TP-3
 Area I 4.455 REMOLD 95 % MDD at OMC
 5-7 feet

Confining Pressure 504

LOAD RANGE 0.000 %

	Initial	Final
Percent Moisture	12.5	5.4
Wet Density pcf	129.8	137.3
Dry Density pcf	115.4	119.0

% LOAD	AXIAL DEFLECT dial in.	HEIGHT change in.	E %	S1-S3 psf	S1-S3/2 Psf
0.0	0.000	0.000	0.00	0	0
290	0.005	0.005	0.10	290	145
590	0.010	0.010	0.20	589	294
1240	0.025	0.025	0.50	1234	617
2410	0.050	0.050	1.00	2386	1193
3130	0.075	0.075	1.50	3083	1541
3730	0.100	0.100	2.01	3655	1828
4130	0.125	0.125	2.51	4026	2013
4390	0.150	0.150	3.01	4258	2129
4580	0.175	0.175	3.51	4419	2210
4770	0.200	0.200	4.01	4579	2289
4910	0.225	0.225	4.51	4688	2344
5040	0.250	0.250	5.01	4787	2394
5150	0.275	0.275	5.52	4866	2433
5240	0.300	0.300	6.02	4925	2462
5340	0.325	0.325	6.52	4992	2496
5510	0.375	0.375	7.52	5096	2548
5600	0.400	0.400	8.02	5151	2575
5700	0.425	0.425	8.52	5214	2607
5780	0.450	0.450	9.03	5258	2629
6030	0.525	0.525	10.53	5395	2698
6120	0.550	0.550	11.03	5445	2722
6200	0.575	0.575	11.53	5485	2742
6260	0.600	0.600	12.03	5507	2753
6350	0.625	0.625	12.54	5554	2777
6410	0.650	0.650	13.04	5574	2787
6810	0.775	0.775	15.54	5751	2876
6960	0.825	0.825	16.55	5808	2904
7050	0.850	0.850	17.05	5848	2924
7120	0.875	0.875	17.55	5871	2935
7180	0.900	0.900	18.05	5884	2942
7260	0.925	0.925	18.55	5913	2957
7340	0.950	0.950	19.05	5941	2971
7420	0.975	0.975	19.55	5969	2985

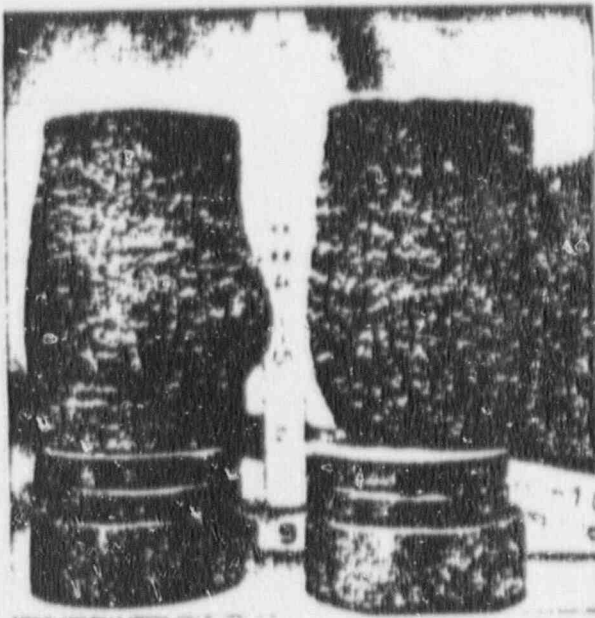
7510	1.000	1.000	20.06	6004	3002
7580	1.025	1.025	20.56	6022	3011
7650	1.050	1.050	21.06	6039	3019
7810	1.100	1.100	22.06	6087	3043
7950	1.150	1.150	23.06	6116	3058
8100	1.200	1.200	24.07	6151	3075
8200	1.250	1.250	25.07	6144	3072

Dames & Moore

**TRIAXIAL TEST DATA SHEET
LOW PRESSURE-ENGLISH**

Owner P. S. I.
 Job # 5461-025-5077
 Location UNK
 Boring # TP-3
 Sample # REMOLO 95% MOO
 Depth 5-7'

Deflecting Speed 0.10 in./Min.
 Lateral Pressure 35 PSI - 504
 Saturated Field Moisture
 Set-Up 11/27/90 Tested CEL (LAB Office)
 Soil Type RED CLAYEY F.C SANDY SILT

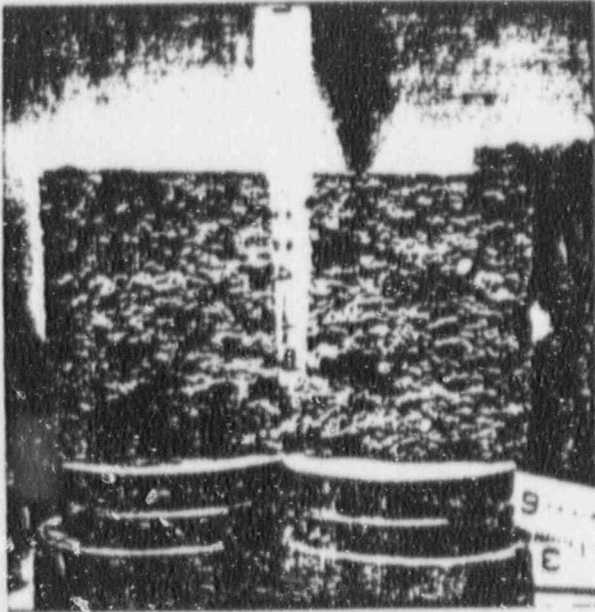


	Initial	Final	
Weight soil & dish no.			
Dry weight soil & dish			
Net loss of moisture			
Weight of dish only			
Net weight of dry soil			
Moisture, % of dry weight	<u>12.5</u>	<u>12.4</u>	
.....			
Wt. solids + moisture	<u>W₀ 281.2</u>	<u>281.6</u>	gms.
W ₀ - #54	<u>W₀</u>		lbs.
Weight solids	<u>W_s 270.5</u>		gms.
Wet density W ₀ ¹ - V ₀ ¹	<u>127.5</u>	<u>127.3</u>	pcf
Dry density	<u>112.4</u>	<u>112.0</u>	pcf
.....			
Net diameter	<u>D₀ 2.416</u>		in.
Area (0.785 D ₀ ²)	<u>A₀ 4.552</u>	<u>4.555</u>	sq. in.
Height	<u>H₀ 5.0</u>	<u>4.766</u>	in.
Volume (A ₀ H ₀) = 1728	<u>V₀</u>		cu. ft.
Volume (A ₀ H ₀) × 16.4	<u>V₀ 375.93</u>	<u>364.32</u>	cc
Specific gravity of solids	<u>G_s</u>		
Volume of solids W _s - G _s	<u>V_s</u>		cc
(V ₀ - V _s) = V ₁	<u>e₁</u>		
Initial burette reading			cc
Burette reading under pressure			cc
(V _p - V _s) = V ₂	<u>e_p</u>		

BACK - PRESSURE PERMEABILITY TEST DATA

Owner PSI
 Job # 5461-625-5077
 Location UNK.
 Boring # 1P-2
 Sample # REHOLD - 95% MOU at OMC
 Depth 5.7'

Deflecting Speed _____ 5 in/Hr
 Lateral Pressure _____ 3.5 PSI
 Saturated Field Moisture
 Set-Up 11/23/50 Tested FGC (E20 office)
 Soil Type BE CLAYEY F-C SANDY SILT



	Initial	Final
Weight soil & dish no.		
Dry weight soil & dish		
Net loss of moisture		
Weight of dish only		
Net weight of dry soil		
Moisture, % of dry weight	<u>12.5</u>	<u>14.8</u>

Wt. solids + moisture	<u>W₀ 468.3</u>	<u>478.5</u> gms.
W ₀ - 454	<u>W₀'</u>	lbs
Weight solids	<u>W_s 416.7</u>	gms.
Wet density $W_0' - V_0'$	<u>129.8</u>	<u>135.7</u> pcf
Dry density	<u>115.4</u>	<u>118.2</u> pcf

Net diameter	<u>D₀ 2.416</u>	in
Area (0.785 D ₀ ²)	<u>A₀ 4582</u>	<u>4477</u> sq. in
Height	<u>H₀ 2.77</u>	<u>2.998</u> in
Volume (A ₀ H ₀) - 1728	<u>V₀'</u>	cu. ft
Volume (A ₀ H ₀) x 16.4	<u>V₀ 22544</u>	<u>220.14</u> cc
Specific gravity of solids	<u>G_s</u>	
Volume of solids $W_s - G_s$	<u>V_s</u>	cc
(V ₀ - V _s) - V _s	<u>e_i</u>	
Initial burette reading		cc
Burette reading under pressure		cc
(V _p - V _s) - V _s	<u>e_p</u>	

$K_{AI} = 2.53 \times 10^{-6} \text{ cm/s}$

PERMEABILITY TEST BY BACK PRESSURE
CONSTANT HEAD (Pbp)

PROJECT: P. S. 5 NO.: 5097 LOCATION: UNE

Boring No.: TP-3 Sample No.: Remaind Depth: 5.7' (ft.)(m.) Tested by: EV 11 25 74 70
95%

DATE	TIME	ELAPSED TIME (MIN)	CHAMBER PRESSURE (PSI)	\bar{h}_1 BACK PRESS 1 (PSI)	\bar{h}_2 BACK PRESS 2 (PSI)	EXTERNAL BURETTE OR DIAL ROD. (CCI/(IN.))	INTERNAL BURETTE (CC)	Q (cc)	ΔQ (cc)
					$\bar{h}_1 - \bar{h}_2 = h$				
11/22/74	0825	0	42.5	20	35	14.0	22.9		
	0810						22.0		
	0827					14.2	17.9		
	0837						14.9		
	0838					14.4	1.9		
11/24/74	0622	0	42.5	29	35	14.5	22.9		
	0705						17.2		
	0717						15.5		
	0722						13.2		
	0747						11.0		
	0837						4.2		
	0810	12				14.5	1.4		
11/25/74	0901	0	42.5	25	35	14.5	21.8		
	0935	14					17.6		
	1004	13				14.5	15.8		
	1042	58					7.3		

$\Delta P = 4 \text{ psi}$

$\bar{P}_0 = 3.5 \text{ psi}$

$$K_{20^\circ C} = \frac{QL}{\tau h A} \times \frac{U_r}{U_{20^\circ C}}$$

- Q = cc
- h = (70.1) = cm
- L = cm
- $U_r = .931$ (23°C)
- $\tau = \text{min. (60)} = \text{sec}$
- A = cm²

SEE 4 OF 4.

$$K_{20^\circ C} \cdot \text{---} \cdot .931 =$$

PERMEABILITY TEST BY BACK PRESSURE CONSTANT-HEAD

Professional Service Industries 5461-025-5077

Sample Number TP-3

Depth 5-7 feet Remold 95% Mdd at omc

	Initial	Final
Wet Density pcf	129.8	135.7
Dry density pcf	115.4	118.2
% Moisture	12.5	14.8

Height Initial	3.000	568.8 Wet soil and dish
Diameter Initial	2.416	516.7 Dry soil and dish
Area Initial	4.582	100 dish only
Volume Initial	225.44	468.8 Ws Initial
Initial dial	1.062	478.5 Final Ws
Final dial	1.064	416.7 Weight solids
Initial cc/in res	0	
Final cc/in res.	14.5	

Height Final	2.998	7.615 cm
Diameter Final	2.388	
Area Final	4.477	26.909 cm ²
Volume Final	220.14	

Height change	-0.002	
cc/in reser.	0.008	
Volume change	-14.500	
Cell Change	9.2	
Net Volume Change	-5.300	
h = T/B PRESS. diff	4	280.40 cm

Standard Water .005 N CaSO4

Hydraulic Gradient
36.82

Elapsed Time	cc's	K
minutes		cm/sec
22.0	2.80	1.86E-06
34.0	5.20	2.23E-06
13.0	1.80	2.02E-06
58.0	8.50	2.14E-06

K Average = 2.06E-06 cm/s



Professional Service Industries, Inc.

TESTED FOR:
MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT:
LOWMAN, IDAHO

Attention: Mr. John Singleton

December 21, 1990

PSI Project #:355-00118-001

Page 44 of 46

REMARKS

TP-4A @ Depth 3'-5'

I. Particle Size Analysis with Hydrometer (ASTM D-422).

Table with 2 columns: Sieve Size and Percent Passing. Lists sieve sizes from 2.0 inches down to 250 microns and their corresponding percent passing values.

II. Atterberg Limits (ASTM D-4318)

Liquid Limit NP
Plastic Limit NP
Plastic Index NP

III. Natural Moisture Content (ASTM D-2216)

Moisture % 3.0

Respectfully submitted;
PROFESSIONAL SERVICE INDUSTRIES, INC.



REPORT OF MOISTURE DENSITY RELATIONSHIP OF SOIL

TESTED FOR: MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT: LOWMAN, IDAHO

Attention: Mr. John Singleton

DATE: September 21, 1990

OUR REPORT NO: 355-00118-001 Pg. 45 of 46

TEST DATA

Visual Classification: Sand with gravel and silt

Sample Source: TP-4A 3-5'

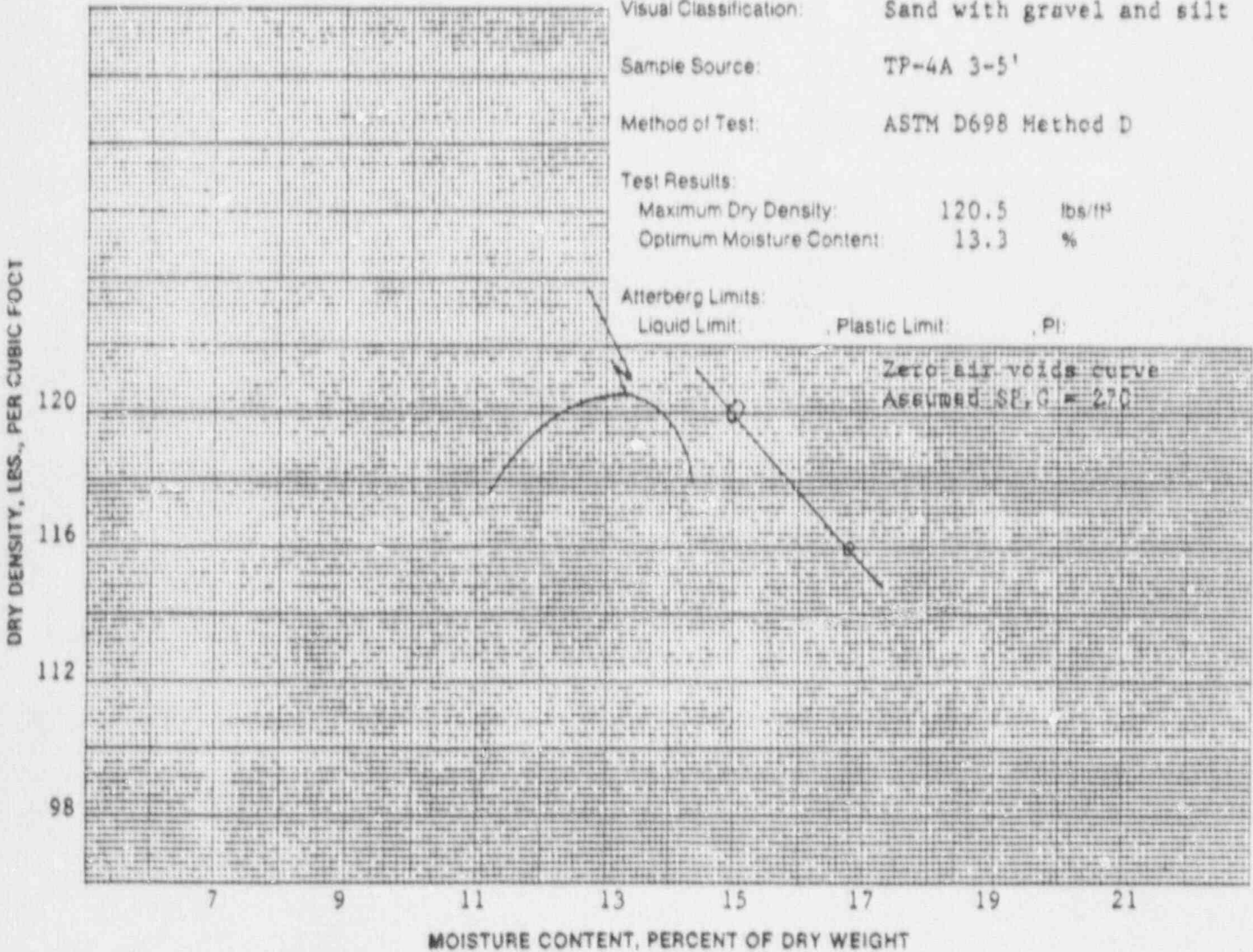
Method of Test: ASTM D698 Method D

Test Results:

Maximum Dry Density: 120.5 lbs/ft³
Optimum Moisture Content: 13.3 %

Atterberg Limits:

Liquid Limit: Plastic Limit: PI:



Zero air voids curve
ASSUMED SP.G = 2.70

Respectfully submitted,
Professional Service Industries, Inc.



Professional Service Industries, Inc.

TESTED FOR:
MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT:
LOWMAN, IDAHO

Attention: Mr. John Singleton

December 21, 1990

PSI Project #:355-00118-001

Page 46 of 46

REMARKS

TP-4B @ Depth 7'-9'

I. Particle Size Analysis with Hydrometer (ASTM D-422).

Sieve Size	Percent Passing
2.0"	100
1.5"	80
1.0"	74
3/4"	72
1/2"	70
3/8"	69
#4	63
#8	60
#16	57
#30	43
#50	30
#100	22
#200	17
1 Min.	14
2 Min.	13
5 Min.	12
15 Min.	11
30 Min.	9
60 Min.	8
250 Min.	6

II. Atterberg Limits (ASTM D-4318)

Liquid Limit	NP
Plastic Limit	NP
Plastic Index	NP

III. Natural Moisture Content (ASTM D-2216)

Moisture % 4.8

Respectfully submitted,
PROFESSIONAL SERVICE INDUSTRIES, INC.



Professional Service Industries, Inc.

REPORT OF MOISTURE DENSITY RELATIONSHIP OF SOIL

TESTED FOR: MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

PROJECT: LOWMAN, IDAHO

Attention: Mr. John Singleton

DATE: December 21, 1990

OUR REPORT NO: 355-00118-004 Pg. 46 of 46

TEST DATA

Visual Classification: Sand with gravel and silt

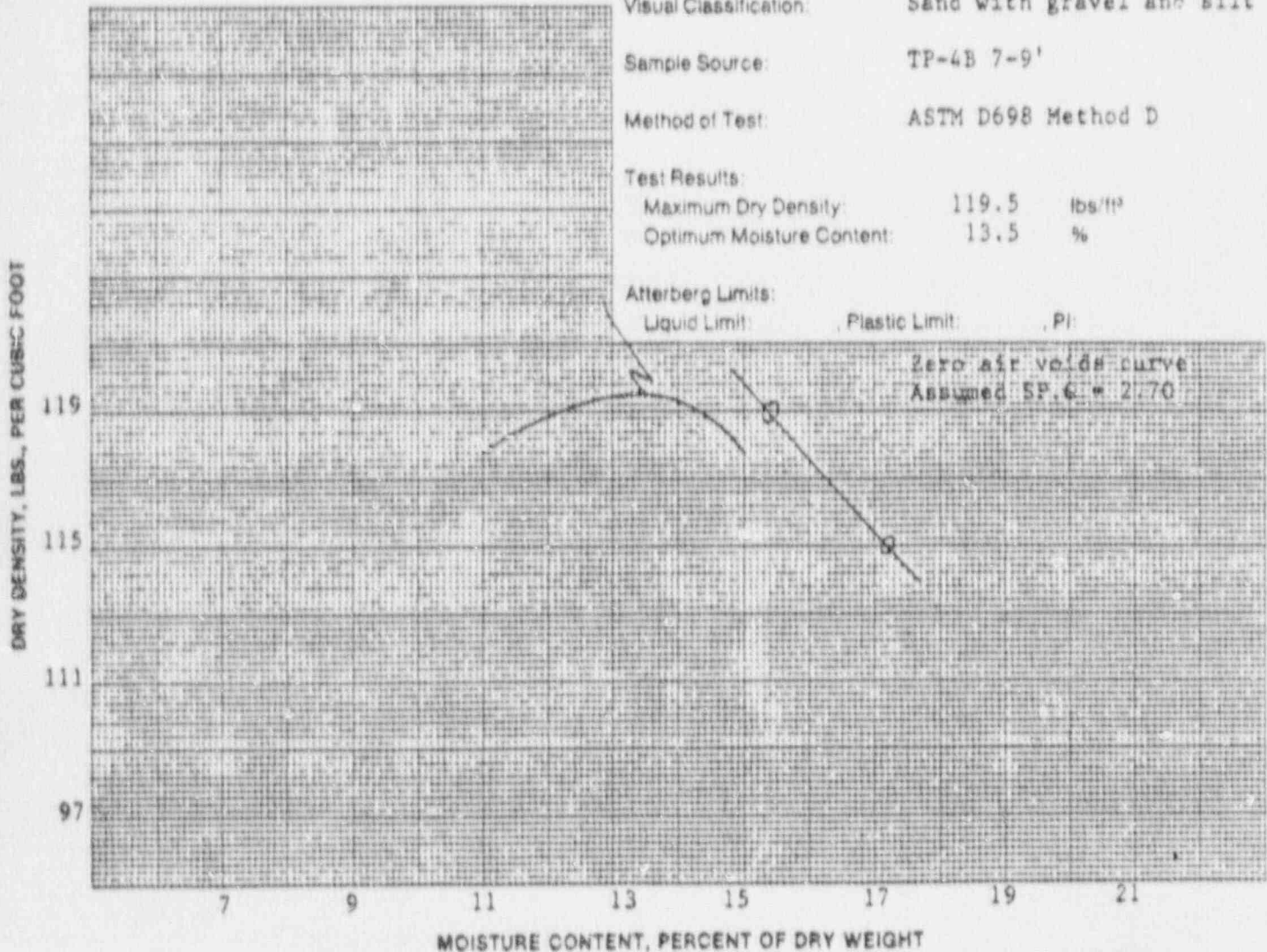
Sample Source: TP-4B 7-9'

Method of Test: ASTM D698 Method D

Test Results:
Maximum Dry Density: 119.5 lbs/ft³
Optimum Moisture Content: 13.5 %

Atterberg Limits:
Liquid Limit: Plastic Limit: PI:

Zero air voids curve
Assumed SP.G = 2.70



SPECIFIC GRAVITY OF SOIL (ASTM D-854)

Respectfully submitted,
Professional Service Industries, Inc.

G = 2.64

RIPRAP EVALUATION

PERFORMED FOR

MK-FERGUSON COMPANY
C/O SOUTH FORK LODGE
HIGHWAY 21
LOWMAN, IDAHO 83637

PROJECT

SAMPLING AND TESTING SERVICES
LOWMAN, IDAHO
P.O. # 3050-511-10133

PROJECT # 355-00084

DATE OF REPORT

OCTOBER 10, 1990



Professional Service Industries, Inc.

230 SOUTH COLE ROAD
BOISE, IDAHO 83709
(208) 376-4748



Professional Service Industries, Inc.

Tested for:

MK-FERGUSON COMPANY
 c/o South Fork Lodge
 Highway 21
 Lowman, Idaho 83637

Project:

SAMPLING AND TESTING SERVICES
 Lowman, Idaho
 P.O. # 3050-511-10133

Attention: Mr. John Singleton
 Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 1/15

FINAL REPORT

On August 23, 1990 your personnel delivered two riprap samples to our laboratory. Sample number one was described as coming from the "Fulfer Source" and sample two was described as coming from the "Stanley Source". Each sample consisted of three or four small boulders between one quarter and one half cubic foot in volume. Each sample contained pieces that are geologically different from each other. Samples are provided within this report show the different materials.

The tests requested include Specific Gravity, Percent Absorption, Sodium Sulfate Soundness, Los Angeles Abrasion, Schmidt Hammer and Splitting Tensile Strength. Petrographic examination of the materials are also required. On August 24, 1990 Mr. Bob Heneks with MK-Ferguson in San Francisco provided written information concerning the required testing methods.

Individual test procedures and results are as follows:

TEST NUMBER ONE AND TWO SPECIFIC GRAVITY & ABSORPTION TESTS

Test was performed in accordance with ASTM C-127, Standard Test Method for Specific Gravity of Coarse Aggregate. Samples consisted of crushed, two inch minus composite materials representing all materials delivered. Test results are as follows:

<u>TEST METHOD</u>	<u>FULFER SOURCE</u>	<u>STANLEY SOURCE</u>
Specific Gravity		
Bulk, SSD	2.58	2.61
Bulk	2.51	2.59
Apparent	2.70	2.65
Absorption		
Percent	2.8 %	0.9 %



Professional Service Industries, Inc.

Tested for:
 MK-FERGUSON COMPANY
 c/o South Fork Lodge
 Highway 21
 Lowman, Idaho 83637

Project:
 SAMPLING AND TESTING SERVICES
 Lowman, Idaho
 P.O. # 3050-511-10133

Attention: Mr. John Singleton
 Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 2/15

TEST NUMBER THREE SODIUM SULFATE SOUNDNESS

Test was performed in accordance with ASTM C-88, Standard Test Method for Soundness of Aggregate Using Sodium Sulfate. Samples consisted of crushed two inch minus composite materials representing all materials delivered. Only aggregate larger than a number four sieve was tested. The original grading used in the calculations is that indicated by the test method. Test Results are as follows:

FULFER SOURCE

TEST DATA REPORT

Sieve Size	Initial Weights	Original Grading(%)	Percent Loss	Weighted Percent Loss
2.0"-1.5"	1848.8	40.0	0.0	0.00
1.5"-1.0"	994.3	33.0	1.8	0.59
1.0"-3/4"	503.9			
3/4"-1/2"	671.9	21.0	0.0	0.00
1/2"-3/8"	299.6			
3/8"-No.4	299.6	6.0	17.8	1.07
TOTAL PERCENT LOSS				<u>1.66</u>

QUANTITATIVE EXAMINATION

Sieve Size	Splitting		Crumbling		Flaking		Number of Particles
	#	%	#	%	#	%	
2.0"-1.5"	-	-	-	-	20	100	23
1.5"-3/4"	-	-	-	-	63	100	63



Professional Service Industries, Inc.

Tested for:
MK-FERGUSON COMPANY
c/o South Fork Lodge
Highway 21
Lowman, Idaho 8637

Project:
SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 3/15

TEST NUMBER THREE SODIUM SULFATE SOUNDNESS (continued)

Test was performed in accordance with ASTM C-88, Standard Test Method for Soundness of Aggregate Using Sodium Sulfate. Samples consisted of crushed two inch minus composite materials representing all materials delivered. Only aggregate larger than a number four sieve was tested. The original grading used in the calculations is that indicated by the test method. Test Results are as follows:

STANLEY SOURCE

TEST DATA REPORT

Table with 5 columns: Sieve Size, Initial Weights, Original Grading(%), Percent Loss, Weighted Percent Loss. Rows include sieve sizes from 2.0"-1.5" down to 3/8"-No. 4, and a total percent loss of 1.39.

QUANTITATIVE EXAMINATION

Table with 5 columns: Sieve Size, Splitting (#, %), Crumbling (#, %), Flaking (#, %), Number of Particles. Rows include sieve sizes 2.0"-1.5" and 1.5"-3/4".



Professional Service Industries, Inc.

Tested for:
MK-FERGUSON COMPANY
c/o South Fork Lodge
Highway 21
Lowman, Idaho 83637

Project:
SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 4/15

TEST NUMBER FOUR LOS ANGELES ABRASION TEST

The test was performed in accordance with ASTM C-131, Standard Test Method for Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine. Samples consisted of crushed, one and one half inch minus composite samples representing all materials delivered. Grading " A " was used for both sample sources. As instructed by information provided by the client the method was altered to include 100 revolutions of the Los Angeles Machine. Test results are as follows:

<u>DESCRIPTION</u>	<u>FULFER SOURCE</u>	<u>STANLEY SOURCE</u>
Grading " A "		
Percent Loss (100 Revolutions)	9.9 %	6.4 %



Professional Service Industries, Inc.

Tested for:
 MK-FERGUSON COMPANY
 c/o South Fork Lodge
 Highway 21
 Lowman, Idaho 83637

Project:
 SAMPLING AND TESTING SERVICES
 Lowman, Idaho
 P.O. # 3050-511-10133

Attention: Mr. John Singleton
 Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 6/15

TEST NUMBER SIX SPLITTING TENSILE STRENGTH TEST

Test was performed in general accordance with Part Two, Suggested Method for Determining Indirect Tensile Strength by the Brazil Test. This method is from a book titled, "Rock Characterization Testing and Monitoring", published by the International Society for Rock Mechanics. This test was performed on core samples cut from the individual rock samples delivered to our laboratory. The size of the pieces and the internal cracks made it impossible to recover the ten core samples suggested in the test method. An electric coring machine with a diamond impregnated core bit was used to remove core samples from the delivered rock samples. Removed cores were then cut to proper thickness using a diamond impregnated rock saw. Samples were tested in an apparatus similar to that shown in the test method. The load was applied using a Forney Model FT-0040-DR compression testing machine. All samples were oven dry at the time of testing. Loading rate was modified to cause failure in one to three minutes as requested by our client. Test results on each rock sample are provided. These values were averaged for each geology and then averaged again for each sample source. Orientation of the axis of loading with respect to bedding planes, foliation, etc. is not known since these cores were taken from delivered quarry samples. It is noted that the orientation of the axis was varied on different samples from the same core. Test results are as follows:

FULFER SOURCE

<u>Sample</u>	<u>Diameter(in)</u>	<u>Thickness(in)</u>	<u>Load(lbs)</u>	<u>Tensile Strength</u>
F-1-A	2.68	1.19	6600	1370
B	2.68	1.31	7100	1290
			Rock Average	1330 PSI
F-2-A	2.67	1.38	5950	1030 PSI
F-3-A	2.67	1.28	3650	680
B	2.67	1.28	4150	770
C	2.66	1.27	3950	740
			Rock Average	730 PSI



Professional Service Industries, Inc.

Tented for:
MK-FERGUSON COMPANY
c/o South Fork Lodge
Highway 21
Lowman, Idaho 83637

Project:
SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 7/15

TEST NUMBER SIX SPLITTING TENSILE STRENGTH TEST (continued)

STANLEY SOURCE

Sample Diameter(in) Thickness(in) Load(lbs) Tensile Strength

Table with 5 columns: Sample, Diameter(in), Thickness(in), Load(lbs), Tensile Strength. Rows include samples 1-S-A, B, 1-S-C, D, and 2-S-A, B, with corresponding measurements and Rock Average values.

STANLEY SOURCE AVERAGE SPLITTING TENSILE STRENGTH 1310 PSI



Professional Service Industries, Inc.

Tested for:

MK-FERGUSON COMPANY
c/o South Fork Lodge
Highway 21
Lowman, Idaho 83637

Project:

SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 8/15

PETROGRAPHIC EXAMINATION

Petrographic examination of each delivered geology was performed by Mr. Kevin Sagez of our staff. Mr. Sagez possesses a Bachelors of Science Degree in Geology. He will complete his Masters Degree in Geology in May 1991. Mr. Sagez's evaluations are as follows:

Sample F-1 Fulfer Source

Hand Specimen Examination: This is a grayish black, fine grained, intergranular, Hypocrystalline rock composed of plagioclase, olivine and a large quantity of mafic minerals.

Thin Section Evaluation: Minerals visible in the thin section include Plagioclase, hornblende, olivine and minor pyroxenes. Under plane polarized light the anhedral hornblende is is pleochroic from green to brown and samples exhibit the 124/56 degree cleavage angles. Twinning is also common. Olivine is typically rounded and anhedral, showing signs of perlitic fracture pattern. Subhedral plagioclase crystals are commonly twinned but no zoning. The pyroxenes are anhedral crystals. A visual estimation of minerals is 70 percent Plagioclase and 30 percent hornblende, olivine and pyroxene. Name is Andesite Basalt.

Sample F-2 Fulfer Source

Hand Specimen Examination: This is a grayish black, medium grained, intergranular-hypocrystalline, Phaneritic rock composed of plagioclase, olivine and a large quantity of mafic minerals. Prominent iron oxide staining is present on weathered surfaces.

Thin Section Evaluation: Minerals visible in the thin section include Plagioclase, hornblende, olivine and minor pyroxenes. Under plane polarized light the anhedral hornblende is is pleochroic from green to brown and samples exhibit the 124/56 degree cleavage angles. Twinning is also common. Olivine is largely dominate and is typically rounded and anhedral, showing signs of perlitic fracture. Large subhedral plagioclase and olivine crystals lay within a background field of elongate subhedral plagioclase lathes. The pyroxenes are anhedral crystals. A visual estimation of minerals is 65 percent Plagioclase and 35 percent hornblende, olivine and pyroxene. Name is Andesite Basalt.



Professional Service Industries, Inc.

Tested for:

MK-FERGUSON COMPANY
c/o South Fork Lodge
Highway 21
Lowman, Idaho 83637

Project:

SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: October 10, 1990 PSI File Number: 355-0074-013 Pg 9/15

PETROGRAPHIC EXAMINATION (continued)

Sample F-3 Fulfer Source

Hand Specimen Examination: This is a grayish black, highly vesicular, porphyritic lava with phenocrysts of medium grained plagioclase and olivine in a porous, aphanitic groundmass.

Thin Section Evaluation: The thin section contains approximately 50 percent rounded voids that represent cross sections of vesicles. The minerals visible in the thin section include Plagioclase, hornblende, olivine and orthopyroxene. Under plane polarized light the hornblende is pleochroic from green to brown and samples exhibiting the 124/56 degree cleavage angles are present along with signs of alterations. Subhedral orthopyroxene displays the effect of pleochroism, ranging from clear to light pink. Small amounts of olivine indicate the normal perlithic fracture pattern. Plagioclase crystals are commonly twinned and display an internal alteration or recrystallization. A visual estimation of minerals is 75 percent Plagioclase and 25 percent hornblende, olivine and pyroxene. Name is Andesite Basalt.



Professional Service Industries, Inc.

Tested for:

MK-FERGUSON COMPANY
c/o South Fork Lodge
Highway 21
Lowman, Idaho 83637

Project:

SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project: Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 10/15

PETROGRAPHIC EXAMINATION (continued)

Sample S-1 Stanley Source

Hand Specimen Examination: This is a medium grained, porphyritic, intergranular-hypocrystalline, microlite rock. Crystals of plagioclase, olivine and some mafic minerals are easily recognized.

Thin Section Evaluation: Minerals visible in the thin section include Plagioclase, hornblende, olivine and orthopyroxenes. Under plane polarized light hornblende is pleochroic from green to brown and samples exhibit the 124/56 degree cleavage angles. Twinning is also common. Hornblende crystals range from anhedral to subhedral. Olivine is typically rounded and subhedral, showing signs of perlitic fracture and simple zoning. Subhedral plagioclase crystals are commonly twinned and show simple to complex zoning. The elongate subhedral orthopyroxenes appear as clear crystals. Some of the mafic minerals show signs of alteration in the thin section. A visual estimation of minerals is 70 percent Plagioclase and 30 percent hornblende, olivine and pyroxene. Name is Andesite Basalt.

Sample S-2 Stanley Source

Hand Specimen Examination: This is a pink, medium grained, phaneritic, microlite rock. Minerals visible in the hand specimen include plagioclase and cryptocrystalline quartz. Oxidation is visible as a clay alteration of the plagioclase.

Thin Section Evaluation: Minerals visible in the thin section include Quartz, potassium-feldspar, and cryptocrystalline quartz. Sericite and at least one type of opaque mineral are present. Deformed grains are sutured and concavo-convex along contacts. Grains are not equal granular. A visual estimation of minerals is 90 percent quartz and 10 percent plagioclase, potassium-feldspar, and cryptocrystalline quartz. Name is quartzolite.



Professional Service Industries, Inc.

Tested for:
 MK-FERGUSON COMPANY
 c/o South Fork Lodge
 Highway 21
 Lowman, Idaho 83637

Project:
 SAMPLING AND TESTING SERVICES
 Lowman, Idaho
 P.O. # 3050-511-10133

Attention: Mr. John Singleton
 Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 11/15

ROCK QUALITY SCORING CRITERIA

For this project rock quality is determined based up on a scoring criteria provided by the client. This scoring criteria is shown on Table 02278-A which has been included in this report as page number ten. Based upon the petrographic evaluations, the weighting factors for Igneous Rock have been used. Scores for each source are as follows:

FULFER SOURCE

<u>Test</u>	<u>Test Result</u>	<u>Weight Factor</u>	<u>Raw Score</u>	<u>Weighted Score</u>
Specific Gravity	2.70	9.0	9.0	81.0
Percent Absorption	2.8	2.0	1.4	2.8
% Loss Sodium Sulfate Soundness	1.66	11.0	9.6	105.6
Percent Abrasion	9.9	1.0	5.1	5.1
Schmidt Hammer	33	3.0	4.1	12.4
Splitting Tensile Strength (PSI)	1030	10.0	8.2	81.5
SCORE TOTAL				288.4

The total possible score for an Igneous Weighting factor is 360. The Fulfer Source scored 288.4 which is 80 percent of possible.

FULFER SOURCE ROCK QUALITY SCORE



Professional Service Industries, Inc.

Tested for:
 MK-FERGUSON COMPANY
 c/o South Fork Lodge
 Highway 21
 Lowman, Idaho 83637

Project:
 SAMPLING AND TESTING SERVICES
 Lowman, Idaho
 P.O. # 3050-511-10133

Attention: Mr. John Singleton
 Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 12/15

ROCK QUALITY SCORING CRITERIA (continued)

STANLEY SOURCE

Test	Test Result	Weight Factor	Raw Score	Weighted Score
Specific Gravity	2.65	9.0	8.0	72.0
Percent Absorption	0.9	2.0	0.6	11.2
% Loss Sodium Sulfate Soundness	1.39	11.0	9.8	107.8
Percent Abrasion	6.4	1.0	7.2	7.2
Schmidt Hammer	58	3.0	7.7	23.0
Splitting Tensile Strength (PSI)	1310	10.0	9.6	96.0
			SCORE TOTAL	317.0

The total possible score for an Igneous Weighting factor is 360. The Stanley Source scored 317 which is 88 percent of possible.

STANLEY SOURCE ROCK QUALITY SCORE

88



Professional Service Industries, Inc.

Tested for:
MK-FERGUSON COMPANY
c/o South Fork Lodge
Highway 21
Lowman, Idaho 83637

Project:
SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: October 10, 1990 PSI File Number: 355-00084-013 Pg 13/15

This completes all requested testing and reporting for this Riprap Evaluation Project.

Based upon the test data reported herein, we obtained a ROCK QUALITY SCORE of 80 on the Fulfer Source and 88 on the Stanley Source.

Thank-you for retaining Professional Service Industries, Inc.
Should you have any questions please let me know.

Respectfully submitted;
PROFESSIONAL SERVICE INDUSTRIES, INC.

Bob J. Arnold, P.E.
Division Manager



TABLE 02276-A
ROCK QUALITY SCORING CRITERIA

Professional Service Industries
Project Number:355-00084-013 Pg. 14 of 15

	Weighting Factor			Score										
	Lime- stone	Sand- stone	Igne- ous	10	9	8	7	6	5	4	3	2	1	0
Specific Gravity	12	5	9	2.75	2.70	2.65	2.60	2.55	2.50	2.45	2.40	2.35	2.30	<2.3
Absorption (%)	13	5	2	0.1	0.3	0.5	0.67	0.83	1.0	1.5	2.0	2.5	3.0	>3.0
Sodium Sulfate (%)*	4	3	11	1	3	5	6.7	8.3	10	12.5	15	20	25	>25
Abrasion (%)**	1	8	1	1	3	5	6.7	8.3	10	12.5	15	20	25	>25
Schmidt Hammer	11	13	3	70	65	60	54	47	40	32	24	16	8	<8
Splitting Tensile Strength (psi)	5	4	10	1400	1200	1000	833	666	500	400	300	200	100	<100

Note: Any rock to be used must be qualitatively rated at least "fair" in a petrographic examination conducted by a geologist experienced in petrographic analysis.

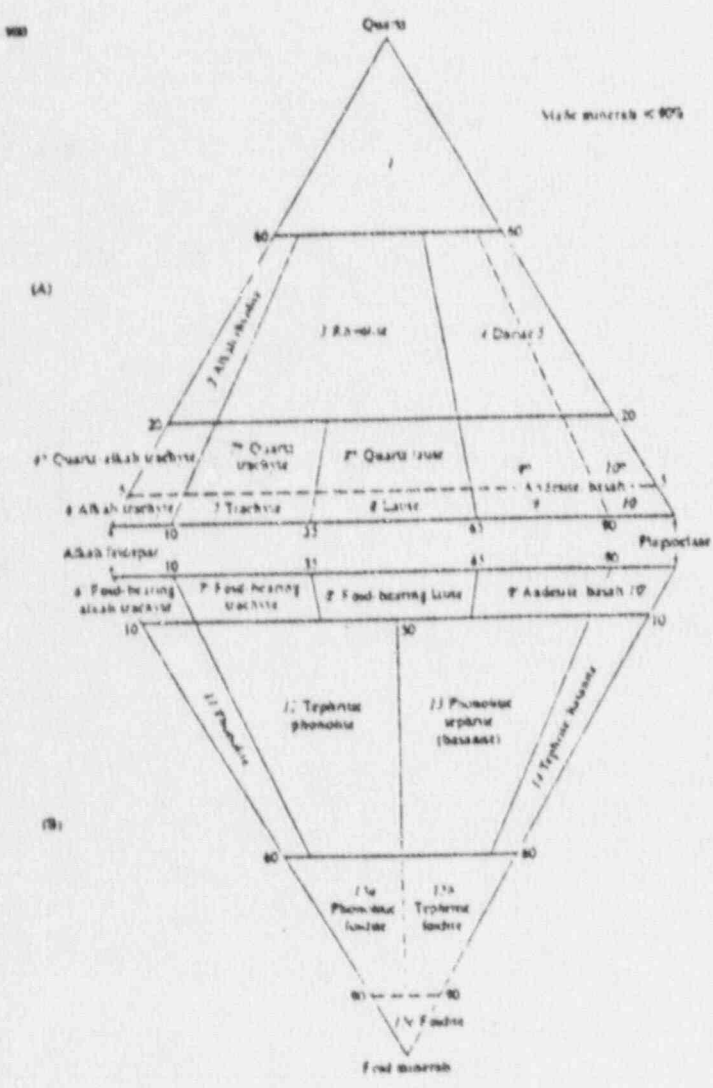
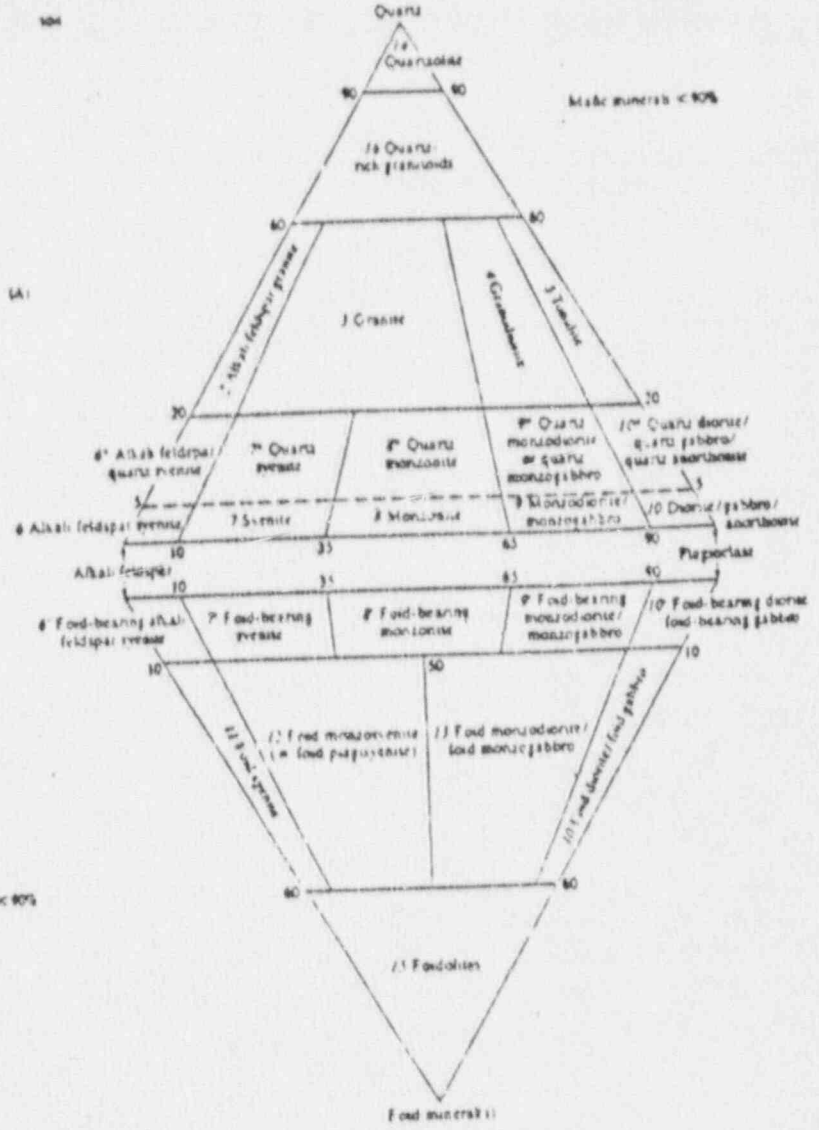
* 5 cycles

** 100 revolutions

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— MKES Doc. No.
3885-LOW-R-09-00579-00 —

*Note: This source, Stanley,
is a different source
than the Stanley source
reported in the October 10, 1990
report.*

RIPRAP EVALUATION

PREFORMED FOR

MK-FERGUSON COMPANY
P.O. BOX 9136
ALBUQUERQUE, NM 87119

PROJECT

SAMPLING AND TESTING SERVICES
LOWMAN, IDAHO
P.O. # 3050-511-10133

PROJECT # 355-00084

DATE OF REPORT

NOVEMBER 30, 1990



Professional Service Industries, Inc.

230 SOUTH COLE ROAD
BOISE, IDAHO 83709
(208) 376-4748



Professional Service Industries, Inc.

Tested for:
MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

Project:
SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: November 30, 1990 PSI File Number: 355-00084-014 Pg. 1/10

FINAL REPORT

On October 25, 1990 your personnel delivered one riprap sample to our laboratory. The sample was described as coming from the "Stanley Source". The sample consisted of ten to fifteen small boulders between one quarter and one half cubic foot in volume. The sample contained pieces that appear geologically similar to each other. Samples are provided with this report.

The tests requested include Specific Gravity, Percent Absorption, Sodium Sulfate Soundness, Los Angeles Abrasion, Schmidt Hammer and Splitting Tensile Strength. Petrographic examination of the materials are also required.

Individual test procedures and results are as follows:

TEST NUMBER ONE AND TWO SPECIFIC GRAVITY & ABSORPTION TESTS

Test was performed in accordance with ASTM C-127, Standard Test Method for Specific Gravity of Coarse Aggregate. Samples consisted of crushed, two inch minus composite materials representing all materials delivered. Test results are as follows:

TEST METHOD

Specific Gravity

Table with 2 columns: Test Type and Value. Rows: Bulk, SSD (2.75), Bulk (2.77), Apparent (2.80)

Absorption

Table with 2 columns: Test Type and Value. Row: Percent (0.68%)



Professional Service Industries, Inc.

Tested for:
MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

Project:
SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: November 30, 1990 PSI File Number: 355-00084-014 Pg 2/10

TEST NUMBER THREE SODIUM SULFATE SOUNDNESS

Test was performed in accordance with ASTM C-88, Standard Test Method for Soundness of Aggregate Using Sodium Sulfate. Samples consisted of crushed 1.5 inch minus composite materials representing all materials delivered. Only aggregate larger than a number four sieve was tested. The original grading used in the calculations is that indicated by the test method. Test Results are as follows:

TEST DATA REPORT

Table with 5 columns: Sieve Size, Initial Weights, Original Grading(%), Percent Loss, Weighted Percent Loss. Rows include sieve sizes 1.5"-1.0", 1.0"-3/4", 3/4"-1/2", 1/2"-3/8", 3/8"-No.4 and a total percent loss of 1.68.

QUANTITATIVE EXAMINATION

Table with 7 columns: Sieve Size, Splitting (#, %), Crumbling (#, %), Flaking (#, %), Number of Particles. Row for 1.5"-3/4" shows 4 flaking and 7 particles.



Professional Service Industries, Inc.

Tested for:

MK-FERGUSON COMPANY
P.O. Box 9136
Albuquerque, NM 87119

Project:

SAMPLING AND TESTING SERVICES
Lowman, Idaho
P.O. # 3050-511-10133

Attention: Mr. John Singleton
Project Manager

Date: November 30, 1990 PSI File Number: 355-00084-014 Pg 3/10

TEST NUMBER FOUR LOS ANGELES ABRASION TEST

The test was performed in accordance with ASTM C-131, Standard Test Method for Resistance to Degradation of Small-Size Coarse Aggregate by Abrasion and Impact in the Los Angeles Machine. Samples consisted of crushed, one and one half inch minus composite samples representing all materials delivered. Grading " A " was used. As instructed by information provided by the client the method was altered to include 100 revolutions of the Los Angeles Machine. Test results are as follows:

DESCRIPTION

Grading " A "

Percent Loss 4.4%

(100 Revolutions)



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TEST NUMBER FIVE SCHMIDT HAMMER TEST

Test was performed in general accordance with Part Three, Suggested Method for Determination of the Schmidt Rebound Hardness. This method is from a book titled, "Rock Characterization Testing and Monitoring", published by the International Society for Rock Mechanics. This reference was provided by our client. The test was performed on two selected individual geologic pieces of the sample. Each piece was held rigid by clamping in our Forney Compression Testing Machine. Rock pieces were shaped and cracked in a manner that made performing the test on flat surfaces with six centimeters of crack free material in all directions not always possible. Each of the twenty recorded readings on each rock are shown below. The highest ten of these values were averaged to determine each sample sources test result. It is noted that a Type N Schmidt Hammer was used for this test. The test anvil and sample test values obtained were proportionately adjusted to a Type L Hammer. Test results are as follows:

	<u>Rock 1</u>	<u>Rock 2</u>		
48	52	42	53	
49	53	48	53	
49	54	49	45	
50	54	50	54	
50	54	50	54	
50	55	50	55	
50	55	50	55	
51	55	51	56	
51	55	52	58	
52	56	52	59	
AVG	54		55	

SCHMIDT REBOUND INDEX 55



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TEST NUMBER SIX SPLITTING TENSILE STRENGTH TEST

Test was performed in general accordance with Part Two, Suggested Method for Determining Indirect Tensile Strength by the Brazil Test. This method is from a book titled, "Rock Characterization Testing and Monitoring", published by the International Society for Rock Mechanics. This test was performed on core samples cut from the individual rock samples delivered to our laboratory. The size of the pieces and the internal cracks made it impossible to recover the ten core samples suggested in the test method. An electric coring machine with a diamond impregnated core bit was used to remove core samples from the delivered rock samples. Removed cores were then cut to proper thickness using a diamond impregnated rock saw. Samples were tested in an apparatus similar to that shown in the test method. The load was applied using a Forney Model FT-0040-DR compression testing machine. All samples were oven dry at the time of testing. Loading rate was modified to cause failure in one to three minutes as requested by our client. Orientation of the axis of loading with respect to bedding planes, foliation, etc. is not known since these cores were taken from delivered quarry samples. It is noted that the orientation of the axis was varied on different samples from the same core. Test results are as follows:

Table with 5 columns: Sample, Diameter(in), Thickness(in), Load(lbs), Tensile Strength. Includes 8 rows of data and an average row.

*Indicates that test sample contained a fracture visible to naked eye.



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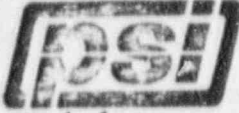
Date: November 30, 1990 PSI File Number: 355-00084-014 Pg 6/10

PETROGRAPHIC EXAMINATION

Petrographic examination of each delivered geology was performed by Mr. Kevin Sager of our staff. Mr. Sager possesses a Bachelors of Science Degree in Geology. He will complete his Masters Degree in Geology in May 1991. Mr. Sager's evaluations are as follows:

Hand Specimen Examination: This is a grayish black, fine grained, intergranular, Hypocrystalline rock composed of plagioclase, olivine and a large quantity of mafic minerals.

Thin Section Evaluation: Minerals visible in the thin section include Plagioclase, hornblende, olivine and minor pyroxenes. Under plane polarized light the anhedral hornblende is pleochroic from green to brown and samples exhibit the 124/56 degree cleavage angles. Twinning is also common. Olivine is typically rounded and anhedral, showing signs of perlitic fracture pattern. Subhedral plagioclase crystals are commonly twinned but no zoning. The pyroxenes are anhedral crystals. A visual estimation of minerals is 70 percent Plagioclase and 30 percent hornblende, olivine and pyroxene. Name is Andesite Basalt.



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 Project Manager

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ROCK QUALITY SCORING CRITERIA

For this project rock quality is determined based up on a scoring criteria provided by the client. This scoring criteria is shown on Table 02278-A which has been included in this report as page number ten. Based upon the petrographic evaluations, the weighting factors for Igneous Rock have been used. Scores for each source are as follows:

Test	Test Result	Weight Factor	Raw Score	Weighted Score
Specific Gravity	2.80	9.0	10.0	90.0
Percent Absorption	0.68	2.0	6.9	13.8
% Loss Sodium Sulfate Soundness	1.68	11.0	9.7	106.7
Percent Abrasion	4.4	1.0	8.3	8.3
Schmidt Hammer	55	3.0	7.2	21.6
Splitting Tensile Strength (PSI)	1950	10.0	10.0	100.0
			SCORE TOTAL	340.4

The total possible score for an Igneous Weighting factor is 360. This source scored 340.4 which is 95 percent of possible.

ROCK QUALITY SCORE



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This completes all requested testing and reporting for this Riprap Evaluation Project.

Based upon the test data reported herein, we obtained a ROCK QUALITY SCORE 95 on this Source.

Thank-you for retaining Professional Service Industries, Inc.
Should you have any questions please let me know.

Respectfully submitted;
PROFESSIONAL SERVICE INDUSTRIES, INC.

A handwritten signature in dark ink, appearing to read 'Bob J. Arnold', is written over the typed name.

Bob J. Arnold, P.E.
Division Manager

TABLE 02276-A
ROCK QUALITY SCORING CRITERIA

Professional Service Industries
Project Number: 355-00084-013 Pg. 9 OF 10

	Weighting Factor			Score										
	lime- stone	Sand- stone	igne- ous	10	9	8	7	6	5	4	3	2	1	0
Specific Gravity	12	5	9	2.75	2.70	2.65	2.60	2.55	2.50	2.45	2.40	2.35	2.30	<2.3
Absorption (%)	13	5	2	0.1	0.3	0.5	0.67	0.83	1.0	1.5	2.0	2.5	3.0	>3.0
Sodium Sulfate (%) ^a	4	3	11	1	3	5	6.7	8.3	10	12.5	15	20	25	>25
Abrasion (%) ^{**}	1	8	1	1	3	5	6.7	8.3	10	12.5	15	20	25	>25
Schmidt Hammer	11	13	3	70	65	60	54	47	40	32	24	16	8	<8
Splitting Tensile Strength (psi)	5	4	10	1400	1200	1000	833	666	500	400	300	200	100	<100

Note: Any rock to be used must be qualitatively rated at least "fair" in a petrographic examination conducted by a geologist experienced in petrographic analysis.

* 5 cycles
** 100 revolutions

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