

SOUTH CAROLINA ELECTRIC & GAS COMPANY

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VICE PRESIDENT AND GROUP EXECUTIVE  
NUCLEAR OPERATIONS

December 2, 1980

Mr. Harold R. Denton, Director  
Office of Nuclear Reactor Regulation  
U. S. Nuclear Regulatory Commission  
Washington, DC 20555

Subject: Virgil C. Summer Nuclear Station, Unit 1  
Docket No. 50-395  
Emergency Feedwater System

Dear Mr. Denton:

In response to questions by Auxiliary Systems Branch Reviewers of the emergency feedwater system reliability analysis and subsequent telephone conversations with Linda Huang, South Carolina Electric and Gas Company acting for itself and as agent for the South Carolina Public Service Authority provides forty-five (45) copies of our commitment to do the following:

1. Install a manually operated valve in parallel with the isolation valve for the condensate storage tank (Valve 1010).
2. Provide the same type of control room indication (i.e. audible alarm and by-pass and inoperable status indication (BISI)) for the parallel valve as already exists for Valve 1010.
3. Remove the handwheels and place locking clamps on the shafts of both Valve 1010 and its parallel valve.
4. Provide the same type of surveillance for the parallel valve as for Valve 1010. In addition, information is attached regarding the time required for switchover to service water to avoid damage to the emergency feedwater pumps versus the actual response time of the plant.

This information will be incorporated in a subsequent FSAR Amendment.

We understand that this allows completion of Task Action Plan Item II.E.1.1 concerning the emergency feedwater reliability analysis.

If you have any questions, please contact us.

Very truly yours,

*T. C. Nichols, Jr.*  
T. C. Nichols, Jr.

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POOR QUALITY PAGES

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Mr. Harold R. Denton  
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ANALYSIS/CALCULATION  
DEPARTMENT NUMBER  
*0428*

PROJECT NAME  
*V. C. Summer Station Unit 1*

W.D. NUMBER  
*04-0001-000*

DEPARTMENT NAME  
*Mechanical*

**COVER PAGE AND DESIGN VERIFICATION RECORD**

REFERENCE NUMBER	3	2	1	0
ORIGINATOR (INITIALS AND SURNAME)				<i>R.J. Sheldon</i>
DATE COMPLETED				<i>5/20/80</i>
THIS ANALYSIS/CALCULATION CONTAINS ASSUMPTIONS REQUIRING LATER CONFIRMATION (YES OR NO)				<i>No</i>
VERIFIER (INITIALS AND SURNAME)				<i>D.T. Hinkley</i>
DATE VERIFIED				<i>3 June 1980</i>
THE DESIGN ANALYSIS/CALCULATION HAS BEEN REVIEWED BY ME AGAINST THE APPLICABLE DESIGN REVIEW QUESTIONS (REFERENCE ANSI N45.2.11). ANY FINDINGS UNCOVERED DURING MY REVIEW HAVE BEEN DIRECTED TO THE ORIGINATOR AND RESOLVED. (VERIFIER'S SIGNATURE)				<i>Daniel T. Hinkley</i>
THE DESIGN REVIEW OF THE ANALYSIS/CALCULATION INCLUDED EVALUATION AGAINST THE FOLLOWING QUESTIONS:  WERE INPUTS, INCLUDING CODES, STANDARDS, AND REGULATORY REQUIREMENTS, CORRECTLY SELECTED AND APPLIED?  ARE ASSUMPTIONS ADEQUATELY DESCRIBED AND REASONABLE?  ARE ASSUMPTIONS REQUIRING REVERIFICATION ADEQUATELY IDENTIFIED?  HAVE APPLICABLE CONSTRUCTION AND OPERATING EXPERIENCES BEEN CONSIDERED?  WAS AN APPROPRIATE ANALYSIS/CALCULATION METHOD USED?  IS THE OUTPUT REASONABLE COMPARED TO INPUTS?	REMARKS	REMARKS	REMARKS	REMARKS <i>NONE</i>

FILE NO. CODE

1. Objective

Determine time available to open SW to EF control valves after a tornado causes loss of normal water supply to the EF pumps.

2. Assumptions

1. Tornado causes rupture of EF section line to condensate storage tank at ground level.
2. The single largest pump (Turbine Drive) is operating at rated capacity.
3. A loss of off-site power occurs simultaneously.
4. Head loss (in calculating NPSH) in the short run of pipe from the vertical line (from 414'-9" to 411'-0") is negligible.

5. Flow is proportional to valve position between the closed and the 1/2 open positions. Inchausti/Darling graph has a gate valve attached to water tank shown round.
6.  $\Delta P$  across two identical valves in series is equal and the pressure drop in the SW to EF piping is insignificant compared to the  $\Delta P$  across partially open valves.



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Reading Room/Office

ANALYSIS/CALCULATION

SUBJECT <i>Seaford no Rupture</i>		CLASS <i>FF-16-a</i>		PAGE <i>3</i>
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ORIGINATOR <i>R. Stollan</i>				
DATE <i>5/20/80</i>				

3. References

- a. *IMS-17-153-0*
- b. *B/m R1d-1*
- c. *E-304-089, Rev 14*
- d. *E-303-024, Rev 11*
- e. *Hydraulic Institute Standards, 12<sup>th</sup> edition, 1969*
- f. *Crane TP No 410, 1976*
- g. *IMS-224-3*
- h. *Anchors, Darling, H. of. 3/12/76*
- i. *Crane TP No 410, 1969*
- j. *D-302-221, Rev 5*
- k. *FSAR*

4. Equations

$$NPSH = h_{20} = \frac{144}{\omega} (P_a - P_{vp}) + h_s \quad \text{Ref (c) pg 69}$$

$$Q = C_v \sqrt{\Delta P \frac{1.41}{\rho}} \quad \text{Ref (c) eqn 3-16}$$

5. Data

Pump NPSH required	27 ft	Ref (a)
Pump Design Capacity	900 gpm	Ref (a)
EF Maximum Temp	120°F	Ref (b)
Pump Suction Elevation	414'-3"	Ref (c)
Pipe length from 414'-6" elev in vented line to ground level	137 ft	Ref (c) + (1)
Pipe size	10" Sch 40	Ref (c)
Area, 10" Sch 40	0.5475 ft <sup>2</sup>	Ref (c) pg B-16



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ANALYSIS/CALCULATION

SUBJECT

Section Line Discharge

CISID

EF-14-A

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ORIGINATOR R.J. Skiridov

DATE 5/20/80

5 Data cont'd

Values 1002, 1008, 1037A+B-EF

Opening time

42 sec

Ref (6)

Cv

4440

Ref (8)

Dissect Ready to Load

10 sec

Ref (11) PE 8.3-106

SW Pressure

40 psig

Ref (1)

SW Pumps loaded onto disect

10 sec.

Ref (11) Fig 7.3

6 Calculations

A. Time from pipe rupture to pump out of NPSH

$$NPSH = h_{su} = \frac{144}{g} (p_a - h_{op}) + h_s$$

h<sub>su</sub> required is 27 ft

p<sub>a</sub> = 14.7 psig

p<sub>op</sub> (120°F) = 1.7 psig

Ref (5) PE A-12

W<sub>water</sub> = 62.7 lb/ft<sup>3</sup>

Ref (1) PE A-12

$$\begin{aligned} h_s &= h_{su} - \frac{144}{g} (p_a - p_{op}) \\ &= 27 - \frac{144}{64.4} (14.7 - 1.7) \\ &= 27 - 30.3 \\ &= -3.3 \end{aligned}$$

∴ Pump will run if water level maintained to level of top of pump suction.

Time for pump to empty line from ground level to elev. 414-8

$$\begin{aligned} \text{Volume pipe} &= \text{Length} \times \text{Area} \\ &= 137 \text{ ft} \times .5475 \text{ ft}^2 \times 7.48 \text{ gal/ft}^3 \\ &= 561 \text{ gal} \end{aligned}$$



SUBJECT	Section Line Rupture			CLASS	FP-16-a	PAGE	5
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ORIGINATOR	J. Sholtz						
DATE	12/91						

to Calc's Cont'd

$$\begin{aligned} \text{Time} &= \text{Vol} / \text{Pump Capacity} \\ &= 921 / 900 = .623 \text{ min} \\ &= 37 \text{ sec} \end{aligned}$$

B. Time from line break to when SW is available

1.  $C_v$  of valve full open = 11140

L/D of full open gate valve is 13 } Ref (1) pg A-30  
 L/D of 1/4 open gate valve is 900 }

$C_v$  is proportional to  $1/\sqrt{L/D}$  Ref (1) Eqn 3-151-316

Using ratio from Cams,

$$C_v \text{ 1/4 open} = 534$$

$$\text{Total } \Delta P = 40 \text{ psig}$$

$\Delta P$  across one valve is 20 psig

$$\begin{aligned} Q &= C_v \sqrt{\Delta P \frac{SG}{\rho}} \quad p = 62.4 \\ &= C_v \sqrt{\Delta P} \\ &= 534 \times \sqrt{20} = 2388 \end{aligned}$$

$$\text{Position for } 900 \text{ gpm} = \frac{900}{2388} \times .25 = .1$$

$$\begin{aligned} \text{Time} &= .1 \times 42 \text{ sec} \\ &= 5 \text{ seconds} \end{aligned}$$



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Reading Room, 4/10/78

ANALYSIS/CALCULATION

SUBJECT

Surf on Line Rupture

LIBID

10-4-a

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ORIGINATOR P.J. Sheahan

DATE

5/30/78

6. Calc's cont'd

Time for valve to open = diesel start time (10 sec)  
 + valve open to 300 gpm position (5 sec) =  
 15 sec.

(Valve are load block 2 which has  
 a 0 sec delay, Ref (4), Fig 7.3-1)

2. Time for SW pumps to be on line =  
 diesel start time (10 sec) +  
 SW Pump start delay (10 sec, Ref (4), Fig 7.3-1)  
 = 20 sec.

Worst case is 20 sec for SW pump to  
 be on line

∴ Since EF pump has 37 sec supply before  
 SW is required 20 sec for SW pump to be  
 on line is satisfactory.