
Reduction of Structural Degrees of Freedom

Manuscript Completed: December 1980
Date Published: April 1981

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Prepared for
Division of Reactor Safety Research
Office of Nuclear Regulatory Research
U.S. Nuclear Regulatory Commission
Washington, D.C. 20555
NRC FIN A3216

810 416 0448

Abstract

Guyan reduction is used extensively to reduce the degrees of freedom of a structural model prior to performing dynamic analysis. A modification to Guyan reduction is developed which results in the same eigenvalue problem as Guyan reduction but gives an improved transformation for expanding the reduced mode shapes. The SAPV computer program is modified to include both reduction methods.

Four sample problems are then solved using both of the reduction methods and the results are compared with an exact solution. For each of the problems the number of degrees of freedom retained for the dynamic analysis is varied. The errors in frequencies are found to be small while the errors in member forces can be quite large. The errors resulting from the modified reduction method are much smaller and more uniform throughout the structure than those resulting when Guyan reduction is used. Member force errors are much larger in systems where there are large differences in element stiffnesses (e.g., piping systems) than in systems where the stiffnesses is about the same (e.g., frames).

Table of Contents

Section	Page
List of Figures	vii
List of Tables	ix
1.0 INTRODUCTION	1
2.0 ANALYSIS	3
2.1 Reduction Methods	3
2.2 Eigenvalue Solution	6
2.3 Expansion of Reduced Eigenvectors	7
3.0 SAPV MODIFICATION	8
3.1 General Form of Modification	8
3.2 Input Data Requirements	9
3.3 Description of Subroutines	11
3.3.1 Storage Allocation	11
3.3.2 REDUC1	12
3.3.3 SIZE	12
3.3.4 STIFF	16
3.3.5 MASS	16
3.3.6 GAUS	16
3.3.7 REDUCE	17
3.3.8 POWER	17
3.3.9 EXPAND	18
3.3.10 LOCATE	18
3.3.11 PPLOC	18
3.3.12 PSLOC	18
3.3.13 INVERT	18
3.3.14 GRAMS	18

Tables of Contents (Cont'd)

	Page
4.0 SAMPLE PROBLEM _____	19
4.1 Ten Story Building _____	19
4.2 Small Structure Added to Ten Story Building _____	25
4.3 Piping System _____	31
4.4 Reactor Containment Building _____	45
4.5 Computer Requirements _____	49
5.0 CONCLUSIONS AND RECOMMENDATIONS _____	49
REFERENCES _____	52
Appendix A Listing of SAPV Modifications _____	A-1
Appendix B Use of Modified SAPV at Brookhaven National Laboratory _____	B-1

List of Figures

Figure		Page
1	General Form of SAPV Modification _____	10
2	Ten Story Frame _____	20
3	Base Response Spectra _____	21
4	Small Structure _____	26
5	Piping System _____	32
6	Containment Building _____	46

List of Tables

Table		Page
1	Data Storage in Vector A _____	13
2	Definition of Control Parameters _____	15
3	Ten Story Frame Frequencies and Member Bending Moments _____	23
4	%Errors in Member Moments _____	24
5	Reduction in Small Structure _____	27
6	Frequencies in Frame with Small Structure _____	28
7	Bending Moments in Small Structure _____	29
8	%Errors in Small Structure Moments _____	30
9	Reduction Schemes for Piping System _____	33
10	Piping System Frequencies _____	34
11	Piping Forces _____	35
12	Percentage Errors in Piping Elements _____	42
13	Reduction Schemes for Containment Building _____	45
14	Frequencies for Containment Building _____	47
15	Stresses and Moments in Containment Building _____	48

1.0 INTRODUCTION

The development of discrete models for structural frameworks requires that compromises be made when deciding on the total number of degrees of freedom to be retained. A precise description of the structure may require many more degrees of freedom than are acceptable from a computational viewpoint. Limitations of the degrees of freedom to be retained arise from both restrictions embedded in the available software and economic considerations of the cost of generating the computer solutions.

Such limitations are usually more serious for dynamic problems than for static problems. Most of the large structural software packages therefore provide the user with the option of describing the structure with more degrees of freedom than are retained for dynamic response computations. Degrees of freedom to be retained in the dynamic analysis are specified by the user and a transformation relating the remaining degrees of freedom to these retained is used to reduce the order of the system (Guyan Reduction, Ref. 1, is used most often).

This dynamic reduction is often (Refs. 1,2,3) performed based upon a transformation which neglects inertial effects for the degrees of freedom to be eliminated. The transformation is then used to generate mass and stiffness matrices for the reduced system that approximate the kinetic and potential energies of the complete system. There are few data available to assess the extent to which this dynamic reduction introduces errors into various aspects of the problem.

The errors introduced in three small problems (cantilever beam, gabled frame, and small piping loop) are discussed in Ref. 4. It is shown that Guyan Reduction may result in very significant errors in the member loads

(20-40 percent for models which appear reasonable) while the modal frequencies appear quite good. A relatively simple modification of Guyan Reduction results in significantly smaller errors.

The purpose of the work reported here is to extend the studies reported in Ref. 4 to include larger structural systems and in particular structural systems found in nuclear power plants. The specific objectives are twofold. First, an assessment is made of the errors in large structural systems when both Guyan and Modified Guyan Reduction are used. Second, the economies that may be achieved by using these reduction methods as compared with obtaining complete solutions (i.e., no reduction) are investigated.

To achieve these objectives both Guyan Reduction and Modified Guyan Reduction are added to the SAPV (Ref. 5) computer program. The current version of SAPV has no reduction capability. SAPV was selected because it has the capabilities (size and element library) to analyze the structures of interest and the program is relatively easy to modify (as compared with other general purpose structural programs).

The analysis upon which the reduction methods are based is described in Section 2.0. The implementation of these methods into SAPV are discussed in Section 3.0 (Appendix A contains a listing of the modification to SAPV and Appendix B contains a users guide indicating how this version of SAPV may be accessed at Brookhaven National Laboratory). The application to sample problems is described in Section 4.0 and the work is summarized in Section 5.0.

2.0 ANALYSIS

The normal flow of computations in the SAPV program involves:

(i) formation of the mass and stiffness matrices; (ii) calculating the required eigenvalues and eigenvectors; (iii) performing the response spectrum analysis; and (iv) evaluating modal displacements and member loads. When the reduction methods are added, this flow is interrupted after (i). At this stage: reduced mass and stiffness matrices are developed; the eigenvalues and eigenvectors for the reduced system are determined; and the eigenvectors are expanded from the reduced degrees of freedom to all degrees of freedom. The normal flow of SAPV computation at (iii) is then reentered and the solution concluded as a normal SAPV run. The analysis upon which each of these three new computations are based is outlined.

2.1 REDUCTION METHODS

The problems of solving for the eigenvectors and forced response of a linear structural system described in terms of its mass and stiffness matrices are considered. In particular it is of interest to consider those problems wherein some reduction in the total number of degrees of freedom is attempted before any solutions are found.

Consider the eigenvalue problem for a discrete structural system where it is desired to eliminate secondary degrees of freedom (X_s) and retain only primary degrees of freedom (X_p) thereby reducing the order of the equations to be solved. The partitioned equations of motion describing free vibration of the system may be written as:

$$\begin{bmatrix} M_{pp} & M_{ps} \\ - & - \\ M_{sp} & M_{ss} \end{bmatrix} \begin{Bmatrix} \ddot{X}_p \\ \ddot{X}_s \end{Bmatrix} + \begin{bmatrix} K_{pp} & K_{ps} \\ - & - \\ K_{sp} & K_{ss} \end{bmatrix} \begin{Bmatrix} X_p \\ X_s \end{Bmatrix} = 0 \quad (1)$$

where M = mass matrix
 K = stiffness matrix
 $\ddot{X}$ = acceleration

For each mode of the eigenvalue problem

$$\ddot{X} = \begin{Bmatrix} \ddot{X}_p \\ \ddot{X}_s \end{Bmatrix} = -\lambda^2 \begin{Bmatrix} X_p \\ X_s \end{Bmatrix} \quad (2)$$

where λ = eigenvalue

Substituting Eq. (2) into (1) gives,

$$-\lambda^2 \begin{bmatrix} M_{pp} & M_{ps} \\ - & - \\ M_{sp} & M_{ss} \end{bmatrix} \begin{Bmatrix} X_p \\ X_s \end{Bmatrix} + \begin{bmatrix} K_{pp} & K_{ps} \\ - & - \\ K_{sp} & K_{ss} \end{bmatrix} \begin{Bmatrix} X_p \\ X_s \end{Bmatrix} = 0 \quad (3)$$

Elimination of the secondary degrees of freedom may be accomplished from the second of the above equations to give,

$$X_s = -T X_p$$

where, $T = (\lambda^2 M_{ss} - K_{ss})^{-1} (\lambda^2 M_{sp} - K_{sp}) \quad (4)$

Substituting Eq. (4) into the first of Eq. (3) yields the eigenvalue problem in the reduced degrees of freedom (X_p),

$$\lambda^2 (M_{pp} - M_{ps} T) X_p = (K_{pp} - K_{ps} T) X_p \quad (5)$$

Unfortunately the transformation matrix (T) involves the eigenvalue which is not known.

Standard methods of solving the eigenvalue problem may be used to find the solution of Eq. 5 provided a new transformation matrix (T) is computed each time a new estimate of the eigenvalue (λ) is found. Thus one would begin with the ($\lambda=0$) solution, determine the eigenvalue, calculate a new transformation based upon the updated eigenvalue and repeat the process until the eigenvalue no longer changes. Several methods of carrying out this iterative process have been considered. For some problems an almost linear relationship has been found to exist between the eigenvalue assumed in calculating the transformation matrix and the eigenvalue determined as the solution of Eq. (5). This would happen if ($\lambda^2 M_{ss}$) and ($\lambda^2 M_{sp}$) are small as compared with (K_{ss}) and (K_{sp}) respectively (see Eq. 4).

This suggests an interesting possibility. If ($\lambda^2 M_{ss} - K_{ss}$)⁻¹ is expanded about (K_{ss})⁻¹ and terms associated with (λ^4) are neglected as compared with terms associated with (λ^2).

$$T = K_{ss}^{-1} K_{sp} + \lambda^2 (-K_{ss}^{-1} M_{sp} + K_{ss}^{-1} M_{ss} K_{ss}^{-1} K_{sp}) \quad (6)$$

This definition of (T) has an advantage over (4) in that an inverse need not be recalculated at each step in the iteration on (λ).

This requirement for an iterative solution is eliminated if Eq. (6) is substituted into the first of (3) and again the (λ^4) terms neglected as compared to the (λ^2) terms to give,

$$\lambda^2 (M_{pp} - M_{ps} K_{ss}^{-1} K_{sp} - K_{ss}^{-1} M_{sp} + K_{ps} K_{ss}^{-1} M_{ss} K_{ss}^{-1} K_{sp}) X_p = (K_{pp} - K_{ps} K_{ss}^{-1} K_{sp}) X_p \quad (7)$$

Eigenvalues can be determined directly from Eq. (7) without a requirement for iteration. This is the eigenvalue problem solved using both Guyan and Modified Guyan Reduction. Since the full mass matrix in SAPV is diagonal the reduced mass matrix is,

$$M^* = M_{pp} + K_{ps} K_{ss}^{-1} M_{ss} K_{ss}^{-1} K_{sp} \quad (8)$$

The reduced stiffness matrix is,

$$K^* = K_{pp} - K_{ps} K_{ss}^{-1} K_{sp} \quad (9)$$

2.2 EIGENVALUE SOLUTION

The SAPV computer program uses a determinant search algorithm to solve for the eigenvalues of small problems and a iteration algorithm for large problems (Ref. 6). An attempt was made to use the subspace iteration algorithm for the reduced system but difficulties were encountered unless considerable care was taken in selecting the trial vectors. This difficulty occurred because the reduced mass matrix is full while the full mass matrix (used in SAPV) is diagonal.

A standard inverse iteration with Gram-Schmidt orthogonalization procedure (Ref. 6) is used. The inverse iteration method may be described as follows. The eigenvalue problem is,

$$\lambda^2 M^* X_p = K^* X_p \quad (10)$$

A trial vector (X_1) is selected and the following iteration performed so that (X_k) goes to a mode shape as the iteration index (k) increases,

$$\begin{aligned}
K^* \bar{X}_{K+1} &= M^* X_K \\
\lambda_{K+1}^2 &= \bar{X}_{K+1}^T K^* \bar{X}_{K+1} / C \\
X_{K+1} &= \bar{X}_{K+1}^T / C \\
C &= \bar{X}_{K+1}^T M^* \bar{X}_{K+1}
\end{aligned} \tag{11}$$

This iteration converges to the mode associated with the smallest eigenvalue. Modes higher than the first are determined by requiring all the vectors (X_k) be orthogonal to the lower modes already evaluated, thereby suppressing those modes in the iteration. Gram-Schmidt orthogonalization is used for this purpose and is done as follows when the i^{th} mode is being calculated

$$X = \bar{X} - \sum_{k=1}^{i-1} Z_k^T M^* \bar{X} \tag{12}$$

where, $\bar{x}$ = trial vector

x = trail vector mode orthogonal to all
of the lower modes Z_k

2.3 EXPANSION OF REDUCED EIGENVECTORS

The eigenvectors found in (2.2) are of a length equal to the number of reduced degrees of freedom. If member loads are to be calculated these eigenvectors must be expanded to include all degrees of freedom.

Guyan Reduction calculates the secondary (eliminated) degrees of freedom from,

$$X_s = -K_{ss}^{-1} K_{sp} X_p \tag{13}$$

The Modified Guyan Reduction calculates the secondary degrees of freedom from Eq. (6) or if the original mass matrix is diagonal,

$$X_s = - \left[K_{ss}^{-1} K_{sp} + \lambda^2 (K_{ss}^{-1} M_{ss} K_{ss}^{-1} K_{sp}) \right] X_p \quad (14)$$

3.0 SAPV MODIFICATIONS

The changes discussed in Section 2.0 have been incorporated into the SAPV program at Brookhaven National Laboratory (BNL). The program is operational on the BNL CDC-7600 computer system. The main program in SAPV is modified and thirteen new subroutines are added. All of the SAPV subroutines are unaltered. A listing of the modified main program and the thirteen new subroutines is contained in Appendix A. Detailed instructions for using the BNL SAPV program incorporating these changes is given in Appendix B. In this section of the report the general form of the modification, input data requirements, and a description of each of the programs are given.

3.1 GENERAL FORM OF MODIFICATION

The SAPV Computer Program is modified so that the total static degrees of freedom may be reduced prior to performing a dynamic analysis. In the previous section of the report two reduction methods are developed:

- (1) Guyan Reduction which is the standard reduction used in most structural software packages. (Eq. 7) is the eigenvalue problem and (Eq. 13) is used to expand the modes after solution of the eigenvalue problem.

- (ii) Modified Guyan Reduction which uses the same (Eq. 7) for solution of the eigenvalue problem. (Eq. 14) is used to expand the modes rather than (Eq. 13) used in the Guyan Reduction.

These two reduction methods are incorporated into SAPV as indicated on Figure 1. As may be seen the modification is made by interrupting the normal flow of computations in SAPV at two locations:

- (i) SAP5.88*- The type of reduction (or no reduction) is read in together with the primary degrees of freedom to be retained. These data are then used to rearrange the equation numbers (defined in the ID matrix) so that the primary degrees of freedom occur first.
- (ii) SAP5.296*- The reduction is performed and the eigenvalue solution obtained for the reduced system. The modes are expanded to the total degrees of freedom and stored in the appropriate locations so that the normal SAP response analysis may be performed to complete the solution.

Each of the subroutines listed in Figure 1 are discussed in Section 3.3.

3.2 INPUT DATA REQUIREMENTS

Only one addition is required to be made to the normal SAPV data input requirements as given in Ref. 5. After the "III Nodal Point Data" and before the "IV Element Data", the following card(s) are inserted:

*Refers to line numbers in the BNL SAPV Program

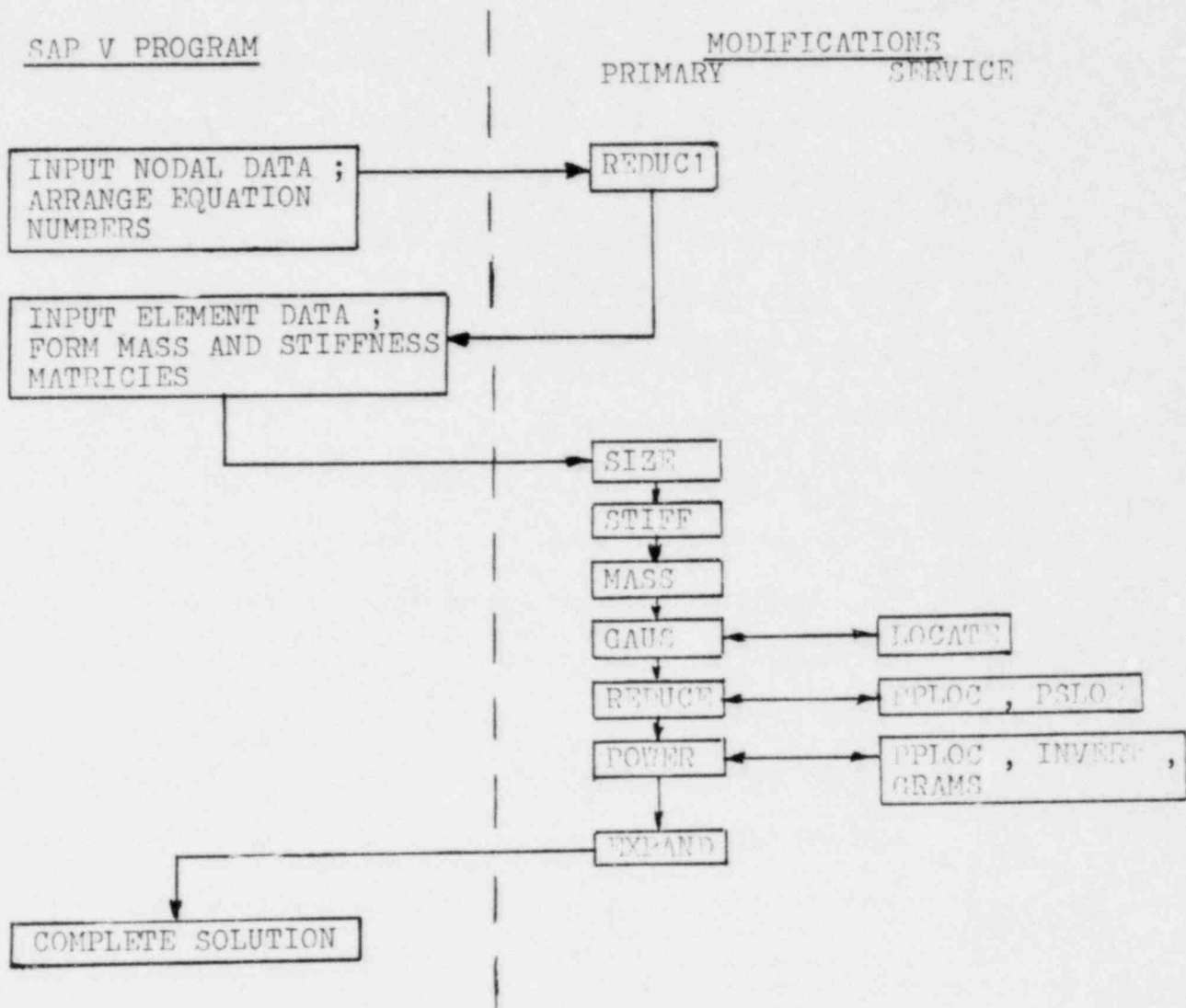


Fig. 1 GENERAL FORM OF SAP V MODIFICATION

<u>No of Cards</u>	<u>Format</u>	<u>Variables</u>	<u>Definition</u>
1	I5	NTYP	NTYP = 0; No Reduction (Standard SAP run) = 2; Guyan Reduction = 3; Modified Guyan Reduction
$\frac{2 \times \text{NRDOF} + 1}{14}$	14I5	NRDOF, (MASN(I), MADF(I), I = 1, NRDOF)	NRDOF= Number of Primary Degrees of Freedom MASN = Node Number of Primary Degree of Freedom I MADF = Direction (1-6) of Primary Degree of Freedom I

Also the Card VIII "Mode Shapes and Frequencies" is omitted if NTYP is not equal to 0.

3.3 DESCRIPTION OF SUBROUTINES

Before describing the subroutines common variables are defined and storage allocations are discussed.

3.3.1 STORAGE ALLOCATION

Most of the data required in Core is stored in vector (A) in the SAP main program and only put into matrix from within the subroutines so that the size of the matrices may be adjusted to the required sizes.

All data stored in (A) at BNL is put into Large Core Memory (Level 2). The data in (A) at the call to each of the primary subroutines is listed in Table 1. Control parameters are defined in Table 2. Note that the storage in (A) is rearranged between the subroutines REDUCE and POWER. Tape storage is discussed where applicable in the subroutines.

3.3.2 REDUC1

The additional input data described in Section 3.2 is read into the program through this subroutine. The SAP generated matrix of equation numbers (ID) is read from Tape 8 and reordered so that the primary degrees of freedom are the first numbered equations. The primary and secondary degree of freedom equation numbers are kept in the order that they appeared in the original ID matrix. The new ID matrix is written on Tape 8. The SAP band width minimizer occurs after this location in the program. It should not be used or it will upset the equation number reordering that is performed here. (Therefore MINBND should be read in as 0 on the Master Control Card).

3.3.3 SIZE

The size parameters required for the solution are generated and printed in this routine. The following parameters, which are defined in Table 2, are calculated here: NST, NRDOF, NS, NS1, NS2, NS3, NOT, and MBS. In addition the required size of the vector (A) used in the various phases of the program is determined and printed.

Table 1
DATA STORAGE IN VECTOR A

Subroutine	Variable	Size	Location in A
REDUC1	ID = Matrix giving equation number	6*NUMNP	1
	IDN = Revised ID	6*NUMNP	1+6*NUMNP
SIZE	X = One band of stiffness matrix	NRQB*MBAND	1
	XL = One band of load matrix	NRQB	1+NRQB*MBAND
STIFF	K _{pp} see Sect. 2.0	NS1	1
	K _{ps} see Sect. 2.0	NS2	NR2=1+NS1
	K _{ss} see Sect. 2.0	NSEQ*MBS	NR3=NR2+NS2
	X = Dummy K _{ss}	NSEQ*MBS	NR4=NR3+NSEQ*MBS
	XL = Dummy Load	NSEQ	NR5=NR4+NSEQ*MBS
MASS	XM = Diagonal Mass	NST	NR3
GAUS	A1 = One block K _{ss}	NSEQ*MBS	NR4=NR3+NST
	A2 = Second block	NSEQ*MBS	NR5=NR4+NSEQ*MBS
	B = Dummy load	NST	NR6=NR5+NSEQ*MBS
	G = Dummy load	NST	NR7=NR6+NST
REDUCE	K _{pp} and K*	NS1	1
	K _{ps}	NS2	NR2
	XM	NST	NR3
	M*	NS1	NR4
	XSS = Dummy	NS	NR5=NR4+NS1
	T1 (one row)	NRDOF	NR6=NR5+NS
	TO (one block)	NTO*NRDOF	NR7=NR6+NRDOF

-continued-

Table 1 cont'd

POWER	K*	NS1	1
	M*	NS1	NR2=1+NS1
	EV = Eigenvectors	NRDOF*MODE	NR3=NR2+NS1
	XD = Factored K	NRDOF*NRDOF	NR4=NR3+NRDOF*MODE
	B = Dummy	NRDOF	NR5=NR4+NRDOF*NRDOF
	BB = Dummy	NRDOF	NR6=NR5+NRDOF
	EDUM = Eigenvalues	MODE	NR7=NR6+NRDOF
EXPAND	EV	NRDOF*MODE	NR3
	TO	INTO*NRDOF	NR4=NR3+NRDOF*MODE
	T1	NRDOF	NR5=NR4+INTO*NRDOF
	ESAP = Block of expanded nodes	NRQB*MODE	NR6=NR5+NRDOF

Table 2
DEFINITION OF CONTROL PARAMETERS

Variable	Definition
NUMNP	Number of nodes
NEQB	Number of equations in one block of stiffness matrix
MBAND	Bandwidth of stiffness matrix
NS1	Number of elements in upper half of K_{pp}
NS2	Number of elements in K_{ps}
NSEQ	Number of equations in block of K_{ss}
MBS	Bandwidth of K_{ss}
NST	Number of unrestrained degrees of freedom
NS	Number of secondary degrees of freedom
NRDOF	Number of primary degrees of freedom
NT0	Number of equations in one block of T_0
MODE	Number of modes

3.3.4 STIFF

The stiffness matrix is generated in the normal SAP routines and stored banded and blocked on Tape 4. This tape is read and (K_{pp}) and (K_{ps}) are formed and kept in Large Core Memory (see Table 1). The secondary degree of freedom stiffness (K_{ss}) is also determined and stored banded and blocked on a new Tape 16.

3.3.5 MASS

The diagonal mass matrix as stored on Tape 9 is read and used to form the vector of all masses. This vector remains in Large Core Memory.

3.3.6 GAUS

This subroutine forms (K_{ss}^{-1}) using elimination as described in Ref. 6. It operates essentially the same as SESOL in the standard SAPV except that it generates "load vectors" required to compute one column of (K_{ss}^{-1}) at a time. The full column of (K_{ss}^{-1}) is stored in Tape 17. The elimination operation is set to operate on a banded symmetric matrix stored in block form. In this instance the K_{ss} matrix is read in from Tape 16.

The transfer list to this subroutine contains the parameters NT1, NT2, NT3, and INDEX. if INDEX = 1 the inverse of the matrix stored on Tape NT1 is calculated and stored one column at a time on tape NT3. If INDEX = 2 the matrix stored on Tape NT1 is factored and the result is stored on Tape NT2. If INDEX = 3 the equation $Ax = B$ is solved. (A) is stored on NT1 and B is transferred through the list. The solution (x) is stored in (B). The parameters have the following values in this application.

NT1=16
NT2=18
NT3=17
INDEX=1

3.3.7 REDUCE

The transformation matrices and the reduced stiffness and mass matrices are determined. The following sequence of calculations are made:

$$(i) \quad T_0 = K_{ss}^{-1} * K_{sp} \dots \text{(first term in Eq. 6)}$$

The transformation matrix T_0 is calculated one block at a time (NTO rows) and the result stored on Tape 18. K_{ss}^{-1} is read from Tape 17.

$$(ii) \quad T_1 = K_{ss}^{-1} * M_{ss} * T_0 \dots \text{(second term in Eq. 6)}$$

The transformation is calculated one now at a time and stored on Tape 16. It is written over and destroys K_{ss} .

$$(iii) \quad K^* = K_{pp} - K_{ps} * T_0$$

The reduced stiffness matrix (K^*) is calculated and written over K_{pp} which is no longer needed.

$$(v) \quad M^* = M_{pp} + K_{ps} * T$$

The reduced mass matrix (M^*) is calculated and the upper half stored in a new location.

3.3.8 POWER

The power method using inverse iteration is used to calculate the lowest (MODE) modes for the reduced mass and stiffness matrices. The iterations are continued until the change in eigenvalue (λ^2) is less than 0.01%. The error at each iteration is printed. After all eigenvalues have been generated, they are written onto Tape 7 as required for the remainder of the SAPV program.

3.3.9 EXPAND

The eigenvectors are expanded to include all unrestrained degrees of freedom according to either Eq. (13) or (14) depending on whether the reduction is Guyan or the Modified Guyan. The full vectors are then stored on Tape 7 after the eigenvalues.

3.3.10 LOCATE

This subroutine locates the (i,j) element in K_{ss} when K_{ss} is stored in block form.

3.3.11 PPLOC

This subroutine locates the (i,j) element in K_{pp} .

3.3.12 PSLOC

This subroutine locates the (i,j) element in K_{ps} .

3.3.13 INVERT

This subroutine factors the reduced stiffness matrix (K^*) as required in the POWER subroutine. It operates the same as the GAUS routine except for the format of the matrix being reduced.

3.3.14 GRAMS

This subroutine performs Gram-Schmidt orthogonalization as described in Section 2.2. At each iteration in POWER the current eigenvector is made orthogonal to all eigenvectors previously calculated.

4.0 SAMPLE PROBLEMS

The modified SAPV computer program is used to study four sample problems: a ten story building; the ten story building with a relatively light structure attached to the top story; a nuclear power plant piping system; and a nuclear power plant containment building. For each of the problems solutions are obtained; with no reduction; with Guyan Reduction; and with the Modified Guyan Reduction.

4.1 TEN STORY BUILDING

The first problem considered is the ten story by three bay frame as shown on Figure 2. The node numbers used for the SAP data are shown by each of the nodes and the element numbers are shown underlined. All of the columns (elements number 1 through 40) have a cross-sectional area of 60 square inches and a moment of inertia of 1500 inches.⁴ The beams (elements 41 through 70) have a cross-sectional area of 100 square inches and a moment of inertia of 1000 inches.⁴ The material of all members has a modulus of elasticity of 30,000 ksi and a density of 490 pounds per cubic foot.

The columns are fixed at the base (nodes 1 through 4) and the response spectrum shown in Figure 3 (solid) is input in the horizontal direction. The frame is solved with no reduction and then with both Guyan and Modified Guyan Reduction. For each reduction method four solutions are obtained retaining 3, 4, 6 and 10 degrees of freedom for the dynamic analysis. In all cases the degrees of freedom retained are in the horizontal direction; with the specific degrees of freedom given as:

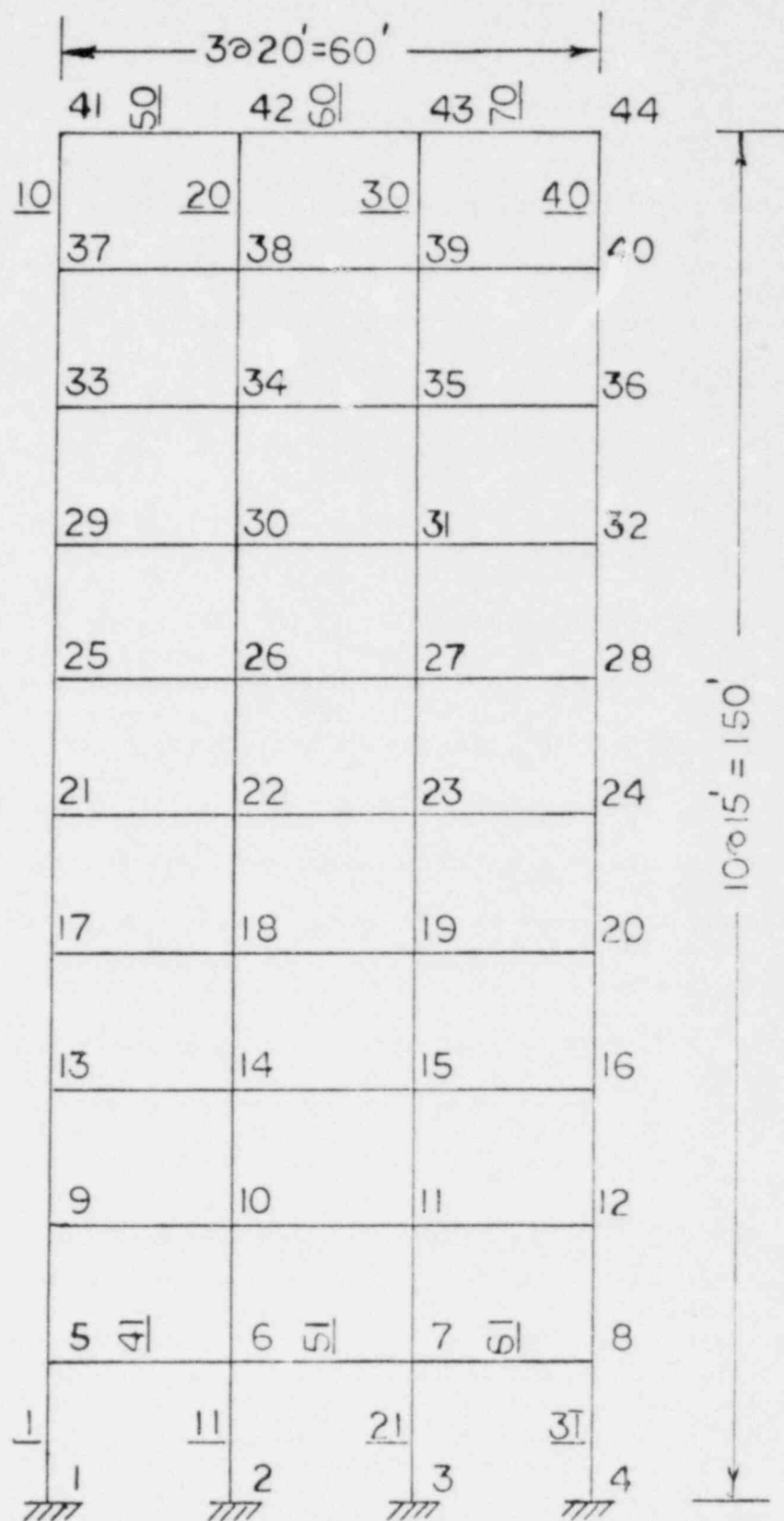


Fig. 2 TEN STORY FRAME

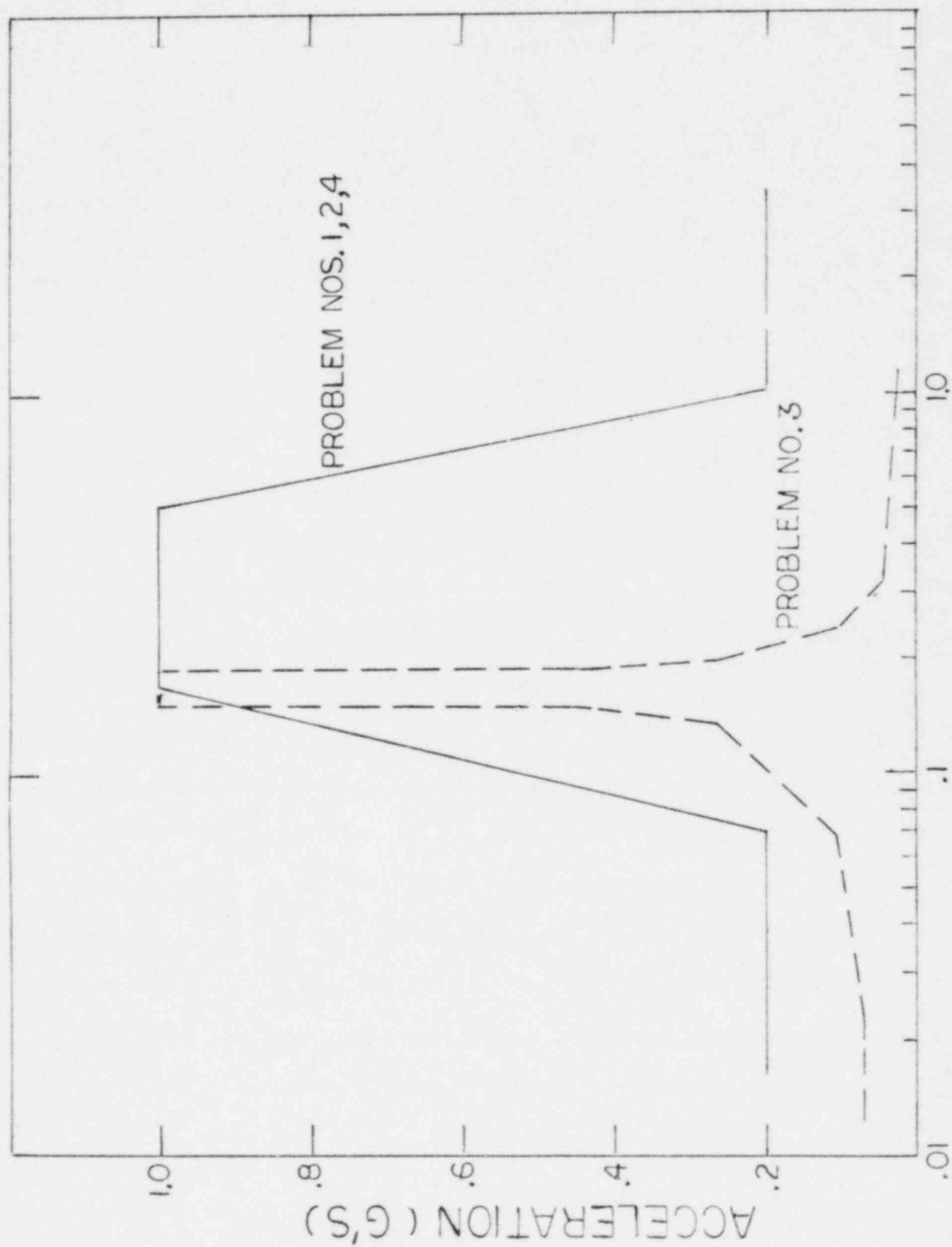


Fig. 3 BASE RESPONSE SPECTRA

Number of Dynamic Degrees of Freedom	Degrees of Freedom - Horizontal Displacement at Nodes
3	11, 26, 41
4	12, 23, 34, 41
6	10, 17, 28, 35, 38, 41
10	6, 9, 16, 19, 22, 25, 32, 35, 38, 41

Three modes are included in the solution and the results for frequencies, lower story columns (1 and 11), upper story columns (10 and 20), lower story beams (41 and 51) and upper story beams (50 and 60) are given in Table 3. As may be seen the results for frequencies are quite good regardless of the type of reduction or number of dynamic degrees of freedom retained in the model. Of course this would not hold true if any particular mode could not be reasonably represented in terms of the degrees of freedom retained. This is the reason for the departure in the mode 3 frequency as calculated with Guyan Reduction from the exact solution.

So that the errors in member bending moments may be evaluated, the percentage errors in four of the columns and four of beams are computed from the data in Table 3 and given in Table 4. The Modified Guyan Reduction gives excellent results (less than 2% error) for all reductions, even when only 3 degrees of freedom are retained. Guyan Reduction gives equally good results for the lower story members (11 and 41) but very poor results for the upper story members unless 6 or more degrees of freedom are retained. This variation in accuracy with location in the structure would make it difficult to review data for a Guyan Reduction run to determine the reliability of the results. It also appears that the Modified Guyan Reduction gives errors of one sign (i.e., a lower bound) while Guyan Reduction gives errors which can be positive or negative.

Table 3

Ten Story Frame Frequencies and Member Bending Moments

Type of Reduction	No. of Dynamic Degrees of Freedom	FREQUENCIES (CPS)			COLUMN MOMENTS				BEAM MOMENTS			
		MODE 1	MODE 2	MODE 3	1	10	11	20	41	51	50	60
Exact		0.86	2.64	4.61	1520.	150.	1738.	315.	1390.	1196.	150.1	193.1
G	3	0.87	2.85	4.85	1535.	344.	1756.	630.1	1402.	1206.	334.	340.7
U	4	0.86	2.71	5.07	1521.	198.7	1740.	387.4	1393.	1198.	198.7	225.3
Y	6	0.86	2.66	4.72	1514.	151.2	1732.	314.1	1391.	1197.	151.2	192.7
A												
N	10	0.86	2.64	4.61	1520.	149.9	1738.	312.5	1390.	1196.	149.9	192.1
M.												
G	3	0.87	2.85	4.85	1496.	148.7	1710.	311.0	1367.	1177.	148.7	190.6
U	4	0.86	2.71	5.07	1513.	149.6	1730.	313.6	1383.	1190.	149.6	192.6
Y	6	0.86	2.66	4.72	1519.	149.7	1736.	314.0	1388.	1195.	149.7	192.9
A												
N	10	0.86	2.64	4.61	1521.	149.8	1738.	314.2	1390.	1196.	149.8	193

Table 4
%Errors in Member Moments

No. of Deg. of Freedom	Guyan Reduction Member No.				Modified Guyan Reduction Member No.			
	11	41	20	60	11	41	20	60
3	1.0	0.9	100.0	76.4	-1.6	-1.6	-1.3	-1.3
4	0.1	0.2	23.0	16.7	-0.5	-0.5	-0.4	-0.3
6	-0.3	0.1	-0.3	-0.2	-0.1	-0.1	-0.3	-0.1
10	0	0	-0.8	-0.5	0	0	-0.2	0

The running time for both Guyan and Modified Guyan Reduction solution are about the same (Modified Guyan is about 0.5% slower). The solution times on the BNL CDC 7600 computer are 6.95, 7.28, 7.30 and 7.90 seconds respectively for the 3, 4, 6, and 10 degree of freedom problem. The computer implications of the reduction schemes are discussed in Section 4.5.

4.2 SMALL STRUCTURE ADDED TO TEN STORY BUILDING

The small structure shown in Figure 4 is added to the central bay of the top floor of the ten story building. The dashed lines represent the main structural frame as described on Figure 2 while the solid lines are the added structure. The new structure is attached to nodes 38 and 39 of the main frame. Member numbers in the new frame are underlined and node numbers shown next to each node.

All members of the added frame have a cross-sectional area of 60 square inches and a moment of inertia of 3 inches.⁴ The relatively large cross-sectional area is used to simulate added mass to the small structure. Material properties of the small structure are the same as the main frame. In a separate computer run the small structure, is found to have frequencies of 1.78 cps and 5.86 cps when isolated from the main frame. Therefore its modes of vibration would be expected to interact with the modes of the ten story building.

The response spectrum of Figure 3 is again used as input at the base of the ten story frame and solutions obtained with no reduction and with both Guyan and Modified Guyan Reduction.

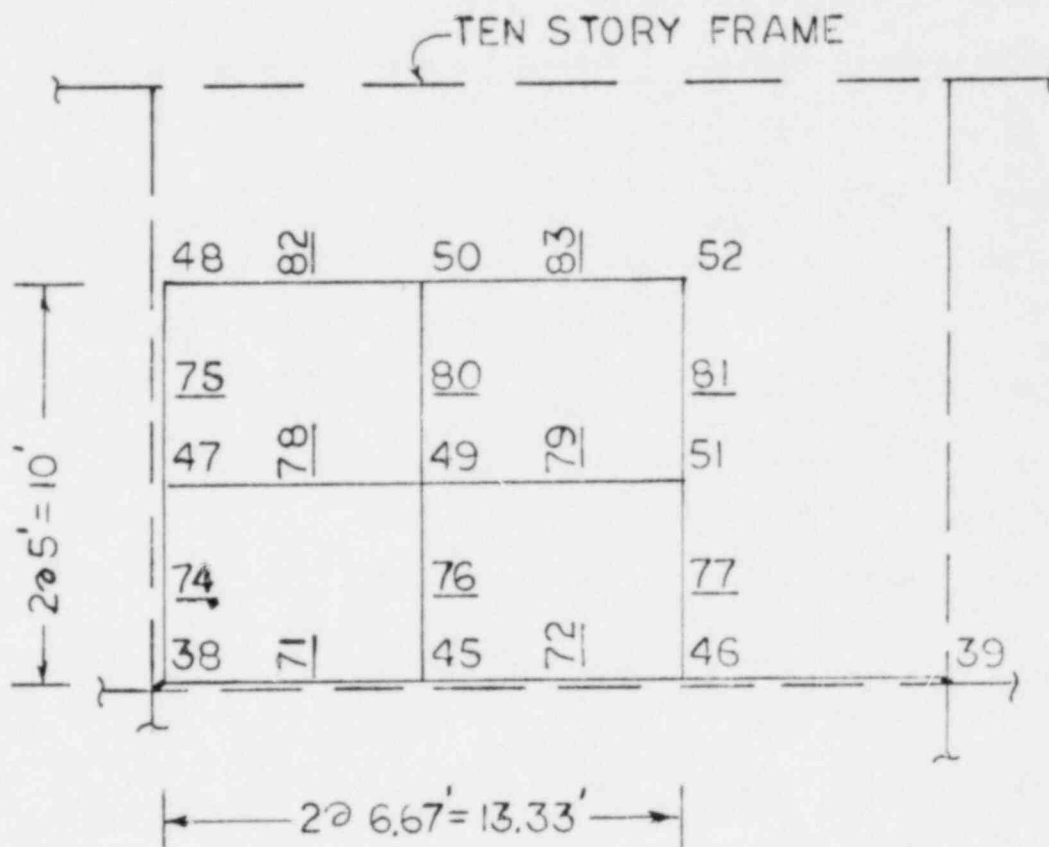


Fig. 4. SMALL STRUCTURE

Six dynamic degrees of freedom in the main structure are retained for all reduction runs since these are shown (Table 4) to give negligible errors in the main frame. Four different reductions are used for the degrees of freedom in the small structure as defined in Table 5.

Table 5
Reduction in Small Structure

Case	Degrees of Freedom Retained*
1	50 (horizontal)
2	50 (horizontal), 49 (horizontal)
3	48(horizontal and vertical), 52 (horizontal and vertical)
4	All horizontal and vertical
*The horizontal degrees of freedom at the following nodes in the main structure are retained: 10, 17, 28, 35, 38, 41	

The results of the computer solution are shown on Tables 6 and 7. Five modes are considered and the calculated frequencies are shown in Table 6. The results for the first four modes are good for all of the cases while there is considerable variability in the fifth mode.

The member moments are shown on Table 7 and percentage errors for each of the reduction runs is computed and tabulated in Table 8. As may be seen the Modified Guyan Reduction gives significantly better results than the Guyan Reduction. It may again be noted that the errors for Guyan Reduction vary significantly from member to member for the same reduction scheme while the errors are much closer to the same when the Modified Reduction is used.

Table 6

Frequencies in Frame with Small Structure

Case (Table 5)	Frequencies (CPS)				
	Mode 1	Mode 2	Mode 3	Mode 4	Mode 5
Exact	.81	1.98	2.67	4.58	4.88
1	.82	2.02	2.69	4.71	7.14
2	.82	1.90	2.69	4.71	6.58
3	.82	1.82	2.69	4.71	4.99
4	.82	1.98	2.68	4.70	4.88

Table 7
Bending Moments in Small Structure

Reduction	Case 5 (Tables)	Moments in Member No.												
		71	72	73	74	75	76	77	78	79	80	81	82	83
Exact		23.2	9.1	20.7	75.0	21.1	32.2	28.2	52.7	74.4	30.7	11.7	22.8	11.7
G U A N	1	19.2	6.5	16.9	60.1	28.1	25.7	22.1	51.1	27.8	42.2	19.9	28.1	19.9
	2	21.8	10.8	19.4	74.8	20.3	32.7	28.8	51.7	26.1	31.1	12.9	21.6	12.9
	3	20.3	5.3	18.0	60.1	28.7	25.2	21.5	51.8	26.3	41.8	18.8	28.7	18.8
	4	23.1	9.0	20.6	74.9	21.1	32.1	28.2	52.6	24.4	30.7	11.6	22.8	11.5
M. G U Y A N	1	22.4	8.7	20.0	72.4	20.6	31.1	27.3	51.1	23.9	30.1	11.6	22.4	11.6
	2	23.1	9.2	20.6	75.1	21.1	32.3	28.3	52.7	24.6	30.8	11.8	22.8	11.8
	3	22.5	8.8	20.1	72.4	20.7	31.1	27.3	51.2	23.8	30.1	11.5	22.4	11.5
	4	23.2	9.0	20.7	75.1	21.1	32.2	28.3	52.8	24.5	30.8	11.7	22.9	11.7

Table 8

%Errors in Small Structure Moments

Case (Tables)	Guyan Reduction Member No.					Modified Guyan Reduction Member No.				
	72	76	79	80	83	72	76	79	80	83
1	-28.6	-20.2	+13.9	+37.5	+70.1	-4.4	-3.4	-2.0	-2.0	-0.9
2	+18.7	+1.6	+7.0	+1.3	+10.3	+1.1	+0.3	+0.8	+0.3	+0.9
3	-41.8	-21.7	+7.8	-14.3	+60.7	-3.3	-3.4	-2.5	-2.0	-1.7
4	-1.1	-0.3	0	0	-0.9	-1.1	0	+0.4	+0.3	0

4.3 PIPING SYSTEM

The piping system shown in Figure 5 is considered. The details of this system are taken from Reference 7 (Piping Benchmark Problem No. 5). Elements 1 through 19 are 14 inch diameter pipes with a wall thickness of 0.438 inches. Element 20 through 32 are 12 inch diameter pipes with a wall thickness of 1.312 inches. Weights of 3864 pounds are placed at nodes 19, 22, and 27 to simulate valve weights and to test the effect of lumped masses. The pipe supports are modeled with boundary elements having stiffnesses of 10^7 , 10^7 , 10^7 , 450, 800 and 600 pounds per inch for elements between nodes 7-37, 18-38, 18-39, 13-34, 25-35, and 31-36 respectively.

The piping system is subjected to the response spectrum shown on Figure 3 (dashed). The response spectrum is input in the two horizontal directions (X,Z) and 0.67 of the spectrum is input in the vertical direction (Y). An exact solution is obtained and solutions are found using 7 sets of reduced degrees of freedom with both Guyan and the Modified Guyan Reductions. The primary degrees of freedom for each of the reduction schemes is shown in Table 9. Note that there are 186 unrestrained degrees of freedom but only the 93 translational degrees of freedom have mass attached.

Five modes are included in the calculation with the frequencies shown in Table 10. The same frequencies are found with Guyan and Modified Guyan Reduction.

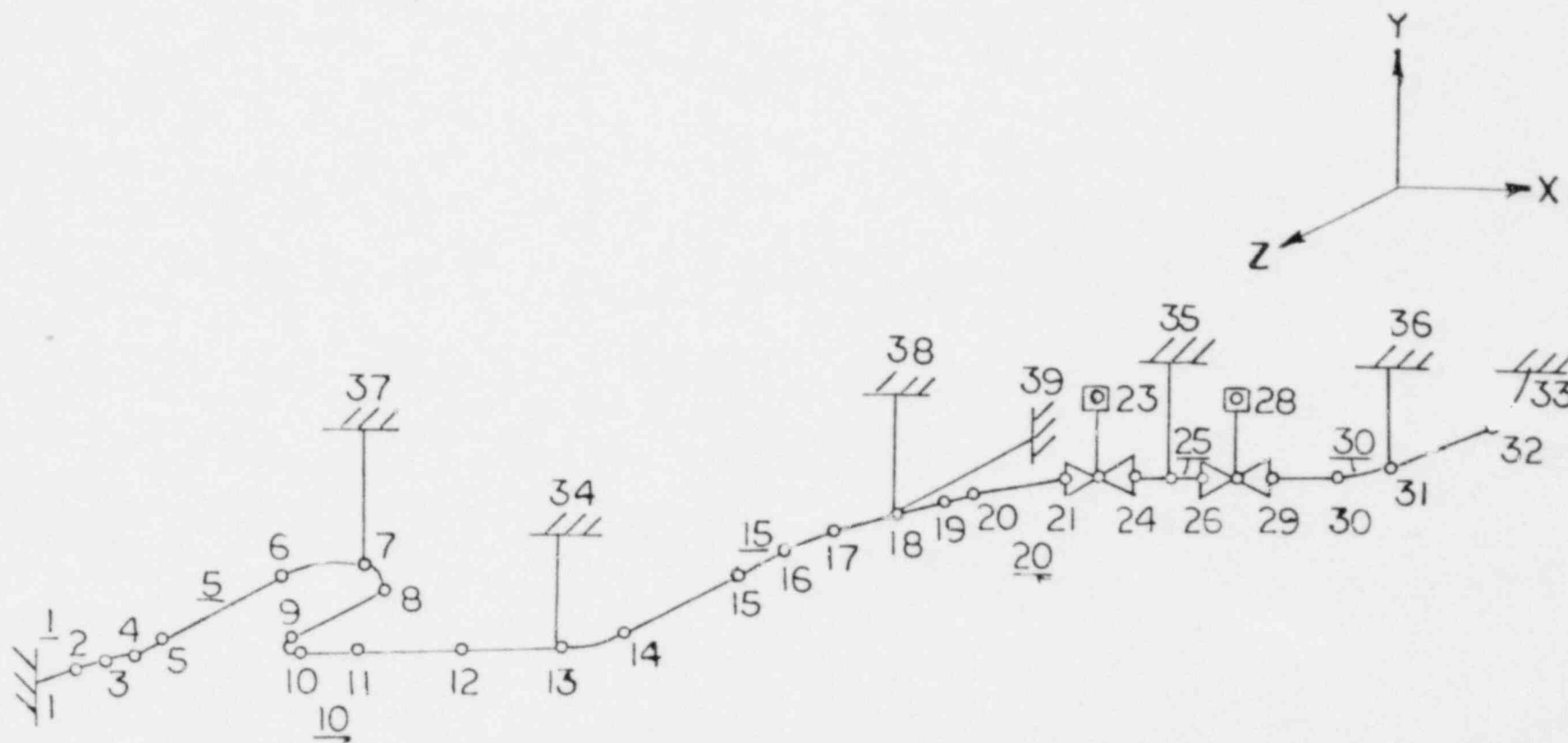


Fig. 5 PIPING SYSTEM

Table 9

Reduction Schemes for Piping System

Case	No. of Primary Degrees of Freedom	Degrees of Freedom Retained
A	3	9-X;9-Y;9-Z
B	3	9-X;15-Y;22-Z
C	5	9-X;15-Y;18-X;22-X;27-Z
D	10	5-Y;9-X;9-Z;13-Z;15-Y;18-X;22-X;22-Y;27-Z;31-Z
E	15	5-X;5-Y;7-Z;9-X;9-Z;13-Z;15-X;15-Y;18-X;22-X;22-Y;27-X;27-Z; 31-X;31-Z
F	21	5-X;5-Y;7-X;7-Z;9-X;9-Y;9-Z;13-X;13-Z;15-X;15-Y;15-Z;18-X; 22-X;22-Y;22-Z;27-X;27-Y;27-Z;31-X;31-Z
G	26	5-X;5-Y;5-Z;7-X;7-Z;9-X;9-Y;9-Z;11-X;11-Y;11-Z;13-X;13-Z; 15-X;15-Y;15-Z;18-X;22-X;22-Y;22-Z;27-X;27-Y;27-Z;31-X; 31-Z;31-Y

Table 10
Piping System Frequencies

Reduction Type (Table 9)	Frequencies (CPS)				
	Mode 1	Mode 2	Mode 3	Mode 4	Mode 5
EXACT	5.71	6.14	12.85	13.32	14.90
A	6.21	7.11	14.70	-	-
B	6.30	6.32	16.40	-	-
C	6.28	6.32	14.30	20.2	42.1
D	6.18	6.22	13.90	14.20	15.70
E	6.15	6.20	13.70	14.00	15.60
F	5.86	6.14	13.00	13.30	15.00
G	5.75	6.14	12.90	13.30	14.90

The frequency computations for the piping system may be seen to behave quite differently than for the frame structures. Ten percent errors occur in the first mode frequency until 21 dynamic degrees of freedom are retained.

The forces in all of the piping members for the cases run are listed in Table 11. The P column is the maximum thrust force in the pipe element while the M column is the maximum bending moment in the pipe. There is a great deal of variability in the results but it may be noted that large errors result when Guyan Reduction is used while Modified Guyan Reduction significantly reduces the errors. To illustrate some of these effects percentage errors for "typical" elements are shown in Table 12.

Table 11

PIPING FORCES

Model		MEMBER NUMBER											
		1		2		3		4		5		6	
		P	M	P	M	P	M	P	M	P	M	P	M
		Exact	1558	171500	1560	143600	1847	137800	1847	134300	1016	73300	965
G U Y A N	MODEL A	1277	193200	1278	156600	1676	148200	1676	143500	1577	75400	1577	496690
	MODEL B	1505	127800	1506	110800	1667	107400	1667	105900	520	59990	520	58020
	MODEL C	1494	126900	1494	110000	1655	106700	1655	105100	516	59550	516	57600
	MODEL D	1069	158400	1070	127900	1381	117000	1381	117000	1346	60940	1346	42610
	MODEL E	1966	158300	1967	129900	2251	124000	2251	192560	1005	53260	1005	39060
	MODEL F	1775	171300	1776	141400	2046	138800	2046	136000	960	73040	960	53170
	MODEL G	1612	174300	1614	145900	1905	139700	1905	136500	857	74070	857	53690
M O D. G U Y A N	MODEL A	1551	171800	1522	143700	1843	137600	1843	134400	1020	73300	970	51940
	MODEL B	1487	158000	1488	132900	1747	124700	1746	124700	894	68380	853	50580
	MODEL C	1455	154900	1456	130300	1710	124900	1710	122200	877	67050	837	49470
	MODEL D	1296	140700	1297	117700	1531	112700	1531	110100	836	60910	793	60910
	MODEL E	1296	141500	1297	118500	1534	113500	1532	110800	839	60560	795	60560
	MODEL F	1574	173500	1575	145200	1867	139100	1866	135800	1031	74180	980	52820
	MODEL G	1577	174100	1579	145700	1871	139500	1871	136300	1034	74410	983	52930

Table 11 (cont'd)

PIPING FORCES

Model		MEMBER NUMBER											
		7		8		9		10		11		12	
		P	M	P	M	P	M	P	M	P	M	P	M
Exact		1302	80730	729	66880	585	17660	501	29440	192	45870	166	52230
G U Y A N	MODEL A	1676	96080	1577	80910	324	11760	1071	31220	1071	50290	1071	62820
	MODEL B	1667	75960	520	51830	520	38700	1005	12580	1005	43520	1005	63740
	MODEL C	1655	75420	516	51480	516	37890	962	11850	962	42580	962	62680
	MODEL D	1381	79850	1346	67720	340	16480	527	23030	527	40660	527	53190
	MODEL E	1327	77930	1383	66810	309	14460	471	22760	471	39090	471	55290
	MODEL F	1095	77740	681	69340	364	18750	449	25060	449	42420	449	55440
	MODEL G	1076	81030	721	69780	536	18480	671	32590	101	43990	101	53640
M O D. G U Y A N	MODEL A	1294	80800	733	67040	586	14530	475	28280	155	46220	176	52440
	MODEL B	1277	76120	681	62000	576	11710	498	22310	199	43850	188	52470
	MODEL C	1252	74520	669	60680	566	13340	488	23990	197	43110	183	51570
	MODEL D	1090	67280	589	56610	481	15740	439	33520	177	49780	143	49780
	MODEL E	1097	67730	588	56280	483	15210	436	33330	171	49520	143	49520
	MODEL F	1316	81500	737	67570	591	19160	510	30480	202	46450	176	52890
	MODEL G	1319	81730	741	67760	593	19040	510	30590	201	46650	174	53070

Table 11 (cont'd)

PIPING FORCES

Model		MEMBER NUMBER											
		13		14		15		16		17		18	
		P	M	P	M	P	M	P	M	P	M	P	M
Exact		395	52230	161	45820	182	19800	204	19690	516	66870	1049	70290
G U Y A N	MODEL A	1071	61020	324	47110	324	19090	324	26360	973	71620	1383	72970
	MODEL B	1005	65290	519	53680	519	26260	519	18010	1144	67610	1110	67680
	MODEL C	962	64470	516	53530	516	30850	516	16510	1137	81590	2608	81670
	MODEL D	527	55730	426	38710	426	38710	426	19110	397	116100	1586	116200
	MODEL E	471	55290	528	38630	528	38630	528	18590	978	115200	1477	115300
	MODEL F	597	55440	309	44780	578	21970	578	10270	819	67530	1295	71470
	MODEL G	584	53640	268	45190	392	20640	392	15080	532	67680	1288	71660
M O D. G U Y A N	MODEL A	416	52440	77	45180	108	14830	134	4638	504	67370	860	70580
	MODEL B	408	52470	56	46890	86	22060	120	22650	542	65390	965	68900
	MODEL C	396	51570	65	46170	101	25520	135	26250	538	64760	1037	68050
	MODEL D	334	47450	140	37400	148	34140	160	34090	415	71900	862	74540
	MODEL E	338	47270	43	37430	154	34170	168	34070	419	71890	869	74240
	MODEL F	404	52890	170	46540	192	19100	213	19070	510	67940	1077	71300
	MODEL G	403	53070	172	46660	192	20440	213	20430	518	68210	1079	71590

Table 11 (Cont'd)

PIPING FORCES

Model		MEMBER NUMBER											
		19		20		21		22		23		24	
		P	M	P	M	P	M	P	M	P	M	P	M
Exact		1317	78150	1321	81610	1254	81390	72	4047	1430	78040	1443	64550
G U Y A N	MODEL A	1331	75220	1381	75520	1071	70980	0	0	1071	53590	1071	36210
	MODEL B	1106	68140	1108	72920	1005	102400	0	0	1005	102400	1005	77020
	MODEL C	2607	78100	2603	76970	3150	78110	0	0	2584	67600	2584	61190
	MODEL D	1588	109400	1586	94710	1671	71140	0	0	2004	61000	2004	53600
	MODEL E	1480	108400	1479	75280	1424	71030	0	0	1496	60830	1496	53780
	MODEL F	1294	79140	1294	82440	1212	82460	0	0	1447	80970	1447	66320
	MODEL G	1288	79400	1288	82550	1204	82590	0	0	1445	80760	1445	66080
M O D G U Y A N	MODEL A	1066	77460	1069	80100	1147	78910	8	2292	1093	70150	1090	52290
	MODEL B	1095	77150	1098	77380	1130	78630	44	3677	1074	77120	1072	62930
	MODEL C	1344	76230	1348	75780	1230	77630	51	3664	1528	77630	1549	67360
	MODEL D	1081	78370	1084	75080	1057	67860	88	5035	1187	65090	1198	53780
	MODEL E	1118	77940	1122	74490	1102	68110	93	5002	1288	64790	1302	53400
	MODEL F	1336	79170	1341	82310	1250	82260	66	3524	1422	79450	1436	66150
	MODEL G	1339	79490	1344	82480	1295	82440	67	3504	1424	79460	1437	66060

Table 11 (Cont'd)

PIPING FORCES

Model		MEMBER NUMBER											
		25		26		27		28		29		30	
		P	M	P	M	P	M	P	M	P	M	P	M
Exact		1454	57430	1468	51970	68	3718	1781	47670	1800	41430	1815	42340
G U Y A N	MODEL A	1071	25480	1071	14930	0	0	1071	19270	1071	29400	1071	33580
	MODEL B	1005	61450	1005	46270	0	0	1005	22980	1005	26050	1005	30700
	MODEL C	2584	61010	2584	67090	0	0	2584	67090	2584	50790	2584	49820
	MODEL D	2004	50950	2004	52850	0	0	2004	52850	2004	39330	2004	41060
	MODEL E	1496	51610	1496	55380			1963	55380	1963	42850	1963	43070
	MODEL F	1447	58970	1447	53760	0	0	1827	50850	1827	42180	1827	42980
	MODEL G	1445	58690	1445	53420	0	0	1820	50390	1820	42000	1820	42890
M O D. G U Y A N	MODEL A	1088	41010	1085	29760	5	1079	1030	11740	1027	25390	1024	31660
	MODEL B	1071	54400	1070	46570	13	2552	1082	36280	1084	27530	1086	31200
	MODEL C	1565	62440	1587	59200	21	3615	2042	57700	2069	47240	2090	46530
	MODEL D	1207	47790	1218	43160	56	3776	1454	39410	1470	34370	1482	35340
	MODEL E	1313	47670	1327	43410	64	3856	1614	40630	1632	36940	1646	37610
	MODEL F	1446	59160	1461	53840	64	3021	1780	49630	1800	42100	1815	42790
	MODEL G	1448	59000	1462	53610	63	2983	1780	49330	1799	42030	1814	42800

Table 11 (Cont'd)

PIPING FORCES

Model		MEMBER NUMBER											
		31		32		33		34		35		36	
		P	M	P	M	P	M	P	M	P	M	P	M
Exact		1737	44290	1739	50700	121.3		905.3		849.5		81.6	
G U Y A N	MODEL A	1329	34060	1329	27650	138.8		53.8		509.4		16.6	
	MODEL B	1520	37990	1520	62180	100.6		1539.		1768.		99.7	
	MODEL C	7428	46310	2428	80590	116.8		1804.		642.2		116.9	
	MODEL D	2203	40660	2103	77260	381.8		3097.		1069.		156.1	
	MODEL E	2515	40890	2516	77920	377.4		3209.		1024.		155.4	
	MODEL F	1743	44850	1743	54060	232.0		1291.		1022.		77.4	
	MODEL G	1745	44760	1745	55220	102.1		976.7		961.4		84.5	
M O D. G U Y A N	MODEL A	1503	35240	1504	35740	68.1		66.9		505.3		17.3	
	MODEL B	1470	40270	1471	64850	114.5		257.2		813.8		109.6	
	MODEL C	1792	45980	1796	77740	130.7		349.3		1001		126.0	
	MODEL D	1457	44870	1459	71640	210.2		931.8		768.9		165.7	
	MODEL E	1543	44710	1546	71260	210.0		1038.0		733.4		164.6	
	MODEL F	1732	45030	1734	53510	124.1		831.9		927.5		81.9	
	MODEL G	1737	44980	1739	55020	125.1		834.3		914.7		86.9	

Table 11(Cont'd)

PIPING FORCES

Model	MEMBER NUMBER											
	37			38								
	P	M		P	M		P	M		P	M	
Exact	17.4			3.4								
G U Y A N	MODEL A	13.8		1.6								
	MODEL B	17.7		3.2								
	MODEL C	18.1		4.3								
	MODEL D	14.9		3.3								
	MODEL E	14.9		3.3								
	MODEL F	17.8		3.6								
	MODEL G	17.9		3.6								
M O D. G U Y A N	MODEL A	16.1		2.3								
	MODEL B	17.8		3.5								
	MODEL C	18.1		4.2								
	MODEL D	13.8		3.0								
	MODEL E	13.7		2.9								
	MODEL F	17.8		3.5								
	MODEL G	17.9		3.6								

TABLE 12

PERCENTAGE ERRORS IN PIPING ELEMENTS

Case (Table 9)	GUYAN REDUCTION										
	PIPE ELEMENTS								PIPE SUPPORTS		
	1		5		12		18		7-37	18-38	13-34
	P	M	P	M	P	M	P	M			
A	-18.	12.7	+55.2	+2.9	545.2	+20.3	+31.8	+3.8	+14.4	-94.1	-79.7
B	-3.4	-25.5	-48.8	-18.2	505.4	+22.0	+5.8	-3.7	-17.1	+70.0	+22.2
C	-4.1	-26.0	-49.2	-18.8	479.5	+120.0	148.6	+16.2	-3.7	+99.3	+43.3
D	-31.4	-7.6	+32.5	-16.9	217.5	+1.8	+51.2	+65.3	214.8	242.1	+91.3
E	+26.2	-7.7	-1.1	-27.3	183.7	+5.5	+40.8	+64.0	211.1	254.5	+90.4
F	+13.9	-0.1	-5.5	-0.4	170.5	+6.1	+123.5	+1.7	91.3	+42.6	-5.1
G	+3.5	+1.6	-15.6	+1.1	-39.2	+2.7	+22.8	+1.9	-15.8	7.9	+3.6

TABLE 12 (Cont'd)

PERCENTAGE ERRORS IN PIPING ELEMENTS

Case (Table 9)	MODIFIED GUYAN											
	PIPE ELEMENTS								PIPE SUPPORTS			
	1		5		12		18		7-37	18-38	13-34	
	P	M	P	M	P	M	P	M				
A	-0.4	+0.2	+0.4	0	+6.0	+0.4	-18.0	+0.4	-43.9	-92.6	-78.8	
B	-4.6	-7.9	-12.0	-6.7	+13.3	+0.5	-8.0	-2.0	-5.6	-71.6	+34.3	
C	-6.6	-9.7	-13.7	-8.5	+10.2	-1.3	-1.1	-3.2	+7.7	-61.4	+54.4	
D	-16.8	-18.0	-17.7	-16.9	-13.9	-4.5	-17.8	+6.0	73.3	+2.9	103.1	
E	-16.8	-17.5	-17.4	-17.4	-13.9	-5.2	-17.2	+5.6	73.1	+14.7	101.7	
F	+1.0	+1.2	+1.5	+1.2	+6.0	+1.3	+2.7	+1.4	2.3	-8.1	+0.4	
G	+1.2	+1.5	+1.3	+1.5	+4.8	+1.6	+2.9	+1.8	3.1	-7.8	+6.5	

The following conclusion may be drawn from the data on Table 12:

- (i) When reduction schemes are used the bending moment data appears to be most reliable with the thrust and pipe support data the poorest.
- (ii) The Modified Guyan Reduction gives significantly better results than the Guyan Reduction. The maximum error for pipe thrust and moment for the latter case is 18% no matter which reduction scheme is used. The maximum error for pipe thrust and moment is 545% when Guyan Reduction is used. The comparable maximum errors for pipe support loads are 103% and 254% respectively for the both reduction methods.
- (iii) Reduction schemes F and G yield good results (maximum error 8.1%) when Modified Guyan Reduction is used while scheme G still yields rather poor (39% error in pipe 12 thrust) results for Guyan Reduction.
- (iv) The good results are obtained for scheme G because degrees of freedom normal to the restraints provided by the pipe supports are included as dynamic degrees of freedom. The degree of freedom active in the direction of the pipe support should probably be included to achieve good results for the pipe support loads with Guyan Reduction.

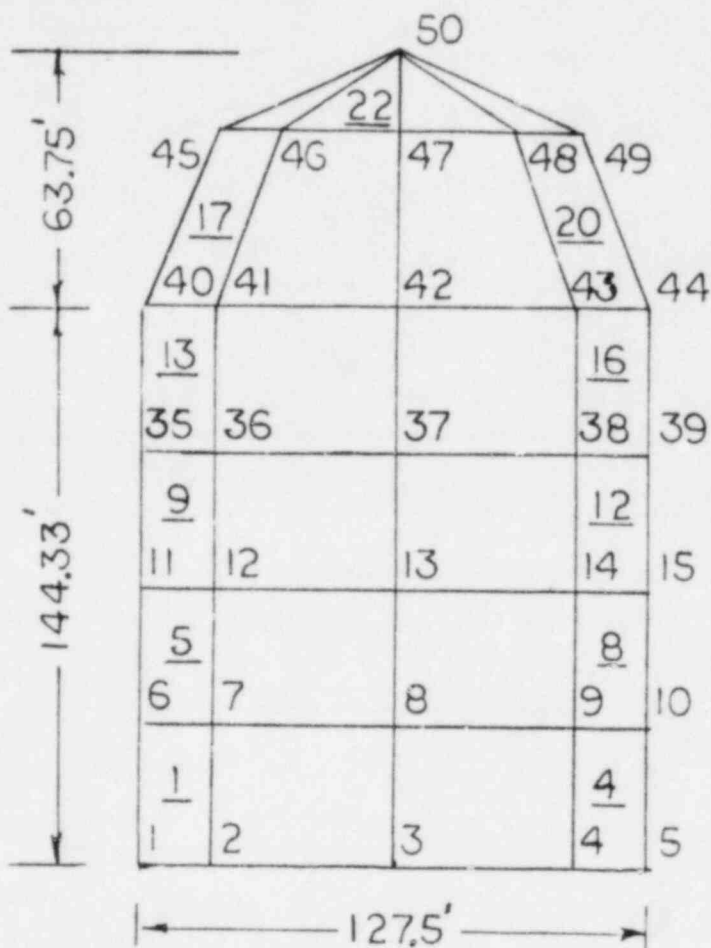
4.4 REACTOR CONTAINMENT BUILDING

The reactor containment building shown in Figure 6 is subjected to the response spectrum of Figure 3 in the X direction. Plate elements 1 through 24 are used to model the exterior containment. The reactor floor is modeled with plate elements 25 through 32. A wall supporting the plate is modeled with plate element 33 through 40. All plate elements are taken as concrete with a thickness of 36 inches. Beam elements 41 through 43 are used to model the reactor. Symmetric boundary conditions are specified at all nodes at ($Y = 0$). These boundary conditions restrain the following degrees of freedom: displacement in the Y direction and rotations about the X and Z axes.

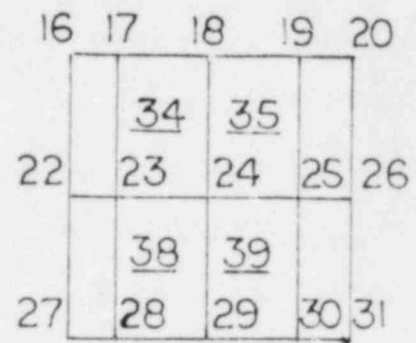
An exact solution is obtained and solutions using both Guyan and Modified Guyan Reduction for the reduced models shown in Table 13. There are 102 unrestrained translational degrees of freedom.

Table 13
Reduction Schemes for Containment Building

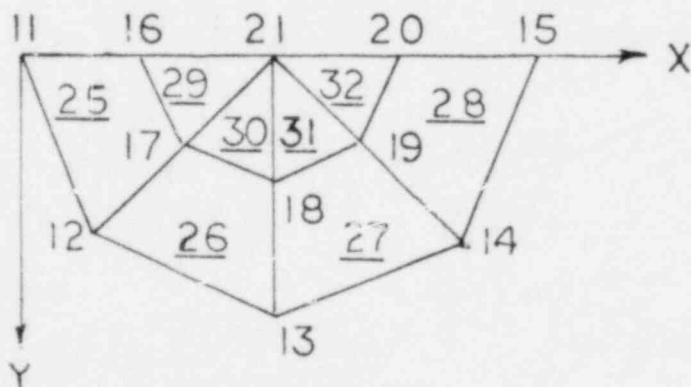
Case	No. of Primary Degrees of Freedom	Degrees of Freedom Retained
A	3	21-X; 34-X; 50-X
B	6	13-Y; 21-X; 34-X; 40-Z; 44-Z; 50-X
C	13	11-X; 11-Z; 15-X; 15-Z; 21-X; 21-Z; 34-X; 40-X; 40-Z; 42-Y; 44-X; 44-Z; 50-X
D	23	11-X; 11-Z; 13-X; 13-Y; 15-X; 15-Z; 16-X; 16-Z; 18-X; 18-Y; 20-X; 20-Z; 21-X; 21-Z; 33-X; 34-X; 40-X; 40-Z; 42-X; 42-Y; 44-X; 44-Z; 50-X
E	46	11-X; 11-Z; 13-X; 13-Y; 15-Y; 15-Z; 16-X; 16-Z; 16-YY; 17-X; 17-Y; 17-Z; 17-YY; 17-ZZ; 18-X; 18-Y; 18-Z; 18-XX; 18-YY; 18-ZZ; 19-X; 19-Y; 19-Z; 19-XX; 19-YY; 19-ZZ; 20-X; 20-Z; 20-YY; 21-X; 21-Z; 21-YY; 33-X; 33-Z; 33-YY; 34-X; 34-Z; 34-YY; 40-X; 40-Z; 42-X; 42-Y; 44-X; 44-Z; 50-X



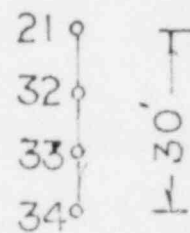
ELEVATION



INTERNAL WALL



REACTOR FLOOR



REACTOR

Fig. 6 CONTAINMENT BUILDING

Ten modes are included in the solution with the resulting frequencies shown in Table 14.

Table 14
Frequencies for Containment Building

Case	Frequency (CPS) for Mode									
	1	2	3	4	5	6	7	8	9	10
EXACT	3.84	9.53	9.89	10.2	10.3	10.7	10.9	14.5	14.8	15.8
A	4.09	13.3	113.	-	-	-	-	-	-	-
B	4.08	12.6	13.6	20.6	22.2	113.	-	-	-	-
C	3.98	9.55	11.2	12.7	13.0	13.6	14.4	23.7	24.5	29.5
D	3.89	9.55	10.4	12.5	13.2	13.4	13.6	20.1	22.5	25.4
E	3.89	9.54	10.4	12.5	13.1	13.3	13.5	20.0	22.3	23.3

As with the piping system it appears that several of the modes from the exact model are missing from the reduced model.

Stresses in typical plate element and the peak bending moment in the reactor beam model are shown in Table 15.

Table 15
Stresses and Moments in Containment Building

Case	Peak Stress in Plate Element						Peak Moment in Beam
	1	5	13	25	29	33	
EXACT	649	481	186	72	83	138	47,740
G A	545	419	225	65	175	109	2,527,000
U							
Y B	548	426	244	63	180	109	2,738,000
A							
N C	600	465	222	87	149	122	2,148,000
D	635	475	192	46	104	140	350,900
E	635	475	192	56	89	141	45,640
M. A	569	420	166	60	24	219	103,300
G							
U B	570	422	167	60	29	117	56,450
Y							
A C	601	444	173	68	82	127	45,170
N							
D	632	466	178	71	93	135	46,410

The plate stresses for all of the elements appear to be quite good. Modified Guyan Reduction appears to be a little better than Guyan Reduction but not to the same degree as for the first three problems considered. The very significant difference shows up with the peak moment in the beam model of the reactor. Guyan Reduction yields totally unreliable results while Modified Guyan gives good results except for Reduction scheme A. As with the stiff pipe supports the floor provides a very stiff support for the reactor and undoubtedly leads to these very poor results. Guyan Reduction gives a reasonable value for this moment only when the rotational degree of freedom connecting the beam to the plate is included as a primary degree of freedom. Note that there is no mass attached to this degree of freedom.

4.5 COMPUTER REQUIREMENTS

For the four sample problems considered the Modified Guyan and Guyan Reduction solutions required almost exactly the same computer time. Both of the reduction methods required solution times about 3 times longer than would be required using the complete SAPV solution. Undoubtedly some of this difference resulted from "patching in" the reduction modification. Changes were made in a manner to expedite the code modification rather than to develop an efficient code. If this facet of the problem were to be investigated further it would be recommended that the current code be used to pinpoint the calculations that require the most time. This could then be used to evaluate the likelihood that the SAPV with reduction would be expected to run faster than SAPV without reductions.

5.0 CONCLUSIONS AND RECOMMENDATIONS

Based upon the four sample problems the following conclusions are made:

1. The errors resulting from Guyan Reduction are significantly larger than those which result from Modified Guyan Reduction. For example the errors in member bending moments in the ten story structure are always less than 2% when Modified Guyan Reduction is used even when only 3 degrees of freedom are retained. The same errors for Guyan Reduction are 100% when 3 degrees of freedom are retained and only become acceptable when 6 or more degrees of freedom are retained.
2. For a given reduction scheme the errors resulting from Modified Guyan Reduction are much more uniform for all members in the problem than when Guyan Reduction is used.

Again using the ten story frames as an example for the same reduction scheme (3 degrees of freedom retained) the error in the bending moments of members 20 and 41 are 100% and 1% when Guyan Reduction is used. The corresponding errors with Modified Guyan Reduction are 1.3% and 1.6%. This variation in error which Guyan Reduction exhibits would make it very difficult to validate a given set of results. If we looked at the forces in member 41, the results for the 3 degrees of freedom model would appear to be quite good.

3. Frequencies calculated with Guyan Reduction appear to be quite good as long as the degrees of freedom retained can adequately represent the mode shapes of interest.
4. The bending moments in piping systems predicted based upon Guyan Reduction are more reliable than are the axial pipe loads. For example element 12 has errors in the axial pipe load in the range of 200% to 300% (for different degrees of freedom retained) while the same errors in moments are in the range of 5% to 20%. The same errors when Modified Guyan Reduction is used are 6 to 14% for axial load and 1% to 5% for bending moments. This again illustrates the better and more uniform results obtained using Modified Guyan Reduction.
5. Much larger errors result with the reduction schemes when there are both "stiff" and "soft" elements in the system. This occurs in the piping problem (supports relative to pipe) and in the containment building problem (floor in plane stiffness to reactor beam model). In either case Guyan Reduction gives totally unreliable results unless

care is taken to isolate the stiff element by containing it within degrees of freedom retained. The same effect is also observed with Modified Guyan Reduction but again the errors are much smaller.

The following recommendations are made:

1. Computer codes which contain Guyan Reduction should be modified to incorporate Modified Guyan Reduction.
2. If computer solutions are to be made using Guyan Reduction care should be taken to identify the stiff elements in the model and to retain all degrees of freedom around these elements as primary degrees of freedom.
3. Additional parametric studies should be made with the following model parameters varied: structural geometry and type; relative stiffness of members; and relative mass. Based in these studies criteria should be developed relating errors to the reduction scheme used. This should be done for both Guyan and Modified Guyan Reduction. The results of such a study would serve two purposes. First, they would provide some insight to the errors which may exist in Guyan Reduction runs made. Second, such criteria could provide the basis for selecting the "optimum" set of primary degrees of freedom to be retained.

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*Available for purchase from the NRC/GPO Sales Program, U.S. Nuclear Regulatory Commission, Washington, DC 20555, and the National Technical Information Service, Springfield, VA 22161.

APPENDIX A
LISTING OF SAPV
MODIFICATIONS

POOR ORIGINAL

76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

1	PROGRAM SAP5 (INPUT,OUTPUT,TAPE2,TAPE3,TAPE4,TAPE5=INPUT,TAPE6=	CORR2	1
	1 OUTPUT,TAPE7,TAPE8,TAPE9,TAPE10,TAPE11,TAPE12,	CORR2	2
	2 TAPE13,TAPE14,TAPE15,TAPE16,TAPE17,TAPE21,TAPE18)	CAM	1
5		SAP5	2
		SAP5	3
		SAP5	4
	SAP5	SAP5	5
	A STRUCTURAL ANALYSIS PROGRAM	SAP5	6
	FOR STATIC AND DYNAMIC RESPONSE OF LINEAR SYSTEMS	SAP5	7
10		SAP5	8
	K.J. BATHE , E.L. WILSON , F.E. PETERSON	SAP5	9
	UNIVERSITY OF CALIFORNIA , BERKELEY	SAP5	10
		CORR3	1
	CDC 7600 CONVERSION BY D. GARDNER	CORR3	2
15	BROOKHAVEN NATIONAL LABORATORY	CORR3	3
	UPTON, N. Y.	CORR3	4
	FEBRUARY, 1977	SAP5	20
	USC IBM-CALCOMP MODIFICATION --- MARCH, 1976	SAP5	21
20		SAP5	22
		SAP5	23
		SAP5	24
		SAP5	25
	LOGICAL ELPRT,ELPCH,GENPRT,GENPCH,DEFPCH,GEOST	SAP5	27
25	COMMON/JUNK/ MED(18),JUK(200)	COMSIZE	1
	COMMON /ELPAR/ NPAR(14),NUMNP,MBAND,NELTYP,N1,N2,N3,N4,N5,MTOT,NEG	SAP5	28
	COMMON/EM/ QQQ(3000)	COMSIZE	2
	COMMON /DYN/ IDUS(11),NDYN,THOPCH,THSPCH	SAP5	31
	COMMON /TAPES/ MOQ(6)	SAP5	32
	COMMON /EXTRA/ MODEX,NTB,NIOSV,NTIO,KEQB,NUMEL,T(12),S(12)	SAP5	33
30	COMMON /SOL/ NBLOCK,NEQB,LL,NF,IDUM,NEIG,NAD,NVV,ANORM,NFO	SAP5	34
	COMMON /CTL/ GRAV,ELPRT,ELPCH,GENPRT,GENPCH,DEFPCH,IWTCO,	SAP5	35
	1 MINGND,IPLT,NPLOTS,NPLT	CORR3	5
	COMMON /GEOSTF/ GEOST,NELGEO	SAP5	37
35	COMMON/SEIS/ NRSC,KAPG,NRS,IT,DOATE,JFLAG,JUNK(2)	COMSIZE	3
	COMMON/AA/A(75001)	CAM	2
	COMMON/CONTL/NRDOF,NTYP,NST,MBW,NS,NS1,NS2,NS3,NOT,MBS,MODE,	CAM	3
	1 MD,MBLRED,NSEQ,NT0	CAM	4
	LEVEL 2, A	LEVEL2	2
	MTOT=75000	CAM	5
40		SAP5	40
		SAP5	41
	CDC ROUTINE ERRSET IS CALLED TO ALLOW PROGRAM TO	CORR3	6
	READ PAST UP TO 100 DATA ERRORS. IF AT LEAST ONE	CORR3	7
	IS FOUND THE RUN IS STOPPED AFTER STATEMENT 900.	CORR3	8
45		SAP5	44
	CALL ERRSET(KOUNT,100)	CORR3	9
		SAP5	46
	NPLOTS=0	SAP5	47
50	5 CALL MEDPRT(KDYN)	SAP5	49
	JFLAG=0	SAP5	50
	WRITE (13) MED	SAP5	51
		SAP5	52
	RE-START FOR MORE PLOTS	SAP5	53
		SAP5	54
55	IF (IPLT .GE. 0) GO TO 14	SAP5	55
	DO 7 I=1,7	SAP5	56
	7 T(I)=T(1)	SAP5	57

POOR ORIGINAL

PROGRAM SAP5

76/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

	C	S(1)=S(1)	SAP5	58
		READ (3) LL,NF,KDYN,NEQB,NEQ,NBLOCK	SAP5	59
60		CALL SAPLOT(LL,NF,KDYN,NEQB,NEQ,NBLOCK)	SAP5	60
		CALL SECOND(T(8))	SAP5	61
		DO 8 I=9,12	SAP5	62
	8	T(1)=T(8)	SAP5	63
65	C	S(1)=S(8)	SAP5	64
		GO TO 90	SAP5	65
	C		SAP5	66
	C	RE-START MODE ACTIVATED IF NOYN.EQ.-2 OR NOYN.EQ.-3	SAP5	67
	C		SAP5	68
70	C	14 IF(NDYN.LT.0) GO TO 20	SAP5	69
	C		SAP5	70
	C	INPUT JOINT DATA	SAP5	71
	C		SAP5	72
		N2=N1+8*NUMNP	SAP5	73
		N3=N2+NUMNP	SAP5	74
75		N4=N3+NUMNP	SAP5	75
		N5=N4+NUMNP	SAP5	76
		N6=N5+NUMNP	SAP5	77
		CALL ERROR(N6)	SAP5	78
80	C		SAP5	79
		CALL INPUTJ(A(N1),A(N2),A(N3),A(N4),A(N5),NUMNP,NEQ)	CORR2	4
	C		SAP5	82
	C	STORE THE NODAL COORDINATES FOR LATER USE	SAP5	83
	C		SAP5	84
85	C	WRITE (13) (A(1),I=N2,N5)	SAP5	85
	C		SAP5	86
	C	FORM ELEMENT STIFFNESSES	SAP5	87
	C		SAP5	88
	C	INPUT PRIMARY DOF AND REORDER ALL DOF SO THAT PRIMARY ARE FIRST	CAM	6
	C		CAM	7
90	C		CAM	8
		CALL REDUC1(A(N1),A(N6),NTYP,NUMNP,NROOF)	SAP5	89
		CALL SECOND(T(2))	SAP5	90
	C		SAP5	91
		NBAND=0	SAP5	92
95		NUMEL=0	SAP5	93
		REWIND 1	SAP5	94
		REWIND 2	SAP5	95
	C		SAP5	96
		DO 900 M=1,NELTYP	SAP5	97
		READ (5,1001) NPAR	SAP5	98
100	C+++	DATA PORTHOLE SAVE	SAP5	99
		IF(MODEX.EQ.1) WRITE (NT8) NPAR	SAP5	100
		NT1=1	SAP5	101
		IF (MINBND .GE. 1 .AND. MODEX .EQ. 0) NT1=4	SAP5	102
		IF (MINBND .GE. 1 .AND. MODEX .EQ. 1) NT1=7	SAP5	103
105		WRITE (NT1) NPAR	SAP5	104
		NUMEL=NUMEL+NPAR(2)	SAP5	105
		MTYPE=NPAR(1)	SAP5	106
	C		SAP5	107
		CALL ELTYPE(MTYPE)	SAP5	108
110	C		SAP5	109
		900 CONTINUE	CORR3	10
		IF(KOUNT.GE.1) STOP	SAP5	110
	C		SAP5	111
	C	ADD AN END-OF-FILE FLAG ON FILE 13		

PROGRAM SAPS

78/78

OPT=1

FTN 4.5+414

11/19/80 17.24.0

115	C	M=-1	SAPS	112
		IF (1PLT.GT.0)	SAPS	113
		XWRITE (13) P,M	SAPS	114
	C		SAPS	115
120	C	MINIMIZE THE BANDWIDTH	SAPS	116
	C		SAPS	117
	C	IF (MINBND.GT.0) CALL SAPHIN	SAPS	118
	C		SAPS	119
	C	DETERMINE BLOCKSIZE	SAPS	120
125	C		SAPS	121
	C	ADOSTF	SAPS	122
	C		SAPS	123
	C	NEQB=(M*OT - 4*LL)/(MBAND + LL + 1)/2	SAPS	124
	C		SAPS	125
130	C	OVER-RIDE THE SYSTEM MATRIX BLOCKSIZE WITH THE INPUT (NON-ZERO)	SAPS	126
	C	VALUE, KEQB.	SAPS	127
	C	THIS OVER-RIDE ENTRY IS TO ALLOW PROGRAM CHECKING OF MULTI-	SAPS	128
	C	BLOCK ALGORITHMS WITH WHAT WOULD NORMALLY BE ONE BLOCK DATA.	SAPS	129
	C		SAPS	130
135	C	IF(KEQB.LT.NEQB) NEQB = KEQB	SAPS	131
	C		SAPS	132
	C	GO TO (690,700,700,700,730,700), KDYN	SAPS	133
	C		SAPS	134
	C	STATIC SOLUTION	SAPS	135
140	C		SAPS	136
	C	690 CONTINUE	SAPS	137
		NEQB1=(MTOT - MBAND)/(2*(MBAND+LL) + 1)	SAPS	138
		NEQB2=(MTOT - MBAND - LL*(MBAND-2))/(3*LL + MBAND + 1)	SAPS	139
		IF (NEQB1.LT.NEQB) NEQB=NEQB1	SAPS	140
145		IF (NEQB2.LT.NEQB) NEQB=NEQB2	SAPS	141
		NBLOCK = (NEQ-1)/NEQB + 1	SAPS	142
		IF(NEQB.GT.NEQ) NEQB=NEQ	SAPS	143
		IF(1PLT.EQ.0) GO TO 695	SAPS	144
		MTOT=20*NUMNP+NEQB*NBLOCK*LL	SAPS	145
150		IF(MTOT.GT.MTOT) GO TO 790	SAPS	146
		WRITE(6,2020)MTOT	SAPS	147
		MODEX=1	SAPS	148
		695 MTOT=6*NUMNP+NEQB*NBLOCK*LL+1	SAPS	149
		IF(MTOT.GT.MTOT) GO TO 790	SAPS	150
155		WRITE(6,2030)MTOT	SAPS	151
		MODEX=1	SAPS	152
		GO TO 790	SAPS	153
	C		SAPS	154
	C	EIGENSOLUTION	SAPS	155
160	C		SAPS	156
	C	1. DETERMINANT SEARCH ALGORITHM	SAPS	157
	C		SAPS	158
	C	700 IF (NEQB.LT.NEQ) GO TO 710	SAPS	159
		NIM=3	SAPS	160
165		NC=NF + NIM	SAPS	161
		NVM=6	SAPS	162
		NCA=NEQ*MAX0(MBAND,NC)	SAPS	163
		MTOT=NCA + 4*NEQ + 2*NVM*NEQ + 5*NC	SAPS	164
		NEIQ=0	SAPS	165
170		IF(MTOT.LE.MTOT) GO TO 720	SAPS	166
	C		SAPS	167
			SAPS	168

PROGRAM SAP5

76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.0

	C	2. SUBSPACE ITERATION ALGORITHM	SAP5	169
	C		SAP5	170
175		710 NY=MIND(2*NF,NF+8)	SAP5	171
		IF (NAD.NE.0) NV=NAD	SAP5	172
		NEQB1=(MTOT - MBAND)/(2*MBAND + 1)	SAP5	173
		NEQB2=(MTOT - MBAND - 2*NV - NV*(MBAND-2))/(3*NV + MBAND + 1)	SAP5	174
		NEQB3=(M.OT - 3*NV*NV - 3*NV)/(2*NV + 1)	SAP5	175
		NEQB4=(M.OT - 8*NV)/(1 + MBAND)	SAP5	176
180		IF (NEQB1.LT.NEQB) NEQB=NEQB1	SAP5	177
		IF (NEQB2.LT.NEQB) NEQB=NEQB2	SAP5	178
		IF (NEQB3.LT.NEQB) NEQB=NEQB3	SAP5	179
		IF (NEQB4.LT.NEQB) NEQB=NEQB4	SAP5	180
185		NEIQ=1	SAP5	181
	C		SAP5	182
		720 CONTINUE	SAP5	183
		NBLOCK = (NEQ-1)/NEQB + 1	SAP5	184
		IF (NEQB.GE.NEQ) NEQB=NEQ	SAP5	185
190	C		SAP5	186
	C	HISTORY OR SPECTRUM ANALYSIS	SAP5	187
	C		SAP5	188
		KREM = 1000	SAP5	189
		NTOT = NBLOCK*NEQB*NF + KREM	SAP5	190
195		IF (MTOT.LT.NTOT)	SAP5	191
		XWRITE (6,320) NTOT	SAP5	192
		IF (IPLT.EQ.0) GO TO 725	SAP5	193
		NTOT=20*NUMNP+NEQB*NBLOCK*NF+2*NF	SAP5	194
		IF (MTOT.GE.NTOT) GO TO 790	SAP5	195
200		WRITE(6,2020)NTOT	SAP5	196
		MODEX=1	SAP5	197
		725 NTOT=6*NUMNP+NEQB*NBLOCK*NF+NF	SAP5	198
		IF (MTOT.GE.NTOT) GO TO 790	SAP5	199
		WRITE(6,2030)NTOT	SAP5	200
205		MODEX=1	SAP5	201
		GO TO 790	SAP5	202
	C		SAP5	203
	C	STEP-BY-STEP DIRECT INTEGRATION	SAP5	204
	C		SAP5	205
210		730 CONTINUE	SAP5	206
	C	DISPLACEMENT COMPONENTS FOR DIRECT OUTPUT (*NSD*)	SAP5	207
		NN2 = NEQ	SAP5	208
	C	DISPLACEMENT COMPONENTS REQUIRED FOR RECOVERY OF ALL OF THE	SAP5	209
	C	REQUESTED ELEMENT STRESS COMPONENTS (*NSS*)	SAP5	210
215		NN3 = NEQ	SAP5	211
	C		SAP5	212
	C	1. DECOMPOSITION	SAP5	213
	C		SAP5	214
		NEQB1 = (MTOT-NN2-NN3-NEQ-MBAND)/(2*MBAND+1)	SAP5	215
220	C		SAP5	216
	C	2. TIME INTEGRATION PHASE	SAP5	217
	C		SAP5	218
	C		SAP5	219
		NEQB2 = (MTOT-MBAND-2*(NN2+NN3)-5*NEQ)/(MBAND+1)	SAP5	220
225	C		SAP5	221
		IF (NEQB1.LT.NEQB) NEQB = NEQB1	SAP5	222
		IF (NEQB2.LT.NEQB) NEQB = NEQB2	SAP5	223
		IF (NEQB.OT.NEQ) NEQB = NEQ	SAP5	224
			SAP5	225

POOR ORIGINAL

PROGRAM SAP5

78/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

230	C	MBLOCK = (NEQ-1)/NEQB + 1	SAP5	226
	C	3. INPUT PHASE	SAP5	227
	C		SAP5	228
	C	NUMBER OF TIME FUNCTIONS (NFN*)	SAP5	229
	C	NUMBER OF TIME FUNCTIONS (*NFN*)	SAP5	230
235	C	NN2 = 10	SAP5	231
	C	MAXIMUM NUMBER OF FUNCTION DEFINITION POINTS (*MXLP*)	SAP5	232
	C	NN3 = 40	SAP5	233
	C		SAP5	234
	C	NN4 = 6*NUMNP + 2*NN2*NEQ	SAP5	235
240	C	IF (NN4.GT.MTOT)	SAP5	236
	C	XWRITE (6,320) NN4	SAP5	237
	C	NN4 = NEQ*2*(NN2+1) + NN2*(1+2*NN3)	SAP5	238
	C	IF (NN4.GT.MTOT)	SAP5	239
	C	XWRITE (6,320) NN4	SAP5	240
245	C		SAP5	241
	C	790 IF (NEQB .GE. 2) GO TO 792	SAP5	242
	C	IF (NEQ .LT. 3) GO TO 792	SAP5	243
	C	WRITE (6,2010) NEQB	SAP5	244
	C	NEQB=2	SAP5	245
250	C	MBLOCK=(NEQ-1)/NEQB + 1	SAP5	246
	C	MODEX=1	SAP5	247
	C	792 CONTINUE	SAP5	248
	C		SAP5	249
255	C	INPUT NODAL LOADS	SAP5	250
	C		SAP5	251
	C	NSB=NEQB*MBLOCK	SAP5	252
	C	N2=N1+6*NUMNP	SAP5	253
	C	N3=N2+LL*NSB	SAP5	254
	C	N4=N3+NSB	SAP5	255
260	C	CALL ERROR(N4)	SAP5	256
	C	WRITE (6,201) NEQ,MBAND,NEQB,MBLOCK	SAP5	257
	C		SAP5	258
	C	CALL SECOND(1(3))	SAP5	259
	C		SAP5	260
265	C	CALL INL(A(N1),A(N2),A(N3),NUMNP,NEQB,NSB,LL,MBLOCK)	SAP5	261
	C		SAP5	262
	C	CALL SECOND(1(4))	SAP5	263
	C		SAP5	264
	C	CHECK FOR UNRESTRAINED DEGREES OF FREEDOM	SAP5	265
270	C		SAP5	266
	C	N3=N2+NEQ	SAP5	267
	C	CALL CHICK(A(N1),A(N2),NEQ,NUMEL,NELTYP,NUMNP)	SAP5	268
	C		CORR2	5
	C		SAP5	270
275	C	FORM TOTAL STIFFNESS	SAP5	271
	C		SAP5	272
	C	NE2B=2*NEQB	SAP5	273
	C	N2=N1+NEQB*MBAND	SAP5	274
	C	N3=N2+NEQB*LL	SAP5	275
	C	N4=N3+4*LL	SAP5	276
	C	NN2=N1+NE2B*MBAND	SAP5	277
	C	NN3=NN2+NE2B*LL	SAP5	278
	C	NN4=NN3+4*LL	SAP5	279
	C		SAP5	280
	C	CALL ADDSTF (A(N1),A(NN2),A(NN3),A(NN4),NUMEL,MBLOCK,NE2B,LL,MBAN	SAP5	281
	C	X0,ANORM,NVV)	SAP5	282

POOR ORIGINAL

PROGRAM SAPS

78/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

	C		SAPS	283
	C	MT AND CG CALCULATIONS	SAPS	284
	C		SAPS	285
290		IF (IWTG.EQ. 0) GO TO 18	SAPS	286
		N2=N1+NUMNP*6	SAPS	287
		N3=N2+NUMNP	SAPS	288
		N4=N3+NUMNP	SAPS	289
		N5=N4+NUMNP	SAPS	290
		N6=N5+NSB	SAPS	291
295		CALL ERROR(N6)	SAPS	292
		CALL WTCG(A(N1),A(N2),A(N3),A(N4),A(N5),NUMNP,NEQB,NEQ,NBLOCK,NSB	SAPS	293
		X)	SAPS	294
		18 CONTINUE	SAPS	295
		CALL SECOND(T(5))	SAPS	296
300	C		CAM	9
	C	IF NTYP.NE.0 GO TO ROUTINES TO FORM EIGENVECTORS	CAM	10
	C		CAM	11
		IF(NTYP.EQ.0) GO TO 100	CAM	12
	C	DETERMINE SIZE REQUIREMENYS	CAM	13
305		NST=NEQ	CAM	14
		MBW=MBAND	CAM	15
		NS=NST-NRDOF	CAM	16
		NS1=(NRDOF*NRDOF+NRDOF)/2	CAM	17
		IF(NRDOF.GT.MBW) GO TO 101	CAM	18
310		NS2=(MBW-NRDOF)*NRDOF+((NRDOF-1)*(NRDOF-1)*(NRDOF-1))/2	CAM	19
		GO TO 102	CAM	20
		101 NS2=(MBW*MBW+MBW)/2	CAM	21
		102 NOT=NS*NRDOF	CAM	22
		NADD=0	CAM	23
315		MODE=NF	CAM	24
		MD=MODE+NADD	CAM	25
		IF(MD.GT.NRDOF) MD=NRDOF	CAM	26
		IF(MODE.GT.NRDOF) MODE=NRDOF	CAM	27
		IF(NF.GT.NRDOF) NF=NRDOF	CAM	28
320		NR2=1+NEQB*MBAND	CAM	29
		CALL SIZE(A(1),A(NR2),MTOT,NEQB,NBLOCK,MBAND)	CAM	30
		NR1=1	CAM	31
		NR2=NR1+NS1	CAM	32
		NR3=NR2+NS2	CAM	33
325		NR4=NR3+NSEQ*MB5	CAM	34
		NR5=NR4+NEQB*MBAND	CAM	35
		CALL STIFF(A(NR1),A(NR2),A(NR3),A(NR4),A(NR5),NBLOCK,NEQB,NRDOF,	CAM	36
		1 NTYP,NST,MBW,NS,NS1,NS2,NS3,NOT,MB5,MODE,MD,MBLRED,NSEQ,NT0)	CAM	37
		NSZ=NST	CAM	38
330		CALL MASS(A(NR3),NEQB,NBLOCK,NSZ)	CAM	39
	C	INVERT KSS AND STORE ON TAPE 17	CAM	40
		NR4=NR3+NST	CAM	41
		NR5=NR4+NSEQ*MB5	CAM	42
		NR6=NR5+NSEQ*MB5	CAM	43
335		NR7=NR6+NST	CAM	44
		CALL GAUS(A(NR4),A(NR5),A(NR6),A(NR7),NSEQ,NS,MB5,MBLRED,	CAM	45
		1 18,19,17,1)	CAM	46
	C	FORM K* AND M*	CAM	47
		NR5=NR4+NS1	CAM	48
340		NR6=NR5+NS	CAM	49
		NR7=NR6+NRDOF	CAM	50
		CALL REDUCE(A(NR1),A(NR2),A(NR3),A(NR4),A(NR5),A(NR6),A(NR7),	CAM	51

POOR ORIGINAL

PROGRAM SAP5

78/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

	1	NRDOF,NTYP,NST,M84,NS,NS1,NS2,NS3,NOT,M85,MODE,	CAM	52
	2	MD,M81RED,NSEQ,NT0)	CAM	53
345	C	SAVE MASS ON 15 FOR NORMALIZATION OF EIGENVECTORS	CAM	54
		REWIND 15	CAM	55
		DO 108 I=1,NST	CAM	56
	108	WRITE(15) (A(NR3+I-1))	CAM	57
		REWIND 15	CAM	58
350		DO 104 I=1,NS1	CAM	59
	104	A(NR2+I-1)=A(NR4+I-1)	CAM	60
		NR1=1	CAM	61
		NR2=NR1+NS1	CAM	62
		NR3=NR2+NS1	CAM	63
355		NR4=NR3+NRDOF*MD	CAM	64
		NR5=NR4+NRDOF*NRDOF	CAM	65
		NR6=NR5+NRDOF	CAM	66
		NR7=NR6+NRDOF	CAM	67
		CALL POWER(A(NR1),A(NR2),A(NR3),A(NR4),A(NR5),A(NR6),A(NR7),	CAM	68
360		1 NS1,NRDOF,MODE)	CAM	69
		NR4=NR3+NRDOF*MD	CAM	70
		NR5=NR4+NT0*NRDOF	CAM	71
		NR6=NR5+NRDOF	CAM	72
		NR7=NR6+NEQB*MODE	CAM	73
365		CALL EXPAND(A(NR3),A(NR4),A(NR5),A(NR6),NRDOF,MD,MODE,NT0,NEQB,	CAM	74
		1 N81BLOCK,NTYP,NS,A(NR7),NST)	CAM	75
	100	CONTINUE	CAM	76
	C		SAP5	297
	C	SOLUTION PHASE	SAP5	298
370	C		SAP5	299
		IF (GEOST) GO TO 30	SAP5	300
	20	GO TO (30,40,50,60,70,80), KDYN	SAP5	301
	C		SAP5	302
	C	STATIC SOLUTION	SAP5	303
375	C		SAP5	304
	30	IF(MODEX.EQ.0) GO TO 32	SAP5	305
		T(6)=T(5)	SAP5	306
	C	S(6)=S(5)	SAP5	307
		T(7)=T(5)	SAP5	308
380	C	S(7)=S(5)	SAP5	309
		IF (IPLT.GT. 0.AND..NOT.GEOST) CALL SAPLOT(LL,NF,KDYN,NEQB,NEQ	SAP5	310
		X,N81BLOCK)	SAP5	311
		CALL SECOND(T(8))	SAP5	312
		DO 31 I=9,12	SAP5	313
385	31	T(I)=T(8)	SAP5	314
	C	S(I)=S(8)	SAP5	315
		GO TO 90	SAP5	316
	C		SAP5	317
	32	REWIND 12	SAP5	318
390		CALL SOLEQ	SAP5	319
		CALL SECOND(T(6))	SAP5	320
		T(7)=T(6)	SAP5	321
	C	S(7)=S(6)	SAP5	322
		IF (IPLT.GT. 0) CALL SAPLOT(LL,NF,KDYN,NEQB,NEQ,N81BLOCK)	SAP5	323
395		CALL SECOND(T(8))	SAP5	324
		DO 33 I=9,12	SAP5	325
	33	T(I)=T(8)	SAP5	326
	C	S(I)=S(8)	SAP5	327
		IF (.NOT. GEOST) GO TO 90	SAP5	328

POOR ORIGINAL

PROGRAM SAP5

76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

400	WRITE (8,2000) NUMEL,NEQEO	SAP5	329
	CALL ADDQEO (A(1),A(1),NE2B,MBAND,NUMEL,NBLOCK,LL,ANORM)	SAP5	330
	GO TO 20	SAP5	331
	C	SAP5	332
	EIGENVALUE EXTRACTION	SAP5	333
405	C	SAP5	334
	40 T(6) = T(5)	SAP5	335
	C S(6)=S(5)	SAP5	336
	CALL SOLEIG	SAP5	337
	CALL SECOND(T(7))	SAP5	338
410	IF (IPLT.GT. 0) CALL SAPLOT(ILL,NF,KDYN,NEQB,NEQ,NBLOCK)	SAP5	339
	CALL SECOND(T(8))	SAP5	340
	T(9) = T(8)	SAP5	341
	T(10) = T(8)	SAP5	342
	T(11) = T(8)	SAP5	343
415	T(12) = T(8)	SAP5	344
	C S(9) = S(8)	SAP5	345
	C S(10) = S(8)	SAP5	346
	C S(11) = S(8)	SAP5	347
	C S(12) = S(8)	SAP5	348
420	GO TO 90	SAP5	349
	C	SAP5	350
	FORCED DYNAMIC RESPONSE ANALYSIS	SAP5	351
	C	SAP5	352
	50 T(6) = T(5)	SAP5	353
425	C S(6)=S(5)	SAP5	354
	IF (INDYN.LT.0) GO TO 52	SAP5	355
	CALL SOLEIG	SAP5	356
	CALL SECOND(T(7))	SAP5	357
	CALL SECOND(T(7))	SAP5	358
430	IF (IPLT.GT. 0) CALL SAPLOT(ILL,NF,KDYN,NEQB,NEQ,NBLOCK)	SAP5	359
	CALL SECOND(T(8))	SAP5	360
	GO TO 54	SAP5	361
	52 DO 53 I=1,7	SAP5	362
	53 T(I+1)=T(I)	SAP5	363
435	C S(I+1)=S(I)	SAP5	364
	REWIND 2	SAP5	365
	READ (2) NEQ,NBLOCK,NEQB,MBAND,N1,NF,(QQQ(I),I=1,NF)	SAP5	366
	REWIND 7	SAP5	367
	IMAX=NEQB*NF	SAP5	368
440	READ (7) (A(I),I=1,NF)	SAP5	369
	DO 56 L=1,NBLOCK	SAP5	370
	56 READ (7) (A(I),I=1,IMAX)	SAP5	371
	54 CALL HISTRY	SAP5	372
	CALL SECOND(T(9))	SAP5	373
445	T(10) = T(9)	SAP5	374
	T(11) = T(9)	SAP5	375
	T(12) = T(9)	SAP5	376
	C S(10) = S(9)	SAP5	377
	C S(11) = S(9)	SAP5	378
450	C S(12) = S(9)	SAP5	379
	GO TO 90	SAP5	380
	C	SAP5	381
	RESPONSE SPECTRUM ANALYSIS	SAP5	382
	C	SAP5	383
455	60 T(6) = T(5)	SAP5	384
	C S(6) = S(5)	SAP5	385

POOR ORIGINAL

PROGRAM SAP5

76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

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      IF (NTYP.NE.0) GO TO 105
      IF (NDYN.LT.0) GO TO 82
      CALL SOLE10
480      105 CONTINUE
          CALL SECOND(T(7))
          IF (IPLT.GT.0) CALL SAPLOTILL,NF,KOYN,NEQB,NEQ,NBLOCK)
          CALL SECOND(T(8))
          T(9)=T(8)
485      C      S(9) = S(8)
          GO TO 64
      62 DO 63 I=1,8
      63 T(I+1)=T(I)
      C      S(I+1)=S(I)
470      REWIND 2
          READ (2) NEQ,NBLOCK,NEQB,MBAND,N1,NF
          REWIND 7
          IMAX=NEQB*NF
          READ (7) (A(I),I=1,NF)
          DO 66 L=1,NBLOCK
475      66 READ (7) (A(I),I=1,IMAX)
      64 IF (NRSC.LE.0) NRSC=1
          DO 69 KAPG=1,NRSC
480      69 CALL RESPEC
          CALL SECOND(T(10))
          T(11)=T(10)
          T(12)=T(10)
      C      S(11)= S(10)
      C      S(12)= S(10)
485      GO TO 90
      C
      C      STEP-BY-STEP (DIRECT INTEGRATION) ANALYSIS
      C
      70 DO 71 I=6,10
490      71 T(I) = T(5)
      C      S(I) = S(5)
          CALL STEP
          CALL SECOND(T(11))
          T(12)=T(10)
495      C      S(12)=S(10)
          GO TO 90
      C
      C      FREQUENCY RESPONSE ANALYSIS
      C
500      80 T(6)=T(5)
      C      S(6)=S(5)
          IF (NDYN.LT.0) GO TO 82
          CALL SOLE10
          CALL SECOND(T(7))
505      IF (IPLT.GT.0) CALL SAPLOTILL,NF,KOYN,NEQB,NEQ,NBLOCK)
          CALL SECOND(T(8))
          T(9)=T(8)
          T(10)=T(8)
          T(11)=T(8)
510      C      S(9)=S(8)
      C      S(10)=S(8)
      C      S(11)=S(8)
          GO TO 88

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CAM	77
SAP5	385
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POOR ORIGINAL

PROGRAM SAP5

76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

515	82 DO 83 I=1,10	SAP5	442
	83 T(I+1)=T(I)	SAP5	443
	C S(I+1)=S(I)	SAP5	444
	REWIND 2	SAP5	445
	READ (2) NEQ,NBLOCK,NEQB,MBAND,N1,NF	SAP5	446
	REWIND 7	SAP5	447
520	IMAX=NEQB*NF	SAP5	448
	READ (7) (A(I),I=1,NF)	SAP5	449
	DO 86 L=1,NBLOCK	SAP5	450
	86 READ (7) (A(I),I=1,IMAX)	SAP5	451
	88 CALL FREQ	SAP5	452
525	CALL SECOND(T(12))	SAP5	453
	C	SAP5	454
	C COMPUTE AND PRINT OVERALL TIME LOG	SAP5	455
	C	SAP5	456
	90 TT = 0.0	SAP5	457
530	C ST = 0.000	SAP5	458
	GO TO 5	CORR2	6
	DO 95 I=1,11	SAP5	459
	T(I) = T(I+1)-T(I)	SAP5	460
	TT = TT + T(I)	SAP5	461
535	95 CONTINUE	SAP5	462
	C S(I) = S(I+1)-S(I)	SAP5	463
	C ST = ST + S(I)	SAP5	464
	C	SAP5	465
	C WRITE (6,203) (T(K),S(K),K=1,11),TT,ST	SAP5	466
540	C WRITE (6,203) (T(K),K=1,11),TT	SAP5	467
	C	SAP5	468
	GO TO 5	SAP5	469
	C	SAP5	470
545	201 FORMAT (38HIE QUATION PARAMETERS, //	SAP5	471
	X 34H TOTAL NUMBER OF EQUATIONS =,15,	SAP5	472
	X /34H BANDWIDTH =,15,	SAP5	473
	X /34H NUMBER OF EQUATIONS IN A BLOCK =,15,	SAP5	474
	X /34H NUMBER OF BLOCKS =,15)	SAP5	475
	203 FORMAT (11H,31H OVERALL TIME LOG, //	SAP5	476
550	X 5X,30H NODAL POINT INPUT =, F8.2 /	SAP5	477
	X 5X,30H ELEMENT STIFFNESS FORMATION =, F8.2 /	SAP5	478
	X 5X,30H NODAL LOAD INPUT =, F8.2 /	SAP5	479
	X 5X,30H TOTAL STIFFNESS FORMATION =, F8.2 /	SAP5	480
	X 5X,30H STATIC ANALYSIS =, F8.2 /	SAP5	481
555	X 5X,30H EIGENVALUE EXTRACTION =, F8.2 /	SAP5	482
	X 5X,30H STRUCTURE PLOTTING =, F8.2 /	SAP5	483
	X 5X,30H FORCED RESPONSE ANALYSIS =, F8.2 /	SAP5	484
	X 5X,30H RESPONSE SPECTRUM ANALYSIS =, F8.2 /	SAP5	485
	X 5X,30H STEP-BY-STEP INTEGRATION =, F8.2 /	SAP5	486
560	X 5X,30H FREQUENCY RESPONSE ANALYSIS =, F8.2 //	SAP5	487
	X 5X,30H TOTAL SOLUTION TIME =, F8.2 /)	SAP5	488
	C	SAP5	489
	320 FORMAT (// 47H ** WARNING. ESTIMATE OF STORAGE FOR A DYNAMIC,	SAP5	490
	X 32H ANALYSIS EXCEEDS AVAILABLE CORE,	SAP5	491
565	X //, * REQUIRED MTOT=,110)	SAP5	492
	C	SAP5	493
	C	SAP5	494
	1001 FORMAT (1415)	SAP5	495
	2000 FORMAT (*0TOTAL NUMBER OF ELEMENTS =,15, //	SAP5	496
570	X * NUMBER OF ELEMENTS WITH GEOMETRIC STIFFNESS=,15)	SAP5	497
	2010 FORMAT (*0+++ WARNING +++ THE AVAILABLE BLANK COMMENT (MTOT) IS*,	SAP5	498
	X* TOO SMALL*,//, * COMPUTED NO. OF EQUATIONS PER BLOCK=,110, //	SAP5	499
	X* PROCEED IN THE DATA CHECK MODE WITH NEQB=2*)	SAP5	500
	2020 FORMAT(*0+++ FATAL ERROR +++ AVAILABLE CORE IS TOO SMALL*	SAP5	501
575	1,* FOR PLOTTING, REQUIRED MTOT=,110/	SAP5	502
	2* CONTINUE IN THE DATA CHECK MODE*)	SAP5	503
	2030 FORMAT(*0+++ FATAL ERROR +++ AVAILABLE CORE IS INADEQUATE*	SAP5	504
	1,* REQUIRED MTOT=,110/*CONTINUE IN THE DATA CHECK MODE*)	CORR2	7
	END	SAP5	507

SUBROUTINE REDUC1 78/76 OPT=1

FTN 4.5+414

11/10/80 17.24.07

1	SUBROUTINE REDUC1 (ID, IDN, NTP, NUMNP, NRDOF)	REDUC1	2
	DIMENSION MASN(200), MADF(200), ID(NUMNP, 6), IDN(NUMNP, 6)	CAM	78
	LEVEL 2, ID, IDN	REDUC1	4
5	C INPUT PRIMARY DOF AND REORDER ALL DOF SO THAT PRIMARY DOF ARE FIRST	REDUC1	5
	C NTP = 0 NO REDUCTION	REDUC1	6
	C 1 EXACT REDUCTION	REDUC1	7
	C 2 GUYAN REDUCTION	REDUC1	8
	C 3 MODIFIED GUYAN	REDUC1	9
10	C M(1) = NODE NUMBER OF 1 PRIMARY DOF, I=1, NRDOF	REDUC1	10
	C MADF(1) = DOF NUMBER (1 TO 6)	REDUC1	11
	C NRDOF = NUMBER OF PRIMARY DOF < 200	REDUC1	12
	C READ(5, 300) NTP	CAM	80
	300 FORMAT(14I5)	REDUC1	14
15	C IF(NTP.GT.1) GO TO 301	REDUC1	15
	C WRITE(6, 302)	REDUC1	16
	302 FORMAT(5X, 19HNO REDUCTION OF DOF)	REDUC1	17
	C RETURN	REDUC1	18
20	301 IF(NTP.EQ.2) WRITE(6, 303)	REDUC1	19
	C IF(NTP.EQ.3) WRITE(6, 304)	REDUC1	20
	303 FORMAT(5X, 15HGUYAN REDUCTION)	REDUC1	21
	304 FORMAT(5X, 24HMODIFIED GUYAN REDUCTION)	REDUC1	22
	C READ(5, 300) NRDOF, (MASN(I), MADF(I), I=1, NRDOF)	REDUC1	23
	C WRITE(6, 305)	REDUC1	24
25	305 FORMAT(20X, 11HPRIMARY DOF/10X, 4HNODE, 6X, 3HDOF)	REDUC1	25
	C WRITE(6, 306) (MASN(I), MADF(I), I=1, NRDOF)	REDUC1	26
	306 FORMAT(9X, 15, 4X, 15)	REDUC1	27
	C READ ID FROM TAPE B AND WRITE NEW ID ON TAPE B	REDUC1	28
30	C REWIND B	REDUC1	29
	C READ(B) ID	REDUC1	30
	C IP=1	REDUC1	31
	C IS=NRDOF+1	REDUC1	3
35	C *****	REDUC1	33
	C WRITE(6, 307)	REDUC1	34
	307 FORMAT(5X, 13HORIGINAL ID =)	REDUC1	35
	C DO 310 I=1, NUMNP	REDUC1	36
	C WRITE(6, 311) (ID(I, J), J=1, 6)	REDUC1	37
40	311 FORMAT(5X, 6(2X, 15))	REDUC1	38
	C DO 312 J=1, 6	REDUC1	39
	C IF(ID(I, J).EQ.0) GO TO 312	REDUC1	40
	C NODE=MASN(IP)	CAM	81
	C NDOF=MADF(IP)	CAM	82
45	C IF(1.EQ.NODE.AND.J.EQ.NDOF) GO TO 313	CAM	83
	C SECONDARY DOF	CAM	84
	C IDN(I, J)=15	CAM	85
	C IS=IS+1	CAM	86
	C GO TO 312	CAM	87
50	C PRIMARY DOF	CAM	88
	313 IDN(I, J)=IP	CAM	89
	C IP=IP+1	CAM	90
	312 CONTINUE	CAM	91
	310 CONTINUE	CAM	92
55	C WRITE ONTO TAPE B AND PRINT NEW ID	CAM	93
	C REWIND B	REDUC1	58
	C WRITE(B) IDN	REDUC1	59
	C WRITE(6, 320)	REDUC1	60
60	320 FORMAT(5X, 8HNEW ID =)	REDUC1	61
	C DO 321 I=1, NUMNP	REDUC1	62
	C WRITE(6, 311) (IDN(I, J), J=1, 6)	REDUC1	63
	C DO 321 J=1, 6	REDUC1	64
	C ID(I, J)=IDN(I, J)	REDUC1	65
65	321 CONTINUE	REDUC1	66
	C REWIND B	REDUC1	67
	C RETURN	REDUC1	68
	C END	REDUC1	69
		REDUC1	70
		REDUC1	71
		REDUC1	72
		REDUC1	73

SUBROUTINE SIZE

76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

1	SUBROUTINE SIZE(X,XL,MTOT,NEQB,NBLOCK,MBAND)	CAM	94
	COMMON/CONTL/NRDOF,NTYP,NST,MBW,NS,NS1,NS2,NS3,NOT,MBS,MODE,	SIZE	3
	1 MD,MBLRED,NSEQ,NT0	SIZE	4
	DIMENSION X(NEQB,MBAND),XL(NEQB)	CAM	95
5	LEVEL 2,X,XL	CAM	96
	C FIND BANDWIDTH OF KSS	SIZE	7
	MBS=0	SIZE	8
	REWIND 4	SIZE	9
	NPP=NRDOF/NEQB	SIZE	10
10	IROW=0	SIZE	11
	IF(NPP.EQ.0) GO TO 1	SIZE	12
	DO 2 I=1,NPP	SIZE	13
	IROW=IROW+NEQB	SIZE	14
	READ(4) ((X(IC,JC),IC=1,NEQB),JC=1,MBAND),(XL(IL),IL=1,NEQB)	CAM	97
15	2 CONTINUE	SIZE	16
	1 NF=NRDOF-IROW+1	SIZE	17
	NSEC=0	SIZE	18
	3 READ(4) ((X(IC,JC),IC=1,NEQB),JC=1,MBAND),(XL(IL),IL=1,NEQB)	CAM	98
	NSEC=NSEC+NEQB-NF+1	SIZE	20
20	NLST=NEQB	CAM	99
	IF(NSEC.GT.NS) NLST=NEQB-(NSEC-NS)	CAM	100
	IF(NSEC.GT.NS) NSEC=NS	SIZE	21
	DO 4 I=NF,NLST	CAM	101
	MTST=0	CAM	102
25	DO 7 J=1,MBAND	CAM	103
	7 IF(X(I,J).NE.0.) MTST=J	CAM	104
	IF(MTST.GT.MBS) MBS=MTST	CAM	105
	4 CONTINUE	CAM	106
	NF=1	SIZE	25
30	IF(NSEC.NE.NS) GO TO 3	SIZE	26
	REWIND 4	SIZE	27
	NS3=NS*MBS	SIZE	28
	WRITE(6,100) NST,NRDOF,NS,NS1,NS2,NS3,NOT,MBS	SIZE	29
	100 FORMAT(//5X,26H SUMMARY OF SIZE PARAMETERS/	SIZE	30
35	1 5X,34HNUMBER OF UNRESTRAINED DOF (NST) =,15/	SIZE	31
	2 5X,31HNUMBER OF PRIMARY DOF (NRDOF) =,15/	SIZE	32
	3 5X,30HNUMBER OF SECONDARY DOF (NS) =,15/	SIZE	33
	4 5X,19HSIZE OF KPP (NS1) =,15/	SIZE	34
	5 5X,19HSIZE OF KPS (NS2) =,15/	SIZE	35
40	6 5X,19HSIZE OF KSS (NS3) =,15/	SIZE	36
	7 5X,19HSIZE OF TO (NOT) =,15/	SIZE	37
	8 5X,18HBANDWIDTH OF KSS =,15/	SIZE	38
	C DETERMINE BAND SIZE OR KSS	SIZE	39
	C FOR STIFF	SIZE	40
45	NSEQ=(MTOT-NS1-NS2-NEQB*MBW)/MBS	SIZE	41
	C FOR KSS=1	SIZE	42
	NDOU=(MTOT-NS1-NS2-3*NST)/(2*MBS)	SIZE	43
	IF(NDOU.LT.NSEQ) NSEQ=NDOU	SIZE	44
	IF(NSEQ.GE.NS) GO TO 3	SIZE	45
50	MBLRED=NS/NSEQ+1	SIZE	46
	GO TO 8	SIZE	47
	5 NSEQ=NS	SIZE	48
	MBLRED=1	SIZE	49
	6 NT0=(MTOT-2*NS1-NS2-NS-NST-NRDOF)/NRDOF	SIZE	50
55	IF(NT0.GT.NS) NT0=NS	SIZE	51
	WRITE(6,101) MBLRED,NSEQ,NT0	SIZE	52
	101 FORMAT(//5X,21HNO OF BLOCKS IN KSS =,15/	SIZE	53
	1 5X,24HNO OF EQUATIONS IN KSS =,15/	SIZE	54
	2 5X,23HNO OF EQUATIONS IN TO =,15/	SIZE	55
60	NA4=2*NS1+NRDOF*(MD+NRDOF+1+MD)	SIZE	56
	WRITE(6,102) NA4	SIZE	57
	102 FORMAT(5X,27HSIZE OF A DURING EIGENSOL =,15/	SIZE	58
	RETURN	SIZE	59
	END	SIZE	60

1		SUBROUTINE STIFF(XKPP,XKPS,XKSS,X,XL,NBLOCK,NEQB,NROOF,NTYP,NST,	CAM	107
		1 MBW,NS,NS1,NS2,NS3,NOT,MBS,MODE,MO,MBLRED,NSEQ,NT0)	STIFF	3
		DIMENSION XKPP(NS1),XKPS(NS2),XKSS(NSEQ,MBS),X(NEQB,MBW),XL(NEQB)	CAM	108
		LEVEL 2,XKPP,XKPS,XKSS,X,XL	CAM	109
5	C		STIFF	6
	C	FORM KPP KPS,KSS FROM TOTAL STIFFNESS ON TAPE 4 WRITE KSS ON	STIFF	7
	C	TAPE 18 IN BLOCKS	STIFF	8
	C		STIFF	9
		REWIND 18	STIFF	10
10		ILPP=0	STIFF	11
		ILPS=0	STIFF	12
		IRS=0	STIFF	13
		IBS=0	STIFF	14
		IROW=0	STIFF	15
15		DO 1 I=1,NBLOCK	STIFF	16
		READ(4) ((X(IC,JC),IC=1,NEQB),JC=1,MBW),(XL(IL),IL=1,NEQB)	CAM	110
		DO 2 I=1,NEQB	STIFF	18
		IROW=IROW+1	STIFF	19
		IF(IROW.GT.NST) GO TO 2	STIFF	20
20		IF(IROW.GT.NROOF) GO TO 3	STIFF	21
	C		STIFF	22
	C	KPP OR KPS	STIFF	23
		DO 4 J=1,MBW	STIFF	24
		IF(J+IROW-1.GT.NROOF) GO TO 5	STIFF	25
25	C	KPP	STIFF	26
		ILPP=ILPP+1	STIFF	27
		XKPP(ILPP)=X(I,J)	STIFF	28
		GO TO 4	STIFF	29
	C	KPS	STIFF	30
30		5 ILPS=ILPS+1	STIFF	31
		XKPS(ILPS)=X(I,J)	STIFF	32
		4 CONTINUE	STIFF	33
		GO TO 2	STIFF	34
	C	KSS	STIFF	35
35		3 IRS=IRS+1	STIFF	36
		DO 6 J=1,MBS	STIFF	37
		6 XKSS(IRS,J)=X(I,J)	STIFF	38
		IF(IRS.LT.NSEQ) GO TO 2	STIFF	39
		WRITE(18) XKSS	STIFF	40
40		IRS=0	STIFF	41
		2 CONTINUE	STIFF	42
		1 CONTINUE	STIFF	44
		REWIND 18	STIFF	45
		RETURN	STIFF	86
45		END	STIFF	87

POOR ORIGINAL

1		SUBROUTINE MASS(XM,NEQB,NBLOCK,NST)	MASS	2
		DIMENSION XM(NST)	MASS	3
		LEVEL 2,XM	MASS	4
	C		MASS	5
5	C	LOAD MASS VECTOR	MASS	6
	C		MASS	7
		REWIND 9	MASS	8
		IROW=1	CAM	111
		DO 1 I=1,NBLOCK	CAM	112
10		JLST=IROW+NEQB-1	CAM	113
		IF(JLST.GT.NST) JLST=NST	CAM	114
		READ(9) (XM(J),J=IROW,JLST)	CAM	115
		IROW=JLST+1	CAM	116
		1 CONTINUE	CAM	117
15		REWIND 9	MASS	10
		RETURN	MASS	17
		END	MASS	18

POOR ORIGINAL

SUBROUTINE GAUS

76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

1	SUBROUTINE GAUS(A1,A2,B,G,NSB,NST,MBS,NBLK,NT1,NT2,NT3,INDEX)	GAUS	2
	DIMENSION A1(NSB,MBS),A2(NSB,MBS),B(NST),G(NST)	GAUS	3
	LEVEL 2,A1,A2,B,G	GAUS	4
	C	GAUS	5
5	C INDEX = 1 FIND INVERSE OF A (ON TAPE NT1) STORE RESULTS ON TAPE NT3	GAUS	6
	C 2 FACTOR A STORE RESULTS ON TAPE NT2	GAUS	7
	C 3 SOLVE A X = B ENTER WITH 2 FIRST TO FACTOR A	GAUS	8
	C IF (INDEX.EQ.3) GO TO 1	GAUS	9
10	C FACTOR A	GAUS	10
	C	GAUS	11
	REWIND NT1	GAUS	12
	REWIND NT2	GAUS	13
	N1=MBS-1	GAUS	14
15	N2=NSB-MBS+1	GAUS	15
	IBMX=NBLK-1	GAUS	16
	IF(NBLK.EQ.1) IBMX=1	GAUS	17
	DO 10 IB=1,IBMX	GAUS	18
	IF(IB.EQ.1) READ(NT1) A1	GAUS	19
20	IF(NBLK.NE.1) READ(NT1) A2	GAUS	20
	JSTRT=2	GAUS	21
	IF(IB.GT.1) JSTRT=JLST+1	GAUS	22
	JLST=2.*NSB	GAUS	23
	IF(IB.GT.1) JLST=JSTRT+N1+N2-1	GAUS	24
25	IF(JLST.GT.NST) JLST=NST	GAUS	25
	DO 11 J=JSTRT,JLST	GAUS	26
	MSK=J-MBS	GAUS	27
	IF(MSK.LT.1) MSK=1	GAUS	28
30	CALL LOCATE(MSK,J,A,A1,A2,MBS,NSB,IB)	GAUS	29
	G(MSK)=A	GAUS	30
	IF=MSK+1	GAUS	31
	IL=J-1	GAUS	32
	DO 12 I=IF,IL	GAUS	33
	CALL LOCATE(I,J,A,A1,A2,MBS,NSB,IB)	GAUS	34
35	G(I)=A	GAUS	35
	KF=MSK	GAUS	36
	IF(I-MBS.GT.KF) KF=I-MBS	GAUS	37
	KL=I-1	GAUS	38
	IF(KF.GT.KL) GO TO 12	GAUS	39
40	DO 13 K=KF,KL	GAUS	40
	CALL LOCATE(K,I,A,A1,A2,MBS,NSB,IB)	GAUS	41
	13 G(I)=G(I)-A*G(K)	GAUS	42
	12 CONTINUE	GAUS	43
	IF=J-MBS	GAUS	44
45	IF(IF.LT.1) IF=1	GAUS	45
	IL=J-1	GAUS	46
	DO 14 I=IF,IL	GAUS	47
	KA=I-(I-1)*NSB	GAUS	48
	IF(KA.GT.NSB) GO TO 15	GAUS	49
50	LA=J-(I-1)*NSB-KA+1	GAUS	50
	IF(LA.GT.MBS) GO TO 14	GAUS	51
	A1(KA,LA)=G(I)/A1(KA,1)	CAM	118
	GO TO 14	GAUS	52
55	15 KA=I-18*NSB	GAUS	53
	LA=J-18*NSB-KA+1	GAUS	54
	IF(LA.GT.MBS) GO TO 14	GAUS	55
	A2(KA,LA)=G(I)/A2(KA,1)	CAM	119
		GAUS	56

POOR ORIGINAL

SUBROUTINE GAUS

78/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

60	14 CONTINUE	GAUS	57
	KF=J-MBS	GAUS	58
	IF(KF.LT.1) KF=1	GAUS	59
	KL=J-1	GAUS	60
	KA=J-(18-1)*NSB	GAUS	61
	DO 18 K=KF, KL	GAUS	62
65	CALL LOCATE(K, J, A, A1, A2, MBS, NSB, 18)	GAUS	63
	IF(KA.GT.NSB) GO TO 17	GAUS	64
	A1(KA, 1)=A1(KA, 1)-A*G(K)	GAUS	65
	GO TO 16	GAUS	66
	17 KA=J-18*NSB	GAUS	67
	A2(KA, 1)=A2(KA, 1)-A*G(K)	GAUS	68
70	16 CONTINUE	GAUS	69
	11 CONTINUE	GAUS	70
	WRITE(NT2) A1	GAUS	71
	IF(NBLK.EQ.1) GO TO 10	GAUS	72
	DO 18 IT=1, NSB	GAUS	73
75	DO 18 JT=1, MBS	GAUS	74
	18 A1(IT, JT)=A2(IT, JT)	GAUS	75
	10 CONTINUE	GAUS	76
	REWIND NT1	GAUS	77
	REWIND NT2	GAUS	78
80	IF(INDEX.EQ.2) RETURN	GAUS	79
	REWIND NT3	GAUS	80
	C SET UP LOAD FOR INVERT STORE RESULTS ON TAPE NT3	GAUS	81
	DO 20 ICOL=1, NST	GAUS	82
	DO 21 J=1, NST	GAUS	83
85	21 B(J)=0.	GAUS	84
	B(ICOL)=1.	GAUS	85
	GO TO 1	GAUS	86
	22 WRITE(NT3) B	GAUS	87
90	20 CONTINUE	GAUS	88
	REWIND NT3	GAUS	89
	RETURN	GAUS	90
	C	GAUS	91
	C SOLVE A X = B HAVING FACTORED A ON TAPE NT2 RETURN SOLUTION IN B	GAUS	92
	C	GAUS	93
95	C REDUCE B	GAUS	94
	1 G(I)=B(I)	GAUS	95
	IBMX=NBLK-1	GAUS	96
	IF(NBLK.EQ.1) IBMX=1	GAUS	97
	DO 30 IB=1, IBMX	GAUS	98
100	IF(IB.EQ.1) READ(NT2) A1	GAUS	99
	IF(NBLK.NE.1) READ(NT2) A2	GAUS	100
	JSTRT=2	GAUS	101
	IF(IB.GT.1) JSTRT=JLST+1	GAUS	102
	JLST=2.*NSB	GAUS	103
105	IF(IB.GT.1) JLST=JSTRT+N1+N2-1	GAUS	104
	IF(JLST.GT.NST) JLST=NST	GAUS	105
	DO 31 I=JSTRT, JLST	GAUS	106
	G(I)=B(I)	GAUS	107
	KS=1-MBS	GAUS	108
110	IF(KS.LT.1) KS=1	GAUS	109
	KL=1-1	GAUS	110
	DO 32 K=KS, KL	GAUS	111
	CALL LOCATE(K, 1, A, A1, A2, MBS, NSB, 18)	GAUS	112
	32 G(I)=G(I)-A*G(K)	GAUS	113

POOR ORIGINAL

SUBROUTINE GAUS		76/76	OPT=1	FTN 4.5+414	11/19/80	17.24.07
115	31	CONTINUE			GAUS	114
	DO 33	IT=1, NSB			GAUS	115
	DO 33	JT=1, MBS			GAUS	116
	33	A1(IT, JT)=A2(IT, JT)			GAUS	117
	30	CONTINUE			GAUS	118
120	C	BACKSUBSTITUTE			GAUS	119
		REWIND NT2			GAUS	120
		IROW=0			GAUS	121
		DO 40 IB=1, NBLK			GAUS	122
		READ(NT2) A1			GAUS	123
125		IMX=NSB			GAUS	124
		IF(IROW+IMX.GT.NST) IMX=NST			GAUS	125
		IFST=IROW+1			GAUS	126
		ILST=IROW+NSB			GAUS	127
		DO 41 I=IFST, ILST			GAUS	128
130	41	G(I)=G(I)/A1(I-IROW, I)			GAUS	129
		IROW=IROW+ILST-IFST+1			GAUS	130
	40	CONTINUE			GAUS	131
		REWIND NT2			GAUS	132
		B(NST)=G(NST)			GAUS	133
135		IB=NBLK-1			GAUS	134
		IF(IB.LT.1) IB=1			GAUS	135
	47	IDUM=IB-1			GAUS	136
		IF(IDUM.LE.0) GO TO 42			GAUS	137
		DO 43 I=1, IDUM			GAUS	138
140	43	READ(NT2) A1			GAUS	139
	42	READ(NT2) A1			GAUS	140
		IF(NBLK.EQ.1) GO TO 44			GAUS	141
		READ(NT2) A2			GAUS	142
145	44	ISTRT=(IB-1)*NSB+NI+1			GAUS	143
		ILST=ISTRT+NI+2*M2			GAUS	144
		IF(ILST.GT.NST) ILST=NST			GAUS	145
		IF(IB.EQ.1) ISTRT=2			GAUS	146
		NI=ILST-ISTRT+1			GAUS	147
		DO 45 I=1, NI			GAUS	148
150		IROW=ILST-I+1			GAUS	149
		KS=IROW-MBS			GAUS	150
		IF(KS.LT.1) KS=1			GAUS	151
		KL=IROW-1			GAUS	152
		DO 46 K=KS, KL			GAUS	153
155		CALL LOCATE(K, IROW, A, A1, A2, MBS, NSB, IB)			GAUS	154
	46	G(K)=G(K)-A*B(IROW)			GAUS	155
		B(IROW-1)=G(IROW-1)			GAUS	156
	45	CONTINUE			GAUS	157
		IB=IB-1			GAUS	158
160		REWIND NT2			GAUS	159
		IF(IB.GT.0) GO TO 47			GAUS	160
		IF(INDEX.EQ.1) GO TO 22			GAUS	161
		RETURN			GAUS	162
		END			GAUS	163

SUBROUTINE LOCATE		76/76	OPT=1	FTN 4.5+414	11/19/80	17.24.07
1	SUBROUTINE LOCATE(I,J,A,A1,A2,MBS,NSB,IB)				LOCATE	2
	DIMENSION A1(NSB,MBS),A2(NSB,MBS)				LOCATE	3
	LEVEL 2,A1,A2				LOCATE	4
5	ILOW=(IB-1)*NSB				LOCATE	5
	IHIGH=IB*NSB				LOCATE	6
	INDEX=3				LOCATE	7
	IF(I.GT.ILOW.AND.I.LE.IHIGH) INDEX=1				LOCATE	8
	ISHG=(IB+1)*NSB				LOCATE	9
10	IF(I.GT.IHIGH.AND.I.LE.ISHG) INDEX=2				LOCATE	10
	GO TO (1,2,3),INDEX				LOCATE	11
	1 K=1-(IB-1)*NSB				LOCATE	12
	L=J-(IB-1)*NSB-K+1				LOCATE	13
	IF(L.GT.MBS) GO TO 5				CAM	120
15	A=A1(K,L)				LOCATE	14
	RETURN				LOCATE	15
	2 K=1-IB*NSB				LOCATE	16
	L=J-IB*NSB-K+1				LOCATE	17
	IF(L.GT.MBS) GO TO 5				CAM	121
20	A=A2(K,L)				LOCATE	18
	RETURN				LOCATE	19
	5 A=0.				CAM	122
	RETURN				CAM	123
	3 WRITE(6,4) I,J,IB				LOCATE	20
25	4 FORMAT(//5X,39#ERROR IN LOCATE ELEMENT NOT IN A1 OR A2/				LOCATE	21
	1 10X,3H) =.15/10X,3HJ =.15/10X,4HIB =.15)				LOCATE	22
	STOP				LOCATE	23
	END				LOCATE	24

SUBROUTINE PPLOC		76/76	OPT=1	FTN 4.5+414	11/19/80	17.24.07
1	SUBROUTINE PPLOC(IR,JC,ILOC)				PPLOC	2
	COMMON/CONTL/ NDOF,NTYP,NST,MBS,NS,NS1,NS2,NS3,NOT,MBS,MODE,				PPLOC	3
	1 MD,MBS,RED,NSEQ,NT0				PPLOC	4
5	C LOCATES I,J IN VECTOR STORING KPP				PPLOC	5
	IDR=IR				PPLOC	6
	JDC=JC				PPLOC	7
	IF(IR.LT.JC) GO TO 3				PPLOC	8
	IDR=JC				PPLOC	9
10	JDC=IR				PPLOC	10
	3 CONTINUE				PPLOC	11
	NL=NDOF				PPLOC	12
	ILOC=0				PPLOC	13
	IS=IDR-1				PPLOC	14
	IF(IS.EQ.0) GO TO 1				PPLOC	15
15	DO 2 I=1,IS				PPLOC	16
	ILOC=ILOC+NL				PPLOC	17
	2 NL=NL-1				PPLOC	18
	1 ILOC=ILOC+JDC-IDR+1				PPLOC	19
20	RETURN				PPLOC	20
	END				PPLOC	21

SUBROUTINE REDUCE 78/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

1	SUBROUTINE REDUCE(XKPP,XKPS,XH,XMR,XSS,T1,T0,	REDUCE	2
	1 NRDOF,NTYP,NST,MNW,NS,NS1,NS2,NS3,NOT,MBS,MODE,	REDUCE	3
	2 MD,MBLRED,NSEQ,NT0)	REDUCE	4
	DIMENSION XKPP(NS),XKPS(NS2),XH(NST),XMR(NS),XSS(NS),T1(NRDOF),	REDUCE	5
5	1 T0(NT0,NRDOF)	REDUCE	6
	LEVEL 2,XKPP,XKPS,XH,XMR,XSS,T1,T0	REDUCE	7
	C	REDUCE	8
	C	REDUCE	9
	C	REDUCE	10
10	FORM T0 = KSS-1 * KSP ONE BLOCK AT A TIME AND STORE ON 18	REDUCE	11
	MBL=NS/NT0+1	CAM	124
	IF(NS.EQ.NT0) MBL=1	REDUCE	2
	REWIND 18	REDUCE	13
	REWIND 17	REDUCE	14
15	DO 1 I=1,MBL	REDUCE	15
	DO 2 I=1,NT0	REDUCE	16
	DO 2 J=1,NRDOF	REDUCE	17
	2 T0(I,J)=0.	REDUCE	18
	DO 3 I=1,NT0	REDUCE	19
	IROW=(I-1)*NT0+1	REDUCE	20
20	IF(IROW.GT.NS) GO TO 3	REDUCE	21
	READ(17) XSS	REDUCE	22
	DO 5 J=1,NRDOF	REDUCE	23
	DO 5 K=1,NS	REDUCE	24
	CALL PSLOC(2,K,J,ILOC)	CAM	125
25	IF(ILOC.EQ.0) GO TO 5	CAM	126
	T0(I,J)=T0(I,J)+XSS(K)*XKPS(ILOC)	CAM	127
	5 CONTINUE	REDUCE	26
	3 CONTINUE	REDUCE	27
	1 WRITE(18) T0	REDUCE	28
30	REWIND 17	REDUCE	29
	REWIND 18	REDUCE	30
	C	REDUCE	31
	C	REDUCE	32
	C	REDUCE	33
35	FORM T1 = KSS-1 * MSS * T0 ONE ROW AT A TIME AND SAVE ON 16	REDUCE	34
	REWIND 18	REDUCE	35
	DO 10 I=1,NS	REDUCE	36
	READ(17) XSS	CAM	128
	DO 11 I=1,NS	REDUCE	38
40	11 XSS(I)=XSS(I)*XH(I+NRDOF)	REDUCE	39
	DO 12 J=1,NRDOF	REDUCE	40
	12 T1(J)=0.	REDUCE	41
	DO 13 I=1,MBL	REDUCE	42
	READ(18) T0	REDUCE	43
45	DO 14 J=1,NRDOF	REDUCE	44
	DO 14 K=1,NT0	REDUCE	45
	KROW=(I-1)*NT0+K	REDUCE	46
	IF(KROW.GT.NS) GO TO 14	REDUCE	47
	T1(J)=T1(J)+XSS(KROW)*T0(K,J)	REDUCE	48
50	14 CONTINUE	REDUCE	49
	13 CONTINUE	REDUCE	50
	WRITE(16) T1	REDUCE	51
	REWIND 18	REDUCE	52
55	10 CONTINUE	CAM	129
	REWIND 18	REDUCE	53
	REWIND 17	REDUCE	54
	C		
	C	COMPUTE K* = KPP KPS * T0 AND STORE IN KPP	

POOR ORIGINAL

SUBROUTINE REDUCE

78/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

	C		REDUCE	55
		DO 20 I=1,NBL	REDUCE	56
60		READ(I) TO	REDUCE	57
		DO 21 K=1,NROOF	REDUCE	58
		DO 21 J=K,NROOF	REDUCE	59
		CALL PPLOC(K,J,ILOC)	REDUCE	60
		DO 22 L=1,NT0	REDUCE	61
65		LROW=(I-1)*NT0+L	REDUCE	62
		IF(LROW.GT.NS) GO TO 22	REDUCE	63
		CALL PSLOC(I,K,LROW,INDEX)	REDUCE	64
		IF(INDEX.EQ.0) GO TO 22	REDUCE	65
		XKPP(ILOC)=XKPP(ILOC)-XKPS(INDEX)*T0(L,J)	REDUCE	66
70	22	CONTINUE	REDUCE	67
	21	CONTINUE	REDUCE	68
	20	CONTINUE	REDUCE	69
		REWIND I8	REDUCE	70
	C		REDUCE	71
75	C	COMPUTE M* = MPP + KPS * T1 AND STORE IN XMR	REDUCE	72
	C		REDUCE	73
		DO 30 I=1,NS1	REDUCE	74
	30	XMR(I)=0.	CAM	130
		DO 31 I=1,NROOF	REDUCE	76
80		CALL PPLOC(I,I,ILOC)	REDUCE	77
	31	XMR(ILOC)=XM(I)	REDUCE	78
		DO 32 L=1,NS	REDUCE	79
		READ(I6) T1	REDUCE	80
		DO 33 K=1,NROOF	REDUCE	81
85		DO 33 J=K,NROOF	REDUCE	82
		CALL PSLOC(I,K,L,INDEX)	REDUCE	83
		IF(INDEX.EQ.0) GO TO 33	REDUCE	84
		CALL PPLOC(K,J,ILOC)	REDUCE	85
		XMR(ILOC)=XMR(ILOC)+XKPS(INDEX)*T1(J)	REDUCE	86
90	33	CONTINUE	REDUCE	87
	32	CONTINUE	REDUCE	88
		REWIND I6	REDUCE	89
		RETURN	REDUCE	134
		END	REDUCE	135

1		SUBROUTINE PSLOC(I1,I,J,ILOC)	PSLOC	2
		COMMON/CONT1/ NDOF,NTYP,NST,MBW,NS,NS1,NS2,NS3,NCT,MB5,MODE,	PSLOC	3
		1 MD,MBLRED,NSEQ,NT0	PSLOC	4
5	C	LOCATE I,J COMPONENT OF KPS IN A IF I1=1 OR J,1 COMPONENT	PSLOC	5
	C	OF KSP IF I1=2 ... ILOC IS LOCATION OF COMPONENT IN A..IF	PSLOC	6
	C	ILOC=0 COMPONENT IS 0	PSLOC	7
	C		PSLOC	8
		ILOC=0	PSLOC	9
10		IF(I1.EQ.2) GO TO 1	PSLOC	10
		IR=1	PSLOC	11
		JC=J	PSLOC	12
		GO TO 2	PSLOC	13
		1 IR=J	PSLOC	14
15		JC=1	PSLOC	15
		2 IF(NDOF.GT.MBW) GO TO 10	PSLOC	16
		IF(JC.GT.MBW-NDOF+(IR-1)) RETURN	PSLOC	17
		IL=MBW-NDOF-1	PSLOC	18
		IF(IR.EQ.1) GO TO 4	PSLOC	19
20		IN=IR-1	PSLOC	20
		DO 3 IS=1,IN	PSLOC	21
		IL=IL+1	PSLOC	22
		3 ILOC=ILOC+IL	PSLOC	23
		4 ILOC=ILOC+JC	PSLOC	24
25		RETURN	PSLOC	25
	C		PSLOC	26
		10 IF(IR.LT.NDOF-MBW+1) RETURN	PSLOC	27
		IF(JC.GT.IR-NDOF+MBW) RETURN	PSLOC	28
		KMIN=NDOF-MBW+1	PSLOC	29
30		IF(IR.LE.KMIN) GO TO 11	PSLOC	30
		IN=IR-1	PSLOC	31
		IL=0	PSLOC	32
		DO 12 IS=KMIN,IN	PSLOC	33
		IL=IL+1	PSLOC	34
35		12 ILOC=ILOC+IL	PSLOC	35
		11 ILOC=ILOC+JC	PSLOC	36
		RETURN	PSLOC	37
		END	PSLOC	38
			PSLOC	39

POOR ORIGINAL

1		SUBROUTINE GRAMS(XM,EV,B,DUM,IM,NS1,NDOF,MODE)	CAM	309
		DIMENSION EV(NDOF,MODE),B(NDOF),DUM(NDOF),ALPH(20),XM(NS1)	CAM	310
		LEVEL 2,XM,EV,B,DUM	CAM	311
5	C	COMPUTE NEW B ORTHOGONAL TO ALL LOWER MODES	CAM	312
	C	BNEW=BOLD - SUM ((EV1 * M * BOLD) * EV)	CAM	313
	C		CAM	314
		M=IM-1	CAM	315
		DO 1 I=1,M	CAM	316
10		DO 2 J=1,NDOF	CAM	317
		2 DUM(J)=0.	CAM	318
		DO 3 J=1,NDOF	CAM	319
		DO 3 K=1,NDOF	CAM	320
		CALL PPLOC(J,K,ILOC)	CAM	321
15		3 DUM(J)=DUM(J)+XM(ILOC)*B(K)	CAM	322
		ALPH(I)=0.	CAM	323
		DO 4 J=1,NDOF	CAM	324
		4 ALPH(I)=ALPH(I)+EV(J,I)*DUM(J)	CAM	325
		1 CONTINUE	CAM	326
20		DO 5 J=1,NDOF	CAM	327
		DO 5 I=1,M	CAM	328
		5 B(J)=B(J)-ALPH(I)*EV(J,I)	CAM	329
		RETURN	CAM	330
		END	CAM	331
			CAM	332

POOR ORIGINAL

SUBROUTINE INVERT 76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

1	SUBROUTINE INVERT(N,MB,A,B,JS,INDEX)	INVERT	2
	DIMENSION A(N,MB),MS(1000),O(1000),B(N)	INVERT	3
	LEVEL 2,A,B	INVERT	4
	C	INVERT	5
5	PROGRAM TO OBTAIN A-1 USING GAUSS ELIMINATION AS OUTLINED	INVERT	6
	C IN BATHE-WILSON PAGES 246-258....A STORED IN BANDED FORM	INVERT	7
	C N EQUATIONS MB BANDWIDTH...INVERSE STORED BANDED IN B	INVERT	8
	C	INVERT	9
	C INDEX= 1 FIND INVERSE OF A ONE COL (JS) AT A TIME	INVERT	10
10	C = 2 FACTOR A	INVERT	11
	C = 3 FIND SOL A X = B...ENTER WITH 2 FIRST TO FACTOR A	INVERT	12
	IF (INDEX.EQ.3) GO TO 21	INVERT	13
	IF (JS.GT.1.AND.INDEX.EQ.1) GO TO 15	INVERT	14
	IF (N.LE.1000) GO TO 16	INVERT	15
15	WRITE(6,100) N	INVERT	16
	100 FORMAT(//5X,27HNO OF EQUATIONS IN INVERT =,15,2X	INVERT	17
	1,12HEXCEEDS 1000)	INVERT	18
	STOP	INVERT	19
	16 CONTINUE	INVERT	20
20	C CALC SKYLINE FOR ALL COLUMNS	INVERT	21
	DO 1 J=1,MB	INVERT	22
	1 MS(J)=1	INVERT	23
	JC=MB+1	INVERT	24
	DO 2 J=JC,N	INVERT	25
25	2 MS(J)=MS(J-1)+1	INVERT	26
	C REDUCE A	INVERT	27
	DO 5 J=2,N	INVERT	28
	JLOC=J-MS(J)+1	INVERT	29
	G(MS(J))=A(MS(J),JLOC)	INVERT	30
30	IS=MS(J)+1	INVERT	31
	IL=J-1	INVERT	32
	IF (IS.GT.IL) GO TO 10	INVERT	33
	DO 6 I=IS,IL	INVERT	34
	ILOC=J-1+I	INVERT	35
35	G(I)=A(I,ILOC)	INVERT	36
	MM=MS(J)	INVERT	37
	IF (MS(I).GT.MM) MM=MS(I)	INVERT	38
	KL=I-1	INVERT	39
	IF (MM.GT.KL) GO TO 6	INVERT	40
40	DO 7 KR=MM,KL	INVERT	41
	7 G(I)=G(I)-A(KR,I-KR+1)*G(KR)	INVERT	42
	6 CONTINUE	INVERT	43
	10 CONTINUE	INVERT	44
	C	INVERT	45
45	IS=MS(J)	INVERT	46
	IF (IS.GT.IL) GO TO 11	INVERT	47
	DO 8 I=IS,IL	INVERT	48
	ILOC=J-1+I	INVERT	49
50	8 A(I,ILOC)=G(I)/A(I,I)	INVERT	50
	C	INVERT	51
	11 CONTINUE	INVERT	52
	IF (IS.GT.IL) GO TO 5	INVERT	53
	DO 9 KR=IS,IL	INVERT	54
	9 A(J,I)=A(J,I)-A(KR,J-KR+1)*G(KR)	INVERT	55
55	C	INVERT	56
	5 CONTINUE	INVERT	57
	IF (INDEX.EQ.2) RETURN	INVERT	58

POOR ORIGINAL

SUBROUTINE INVERT

78/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

	21 IF (INDEX.EQ.3) GO TO 23	INVERT	59
	C FORM INVERSE	INVERT	60
60	15 CONTINUE	INVERT	61
	J=JS	INVERT	62
	C REDUCE RHS	INVERT	63
	DO 23 I=1,N	INVERT	64
	23 B(I)=0.	INVERT	65
65	B(J)=1.	INVERT	66
	22 G(I)=W(I)	INVERT	67
	DO 41 I=2,N	INVERT	68
	G(I)=B(I)	INVERT	69
	KS=MS(I)	INVERT	70
70	KL=I-1	INVERT	71
	IF (KS.GT.KL) GO TO 41	INVERT	72
	DO 42 K=KS,KL	INVERT	73
	ILOC=I-K+1	INVERT	74
	42 G(I)=G(I)-A(K,ILOC)*G(K)	INVERT	75
75	41 CONTINUE	INVERT	76
	C BACKSUBSTITUTE TO GENERATE J COL OF A-1 FROM N TO 1	INVERT	77
	DO 49 I=1,N	INVERT	78
	49 G(I)=G(I)/A(I,I)	INVERT	79
	B(N)=G(N)	INVERT	80
80	IT=N-1	INVERT	81
	IF (IT.EQ.0) GO TO 40	INVERT	82
	DO 45 IN=1,IT	INVERT	83
	I=N+1-IN	INVERT	84
	KS=MS(I)	INVERT	85
85	KL=I-1	INVERT	86
	DO 46 K=KS,KL	INVERT	87
	46 G(K)=G(K)-A(K,I-K+1)*B(I)	INVERT	88
	B(I-K+1)=G(I-K+1)	INVERT	89
	45 CONTINUE	INVERT	90
90	40 CONTINUE	INVERT	91
	RETURN	INVERT	92
	END	INVERT	93

POOR ORIGINAL

SUBROUTINE EXPAND 76/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

1	SUBROUTINE EXPAND(EV,TO,TI,ESAP,NRDOF,MD,MODE,NT0,NEQB,NBLOCK,	CAM	131
	1 NTYP,NS,XM,NST)	CAM	132
	COMMON/EVAL/EF(20)	CAM	133
	DIMENSION EV(NRDOF,MD),TO(NT0,NRDOF),TI(NRDOF),ESAP(NEQB,MODE)	CAM	134
5	1 ,XM(NST),XMTOT(20)	CAM	135
	LEVEL 2,EV,TO,TI,ESAP,XM	CAM	136
	C	CAM	137
	C EXPAND MODES AND WRITE ONTO 7 IN NBLOCKS	CAM	138
	C	CAM	139
10	C	CAM	140
	C STORE TO ONE ROW AT A TIME ON 17	CAM	141
	REWIND 18	CAM	142
	REWIND 17	CAM	143
	NBL=NS/NT0+1	CAM	144
15	IF(NS.EQ.NT0) NBL=1	CAM	145
	NROW=0	CAM	146
	DO 1 IB=1,NBL	CAM	147
	READ(18) TO	CAM	148
	DO 2 I=1,NT0	CAM	149
20	IF(NROW+1.GT.NS) GO TO 2	CAM	150
	WRITE(17) (TO(I,J),J=1,NRDOF)	CAM	151
	2 CONTINUE	CAM	152
	1 NROW=NROW+NT0	CAM	153
	C	CAM	154
25	C EXPAND MODES	CAM	155
	C	CAM	156
	REWIND 16	CAM	157
	REWIND 17	CAM	158
	REWIND 18	CAM	159
30	NROW=0	CAM	160
	DO 10 IB=1,NBLOCK	CAM	161
	DO 12 IRW=1,NEQB	CAM	162
	DO 16 IFR=1,MODE	CAM	163
	16 ESAP(IRW,IFR)=0.	CAM	164
35	IF(NROW+IRW.GT.NS+NRDOF) GO TO 12	CAM	165
	IF(NROW+IRW.GT.NRDOF) GO TO 13	CAM	166
	DO 20 IFR=1,MODE	CAM	167
	20 ESAP(IRW,IFR)=EV(IRW+NROW,IFR)	CAM	168
	GO TO 12	CAM	169
40	13 READ(17) (TO(I,J),J=1,NRDOF)	CAM	170
	DO 14 L=1,NRDOF	CAM	171
	DO 14 IFR=1,MODE	CAM	172
	14 ESAP(IRW,IFR)=ESAP(IRW,IFR)-TO(I,L)*EV(L,IFR)	CAM	173
	IF(NTYP.EQ.2) GO TO 12	CAM	174
45	READ(18) TI	CAM	175
	DO 15 L=1,NRDOF	CAM	176
	DO 15 IFR=1,MODE	CAM	177
	EIV=(EF(IFR)*6.28)**2.	CAM	178
	15 ESAP(IRW,IFR)=ESAP(IRW,IFR)-EIV*TI(L)*EV(L,IFR)	CAM	179
50	12 CONTINUE	CAM	180
	WRITE(18) ESAP	CAM	181
	10 NROW=NROW+NEQB	CAM	182
	C NORMALIZE VECTORS AND WRITE ON 7	CAM	183
	DO 30 I=1,NST	CAM	184
55	30 READ(15) XM(I)	CAM	185
	REWIND 15	CAM	186
	REWIND 18	CAM	187

POOR ORIGINAL

SUBROUTINE EXPAND

76/76 OPT=1

FTN 4.5+414

11/19/80 17.24.07

	DO 31 I=1,MODE	CAM	188
	31 XMTOT(I)=0.	CAM	189
60	IRON=0	CAM	190
	DO 32 IB=1,NBLOCK	CAM	191
	READ(18) ESAP	CAM	192
	DO 33 JM=1,MODE	CAM	193
	DO 33 I=1,NEQB	CAM	194
65	NROW=IROW+1	CAM	195
	IF(NROW.GT.NST) GO TO 33	CAM	196
	XMTOT(JM)=XMTOT(JM)+ESAP(I,JM)*ESAP(I,JM)*XM(NROW)	CAM	197
	33 CONTINUE	CAM	198
	IROW=IROW+NEQB	CAM	199
70	32 CONTINUE	CAM	200
	REWIND 18	CAM	201
	IRON=0	CAM	202
	DO 34 IB=1,NBLOCK	CAM	203
	READ(18) ESAP	CAM	204
75	DO 35 JM=1,MODE	CAM	205
	DO 35 I=1,NEQB	CAM	206
	IF(IROW+1.GT.NST) GO TO 35	CAM	207
	ESAP(I,JM)=ESAP(I,JM)/SQRT(XMTOT(JM))	CAM	208
	35 CONTINUE	CAM	209
80	IROW=IROW+NEQB	CAM	210
	WRITE(7) ESAP	CAM	211
	34 CONTINUE	CAM	212
	RETURN	CAM	213
	END	CAM	214

POOR ORIGINAL

SUBROUTINE POWER

78/78 OPT=1

FTN 4.5+414

11/19/80 17.24.07

1	SUBROUTINE POWER(XK,XM,EV,XD,B,BB,EDUM,NS1,NDOF,MODE)	CAM	215
	COMMON/EVAL/EF(20)	CAM	216
	DIMENSION XK(NS1),XM(NS1),EV(NDOF,MODE),B(NDOF),BB(NDOF)	CAM	217
	1,XD(NDOF,NDOF),EDUM(MODE)	CAM	218
5	LEVEL 2,XK,XM,EV,XD,B,BB,EDUM	CAM	219
	C	CAM	220
	C INVERSE ITERATION TO COMPUTE 'MODE' LOWEST MODES....	CAM	221
	C EACH ITERATION STARTS WITH UNIT VECTOR FOLLOWED WITH	CAM	222
	C GRAM-SCHMIDT ORTHOGONALIZATION	CAM	223
10	C	CAM	224
	WRITE(B,100) MODE	CAM	225
	100 FORMAT(//5X,22H INVERSE ITERATION FOR ,15,BH MODES//	CAM	226
	1 2X,4HMODE,2X,9H ITERATION,2X,10H EIGENVALUE,2X,5H ERROR)	CAM	227
	TOL=.0001	CAM	228
15	IMAX=30	CAM	229
	C FACTOR K	CAM	230
	DO 1 I=1,NDOF	CAM	231
	DO 1 J=1,NDOF	CAM	232
	1 XD(I,J)=0.	CAM	233
20	DO 2 I=1,NDOF	CAM	234
	DO 2 K=1,NDOF	CAM	235
	CALL PPLOC(I,K,ILOC)	CAM	236
	KCOL=K-1+1	CAM	237
	2 XD(I,KCOL)=XK(ILOC)	CAM	238
25	CALL INVERT(NDOF,NDOF,XD,B,JOUN,2)	CAM	239
	DO 20 IM=1,MODE	CAM	240
	C TRIAL VECTOR	CAM	241
	DO 3 I=1,NDOF	CAM	242
	3 B(I)=1.	CAM	243
30	IF(IM.GT.1) CALL GRAMS(XM,EV,B,BB,IM,NS1,NDOF,MODE)	CAM	244
	ITER=1	CAM	245
	ERR=0.	CAM	246
	C M * X	CAM	247
35	12 DO 4 I=1,NDOF	CAM	248
	BB(I)=0.	CAM	249
	DO 4 J=1,NDOF	CAM	250
	CALL PPLOC(I,J,ILOC)	CAM	251
	4 BB(I)=BB(I)+XM(ILOC)*B(J)	CAM	252
	CALL INVERT(NDOF,NDOF,XD,BB,JOUN,3)	CAM	253
40	C X' * M * X	CAM	254
	DO 5 I=1,NDOF	CAM	255
	B(I)=0.	CAM	256
	DO 5 J=1,NDOF	CAM	257
	CALL PPLOC(I,J,ILOC)	CAM	258
45	5 B(I)=B(I)+XM(ILOC)*BB(J)	CAM	259
	DUM=0.	CAM	260
	DO 6 I=1,NDOF	CAM	261
	6 DUM=DUM+B(I)*BB(I)	CAM	262
50	C XLAM = X' * K * X / DUM	CAM	263
	DO 7 I=1,NDOF	CAM	264
	B(I)=0.	CAM	265
	DO 8 J=1,NDOF	CAM	266
	CALL PPLOC(I,J,ILOC)	CAM	267
	8 B(I)=B(I)+XK(ILOC)*BB(J)	CAM	268
55	7 CONTINUE	CAM	269
	DUMK=0.	CAM	270
	DO 9 I=1,NDOF	CAM	271

POOR ORIGINAL

SUBROUTINE POWER		78/78	OPT=1	FTN 4.5+414	11/19/80	17.24.07
	9	DUMK=DUMK+B(1)*BB(1)			CAM	272
		XLAM=DUMK/DUM			CAM	273
60		DUM=SQRT(DUM)			CAM	274
		DO 10 I=1,NDOF			CAM	275
	10	B(1)=BB(1)/DUM			CAM	276
		WRITE(6,101) IM,ITER, XLAM,ERR			CAM	277
	101	FORMAT(1X,15,3X,15,2X,E13.3,2X,E13.3)			CAM	278
65		IF(1M.GT.1) CALL GRAMS(XM,EV,B,BB,1M,N51,NDOF,MODE)			CAM	279
		ITER=ITER+1			CAM	280
		IF(ITER.GT.2) GO TO 11			CAM	281
		XLAML=XLAM			CAM	282
		GO TO 12			CAM	283
70	11	IF(ITER.GT.IMAX) GO TO 15			CAM	284
		ERR=ABS(XLAM-XLAML)/XLAM			CAM	285
		IF(ERR.LE.TOL) GO TO 13			CAM	286
		XLAML=XLAM			CAM	287
		GO TO 12			CAM	288
75	15	WRITE(6,104)			CAM	289
	104	FORMAT(5X,25HNO OF ITERATIONS EXCEEDED)			CAM	290
	13	DO 14 I=1,NDOF			CAM	291
	14	EV(1,1M)=B(1)			CAM	292
		EF(1M)=SQRT(XLAM)/6.28			CAM	293
80	20	CONTINUE			CAM	294
		WRITE(6,110)			CAM	295
	110	FORMAT(//5X,31HITERATION CONVERGED MODAL DATA/)			CAM	296
		DO 21 IM=1,MODE			CAM	297
		WRITE(6,111) IM,EF(1M)			CAM	298
85	111	FORMAT(5X,5HMODE ,15,12HEIGENVALUE =,E13.3,4H CPS/			CAM	299
		1 10X,12HMODE SHAPE =)			CAM	300
		WRITE(6,112)(EV(1,1M),I=1,NDOF)			CAM	301
	112	FORMAT(5X,6(E13.3,2X))			CAM	302
		EDUM(1M)=EF(1M)*6.28			CAM	303
90	21	CONTINUE			CAM	304
		REWIND 7			CAM	305
		WRITE(7) EDUM			CAM	306
		RETURN			CAM	307
		END			CAM	308

APPENDIX B

USE OF MODIFIED SAPV AT
BROOKHAVEN NATIONAL LABORATORY

The following card deck should be used to run the modified SAPV program on Brookhaven National Laboratories' CDC 7600 computer:

MSAP, STMFZ, T(t), P2.

t = running time

ACCOUNT (Name, Number)

Name, Number = Valid Account

ATTACH (OLDPL, SAP5, ID=ZZGAJPH)

ATTACH (CORR, GUYAN, ID=ZZGCAM)

ATTACH (LIB1, FR8OLIB)

ATTACH (LIB2, CALCOMPLIB)

LIBRARY (LIB1, LIB2)

UPDATE (F, M=CORR, L=A12)

FTN (I=COMPILE, L=0)

MAP(OFF)

SEGLOAD.

LD SET (PRESET = ZERO)

LGO.

EOR

* ID CAM

* D PLOTTT.1

* I CORR2.2

Card CAM.1 (All CAM - cards are listed in Appendix A).

* D LEVEL2.1

* I COMSIZE.3

Cards CAM.2 through CAM.4

* D LEVEL 2.3

* I LEVEL 2.2

Card CAM.5

* I SAP 5.88

Card CAM.6 through CAM.8

* I SAP 5.296

Card CAM.9 through CAM.76

* I SAP.5 385

Card CAM.77

* I SAP.5 387

Card CAM.78

EOR

Segment cards as for normal SAPV run

EOR

Data as specified in SAPV manual Ref. 5 and modified in Section 3.2

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NRC FORM 335 (7-77)		U.S. NUCLEAR REGULATORY COMMISSION BIBLIOGRAPHIC DATA SHEET		1. REPORT NUMBER (Assigned by DDC) NUREG/CR-1938 BNL-NUREG-51341	
4. TITLE AND SUBTITLE (Add Volume No., if appropriate) Reduction of Structural Degrees of Freedom				2. (Leave blank)	
7. AUTHOR(S) C. A. Miller				3. RECIPIENT'S ACCESSION NO.	
9. PERFORMING ORGANIZATION NAME AND MAILING ADDRESS (Include Zip Code) Brookhaven National Laboratory Upton, NY 11973				5. DATE REPORT COMPLETED MONTH: December YEAR: 1980	
12. SPONSORING ORGANIZATION NAME AND MAILING ADDRESS (Include Zip Code) Office of Nuclear Regulatory Research Division of Reactor Safety Research U.S. Nuclear Regulatory Commission Washington, DC 20555				DATE REPORT ISSUED MONTH: April YEAR: 1981	
				6. (Leave blank)	
				8. (Leave blank)	
				10. PROJECT/TASK/WORK UNIT NO.	
				11. CONTRACT NO. FIN A3216	
13. TYPE OF REPORT Technical Report			PERIOD COVERED (Inclusive dates) June 1980 - December 1980		
15. SUPPLEMENTARY NOTES				14. (Leave blank)	
16. ABSTRACT (200 words or less) An assessment is made of the errors in large structural systems when both Guyan and Modified Guyan Reduction are used. Second, the economies that may be achieved by using these reduction methods as compared with obtaining complete solutions (i.e., no reduction) are investigated.					
17. KEY WORDS AND DOCUMENT ANALYSIS			17a. DESCRIPTORS		
17b. IDENTIFIERS/OPEN-ENDED TERMS					
18. AVAILABILITY STATEMENT Unlimited			19. SECURITY CLASS (This report) Unclassified		21. NO. OF PAGES
			20. SECURITY CLASS (This page) Unclassified		22. PRICE \$