

UNITED STATES NUCLEAR REGULATORY COMMISSION

WASHINGTON, D. C. 20555

NOV 0 5 1980

50-369 Docket Nos.:

50-370 and

Duke Power Company

ATTN: Mr. William O. Parker, Jr.

Vice President - Steam Production

P. O. Box 33189

422 South Church Street

Charlotte, North Carolina 28242

Dear Mr. Parker:

SUBJECT: CATEGORY I MASONRY WALL DESIGN

(MCGUIRE NUCLEAR STATION, UNITS 1 AND 2)

We have reviewed your letter of September 23, 1980 regarding Category I Masonry wall design and find that we require some additional information which is described in the enclosure.

We request that this information be provided no later than November 14, 1980.

Sincerely,

R. L. Tedesco, Assistant Director

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TOT

for Licensing

Division of Licensing

Enclosure: As stated

cc: See service list

REQUEST FOR ADDITIONAL INFORMATION
ON
MC GUIRE MASONRY WALL DESIGN
STRUCTURAL ENGINEERING BRANCH
(REF: DPCO LETTER DATED SEPT. 23, 1980)
Docket Nos. 50-369, 370

- 8. In your responses to questions 3 and 4 it is stated that because of your use of assumptions such as rigid wall and simply supported end conditions together with a design factor of 1.875, no rigorous response spectra type of analysis is required and it is reasonable to conclude that the masonry wall design approach is reasonable, adequate and conservative. Your conclusion is not so obvious to the staff. From attachment 7, for a structure or structural element having a period of 0.08 second the response acceleration is found to be 0.5 (broadened) vs. 0.16 for zero period. The factor is 0.5/.16 = 3.125 vs. 1.875. In the reinforcing details provided (attachment #9) especially at corners, the reinforcing steel is placed either on one face or at the middle of the section. In view of these observations it is requested that in order to substantiate your conclusion a rigorous response spectra analysis be performed, taking into consideration such factors as interstory drift, effect of upper floor response, actual support condition, etc.
- 9. In your response 4(a) it is stated that for collar joints in multiple wythe walls, mortar was applied to adjoining faces of both wythes and pressed firmly to insure full bond between wythes, thus constituting a shear transform mechanism between wythes. From your computation check the resulting shear stresses due to the inertial loads are found to be about 11 psi. Since shear in collar joints is different from shear in other joints, indicate what the allowable value for such shear is and how it is established.
- In your response 6(a) in discussing the effects of the combined action of local and global loads, it is stated that local loads are considered as global in-plane loads only when they are of significant magnitude. Indicate your criterion for "significant magnitude".

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Mr. Tom Donat Resident Inspector McGuire NPS c/o USNRC Post Office Box 216 Cornelius, North Carolina 28031