067

QUESTION 2H.1 (DRL 2.7)

Discuss the water flow patterns in the vicinity of the plant and their associated consequences on plant operations following a failure of the on-site water storage facilities.

ANSWER

Section 2.4 of the PSAR describes the topography and drainage patterns of the site. As stated, the site is located very favorably for natural drainage.

Open ditches and culverts will be provided around the plant area to intercept any overland flow. The paved area within the plant slopes away from the buildings and toward the perimeter drainage system. Storage tanks are located between the buildings and perimeter drainage system. Any discharge from these tanks will flow away from the buildings. The circulating water system canal and intake have a water surface below nominal plant grade.

The result of a postulated failure of the on-site reservoir has been investigated. The flow from a reservoir failure would pass to the south of the plant. The invert of the natural drainage channel is approximately 20 feet below the plant grade with a top width of over 1500 feet and could safely pass a flow in excess of 175,000 cfs.

Due to the elevation of the plant above the adjacent Sacramento River Delta (see Fig. 2.2-3), there is no downstream control which would affect the stage-discharge relationship of water flowing past the plant. The water flowing past the plant is free to flow at normal depth. The reservoir is far enough upstream from the plant for the bore wave to collapse prior to passing the plant. It is difficult to postulate a sudden failure of an earth structure and due to the relative small quantity of water stored in the reservoir any time lag to reach the maximum break will have a substantial influence on the peak flow.

An instantaneous break 50' wide and the full height of the dam, occurring simultaneously with the peak discharge of the maximum probable flood, resulting from storm runoff at the plant and adjacent catchment area, will produce a flow of less than 50,000 cfs past the plant. Peak discharge of the maximum probable flood, computed by methods developed by the Soil Conservation Service, is less than 2,000 cfs.

Peak runoff from the maximum probable storm was based on the probable maximum 6-hour point storm value of 6 inches from storm analyses data prepared by the Bureau of Reclamation.

It is concluded that a failure of a dam occurring simultaneously with the peak discharge of the maximum probable storm will not flood the site. The resulting 50,000 cfs flow will have its water surface more than 10 feet below any of the safeguards equipment located in the plant yard. This would be the controlling elevation in the event of flooding.

QUESTION 2H.2 (DRL 2.8) Provide a map of earthquake epicenters within a radius of 200 miles showing all earthquakes of intensity V or greater at the epicenter.

ANSWER

The instrumented epicenters and non-instrumented seismic events within a 50 mile radius of the Rancho Seco site are shown on Figures 2H.2-1 and 2 respectively.

The epicenter map shows the magnitude of earthquakes actually recorded, while the non-instrumented seismic event map shows the highest intensity reported for any earthquake. It is considered that the 50 mile radius map presents a meaningful description of the site and adjacent area since varying geological conditions need not be evaluated. Similar maps for a 200 mile radius are shown in Figures 2H.2-3 and 4.

Appendix A to this answer is Professor Byerly's explanation of epicenter maps (Figures 2H.2-1 and 2).

QUESTION 2H.3 (DRL 5.8) Our current review indicates that the design basis earthquake (the maximum earthquake) for the Rancho Seco site should correspond to a maximum horizontal ground acceleration of at least 0.25g. The basis for this conclusion stems from the possibility that historical evidence may underestimate the maximum earthquake likely to occur in this province and from a need to provide both for the possible occurrence of local faults that may be unknown in this region because of the great depth of overburden and for the possible amplification in the alluvium in this geological region. You are asked to consider this basis for the design basis earthquake for the Rancho Seco site.

ANSWER

A value of horizontal ground acceleration of 0.25g will be used for the maximum hypothetical earthquake and 0.13 g will be used for the design earthquake.

QUESTION 2H.4 (DRL 5.9) Provide the elevations of the proposed foundations for the containment structure, turbine buildings, and auxiliary buildings, in order that they can be compared with the boring data provided in the PSAR.

ANSWER

The following structure foundation elevations can be used for correlation with the boring logs:

Structure		Elevation
Containment Structure		130'-0
Spent Fuel Storage Pool		158'-6
Auxiliary Building		145'-0
Radwaste Building	4	140'-0
Turbine Building - Mat	÷	155'-6
Turbine Building - Condenser Area	-	150'-0

Elevation 0'-0" on general arrangement drawings equals elevation 165 MSL.

Docket 50-312 Amendment No. 2 April 15, 1968 Appendix A to Question 2H.2

March 25, 1968

Mr. John Mattimoe Sacramento Municipal Utility District Box 2391 Sacramento, California 95811

Re: Epicenter Mans

Dear Mr. Mattimoe:

You have asked for epicenter maps and Bechtel has constructed them according to my specifications.

There must be two of them because two different kinds of epicenters are involved in the historical record.

1) Since and where there have been established seismographic stations, instrumental opicenters may be located. Insofar as we know the earth's structure and the velocities of seismic waves we may from seismograms locate the place where the waves start, i.e. the first source of the earthquake or its focus. The point on the earth's surface above the focus is the epicenter.

If we hold to the theory that all earthquakes originate by fault breaks the instrumental focus should lie on a fault surface and the epicenter near the fault trace on the earth's surface. In practice we find in California that the instrumental epicenters of the larger shocks do lie near the traces of recognized faults although very many of the smaller shocks are not so located.

2) In the early days, say over 35 years ago in California, and in some parts of the United States even today, there are not enough seismographic stations to locate the centers of smaller shocks. In no part of the world were seismographs used to locate enicenters before about 1885, and the instrumental location of the epicenters of large distant earthquakes awaited the turn of the century.

Lacking seismographs, the term epicenter, now called field concenter, was taken as the center of greatest damage. This location depends on the presence of structures that can be damaged, both natural (steep slopes for landslides, loose natural filled land and swamps to be cracked) and built by man. It depends on the geologic foundations as well as the distance to the source of

posed faults on maps through epicenters, we must supply one map showing the instrumental epicenters, on which these men may work, and another for the "center of greatest damage epicenters" which are not supposed to lie on faults by any theory.

On the first map we plot circles for instrumental epicenters, and by each the year of the shock and its magnitude in Arabic numerals.

On the second map we designate the center of greatest shaking by a Roman numeral, indicating the Modified Mercalli Intensity there and also the year of the shock.

The construction of the second map requires somewhat more than consulting the Earthquake History of the United States (U.S.C.& G.S.) because in the early days Captain Heck and others having only intensity data were inclined to "round out" the latitudes and longitudes of their field epicenters. Thus too many lay at intersections of the even degree parallels and meridians—an example from our map is the earthquake of April 10, 1881, in which the catalog gives Modesto as having the greatest intensity although the History puts it half a degree north. (The reports suggest strongly that this shock centered in the Coast Ranges.)

Yours sincerely,

Perry Byerly

cc Mr. Cole R. McClure, Jr.























