



**Florida  
Power**  
CORPORATION

February 1, 1974

Donald F. Knuth, Director  
The Directorate of Regulatory Operations  
U. S. Atomic Energy Commission  
Washington, D. C. 20545

Subject: Crystal River Plant Unit #3  
Docket 50-302

Dear Mr. Knuth:

In order to provide you with the necessary data to evaluate Florida Power Corporation's actions regarding the reported non-conformance discussed in our letter to you of December 27, 1973 we are enclosing copies of the following data:

1. GAI letter #FPC-9738, dated January 23, 1974.
2. GAI letter #FPC-9093, dated October 8, 1973.
3. FPC letter, dated November 5, 1973.
4. Crystal River Unit #3 Construction Procedure #JAJ-W52, Revision 2.
5. Signed Quality Control Checklist (Form #JAJ-132).
6. Field Changes 1 - 8 to Crystal River #3 Construction Procedure JAJ-W52.

These documents are the documentation referenced in our December 27th letter. The GAI letter #FPC-9738, dated January 23, 1974, is a formal transmittal of documentation that was originally at the site in the form of a telecopier communication to Florida Power Corporation. This is the reason for this letter being dated subsequent to the issuance of our final report on December 27th.

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Donald F. Knuth  
February 1, 1974  
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We hope that this additional information will provide you with the necessary data to close this item of non-conformance. Please advise us if you require any further information.

Very truly yours,

*J. T. Rodgers*  
J. T. Rodgers  
Assistant Vice President

ldh

Enc.

cc: Norman C. Moseley  
Director, Region II

This copy to:

GILBERT ASSOCIATES, INC.

G

January 23, 1974

FPC - #9738

Mr. W. A. Szelistowski  
Director, Generation Engineering  
Florida Power Corporation  
P. O. Box 14042  
St. Petersburg, Florida 33733

Re: Nuclear Services and Decay Heat  
Sea Water System Analysis

Dear Mr. Szelistowski:

In reference to the above subject as it relates to the 48" underground sea water discharge lines, we have analyzed the in-situ condition to determine whether or not rectification would be required. Based on the information received from the field, GAI concludes that the pipeline can remain installed in the as-built condition. It is, however, assumed that FPC Construction can in fact document the details on the deviation between the original design and the as-built condition.

We feel that this brings to a close this subject unless we hear further from you.

Very truly yours,

  
E. R. Hottenstein

Project Manager

ERH:ejv

EVALUATION REPORT1-23-74  
Dr. C. ChenNUCLEAR SERVICES AND DECAY HEATSEA WATER SYSTEM

Reference Drawings:

P-304-611

C-300-747

C-300-748

48" - SEA WATER DISCHARGE LINE SEISMIC DISTURBANCE ANALYSIS

The purpose of this investigation is to find out whether the 3" allowable movement is enough to warrant no separation at pipe joints due to seismic disturbance. This analysis covers only the soil movement caused by the wave action of ground shaking. The other ground motions caused by surface faulting, land sliding and ground fissuring are avoided during the site selection. The consolidation of backfill soil is avoided by properly specifying the backfill material.

The basic tools used in the analysis are the formulae derived by Newmark (Ref. 1).

$$\text{Axial strain } \epsilon_m = V_m/C \quad \dots \quad (1)$$

$$\text{Curvatural } 1/R = A_m/C^2 \quad \dots \quad (2)$$

where  $V_m$ ,  $A_m$ , and  $C$  are the particle velocity, acceleration and the wave velocity respectively.

The following site informations are furnished by Mr. W. J. Santamour of Civil Engineering Department after the reviewing of the Unit No. 2 Site Seismic Survey done by the Weston Geophysical Research Corporation.

Below Elevation 84: Dilatational wave velocity  $C_D = 6900 \text{ ft/sec}$

Shear wave velocity  $C_S = 2700 \text{ ft/sec}$

Poisson's ratio  $\nu = 0.38$

Above Elevation 84: Dilatation wave velocity  $C_D = 1600 \text{ ft/sec}$

The pipes are installed between elevation 94 and 96.

In order to perform the analysis, the following assumptions are made:

1. The soil particle velocity in the direction of wave propagation is caused by the dilatational wave and the longitudinal component of Rayleigh wave in a complex manner. The percentage of contribution from each component is difficult to estimate. For teleseismic waves, interpretations of different wave forms can be achieved because the great distance and long travel time makes different frequency wave component dispersed and cause natural separation of various wave types. However, the strong motion record, close to the source of energy release, is less obvious to interpret. Trifunac (Ref. 2) showed qualitatively that surface (Love and Rayleigh) may contribute significantly to strong, near field, ground motion.

For an elastic half space, the Rayleigh wave velocity approaches shear wave velocity. For layered soil and rock the Rayleigh wave is dispersive; hence, its velocity is the function of periods and modes. For engineering purposes the wave velocity is Equation (1) will be taken as the average of the dilatational wave and the shear wave velocity. This is more conservative than just taking the dilatational wave velocity.

2. The soil particle acceleration perpendicular to the direction of wave propagation is caused by the shear wave and the Love wave in a complex manner. Since the Love wave velocity is sandwiched between the shear wave velocities of the upper and lower layer (Ref. 3), it is conservative to use upper layer shear wave velocity in Equation (2).
3. The site SSE value is 0.1G. For conservative reasons, this G value is assumed at the base rock. Assuming that the ground motion is effected by the local geology only and that SH wave propagates upwards vertically, the soil will amplify base rock G value (Ref. 4, 5). However, other works (Ref. 7, 8, 9) indicated that the effects of source mechanism and travel path may overshadow the local geological effects. Nevertheless, the soil amplification will be assumed for conservative reason. From Tables 1 and 2 of Reference 6, the maximum amplification factor among various site condition is less than 3. Applying the ultraconservative factor of 3, the SSE value for soil will be 0.3G.
4. Wave velocity obtained by in SITU wave propagation test is for soil at very low strain level. The velocity will be reduced at higher strain. As shown in Equations (1) and (2), it is more conservative to use lower velocity. For conservative reason, the velocity will be reduced by a factor of 2.
5. Above elevation 84, the dilatational wave velocity is  $C_D = 1600$  ft/sec. Assuming the Poisson's ratio  $\nu = 0.35$ , the shear wave velocity is approximately  $C_S = 1600/2(1 + 0.35) = 600$  ft/sec. The average of  $C_D$  and  $C_S$  is  $C = 1100$  ft/sec, and  $C = 550$  ft/sec at higher strain.
6. Based on the statistic data (Ref. 10), 1G acceleration corresponds to 48 in./sec velocity on alluvium.
7. The field test showed that the underground pipe will move with the surrounding soil with no appreciable relative displacement. Under this situation the pipes will not separate at the joints. The only way that the pipes may separate is under the unlikely assumption that one piece of the pipe moves with the ground and the adjacent one slips totally from the surrounding soil.

Based on the foregoing assumptions, the axial strain is

$$\epsilon_m = 0.3 \times 48/(550 \times 12) = 0.00218$$

and curvature is

$$1/R = 0.3 \times 32.2/(550)^2 = 0.0000318/\text{ft}$$

By Taylor's series expansion

$$f(x) = f(b) + f'(b)(x - b) + f''(b)(x - b)^2/2 + \dots$$

where  $x - b$  is the pipe section length 18 ft,  $f'(b)$  is the axial strain,

$f''(b)$  is the curvature. The relative displacement is  $f(x) - f(b) = f'(b)(x - b) + f''(b)(x - b)^2/2 + \dots$ . Since the total velocity and acceleration were used to calculate  $\varepsilon_m$  and  $1/R$ , it is justified to combine them by root sum square approach. However, the absolute sum will be used for conservative reason. Hence, the displacement is

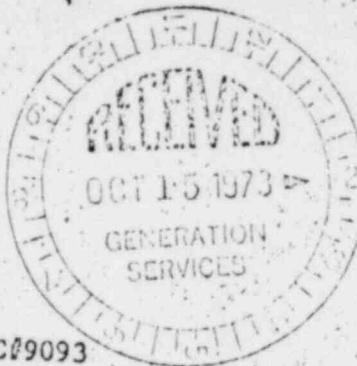
$$\begin{aligned} D &= 0.00218 \times 18 \times 12 + 0.0000318 \times 12 \times (18)^2/2 \\ &= 0.47 \text{ in.} + 0.062 \text{ in.} = 0.53 \text{ in.} \ll 3 \text{ in. allowable} \end{aligned}$$

The conclusion of this investigation is that the pipes will not separate at joints during earthquake disturbance.

#### REFERENCES

1. Newmark, N. M., "Problems in Wave Propagation in Soil and Rock". Proceeding of International Symposium on Wave Propagation and Dynamic Properties of Earth Material, The University of New Mexico, August 1967.
2. Trifunac, M. D., "Response Envelope Spectrum and Interpretation of Strong Earthquake Ground Motion," BSSA\*, Vol. 61, No. 2, P. 343-356, April 1971.
3. Fung, Y. C., "Foundations of Solid Mechanics," Prentice-Hall, 1969.
4. Roesset, J. M., "Fundamentals of Soil Amplification," in "Seismic Design for Nuclear Power Plants," Ed. R. J. Hansen, MIT Press, 1970.
5. Schnabel, P., H. B. Seed, and J. Lysmer, "Modification of Seismograph Records for Effects of Local Soil Conditions," BSSA, Vol. 62, No. 6, P. 1649-1664, December 1972.
6. Lysmer, J., H. B. Seed, and Schnabel, "Influence of Base-Rock Characteristics on Ground Response," BSSA, Vol. 61, No. 5, P. 1231-1231, October 1971.
7. Udwadia, F. E. and M. D. Trifunac, "Comparison of Earthquake and Microtremor Ground Motions in El Centro, California," BSSA, Vol. 63, No. 4, P. 1227-1253, August 1973.
8. Hudson, D. E., "Local Distribution of Strong Earthquake Ground Motions," BSSA, Vol. 62, No. 6, P. 1765-1786, December 1972.
9. Hall, W. J., N. M. Newmark, and D. B. Mohraz, "Comments on Earthquake Transmission from Basement Rock to Surface," Paper No. 83, 5th WCEE\*, Rome 1973.
10. "A Study of Vertical and Horizontal Earthquake Spectra," by N. M. Newmark Consulting Engineering Service for USAEC, Wah-1225, April 1973.
11. Nasu, N., S. Kazama, T. Morioka, and T. Tamura, "Vibration Test of the Underground Pipe with a Comparatively Cross-Section," Paper No. 64, 5th WCEE, Rome 1973.

\* BSSA: Bulletin of the Seismological Society of America.  
WCEE: World Conference on Earthquake Engineering.



October 8, 1973

Mr. W. A. Szelistowski  
Director Generation Engineering  
Florida Power Corporation  
P.O. Box 14042  
St. Petersburg, Florida 33733

Re: Crystal River Unit Three 48" Nuclear  
Service and Decay Heat Service Seawater  
System, Underground Concrete Discharge  
Piping

Ref: FPC-GAI-150, Telex from FPC dated  
October 5, 1973

Dear Mr. Szelistowski:

In reference to the above communications, CAI realizes the seriousness of your problem and will review the situation with a view to making every effort possible to determine a solution that will have the least total input on the Project.

We propose the following: An engineering analysis be made to determine, if in fact, the pipe line as constructed can meet design criteria. Our basic concern is the ability of the pipe to withstand a seismic disturbance. The analysis will not commence before October 16, 1973. By November 1, 1973 an initial reply is expected provided that in-put information is available.

Concurrently with the above effort, we further propose to investigate alternative "fixes" should the analysis indicate such a course of action is required.

This letter has been telecopied to the site.

Very truly yours,

F. J. Tomazic  
Project Control Analyst

FJT:dc

**FLORIDA POWER**  
P. O. BOX 226  
**CORPORATION**  
CRYSTAL RIVER, FLORIDA 32629

FPC  
69  
4-17-3

November 5, 1973

Gilbert Associates, Inc.  
P. O. Box 1498  
Reading, Pa. 19603

Attention: Mr. E. R. Hottenstein

Subject: Crystal River Unit #3  
48" NSSW Concrete Pipe

*Kahler Service Department  
48" pipe*

In response to your 11-1-73 letter (FPC-9267) we transmit one copy of the procedure (JAJ-W52) used for installation of the 48" pipe. We specifically refer you to section 6.4.D of the procedure. We also transmit one copy of the completed, signed off Quality Control Check Lists (JAJ-132) for QC inspection of the pipe installation.

Should additional information be needed, please contact us.

*J. E. Colby*  
J. E. Colby, Engineer

R. S. Burns, Manager Mechanical  
& Structural Engineering

JFC/RSB/kah

cc: C. E. Jackson

# J. A. JONES CONSTRUCTION CO.

## COVER SHEET

DOCUMENT NUMBER: JAJ-W52	ORIGINAL DATED: 7/14/72	ATTACHMENTS: None
<p><b>TITLE:</b>          WORK PROCEDURE-INSTALLATION OF NUCLEAR SERVICE          AND DECAY HEAT SEA WATER UNDERGROUND PIPING          (Florida Power Corporation-Crystal River #3)</p> <p><b>SCOPE OF REVISION:</b></p> <p>Revision 1, dated 12/1/72</p> <p>1. General Revision.          Revision 2, dated 12/21/72</p> <p>1. Page 6: Paragraph 5.20 added. Section 6          changed. Paragraphs 7.0, 8.0, 8.1          and 8.2 deleted.</p> <p>2. Exhibits 4,5,6 and 7 added.</p>		

**POOR ORIGINAL**

### APPROVAL DATES, SIGNATURES AND INITIALS

	ORIGINAL ISSUE	DATE	REV.NO.	INITIAL & DATE	REV.NO.	INITIAL & DATE
AUTHOR	<i>R. Jones</i>	7/14/72	1	<i>R. Jones 7/14/72</i>	2	<i>Rao 12/19/72</i>
JONES QC	<i>J. Jones</i>	7/14/72	1	<i>J. Jones 7/14/72</i>	2	<i>J. Jones 12/19/72</i>
JONES PROJECT	<i>J. Jones</i>	7/14/72	1	<i>J. Jones 7/14/72</i>	2	<i>J. Jones 12/19/72</i>
FPC	<i>W. Fletcher</i>	7/14/72	1	<i>W. Fletcher 7/14/72</i>	2	<i>W. Fletcher 12/22/72</i>

# J. A. JONES CONSTRUCTION CO.

POST OFFICE BOX 966  
CHARLOTTE, NORTH CAROLINA 28201

## WORK PROCEDURE JAJ-W52

### INSTALLATION OF NUCLEAR SERVICE AND DECAY HEAT SEA WATER UNDERGROUND PIPING

#### 1.0 PURPOSE

1.1 To provide instructions for the installation of Nuclear Service and Decay Heat Sea Water Underground Piping.

#### 2.0 SCOPE

2.1 This procedure includes the Nuclear Sea Water intake and discharge piping.

#### 3.0 RESPONSIBILITY

3.1 The J. A. Jones Construction Co. Mechanical General Superintendent has the primary responsibility for implementation of this procedure.

#### 4.0 REFERENCES

4.1 FPC-W8 Procedure for Warehouse Functions.

4.2 GAI Dwg. P-304-611, Nuclear Service and Decay Heat Sea Water Piping Plan and Sections

4.3 GAI Dwg. FD-302-611, System Flow Diagram

4.4 GAI Dwg. G-736-002, Plot-Main Plant Area

4.5 GAI Dwg. G-744-010, Intake and Discharge Canal-Excavation

4.6 GAI Dwg. SC-426-501, Instake Structure

4.7 GAI Dwg. SC-422-001, Nuclear Service Sea Water Pump Chamber

4.8 GAI Dwg. SC-422-015, Aux. Bldg.

4.9 GAI Dwg. P-304-613, Nuclear Service and Decay Heat Sea Water-Plan-Aux. Bldg.

4.10 RO-2766 Nuclear Services Seawater Piping

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- 4.11 Nuclear Service and Decay Heat Suspended Seawater Piping, Gilbert Associates, Inc. RO-3018
- 4.12 FPC-W48, Work Procedure-Pipe Erection.
- 4.13 ES-111A American Cast Iron Pipe Company drawing.
- 4.14 Interpace Pipe Laying Schedule, Revision F, dated, 6/21/71
- 4.15 Interpace Instructions for Installation of Concrete Pressure Pipe with Rubber and Steel Joints.
- 4.16 GAI Dwg. SC-401-041, Rev. 1, Filter Blanket Decay Heat Pit.
- 4.17 P. C. Shah letter FPC-6924, dated 10/13/72.
- 4.18 Specification for Excavation and Placement of Structural Fill, Gilbert Associates, Inc., SP-5629.
- 4.19 TP-7-3-265-5, TP-7-3-266-4, Nuclear Services and Decay Heat Seawater Systems, Buried Ductile Intake Piping Hydrostatic Test.
- 4.20 TG-00046, Hydrostatic and Pneumatic Components.

5.0 INSTRUCTIONS-LAYING 48" CAST IRON INTAKE PIPE (DRY CONDITION)

- 5.1 Dewatering shall be effected using a filtered dewatering system. Water will collected from the trench through lines of 6", 8" or 10" perforated, galvanized pipe. Pipe size selection will depend on volume of water to be removed. One pipe will be run along each bottom corner of the trench as shown in Exhibit 1. If necessary, a third dewatering pipe will be laid down the middle of the trench as shown in Exhibit 1. Cross connections between the parallel dewatering lines will be laid as necessary to achieve efficient dewatering.
- 5.2 Excavation, placing of pipe, and fill shall be in accordance with Exhibits 1 and 2.
- 5.3 The trench bottom shall be excavated as close to grade as possible (at least 8" below the bottom of the pipe).
- 5.4 In the area of the pipe joints, over-excavation will be allowed to permit assembly and testing. Over-excavate for the dewatering pipe as shown in Exhibit 1.

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- 5.5 Hold for Quality Control Check.
- A. Prior to any backfilling, verify that all mud, sludge, sediment and poor fill material has been removed and that a general state of cleanliness exists.
  - B. Check Zone "A" material for conformance to Reference 4.16 prior to use in backfilling. On-site testing laboratory perform at least one gradation test for each 150 tons of fill.
- 5.6 General over-excavation shall be leveled with Zone "A" fill compacted to 98% maximum density. Maximum density shall be determined from Zone "A" test strip as described in PTL Report of September 1972.
- 5.7 A uniform layer of Zone "A" material shall be compacted per section 5.6 up to the elevation of the bottom of the pipe. The level of this Zone "A" material shall be carefully graded, by hand wherever necessary, to meet the design grade line.
- 5.8 Approximately 5 inches of Zone "A" material shall be compacted to 98% maximum density (see section 5.6) and subsequently cut to leave two trapezoidal trenches four feet wide in which the pipe will rest (see Exhibit 1).
- 5.9 Hold for Quality Control check. One compaction test shall be made for each 200 yards of fill.
- 5.10 Handling Pipe
- A. Slings shall be rigged on the external surface of the pipe.
  - B. No temporary welds shall be made to the pipe to facilitate handling.
  - C. Each section of pipe shall rest on the shaped trench bottom for about one-half, or more, of its full length (except at the joints where over-excavation has been provided for assembly and testing) and shall be firmly held in position so that the invert forms a continuous grade with the invert of the section of pipe previously placed.
  - D. The angular deflection between centerlines of any two adjoining pipe sections shall not exceed one degree (equivalent to about 3" lateral deflection over 16' length).
  - E. All hub interiors and spigot ends shall be cleaned of loose dirt and sand.

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5.11 Hold for Quality Control check of cleanliness per 5.10 E.  
Record on Form No. JAJ-131 (Exhibit 3).

5.12 Joining pipe sections

A. Cast Iron Mechanical (Locked Fastite) Joints

1. Clean the joint mating surfaces.
  2. Insert the gasket into the gasket recess located in the socket end.
  3. Melt the American Locked Fastite joint lubricant by heating and apply a thin coat of this heated lubricant to the plain end of the pipe with a brush. Caution is necessary to avoid overheating the lubricant. The melting point of the lubricant is 150 degrees F to 160 degrees F. The flash point is 525 degrees F. Do not use solvents or other substances, which might be injurious to the rubber gasket, for cleaning the joint or for thinning the lubricant.
  4. With the joint reasonably straight, pull the plain end into the socket.
  5. The mechanical joint shall be pulled up and torqued to 65 ft. pounds or to a  $\frac{1}{4}$ " gap in the joint at the closest point, whichever occurs first. Place two  $\frac{1}{2}$ " spacers, separated about 180 degrees in the joint to assure  $\frac{1}{4}$ " gap is maintained. The torque wrench shall be calibrated at least once every six months.
- B. The four flanged cast iron pipe joints on the 90 degree ell sections shall be bolted in accordance with FPC-W48 and Ref. 4.2.

5.13 Hold for Quality Control check. Record on Form No. JAJ-131.

- A. Verify thermal gap is  $\frac{1}{4}$ " minimum.
- B. Verify alignment as performed by field engineers.
- C. Verify 65 foot pound maximum torque has been applied; check 10% of the mechanical joint bolts.
- D. Verify absence of loose material from the pipe.

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- E. Make inspection of the pipe liner for visually apparent cracks or loose lining. Cracks or loose lining shall be repaired per Manufacturer's instruction.
- 5.14 The joints shall be leak tested in accordance with Reference 4.19 and Reference 4.20 and section 3.06 of Reference 4.10.
- 5.15 Quality Control verify that leak test results are within requirements established by Reference 4.19. Record results of leak test on Form No. JAJ-131. (Exhibit 3, attached).
- 5.16 After successful leak test, each joint shall be wrapped in accordance with detail 5 of Reference 4.2. If practicable, leave water in pipe to add inertia during the backfilling operation called out in paragraph 5.18.
- 5.17 Hold for Quality Control check.
- A. Verify that joints have been wrapped in accordance with detail 5 of Reference 4.2.
- B. Verify that backfill materials called out in paragraph 5.18 meet requirements established in References 4.16 and 4.18.
- 5.18 Complete Backfill (see Exhibits 1 and 2)
- A. A coarse, washed sand shall be sluiced in the trench beneath the pipe. In addition, sand shall be sluiced into the over-excavation for the joints. It is necessary that excess water used in sluicing be drained off. During subsequent backfilling operations sand will be used exclusively around joints to a point 2 feet above the top of the pipe.
- B. Zone "A" fill shall be brought up to the pipe's spring-line. Thin (less than 12" loose) lift construction shall be used and after compaction the elevation of the top of each lift shall be uniform across the trench width in order to preclude movement of the pipes. The Zone "A" fill material shall be compacted to 90% maximum density ( see section 4.6).
- C. Backfilling shall be continued using thin lift construction: Zone III fill material placed in horizontal layers of approximately twelve inches thickness. The fill shall be spread and graded to prevent excessive particle segregation. Compaction shall be 90% Modified. This method shall continue from springline to the elevation of the top of the pipe.

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- D. The next lift above the elevation of the top of the pipe shall be at least two feet thick and shall be placed all at one time using Zone III material compacted to 90% Modified.
- E. Subsequent lifts shall utilize the thin (less than 12" loose fill) lift construction method using Zone III fill compacted to 90% Modified up to a height of 5 feet above the top of the pipe. This 5-foot "cushion" will be adequate to protect the pipe during standard compaction methods to be used for the remainder of the trench.
- F. From 5 feet above the top of the pipe up to the surface, backfill with Zone III and compact to 95% Modified.
- 5.19 Quality Control checks shall be made and documented during the backfill operations. Reports shall be included in Quality Control files.
- A. Verify gradation and plasticity index for Zone III material.
  - B. Verify gradation for Zone "A" material.
  - C. Verify that method of spreading and maximum amount of lift for Zone III materials meet requirements.
  - D. Make at least one compaction test per lift.
- 5.20 The On-site Independent Testing Laboratory shall make the inspections and tests called for by section 5 and submit results on the appropriate form for retention in the Quality Record files:
- A. Form No. JAJ-131 (see Exhibit 3) for individual pipe sections,
  - B. Form No. FPC Q-009 for discrepancies, and
  - C. Special Pittsburgh Testing Laboratory forms (see Exhibits 5, 6 and 7) for reports of compaction, plasticity and gradation.
- 6.0 INSTRUCTIONS - LAVING 48" CONCRETE (DISCHARGE) PIPE SECTIONS
- 6.1 The requirements of sections 5.1 through 5.9 shall be met for concrete pipe. Before each section of pipe is laid, Quality Control shall check the bedding requirements and report completion of that inspection on Form No. JAJ-132 (see Exhibit 4) under "Bedding".

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6.2 Handling Pipe sections

- A. Reference 4.15 provides guidance for laying and joining concrete pipe.
- B. Slings shall be rigged externally; no temporary welds.
- C. The pipe sections are serialized. Each section shall be laid in its assigned location as designated in Reference 4.2.
- D. Both ends to be joined shall be cleaned with rags or other suitable means and clear water before being joined.
- E. Before being joined, both ends shall be lubricated with vegetable soap (about 7 ounces per joint).
- F. The pipe section being laid shall be held in suspension after alignment until the joint has been thrust fully home.
- G. The gasket shall be thoroughly lubricated by immersion in a viscous solution of vegetable soap, and stretched around the spigot to obtain an even tension in the circumferential groove. The gasket shall be placed so as to lie true in the groove without twists.

6.3 Hold for Quality Control check. Report completion of inspection of joint cleanliness, joint lubrication and gasket lubrication and placement under "Joint Preparation" on Form No. JAJ-132.

6.4 Joint Closure

- A. As the joint is being closed, the closure shall be checked by two spacers (3/4" steel bar inserts held about 180° apart).
- B. A feeler gauge shall be used to detect any irregularities in the gasket by checking through the recess provided by the spacers.
- C. If the gasket is out of place, the pipe section shall be backed out, the joint re-lubricated, an undamaged and lubricated gasket installed, and the sections re-closed to the spacers.
- D. Given gasket alignment, the spacers shall be removed and the joint thrust fully home.

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- 6.5 Hold for Quality Control check. Quality Control report verification of gasket placement, joint closure and alignment (as performed by field engineers) under "Joint Fitup Alignment" on Form No. JAJ-132.
- 6.6 The pipe joints shall be mortared from the outside at a convenient time. Follow instructions from the mortar manufacturer when applying mortar.
- Outside joints shall be wrapped before mortaring with a special cloth strip which has metal reinforcing on each side. This strip shall be drawn tight around the pipe to provide a form to hold the mortar in the joint. Leave an opening across the top of the joint.
  - Mortar of a consistency which will flow into the joint shall be used. A wire curved to about a 24" radius shall be used to force the mortar into the lower portions of the joint recess.
  - Mortar of a thicker consistency shall be used to completely fill the joint recess across the top of the pipe.
- 6.7 Verify absence of loose material from the pipe. Make inspection of the pipe liner for visually apparent cracks or loose surface. Cracks or loose surface shall be repaired per Manufacturer's instructions. Report completion of foregoing inspections under "Pipe Condition" on Form No. JAJ-132. The pipe joints shall be mortared from the inside at a convenient time. These joints may be mortared using trowels. All excess mortar shall be removed. Follow instructions from the mortar manufacturer when applying mortar.
- 6.8 Quality Control check exterior mortaring for each joint and record completion of inspection in the "Exterior Joint Mortar" column of Form No. JAJ-132.
- 6.9 Quality Control check interior mortaring for each joint and record completion of inspection in the "Interior Joint Mortar" column of Form No. JAJ-132.
- 6.10 The requirements of sections 5.18 and 5.19 shall be met for concrete pipe subject to the following modifications:
- The use of sand to protect joints is optional.
  - If Zone "A" material is no longer available, Zone III material may be substituted in backfilling to the pipe's springline.

DOCUMENT CONTROL

1/14/72  
ORIGINAL DATED

2  
REVISION      12/19/72  
EFFECTIVE

J. A. JONES CONSTRUCTION CO.

JAJ-HS2  
DOCUMENT NO.

PAGE 7A OF 10

C. From two feet above the top of the pipe up to the surface, backfill with Zone III material and compact to 95% modified.

6.11 The On-site Independent Testing Laboratory shall make the inspections and tests called for by section 6 and submit results on the appropriate form for retention in the Quality Record files.

- A. Form No. JAJ-132 (see Exhibit 4) for individual pipe sections,
- B. Form No. FPC Q-009 for discrepancies, and
- C. Special Pittsburgh Testing Laboratory forms (see Exhibits 5, 6 and 7) for reports of compaction, plasticity and gradation.

DOCUMENT CONTROL

7/14/72  
ORIGINAL DATED

2            12/19/72  
REVISION      EFFECTIVE

J. A. JONES CONSTRUCTION CO.

JAJ-W52  
DOCUMENT NO.  
PAGE 7B OF 10

JAJ-W52  
12/1/72

INTAKE

(ZONE II (75% Mod))

ZONE III (90% Mod)  
USE THIN LIFT CONSTRUCTION

ZONE III (90% Mod)  
PLACE THIS ZONE IN  
ONE LIFT

ZONE III (90% Mod)  
USE THIN LIFT  
CONSTRUCTION

ZONE 'A' (75% Mod)

ZONE 'K' (75% Mod)

6", 8", OR 10" PERFORATED GRAVITY  
DEWATERING PIPES

INTAKE

CONTROL

POOR ORIGINAL

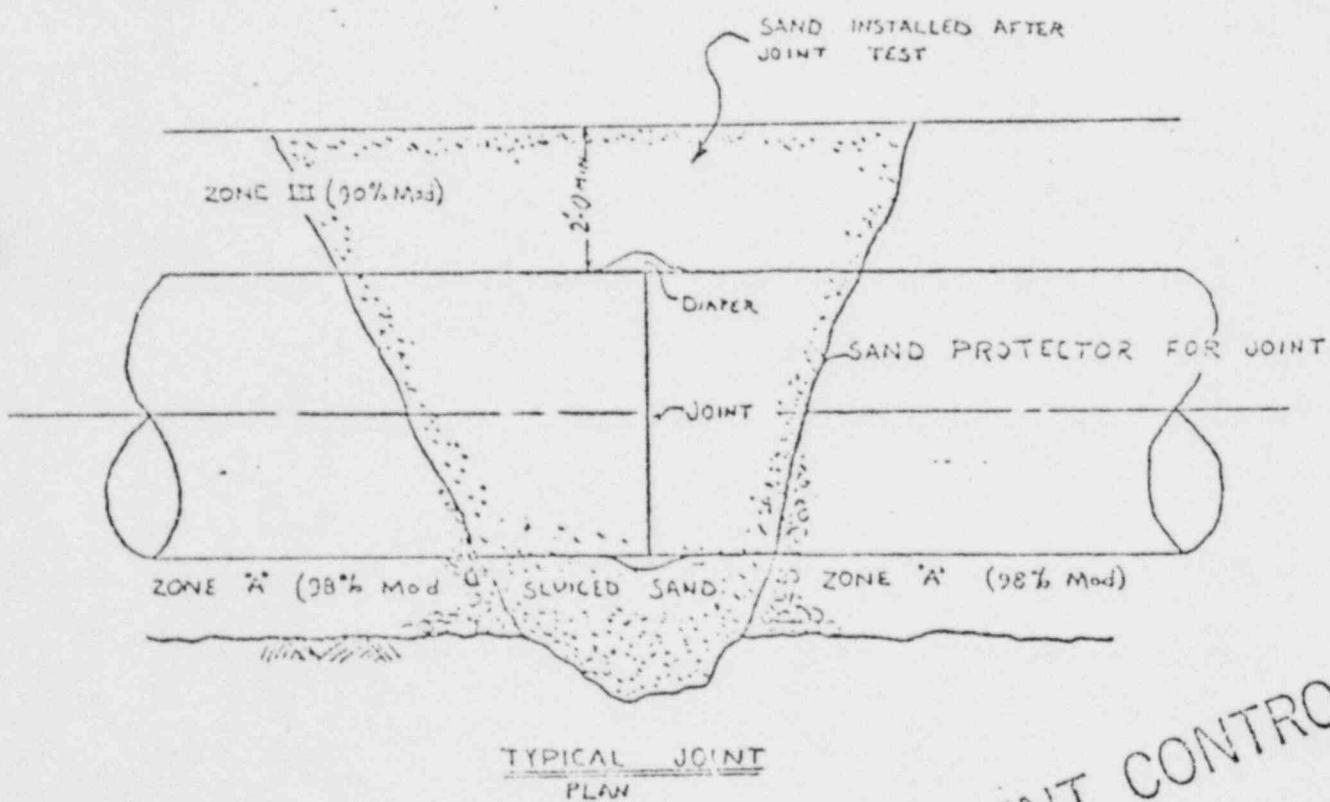
Exhibit 1

Page 8 of 10

BY DATE

TYPICAL JOINT  
TYPICAL JOINT

JOB NO.



DOCUMENT CONTROL

POOR ORIGINAL

J. A. JONES CONSTRUCTION CO.  
FLORIDA POWER CORPORATION CRYSTAL RIVER NO. 3

QUALITY CONTROL CHECK LIST FOR NUCLEAR SERVICE AND DECAY HEAT UNDERGROUND PIPING INSTALLATION

Distribution: White  
Canary  
Pink  
Golden

FPC Quality Engr.  
PTA Chief Inspector  
JAJ Quality Control  
FPC Const. Supt.

Form No. JAJ-131

**Exhibit 3**

Note: In each column, opposite each pipe piece no., the inspector shall sign in the upper of the two squares and record the inspection date just below the signature.

Page 16 of  
JAN-3/22, 1972

J. A. JONES CONSTRUCTION CO.  
FLORIDA POWER CORPORATION

CRYSTAL RIVER NO. 3

## QUALITY CONTROL CHECK LIST FOR NUCLEAR SERVICE AND DECAY HEAT UNDERGROUND PIPING INSTALLATION

### 48" CONCRETE PIPE

**Distribution:**

White	FPC Quality Engr.
Canary	PTL Chief Inspector
Pink	JAJ Quality Control
Goldenrod	FPC Const. Supt.

122

**Exhibit 4**

JAJ-W52, Rev. 2

NOTE: In each column, opposite each pipe size  
no., the inspector shall sign in the center of the  
squares and record the inspection date just below  
signature. Report discrepancies on  
Form No. FPC Q-009.

# POOR ORIGINAL

**J. A. JONES CONSTRUCTION CO.**  
**FLORIDA POWER CORPORATION**  
**CRYSTAL RIVER NO. 3**

CENTRALITY CONTRASTS IN

48" CONCRETE PIPE							5. 18 C. 6. 10 C.
PIRAGUAS		6. 1	6. 3	6. 5	6. 7	6. 8	5. 18 B. 6. 10 B.
PIPE SECTION	SEATING PREPARATION	JOINT FITUP ALIGNMENT	PIPE CONDITION	EXTERIOR JOINT MORTAR	INTERIOR JOINT MORTAR	ZONE A/M TO SPRINGLINE	ZONE M
4C7	✓ Joint seating	✓ J/A	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A	✓ J/A	✓ J/A
5S2	✓ Joint seating	✓ J/A	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A	✓ J/A	✓ J/A
5J	✓ Joint seating	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A	✓ J/A	✓ J/A
5S0	✓ Joint seating	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A	✓ J/A	✓ J/A
5E4	✓ Joint seating	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A	✓ J/A	✓ J/A
1C5	✓ Joint seating	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A	✓ J/A	✓ J/A
4J9	✓ Joint seating	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A	✓ J/A	✓ J/A
4J7	✓ Joint seating	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A ✓ Settlement	✓ J/A	✓ J/A	✓ J/A

# POOR ORIGINAL

**U. A. VONES CONSTRUCTION CO.** **MONDA POWER CORPORATION**

CRYSTAL RIVER

UNDERGROUND BURGERS

QUALITY CONTROL CHECK LIST FOR INCOMING AND OUTGOING PIPE

**NOTE:** In each column, opposite the upper of the two no., the inspector shall sign in the inspection date just below the lines and record the inspection date just below the lines.

# POOR ORIGINAL

ANNE JONES CONSTRUCTION CO.  
TALLAHASSEE, FLORIDA  
POWER CORPORATION  
TALLAHASSEE, FLORIDA  
RIVER NO. 3

QUALITY CONTROL CHECK LIST FOR NUCLEAR SERVICE	48" CONCRETE PIPE	5, 12 B.	5.
			6.

NOTE: In each column, opposite the half sign in the upper of the two

# POOR ORIGINAL

W. A. COOKS CONSTRUCTION CO.  
EQUINOX POWER CORPORATION  
EQUINOX RIVER NO. 3  
CRYSTAL RIVER  
WELL UNDERGROUND

COCA-COLA  
SODA

CORPORATION DOCUMENTS

CONSTANTINOPLE NO. 3

SOLAR ENERGY CONVERSION CHECK LIST FOR NUCLEAR SERVICE AND DECAY HEAT UNDERGROUND PIPING INSTALLATION

48" CONCRETE PIPE

**NOTE:** In each column, opposite each piece of  
gear, the inspector shall sign in the center of the two  
squares and record the inspection date just below the  
signature.

**Signature:** Report discrepancies on  
Form 100-100-000

Form No. FPC C-023

# POOR ORIGINAL

# POOR ORIGINAL

THE COTTON PRODUCTION CO.

CRYSTAL RIVER NO. 3

CITYITY CONTROL CHECK LIST FOR NUCLEAR SERVICE AND DECAY HEAT UNDERGROUND PIPING INSPECTION

45' CONCRETE PIPE

**NOTE:** In each column, opposite each piece no., the inspector shall sign in the upper half the squares and record his inspection date in the lower half.

PRACTICAL

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COSTAL

THE COUNCIL OF THE FEDERAL REPUBLIC OF GERMANY

490 CONCRETE PIPE

**NOTE:** In each column, opposite each piece of  
gear, the inspector shall sign in the upper left two  
squares and record the inspection date just below the  
**signature.** Report discrepancies on  
Form No. F.P.C. G-609.

# POOR ORIGINAL

COCONUT OIL

DAIRY CORPORATION

C 2-5 T L R-1 V E S N O : 3

46 CONCRETE PIPE

## **QUALITY CONTROL CHECK LIST FOR NUCLEAR SERVICE AND DECAY HEAT UNDERGROUND PIPING INSTALLATION**

PARASOL PIPES	PIPE SIZE, IN.	SEEDING	JOINT SEPARATION	JOINT FITUP ALIGNMENT	PIPE CONDITION	EXTERIOR JOINT MORTAR		INTERIOR JOINT MORTAR		ZONE A'11 TO SPRINGLINE		ZONE III
						6.3	6.5	6.7	6.8	6.9	5.18 B. 6.10 B.	5.12 C. 6.10 C.
14	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
14	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓
13	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
13	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓
12.5	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
12	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
12	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓
12	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
12	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓
11	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
11	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓
10.3	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
10.3	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓
10	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
10	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓
9	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
9	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓
8	12	25-72	6.25-22	6.25-22	6.25-22	✓	✓	✓	✓	✓	10.29-73	✓
8	12	25-73	6.25-23	6.25-23	6.25-23	✓	✓	✓	✓	✓	10.29-73	✓

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FPC Quality Control  
PTL Quality Manager  
JAJ Quality Control  
FPC Quality Control  
S-1 Quality Control

**NOTE:** In each column, opposite each photo piece no., the inspector shall sign in the lower of the two squares and record the inspection date just below his signature. Report discrepancies on

J. A. JONES CONSTRUCTION CO.  
MICHIGAN POWER CORPORATION  
CRYSTAL RIVER NO. 3

# POOR ORIGINAL

## **QUALITY CONTROL CHECK LIST FOR NUCLEAR SERVICE AND DECAY HEAT UNDERGROUND PIPING INSTALLATION**

## 43" CONCRETE PIPE

**POOR ORIGINAL**

JOHN COONES CONSTRUCTION CO.

FLORIDA POWER CORPORATION

CRYSTAL RIVER NO. 3

QUALITY CONTROL CHECK LIST FOR NUCLEAR SERVICE AND DECAY HEAT UNDERGROUND PIPING INSTALLATION

48" CONCRETE PIPE

PARAGRAPHS	6.1	6.3	6.5	6.7	6.8	6.9	5.18 B. 6.10 B.	5.18 C. 6.10 C.
PIPE SEGMENT	SEATING	JOINT PREPARATION	JOINT FITUP ALIGNMENT	PIPE CONDITION	EXTERIOR JOINT MORTAR	INTERIOR JOINT MORTAR	ZONE A/H TO SPRINGLINE	ZONE H
8	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
6.C	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
5	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
6	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
7	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
8	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
5.D	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
6.C	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
5.F	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1
5.A	✓ 2.6.1.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1	✓ 2.2.2.2.1

NOTE: In each column, opposite each pipe piece no., the inspector shall sign in the upper left square, and record the inspection date just below the signature. Report discrepancies on Form No. FPC Q-009.

Date checked  
by  
Signature  
FPC Quality Engineer  
FPC Chief Inspector  
FPC Quality Control  
Date checked  
Signature

POOR ORIGINAL

J. A. JONES CONSTRUCTION CO.  
FLORIDA POWER CORPORATION

CRYSTAL RIVER NO. 3

## **EQUALITY CONTROL CHECK LIST FOR NUCLEAR SERVICE AND DECAY HEAT UNDERGROUND PIPING INSTALLATION**

## 48" CONCRETE PIPE

REVISION TO THE JAI-1011 PROCEDURE

R.B.J. JAN 11 '73

Construction Procedure No. JAI-WS2 Date 7/14/72

Field Change No. 1 D.A. OLIVER 1/10/73  
Originator \_\_\_\_\_ Date \_\_\_\_\_

Reason for Change:

Extreme difficulty is being encountered in compacting Zone III fill in the confined space between the Auxiliary Building and the alongside 48" cast iron pipe running south from the anchor block at the north end.

Description of Method:

Backfill requirements of paragraphs 5.18 C,D,E and F are changed for the space confined by the Auxiliary Building foundation, Auxiliary Building foundation overhang and the 48" cast iron nuclear services intake piping alongside the Auxiliary Building.

In this confined space, sand shall be sluiced in from the pipe's spring line up to the bottom of the building overhang.

Sand shall be used outside the confined space where it is practicable to compact Zone III fill.

Approved:

FPC Construction Supervisor

C. Pechler 10 Jan 73

Concurrence:

FPC Quality Engineer

R.S. Price for DIP 10 Jan 73

Notification:

Q.C. Dept. Representative

H. Gandy 1/10/73

Review:

GAI/QA Dept.

A. Ray Eastington 1/10/73

Comments:

**POOR ORIGINAL DOCUMENT QUALITY**

Construction Procedure No. JAJ-W52

Date 7/14/72

Field Change No. 2

D.A. OLIVER

Originator

Date

Reason for Change:

Hydrostatic tests of the Nuclear Service and Decay Heat Sea Water Underground Piping will be performed by the Florida Power Corp. Test Group and results recorded on FPC forms.

Description of Method:

The leak test requirement (section 5.14) and associated quality control reports (section 5.20 and the "Leak Test" column of Form No. JAJ-131, Exhibit 3) are deleted from JAJ-W52.

Approved:

FPC Construction Supervisor

Gettibes Jr. 1-16-73

Concurrence:

FPC Quality Engineer

R. Davis for D.Q.P. 16 Jan '73

Notification:

Q.C. Dept. Representative

J. Gandy 1/16/73

Review:

GAI/QA Dept.

S-Ray Engineering 1/16/73

Comments:

**POOR ORIGINAL**

**DOCUMENT CONTROL**

111-520 - FIELD CHANGES TO CONSTRUCTION PROCEDURES

L-2-72

Construction Procedure No. JAJ-W52 Date 7/14/72  
Field Change No. 3 Originator T. McAllister Date February 2, 1973

Reason for Change:

To define "visually apparent cracks" as stated in paragraph 6.7.  
Reference: Letter from Lock Joint Pipe, Interpace Corporation,  
dated January 25, 1973.

Description of Method:

Minor or "hairline" cracks need not be reported, as this condition is  
considered normal by the manufacturer.

Approved:  
FPC Construction Supervisor

C. Parker 2 FEB 73

Concurrence:  
FPC Quality Engineer

J.W. Rednick, P.E. 2 FEB 1973

Notification:  
Q.C. Dept. Representative

J.W. Campbell 2/2/73

Review:  
GAI/QA Dept.

S. Jay Brinkley, C.E. 2/2/73

Comments:

**POOR ORIGINAL**

DOCUMENT OF COMPLETION

Construction Procedure No. JAC-WF2

7/14/72

Field Change No. 4

J.R.Lundsen 2/2/73  
Originator Date

Reason for Change:

The Neolube lubrication requirement of section 7.1.2 of FPC-W48 does not apply for installation of the nuclear service and decay heat sea water underground piping.

Description of Method:

Add to Section 5.12 B: Neolube lubrication shall not be required.

Approved:  
FPC Construction Supervisor

C. Pacion 2/2/73

Concurrence:  
FPC Quality Engineer

D.W.Pedersen, Jr. 2/2/73

Notification:  
Q.C. Dept. Representative

J. G. M. 2/2/73

Review:  
GAI/QA Dept.

L. Ray Bruberg, Jr. 2/2/73

Comments:

POOR ORIGINAL

DOCUMENT CONTROL

Construction Procedure No.

JAJ-WER

D 7/11/72

Field Change No.

5

D. Sheet 2/14/73

Original

Date

Reason for Change:

Testing will not be required for the first joint (intake end) of the 48" Cast Iron Intake Pipe (Nuclear Service and Decay Heat Sea Water Piping) since this joint will be completely encased in concrete.

Description of Method:

Add to section 5.14: No leak test is required for the first joint at the intake end for both lines of pipe. These two joints will be completely encased in concrete.

Approved:

FPC Construction Supervisor

C. Parker

2/14/73

Concurrence:

FPC Quality Engineer

J.W. Pedersen

2/14/73

Notification:

Q.C. Dept. Representative

R.L. Hart

2/14/73

Review:

GAI/QA Dept.

O.W. Hall

2/14/73

Comments:

POOR ORIGINAL

Construction Procedure No. JAM-552

Date 7/11/13

Title Work Procedure - Installation of Nuclear Service and Decay Heat  
Field Change No. 82 Water Underground Piping

R. R. Amundson

Date 9/27/13

Reason For Change: to add welding requirements of steel wedge ring.  
See attach Interpace Dwg. D-4-390 Rev. A.

## DOCUMENT CONTROL

Description of Method: welding in accordance with Mfg. Dwg. D-4-390  
weld to be a 1/4" fillet weld around the  
circumference, except for the bottom 12 inches.  
The welding sequence to be the skip weld method  
as shown on D-4-390. Use qualified welders and  
a qualified welding procedure for this weld.  
Weld shall be visually inspected to assure the  
weld is at least 1/4 inch fillet and that the  
welds are free from cracks.  
The weld inspection will be reported on a Q-009  
inspection report, and identified to the follow-  
ing example (piece #61 to #60).

## POOR ORIGINAL

Route in order shown below:

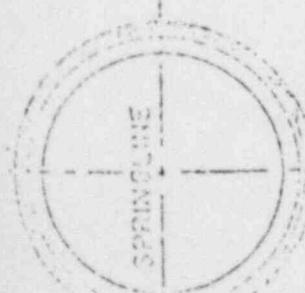
- (1) Approval: FPC Construction Supv.
- (2) Notification: QC Dept. Supv.
- (3) Review: GAI/QA Representative
- (4) Concurrence: FPC Quality Engineer
- (5) Distribution: Document Control

Signature

Date

WIRELESS PRINTING SUPPLIED BY ONE SECTION.

POOR ORIGINAL



SUMMARY

MORTEN COVARI

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STEEL DELLING

DOD

## COLLECTOR'S GUIDE

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0303m 51 5500 030335 20 1234567 1-1

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1

NA

**STYLISH SPICED RINGS**

ORGANIC MELT REINFORCEMENT

JOINT D(A.)

POC

4.5" 3" C

VISCOSE PING TOGE SUPPLIED IN

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RECD - 2/11/61

D-4-583

4.5-11. HIGH STRENGTH PRESTRESSED CONCRETE  
EMBEDDED CYLINDER WITH RUGGED AND  
SOLID INSULATING JOINT

CRYSTAL SIZE IN MILLIMETERS						CRYSTAL SIZE IN MILLIMETERS
NAME OR SYN. ON PLATE	CONE	JOINT DIAM.	JOINT CIR.	A	B	SELL O.D.
43"	3" or 5"	4 $\frac{1}{2}$ "	5 $\frac{1}{2}$ "	10 $\frac{1}{2}$ "	2 $\frac{1}{2}$ "	5 $\frac{1}{2}$ "

# POOR ORIGINAL

## FIELD CHANGE TO CONSTRUCTION PROCEDURE

Construction Procedure No. JAJ-W52 Date July 14, 1972

Title Work Procedure - Installation of Nuclear Service and Decay Heat Sea

Field Change No. 7 J. R. Amundson Originator 10/5/73 Date

### Reason For Change:

Void Field Change #6 to Procedure JAJ-W52, and write Field Change #7 to have the welding of the wedge ring to bell ring meet the vendors approved welding requirements.

### Description of Method:

Weld the wedge ring to the bell ring, to the requirements of Interpar drawing D-4-390, using E-6010 Electrodes.

Visually inspect the welds to assure the 1/4" Fillet or Flush weld is free of crack.

The welding inspector will report the results of all acceptable welds on a Q-009 inspection form. Example (Piece #61 to 60).

### AWS recommended Amperage for:

1/8" rod 75 to 125 AMPS  
5/32 rod 110 to 170 AMPS

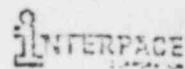
DOCUMENT CONTROL

### Route in order shown below:

- (1) Approval: FPC Construction Supv. C. G. Jackson 10
- (2) Notification: QC Dept. Supv. M. J. Smith 11
- (3) Review: GAI/QA Representative D. H. Price 12
- (4) Concurrence: FPC Quality Engineer P. O. Stiles 13
- (5) Distribution: Document Control \_\_\_\_\_

# POOR ORIGINAL

## Lock Joint Pipe



260 CHERRY HILL ROAD / PARSIPPANY, NEW JERSEY 07054 / 201-335-1111 / TELEX: 136421 / CABLE: INTERPACE

September 28, 1973

Florida Power Corporation  
P. O. Box 14042  
St. Petersburg, Florida 33733

Attention: Mr. J. Landers

Subject: Job No. CO-69-15  
Rai Level, Florida

Gentlemen:

We have been requested by our Mr. Burdell to comment on the welding method used at the seismic joint end of the pipe.

The seismic steel wedge ring may be welded with a flush weld rather than a fillet weld as shown on the drawings provided that the weld develops the full strength equivalent to a one-quarter inch fillet.

Very truly yours,

A handwritten signature in cursive ink that appears to read "Harry W. Brenner".

Harry W. Brenner  
Project Engineer

HWB:cmas

DOCUMENT CONTROL

SEALING JOINT IN PRESTRESSED CONCRETE  
EMBEDDED CYLINDER TYPE WITH RUBBER AND  
STEEL EXPANSION JOINT

DRAWING NUMBER  
D-4-300

SCALE 1:50

RED LEVEL

PLATE 6 1573  
GILBERT ASSOCIATES, INC.  
APPROVED

FLOOD PROOF CONCRETE  
CRYSTAL SPRINGS UNIT 3  
OR EQUAL (5 YD. OCEAN)

STEEL WEARSE SEAL

JOINT DIA.

WIRE MESH REINFORCEMENT

CENTER

LINE

FIELD CHANGE TO CONSTRUCTION PROCEDURE

8/18/73

Construction Procedure No. JAJ-W52 Date July 14, 1972

Title Work Procedure - Installation of Nucleair Service & Decay Heat Sea Water Underground Pipe

Field Change No. 8 J. R. Amundson 10/18/73  
Originator Date

Reason For Change: Delete Para 6.6 and last four sentences of para. 6.7

Add: Pipe joint shall be sealed at a convenient time  
following the Manufacturers Drawing.

Ref. Drawings : D-3-682  
SS-CO-69-15-7 & D-3-691  
D-4-390 (See Field Change #7)

Route in order shown below:

- (1) Approval: FPC Construction Supv.
- (2) Notification: QC Dept. Supv.
- (3) Review: GAI/QA Representative
- (4) Concurrence: FPC Quality Engineer
- (5) Distribution: Document Control

DOCUMENT CONTROL

Signature	Date
<u>C. J. Schmitz</u>	<u>10-18-73</u>
<u>J. R. Amundson</u>	<u>10/18/73</u>
<u>L. Ray Oberholzer</u>	<u>10/18/73</u>
<u>T. P. Schaefer</u>	<u>10-18-73</u>

Form No. FPC Q-50

Rev. 5/4/73

POOR ORIGINAL

100' USE BURIED DIAPER WITH  
SILVER AND RAIL - NEK  
SANDING COATING LINING

STEEL CLAMP RING

WHITE MESH REINFORCEMENT

STEEL SPACER

CLAMP

—MORTAR PLACED AFTER INSTALLATION

WHITE MESH REINFORCEMENT

THE GRUSH COAT OF RAM - NEK  
PATCH (LITHON APPLIED)

A HIGH TENSILE  
CONCRETE  
PRESTRESSED PIPE

COATING

NOTES:

1. GROUT MUST BE CEMENTED WITH FRESH CEMENT  
DURING OPERATION #52 IN THE SAME DAY AS  
FOR REINFORCING JOINT.

2. ADD 0.05' TO TOTAL LENGTH OF 9.50' FOR  
PRODUCT DUE FROM LENGTH OF 9.50'  
OF STANDARD FITTINGS.

STEEL CYLINDER

JOINT DEPTH

MORTAR PLACED AFTER  
INSTALLATION

STEEL SPACER RING

DIA. X 2" BOLT

ASTM A-325

SAE GRADE 5

SCREWCUT CASTING

ASTM A-27

GRD 50-36

VIEW OF CLAMP RING, AT CONCRETE LINE

SECTION XX

SECTION XX TO BE REFERENCED IN THE FIELD

CLAMP RING IS ATTACHED TO REEL  
BY SPACER SPACER UNTIL SPACER IS IN CONTACT WITH REEL.

CLAMP RING IS TIGHTENED INTO POSITION.

REASSEMBLED DIAPER IS TIGHTENED ON PIPE AND TIGHTEN STRAPPING. GROUT JOINT WITH MORTAR.

REASSEMBLED DIAPER IS SEPARATE 18' LONG RAM - NEK PATCH AND PLACE OVER OPENING TO GIVE MORTAR WITH RAM - NEK.

JOINTS AT ENDS ARE TO OVERLAP 2 5/8" FOR ALL PIPE SIZES.

ELEVATION

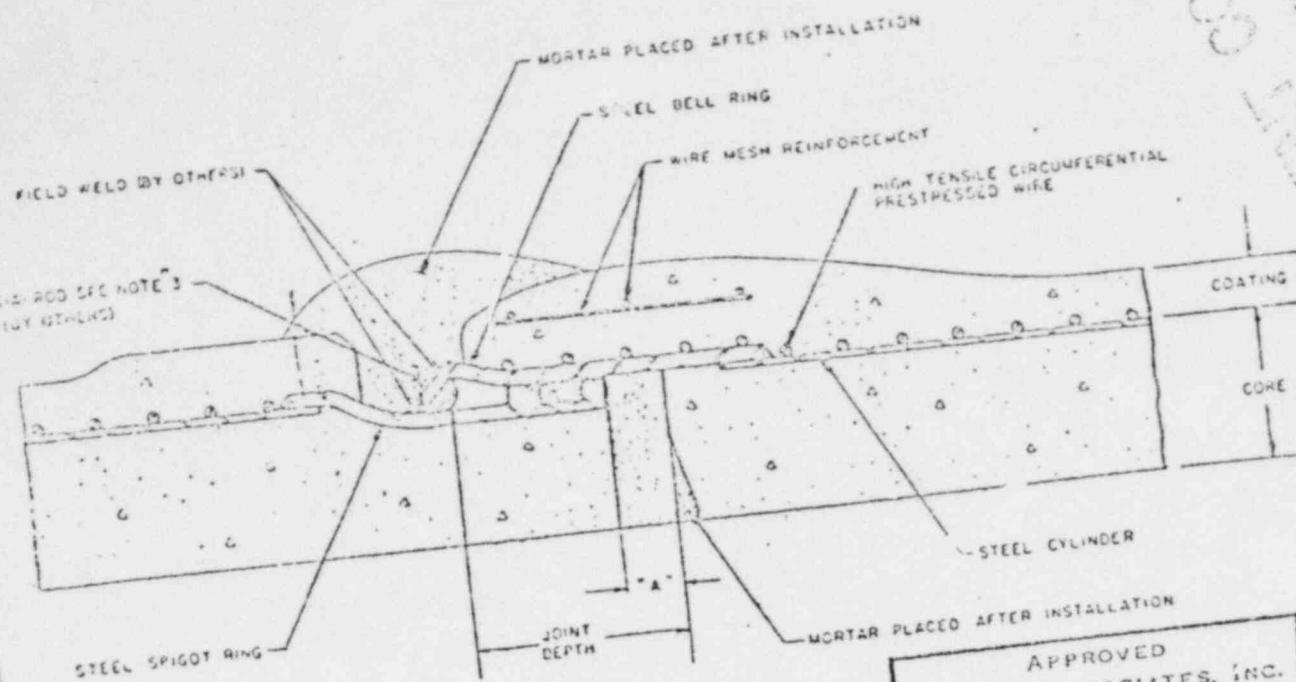
DESCRIPTION

100' 1 1/4"

PIPE DIAM	Y. MARKS	JOINT DEPTH	GROUT LINE	CLAMP TYPE
42"	10'-0"	4 1/2"	10'-0"	1
36"	10'-0"	4 1/2"	10'-0"	2

ONE MEDIUM SIZE TIE RAIL TWO TIES  
ONE TIE RAIL AND ONE TIE RAIL

POOR ORIGINAL



NOTES

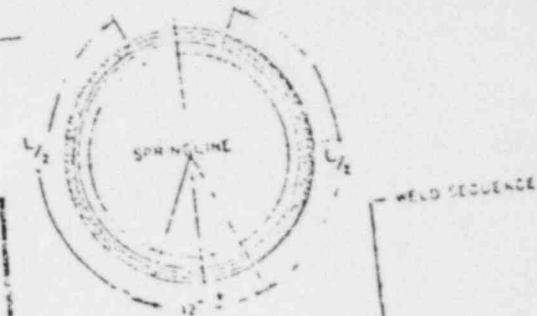
- 1 GASKET MUST BE CHECKED WITH FEELER GAGE THE SAME AS FOR REGULAR JOINT
- 2 USE SKIP WELD METHOD
- 3 USE 5/8 DIA PLATE ROD
- 4 USE E-6010 ELECTRODES
- 5 WHEN JOINT OPENINGS ARE USED WITH FIELD WELDED JOINTS, ONE SIDE OF EXCUTED SCAVENGE ABOVE AND THE OTHER SIDE KEPT AT THE "A" WELDING POSITION.
- 6 INTERVALS WILL NOT BE RESPONSIBLE FOR WELDED JOINTS IF THE SKIP WELD METHOD IS NOT USED. HEAT CONCENTRATIONS FROM WELDING ARE SUFFICIENT TO MELT THE RUBBER GASKET IF EXTREME CARE IS NOT USED.
- 7 LAD 005 TO LAD LENGTH OF SPOT END OF STANDARD 45° FITTINGS AND 006 TO LAD LENGTH OF SPOT END OF STANDARD 48° FITTINGS.

FLORIDA POWER CORP.  
CRYSTAL RIVER PLANT UNIT 3

A	CHANTEL ANGELA GUY OF WILDO OPPOSITION	11-24-63	KEY A
NEW			

PIPE DIAMETER	PIPE LENGTH	JOINT DEPTH	OPEN END LENTH	CLOSED END LENTH
42	10.00	3/4	10.33	10.00
40	10.00	3/8	10.25	10.00

PIPE HAVING ONLY THE ONE END HELD IN POSITION  
END LENGTH.



**FR T T ST T T S**  
SKIN-SEAL METHOD  
ALLOWS SAME FOLLOW SEQUENCE UNTIL FINAL LENGTH OF RING IS REACHED TO RINGS  
STEEL TIEO JOINT

LACED AFTER INSTALLATION  
APPROVED  
GILBERT ASSOCIATES, INC.  
W. E. MEEK  
MAR 8 1973

ALWAYS MAKE & FOLLOW SEQUENCE UNTIL TIME LENGTH OF  
WELDED TO RINGS

<p><b>RUBBER &amp; STEEL TIE JOINT</b>  <b>FIELD WELDED TYPE</b></p> <p><b>12 AND 16 PRESTRESSED CONCRETE CYLINDERS</b></p>	<p><b>12-13-69</b></p> <p><b>3-3-691</b></p>
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200 LEVEL, F.M.

