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ACTION PLAN

FOR

MARK I LOAD DEFINITION REPORT

REVISIONS TO

IMPLIMENT NRC

ACCEPTANCE CRITERIA FOR

.

COMPLETION OF LONG TERM PROGRAM

8992209112 R.M.N.

NRC ACCEPTANCE CRITERIA

OCTOBER 31, 1979

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- O CLARIFICATION OF REQUIREMENTS IN CRITERIA
- DIRECT IMPLICATION OF NRC CRITERIA
- FOLLOW ON CHANGES IN IMPLIMENTATION DUE TO CLARIFICATION DISCUSSIONS

LOAD DEFINITION REPORT FOR MARK I LONG TERM PROGRAM COMPLETION

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o LDR REVISION PROGRESSING BASED ON NRC ACCEPTANCE CRITERIA IMPLEMENTATION

o ISSUE REVISED LDR TO UTILITY/AE's FOR STRUCTURAL EVALUATIONS

SPRING 1980

REVISION A - LDR CATEGORIES

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- CATEGORY I LOADS WHICH CAN BE CHANGED IN THE LDR IMMEDIATELY
- CATEGORY II LOADS WHICH WILL REQUIRE A NOMINAL AMOUNT OF WORK TO REVISE LDR
- CATEGORY III- LOADS WHICH WILL REQUIRE A MORE EXTENSIVE AMOUNT OF WORK TO REVISE LDR

CATEGORY I LOADS

- o VENT SYSTEM THRUST LOADS
- o POOL SWELL LOADS

- TORUS NET VERTICAL
- TORUS SHELL PRESSURE HISTORY
- VENT SYSTEM IMPACT & DRAG
- DOWNCOMER IMPACT & DRAG
- MAIN VENT IMPACT & DRAG
- FROTH IMPINGEMENT
- O T-QUENCHER LOADS
 - TORUS SHELL PRESSURES

CATEGORY II LOADS

O POOL SWELL LOADS

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- LOCA JET
- VENT HEADER DEFLECTOR LOADS QSTF METHOD
- o CONDENSATION OSCILLATION
 - LOCA DRAG LOADS
- o T-QUENCHER LOADS
 - WATER JET LOADS
 - BUBBLE DRAG LOADS

CATEGORY III LOADS

- O POOL SWELL LOADS
 - IMPACT & DRAG ON OTHER STRUCTURES ABOVE THE POOL
 - LOCA BUBBLE DRAG
 - VENT HEADER DEFLECTOR LOADS

ANALYTICAL METHOD

- o CONDENSATION OSCILLATION
 - LATERAL LOADS ON DOWNCOMERS
- o CHUGGING
 - LOCA DRAG LOADS
 - LATERAL LOADS ON DOWNCOMERS

POOL SWELL DOWNLOAD

NRC CRITERIA:

 $DOWN = DOWN_{MEAN} + 2 \times 10^{-5} (DOWN_{MEAN})^2$

CLARIFICATION:

DOWN = DOWN_{MEAN} (1 + 0.00002 PEAKDOWN_{MEAN})

DEFLECTOR LOAD DEFINITION

GOVERNING EQUATIONS

O DEFLECTOR FORCE (REF 1)

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='= pg Ai +	(miy)+pAi)7	+ 21 1 +	-(Fs - 7	Fis) W
in				-
BOUYANET	AICELERA TIV	VELAUT	LOCAL	STATIC
	22.26-	DRAG-	PRES	SUICE



REF 1 KAPLAN, P. & SILGERT M.N. "IMPACT FORCES ON PLATFORM MERIZON THE MEMBERS IN THE SPLASH ZONE", OFF. SHOKE TECHNOLOGY CONF. DADER OTC 2494, MAY 1976.

DEFLECTOR LOAD DEFINITION COVERNING EQUATIONS (CONT)

· ADDED HASS FROM DRAG COEFFICIENT

$$\frac{\partial nu'}{\partial y} = \pm \rho W C_{D}(y)$$

$$m' = \pm \rho W \int_{0}^{y} C_{D}(s) ds$$

$$C_{D}(y) PER ATTACHED GERDAS EXCEPT THAT
STEADY PORTION IS EXCLUDED (G = 0.7)$$

· LOCAL STATIC PRESSURE

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$$(F_s - P_{fs})W = SWp(1 + \frac{F_s}{m_e})$$

WHERE $F_s = 70RUS$ DOWINGLOAD

Me = EFFECTIVE MASS



RANGE OF PARAMETERS INFLUENCING DEFLECTOR LOADS (FULL SCALE VALUES)

DEFLECTOR LOADS MEASURED IN QSTF (6 PLANTS - 12 CONFIGURATIONS)

REMAINING PLANTS FOR WHICH DATA IS NOT AVAILABLE (7 PLANTS)

1)	(DISTANCE FROM BOTTOM OF DEFLECTOR TO WATER SURFACE)	0 - 21.05	0 - 14.29
2)	DEFLECTOR WIDTH (IN)	. 25.3 - 30.0	20.0 - 26.0
3)	P (PSI/SEC)	46.1 - 74.0	. 54.4 - 74.7
4)	DOWNCOMER SUBMERGENCE (FT)	3.0 - 4.25	3.33- 4.4

CI FADANCI



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IMAGE EVALUATION TEST TARGET (MT-3)



MICROCOPY RESOLUTION TEST CHART



- 2) Surface acceleration given by



AND

= s x z/L ·xz/L de - 1 ×z/L ×z/L de

Run existing analysis (NEDO-24612) using NRC criteria drag coefficients and histories from 2) to produce 4 load histories $\binom{2}{L} = 0, .5, 1.0 \notin QSTE$ 3)

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4) ADD IMPACT SPIKE FROM STEP 3 (QSTF ANALYSIS) TO QSTF MEASURED DATA AT TIME OF FIRST REST - IN DATA AND ADJUST TIME SCALE TO CAUSE TIME OF IMPACT, Gi, OF QSTF ANALYSIS AND DATA TO AGREE



5) COMPARE CALCULATED FORCES FROM STEP 31 AT

IDEN TICAL VALUES OF Y, THE PENETRATION DEDTH

F240/Fast GUSTE QZ/2 FUSTE Fzy, 4 d. Fy' F.' θ, θ, Ч, E Q, 4. 0,-F5' 42 F' F.' 43 8, 00 4.

6) MULTIPLY THE CURVE FROM STEP 4) by AF Buste THE APPROPRATE FUX/FUSTE RATIO, AND PLOT AT THE ASSOCIATED B2/2 VALUE TO CREATE 3 FORCE HISTORIES (2/2 = 0, .5 = 1.0)

7/ SCALE THE RESULTS OF STEP 6) TO FULL SCALE 64:

 $F_{FS}' = \frac{1}{\lambda^2} F_{STEP 0}'$ force per which length

AT CORRESPONDING THES

BES = The Brep 4

WHERE $\lambda = OSTE STALE FACTOR (LESS THAN$ ONE) FIGURE 2.10-3



Impact and Steady Drag Force Correlation for Type 2 Deflector





THRUST LOAD

- ANALYTICAL MODEL FOR DOWNCOMER INTERNAL
 PRESSURE CONSERVATIVE FOR △P AND ZERO △P
 PREDICTIONS
 - VENT CLEARING PREDICTED LATER THAN QSTF
 - DOWNCOMER PRESSURE DURING TIME BETWEEN
 VENT CLEARING AND BUBBLE BREAKTHROUGH
 IS GREATER THAN QSTF
 - ZERO △P VENT CLEARING PREDICTED EVEN LATER

S. A. HUCIK 12/19/79





SRV TORUS SHELL PRESSURES

FIRST ACTUATIONS:

- USE LDR FIRST ACTUATION PRESSURE PREDICTION
- USE LDR FIRST ACTUATION FREQUENCY PREDICTION ± 25%

SUBSEQUENT ACTUATIONS

- USE LDR FIRST ACTUATION PRESSURE PREDICTION
- USE LDR SUBSEQUENT ACTUATION FREQUENCY PREDICTION ± 40%

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SRV TORUS SHELL PRESSURE ATTENUATION

SOME PRESSURE PREDICTIONS RESULT IN:



RESOLUTION: LIMIT PRESSURE TO "X" AT AND BEYOND a

SEE EQUATION 2-22 NEDE - 21878-P



SRV TORUS SHELL PRESSURE ATTENUATION

PRESSURE PREDICTIONS AT LARGE DISTANCES FROM

THE QUENCHER RESULT IN:



RESOLUTION: ATTENUATE TO ZERO AT WATER SURFACE

SEE EQUATION 2-23 NEDE - 21878 -P

LDS 12/19/79 SRV TORUS SHELL PRESSURES FOR

MULTIPLE DISCHARGE LOADS

NRC CRITERIA:

COMBINED PEAK TORUS SHELL PRESSURE LIMITED TO MAXIMUM OF 1.65 TIMES PREDICTED PEAK BUBBLE PRESSURE



CLARIFICATION:

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SUBMERGED STRUCTURES

LOCA BUBBLE DRAG LOAD - FLOW FIELD

NRC CRITERIA :

- AFTER CONTACT BETWEEN BUBBLES OF ADJACENT DOWNCOMERS, POOL SWELL FLOW FIELD ABOVE DOWNCOMER EXIT.
 - UNIFORM FLOW FIELD ABOUE DOWNCOMER EXIT.
 - FLOW FIELD VELOCITY DETERMINED FROM ASTE PLANT UNIQUE TESTS.
 - DRAG LOAD CALCULATION ENDS WHEN BUBBLE ENGULFS STRUCTURE.

SAH-1 12/19/79



LOCA BUBBLE DRAG LOAD - FLOW FIELD.



SAH-2 12+19/79

SUBMERGED STRUCTURES

QUENCHER ARM LOADS

- QUENCHER ARM LOADS BASED ON ASSUMED BUBBLE ASYMMETRY.
- · MAXIMUM SIDE FORCE CALCULATION
 - MAXIMUM BUBBLE PRESSURE ON ONE SIDE OF ARM.
 - ZERU ON OPPOSITE SIDE



- S MAXIMUM MOMENTUM CALCULATION.
 - MAXIMUM POSITIVE BUBBLE PRESSURE ON DIAGONAL OF ARMS
 - ZERO ON REMAINING POSITIONS



SAH-1 12/19/79 QUENCHER ARM LOADS

- MONTICELLO TQ TEST DATA SUPPORTS
 NO BUBBLE ASYMMETRY.
 - BUBBLE PRESSURES IN PHASE ON EACH SIDE OF TO ARMS
 - DIFFERENTIAL PRESSURES SUPPORT
- · LOAD DEFINITION METHODOLOGY CONSERVATIVE BASED ON MONTICELLO DATA.

54H-2 12/19/79

FSTF TESTING STATUS

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FSTF SNAP TEST-TEST MATRIX AND STATUS

PURPOSE:

To Determine Damping and Natural Frequency of Mark I Downcomers

TEST MATRI	X:	DC WATER	WETWELL	
TEST NO.	D/C PAIR	CONDITION	LEVEL	LEVEL
1	5&6	UNTIED	Dry	FLOODED
2	788	UNTIED	Dry	FLOODED
3	5&6	TIED	Dry	FLOODED
4	788	TIED	Dry	FLOODED

SCHEDULE :

COMPLETE	TESTS	FEBRUARY 1980
COMPLETE	Test Report	March 1980
COMPLETE	EVALUATION REPORT	APRIL 1980

SNAP TEST SCHEDULE

Test	1				S	chedu	le By We	eek Begt	Inning			
	11/26 th	ru] '7	1/14	1/21	1/26	2/4	2/11	2/18	2/25	3/4	3/11	3/18
Finalize Test Procedure]										
Design Test Hardware	Luman											
Install Instrumentation												
Checkout Instrumentation '					C							
Snap Tests 1 & 2						1						
Snap Tests 3 & 4												
Report Preparation (Test) (Wyle)												
Model/Data Evaluation Report (Bechtel)										[3
Meeting with NRC								[]				

STF C/O RETEST -TEST MATRIX AND STATUS

PURPOSE:

To provide additional statistical basis for C/O test data during large liquid break to assure LDR C/O load bounds test data

TEST MATRIX:

TEST No.	SIMILAR TO:
M-11	M-8
M-12	M-8

SCHEDULE:

COMPLETE	Tests	JULY 1980			
COMPLETE	TEST REPORT	November 1980			
COMPLETE	EVALUATION REPORT	DECEMBER 1980			

NON 001 -SEPT ----1.2 440 JULY | AUG ***** MAY JUNE -----Sec. 8.1 G.E. MK I FSIF States a -MAR APR 1.2 and the Ar be - TU BLAN FEB ----1980 JAN 1979 70EC Prepare Evaluation Rpt. Prepare Final Test Rpt. M-11 (Repeat M-8) M-12 (Repeat M 8) Site Activation Procedure Prep. Data Reduction Data Analysis Checkout Test Program Mgt. Task WYLE LABS Snap Test

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MILLSTONE

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CLARIFICATION OF DRAG FOR POOL FALLBACK LOADS

The standard drag on a structure o° length L is given by:

 $F_s = C_D D_{eq} L \frac{\rho U^2}{2g_c}$

where D_{eq} is the equivalent diameter, as defined in Section 2.14. For cylinders, D_{eq} is simply the diameter, while for structures with sharp corners, $D_{eq} = 2^{\frac{1}{2}} L_{max}$, where L_{max} is the maximum transverse dimension.

The acceleration drag is given by:

$$F_{A} = \frac{\rho V_{A}}{g_{c}} \frac{dU}{dt}$$

where V_A is the acceleration volume, as given in Table I of NEDO 21471, and dU/dt for a free fallback is the gravitational acceleration, g.

The initial fallback load on structures within the air bubble, estimated using the froth fallback criteria of 25% water density and a maximum fallback velocity based on $(y_m - y_s)$ is:

$$F_i = SL \frac{\rho}{4} \frac{U^2}{g_c}$$

where S is the maximum horizontal cross-sectional dimension.

By inspection it may be noted that:

$$F_s = (2C_D) \frac{D_{eq}}{S} F_i$$

Since $D_{eq} \ge S$, it follows that for any value of $C_D > 0.5$, the standard drag load will bound the initial impact load. To provide sufficient conservatism for fallback densities greater than 25%, the minimum value of C_n should be taken as 1.2.

The ratio of standard drag to acceleration drag depends on the shape of the structure,

$$\frac{F_s}{F_A} = \frac{C_D D_{eq} L (y_m - y_s)}{V_A}$$

or

$$rac{s}{A} = C_D \frac{(y_m - y_s)}{\pi R}$$
 for cylinders

 $\frac{F_{s}}{F_{A}} = C_{D} \left(\frac{2}{4+\pi}\right) \frac{(y_{m}-y_{s})}{S} \text{ for square rods (side, S)}$

Since $(y_m - y_s)$ is on the order of 10 feet and the submerged structures have cross-sectional dimensions on the order of 0.1 to 1.0 foot, the standard drag is normally dominant. This is in contrast to the pool swell drag loads, where acceleration drag dominates due to rapid bubble growth.

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c_D

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POOR ORIGINAL