

**IRSN**

INSTITUT  
DE RADIOPROTECTION  
ET DE SÛRETÉ NUCLÉAIRE

*Faire avancer la sûreté nucléaire*

# Global sensitivity analysis applied to riverine flood modelling

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IRSN/SCAN

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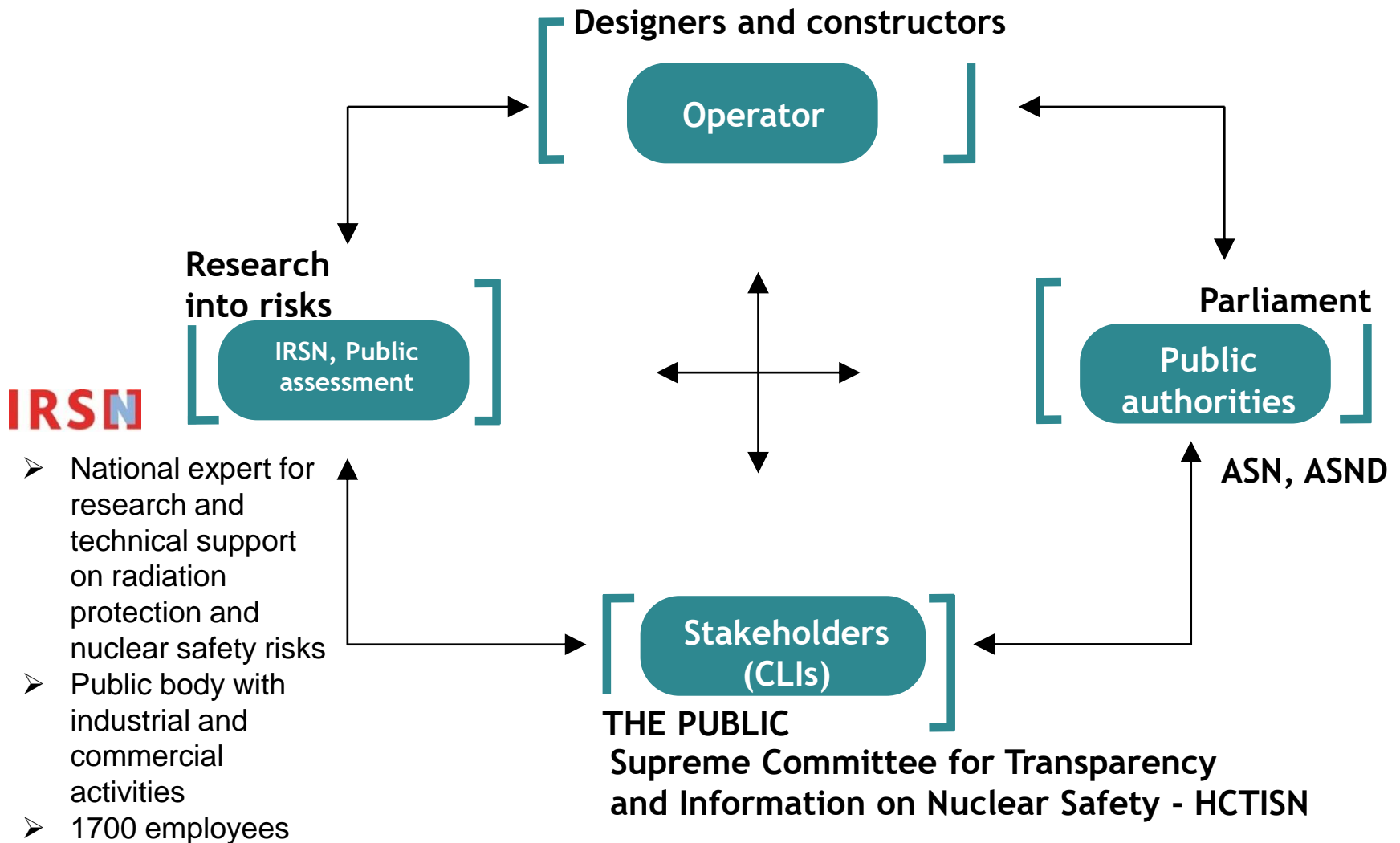


*La Garonne river, France  
Picture taken during the 1981 flood event*

# Presentation outline

- I. Context
- II. Uncertainty analysis (UA) and global sensitivity analysis (GSA)
- III. Preliminary studies applied to hydrodynamic models
- IV. Levee breaches study on La Garonne river
- V. Dependent inputs in hydraulic studies
- VI. Conclusions and perspectives

# Institutional environment



# French ASN guide “Protection of Basic Nuclear Installations against External Flooding” (2013)

□ Uncertainties taken into account through a **robust, conservative and deterministic approach**

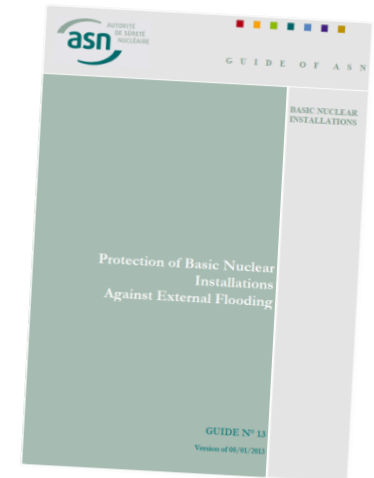
□ Upper bound of confidence interval, conservative assumptions defined for initial states...

□ Concerning the hydraulic modelling, **penalization of the most influencing parameter**

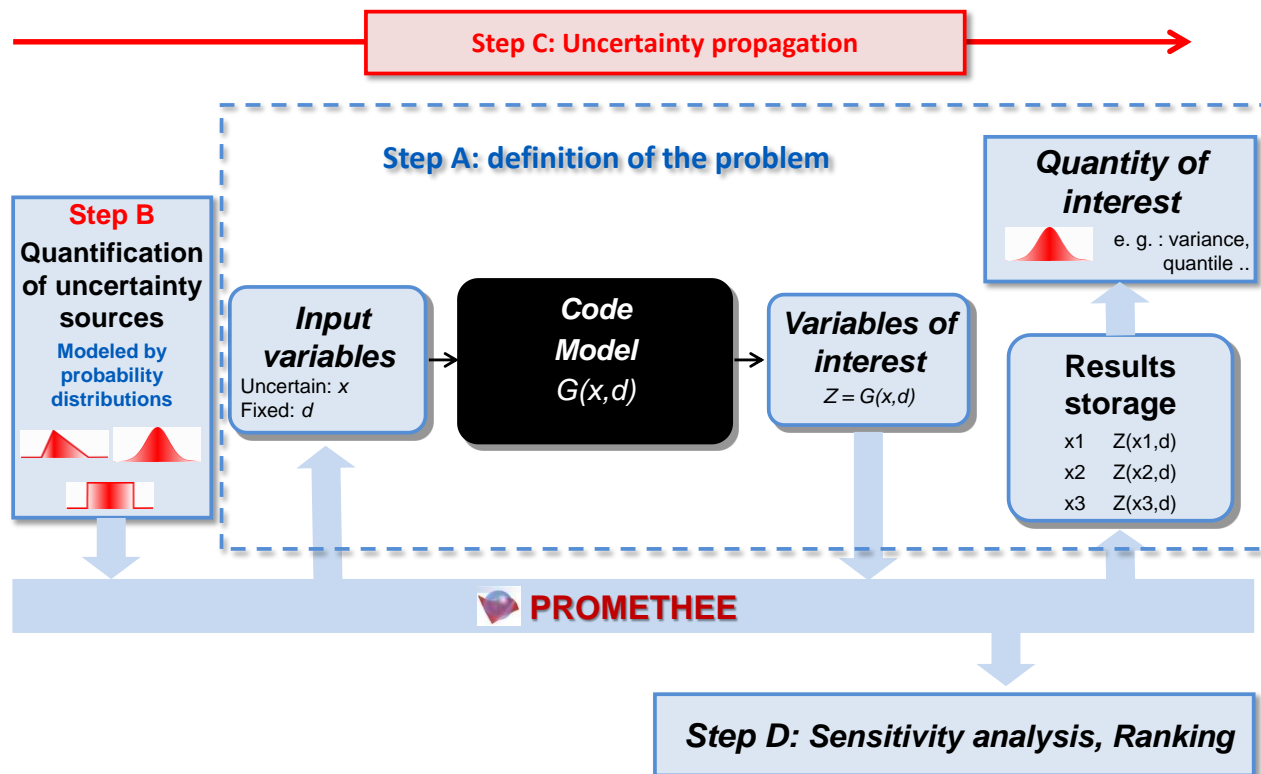
□ Identifying the most influencing parameter and giving it a penalizing value **is challenging and usually questionable...**

⇒ objective to develop a rigorous methodology to identify and penalize the most influencing parameter

⇒ objective to develop a probabilistic flood hazard assessment method

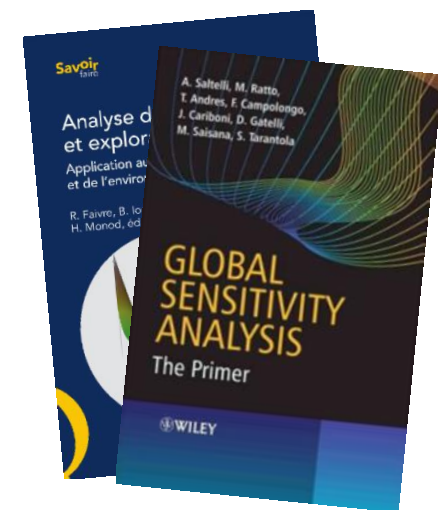


### ■ Main steps of uncertainty analysis and global sensitivity analysis



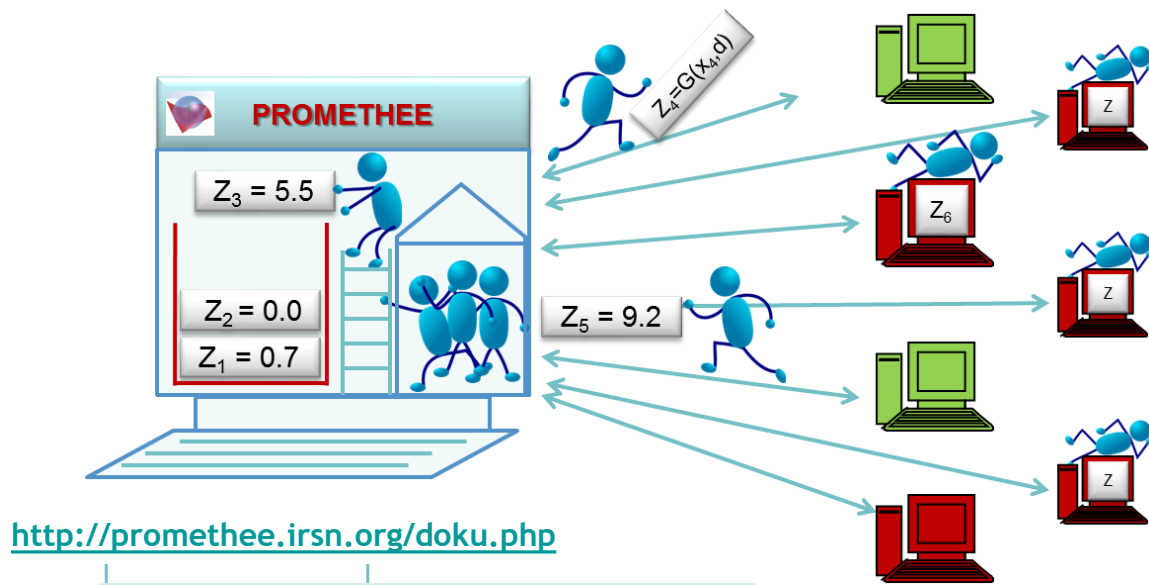
IRSN  
computational  
environment  
PROMETHEE

- A : definition of the problem
- B : definition of the input affected by uncertainty
- C : uncertainty propagation
- D : sensitivity analysis ranking



### ■ The key role of Promethee in performing UA and GSA

- Promethee environment coupled to different numerical models
  - *Allows the parameterization of any numerical code to carry out a huge number of simulations*
  - *Graphical user interface*
  - *Takes advantage of [R] algorithms to perform uncertainties propagation, sensitivity analysis, ...*
  - *Deploys computational resources (e.g. work stations, servers, clusters)*



### Steps C : Monte-Carlo sampling for UA

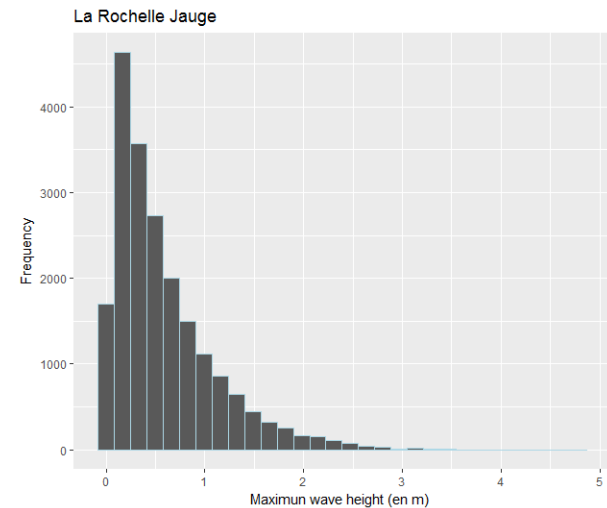
Sample of size N-inputs



Hydrodynamic numerical model



N-outputs



□ Law of response : statistic estimation

Mean

$$E[Y] = \mu_Y = \frac{1}{N} \sum_{i=1}^N G(x^i)$$

Variance

$$Var(Y) = \frac{1}{N-1} \sum_{i=1}^N [G(x^i) - \mu_Y]^2$$

St. deviation

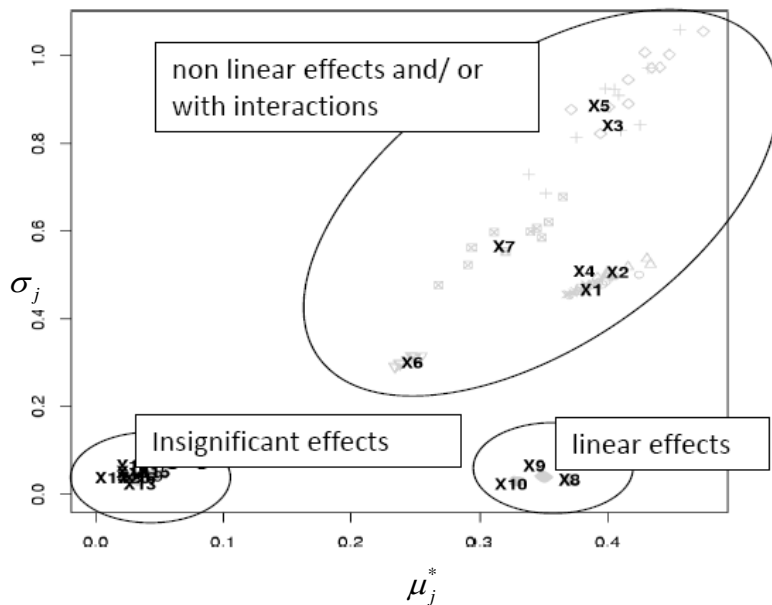
$$\sigma_Y = \sqrt{Var(Y)}$$

Convergence speed

$$o\left(\frac{1}{\sqrt{N}}\right)$$

### Step D: sensitivity analysis

- D.1) Morris screening-method (One-at-a-time) - Morris, 1991



$\mu_j^*$  is a measure of influence of the  $j$ -th input on the output  
 $\sigma_j$  is a measure of non-linear and/or interaction effects of the  $j$ -th input

- D.2) Sobol' index computation

$$S_i = \frac{D_i(Y)}{\text{Var}(Y)}, \quad S_{ij} = \frac{D_{ij}(Y)}{\text{Var}(Y)}, \quad \dots$$

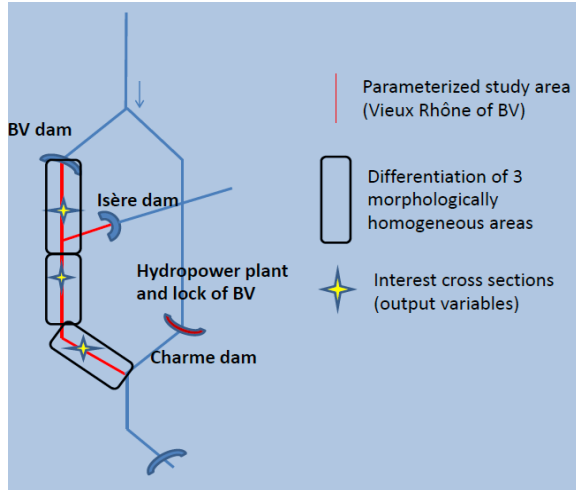
$$S_{Ti} = S_i + \sum_{i < j} S_{ij} + \sum_{j \neq i, k \neq i, j < k} S_{ijk} + \dots = \sum_{l \in \#i} S_l$$

- Results of ANOVA (ANalysis Of VAriance) decomposition
- Quantify the contribution of each input parameter on the output variance
- Independent input parameters

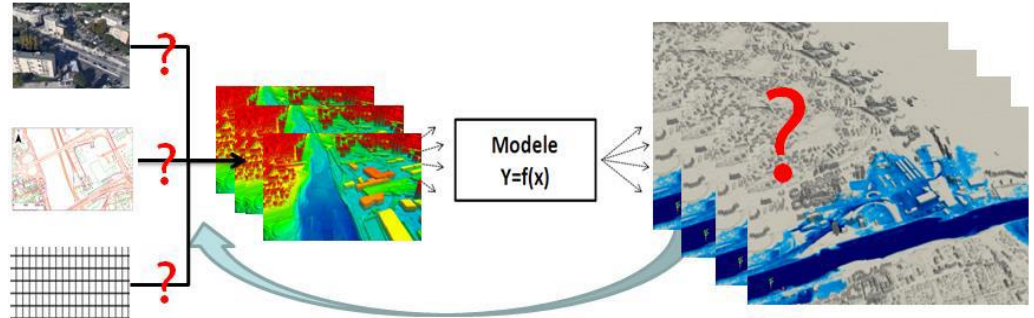


# III. Preliminary studies applied to hydrodynamic models

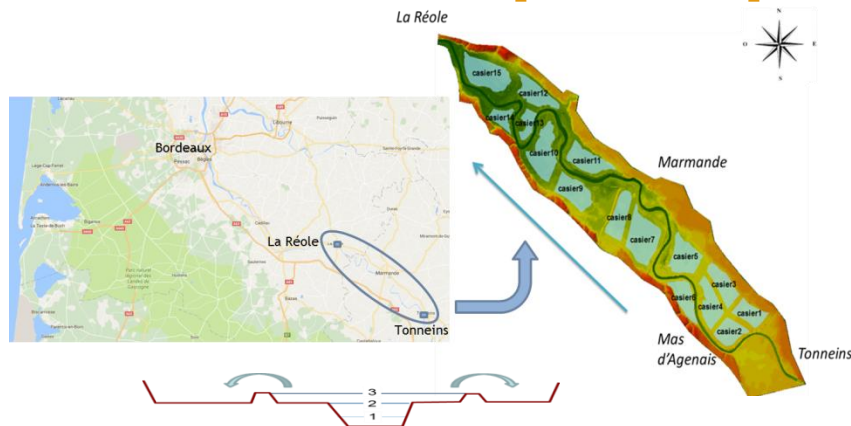
## 1D hydraulic model of the Rhône river [2011-2014]



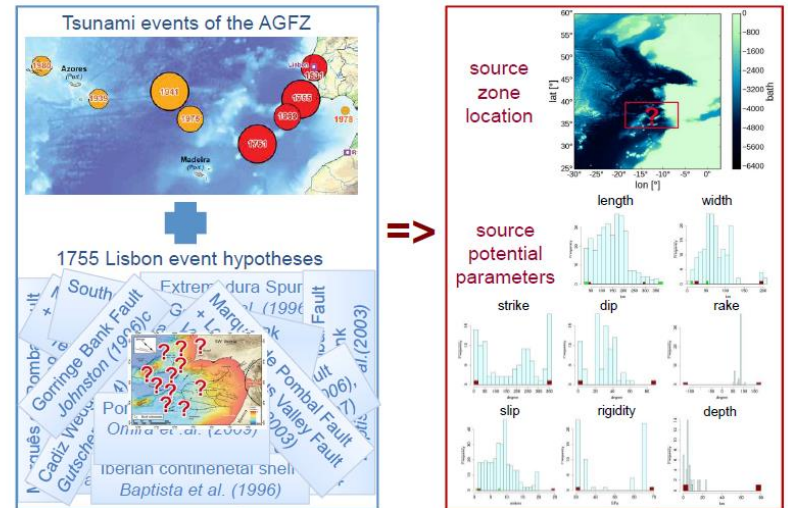
## Topography with a 2D model [2013-2016]



## Flooding and levee breaches study on La Garonne river [2015-2019]



## 2D Tsunamigenic potential of the AGFZ [2014-2017]



## Conclusions of preliminary studies

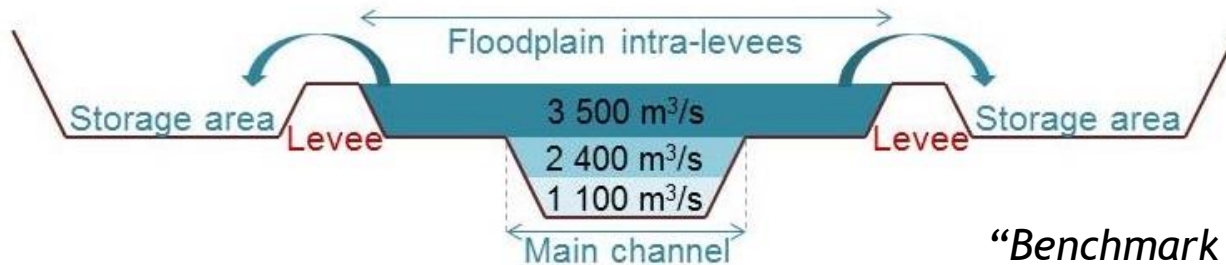
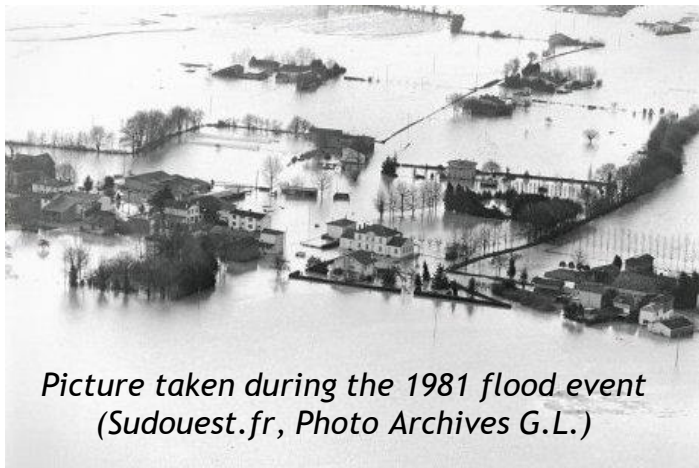
#### Interest for flood hazard assessment:

- ❑ In the context of nuclear safety UA and GSA allow to *identify the influencing parameters in a rigorous way*
- ❑ Identify *some rare combinations* of critical flooding situation that would have not been identified with an expert opinion
- ❑ Can be a *complementary approach* to the current state of practices concerning uncertainties on flooding hazard assessment

#### Main challenges:

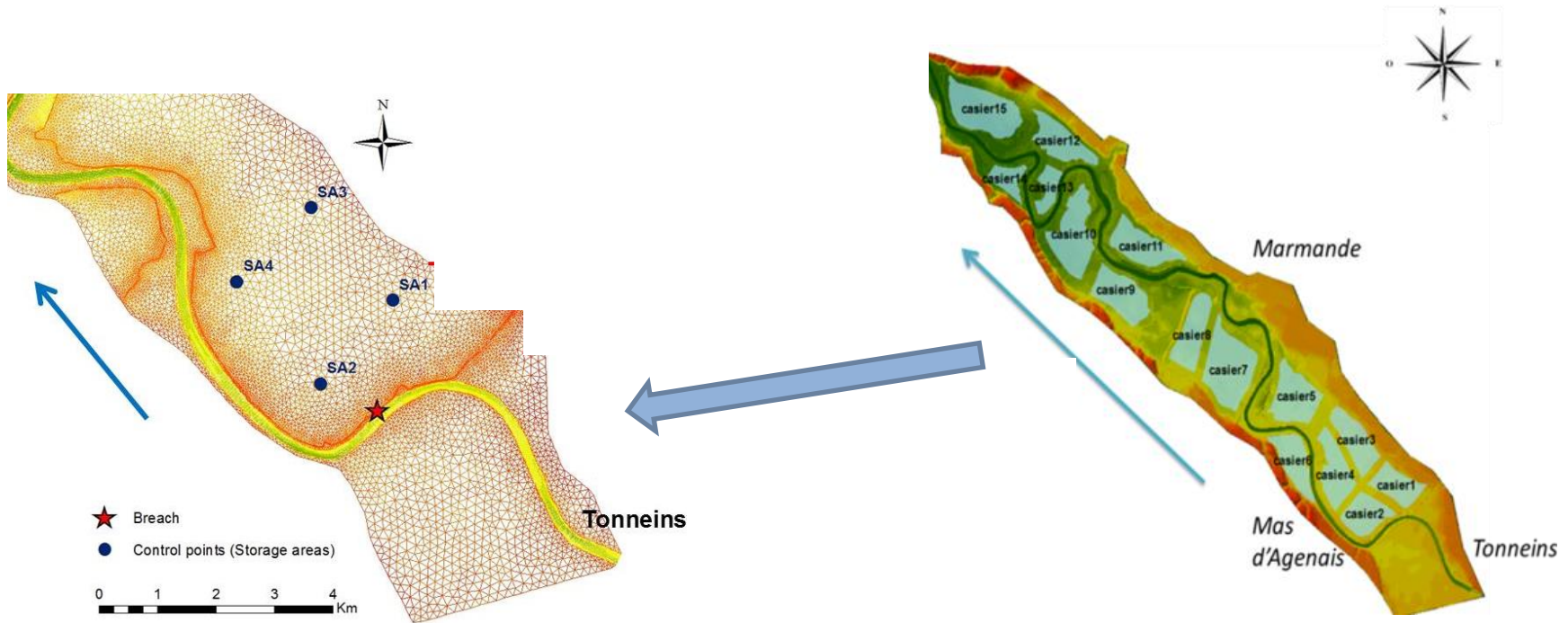
- ❑ Time consuming calculations (interest of meta-model approaches...)
- ❑ Dealing with *dependent input parameters*

# Case study on La Garonne river



“Benchmark Garonne” project by EDF

## TELEMAC 2D model



### TELEMAC 2D:

- 82,116 cells with different length varying from 10 to 300 m
- Upstream boundary condition: triangular hydrograph with a flow peak of  $3,081 \text{ m}^3/\text{s}$
- The peak discharge is achieved after 18 hours and the simulation ends after 5 days



### Levee breaches study

■ **TELEMAC** breaching process :  
when the water level above the  
dyke reaches a given value “ $H_w$ ”

■ Uncertain parameters :

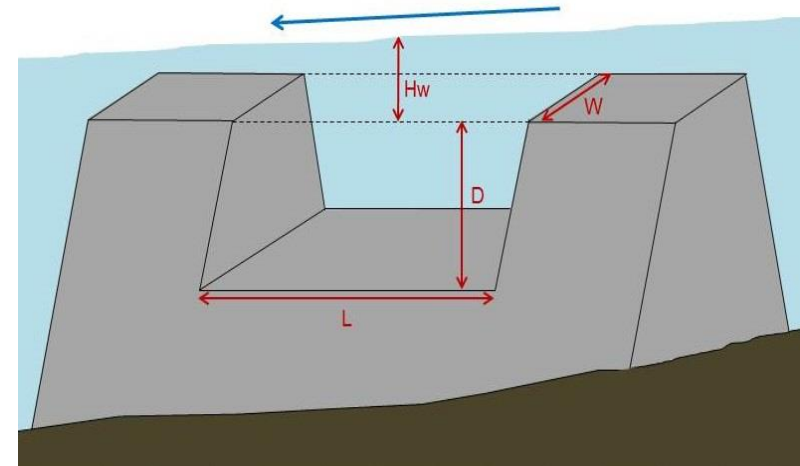
□ Overflow  $H_w$  : from 50 cm below  
levee crest to 10 cm above

+ 2 geometrical parameters :

□ Depth  $D$  : from 0 to 100% of the  
levee height

□ Length  $L$  : between 40 and 200 m

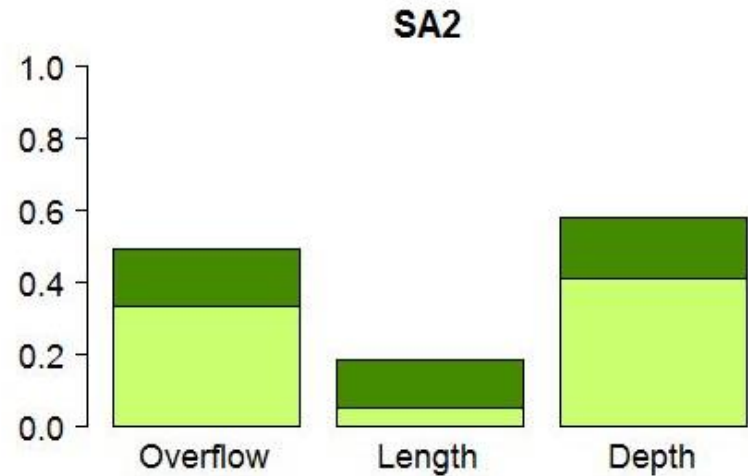
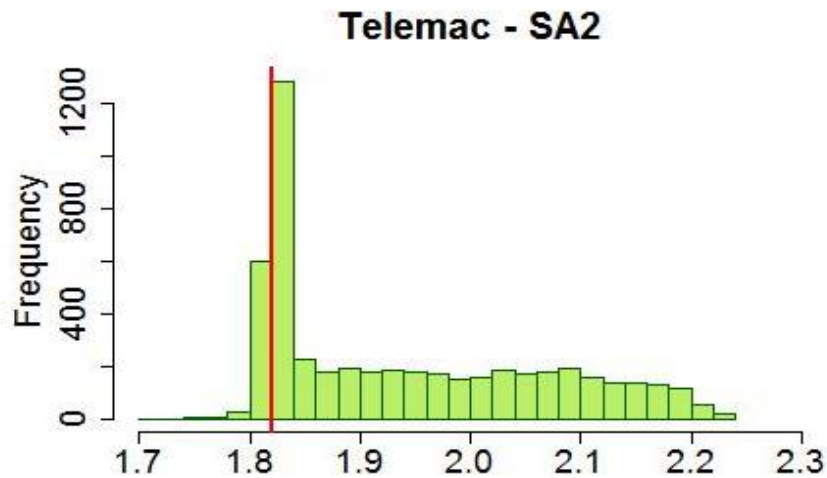
■ 200 simulations performed => raised to 5,000 with kriging meta-model  
(validated as a good emulator for reproducing the TELEMAC-2D code  
behavior)



*Levee breach diagram.*

*The parameters are the length ( $L$ ),  
the depth ( $D$ ), the width ( $W$ ) and  
the water level above the crest,  
that means the overflow ( $H_w$ ).*

## Uncertainty propagation and GSA



*Frequency distributions of the maximum water levels in four storage areas*

*SA Sobol' indices for the 3 uncertain parameters*

- ⇒ Large variation of water height compared to the simulation without breach (red lines), influence of Depth...
- ⇒ No dependency taken into account between Overflow, Length nor Depth
- ⇒ See *SimHydro 2019, Pheulpin & al - Comparison between uncertainty propagations and sensitivity analyses from two hydraulic models (1D and 2D) of the Garonne River: Application to levee breach parameters*

Dependant inputs taken into account in a simplified case :  
1D equations of Saint-Venant, with uniform and constant flowrate and large rectangular sections

## Step B: Uncertainty sources quantification

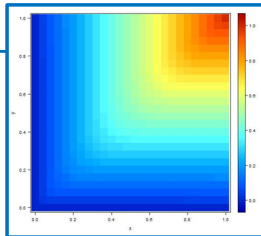
**For all parameters, definition of:**

- Parameter bounds
- Parameter distribution laws

**For dependent parameters:**

- Groups of parameters identification
- Copula selection (e.g. normal copula) adapted to each group of parameter and definition of the correlation coefficients ( $r$ )
- Construction of multivariate distributions

*Example of a normal copula cumulative distribution function*



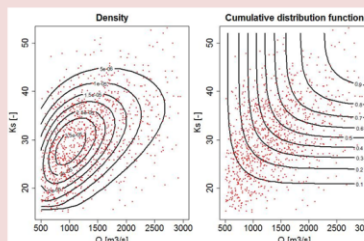
## Uncertainty sources quantification

Inputs	Symbols	Units	PDF
Maximal annual flow rate	Q	m <sup>3</sup> /s	Truncated Gumbel
Strickler coefficient	K <sub>s</sub>	-	Truncated Normal
River downstream level	Z <sub>v</sub>	m	Triangle
River upstream level	Z <sub>m</sub>	m	Triangle
Levee height	H <sub>d</sub>	m	Uniform
Bank level	C <sub>b</sub>	m	Triangle
Length of the river stretch	L	m	Triangle
River width	B	m	Triangle

## UQ for independent and dependent parameters

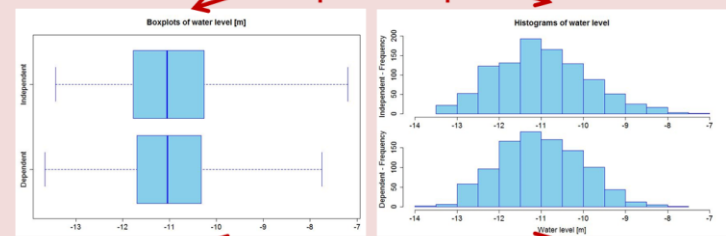
Dependent inputs → 3 normal copulas: Q/K<sub>s</sub> ( $r = 0.5$ ) ; Z<sub>v</sub>/Z<sub>m</sub> ( $r = 0.3$ ) ; L/B ( $r = 0.3$ )

Normal copula Q/K<sub>s</sub>



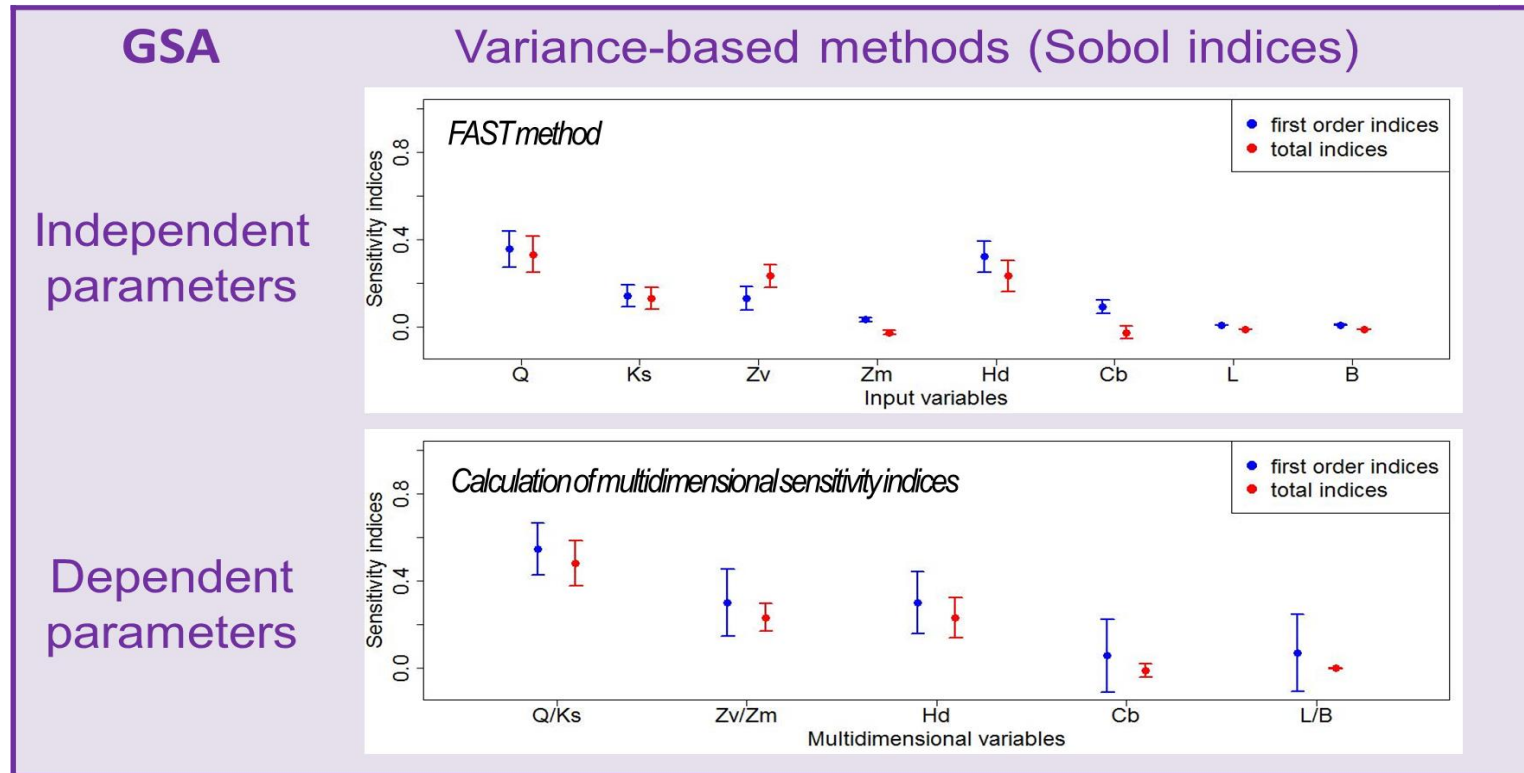
Outputs Distribution

Independent inputs



Dependent inputs

## Simplified case: global sensitivity analysis



- ❑ In this example, the choice of the copula has very few impact on the outputs
- ❑ Some parameters (e.g.  $Z_m$ ) can have more influence once included in a group than considered independent

⇒ More information : see *IRSN EGU 2019 poster (Pheulpin & al)*



## Application to a real case study (perspective)

### Step A: Problem specification

#### Input parameters:

- **Fixed:** Time step, grid resolution, *etc.*
- **Uncertain:**
  - Hydraulic parameters: hydrograph parameters, Strickler coefficient, *etc.*
  - Breach parameters: length, depth, time formation, *etc.*



**Independent parameters or not?**

#### Variables of interest

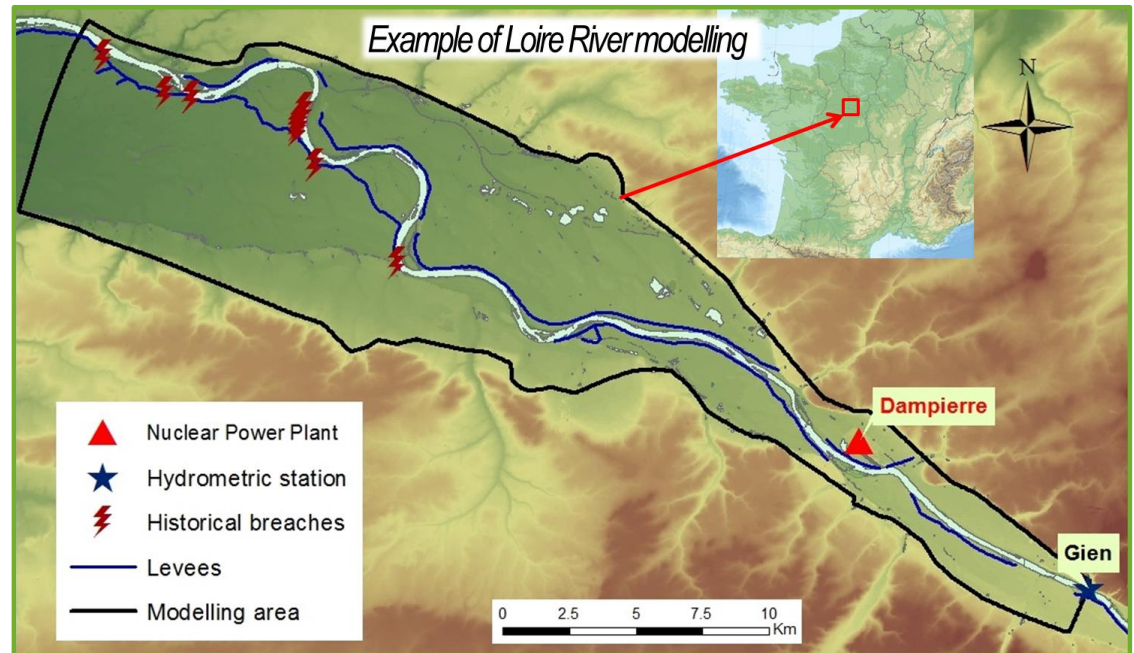
- Water levels at certain location in the flood plain (e.g. near the breaches)

#### Quantities of interest

- Probability, variance, *etc.*

#### Hydraulic and levee breach modelling: Example for the Loire River

- 50 km-long reach modelling, between Gien and Orléans
- 2D modelling with Telemac-2D
- Numerous levees along this reach with known historical breaches



- Conclusion of recent and on going studies on riverine flood modelling
  - Uncertainty quantification related to levee behavior during an inundation event can be a very difficult (but essential) task
  - Additional uncertainty associated to the chosen numerical model representing the breach process (1D vs 2D...)
  - Theoretical framework available to take into account dependencies, data needed to characterize dependencies
  - Interest of meta-models and inversion approach to control calculation time

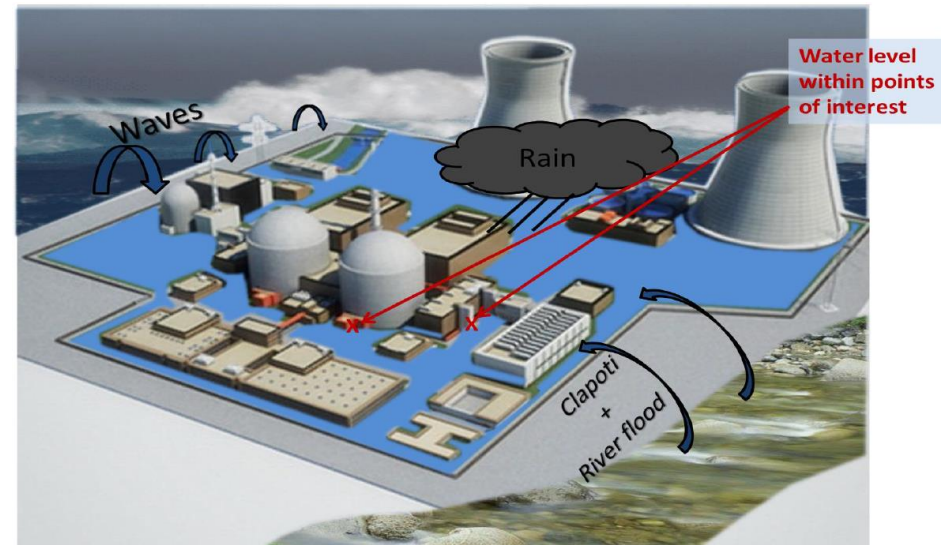
# Probabilistic Flood Hazard Assessment (perspectives...)

## Riverine flood

- objective of including a probabilistic assessment through uncertain input parameters (e.g. peak flow rate distribution and duration of flood...)
  - propagate uncertainties or use inversions methods to define the probability of some outputs safety criteria
- ⇒ See Bacchi & al, CMWR conference in June 2018

## Combining hazards

- on going PhD
- ⇒ see Ben Daoued & al “Modeling coincidence and dependence of flood hazard phenomena in a Probabilistic Flood Hazard Assessment (PFHA) » (under revision)



# Thank you for your attention

