

GENE 771-81-1194, Revision 2, "Commonwealth Edison Company Dresden Nuclear Power Plant Units 2 & 3, Shroud and Shroud Repair Hardware Analysis, Volume I, Shroud Repair Hardware".

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GE Nuclear Energy

**COMMONWEALTH EDISON COMPANY  
DRESDEN NUCLEAR POWER PLANT  
UNITS 2 AND 3  
SHROUD AND SHROUD REPAIR HARDWARE ANALYSIS  
VOLUME I:  
SHROUD REPAIR HARDWARE**

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## ABSTRACT

Volumes I and II of this document provide the results of the stress analysis of the Dresden 2 and 3 and Shroud Repair Hardware, demonstrating that structural integrity is maintained when subjected to the loading and limits specified in Design Specification 25A5688.

## EXECUTIVE SUMMARY

The volumes I and II of this report provides the results of the stress analysis of the Dresden Units shroud and shroud repair hardware when subjected to all applied loadings including seismic, pressure, deadweight, and thermal effects.

The shroud restraint hardware consists of four identical sets of tie rod and spring assemblies. The four sets are spaced 90° apart, beginning at 20° from vessel zero. Each set consists of the following major elements:

1. An Upper Spring, located in the reactor pressure vessel (RPV)/shroud annulus at the top guide elevation. This spring provides lateral seismic support to the shroud at the top guide elevation and transmits seismic loads from the nuclear core directly to the RPV.
2. An Upper Support Assembly, located in the annulus from the top guide elevation to the top of the shroud. This assembly provides a connection for the tie rod to the shroud top.
3. A Middle Spring, located in the annulus at the elevation of the jet pump support brackets. This spring provides lateral seismic support to the shroud, keeps the shroud from coming in contact with the jet pump support brackets during a seismic event, and restrains the tie rod movement for proper tie rod vibration characteristics.
4. A Lower Spring, located in the annulus at the core plate and shroud support region. This spring provides lateral seismic support to the shroud, transmitting core seismic loads to the RPV. In addition, this spring provides a connection for the tie rod to the shroud support plate.
5. The Tie Rod, which connects to the upper end of the top of the shroud and to the lower end of the lower spring. This component develops a thermal preload due to normal operating temperature, which in turn provides vertical clamping forces to the shroud.

The upper, middle and lower springs are optimized to transfer the lateral operational, hydrodynamic and seismic loads while meeting the stress limits.

The stress analysis of the overall core shroud was performed with the ANSYS code [Reference 1]. A three-dimensional finite element model was constructed which included the shroud from the upper flange at the shroud head joint down to the connections at the RPV. Because of the symmetrical behavior of the shroud under the applied loads, a 180° circumferential segment was modeled.

The stress analysis of the major shroud repair hardware components was performed with the COSMOSM [Reference 10] and ANSYS codes. For the smaller components, hand calculations were performed.

The load combinations and structural acceptance criteria are contained in the Design Specification [Reference 2]. The results of the stress analysis demonstrate that the shroud and shroud repair hardware meet the requirements of that specification.

The Volume I of this report is describing the analysis of the shroud repair hardware.

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## 1.0 INTRODUCTION

Intergranular stress corrosion cracking (IGSCC) has been found in the core shroud welded joints of several Boiling Water Reactors. Similar cracking may also exist in the welded joints of the Dresden Units 2 and 3 Core Shroud. GENE has designed a shroud repair system that reinforces the shroud in the event that any or all of the seven shroud horizontal weld joints are cracked. The stress analysis discussed in this report demonstrates that the shroud and the shroud repair system structural integrity is maintained if any or all of these seven welded joints are cracked completely through their thickness and around their entire 360° circumference. The structural integrity of the shroud and shroud repair system is also demonstrated in the event that the shroud is uncracked and the repair system is installed.

The Volume I of this report is describing the analysis of the shroud repair hardware.

## 2.0 SHROUD REPAIR SYSTEM DESIGN FEATURES

The shroud repair system consists of four identical sets of tie rod and spring assemblies. The four sets are spaced at 90° intervals beginning at 20° from vessel zero. A layout of one of the tie rod and spring sets is shown in Figure 2.1.

The tie rods are thermally preloaded to provide vertical compressive clamping forces on the shroud. The magnitude of the tie rod thermal preload is greater than the net uplift forces on the shroud due to normal operating pressures and postulated Loss of Coolant Accident (LOCA) recirculation line break pressures, so that no vertical separation of shroud sections would occur in those cases if the welded joints are postulated to be completely cracked. This is not the case for postulated LOCA main steam line break uplift pressures, which are sufficient to overcome the tie rod preload and momentarily separate shroud sections.

The upper, middle, and lower springs provide a lateral seismic load path from the top guide and core plate to the RPV. The magnitude of the seismic loads in these springs is a function of their stiffness. The stiffness has been optimized to minimize the seismic loads while still meeting the stress and displacements limits. The U-shaped upper springs consists of tapered legs that flex towards each other under lateral seismic loads. The taper in these legs has been optimized to produce constant stress along their length while providing the required stiffness. For the middle spring, the flexibility of the taper beam section provides the needed lateral stiffness to keep the middle section of the shroud from coming in contact with the jet pump support brackets during a seismic event. This keeps the shroud from moving closer than 1/2-inch to the jet pump support bracket. The rigid middle section of the middle spring also provides an intermediate lateral support to the tie rod. The natural vibration frequency of the tie rod with this intermediate support is then well removed from the flow-induced forcing frequency (flow induced vibration is discussed in detail in Section 6.6). For the lower spring, the flexibility of the Y-shaped feature at the top provides the lateral stiffness property, whereas the flexibility of the straight middle section provides the axial stiffness property, which in combination with the stiffness of the tie rod and upper axial component determines the tie rod thermal preload.

The shroud geometry and location and designation of the seven shroud horizontal weld joints are shown in Figure 2.2.

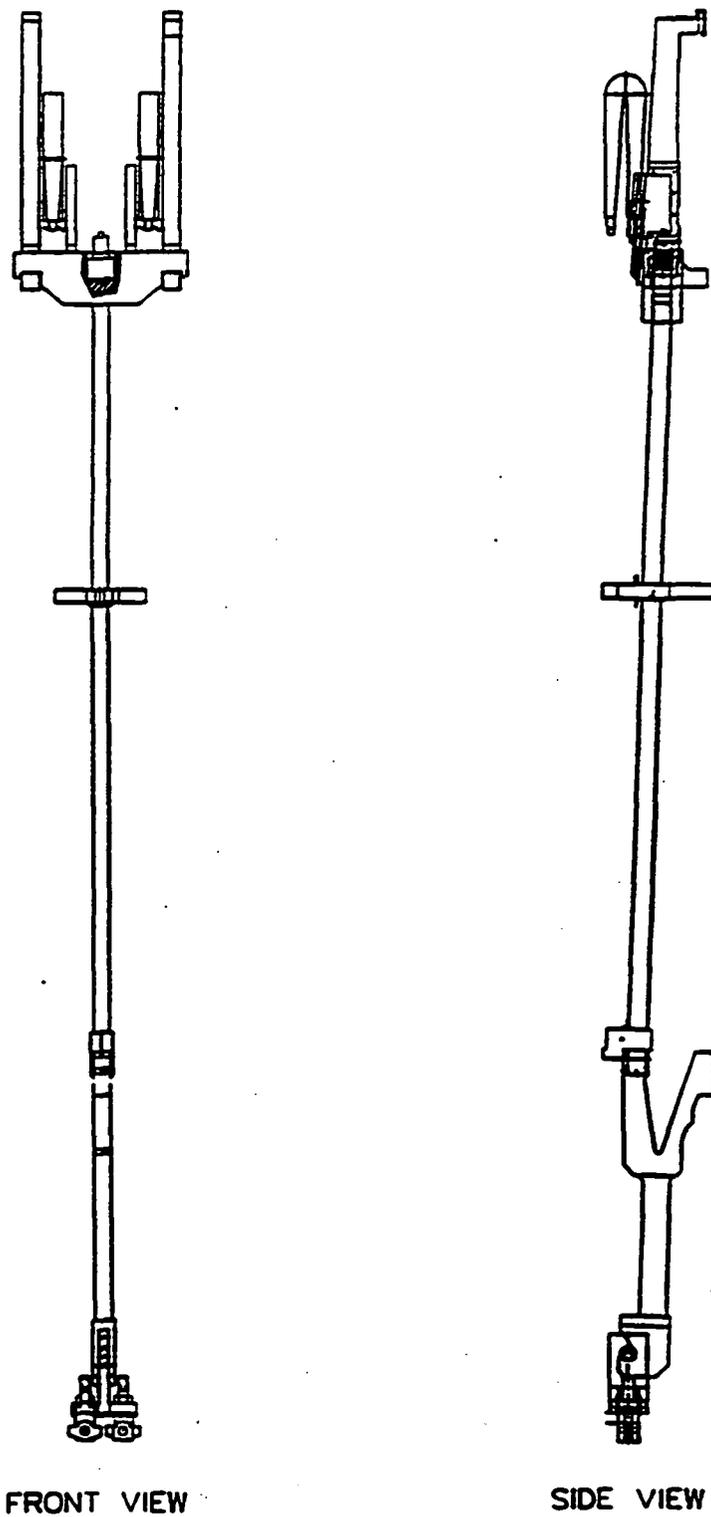


Figure 2.1 Shroud Repair Hardware Layout

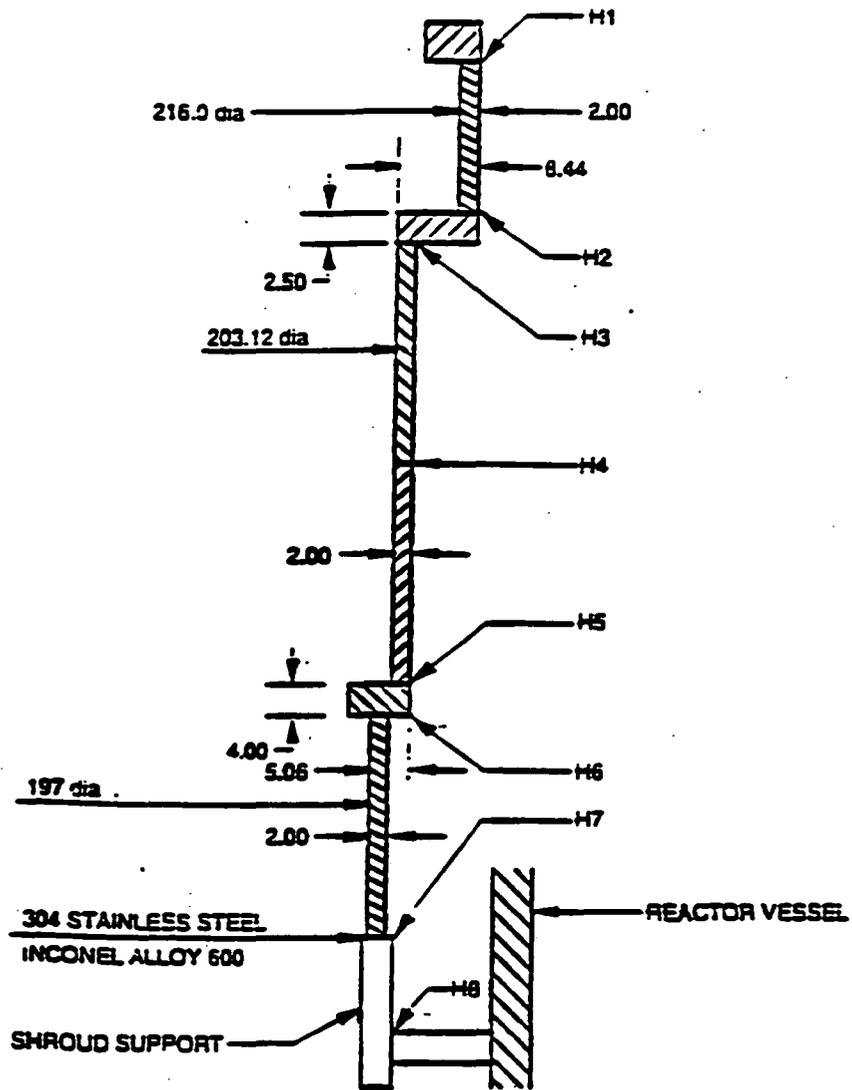


Figure 2.2 Shroud Horizontal Weld Designations

### 3.0 MATERIAL PROPERTIES

The following material properties for the primary load bearing restraint components are taken from Appendix I of the ASME B&PV Code [Reference 3] and GENE Testing Report [Reference 4]. A 575° F temperature applies to the Normal and Upset condition, and a 550° F temperature applies to the Faulted and Emergency conditions.

#### 3.1 Tie Rod (Drawing 112D6672)

XM-19	at 550° F (oper.)	
Young's Modulus	25.6 E6 psi	Table I-6 [Reference 3]
Thermal Expansion Coefficient	8.98 E-6 in/in/°F	Table I-5 [Reference 3]
S <sub>m</sub>	29450 psi	Table I-1.2 [Reference 3]
Sp.l.*	31440 psi**	Table I-2.2 [Reference 3]

\* The proportional limit for the material is 80 percent of the yield strength.

\*\* This is the Sp.l. at 300°F.

#### 3.2 Spring and Upper Assemblies (Upper Spring Drawing 112D6670; Long Upper Support Drawing 112D6669, Bracket Drawing 112D6675; Middle Spring Drawing 112D6681; and Lower Spring Drawing 112D6671)

X-750	at 550° F (oper.)	
Young's Modulus	28.4 E6 psi	[Reference 4]
Thermal Expansion Coefficient	7.5 E-6 in/in/°F	[Reference 4]
S <sub>m</sub>	47500 psi	[Reference 4]
Sp.l.	74560 psi	[Reference 4]

#### 4.0 LOADS AND LOAD COMBINATIONS

The Design Specification [Reference 2] specifies that the shroud and shroud repair hardware shall be analyzed for the following load combinations:

Normal / Upset	$\Delta P_N + DW + OBE$
Emergency 1	$\Delta P_N + DW + DBE$
Emergency 2	$\Delta P_{MS-LOCA} + DW$
Emergency 3	$\Delta P_{RC-LOCA} + DW$
Faulted 1	$\Delta P_{MS-LOCA} + DW + DBE$
Faulted 2	$\Delta P_{RC-LOCA} + DW + DBE$

where:  $\Delta P_N$  = Normal Pressure Difference  
 DW = Dead Weight Loads  
 OBE = Operating Basis Earthquake  
 DBE = Design Basis Earthquake  
 $\Delta P_{MS-LOCA}$  = Main Steam Line LOCA  
 $\Delta P_{RC-LOCA}$  = Recirculation Line LOCA  
 LOCA = Loss of Coolant Accident

The OBE and DBE loads are reported in Reference [5]. Since the configuration of the seismic model depends on the assumed behavior at weld joints postulated to be cracked, and the resulting seismic loads depend on this assumed behavior, two sets of DBE seismic loads were established. One set corresponds to the configuration for normal pressure differences and was used in the Emergency 1 load combination. The second set of seismic loads corresponds to the configuration for Main Steam Line LOCA pressure differences and was used in the Faulted 1 load combination. The configuration of the seismic model for the recirculation line outlet LOCA corresponds to that for normal pressure differences, and hence the seismic loads to be combined in the Faulted 2 load combination for the recirculation line outlet LOCA are from the first set of loads.

After reviewing the FSAR, it was determined that the shroud loads due to a feedwater line break are bounded by the main steam line break loadings and the recirculation line break loadings.

The appropriate deadweight loads were used in this stress analysis. The effect of the vertical seismic accelerations on the deadweight were also included.

The pressure difference loads are taken from the Design Specification [Reference 2].

### 5.0 STRUCTURAL ACCEPTANCE CRITERIA

The Design Specification specifies the following stress intensity limits in the Repair Hardware.

	Resulting Stress			$S_{allow}$
Upset	Primary Membrane	$P_m$	<	$1.00 \times S_m$
	Primary Membrane + Primary Bending	$P_m + P_b$	<	$1.50 \times S_m$
Emergency	Primary Membrane	$P_m$	<	$1.50 \times S_m$
	Primary Membrane + Primary Bending	$P_m + P_b$	<	$2.25 \times S_m$
Faulted	Primary Membrane	$P_m$	<	$2.00 \times S_m$
	Primary Membrane + Primary Bending	$P_m + P_b$	<	$3.00 \times S_m$
Thermal Upset	Primary Membrane + Primary Bending + Secondary	$P_m + P_b + Q$	<	$3.00 \times S_m$

For the evaluation of lower springs under Alternate Normal / Upset and Thermal Upset conditions, the maximum stress will be limited to the proportionality limit, i.e.,  $0.8 \times S_y$ .

## 6.0 SHROUD REPAIR HARDWARE STRESS ANALYSIS

The shroud repair hardware, i.e. the tie rod system, provides axial stiffness and thermally-induced shroud hold-down forces. The tie rod system, as shown in Figure 2.1, includes: the Tie Rod (Drawing 112D6640), the Upper Spring and Support Assembly (Drawing 112D6641), the Middle Spring (Drawing 112D6680), and the Lower Spring and Support Assembly (Drawing 112D6638). The axial stiffness of these components act in series creating a total tie rod system stiffness of 650 kips/inch, which is very close to the value used in the seismic analysis (609 kips/inch). The upper supports and lower spring stiffness values were obtained from the finite element analyses discussed below.

Two steady state thermal conditions are identified in the design specification. The first is Normal operation with the shroud at 550<sup>o</sup> F and the tie rod system at 538<sup>o</sup> F. The second is an Upset condition with the shroud at 433<sup>o</sup> F and the tie rod system at 300 F. Using the 609 kips/inch system stiffness and the appropriate dimensional and thermal expansion values, the tie rod system axial thermal preloads corresponding to these two thermal conditions are given in Ref. [11].

The tie rod net load is a combination of the thermal preload, seismic load, and the effect of the net uplift force from the shroud head due to pressure difference and deadweight. For the upset and emergency load combinations, the thermal preload is not overcome and the shroud has a compressive clamping force. In this case, the tie rod load due to the shroud head net uplift force is proportional to the relative stiffness of the tie rod versus the shroud, as in a preloaded bolted joint. For the faulted combination with Main Steam Line LOCA and RRLB, the preload is overcome and the net shroud head uplift force is reacted by the tie rods. The maximum tie rod tensile loads for the load combinations were derived on that basis, and have the following magnitudes [Ref. 12]:

Case	F (kips)
Upset with OBE	194.00
Upset Thermal	170.00
Emergency	339.00
Faulted	339.00

Since the number of applied load cycles for upset/OBE, emergency and faulted cases is very small, no formal fatigue analysis is required for the shroud repair hardware. As for the Normal/Upset case, the resulting stress is relatively low, and the allowable number of fatigue cycles is high. Therefore only the tie rod fatigue is analyzed for thermal upset condition is addressed in Section 6.6.

## 6.1 Shroud Upper Stabilizer

### 6.1.1. INTRODUCTION

This section summarizes the results of the stress analysis performed on upper spring of the shroud repair hardware for Dresden 2 and 3 Project

This spring (stabilizer) is part of tie rod assembly and provides lateral seismic support to the shroud and transmits seismic loads from the nuclear core directly to the RPV. Normal/Upset, Emergency and Faulted conditions, that include seismic, pressure, gravity and thermal loads, were analyzed and the maximum stresses were shown to be within the allowable limits.

In addition the results of the analysis were used to compute stiffness constants for the Spring for use in global shroud model.

### 6.1.2. ANALYSIS METHOD

Detailed finite element model of the spring was constructed to evaluate the upper stabilizer's mechanical characteristics and stresses. The model was built in detail and analyzed for different loading conditions using the static analysis option of the COSMOS finite element code [Ref.10]. The code is developed by Structural Research and Analysis Corporation (SRAC) of Los Angeles California. It has been verified for use in the nuclear power industry per the requirements of 10CFR50 Appendix B and the applicable section of ANSI/ASME QA-1 and related supplements.

### 6.1.3. MATERIAL PROPERTIES AND APPLIED LOADS

The upper spring is made of INCONEL X-750 material, for which the material properties have been tabulated below.

TABLE 6.1-1: Upper Spring Material Properties

Property	Description	Value
$\rho$	Density	0.29 lb/in <sup>3</sup>
E	Modulus of Elasticity	28.4 x 10 <sup>6</sup> psi
$\mu$	Poisson's ratio	0.3

Applied loads for different conditions and corresponding allowable stresses are shown in the following table.

TABLE 6.1-2: Upper Spring Applied Side Loads and Allowable Stresses

Identification	Condition	Applied loads (lb.)	Allowable Stress Pm+Pb,(psi)
Upper Spring	Normal/Upset	33,500	70,500
	Emergency	67,000	106,875
	Faulted	70,000	142,500

#### 6.1.4. MODELING DETAILS

The shroud upper spring located in the reactor pressure vessel (RPV) shroud annulus at the top guide elevation. This spring provides lateral seismic support to the shroud at the top guide elevation and transmits seismic loads from the nuclear core directly to RPV. To evaluate the accurate linear spring constant and stress values, a finite element model was made with solid elements. Figure 6.1-1 depicts the meshing of this spring, applied loads and boundary condition. Figure 6.1-2 through 6 show the distribution of stress and displacements under Normal/Upset, Emergency and Faulted condition. The upper spring's linear spring constant extracted from the detailed model is used in the global model to represent the spring. To calculate the proper actual maximum stresses, the maximum stresses extracted from this model are prorated with actual loads extracted from the global model.

DRESDEN PROJECT UPPER SPRING

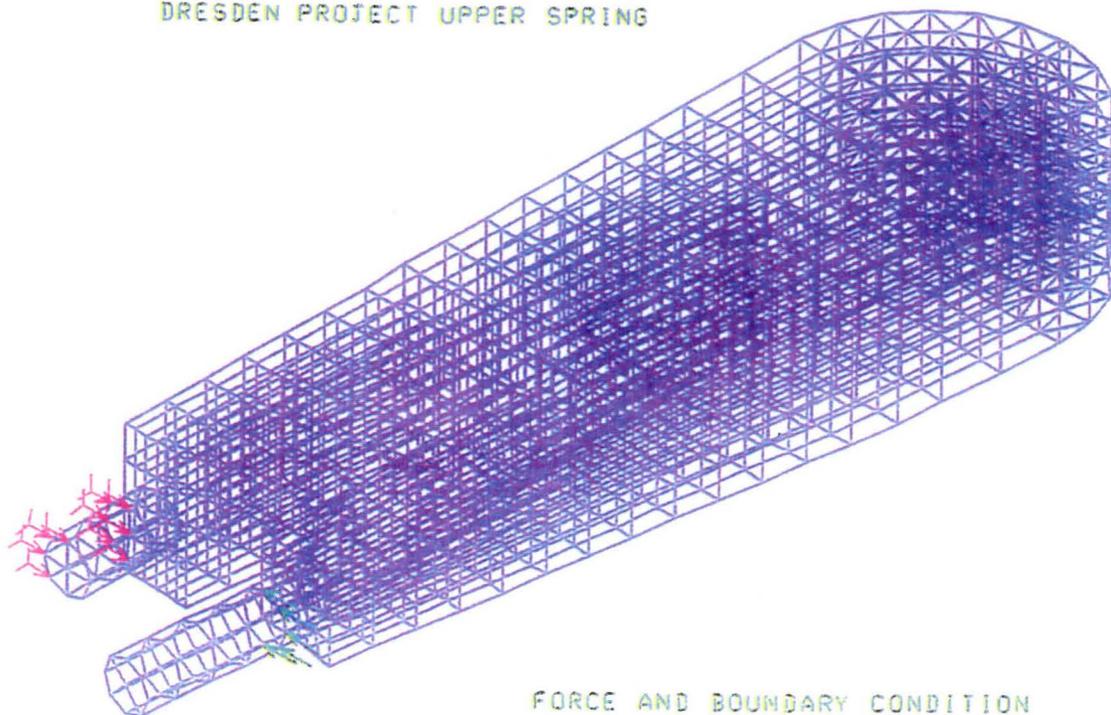


Figure 6.1-1 Upper Spring Finite Element Model

### 6.1.5. ANALYSIS RESULTS

The table below shows the shroud upper spring stresses during Normal/Upset and emergency and faulted events .

TABLE 6.1-3: Upper Spring Stress Summary

Condition	Applied loads (lbs)	Stress Intensity (psi)	Allowable stress (Pm+Pb)(psi)
Normal/Upset	33500	51,800	70,500
Emergency	67000	103,600	106,875
Faulted	70000	108,200	142,500

As is seen in Table 6.1-3 all stresses for the critical load combinations are below the corresponding allowables.

### 6.1.6. SPRING CONSTANT

Based on the results of the finite element model of the upper spring the following spring constants are calculated:

Horizontal spring constant: 62 kips/in

The local axial shroud flexibility is negligible in comparison to the upper spring flexibility. Therefore the effect of the shroud axial flexibility is neglected in the calculation of the above spring constant.

STRESS Lc=1

DRESDEN PROJECT UPPER SPRING FINITE ELEMENTS MODEL

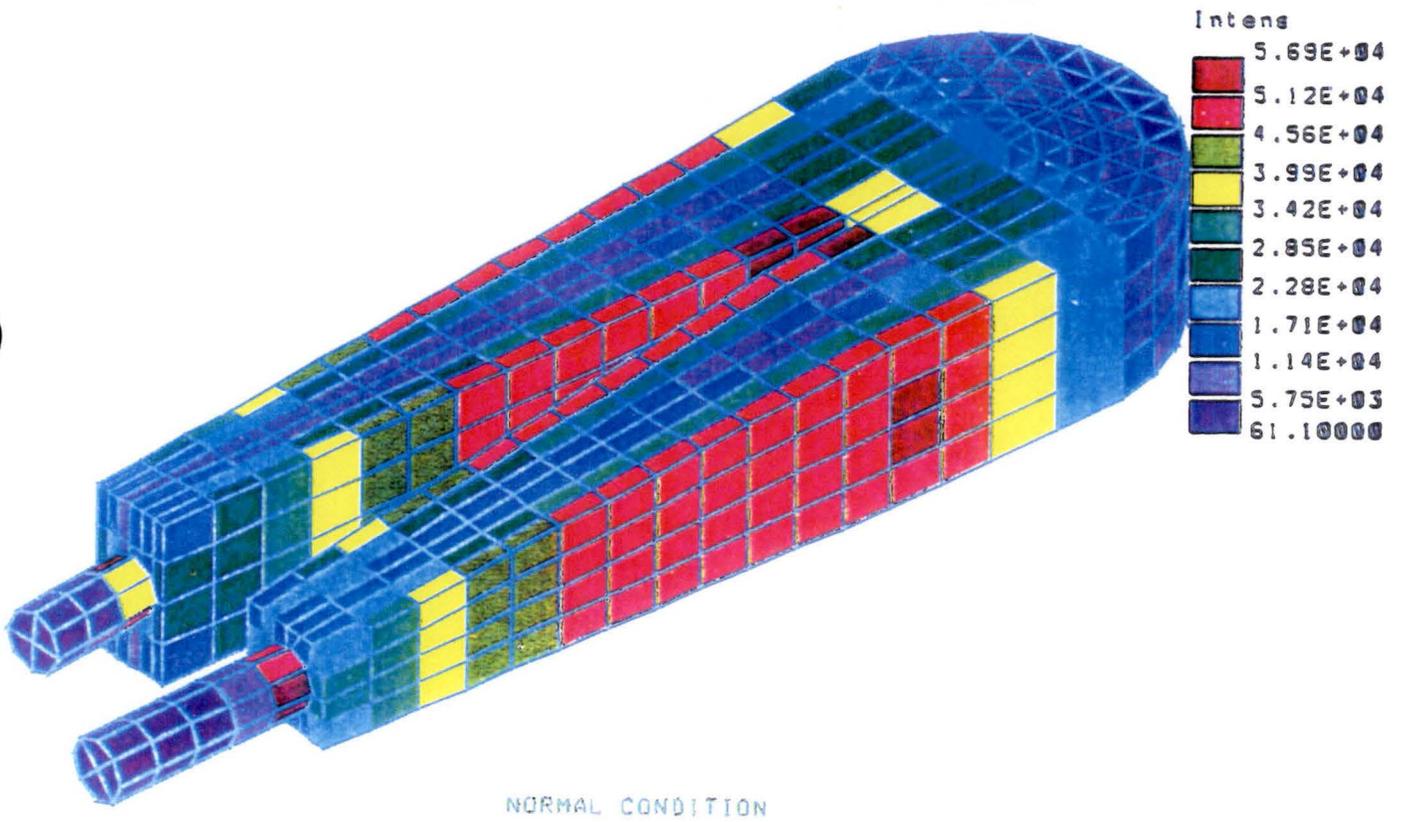
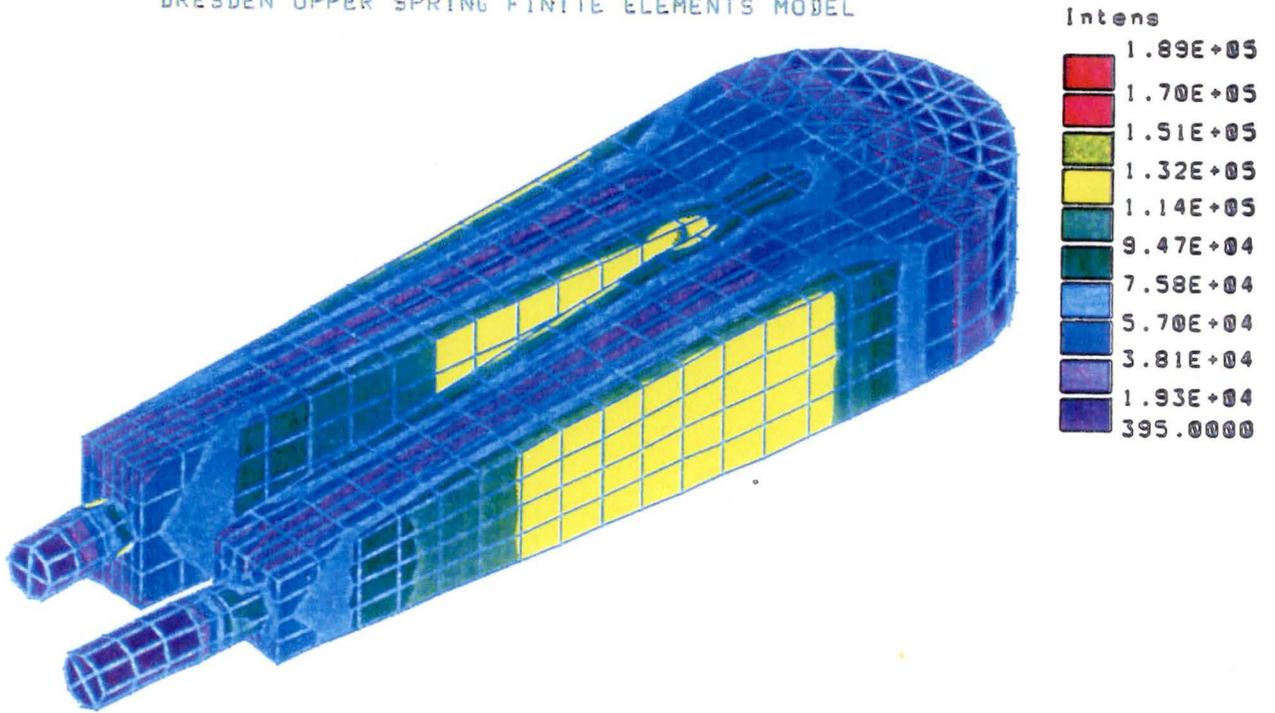


Figure 6.1-2 Upper Spring Normal/Upset Condition, Stress Intensity

STRESS Lc=1

DRESDEN UPPER SPRING FINITE ELEMENTS MODEL

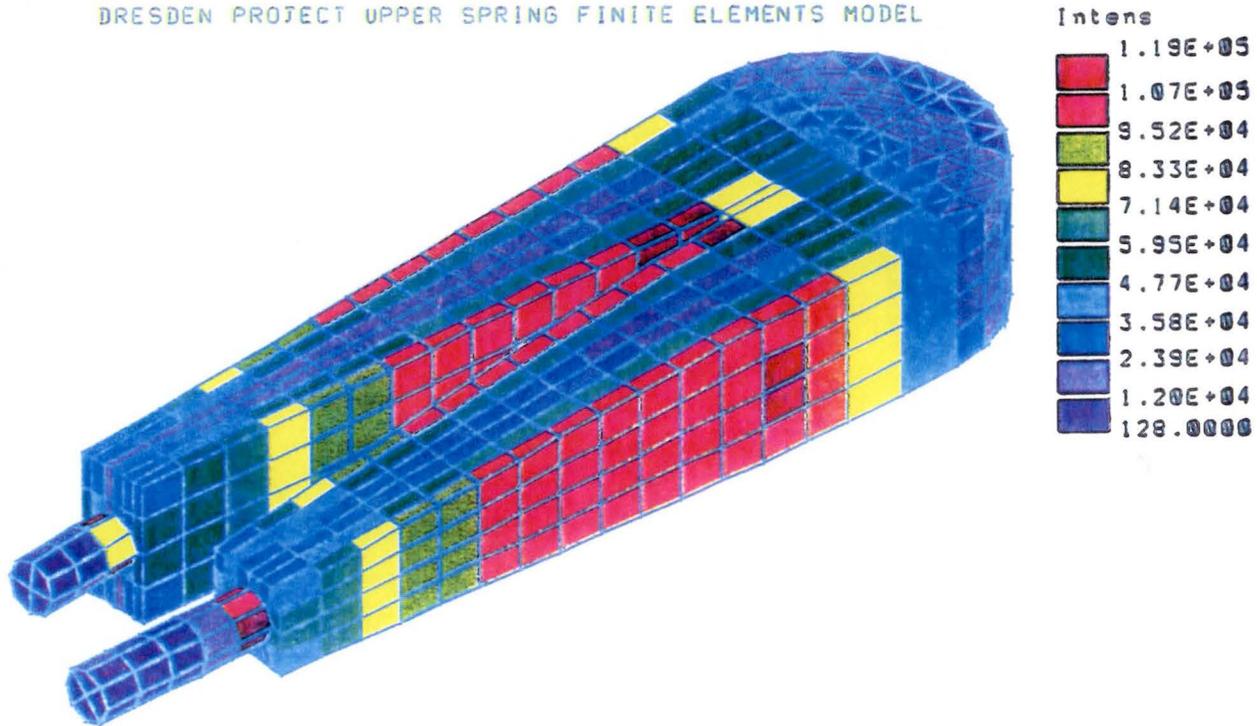


EMERGENCY CONDITION

Figure 6.1-3 Upper Spring Emergency Condition, Stress Intensity

STRESS Lc=1

DRESDEN PROJECT UPPER SPRING FINITE ELEMENTS MODEL



FAULTED CONDITION

Figure 6.1-4 Upper Spring Faulted Condition, Stress Intensity

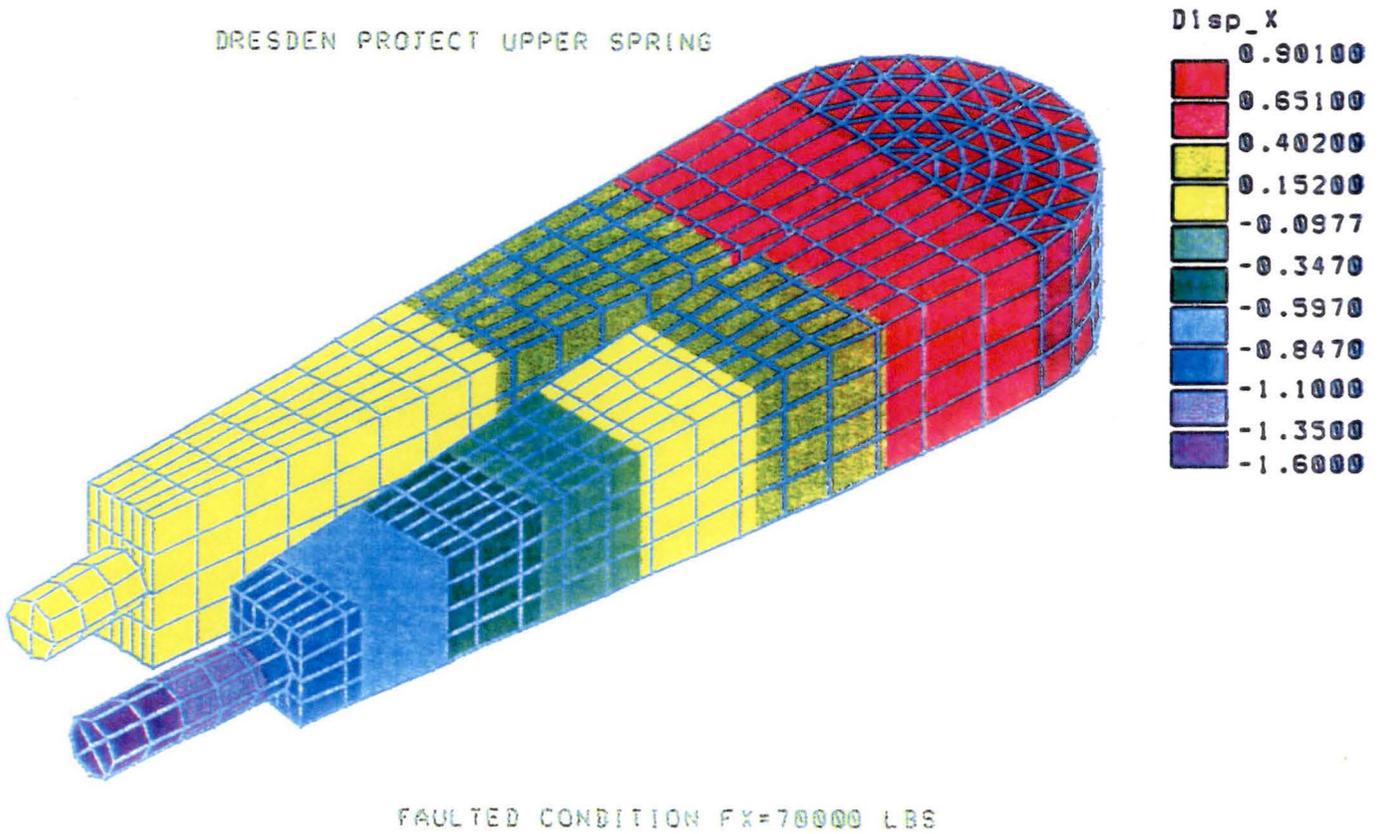
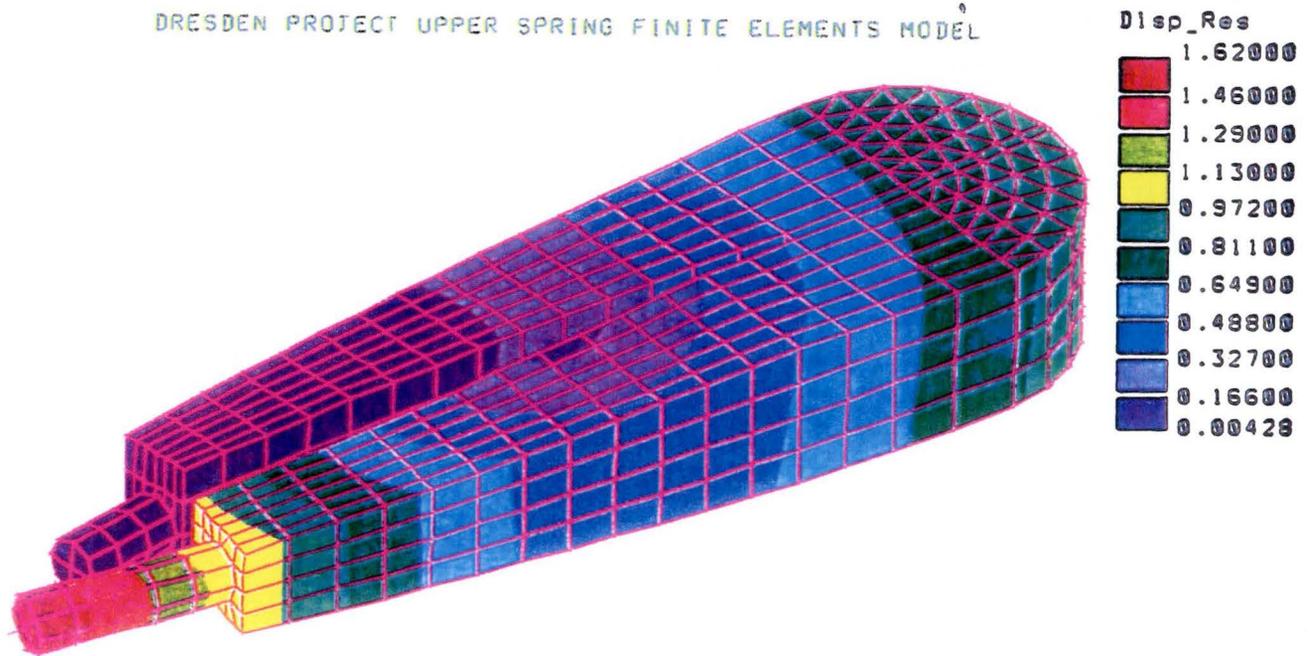


Figure 6.1-5 Upper Spring Faulted Condition, Displacement

DISP Lc=1

DRESDEN PROJECT UPPER SPRING FINITE ELEMENTS MODEL



FAULTED CONDITION DEFORMATION

Figure 6.1-6 Upper Spring Faulted Condition, Deformation (Magnified)

## 6.2 Shroud Lower Stabilizer

### 6.2.1. INTRODUCTION

This section summarizes the results of the stress analysis performed on lower spring of the shroud repair hardware for Dresden 2 and 3 Project. This spring (stabilizer) is part of tie rod assembly and provides lateral seismic support to the shroud and transmits seismic loads from the nuclear core directly to the RPV. Normal/Upset, Emergency and Faulted conditions, that include seismic, pressure, gravity and thermal loads, were analyzed and the maximum stresses were shown to be within the allowable limits.

In addition the results of the analysis were used to compute stiffness constants for the Spring for use in global shroud model.

### 6.2.2. ANALYSIS METHOD

Detailed finite element model of the spring was constructed to evaluate the lower stabilizer's mechanical characteristics and stresses. The model was built in detail and analyzed for different loading conditions using the static analysis option of the COSMOS finite element code. The code is developed by Structural Research and Analysis Corporation (SRAC) of Los Angeles California. It has been verified for use in the nuclear power industry per the requirements of 10CFR50 Appendix B and the applicable section of ANSI/ASME QA-1 and related supplements.

### 6.2.3. MATERIAL PROPERTIES AND APPLIED LOADS

The lower springs is made of INCONEL X-750 material, for which the material properties have been tabulated below.

TABLE 6.2-1: Lower Spring Material Properties

Property	Description	Value
$\rho$	Density	0.29 lb/in <sup>3</sup>
E	Modulus of Elasticity	28.4 x 10 <sup>6</sup> psi
$\mu$	Poisson's ratio	0.3

Applied loads for different conditions and corresponding allowable stresses are shown in the Table 6.2-2.

TABLE 6.2-2: Lower Spring Applied loads and Allowable Stresses

Identification	Condition (*)	Applied loads (lb.)	Allowable Stress (Pm+Pb)(psi)
LOWER SPRING	Normal / Upset:		
	Side Load	93,000	70,500
	Vert. Load	194,000	70,500
	Emerg.: Side Load	186,000	106,875
	Vert. Load	339,000	106,875
	Faulted: Side Load	190,000	142,500
	Vert. Load	339,000	142,500

\* Vertical and side loads do not act simultaneously on the same spring.

#### 6.2.4. MODELING DETAILS

The shroud lower spring, located in the annulus at the core plate and shroud support region. This spring consists of a diapason-like structure with the fork handle on a simple support, and provides lateral seismic loads to the RPV. In addition this spring provides a connection for the tie rod to the shroud support plate. To evaluate the accurate linear spring constant and stress values, a finite element model was made with solid elements. Figure 6.2-1 depicts the meshing of this spring, applied loads and boundary condition. The model is assumed hinged at the support locations and loads shown above were applied at the location of contact with shroud and tie rod. Figures 6.2-2 through 6.2-6 show distribution of stress and displacements under Normal/Upset, Emergency and Faulted condition. The lower spring's linear spring constant, extracted from the detailed model, is used in the global model to represent this spring.

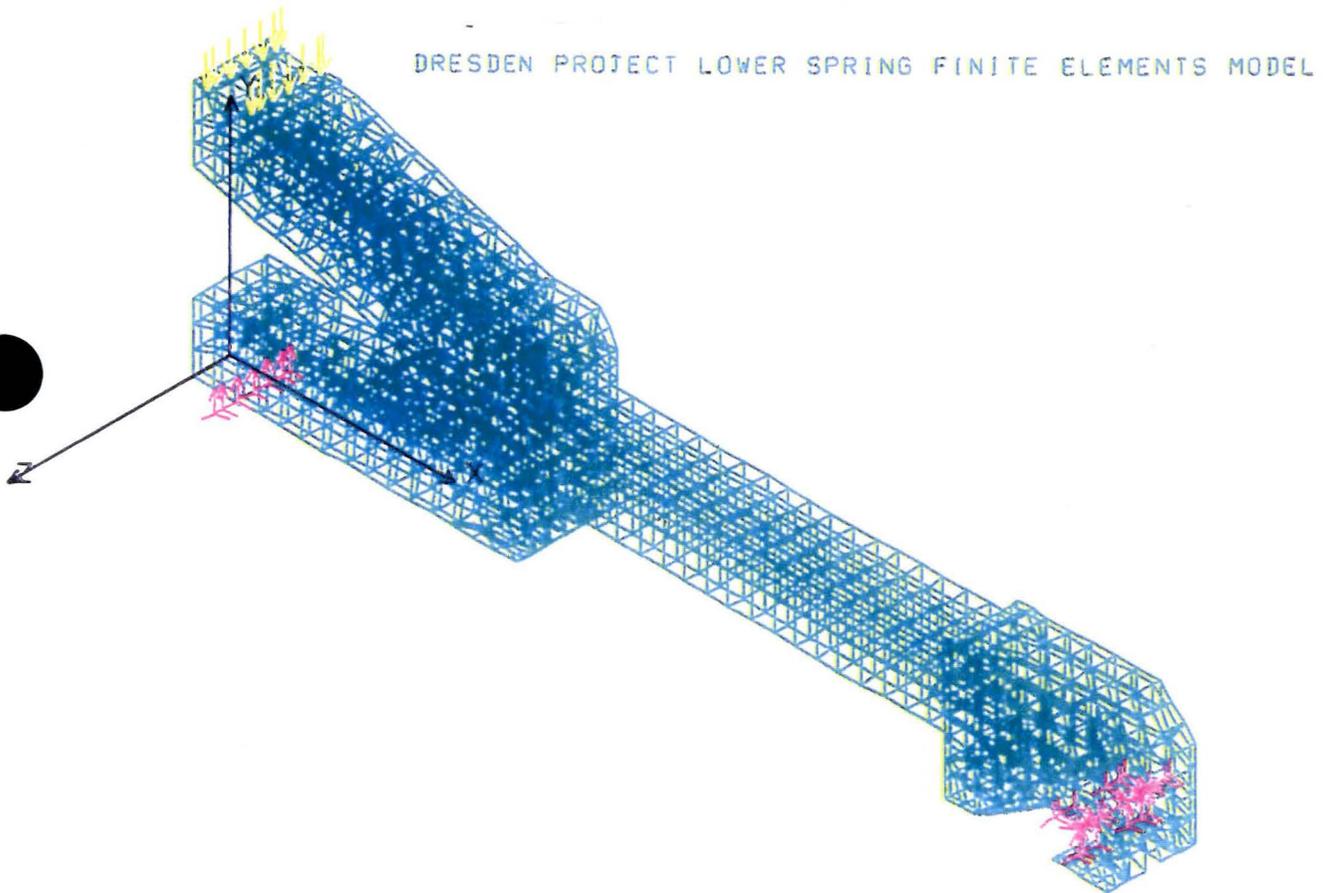


Figure 6.2-1 Lower Spring Finite Element Model

### 6.2.5. ANALYSIS RESULTS

The table below shows the shroud lower spring stresses during Normal/Upset and emergency and faulted events .

TABLE 6.2-3: Lower Spring Stress Summary

Condition	Applied loads (lbs)		Stress Intensity (psi)	Allowable stress (psi)
	Side Load	Vert. Load		
Normal / Upset	Side Load	93000	43400	70,500
	Vert. Load	194000	59500	70,500
Emergency	Side Load	186000	86800	106,875
	Vert. Load	339000	104000	106,875
Faulted	Side Load	190000	88700	142,500
	Vert. Load	339000	104000	142,500

As is seen in Table 6.2-3, all stresses for the critical load combinations are below the corresponding allowables.

### 6.2.6. SPRING CONSTANT

Based on the results of the finite element model of the lower spring the following spring constants are calculated:

Transverse spring constant: 325 kips/in

This spring constant includes the local flexibilities of shroud and RPV. The spring constant used in the seismic analysis is 300 kips/in which is 7.69% different from the calculated value, which is an acceptable difference.

Axial spring constant: 1973 kips/in

STRESS  $L_c=1$

DRESDEN LOWER SPRING FINITE ELEMENTS MODEL

Intens



NORMAL CONDITION

Figure 6.2-2 Lower Spring Normal/Upset Condition, Stress Intensity

STRESS  $L_c=1$

DRESDEN LOWER SPRING FINITE ELEMENT MODEL

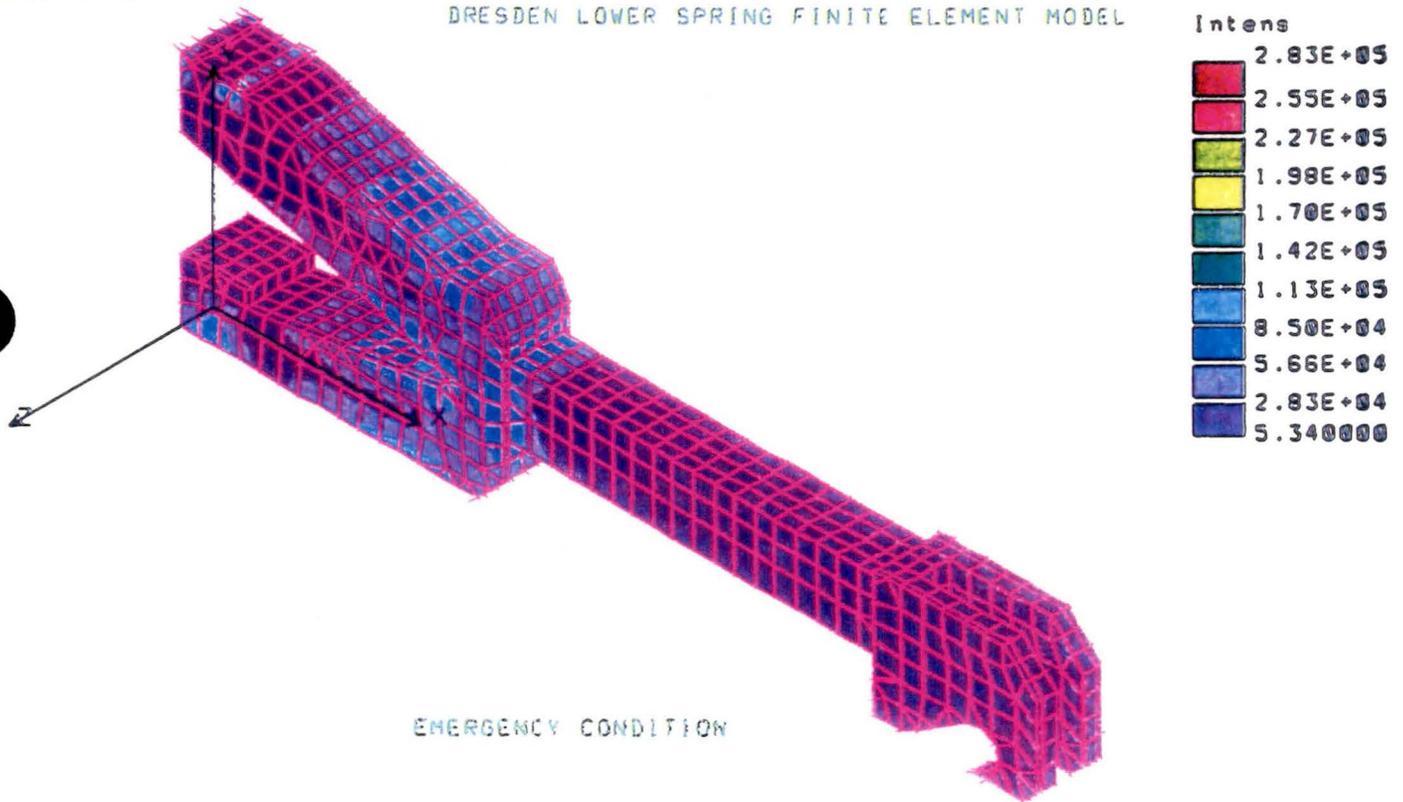
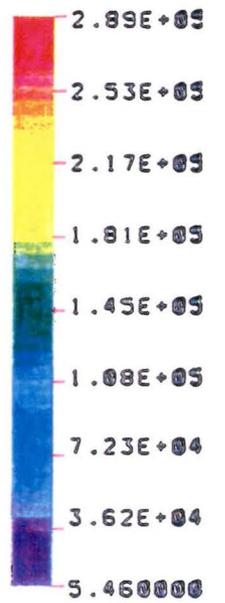


Figure 6.2-3 Lower Spring Emergency Condition, Stress Intensity

STRESS Lc=1

DRESDEN LOWER SPRING FINITE ELEMENT MODEL

Intens

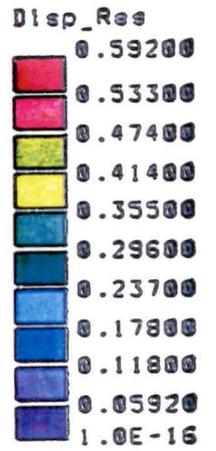


FAULTED CONDITION

Figure 6.2-4 Lower Spring Faulted Condition, Stress Intensity

DISP Lc=1

DRESDEN LOWER SPRING FINITE ELEMENT MODEL

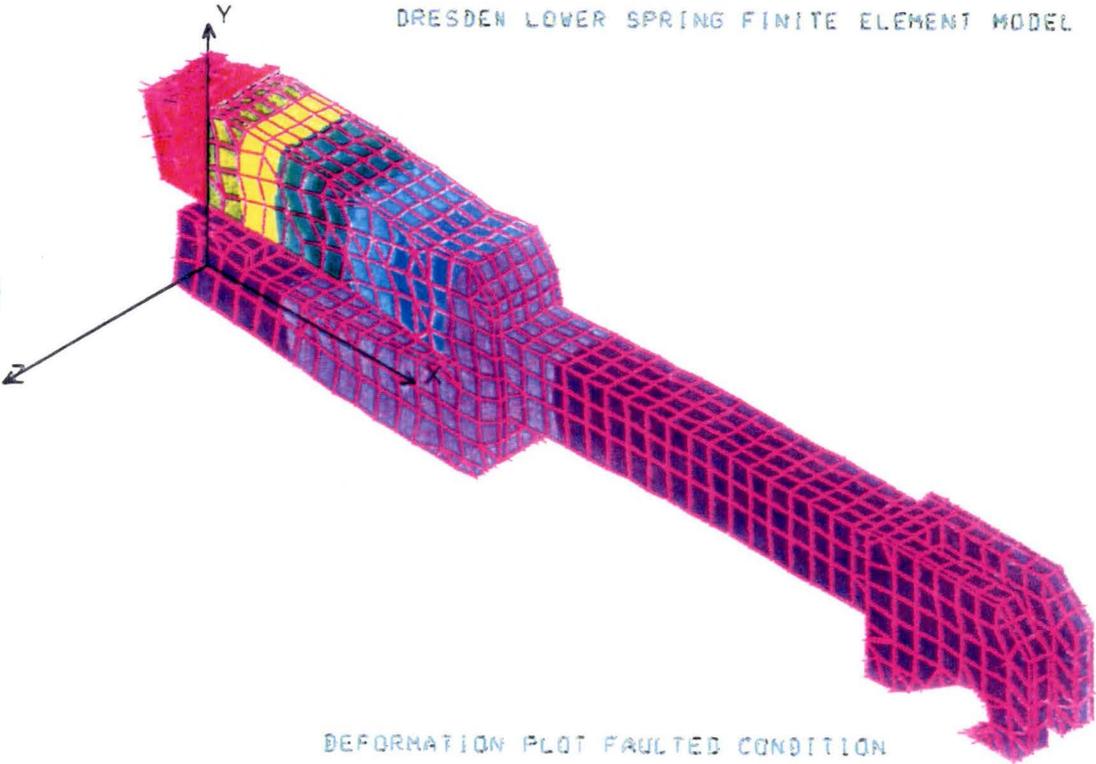
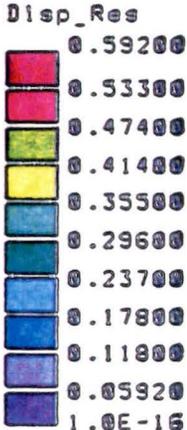


FAULTED CONDITION

Figure 6.2-5 Lower Spring Faulted Condition, Displacement

DISP Lc=1

DRESDEN LOWER SPRING FINITE ELEMENT MODEL



DEFORMATION PLOT FAULTED CONDITION

Figure 6.2-6 Lower Spring Faulted Condition, Deformation (Magnified)

### 6.3 Long Upper Support

Since the upper inclined portion of the long upper support (Drawing 112D6669) is embedded in the shroud, gaps exist between it and the shroud flange and the shroud head flange. Depending on the actual gap size, and the magnitude of the applied load, contact between the shroud head and the top surface of the upper inclined support can occur. The size of this gap may be as large as 0.1 inch. Parametric studies conducted (see DRF Section E.2) indicate that the case with a zero gap size rather than with a gap of 0.1 inch will result in a higher stress distribution, and therefore this case will be taken as the bounding case for a conservative design evaluation.

The long upper support is subjected to the following upper spring horizontal seismic load and the tie rod axial load [5,7].

Case	Seismic Load* (kips)	Tie Rod Load** (kips)
Normal / Upset	16.75	97.00
Emergency	33.50	169.50
Faulted	35.00	169.50
Thermal Upset	0.00	85.00

\* The load shown is for one of the four upper supports (two long and two short)

\*\* The load shown is for one of the two long upper supports

The horizontal load produces low bearing and compressive stresses. The critical stress occurs in the vicinity of the lip at the shroud flange interface (see Figure 6.3.2) due to the tie rod axial load. Following are the resulting stress intensities and the comparison with allowables for the normal/upset, emergency, faulted, or thermal upset conditions.

Case	$P_m$ (ksi)		$S_{allow}$ (ksi)	
Normal / Upset	27.79	<	47.50	1.00 $S_m$
Emergency	50.52	<	71.25	1.50 $S_m$
Faulted	51.23	<	95.00	2.00 $S_m$

Case	$P_m + P_b$ (ksi)		$S_{allow}$ (ksi)	
Normal / Upset	47.22	<	71.25	1.50 $S_m$
Emergency	85.45	<	106.88	2.25 $S_m$
Faulted	86.50	<	142.50	3.00 $S_m$

Case	$P_m + P_b + Q$ (ksi)		$S_{allow}$ (ksi)	
Thermal Upset	37.20	<	142.50	3.00 $S_m$

Note that the maximum stress intensity during the thermal upset condition is below the material's proportional limit of 74 ksi, and the preload on the shroud will thus be maintained in such event.

The finite element (FE) model and the stress results of the long upper support are shown in Figures 6.3.1 and 6.3.2, respectively

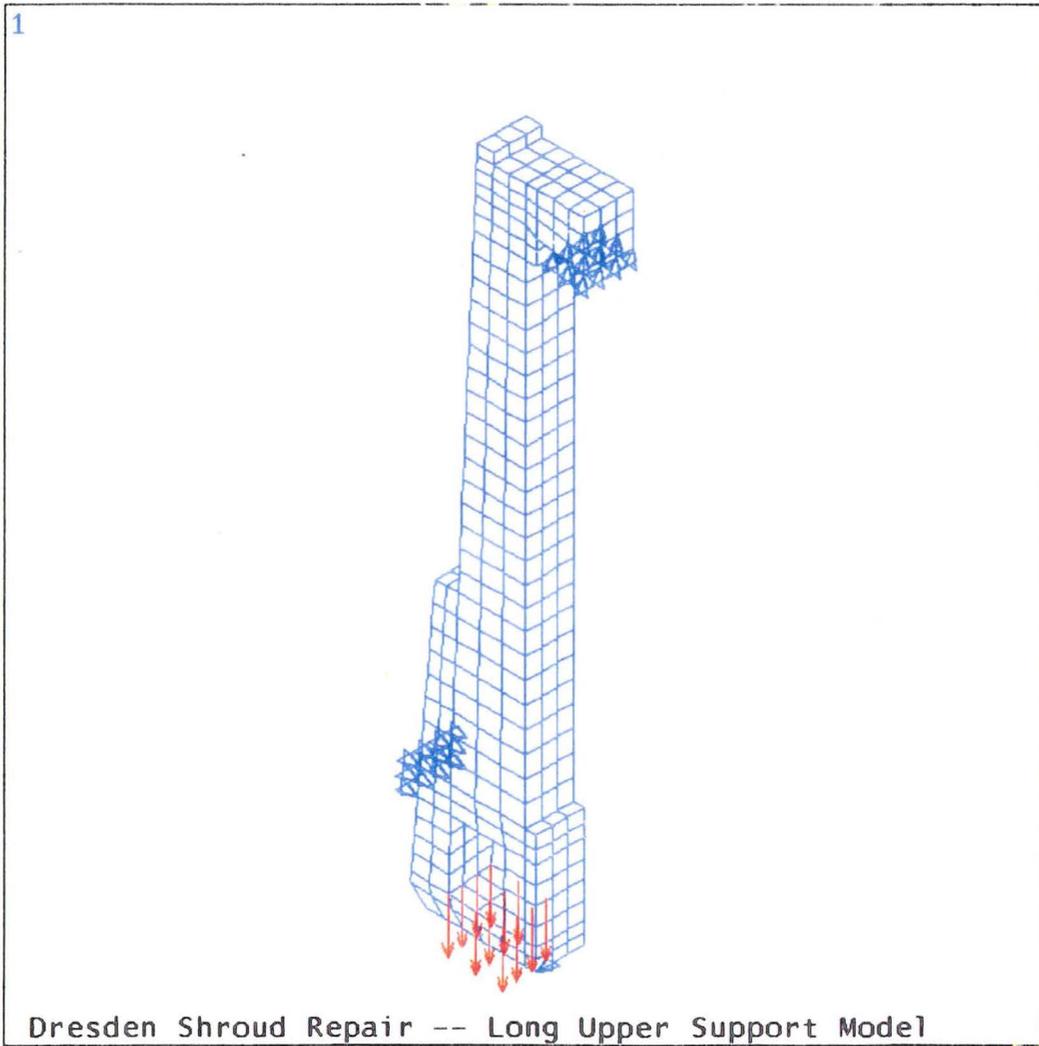


Figure 6.3.1 Long Upper Support FE Model and Boundary / Loading Conditions

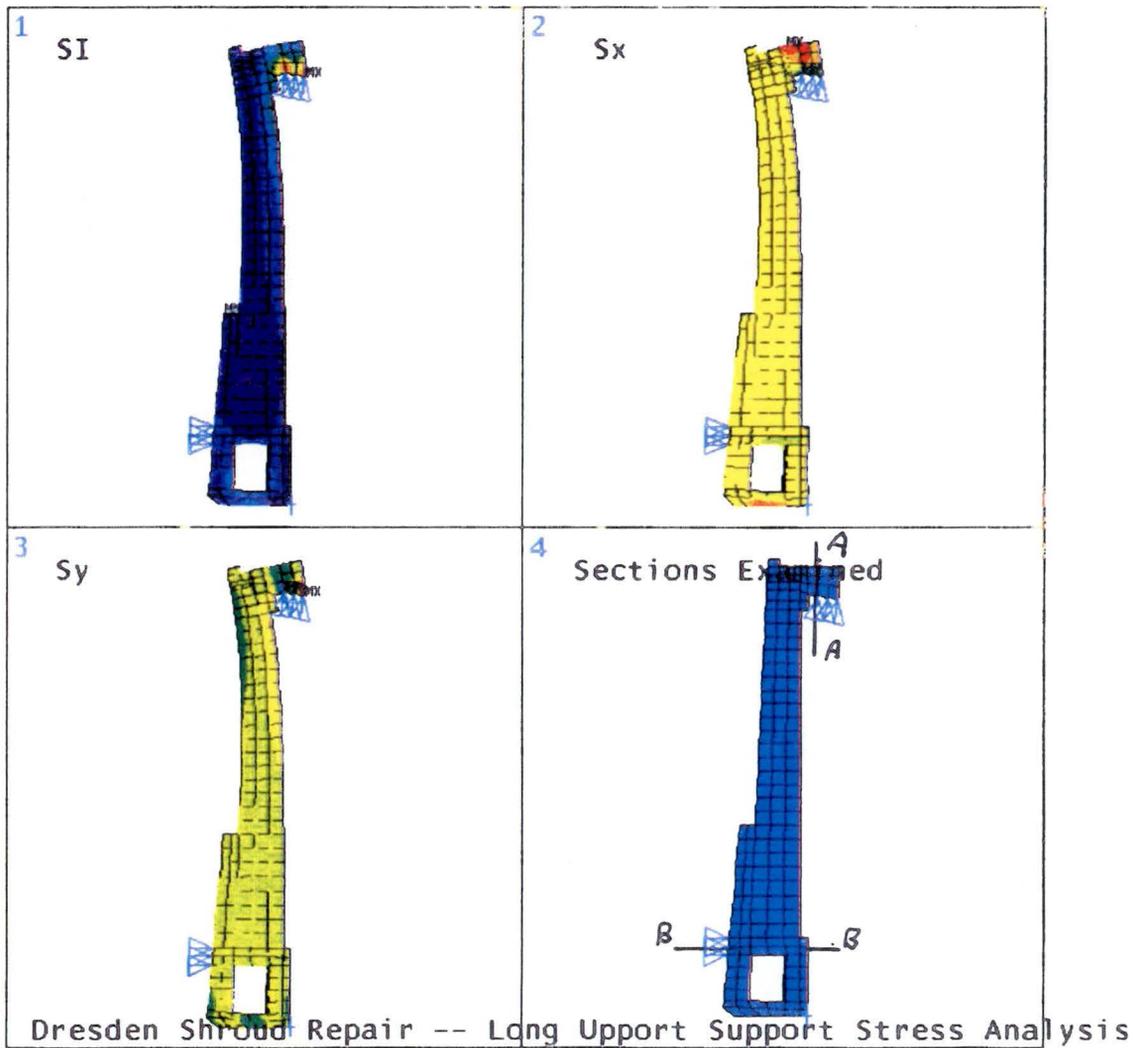


Figure 6.3.2 Long Upper Support FE Analysis Stress Plots

## 6.4 Bracket Yoke

The bracket yoke (Drawing 112D6675) is subjected to the following tie rod axial load [7].

Case	Tie Rod Load (kips)
Normal / Upset	194.00
Emergency	339.00
Faulted	339.00
Thermal Upset	170.00

The critical primary membrane stress occurs in the vicinity of the yoke support at the long upper support interface. Following are the resulting stress intensities and the comparison with allowables.

Case	$P_m$ (ksi)		$S_{allow}$ (ksi)	
Normal / Upset	15.32	<	47.50	1.00 $S_m$
Emergency	26.76	<	71.25	1.50 $S_m$
Faulted	26.76	<	95.00	2.00 $S_m$

The critical primary membrane plus primary bending stress occurs in the vicinity of the yoke at the tie rod interface. Following are the resulting stress intensities and comparison with allowables for the normal/upset, emergency, faulted or thermal upset conditions.

Case	$P_m+P_b$ (ksi)		$S_{allow}$ (ksi)	
Normal / Upset	31.35	<	71.25	1.50 $S_m$
Emergency	54.78	<	106.88	2.25 $S_m$
Faulted	54.78	<	142.50	3.00 $S_m$

Case	$P_m+P_b+Q$ (ksi)		$S_{allow}$ (ksi)	
Thermal Upset	27.47	<	142.50	3.00 $S_m$

Note that the maximum stress intensity during the thermal upset condition is below the material's proportional limit of 74 ksi, and the preload on the shroud will thus be maintained in such event.

The finite element model and the stress results of the bracket yoke are shown in Figures 6.4.1 and 6.4.2, respectively.

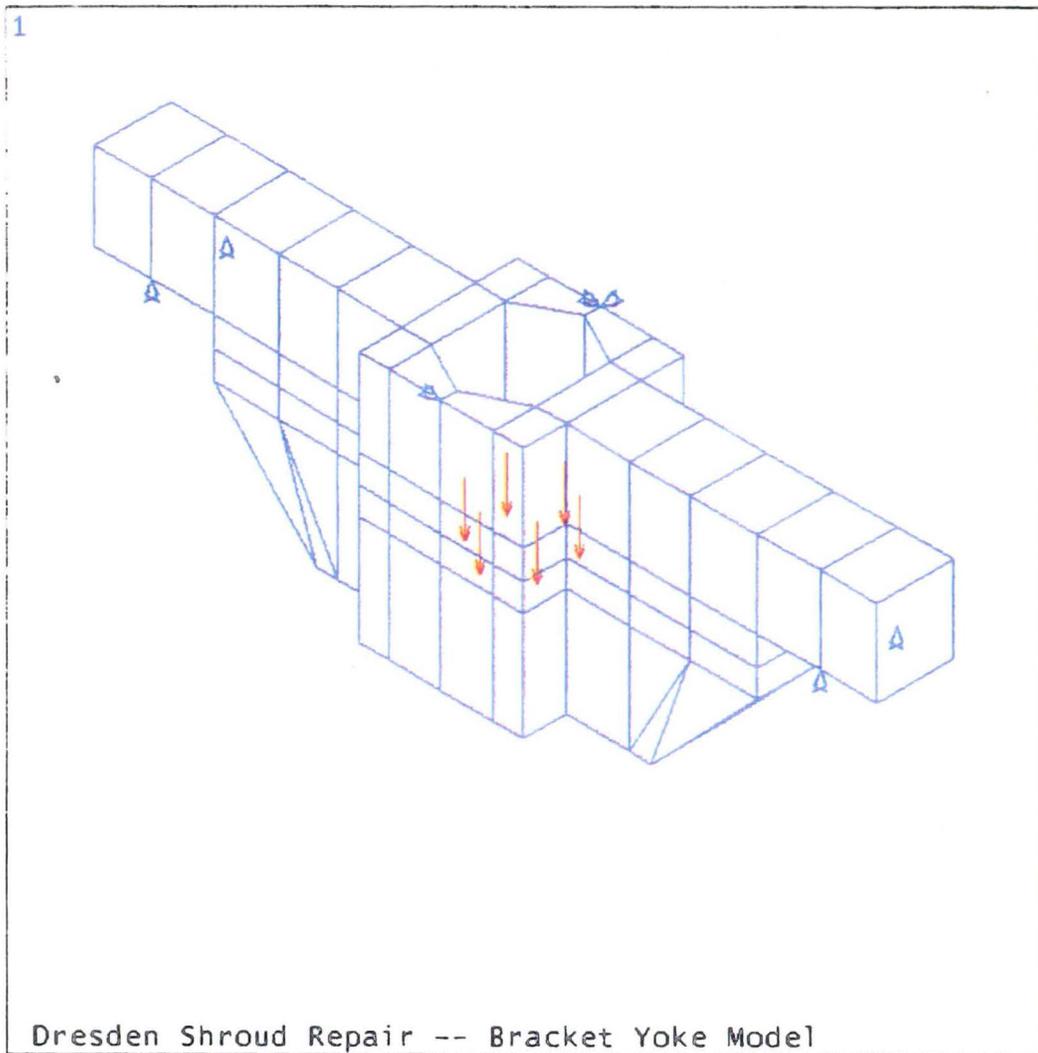


Figure 6.4.1 Bracket Yoke FE Model and Boundary / Loading Conditions

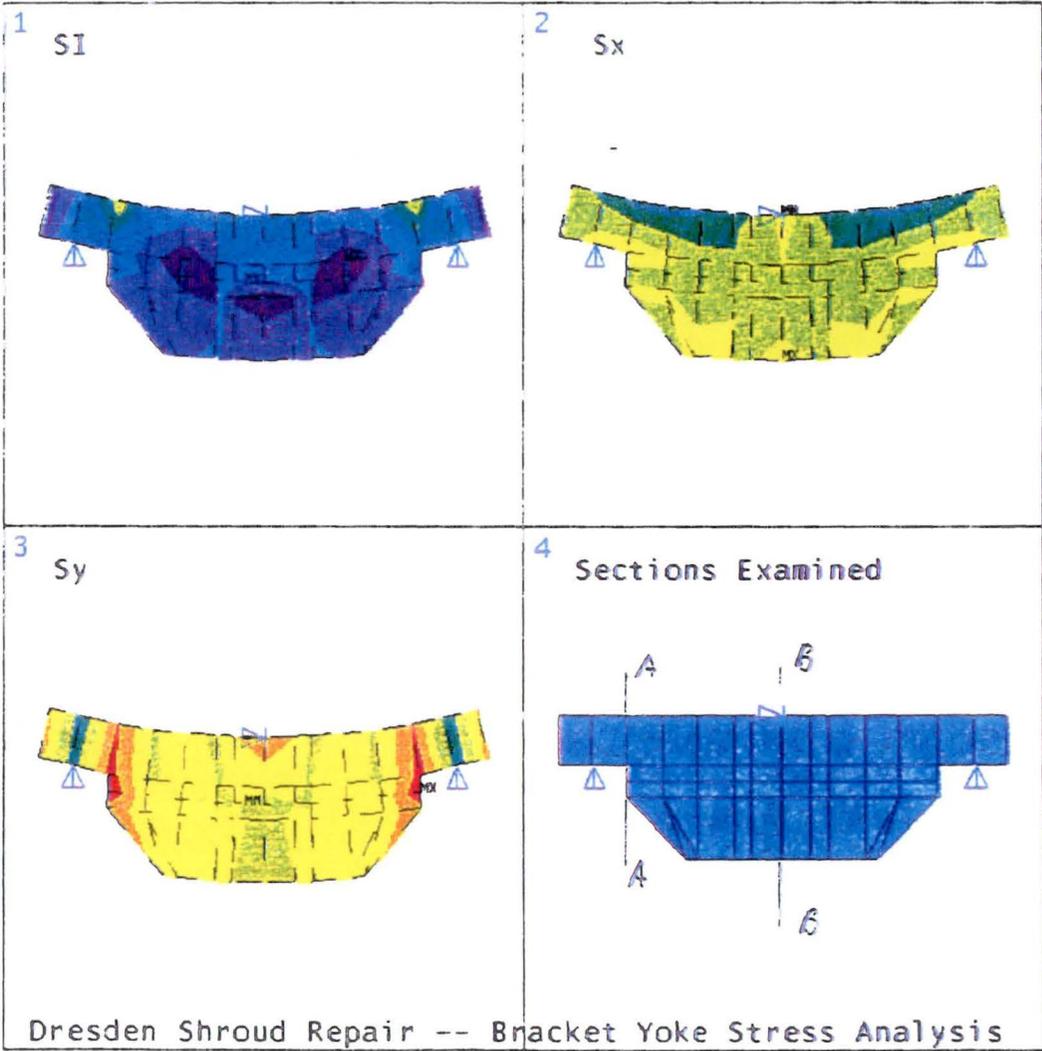


Figure 6.4.2 Bracket Yoke FE Analysis Stress Plots

## 6.5 Middle Spring

The middle spring (Drawing 112D6681) is subjected to the following horizontal seismic loads [5]:

Case	F (kips)
Normal / Upset	12.00
Emergency	23.00
Faulted	24.00

The primary membrane plus primary bending stress intensity at the leaf spring region of the spring govern. The values and comparison with allowables follow.

Case	$P_m$ (ksi)		$S_{allow}$ (ksi)	
Normal / Upset	4.41	<	47.50	1.00 $S_m$
Emergency	8.45	<	71.25	1.50 $S_m$
Faulted	8.82	<	95.00	2.00 $S_m$

Case	$P_m+P_b$ (ksi)		$S_{allow}$ (ksi)	
Normal / Upset	37.46	<	71.25	1.50 $S_m$
Emergency	71.81	<	106.88	2.25 $S_m$
Faulted	74.93	<	142.50	3.00 $S_m$

The horizontal stiffness of the middle spring is 80 kips/inch based on finite element analysis. Accounting for the shroud local flexibility at the spring interface, the effective horizontal stiffness at the middle spring is 37 kips/inch. This is very close to the 35 kips/inch value used in the seismic analysis. The finite element model of this middle spring and its stress results are shown in Figures 6.5.1 and 6.5.2, respectively.

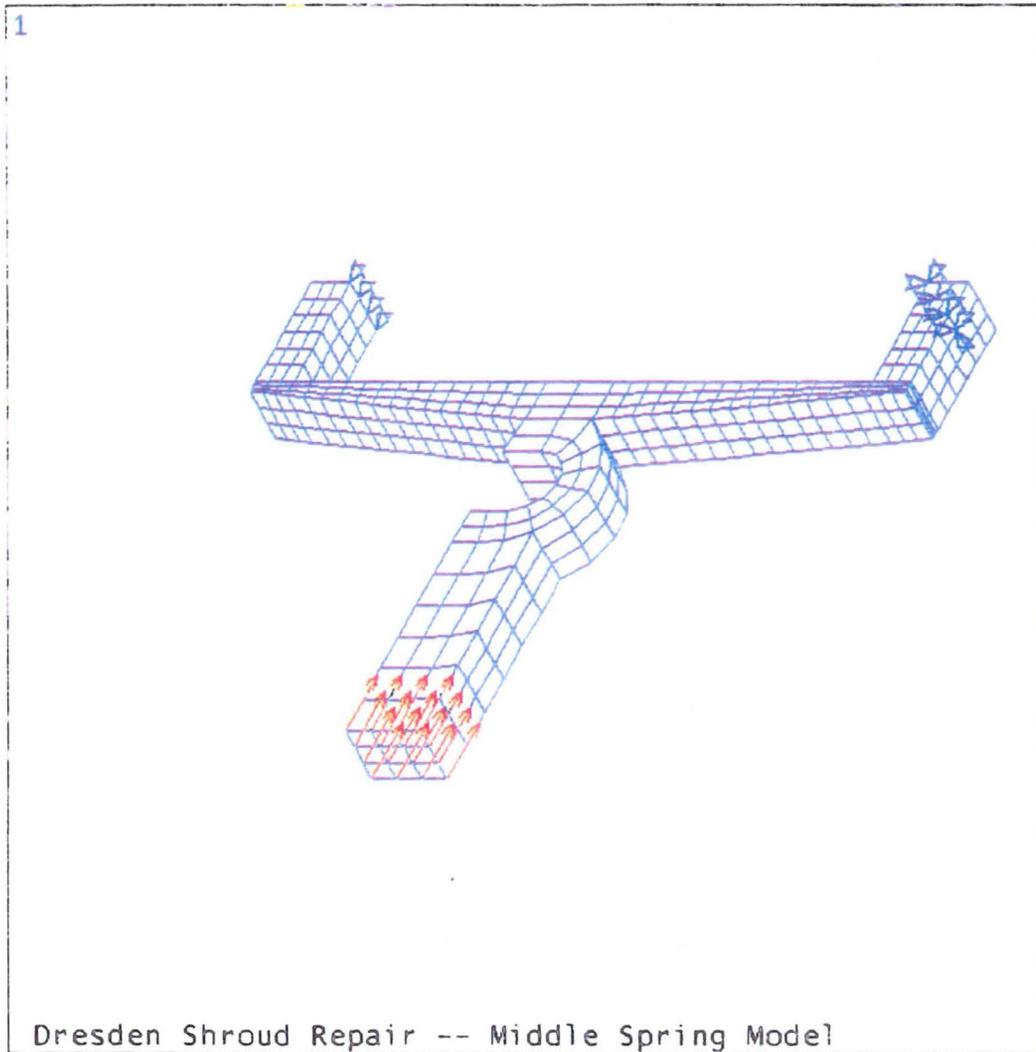


Figure 6.5.1 Middle Spring FE Model and Boundary / Loading Conditions

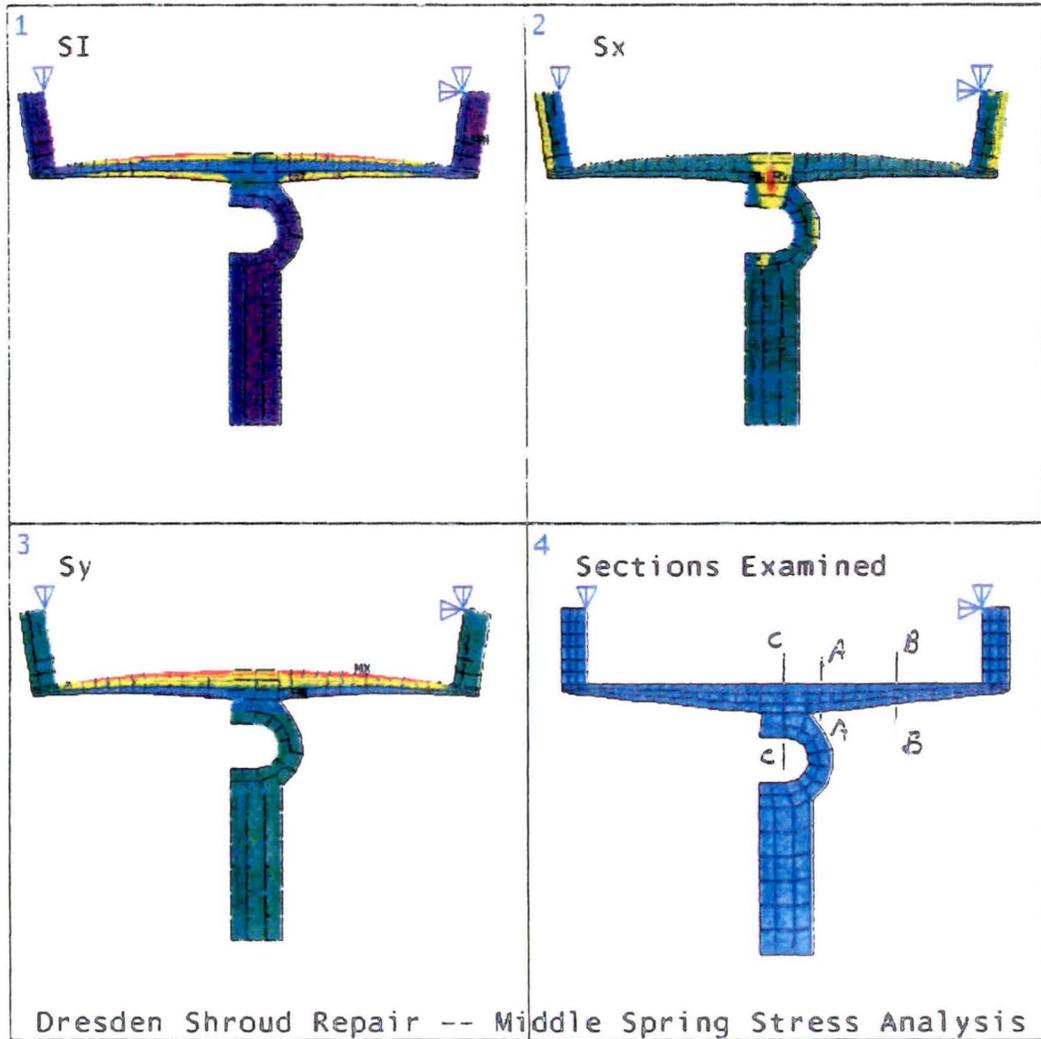


Figure 6.5.2 Middle Spring FE Analysis Stress Plots

## 6.6 Tie Rod

The tie rod (Drawing 112D6672) is subjected to the following middle spring horizontal seismic load and the tie rod axial load [5,7].

Case	Seismic Load (kips)	Tie Rod Load (kips)
Normal / Upset	12.00	194.00
Emergency	23.00	339.00
Faulted	24.00	339.00
Thermal Upset	0.00	170.00

The maximum rod primary membrane stress intensities during the normal/upset, emergency, or faulted conditions are as follows,

Case	$P_m$ (ksi)	$S_{allow}$ (ksi)	
Normal / Upset	20.17	< 29.58	1.0 $S_m$
Emergency	35.24	< 44.37	1.5 $S_m$
Faulted	35.24	< 59.16	2.0 $S_m$

And the corresponding rod critical primary membrane plus bending plus secondary stress intensity during a thermal upset event is as follows,

Case	$P_m + P_b + Q$ (ksi)	$S_{allow}$ (ksi)	
Thermal Upset	17.67	< 88.74	3.0 $S_m$

Note that the maximum stress intensity during the thermal upset condition is below the material's proportional limit of 31.44 ksi, and the preload on the shroud will thus be maintained in such event. The Tie Rod assembly stiffness is 650 kips/inch, which is very close to the value used in the seismic analysis (609 kips/inch).

Since the tie rod is subjected to a cross flow of coolant, its susceptibility to flow induced vibration was investigated. Natural vibration frequencies were derived using the finite element analysis (the model used is shown in Figure 6.6.1). The tie rod axial load of 138 kips under normal/upset condition was included in the model. The lowest natural frequency was found to be 35.5 Hz. To conservatively derive the flow vortex shedding frequency for comparison with the natural frequency, the bulk flow velocity of 4.9 feet/second near the tie rods adjacent to the jet pump inlet was assumed to be directed normal to the tie rod. The resulting vortex shedding frequency of 4.6 Hz is judged to be sufficiently lower than the lowest natural frequency such that no flow induced tie rod vibrations are expected. For the fatigue evaluation in the thermal upset event, the allowable number of fatigue cycles has been determined to be greater than 2,000 [Figure I-9.2.2 in Reference 3], which far exceeds the actual number of cycles anticipated. Therefore, the fatigue requirements are satisfied.

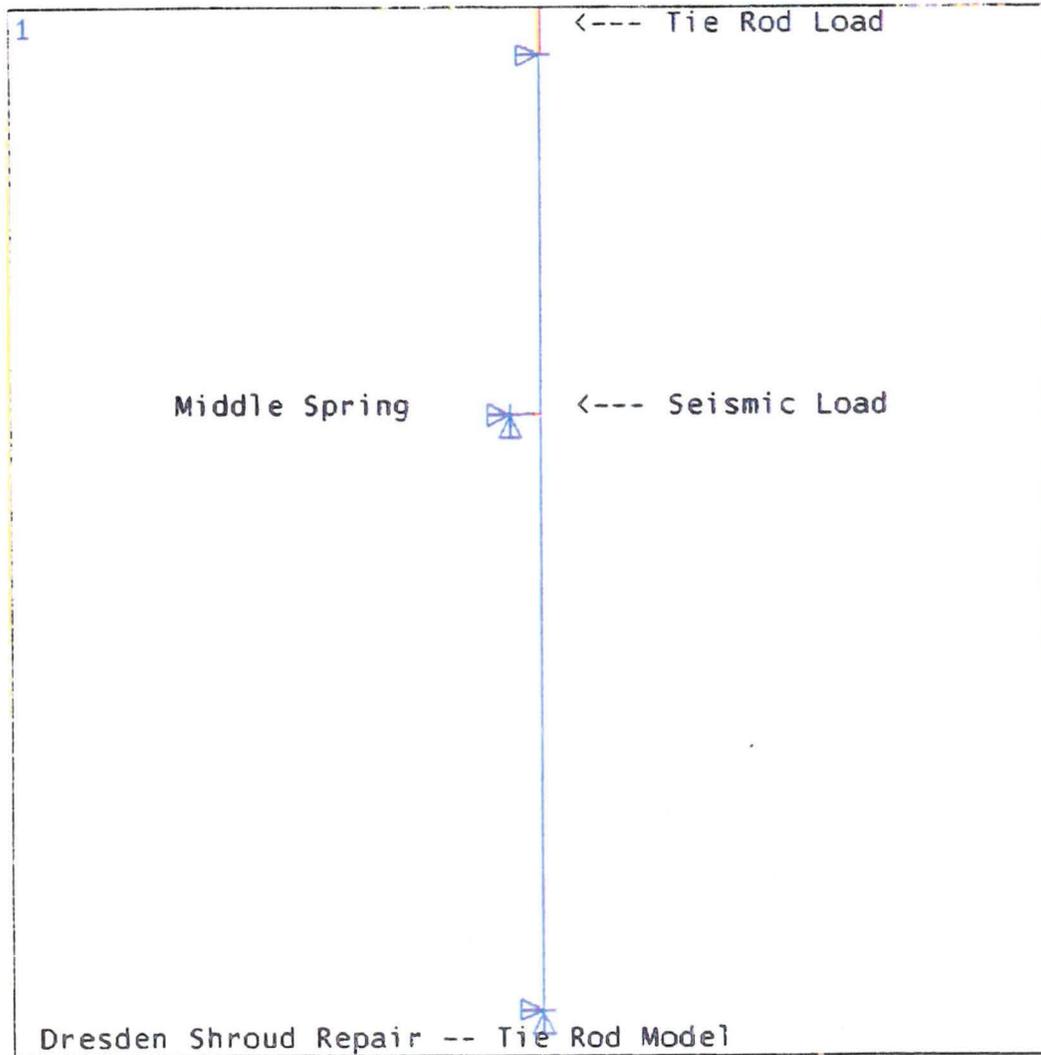


Figure 6.6.1 Tie Rod FE Model and Boundary / Loading Conditions

## 6.7 Lower Support and Toggle Assembly

The toggle bolt is the limiting component for this evaluation. The results for this X-750 material component are as follows:

Component	Limiting Load	Stress (ksi)		$S_{allow}$ (ksi)
Horizontal Plate	$P_m+P_b$	67.54	<	106.88
Vertical Plate	$P_m$	43.30	<	71.25
Clevis Pin	$P_m+P_b$	67.38	<	106.88
(normal/upset)	Shear	12.65	<	36.92
(emergency)	Shear	22.10	<	48.00
(faulted)	Shear	22.10	<	48.00
Toggle Pin	$P_m+P_b$	70.39	<	106.88
(normal/upset)	Shear	20.14	<	36.92
(emergency)	Shear	35.19	<	53.44
(faulted)	Shear	35.19	<	71.25
Toggle Assembly	$P_m$	46.10	<	71.25
Toggle Assembly Fork	$P_m+P_b$	67.00	<	106.88
Toggle Bolt	$P_m+P_b$	104.63	<	106.88

The toggle bolt stress calculation is based on a very low preload of the bolts. If the bolt preload is increased sufficiently the bolt stress will be reduced accordingly.

## 7.0 REFERENCES

1. ANSYS, General Purpose Finite Element Program, Version 4.4. Swanson Analysis Systems, Inc.
2. "Dresden Units 2 and 3, Shroud Stabilizer Hardware Design Specification", 25A5688, Rev.2.
3. ASME Boiler and Pressure Vessel Code, Section III, Appendices, 1989 Edition.
4. GENE Y1002A051, Rev.1.
5. "Dresden Units 2 and 3 Shroud Repair Seismic Analysis", GENE-771-84-1194, Rev.2.
6. "ComEd Technical Requirements Document for Dresden / Quad Cities Core Shroud Repair", NEC-12-4056.
7. " Back-up Calculations for Dresden Units 2 and 3 Pressure Vessel Stress Report", GENE-771-77-1194, Rev.2.
8. "Project Instruction Shroud Repair For H1 Through H7 Welds For Commonwealth Edison Dresden Units Nuclear Power Station", GENE-771-80-1194, Rev. 1.
9. "Dresden Units 2 and 3 UFSAR", Commonwealth Edison , Rev. 0.
10. COSMOSM, General Purpose Finite Element Program, Version 1.7.1, Structural & Analysis Corporation.
11. "Section E.2 of Design Record File (DRF) for the Dresden Shroud Repair Program", DRF B13-01749.
12. "Dresden Units 2 and 3, RPV Code Design Specification", 25A5689, Rev.2.

## **ATTACHMENT C**

**GENE Materials Technology  
A Stress Corrosion Cracking Evaluation  
of  
XM-19 in the BWR Environment  
June 15, 1995**