

**Attachment B**  
**Flood Plain Report**  
(1014 pages)

## CENTRALIZED INTERIM STORAGE FACILITY DRAINAGE EVALUATION AND FLOODPLAIN ANALYSIS

MARCH 2016  
REVISED NOVEMBER 2016  
REVISED DECEMBER 2016

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12 December 2016



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## 1.0 INTRODUCTION

This report presents the results of a hydrologic and hydraulic analysis of the proposed conditions in and around the area of the Centralized Interim Storage Facility (CISF) proposed to be licensed by the Nuclear Regulatory Commission at the Waste Control Specialists, LLC (WCS) site located in Andrews County, Texas. This report is prepared in support of the Safety Analysis Report (SAR) as described at 10 CFR 72.24 and addresses items contained in the “Standard Review Plan for Spent Fuel Dry Storage Facilities”, NUREG-1567, dated March 2000, Section 2.4.4 Surface Hydrology.

### 1.1 HYDROLOGIC DESCRIPTION

The CISF site is located in western Andrews County, Texas nearly at the Texas – New Mexico border, just north of Texas Highway 176 approximately 31 miles west of Andrews, Texas and 5 miles east of Eunice, New Mexico. There are no maps of special flood hazard areas for this location published by the Federal Emergency Management Agency (FEMA). The Site Location and Surrounding Topography Map, Figure 1.1-1, shows the CISF site location with respect to the surrounding topography and drainage features and the WCS property boundary.

#### 1.1.1 Hydrosphere

From a surface water perspective, the general area is characterized by ephemeral drainages, sheet flow, minor gullies and rills, internally-drained playas, and a salt lake basin (identified on Figure 1.1-1 as the Depression Pond). The salt lake basin is the only naturally-occurring, perennial (year-round) water body located near the CISF site; the internally-drained salt lake basin is located approximately 5 miles from the eastern boundary of the CISF site and rarely has more than a few inches of water at scattered locations within the bottom footprint. Surface drainage from the CISF site does not flow into this basin. Other perennial surface water features are man-made, including various stock tanks (often replenished by shallow windmill wells) located across the area and the feature denoted as the Fish Pond on Figure 1.1-1, which is located at the existing Permian Basin Materials quarry west of the CISF site and is also replenished by well water. In addition, Sundance Services, LLC operates the Parabo Disposal Facility for oil and gas waste on portions of the Permian Basin Materials quarry property. Water collects periodically in excavated and/or diked areas at this disposal facility and in the active quarry areas at this property adjacent to and west of the WCS property in New Mexico.



Baker Spring, another man-made feature, is located at a historic quarry on WCS property about 2,150 ft west of the CISF site in Lea County, New Mexico. This feature was formed by excavation of the caliche caprock to the top of the underlying red bed clays. After periods of rainfall, the depression holds water for some period until it evaporates. During wet cycles, the depression may hold water for an extended period; during dry cycles, the depression may be dry for extended periods.

The National Oceanic and Atmospheric Administration's National Weather Service Office for Hobbs, New Mexico indicates that the minimum average annual precipitation recorded is 2.01 inches in 2011 and the maximum average annual precipitation recorded is 32.19 inches in 1941. The annual precipitation on average is approximately 14 inches.

The CISF site is located on the southwest-facing slope that transitions from the Southern High Plains to the Pecos Valley physiographic section. The Southern High Plains is an elevated area of undulating plains with low relief encompassing a large area of west Texas and eastern New Mexico. In Andrews County, the southwestern boundary of the Southern High Plains is poorly defined, but in this report is considered to be where the caprock caliche is at or relatively close to surface, such as on and near the CISF site.

The main surface water drainage in the area is Monument Draw, an ephemeral stream about 3 miles west of the WCS site in New Mexico. Ephemeral streams or drainage ways flow briefly only in direct response to precipitation in the immediate locality. Monument Draw is a reasonably well-defined, southward-draining feature (although not through-going) that is identified on the USGS topographic maps that serve as the base map source for Figure 1.1-1.

An ephemeral drainage feature, referred to as the Ranch House Draw crosses the WCS property from east to west, generally to the south of the CISF site, as shown on Figure 1.1-1. This feature is discernible from the topographic relief depicted on Figure 1.1-1, although it is much less pronounced than Monument Draw. This drainage feature is a relict drainage way that is choked with windblown sand and is not through-going to Monument Draw. Most of the drainage from the area of the CISF site is down slope toward the Ranch House Draw, with a small portion of the drainage from this area toward the southwest. Surface water eventually infiltrates into the windblown sands and dune fields to the south and southwest of the CISF site.



There are no ephemeral drainages that cross the CISF site. Most of the immediate area of the CISF site is drained from northwest to southeast by sheet flow. Sheet flow is a term describing overland flow or down slope movement of water taking the form of a thin, continuous film.

Playas, or small, internally-drained basins, occur on the WCS property. The playas are dry most of the time. Some of the playas occasionally hold water after relatively large precipitation events; however, the ponded water rapidly dissipates through infiltration, evaporation, and plant uptake. An established playa basin is present on the eastern edge of the CISF site. Surface topography maps indicate approximately 10 ft of relief in the playa.

The combination of low annual precipitation, relatively high potential evapotranspiration, permeable surficial soils down gradient of the CISF site, and topographic relief results in well-drained conditions. The engineering design and construction of the CISF site will eliminate areas that might promote ponding. Diversion berms and a collection ditch will direct stormwater from upstream drainage areas around the CISF.

There are no public or private surface water drinking-water supplies in the site vicinity. Potable water supply for the WCS facility is provided by the City of Eunice, which gets its water from wells in the Hobbs area. There are scattered windmills in the general area that take water from isolated pockets of groundwater perched on top of the red bed clay. This water is utilized primarily for livestock watering.

#### 1.1.2 Site and Structures

The CISF site is defined as the area within the owner controlled fence and is approximately 320 acres as depicted on the Developed Drainage Plan, Figure 1.1.2-1. The CISF site is undeveloped and the existing land surface is fairly flat with an average slope of 0.8 percent (%). The existing maximum and minimum elevations of the site are about 3520 ft and 3482 ft msl, respectively. The cover type is desert shrub. The existing WCS railroad is generally aligned parallel with and south of the proposed southern CISF site boundary.

The CISF storage area, which is within the CISF site, is defined as the area within the protected area fence whose boundary is defined by a rectangle 2360 feet by 2430 feet, as indicated on the Developed Drainage Plan, Figure 1.1.2-1. Included in the storage area are the security/administration building, the transfer building, the storage pads and a portion of the CISF



rail side track. The CISF storage area is approximately 132 acres and is graded for surface drainage with slopes of approximately 0.8 % from the northwest to the southeast. Developed elevations across the CISF storage area range from 3506 ft msl at the northwest corner to 3486 ft msl near the southeast corner.

All of the surface water runoff from the storage area will drain into the large playa southeast of the site. Flow arrows on Figure 1.1.2-2, Developed Drainage Area Map provide the detailed drainage patterns for the CISF site.



## 2.0 FLOODS

There is no evidence that the CISF site area has experienced flooding in the past. The ranch house drainage within the WCS property was evaluated as part of a Flood Plain Study conducted in February 2004 (Revised December 2004 and March 2006) for the Application for License to Authorize New-Surface Land Disposal of Low-Level Radioactive Waste (LLRW) that was approved by the Texas Commission on Environmental Quality (TCEQ) in 2009 as Radioactive Material License No. R04100. The 2004 Flood Plain Study as revised through March 2006 is provided as Appendix A and includes maps depicting the drainage areas within the WCS property and the location of the 100-year, 500-year and Probable Maximum Precipitation (PMP) flood plain. The 100-year flood plain extends across the southern portion of the WCS property area along the ranch house drainage. The northernmost limit of the 100-year floodplain is approximately 4,000 ft southeast of the CISF site while the northernmost limits of the 500-year and PMP floodplains are 3965 feet and 3895 feet southeast of the CISF site respectively.

The prior floodplain analysis indicated that the PMP elevation of the large playa located mostly east of the CISF site is 3488 ft msl. A portion of the CISF site is located over the large playa. Elevations of the storage pads, security/administration building, and the transfer facility are above 3490 ft msl.

An analysis of the drainage features around the CISF site is performed for the PMP to ensure that the structures important to safety are safe from flooding.

### 2.1 FLOOD HISTORY

The climate of the area is classified as semiarid, characterized by dry summers and mild, dry winters. Annual precipitation on average is approximately 14 inches and annual evaporation exceeds annual precipitation by nearly five times. The area is subject to occasionally winter storms, which produce brief snowfall events of short duration.

Rainfall records from July 2009 through December 2015, provided by WCS from a weather station near the CISF site, indicate an average annual rainfall of 12.6 inches and a maximum twenty-four hour rainfall total of 3.62 inches. According to WCS personnel, surface water runoff



has not overflowed roads or existing drainage features at the WCS facility during this time frame.

## 2.2 FLOODPLAIN ANALYSIS DEVELOPED CONDITIONS

This analysis identifies the limits of the watershed in which the CISF site is proposed to be located and determines the local peak flow rates and water elevations at the watershed analysis points resulting from the 100-year and 500-year frequency storm events and the Probable Maximum Precipitation event (PMP) after the CISF site is fully developed. This analysis also identifies the location of the local PMP floodplain associated with a large playa/depression located within the subject watershed.

### 2.2.1 Description of Watershed

The contributing watershed that crosses the CISF site contains about 869 acres (1.4 square miles). For the most part, the CISF site is located on top of a hill and will be graded to allow drainage away from the site. Fully developed conditions result in four distinct drainage areas that predominantly slope away from the CISF site. The Developed Drainage Area Map, Figure 1.1.2-2, identifies the developed drainage area boundaries in relation to the CISF site and the associated analysis points described below.

Drainage Area P DA 1 contains 100.9 acres and drains the northwest portion of the site outside of the storage area. Analysis Point P AP 1 is located where surface water runoff from P DA 1 flows across State Line Road. Drainage Area P DA 2 contains 46.1 acres and drains the southwest portion of the CISF site contained between the existing WCS railroad and the CISF rail side track outside of the storage area. Analysis Point P AP 2 is located at the western intersection of the CISF rail side track and the existing WCS railroad. Drainage Area P DA 3 contains 42.8 acres and drains the southeast portion of the CISF site bounded by the existing WCS railroad and the CISF rail side track. Surface water runoff from P DA 3 discharges into the large playa located east of the facility. Drainage Area P DA 4 contains 679.3 acres encompassing the large playa and the majority of the CISF site; surface water from this portion of the CISF site also discharges into the large playa. Analysis Point P AP 3 refers to the location where surface water runoff in the large playa will overtop the existing ground to the south.



The watershed is located in Andrews County, Texas. The *Custom Soil Resource Report for Andrews County, Texas, and Lea County, New Mexico*, prepared by the United States Department of Agriculture (USDA), Natural Resources Conservation Service (NRCS), located in Appendix B, shows the watershed contains soils from the Blakeney and Conger, Jalmarpennwell, Ratliff, and Triomas and Wickett series. These soils are classified with the hydrologic groups A, B and D. Group A soils have high infiltration and transmission rates. Group B soils have moderate infiltration and transmission rates. Group D soils have very low infiltration and transmission rates. The Soils Boundary Map with the CISF site location, topographic information and drainage area boundaries is included as Figure 2.2.1-1.

## 2.2.2 Description of Hydrologic Analysis Methodology

Surface water runoff from the watershed in which the CISF site is located is modeled using the U.S. Army Corps of Engineers Hydrologic Engineering Center's Hydrologic Modeling System (HEC-HMS), version 4.0. The rainfall amount for the 100-year frequency storm event is taken from the *USDA Soil Conservation Service (SCS) Texas Engineering Technical Note No. 210-18-TX5, October 1990 (TETN 210)*. A 24-hour storm duration is used. The 100-year 24-hour rainfall amount from *TETN 210* for the CISF site is six (6) inches and is the same rainfall amount used in the floodplain study in Appendix A. The 500-year, 24-hour and PMP, 72-hour rainfall amounts are taken from the floodplain study in Appendix A and are 8.71 inches and 40.5 inches, respectively. The precipitation amounts used as input for the HEC-HMS model are as follows:

<u>Return Period</u>	<u>Rainfall (In.)</u>
100-Year, 24 Hour	6.0
500-Year, 24 Hour	8.71
PMP, 72 Hour	40.5

Peak discharges from small watersheds are usually caused by intense, brief rainfalls. Utilizing synthetic rainfall distribution as taken from *TETN 210* in this case is common practice instead of using actual storm events. The synthetic Type II, 24-hour rainfall distribution for Andrews County, Texas, as shown on Figure 1 of *TETN 210*, and the SCS dimensionless unit hydrograph method are used for the model. The method requires curve numbers to indicate the



runoff potential of a hydrologic soil-cover complex and watershed lag to model watershed response. The development of these values is described in the following paragraphs.

The curve number (CN) is computed based on land use, cover type, hydrologic condition and soil group. A December 16, 2015 site visit supported determination of land use, cover types and hydrologic condition. Hydrologic condition indicates the effects of cover type and treatment on infiltration and runoff. The hydrologic condition of the cover at the site is considered poor. The soil group information is taken from the Soil Report in Appendix B. The variability of the CN from rainfall intensity and duration, total rainfall, soil moisture conditions, cover density, stage of growth, and temperature are collectively accounted for in the Antecedent Runoff Condition (ARC). The three classes of ARC are as follows: I for dry conditions, II for average conditions, and III for wetter conditions. Figure 5 of *TETN 210* indicates that the ARC across the state of Texas varies greatly and Andrews County is ARC I. In order to be conservative and check the sensitivity of the model to the various ARC conditions, all three classes are used in the CN determinations and the model.

The *USDA NRCS, Part 630 Hydrology, National Engineering Handbook (NEH)* explains that lag is the delay between the time runoff from a rainfall event over a watershed begins until runoff reaches its maximum peak. Lag is empirically estimated as six-tenths (0.6) of the time of concentration, (USDA NRCS, Part 630, NEH, Equation 15-3). The time of concentration is the time it takes for runoff to travel from the hydraulically most remote part of a watershed to a point of consideration. In hydrograph analysis it represents the time from the end of “excess rainfall” to the point of inflection of an SCS unit hydrograph.

Time of concentration is computed by determining the travel times for different segments of the flow path. The segments consist of sheet flow, shallow concentrated flow and concentrated flow. The sheet flow and shallow concentrated flow components are calculated for all of the drainage areas using the equations from *USDA SCS Technical Release 55, Urban Hydrology for Small Watersheds*. Drainage Areas P DA 1 and P DA 2, as shown on Figure 1.1.2-2, also exhibit channelized flow. Broad channelized flow occurs in P DA 1 as the surface water flows southwest out of the CISF site and crosses State Line Road. Channelized flow occurs in P DA 2 as the surface water flows southwest in the existing ditch along the northern side of the existing WCS railroad. Concentrated flow is calculated based on the flow velocity for the



channel being analyzed. Channel velocities are calculated using Manning's Equation or they are estimated based on the results of the HEC-HMS model. All time of concentration parameters for the various drainage areas are included in Appendix C, Calculations.

Storage, elevation, and outflow curves are developed for the playa/depression located within the subject watershed to determine its effect on the runoff from this area and are included in Appendix C. All watershed parameters that are topography dependent are based on the WCS provided aerial survey dated May 29, 2014 flown by Dallas Aerial Surveys, Inc and the WCS provided proposed CISF elevations.

### 2.2.3 Site Drainage and Model Strategy

The CISF site drainage features consist of a collection ditch and four culverts through the CISF rail side track that are located as shown on the Developed Drainage Plan, Figure 1.1.2-1. The design criterion for the site drainage features are the 100-Year, 24 Hour, ARC I, peak flow rates as determined by HEC-HMS. Whenever possible, surface water runoff will be maintained as sheet flow. Conservative input parameters and strategies are used in the HEC-HMS modeling of the peak flow rates.

#### 2.2.3.1 Site Drainage

Surface water runoff from the up gradient area north of the storage area will be diverted by a collection ditch located just north of the storage area boundary as shown on Figure 1.1.2-1. Onsite surface water runoff will be mainly sheet flow off of the sloped storage pads and the sloped areas in between the pads. The land surface adjacent to the eastern and western perimeters of the storage pads will be sloped to drain as sheet flow toward the protected area fence and beyond through the owner controlled area fence. Surface water runoff between the collection ditch and the northern storage pads within the storage area will sheet flow to the southeast. Surface water runoff south of Phase 1 storage pad will drain southeast into Culvert 2 under the CISF rail side track just west of the transfer building. Surface water runoff south of the Phase 5 storage pad and the CISF rail side track will sheet flow to the east.

The transfer building roof drains half to the north and half to the south. The western portion of the area between the CISF rail side track and the existing railroad outside of the storage area will drain to the west with some of the surface water runoff flowing through the existing culvert



under the WCS railroad crossing at State Line Road and the rest of it flowing through Culvert 1 into existing surroundings. The eastern portion of the area between the CISF rail side track and existing railroad will drain to the east and empty into the large playa through Culverts 3 and 4.

#### 2.2.3.2 Model Strategy

Conservative parameters are input into the HEC-HMS model to determine peak runoff rates and overflow elevations. Conservative assumptions include the following: (1) all areas inside the storage area are assumed to be impervious for the CN calculation; (2) all three ARC conditions are used for the CN calculation even though Andrews County exhibits ARC I conditions; (3) no consideration is given to initial losses or infiltration rates of the precipitation; (4) all culverts are presumed clogged and do not allow any flow through them; and (5) the collection ditch and berms are not in place in order to model the greatest possible area contributing runoff into the playa. The probable maximum flood (PMF) flow is modeled over the existing railroad and the proposed CISF rail side track.



### 3.0 SUMMARY OF RESULTS

The Developed Drainage Area Map, Figure 1.1.2-2 delineates the subject watershed including drainage areas and analysis points. The 100-year, 500-year, and PMP peak discharges for each drainage area and ARC condition as determined by the HEC-HMS model are shown in Table 1, Post-Development Drainage Areas – Peak Flow. The 100-year, 500-year, and PMP runoff volumes for each drainage area and ARC condition as determined by the HEC-HMS model are shown in Table 2, Post-Development Drainage Areas – Runoff Volumes.

The 100-year, 500-year, and PMP water surface elevations at analysis points as determined by HEC-HMS for every ARC condition are shown in Table 3, Post-Development Analysis Points - Peak Elevation.

At Analysis Point 1, the peak discharge resulting from all modeled storm events flows over State Line Road. The peak discharge (during the PMP and ARC III conditions) is 424 cubic feet per second (CFS). The maximum depth of flow over the road (during the PMP and ARC III conditions) is approximately 0.8 ft. which is equivalent to elevation 3487.3 ft. msl.

The peak discharge resulting from all modeled storm events flows over the railroad tracks at Analysis Point 2. The peak discharge (during the 500-year and ARC III conditions) is 284 CFS. The maximum depth of water over the rail (during the 500-year and ARC III) is approximately 1.4 ft. which is equivalent to elevation 3466.4 ft. msl.

The playa/depression contains all the runoff from drainage areas P DA 3 and P DA 4. The limit of the PMP, ARC III condition, water surface elevation of the playa/depression based on the topographic information provided by WCS is 3488.9 ft. msl and is shown on Figure 1.1.2-2, Developed Drainage Area Map. The results indicate that the playa/depression does not discharge during the 100-year frequency event but does discharge at Analysis Point 3 during the other modeled events. The peak discharge (during the PMP and ARC III conditions) flowing out of the playa is 3005 CFS. The depth of the PMP, ARC III, peak discharge flow over the railroad tracks at Analysis Point 3 is approximately 1.5 ft. which is equivalent to elevation 3488.9 ft. msl.



#### **4.0 CONCLUSIONS**

The local PMP floodplain analysis yields the PMF elevation near the CISF site of 3488.9 ft msl. Elevations of the storage pads vary from 3490 ft msl to 3504 ft msl. Elevations of the foundations of the security/administration building and the transfer facility are 3496 ft msl and 3493 ft msl, respectively.



## 5.0 OTHER CONSIDERATIONS

The naturally occurring playa/depression will reach its maximum elevation for a brief time as the surface water flows out over the rail and the natural ground and infiltrates into the existing ground. At the peak elevation the area of the water surface in the playa/ depression is approximately 280 acres which is too small to produce any wind wave activity.

No PMP analysis of perennial streams or rivers is considered since they do not exist in the vicinity of the CISF site.

There are no dams on any upgradient areas from the site; therefore, no analysis is required.

Since no large bodies of water exist near the site, no surge, seiche, or ice flooding is possible.

The site is located 480 miles from the Gulf of Mexico, which is the nearest coastal area; therefore, no tsunami sea waves are possible.

There are no liquid releases that result from the normal operation of the CISF.

The local short-term overland flow depth of surface water runoff and velocity on the CISF Phase 1 pad for the 500-year rainfall event are calculated using Manning's Equation. The maximum rainfall intensity for all analyzed storms is used which is the 500-year rainfall event and is taken from the HEC-HMS output. Calculations are found in Appendix C and the results are as follows:

Maximum depth: 1.1 inches

Maximum velocity: 1.7 feet/second



## 6.0 REFERENCES

Waste Control Specialists LLC, Application for License to Authorize Near-Surface Land Disposal of Low-Level Radioactive Waste, Appendix 2.4.1: Flood Plain Study, March 2006.

United States Department of Agriculture, Natural Resources Conservation Service. *Custom Soil Resource Report for Andrews, County, Texas, and Lea County, New Mexico*, December 2015.

United States Department of Agriculture, Soil Conservation Service. *Texas Engineering Technical Note No. 210-18-TX5*, October 1990 (TETN 210).

United States Department of Agriculture, Natural Resources Conservation Service. *Part 630 Hydrology*, National Engineering Handbook (NEH), Chapter 15, Time of Concentration, May 2010.

United States Department of Agriculture, Natural Resources Conservation Service Technical Release 55. June 1986. Urban Hydrology for Small Watersheds.



## **TABLES**



**TABLE 1**  
**WCS - CISF FLOOD ANALYSIS**  
**POST-DEVELOPMENT DRAINAGE AREAS - PEAK FLOW**

**ARC I**

<b>Drainage Area</b>	<b>100 YR Peak Flow (CFS)</b>	<b>500 YR Peak Flow (CFS)</b>	<b>PMP Peak Flow (CFS)</b>
P DA 1	118.3	245.4	410.7
P DA 2	118.1	209.2	191.1
P DA 3	127.5	218.2	178.4
P DA 4	803.6	1523.1	2786.9

**ARC II**

<b>Drainage Area</b>	<b>100 YR Peak Flow (CFS)</b>	<b>500 YR Peak Flow (CFS)</b>	<b>PMP Peak Flow (CFS)</b>
P DA 1	223.4	373.1	421.5
P DA 2	170.8	264.8	193.1
P DA 3	173.8	265.4	179.8
P DA 4	1324.0	2113.8	2839.4

**ARC III**

<b>Drainage Area</b>	<b>100 YR Peak Flow (CFS)</b>	<b>500 YR Peak Flow (CFS)</b>	<b>PMP Peak Flow (CFS)</b>
P DA 1	292.0	440.6	424.2
P DA 2	193.2	284.4	193.5
P DA 3	191.1	279.9	180.1
P DA 4	1574.7	2346.9	2849.7



**TABLE 2**  
**WCS - CISF FLOOD ANALYSIS**  
**POST-DEVELOPMENT DRAINAGE AREAS - RUNOFF VOLUMES**

**ARC I**

<b>Drainage Area</b>	<b>100 YR Runoff Volume (IN)</b>	<b>500 YR Runoff Volume (IN)</b>	<b>PMP Runoff Volume (IN)</b>
P DA 1	2.09	4.11	33.97
P DA 2	3.09	5.44	36.38
P DA 3	3.38	5.81	36.94
P DA 4	2.62	4.84	35.35

**ARC II**

<b>Drainage Area</b>	<b>100 YR Runoff Volume (IN)</b>	<b>500 YR Runoff Volume (IN)</b>	<b>PMP Runoff Volume (IN)</b>
P DA 1	3.68	6.17	37.48
P DA 2	4.52	7.14	38.76
P DA 3	4.74	7.38	39.05
P DA 4	4.20	6.78	38.30

**ARC III**

<b>Drainage Area</b>	<b>100 YR Runoff Volume (IN)</b>	<b>500 YR Runoff Volume (IN)</b>	<b>PMP Runoff Volume (IN)</b>
P DA 1	4.96	7.63	39.34
P DA 2	5.41	8.11	39.88
P DA 3	5.53	8.23	40.00
P DA 4	5.18	7.87	39.61



**TABLE 3**  
**WCS - CISF FLOOD ANALYSIS**  
**POST-DEVELOPMENT ANALYSIS POINTS - PEAK ELEVATION**

**ARC I**

<b>Analysis Point</b>	<b>100 YR MAX WSE (FT)</b>	<b>500 YR MAX WSE (FT)</b>	<b>PMP MAX WSE (FT)</b>
P AP 1	3486.9	3487.1	3487.2
P AP 2	3466.0	3466.3	3466.2
P AP 3	3484.4	3485.8	3488.8

**ARC II**

<b>Analysis Point</b>	<b>100 YR MAX WSE (FT)</b>	<b>500 YR MAX WSE (FT)</b>	<b>PMP MAX WSE (FT)</b>
P AP 1	3487.0	3487.2	3487.3
P AP 2	3466.2	3466.4	3466.2
P AP 3	3485.4	3486.5	3488.9

**ARC III**

<b>Analysis Point</b>	<b>100 YR MAX WSE (FT)</b>	<b>500 YR MAX WSE (FT)</b>	<b>PMP MAX WSE (FT)</b>
P AP 1	3487.1	3487.3	3487.3
P AP 2	3466.2	3466.4	3466.2
P AP 3	3486.0	3486.8	3488.9

**NOTES:**

1. Water surface elevation (WSE) represent elevation above mean sea level (AMSL).
2. Elevations are taken from topographic aerial survey provided by Dallas Aerial Surveys, Inc., flown 5-29-2014. 10220 Forest Lane, Dallas, Texas 214-349-2190, 800-862-2190, Fax 214-349-2193.