

Docket No.: 50-275

OCT 27 1983

MEMORANDUM FOR: Chairman Palladino
 Commissioner Gilinsky
 Commissioner Roberts
 Commissioner Asselstine
 Commissioner Bernthal

FROM: Darrell G. Eisenhut, Director
 Division of Licensing
 Office of Nuclear Reactor Regulation

SUBJECT: DIABLO CANYON - ALLEGATIONS CONCERNING SMALL BORE PIPING
 AND SUPPORTS (Board Notification No. 83-171)

In accordance with the procedures for Board Notification the enclosed information is being provided directly to the Commission. The Boards and Parties are being informed by copy of this memorandum.

On October 27, 1983 the NRC staff received the enclosed allegations concerning the analysis and design of small bore piping and supports. The NRC staff will evaluate these allegations. We will inform you of the results of these efforts when available.

Darrell G. Eisenhut, Director
 Division of Licensing
 Office of Nuclear Reactor Regulation

Enclosure:
 As stated

cc: See next page

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 JLee
 10/ /83~~

~~DL:LB#3
 GWRighton
 10/27/83~~

~~DL:LB#3
 SRlack
 10/ /83~~ ~~DL:AD/L
 TMovak
 10/ /83~~

DL:DIR
 DGEisenhut
 10/ /83

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The Commissioners

- 2 -

cc: J. F. Wolf, ASLB
G. O. Bright, ASLB
J. Kline, ASLB
T. S. Moore, ASLAB
W. R. Johnson, ASLAB
J. H. Buck, ASLAB
SECY
OGC
EDO
Parties to the Proceeding



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DISTRIBUTION LIST FOR BOARD NOTIFICATION

Diablo Canyon Units 1&2
Docket Nos. 50-275/323 OL

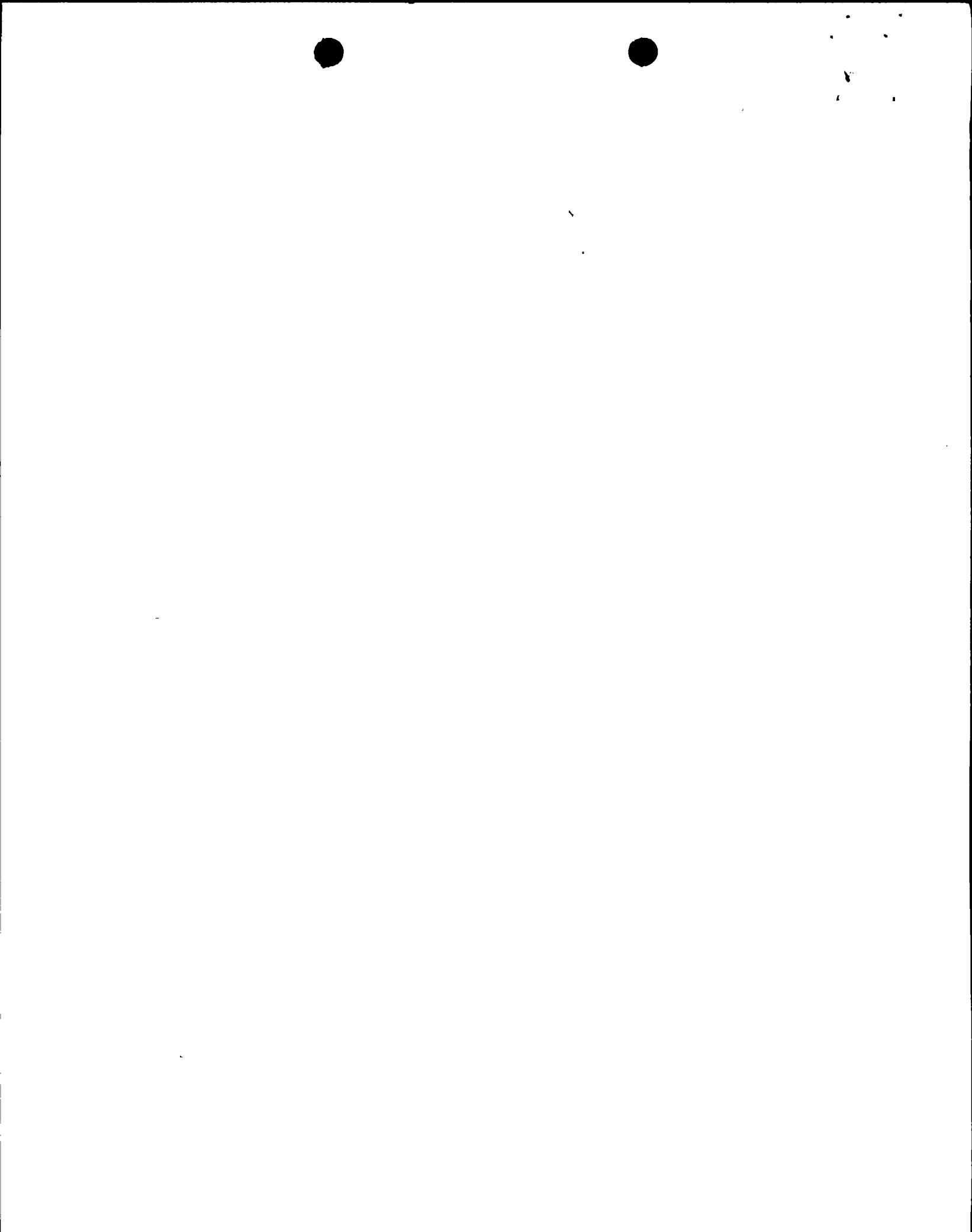
Ms. Elizabeth Apfelberg
Maurice Axelrad, Esq.
Andrew Baldwin, Esq.
Mr. Richard E. Blankenburg
Mr. Glenn O. Bright
Dr. John H. Buck
Philip A. Crane, Jr., Esq.
Mr. Frederick Eissler
David S. Fleischaker, Esq.
Mrs. Raye Fleming
Arthur C. Gehr, Esq.
Mr. Mark Gottlieb
Mr. Thomas H. Harris
Mr. Richard B. Hubbard
Dr. W. Reed Johnson
Janice E. Kerr, Esq.
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Mr. John Marrs
Thomas S. Moore, Esq.
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Paul C. Valentine, Esq.
Harry M. Willis
John F. Wolf, Esq.

Atomic Safety and Licensing
Board Panel
Atomic Safety and Licensing
Appeal Panel

Mr. Malcolm H. Furbush
Resident Inspector/Diablo Canyon NPS
Dr. William E. Cooper
Mr. W. C. Gangloff
Mr. Owen H. Davis
Dr. Jose Roesset

ACRS Members

Dr. Robert C. Axtmann
Mr. Myer Bender
Dr. Max W. Carbon
Mr. Jesse C. Ebersole
Mr. Harold Etherington
Dr. William Kerr
Dr. Harold W. Lewis
Dr. J. Carson Mark
Mr. William M. Mathis
Dr. Dade W. Moeller
Dr. David Okrent
Dr. Milton S. Plesset
Mr. Jeremiah J. Ray
Dr. Paul C. Shewmon
Dr. Chester P. Siess
Mr. David A. Ward



Enclosure

It has become evident that Bechtel Corporation management (backed by PSE) lacks the discipline and engineering integrity to insure code compliance and quality assurance involved in the design, analysis and construction of piping and pipe supports necessary for the safe operation of Diablo Canyon Nuclear Power Plant.

Improper design and analysis is now final for Unit I that will endanger the operational systems of Diablo and possibly the surrounding environment, all due to Bechtel's attempt to mitigate required design and analysis set forth by the nuclear industry. Bechtel's own design and analysis documentation is, in itself, proof that both the integrity of the piping as well as the pipe supports are not meeting industry standards that would otherwise insure proper safety operation at Diablo. In an attempt to meet schedule goals (low power loading, Nov. 1955), Bechtel has purposely approved analyses that has FAILED by altering current documentation that indeed shows FAILING piping systems and FAILING pipe supports...all by a statistical "copy" known to Bechtel as The Sample.

The purpose of this report is to indicate how Bechtel/PSE has altered FAILING systems such that they now appear to PASS all code requirements. This report will illustrate how Bechtel has mitigated the required analysis at Diablo in order to maximize their "engineering profits". It is important for the public to be made aware that it will ultimately be their environmental and financial risk alone when Diablo is allowed to go on-line as a result of PSE, the NRC and the IATP having allowed Bechtel to tragically substitute Economics for Engineering in the building of this nuclear power plant.

Attached you will find copies of Analysis 12-500 and 3-315 and various sheets from other analyses that illustrate major quality assurance problems at Diablo. Many modelling techniques that have been imposed by management (under the direction of Myron Lepply) upon their technical staff have resulted in numerous fictitious analyses in the Unit I Sample. Management has been informed by competent engineers from the technical staff that such modelling techniques yield erroneous results and, subsequently, lead to violations of code requirements and analysis procedures. Management, however, has reacted to these concerns by "pulling" the analyses away from the concerned staff and re-assigning them to engineers that are not totally cognizant of standard nuclear analysis procedures and/or guidelines. The sole purpose in this effort is to alter as many pipe stress failures and pipe support failures as necessary such that the Unit I Sample can statistically be shown to meet all code requirements--and hence, the remaining 400+ yet unanalyzed piping systems in Unit I need not be analyzed.

The current modelling techniques of support flexibilities is a prime example of the "tricks" which management is undertaking in order to statistically pass the small bore piping and pipe support sample. Engineers have been instructed to selectively model individual failing "rigid" supports as flexible "in selective load cases" such that the previous failing loads disappear or are absorbed into the surrounding rigid supports--eventhough there is no actual difference in the relative stiffness of these supports. In this way, Sample supports can be shown to be "passing", eventhough in reality, they are still failing. Needless to mention that as a result ① the surrounding supports are now designed to fictitious loads and ② the pipe stress analysis is no longer valid for the qualification of the pipe itself due to these imposed flexibility imbalances.



Having realized that modelling selective rigid supports as flexible is a viable method for the reduction of construction problems and subsequent fixes, management is proceeding with "passing" failing piping systems and pipe supports that are necessary in order to pass the overall Sample Program. Can Bechtel, F&B, and the NRC explain how selective flexibilities, in selective load cases, can be used at Diablo, whereas code compliance has not allowed it to be used at other nuclear plants?

In order to approve these Pass-Altered Failures, Bechtel management has acquired engineering signatures from employees that are less compelled to follow good engineering practice and guidelines set forth by Diablo Canyon procedures. Upon inspection, the basic documentation of Analysis 12-300 reveals the random guess-work that employees are capable of making during the analysis of Class 1 Nuclear piping.

Inspection of Analysis 3-313 Rev(0) illustrates how a competent engineer has proven that the inclusion of an anchor is necessary to validate this analysis. The next revision, Rev(1), now shows how management has over-ridden this warning in an effort to circumvent the necessary field construction to fix this failing system. The attached documentation shows the three distinct phases of Analysis 3-313. The first shows the failing system before the proper anchor was modelled into the analysis (see attached computer print-out). The second, Rev(0), shows the anchor included per proper code-organ requirements. The third, Rev(1), shows how management has removed the necessary anchor and, instead, placed lumped masses to fictitiously represent the original piping model. The lumped-mass technique currently shows a passing piping system—even though it is supposed to be the equivalent of the original "no anchor" model that was proved to fail initially. (Management's proud explanation of the lumped-mass technique is also attached).

Another technique that has been employed to pass failing Sample supports is the addition of new supports adjacent to the failing Sample supports such that the failing loads will be distributed among the new supports. In this manner, the Sample supports can be reported as having passed the load allowables. Management justifies these new support additions under different reasons (thermal expansion problems, seismic problems, etc.) to hide the real reason for the fix: A Failing Sample Support. Although no documentation is attached in this report, it is readily observable upon inspection of the "before and after" drawings of the piping and pipe support locations. The initial "as-built" isometrics show scattered supports and Sample supports...and the final drawings show the addition of new supports placed next to the Sample supports.

This report is being sent to you due to the lack of response from the Diablo Canyon management. Violations of modelling techniques, if not appropriately rectified by other standard nuclear regulations, could jeopardize the functional requirements of many operational systems at Diablo. I have already properly expressed numerous other generic design problems to Bechtel and F&B. No longer under the employment of Bechtel/F&B, I am compelled to submit these concerns externally to insure proper review and resolution to these existing problems.

To aid in your review of this information, the following table categorizes the specific quality assurance problem with the particular analysis:



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ANALYSIS #	DESCRIPTION
8-301	Use of supports for seismic analysis but not for thermal analysis (modelling of penetrations)
8-307	
8-322A	
8-322B	
8-322C	
8-324	Support flexibility
8-307	
8-326	
8-320	
12-300	Documentation guess-work
8-315 (init)	Lumped-mass modelling vs. inclusion of anchor
Rev(0)	
Rev(1)	
REPORT	Weld design & procedures

(11)
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 G

* Documentation not attached

It is my personal opinion that the small bore Sample should be allowed (as originally planned) to indicate the actual credibility of the entire small bore piping and pipe supports of Unit 1. I am currently withholding my name such that the above quality assurance concerns are, in themselves, focused upon instead of any "employment" issues that Bechtel/PG&E may use to cloud the "engineering" issues. These Pass-Altered Failures are just part of the TOTAL failing systems still existing at Diablo, the only difference being that these are known to fail "yet, just on paper!". The full realization of the total number of failing systems that still exist at Diablo may never be seen: lost with the acceptance of the Pass-Altered Failures... and, in the process, the pass-altered Sample itself.

~~cc: [redacted]~~

- G.A. Manassis, Pacific Gas & Electric Co.
- Mothers Of Peace, San Luis Obispo
- The San Francisco Chronicle
- San Luis Obispo Telegram-Tribune



FOR INFORMATION ONLY

ANALYSIS ISSUE

<input checked="" type="checkbox"/> New Analysis	<input checked="" type="checkbox"/> Change in Analysis	SEP 6 1988
Design Review Isometric No.	HD-359, HS-353, HS-741, H7-215 H7-234, H4-286	D. C. Unit: 1
<input checked="" type="checkbox"/> Seismic/Dead	<input checked="" type="checkbox"/> Seismic/DE	<input checked="" type="checkbox"/> Srs
<input checked="" type="checkbox"/> Tributary Mass	<input checked="" type="checkbox"/> Flex	<input checked="" type="checkbox"/> Dead Load

To: EG/TM/DJP/DOCUMENT LIBRARY G.O.

Analysis No. 8-304 Rev. 4

If change in analysis, list changes made:
If new analysis, list superseded analysis numbers (Seismic and Flex):

... to reflect new boundaries.
...
... supports or support modifications required:

All modifications from 8-304 Rev. 3 are still applicable to 8-304 Rev. 4

Dan Pagan

Approval Date: 8-25-83



611 53

SEP 8 1966

ISSUES NO. 8-304

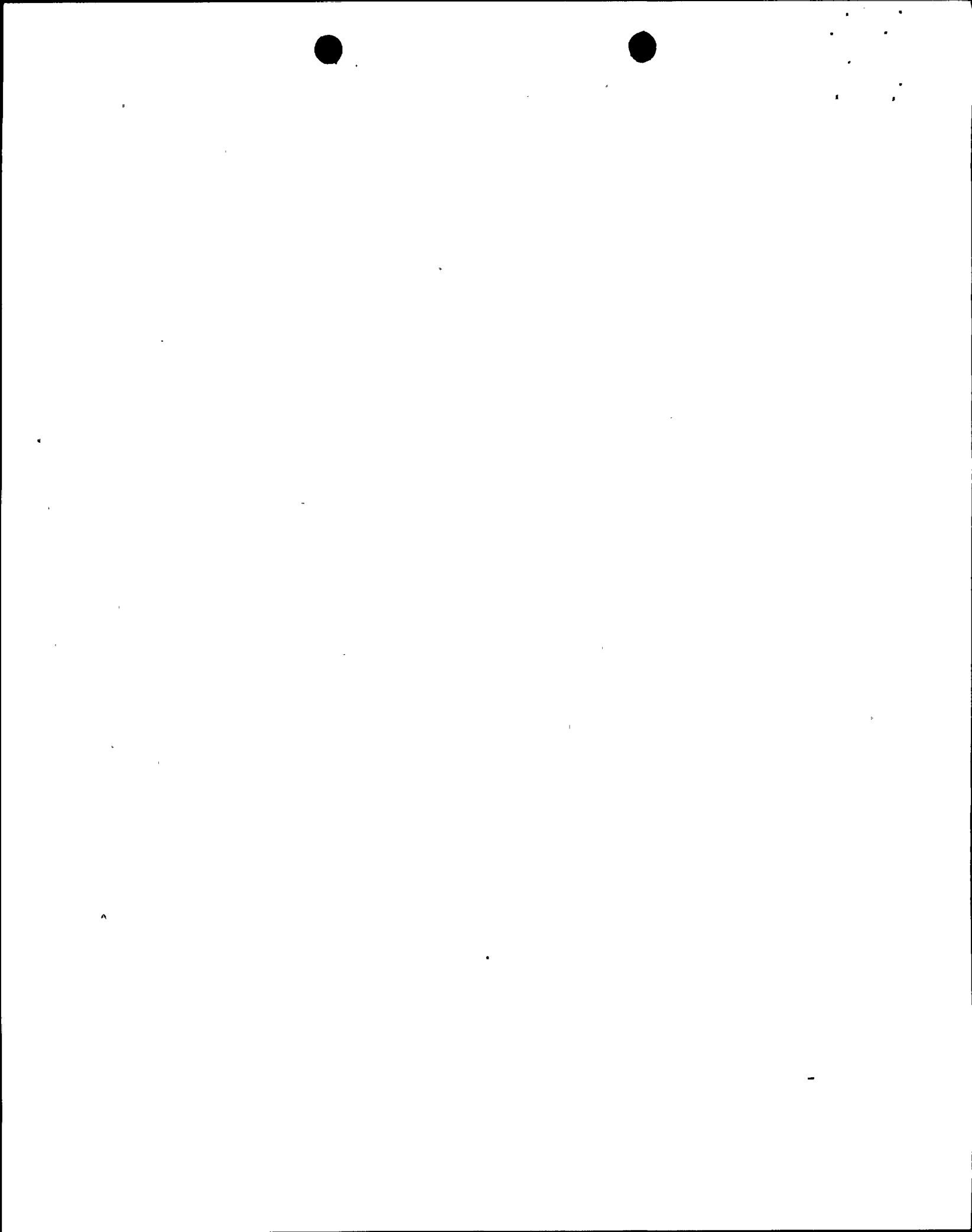
REV. 4

DATE: 8-19-63

ISSUES M. LENOV

CHECKER DEF

NO. S.	COMMENT	RESOLUTION	DATE	
			BY	BY
	<p>ALL COMMENTS from 8-304 Rev. 3 are applicable to 8-304 Rev. 4</p> <p>For supports 63-2 (p. 278) and 63-3 (p. 750) stiffness has been calculated by using STRIBL program. Stiffness for these supports was considered <u>only</u> for thermal case due to the fact that thermal loads on these supports were too high and infinite stiffness which was used before does not reflect the real situation.</p> <p style="text-align: center;">M.L.</p>	<p>Stiffness were used per group leader Mr. Grey's instruction.</p>		



PACIFIC GAS AND ELECTRIC COMPANY
 DESIGN CALCULATION COVER SHEET

Sheet 21

File No. 124-107

Calculation No. F-307 REV 1

Project Diablo Canyon Unit # 1 Date _____

Engineering Discipline M & E

Structure, System, or Component Piping System # 8 (CVCS)

Type of or Purpose of Design Calculations Design Reverification Program

No. of Sheets 21

SAR Change Required Yes No
(Nuclear Projects only)

	Signature	Discipline/Dept.	Date
Preparer	<u>J. T. ...</u>	<u>M & E</u>	<u>8-15-83</u>
Checker	<u>Richard Kling...</u>	<u>M & E</u>	<u>8-16-83</u>
Approval:			
Discipline Engineer <small>(Required)</small>	<u>Dan Pagan</u>	<u>M & E</u>	<u>7-15-83</u>
Group Leader/Supervisor			

Record of Revisions

Rev. No.	Date	Reason for Revision	Rev. by	Checked by	Approval
0	7-15-83	Design Reverification Program	<u>[Signature]</u>	<u>[Signature]</u>	<u>[Signature]</u>
1	7-20-83	INTRODUCTION OF SUPPORT STIFFNESS FOR SUPPORT LOAD REDUCTION.	[Signature]	[Signature]	[Signature]
2	7-20-83	SEE MEMO TO FILE FOR DUALIFICATION OF THE ANALYSIS	[Signature]	[Signature]	[Signature]
1	7-20-83	INTRODUCTION OF SUPPORT STIFFNESS FOR SUPPORT LOAD REDUCTION.	<u>[Signature]</u>	<u>[Signature]</u>	<u>[Signature]</u>

Y



ANALYSIS ISSUE

<u>Y</u>	<u>New Analysis</u>	<u>Change In Analysis</u>	
	Design Review Isometric No.	H8-750 (6-29-83), H8-384 (6-27-83) H8-340 (5-25-82), H8-392 (6-28-83)	D. C. Unit <u>1</u>
<u>X</u>	<u>Seismic/Design</u>	<u>X</u> Seismic/DE	<u>N/A</u> SAH
<u>X</u>	<u>Tributary Mass</u>	<u>X</u> Flex	<u>X</u> Dead Load

To: EG/TM/DJP/DOCUMENT LIBRARY 3.0.

Analysis No. S-307 Rev. 1

If change in analysis, list changes made:
 If new analysis, list superseded analysis numbers (Seismic and Flex):

- 2) This calc reflects a Class-1 design/analysis, although all piping was later downgraded to Class-E in accordance with DCO-E-1314 (See Appendix-C).
- 3) Due to modeling limitations, pipe supports attached at the following node points: 460, 465, 5, 50, 65, 75, 595, 596, 565, 875, and 900, cannot be qualified using loads derived from this (computer) analysis.

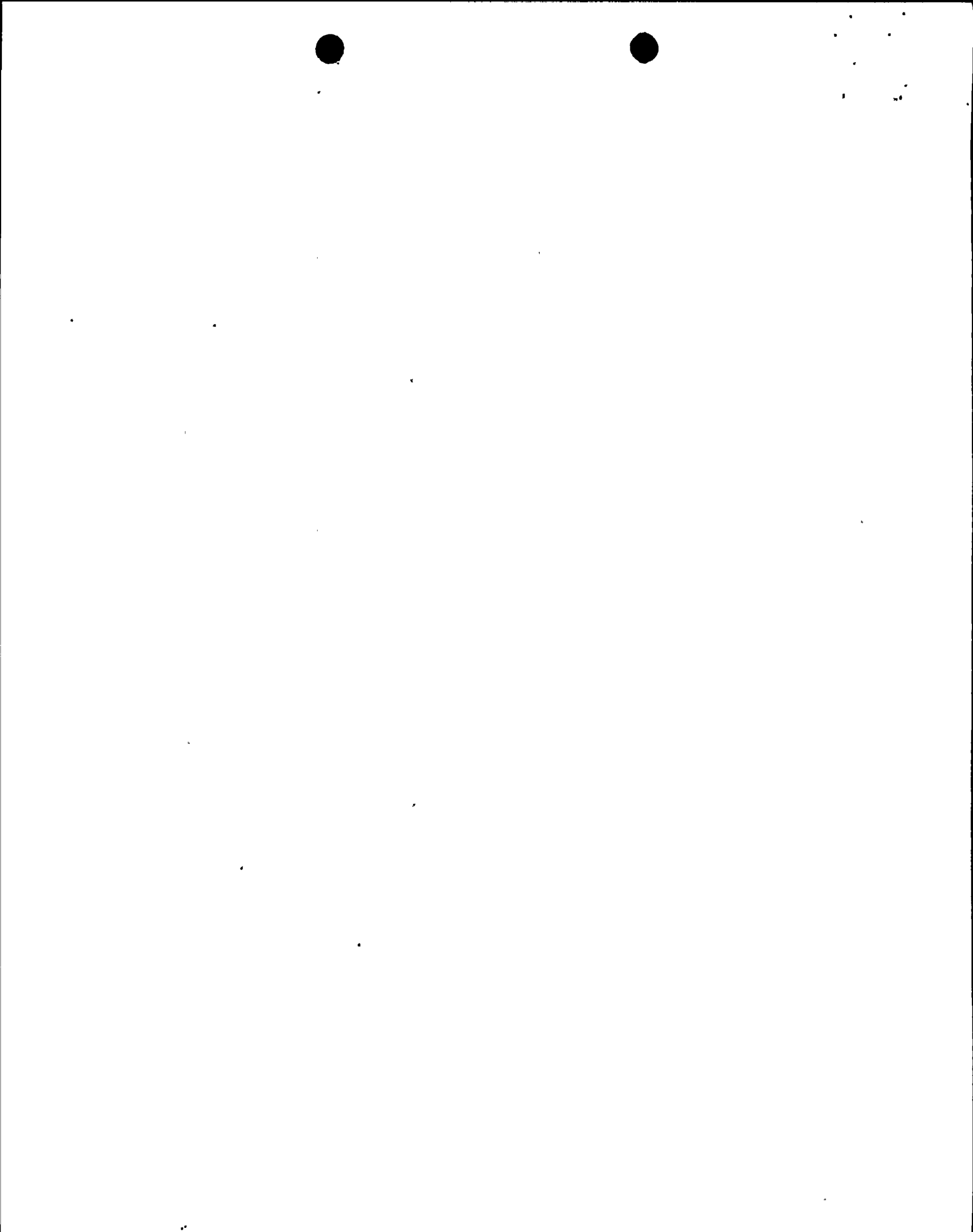
Note: Rev-1 is being issued for the purpose of reducing loads for supports 512-4, 512-2, and 512-3 attached at node points 475, 510, 535, 585 and 516. (Stiffness values were introduced in the model.)

New supports or support modifications required:

<u>DATA POINT</u>	<u>PIPE SUPPORT</u>	<u>REMARKS</u>
5	1509-148	Modify existing bilat to an anchor.
127	New	Install new x-direction restraint (rigid).
140	New (*)	Install new z-direction restraint (rigid).
245	509-132	Modify existing bilat to an anchor.
295	New. (*)	Install new z-direction restraint (rigid).
511	511-3	Modify existing 3-way to a bilat.

(*) Provide 1/16" gap in both directions.










Issued By: _____ Approval Date: _____

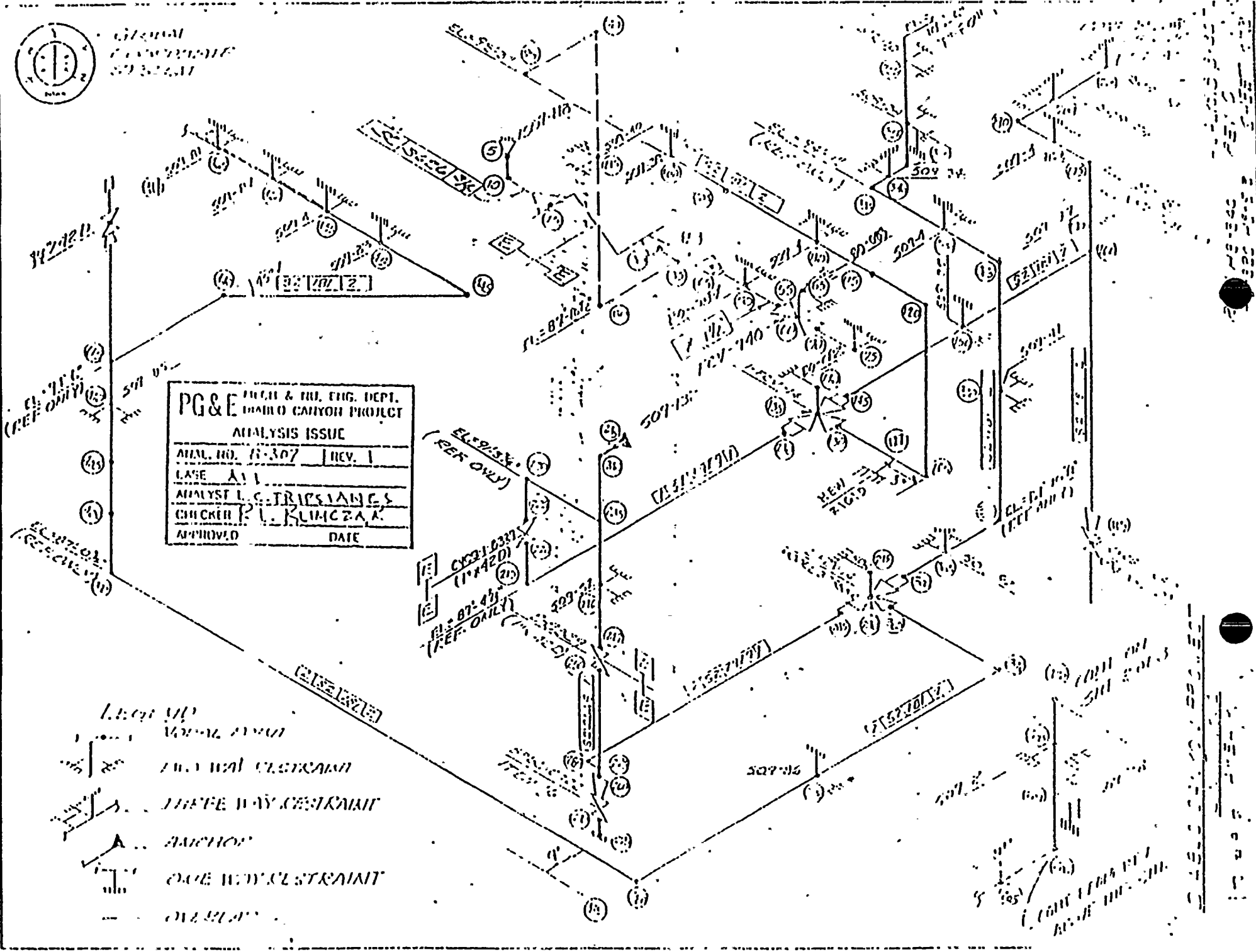




GENERAL
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DRAWING

PG&E ARCH. & M.E. ENGR. DEPT.	
GRAND CANYON PROJECT	
ANALYSIS ISSUE	
ANAL. NO. 11-307	REV. 1
CASE A1	
ANALYST L. C. TRIPSIANES	
CHECKER P. L. KLINCZAK	
APPROVED	DATE

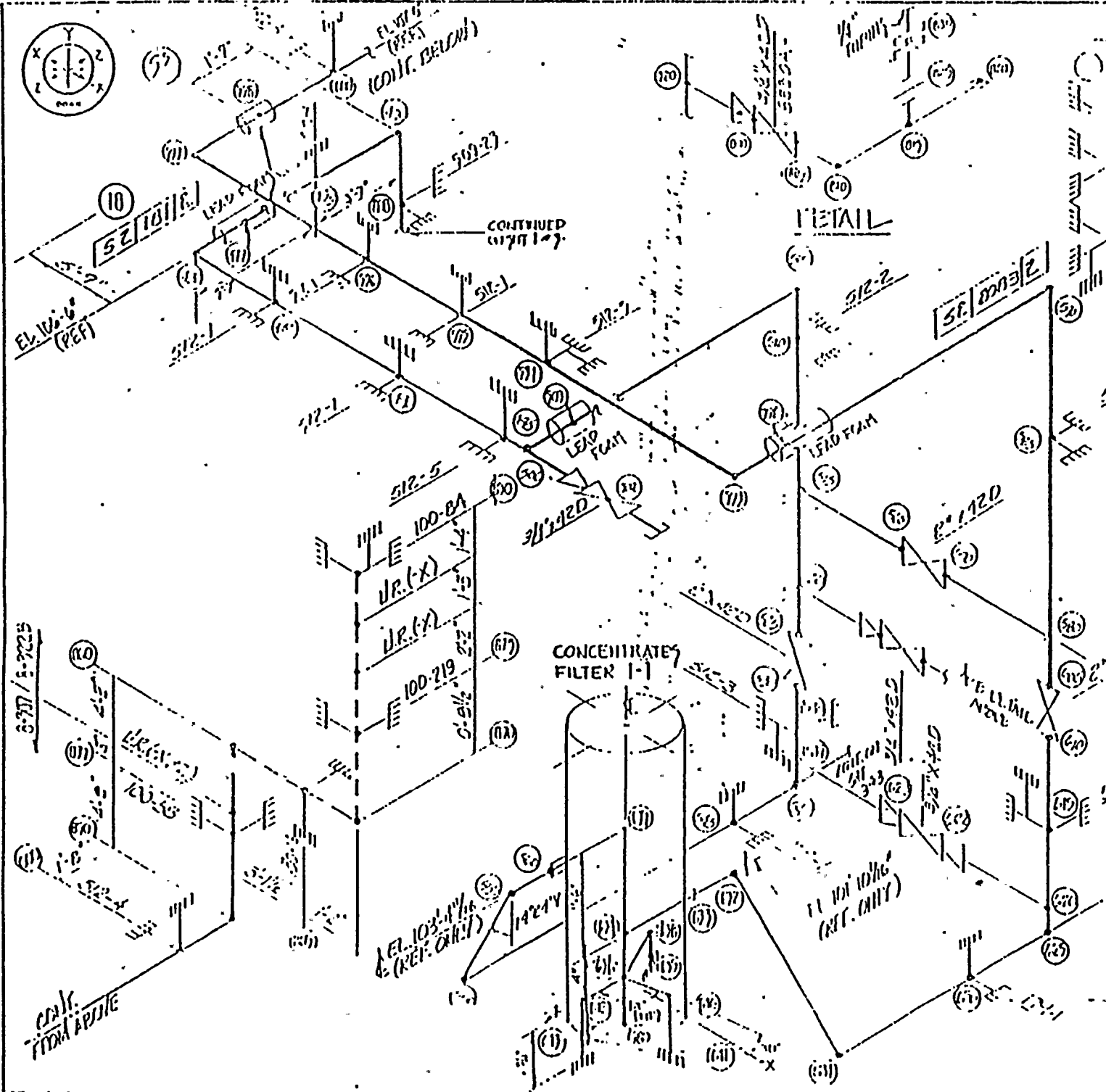
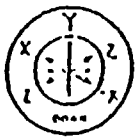
-  LEG. 110
-  LEG. 111
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-  LEG. 118



11-307-118



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U.S. GOVERNMENT	REPORT NUMBER
ANALYSIS NUMBER	DATE
CASE NAME	ANALYST
CHECKER	DATE

NOTE

1. ALL ENTRIES ARE
 2. SECRET MATERIAL

85-76-1402

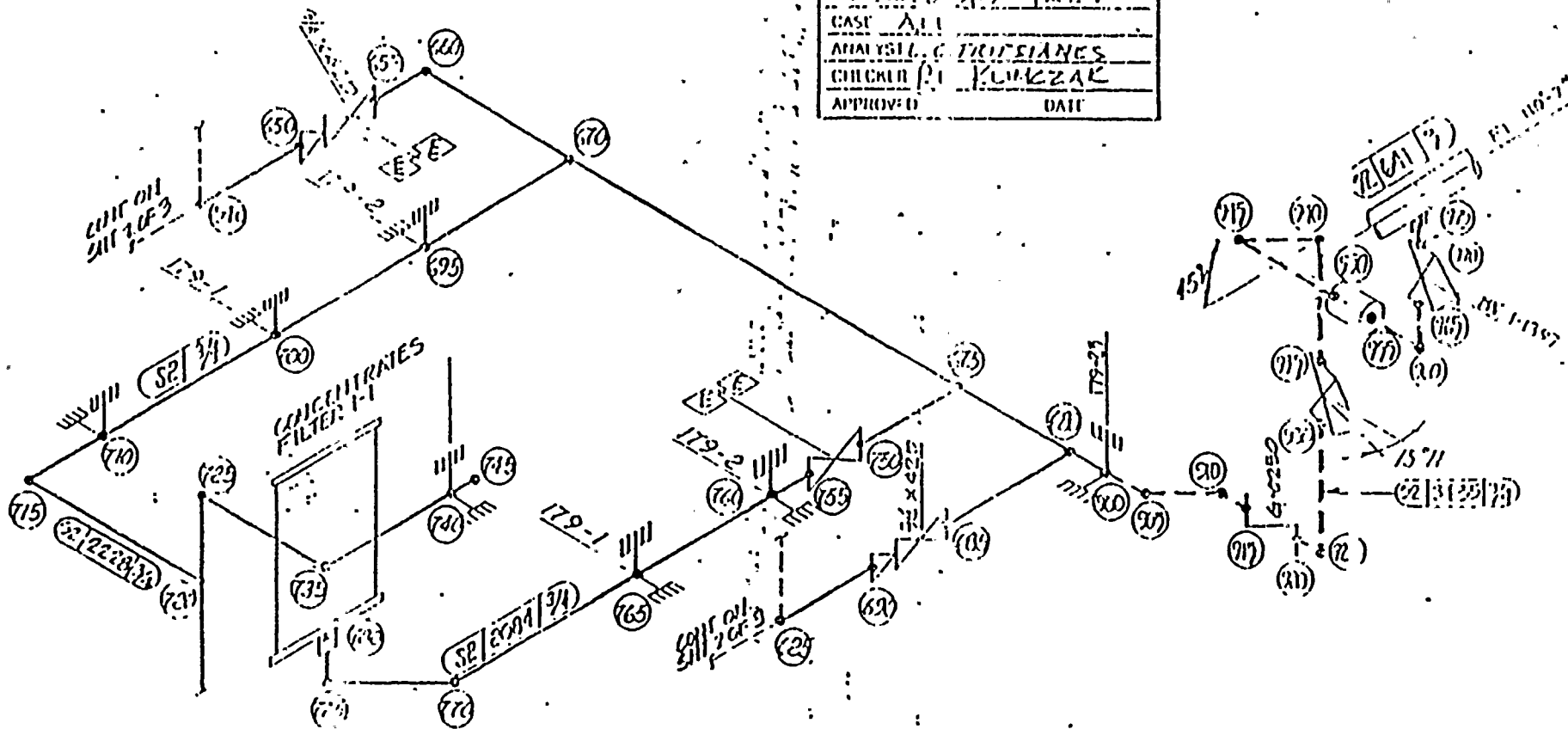


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GLOBAL
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SYSTEM

PG&E	POWER & IND. ENG. DEPT.
DIABLO CANYON PROJECT	
ANALYSIS ISSUE	
ANAL. NO. 8	REV. 1
CASE ALL	
ANALYST G. TRIFIANOS	
CHECKED BY K. KUMAR	
APPROVED	DATE



DIABLO CANYON PROJECT
UNIT ONE AREA K
REF ISO. # 119-392
8-750

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PIPE STRESS SUMMARY CHECK AND COVER SHEET

ANSI-B31.1

PROJECT DIABLO CANYON UNIT #1
 JOB NO. 15321 PLANT DESIGN GROUP
 SYSTEM CHEMICAL AND VOLUME CONTROL
 Dwg NO R-307 ISS NO _____ REV NO _____

DESIGN CONDITION	LEVEL	LOCATION OF MAXIMUM END ELEMENT	MAXIMUM COMPUTED STRESS (PSI)	ALLOWABLE STRESS (PSI)	COMPUTED ALLOWABLE
SUSTAINED LOADS EQU. 11		245 240 245	7081	SH 16375	.432
OCCASIONAL LOADS EQU. 10	B	245 240 245	11126	1.2 SH 19151	.566
OCCASIONAL LOADS EQU. 12	C	291 291 295	12263	1.5 SH 29475	.621
OCCASIONAL LOADS EQU. 10	B	135 135 145	14131	2.4 SH 39315	1.153
THERMAL EXPANSION EQU. 13		445 440 445	12743	SH 27465	.681

REFERENCE CALCULATIONS:

WEIGHT SEISMIC-INERTIA PORTION OTHERS N.A.
 THERMAL EXP SEISMIC-ANCHOR MOVEMENT N.A.
 DYNAMIC _____

NAME SIGNATURE DATE
 PREPARED BY LARRY TRIDIANES [Signature] 5-15-83
 REVIEWED BY RICHARD L. KLINCZAK [Signature] 5-16-83
 APPROVED BY _____

** THIS STRESS FAILURE IS NULLIFIED, SINCE IT OCCURS IN THE YOKE OF THE VALVE - WHICH VALVE IS QUALIFIED



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PROJECT NO. B-307

REV. 1

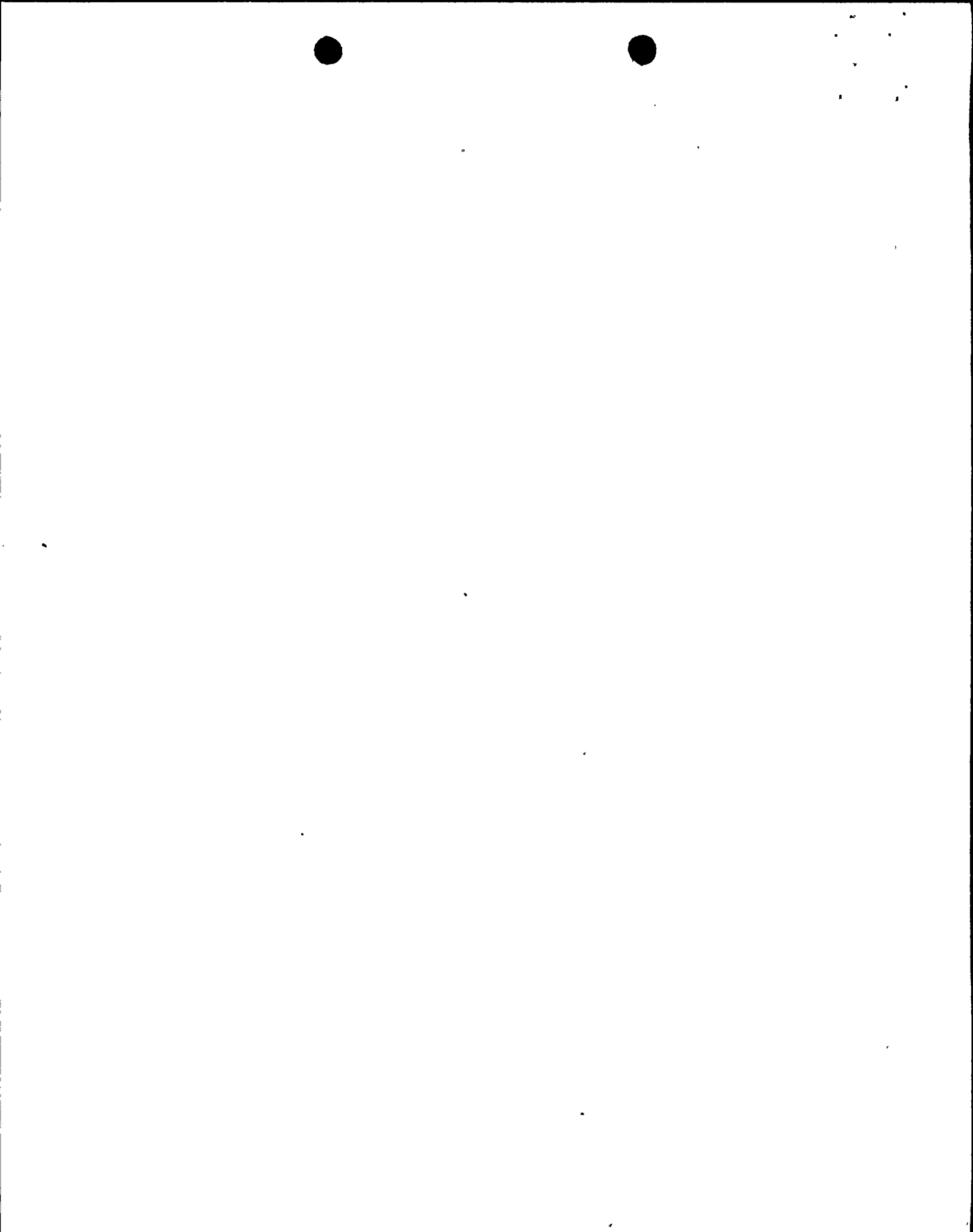
DATE: 8.15-83

PROJECT L.C. TRIPSIANES

DESIGNER RLK

COMMENT NO.	COMMENT	RESOLUTION	CHECKED	
			A	C
1	<p>THE FOLLOWING SUPPORTS NEED BE SUPPLIED FOR LOCAL PIPE STRESS.</p> <p>512-3 → N.PT. 535 512-3 → N.PT. 615 509-92 → N.PT. 305 509-81 → N.PT. 465 509-38 → N.PT. 207 509-34 → N.PT. 200 509-34 → N.PT. 350 509-4 → N.PT. 110</p>	S.F.E.G. GROUP		RLK
2	<p>DUE TO THE PRESENCE OF GAP, SOME OF THE RESTRAINTS (SUCH AS THOSE AT N.PTS. 140, 295, WILL NOT EXPERIENCE THERMAL LOADING - FOR MODELING PURPOSES THESE (RIGID) SUPPORTS WERE MODELED AS SNUBBERS.</p>	NONE REQ'D		RLK
3	<p>RESTRAINTS (RIGID) AT N.PTS. 510, 535, 585, 615 WERE MODELED WITH APPROPRIATE STIFFNESS PROPERTIES. (SEE APPENDIX-C FOR CALCULATIONS).</p>	NONE REQ'D		RLK

Column C is to be used by checker. Column A is to be used by Analyst.



ANALYSIS NO. 8-707 REV. 1 DTD: 8-15-83

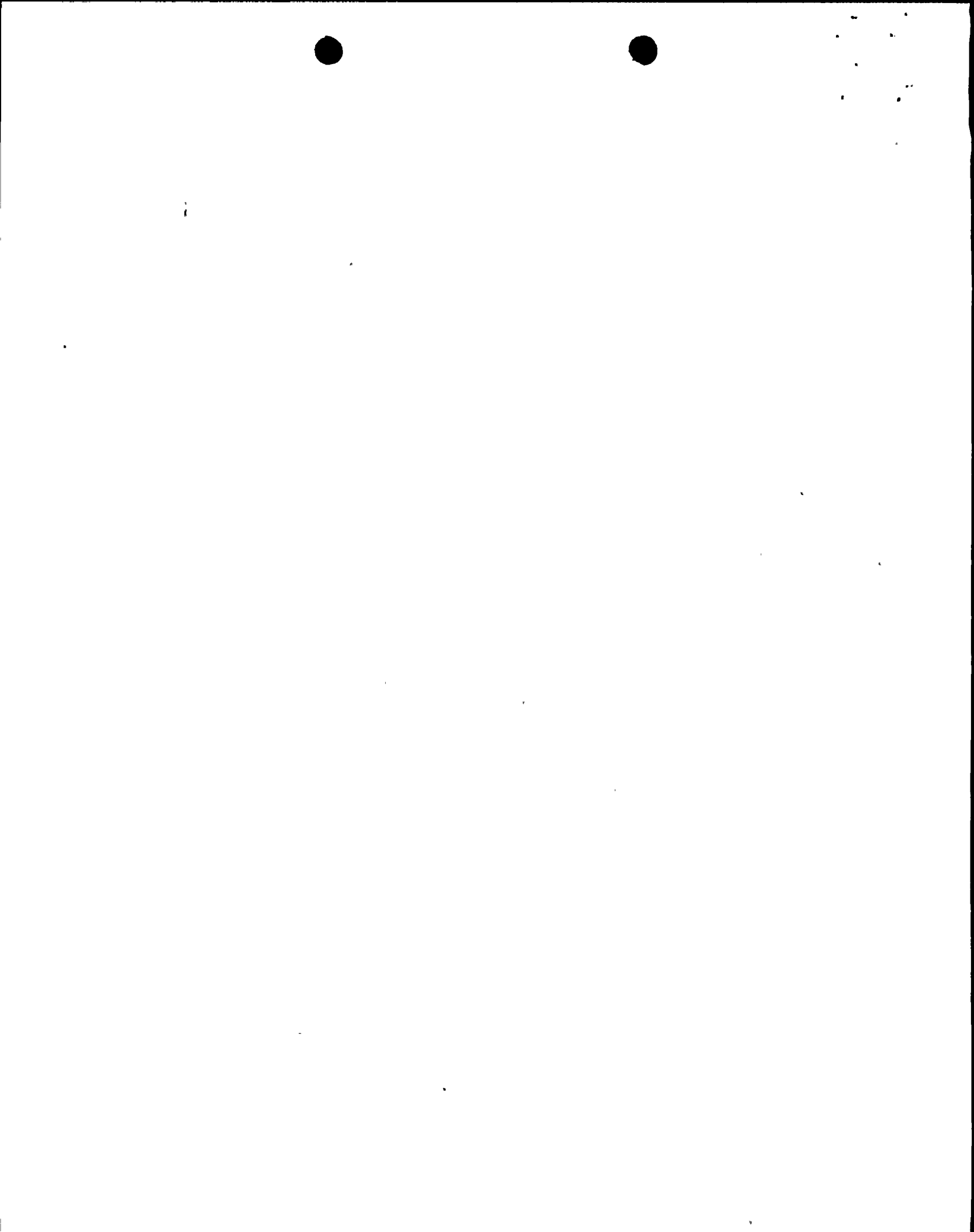
ANALYST L.C. TRIPCIANES ~~CHECKER~~ RICHARD KLIMCZAK

COMMENT NO.	COMMENT	RESOLUTION	DATE	
			A	C
4	<p>IN THE CASE OF AN INSULATED PIPE, GOING THROUGH A LEAD FOAM PENETRATION (SEE N.P.T.S 503, 577, 502 & 598 OF STRESS LOG), WHERE THERMAL EXPANSION MOVEMENTS ARE CONTROLLED AND OF SMALL MAGNITUDE, NO RESTRAINTS WOULD BE CONSIDERED IN THE 'FLEXIBILITY' PORTION OF THE ANALYSIS. FOR THE DYNAMIC PORTION OF THE ANALYSIS A RESTRAINT WAS MODELED AT THE MIDPOINT OF THE LEAD FOAM PENETRATION.</p> <p style="text-align: right;">RKM 2-11-93</p>	<p>REQUIRES APPROVAL OF SUPERVISOR.</p>		

LCT:RLK

OK

Column C is to be used by checker. Column A is to be used by Analyst.



INTEROFFICE MEMORANDUM

Diablo Canyon Project



PACIFIC GAS AND ELECTRIC COMPANY
BECHTEL POWER CORPORATION

To: R. Gray
From: Leo Mangoba
CC: Onsite Project Engineering Group
At: Jobsite Extension 3067

Date: August 4, 1985
File No. 925
Subject: Calculated stiffness of supports

Per your request, we calculated stiffness of supports:

1512-2		1512-3	
$K_y = 1536 \text{ lb./in.}$		$K_{yy} = 2625 \text{ lb./in.}$	$K_{yy} = 6706 \text{ lb./in.}$
$K_z = 896 \text{ lb./in.}$		$K_y = 1536 \text{ lb./in.}$	$K_x = 7136 \text{ lb./in.}$
		$K_z = 896 \text{ lb./in.}$	$K_z = 3362 \text{ lb./in.}$

Thank you,

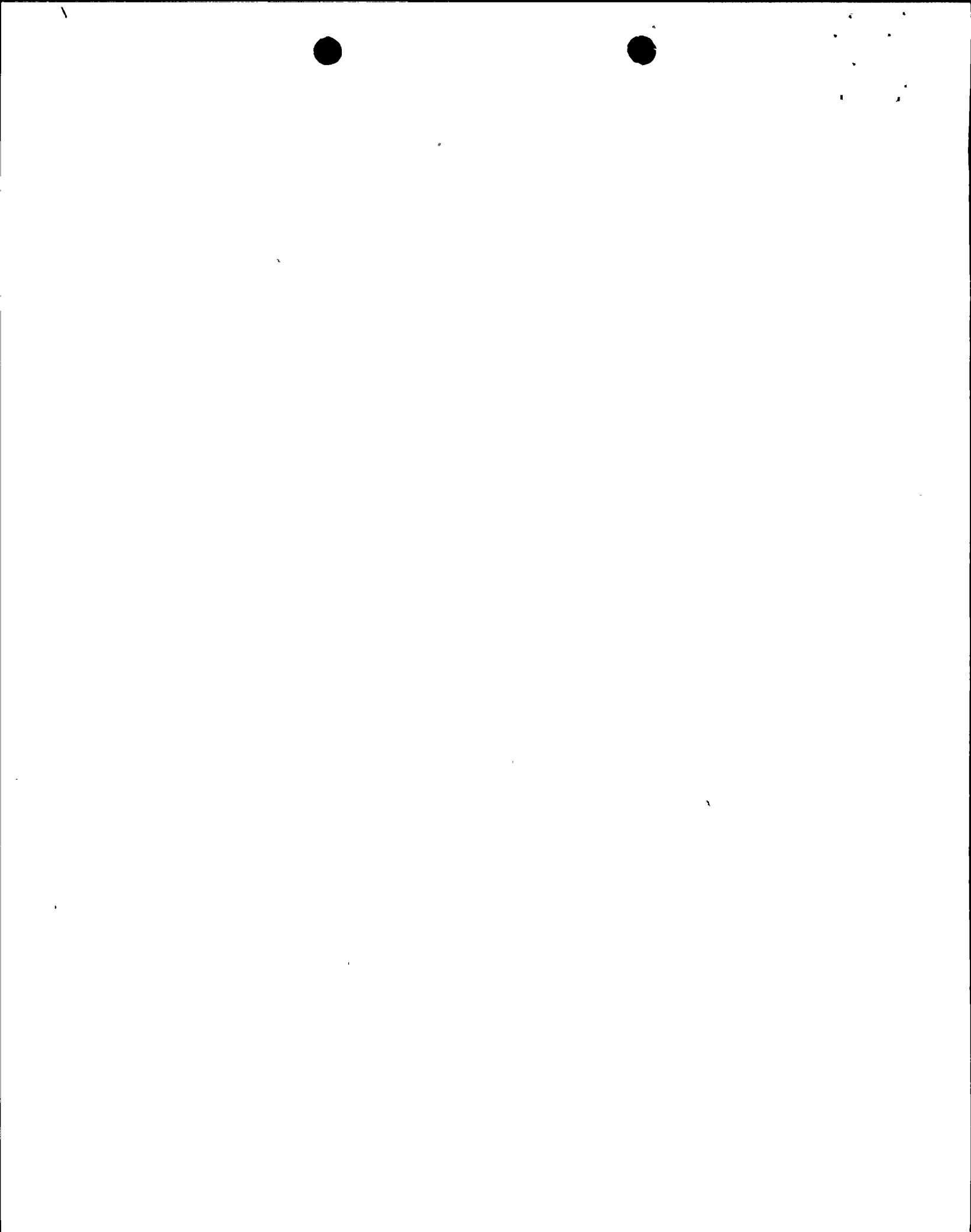

Leo Mangoba

L. Mangoba/ jfb

Reply Requested: No

Attachments: None

cc: P. Schurrer
A. Sruostaman



ANALYSIS ISSUE

FS New Analysis _____ Change in Analysis _____
 Design Review Isometric No. (76-14) HE-370, HE-300, HE-384 D. C. Unit: 1
ES Seismic/Hogri YES Seismic/DE NO SAM
ES Tributary Mass YES Flex YES Dead Load

By: EG/TM/DJP/DOCUMENT LIBRARY G.O.

Analysis No. E-522B Rev. D

If change in analysis, list changes made:
 If new analysis, list superseded analysis numbers (Seismic and Flex):

ANALYSIS E-522B WAS QUALIFIED ONLY TO EQUATIONS
 11 AND 13 OR 14. SEISMIC LOADING WAS ONLY CONSIDERED
 FOR SPECIFIC SAMPLE SUPPORT QUALIFICATION:

Are supports or support modifications required:

1983

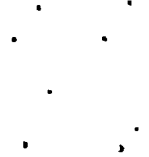
FOR INFORMATION ONLY

NONE REQUIRED

RESTRAINT 100-72 DATA POINT 195 EXCEEDS RECOMMENDED SUPPORT LOADS.

Used by: Dawn Pagan Approval Date: 8-15-83

(15)



NO. P-1225 REV. 0 DATE: 8-13-83

BY: BLANK MAHNCKE CHECKER: M. LENCU

OWNER	DESCRIPTION	REV.
<p>THE PIPE PASSING THROUGH THE WALL PENETRATION IN ANALYSIS P-1225 IS INSULATED WITH INSULATION PLATE AND SEALED WITH GROUT. EACH SUCH PENETRATION WAS MODELED AS A BILATERAL RESTRAINT AT THE MID-POINT OF THE FOOTPRINT FOR SEISMIC ANALYSIS. NO RESTRAINT WAS MODELED AT THIS POINT FOR FLEXIBILITY ANALYSIS.</p> <p style="text-align: right;">M. LENCU 8-23-83</p>	<p>MODELED PER GROUP LEADER'S INSTRUCTIONS</p>	<p>1</p>

SEP 7 1983

FOR APPROVAL



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ANALYSIS NO. E-322C REV. 0 DATE: E-322CANALYST ELLEN MAHNCKECHECKER Jim J. J. J.

COMMENT NO.	COMMENT	RECOMMENDATION	STATUS	
			A	C
1.	ANALYSIS ONLY QUALIFIED TO EQUATIONS 11 AND 13 OR 14.	NONE REQUIRED	AJM	ML
2.	ALL LOCAL STRESS CALCULATIONS PERFORMED AT S.F.G.O.	NONE REQUIRED	AJM	ML
3.	THE PIPING PASSING THROUGH THE WALL PENETRATION IN ANALYSIS E-322C IS INSULATED WITH CALCIUM SILICATE AND SEALED WITH HARD LEAD FOAM. EACH SUCH PENETRATION WAS MODELED AS A BILATERAL RESTRAINT AT THE MIDPOINT OF THE PENETRATION FOR STRESS ANALYSIS. NO RESTRAINT WAS MODELED AT THESE POINTS FOR FLEXIBILITY ANALYSIS.	NONE REQUIRED	AJM	ML

RM
3-1-83

*Column C is to be used by checker. Column A is to be used by Analyst.



ANALYSIS NO. 8-301 REV. 3 DATE: 8-15-93

ANALYST ALLEN J. MAHNCKE CHECKER VELLY J. CAZZAN

COMMENT NO.	COMMENT	RESOLUTION	STATUS	
			A*	C
1.	INSULATION WEIGHTS USED IN ANALYSIS ARE HIGHER THAN ACTUAL.	NONE REQUIRED	AJM	
2.	SOIL AND TANK STIFFNESS ACCOUNTED FOR BY EQUIPMENT SAM MOVEMENTS.	NONE REQUIRED	AJM	
3.	LOCAL STRESS CALCULATIONS ARE PERFORMED AT SAN FRANCISCO GENERAL OFFICE.	NONE REQUIRED	AJM	
4.	THE PIPING PASSING THROUGH THE WALL PENETRATIONS IN ANALYSIS 8-301 (DATA PG. 905) IS INSULATED WITH CALCIUM SILICATE AND SEALS WITH HARD LEAD FOAM. EACH PENETRATION WAS MODELED AS LATERAL RESTRAINT AT THE MIDPOINT OF THE PENETRATION FOR SEISMIC ANALYSIS. NO RESTRAINT WAS MODELED AT THESE POINTS FOR FLEXIBILITY ANALYSIS.	NONE REQUIRED (MODELING PER PIPE STRESS GROUP LEADER)	AJM	

*Column C is to be used by checker. Column A is to be used by Analyst.



ANALYSIS ISSUE

YES New Analysis Change In Analysis

Design Review Isometric No. (79-148) HE-370, HE-721, HE-722 D. C. Unit 1

YES Seismic/Hogri YES Seismic/DE SAM

YES Tributary Mass YES Flex YES Dead Load

To: EG/TM/DJP/DOCUMENT LIBRARY G.O.

Analysis No. 8-322A Rev. 0

∴ change in analysis, list changes made:

∴ new analysis, list superseded analysis numbers (Seismic and Flex):

ANALYSIS 8-322A HAS ONLY QUALIFIED TO EQUATIONS 11 AND 13 OR 14. SEISMIC LOADING WAS ONLY CONSIDERED FOR SPECIFIC SAMPLE SUPPORT QUALIFICATIONS.

B

FOR INFORMATION ONLY

New supports or support modifications required:

SEP 7 1983

- NONE REQUIRED
- SUPPORTS ABOVE RECOMMEND LOAD.
- L) 950-148 (NODE 855) C) 950-149 (NODE 810)
- L) 950-25 (NODE 830)

Issued By: Dan Pegan Approval Date: 3-29-83



NO. B-322 A REV. 0 DATE: 8-13-83

BY J. M. [unclear] CHECKED M. LENOV

COMMENT	RESOLUTION	DATE	
		A	C
THE PIPING PASSING THROUGH PENETRATION 563 IN ANALYSIS IS INSULATED WITH INSULATION AND SEALED WITH THE FRAME. PENETRATION WAS GROUP LEADERS CALLED AS A DILATORIAL RESTRAINT. INSTRUCTIONS FOR SEISMIC ANALYSIS. NO RESTRAINT WAS MODELED AT THE MID POINT OF PENETRATION FOR FLEXIBILITY POINTS.	NONE REQUIRED	AJM	MML

B

FOR INFORMATION ONLY

SEP 7 1983

Column A is to be used by checker. Column C is to be used by Analyst.



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UNITED STATES NUCLEAR REGULATORY COMMISSION
 ENGINEERING DEPARTMENT
 DESIGN CALCULATION COVER SHEETS

File No. 10-143
 Calculation No. 12-300

Project WASHO CANYON TOWN PLANT Date 11-10-82

Engineering Discipline MECHANICAL

Structure, System, or Component PIPING SYS # 12

Type of or Purpose of Design Calculations VERIFY PIPING STRESS AND PIPE SUPPORT LOADS FOR THE REVERIFICATION PROGRAM

No. of Sheets 10-3A BC D E F G H I J K L M N O P Q R

Are changes required Yes No
 (Nuclear Projects only)

	Signature	Discipline/Dept.	Date
Preparer	<u>R.M. GARAY</u>	<u>M E N E</u>	<u>10-9-82</u>
Checker	<u>J. CHEN</u>	<u>M E N E</u>	<u>11-5-82</u>
Approval:			
Discipline Engineer (if required)			
Group Leader/Supervisor	<u>[Signature]</u>	<u>M E N E</u>	<u>11-6-82</u>

History of Revisions

Rev. No.	Date	Reason for Revision	Rev. By	Checked By	Approved By
0	10-9-82	ORIGINAL ISSUE	RG	JL	MD
1	10-20-82	SPECTRA, SAM/TAMS + RENUMBERED.	ML	WHA	DJP
2	6-30-82	Final per spectra letter	ML	WHA	DJP
1	10-11-82	M Tractor S-117 Rev. 3, BCH-146 Rev. 3			
2	10-11-82	SAM/TAM Librarian: B-103 Rev. 3			
3	10-11-82	S-117 Rev. 3, S-113 Rev. 2			
3	2-16-83	Update SAM/TAM data with LIB changes 2-117 Rev. 4	MML	WHA	DJP



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AM

12-300 Rev 1

13 3-03 Rev 3

ANC 5 $\Delta X = .116, \Delta Y = .002, \Delta Z = .005$ (DE)
 $\Delta X = .219, \Delta Y = .005, \Delta Z = .133$ (DD)
 $\Delta X = .206, \Delta Y = .023, \Delta Z = .122$ (HS)

ANC 40 $\Delta X = .096, \Delta Y = .002, \Delta Z = .056$ (D)
 $\Delta X = .132, \Delta Y = .004, \Delta Z = .109$ (DD)
 $\Delta X = .140, \Delta Y = .003, \Delta Z = .036$ (HS)

L/B 0 110 Rev 2

ANC 85 $\Delta X = .170, \Delta Y = .035, \Delta Z = .144$ (DE)
 $\Delta X = .340, \Delta Y = .07, \Delta Z = .283$ (DD)
 $\Delta X = .246, \Delta Y = .152, \Delta Z = .06$ (HS)

ANC 260 $\Delta X = .172, \Delta Y = .034, \Delta Z = .143$ (DE)
 $\Delta X = .344, \Delta Y = .068, \Delta Z = .286$ (DD)
 $\Delta X = .242, \Delta Y = .152, \Delta Z = .200$ (HS)

L/B 3-117 Rev 3

ANC 125 $\Delta X = .219, \Delta Y = .114, \Delta Z = .145$ (DE)
 $\Delta X = .433, \Delta Y = .228, \Delta Z = .290$ (DD)
 $\Delta X = .219, \Delta Y = .114, \Delta Z = .145$ (HS)

ANC 2 $\Delta X = .167, \Delta Y = .107, \Delta Z = .11$ (DE)
 $\Delta X = .333, \Delta Y = .214, \Delta Z = .223$ (DD)
 $\Delta X = .242, \Delta Y = .152, \Delta Z = .177$ (HS)

Geometry Minor errors in geometry have been found in analysis 12-300 Rev 1. These errors will not effect the analysis significantly.

Hot loads instead of cold loads and wrong stiffnesses of springs being used in the analysis. This will not effect analysis significantly.

AM/TAM: Old SAM/TAM and new SAM/TAM values have been compared (see the attached sheets) and it is concluded that minor differences will not effect the analysis. In majority of cases old values are bigger than new.

Thermal modes: The old thermal modes and new thermal modes have been compared (see the attached sheet). It is found out that two thermal modes: one normal with $T = 250^\circ F$ up to 10 dia AT L/B 29:



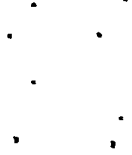
and 5, 6, 25 8 and one coefficient with $T = 40^{\circ}F$ have not been considered in the analysis but this will not affect the analysis significantly.

Rigidity of the system. Z.P.A. and modes have been reviewed and found out that system is rigid dynamically. Thrust load due to SPV was not considered in the analysis.

The fluid is water instead of steam or gas and pressure is low therefore it will not affect the analysis significantly.

Response spectra. Old spectra is valid (letter from M. Tresler, 5/25/53)

Conclusion. The above mentioned changes will not affect the analysis significantly therefore problem 12-300 Rev. I is not to be rerun.



PACIFIC GAS AND ELECTRIC COMPANY
DESIGN CALCULATION COVER SHEET

File No.

Calculation No. 2-2-2

Project DIABLO CANYON UNIT #1 Date 7-13-83

Engineering Discipline MECH. and NUCLEAR ENGR.

Structure, System, or Component PIPING SYSTEM #3 - FEEDWATER
LINE NO. 2477

Type of or Purpose of Design Calculations DESIGN REVERIFICATION PROGRAM -
ANALYSIS REQUESTED BY PIPE SUPPORT GROUP.
FOR CODE BREAK ANALYSIS.

No. of Sheets 21 - PA 33 30 - 31 32 33 34 35 36 37 38 39 40 41 42 43 44 45 46 47 48 49 50

SAF Change Required Yes No
(Nuclear Projects only)

	Signature	Discipline/Dept.	Date
Preparer	<i>W. J. Jordan</i>	ME NE	7-13-83
Checker	<i>W. J. Jordan</i>	ME NE	7-14-83
Approval:			
Discipline Engineer <small>(if required)</small>			
Group Leader/Supervisor	<i>P. TAM</i>	ME NE	7-15-83

Record of Revisions

Rev. No.	Date	Reason for Revision	Checked		Approved	
			by	by	Engr.	GL/Supr.
0	7-13-83	REVISION FOR DESIGN REVERIFICATION PROGRAM FOR CODE BREAK ANALYSIS		WJM		
		FOR CODE BREAK ANALYSIS				
	7-13-83	Final Issue				

G



ANALYSIS ISSUE

 New Analysis ✓ Change In Analysis
Design Review Isometric No. (79-14) AE-2/2 D. C. Unit /
 ✓ Seismic/Eosgt ✓ Seismic/DE — SAM
 ✓ Tributary Mass ✓ Flex ✓ Dead Load

To: EG/TM/DJP/DOCUMENT LIBRARY G.C.

Analysis No. E-2/2 Rev. /

If change in analysis, list changes made:
If new analysis, list superseded analysis numbers (Seismic and Flex):

E-2/2 Rev 0

New supports or support modifications required:

- (YE)
- 1. The existing stat. 2105-22 @ DP 75 is still valid*
 - 2. The existing restraint 2105-15 @ DP 50 is still valid*

Issued By:

R R Green

Approval Date:

2-12-95



ANALYSIS ISSUE

New Analysis Change In Analysis

Design Review Isometric No. (79-14) 113-212

D. C. Vaid

Seismic/Hogri Seismic/DE ~~NOT REQUIRED~~ SAM

Tributary Mass Flex Dead Load

To: EG/TM/DJP/DOCUMENT LIBRARY G.O.

Analysis No. 3-313 Rev. 0

If change in analysis, list changes made:
If new analysis, list superseded analysis numbers (Seismic and Flex):

NEW CODE BREAK ANALYSIS SUPERSEDES PREVIOUS CALCULATION BY THE PIPE SUPPORT GROUP.

*NOTE: FOR SEISMIC ANALYSIS AND TRIBUTARY WEIGHT THIS ANALYSIS IS ONLY VALID BETWEEN NODES 5 AND 75.

New supports or support modifications required:

1) MODIFY EXISTING BILAT 2165-32 AT NODE 75 TO A CODE BREAK ANCHOR.

2) REMOVE SUPPORT 2165-15 AT NODE 50 TO ELIMINATE THERMAL OVERSTRESS.

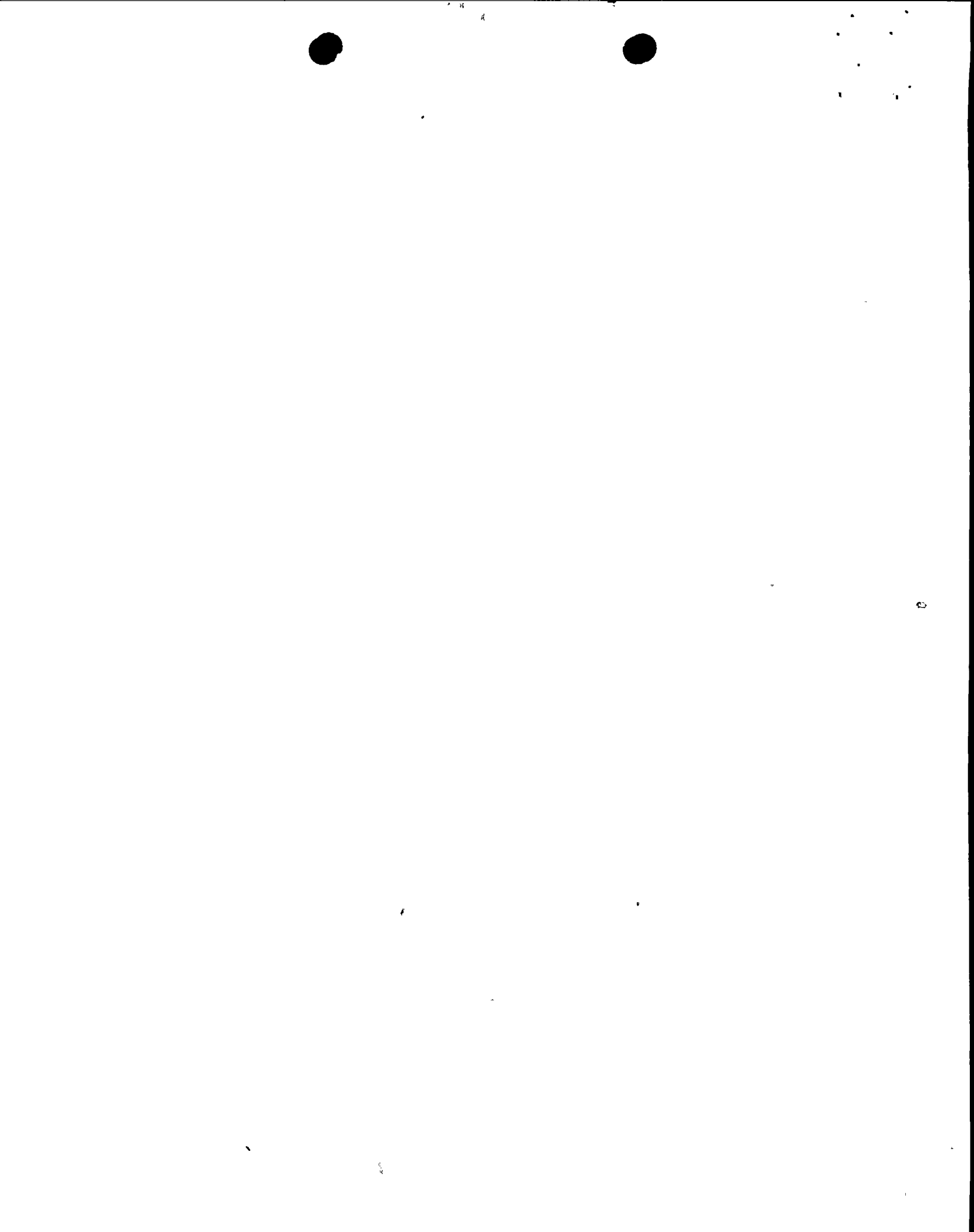
SUPERSEDED
BY REV 1 OF CALC 3-313
DATE 5/15/83 BY P. TAN
CHKD 1/15/83 APP'D

Issued By:

Approval Date: 7-15-83

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The hangers shown on the pipe stress isometric have been qualified for loads by N-49 calc number 3-303-H which are substantially greater than these loads.

The purpose of this analysis is to qualify the stress induced in the piping because of thermal, dead load, seismic, and GMI/TMI effects.

The boundary between the seismic class I and the non-seismic class E piping is the downstream side of the check valve (data point 25).

Structurally, the model was terminated at hanger number 2165-31, data point 85. The hangers beyond this point are dead load supports and cannot be considered in a seismic analysis. At data point 85, a mass of three seismic spans of piping were added to simulate the effects of the remaining piping in the system. For the purpose of performing a stress analysis of this code break, this model is more than adequate.

Reviewing the calculated stress reveals the following:

- a) For the static analysis (Eq 11) the stress in the pipe is well below code allowable in both the class I and class E sections.
- b) For the thermal expansion analyses (Eq 13/14) the stress in the piping is well below code allowable in both class I and class E sections.
- c) For the Design Earthquake (D E) seismic analysis (Eq 12/1.2 Sn) the stress in the pipe in the Class I section is well below the code allowable (1.2 Sn). In the class E section, the stress in the pipe does not exceed code allowable until data point 55, which is downstream of the second isolation valve. Note that for all points in the class E section, the calculated stress does not exceed the Hosgri allowable stress (2.4 Sn) implying that plastic deformation does not occur.

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d) For the Double Design Earthquake ... in the class I section of the pipe are well below the code allowable. In the class E section of the pipe, the code allowable stress, 1.0 Sn, is exceeded at data point 35. However, this stress does not exceed the Hsgrl allowable stress. The Hsgrl allowable stress (2.4Sn) is not exceeded until data point 35, which is downstream of the second isolation valve.

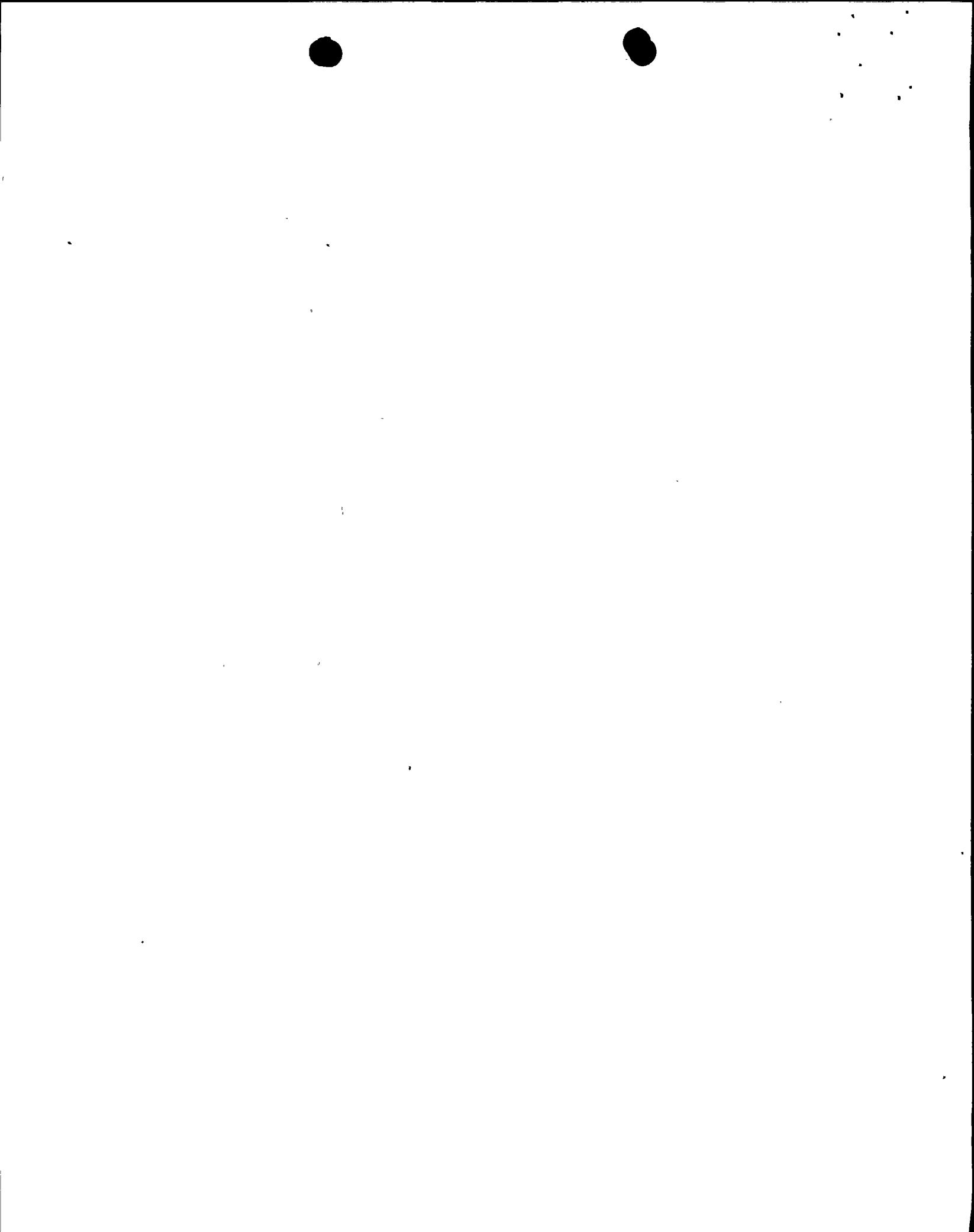
e) For the Hsgrl (HS) earthquake, the calculated stress in the class I section is well below code allowable (2.4 Sn). In the class E section, the code allowable is not exceeded until data point 45 which is on the downstream side of the second isolation valve.

EXAMINATION:

The lateral seismic movements calculated at data point 85 are $\Delta X(DE)=2.817''$, $\Delta X(DDE)=5.633''$, and $\Delta X(HS)=8.162''$. The drawing shows the existence of a wall penetration 2'-10 1/2" from data point 85. This wall penetration will restrict the seismic deflection of the pipe to 5-5/8" laterally at any point in the penetration. Because of this penetration, the maximum deflection at data point 85 is expected to be approximately 2-1/8" which is comparable to the "DE" case where stress would be proportional to deflection. Clearly, it can be concluded that the seismic stresses in the piping will be significantly less than those calculated.

Based on this analysis, it is concluded that the criteria for a code break has been met.

5



BACKGROUND INFORMATION

ANALYSIS E-313 WAS INITIALLY REQUESTED BY THE PIPE SUPPORT GROUP TO CERTIFICATE THAT STRESSES IN THE REGION BETWEEN SUPPORT RIGS 32 AND LINE RIG 574 (NODE 5-75) WERE ACCEPTABLE. THE ANALYSIS BOUNDARY WAS EXTENDED TO LINE RIG 557 TO INSURE THAT SELF-WEIGHT EXCITATION CONTRIBUTIONS FROM THE NON-SEISMICALLY SUPPORTED PORTION OF LINE 2477 WERE INCLUDED. THE RESULTS OF THIS ANALYSIS SHOWED THAT PIPING IN THE REGION OF CONCERN (NODE 5-75) WAS SEISMICALLY OVERSTRESSED (100% HBS PSI @ NODE 5). THE REGION WAS ALSO THERMALLY OVERSTRESSED AT NODE 5 (RATIO = 1.005 - EXCEEDED). THE FUNDAMENTAL PROBLEM WAS TO REDUCE SEISMIC STRESSES WHICH

WAS ACCOMPLISHED BY SUPERSEDED THE BREAK ANCHOR AT THE LOCATION FOR RIGS 32 (EXISTING) - NODE

DATE 9-12-83 BY P. TAN

FOR THE PURPOSES OF THIS ANALYSIS, THE ANALYSIS WAS TERMINATED AT NODE 75. HOWEVER, THERMAL ANCHOR MOVEMENTS AT NODE 135 ARE SUBSTANTIAL AND THE NON-SEISMICALLY SUPPORTED PIPING BEYOND NODE 75 WAS RETAINED IN THE ANALYSIS TO SUBSTANTIATE THAT THE ADDITION OF THE CODE BREAK ANCHOR WOULD NOT CAUSE THERMAL OVERSTRESS IN THE CLASS PIPING. SEISMIC STRESSES AND LOADS IN THE REGION OF NODE 75 TO 135 ARE THEREFORE NOT NOT APPLICABLE TO THIS ANALYSIS.

ANALYSIS ASSUMPTIONS:

- 1) FOR THERMAL ANCHOR MOVEMENTS AT NODE 135 THE DISPLACEMENTS WERE ASSUMED TO BE EQUAL TO THE DISPLACEMENTS AT NODE 75. (FLX ANALYSES ARE NOT AVAILABLE ON SITE; PER THE PIPE STRESS GROUP LEADER P. SNEY, THIS ANALYSIS



GENERAL COMPUTATION SHEET

JOB NO.
FILE NO.
LOCATION

SECTION
CODE BREAK ANALYSIS
PAGE BY DATE CHECKED BY APPROVED BY

IS STILL CONSIDERED FINAL SINCE THESE THINGS AT THIS POINT
WILL NOT AFFECT THE CODE EFFECTS.

BY CORRELATION OF THERMAL ANCHOR MOVEMENTS

FOR NODE POINT 5 - ANALYSIS 2-120 USES 3 THERMAL
MODES AS FOLLOWS:

- 1. THERM1 = 110°F
- 2. THERM2 = 37°F
- 3. THERM3 = 120°F

ANALYSIS 3-313 UTILIZES 2 THERMAL MODES
AS FOLLOWS:

- 1. THERM1 = 37°F NODES 20-25, 110°F NODES 20-135
- 2. THERM2 = 37°F NODES 5-6, 135

THERM1 CORRELATES WITH THERM2 FOR THERM2
USE THE MAXIMUM ENVELOPE OF DISPLACEMENTS
ROTATIONS OF THERM1 & THERM2.

MODELING

REFER TO THE NEXT SHEET

6

SUPERSEDED			
BY REV	<u>1</u>	OF CALC	<u>3-313</u>
DATE	<u>9-12-32</u>	BY	<u>P. TAM</u>
CHKD	<u>M.E.P.</u>	APP'D	<u>R.P.</u>



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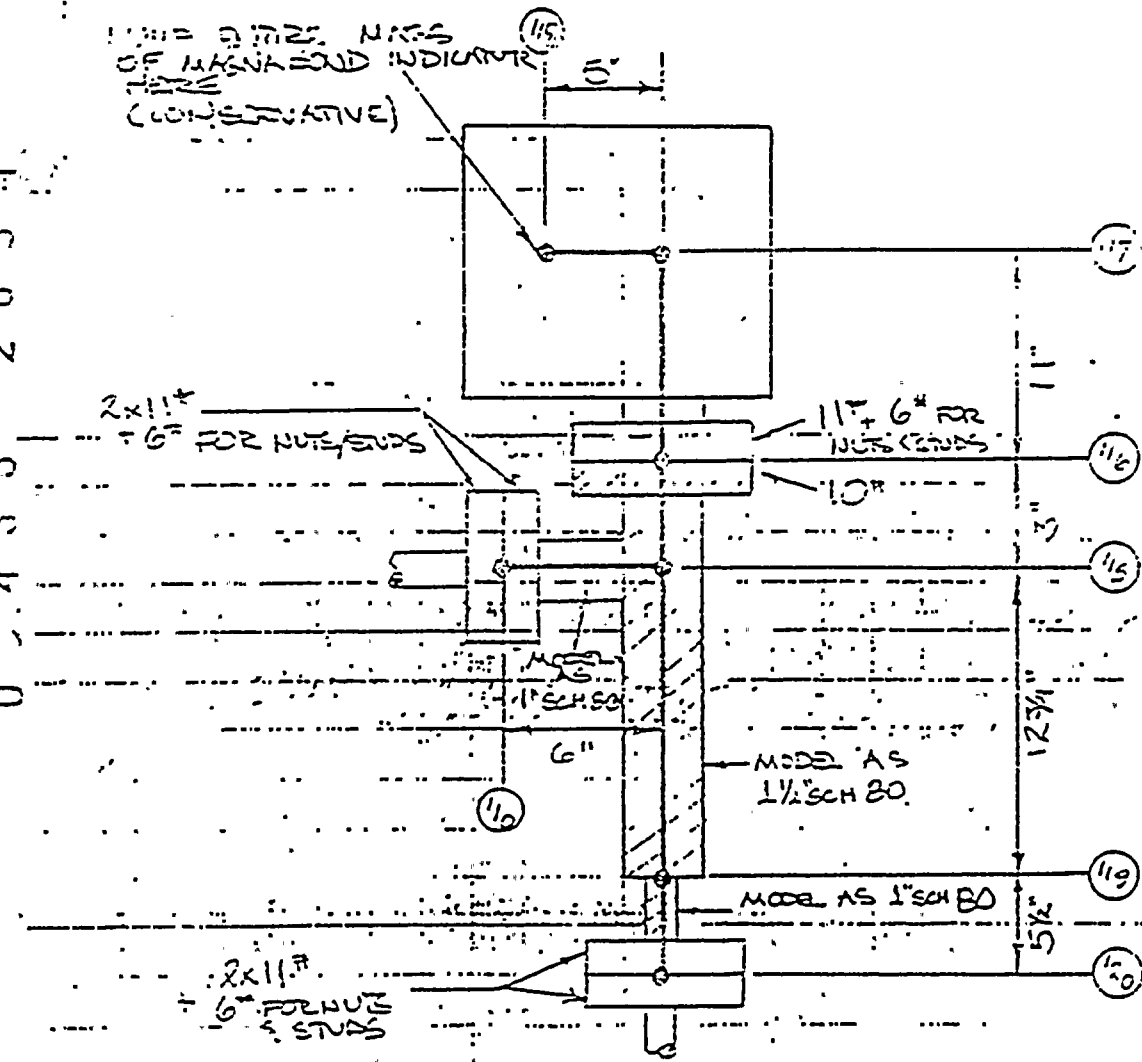
ANALYSIS ASSUMPTIONS

MODELING FOR FEA
 FOR ANALYSIS MODEL THE FEA ASSEMBLY IS SHOWN BELOW

LINE SIZE MASS
 OF MAGNETIC INDICATOR
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 (CONSERVATIVE)

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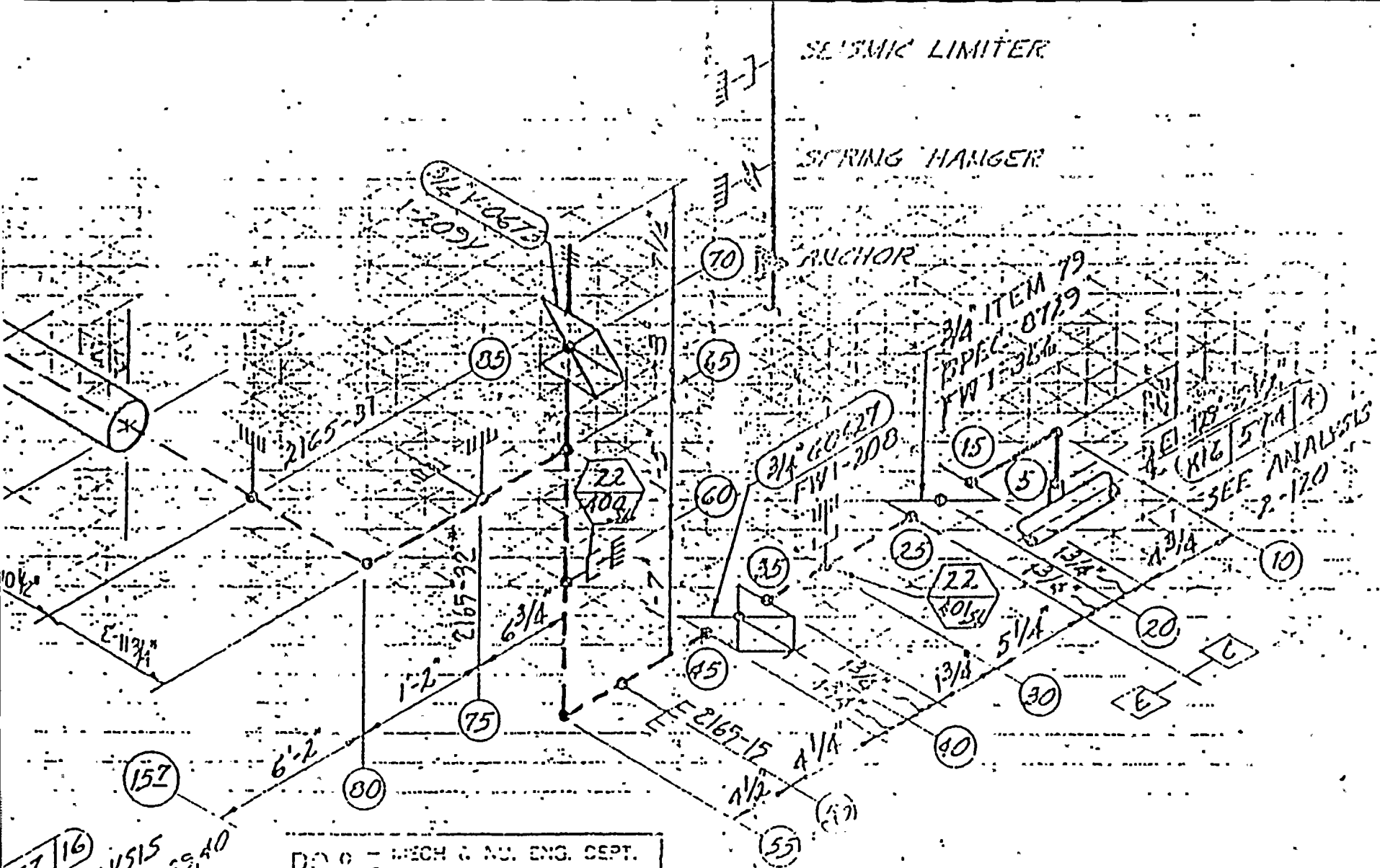
WEIGHT OF FLOWRATOR (CROSS HATCHED REGION) = 23"
 LUMP 10" FOR FLG. AT NODE 116 & USE UNIFORM WEIGHTS
 AS FOLLOWS:

FROM NODE 110 TO 115	4.38 ^{lb} /ft	=	2.19 ^{lb}
FROM NODE 115 TO 116	6.70 ^{lb} /ft	=	1.68 ^{lb}
FROM NODE 116 TO 119	6.70 ^{lb} /ft	=	7.12 ^{lb}
FROM NODE 119 TO 120	4.38 ^{lb} /ft	=	2.2 ^{lb}

SUPERSEDED
 BY REV _____ CFCALC 3-3-82
 DATE 0-12-83 BY P.T.M.
 CHKD [Signature] APPD [Signature]



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57/16
ANALYSIS
SEE FLEX 38, 39, 40

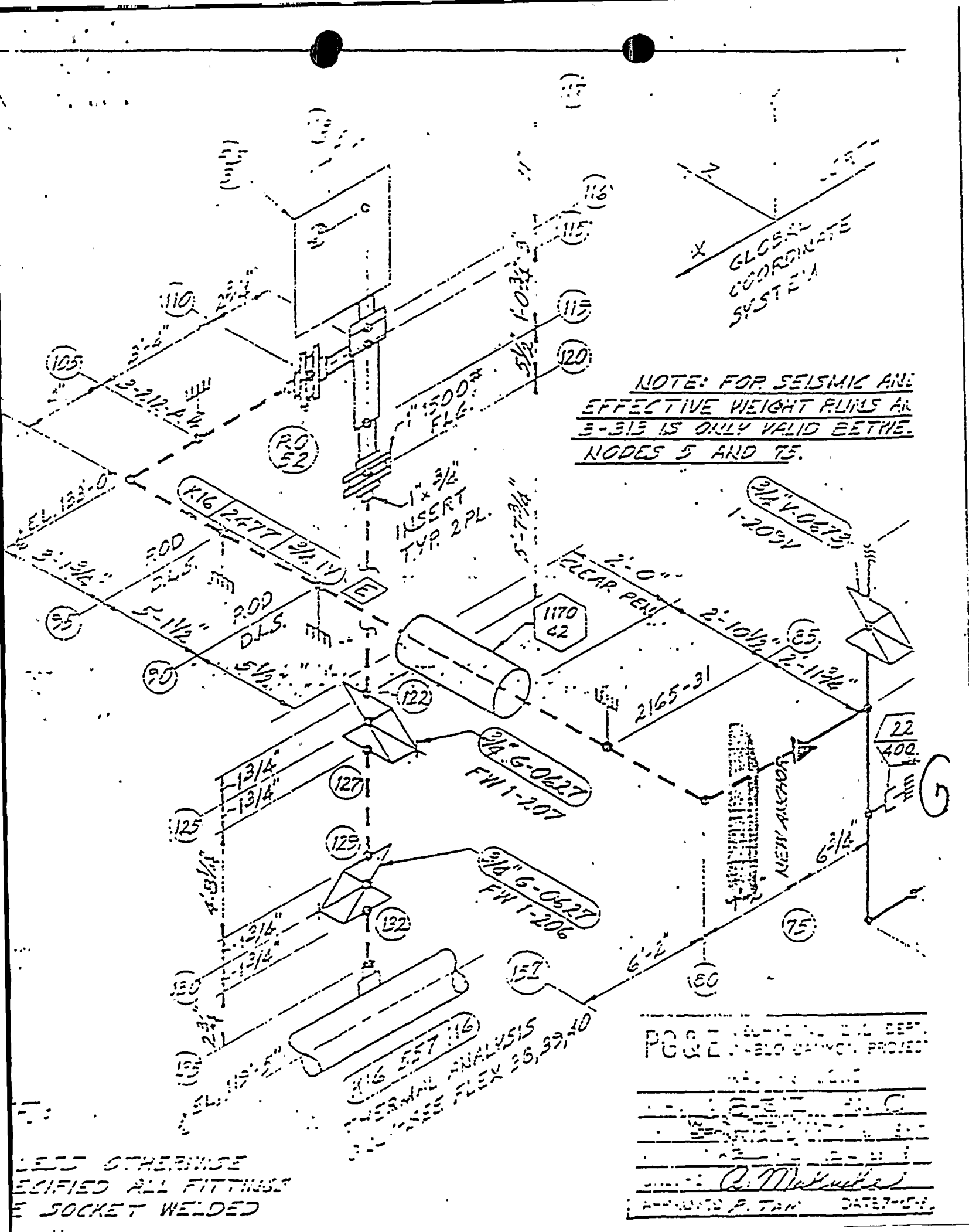
PROJ - MECH & CIV. ENG. DEPT.
 PROJECT - DIABLO CANYON PROJECT
 SHEET NO. 1
 APPROVED BY: [Signature] DATE: 7-7-83

UNIT ONE
 REF. 150, HS-212, WLRDN. DATE 7-7-83





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GLOBAL
COORDINATE
SYSTEM

NOTE: FOR SEISMIC AN EFFECTIVE WEIGHT PLUMB AN E-313 IS ONLY VALID BETWEEN NODES 5 AND 75.

LESS OTHERWISE
SPECIFIED ALL FITTINGS
TO BE SOCKET WELDED

3/4" G-0527 FH 1-206
SEISMIC ANALYSIS
DATE: 26, 37, 40

PG&E
FIELD CONTROL PROJECT

DATE: 7-5-88



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PLANT DESIGN GROUP

FOR FEED PP B.E. DISC

ISS NO 11 MS-E 2 (WARRINGTON, DATE 7-1967)

SECTION	LEVEL	PERCENT	MAXIMUM COMPUTED (PSI)	ALLOWABLE
...	12799	0.711
...	22651	0.639
...	30063	0.635
...	25237	.682

REFERENCE CALCULATIONS:

3-313	SECTION-INERTIA PORTAL	3-313	EFWT 3-313
3-313	SECTION-INERTIA PORTAL	-	-
	DYNAMIC		

PETER TAM *Peter Tam*

MURRAY LEPPKE *Murray E. Leppke*

R. J. GEFY *R. J. GEFY*

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05433-26/1

LAABLO CANYON NUCLEAR POWER PLANT UNIT NO. 1
 113-212 (VIAWAD 7-15-03)

UNIT	STRESS	DESIGN STRESS	ALLOWABLE STRESS	CRIPPLED
11	15100	15100	15100	0.351

5	5	1122	14000	0.250
5	5	6011	27500	0.224
5	5	8872	36500	0.216

THE PRINTED VALUES IN THE STRESS RATIOS REFLECT VALUES IN THE NON-EMERGENCY PORTION OF THE TABLES, THEREFORE NOT VALID IN AN EMERGENCY CODE BREAK SITUATION (NODE 5 TO 75) AND CORRECTED VALUES ARE...

SUPERSEDED
 BY REV. 9-15-03
 DATE 9-15-03
 CHG

3-313 RND	3-313 RND	3-313 RND
3-313 RND Q	3-313 RND Q	3-313 RND Q
-NA-		

THESE VALUES ARE... TOP VALUES... BEYOND... IS THE... OF THE... IS... AND...

7-15-03
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The hangers shown in the pipe stress analysis have been qualified for loads by N-0 calc number 3-303-R which are substantially greater than these loads.

The purpose of this analysis is to qualify the stress induced in the piping because of thermal, dead load, seismic, and SAM/TAM effects.

The boundary between the seismic class I and the non-seismic class E piping is the downstream side of the check valve (data point 25).

Structurally, the model was terminated at hanger number 2165-31, data point 85. The hangers beyond this point are dead load supports and cannot be considered in a seismic analysis. At data point 85, a mass of three seismic spans of piping were added to simulate the effects of the remaining piping in the system. For the purpose of performing a stress analysis of this code break, this model is more than adequate.

Reviewing the calculated stress reveals the following:

- a) For the static analysis (Eq 11) the stress in the pipe is well below code allowable in both the class I and class E sections.
- b) For the thermal expansion analyses (Eq 13/14) the stress in the piping is well below code allowable in both class I and class E sections.
- c) For the Design Earthquake (D E) seismic analysis (Eq 12/1.2 Sh) the stress in the pipe in the Class I section is well below the code allowable (1.2 Sh). In the class E section, the stress in the pipe does not exceed code allowable until data point 55, which is downstream of the second isolation valve. Note that for all points in the class E section, the calculated stress does not exceed the Hosgri allowable stress (2.4 Sh) implying that plastic deformation does not occur.

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ANALYSIS NO. 3-313

REV. 0

DATE: 7-12-85

ANALYST KELLY J. CONDRON

CHECKER A. MAHNCKE E/M

15

ITEM NO.	COMMENT	RESOLUTION	DATE
1. 2. 3. 4. 5.	<p>FOR PURPOSES OF THIS ANALYSIS, ONLY PIPING BETWEEN NODES 75 AND 75 IS EVALUATED FOR STRESS. THE REMAINDER OF PIPING (NODES 75 TO 75) IS INCLUDED FOR OVERLAPPING EXPOSURE.</p>	<p>NONE REQUIRED</p> <p>REV. 1</p>	<p>1/10 AM</p> <p>6</p>
2.	<p>APPLICABLE TO BRANCH LINE CONNECTION @ NODE 5 ONLY.</p>	<p>2. NONE REQUIRED</p>	<p>1/10 AM</p> <p>1.30</p>

*Column C is to be used by checker. Column A is to be used by Analyst.



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For the Double Design Earthquake (DDE), the calculated stress in the class I section of the pipe are well below the code allowable. In the class II section of the pipe, the code allowable stress was exceeded at data point 35. However, this stress does not exceed the Hosgri allowable stress. The Hosgri allowable stress ($2.4S_n$) is not exceeded until data point 55, which is downstream of the second isolation valve.

e) For the Hosgri (HS) earthquake, the calculated stress in the class I section is well below code allowable ($2.4 S_n$). In the class II section, the code allowable is not exceeded until data point 45 which is on the downstream side of the second isolation valve.

EXPLANATION:

The lateral seismic movements calculated at data point 85 are $\Delta X(DE)=2.817''$, $\Delta X(DDE)=5.633''$, and $\Delta X(HS)=8.162''$. The drawing shows the existence of a wall penetration 2'-10 1/2" from data point 85. This wall penetration will restrict the seismic deflection of the pipe to 5-5/8" laterally at any point in the penetration. Because of this penetration, the maximum deflection at data point 85 is expected to be approximately 2-1/8" which is comparable to the "DE" case where stress would be proportional to deflection. Clearly, it can be concluded that the seismic stresses in the piping will be significantly less than those calculated.

Based on this analysis, it is concluded that the criteria for a code break has been met.

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INSITE PROJECT ENGINEERING DIVISION
REQUEST FOR WELD INSPECTION

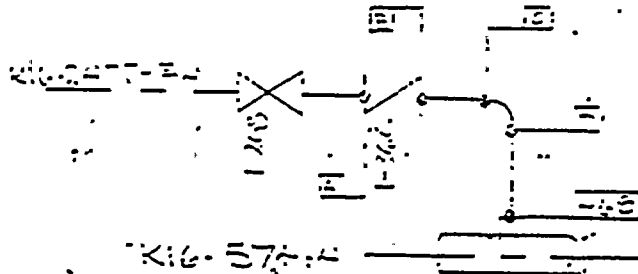
WELD NO.
 TO: HARRY H. HILL
 FROM: KELLY J. CONDRON
 EXT: 3502

REQUEST NUMBER Not Applicable
 PAGE 1 OF 1

PLEASE CONFIRM THAT FOR A SOCKET WELDED FITTING TOE OF WELD BLENDS SMOOTHLY WITH NO UNDERCUT WITH PIPE WALL.

WELD NO. SEE SKETCH BELOW LINE NO. 2477
 AREA GE ISOMETRIC NO. H3-212
 ELEVATION 130' P.I. of COL. 4 1/8" E of R Rad. of LINE 7/16" of 15

DESCRIPTION:



VERIFY THAT WELDS A, B, C, D, 468 HAVE NO UNDERCUT AND BLEND SMOOTHLY WITH THE BASE MATERIAL.

INSPECTOR'S COMMENTS:

- WELD A - SLIGHT UNDERCUT / DOESN'T BLEND SMOOTHLY
- WELD B - UNDERCUT & GRINDING MARK / DOESN'T BLEND SMOOTHLY
- WELD C - NO UNDERCUT / DOESN'T BLEND SMOOTHLY
- WELD D - UNDERCUT / DOESN'T BLEND SMOOTHLY
- WELD 468 - UNDERCUT / DOESN'T BLEND SMOOTHLY

INSPECTOR'S SIGNATURE

FT 56 [Signature]



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DIABLO CANYON PROJECT
TELEPHONE
CALLS

TO: E. NICHOLSON OF OREG. DATE: 11/6/73 TIME: 1:00 PM
 FROM: L. NICHOLSON OF BECHTEL SFO 15220 TIME: 3:00 PM

ITEMS OF DISCUSSION | ACTION REQ'D. (INCLUDE NAMES & DATES)

SUBJECT: RESPONSE SPECTRA

1. SPECTRA CALC #3-313

2. THE STRESS ANALYST HAD MODIFIED
 3. A BILT INTO AN ANCHOR.

3. THE BILT IS STRUCTURALLY
 4. ATTACHED TO A WALL IN THE
 5. CONTAINMENT - PENETRATION AREA AND
 6. THE NEW ANCHOR WILL ALSO BE
 7. ATTACHED TO THE SAME STRUCTURE:

PLEASE NOTE THAT NONE OF THE RESTRAINTS
 ARE ATTACHED TO CONTAINMENT
 EXTERIOR WALL.

THEREFORE, THERE WILL NOT BE
 ANY CHANGES IN THE SPECTRA
 ENVELOPE. SPECTRA IS FINAL.

WRITTEN RESPONSE REQUESTED

DATE: 11/6/73



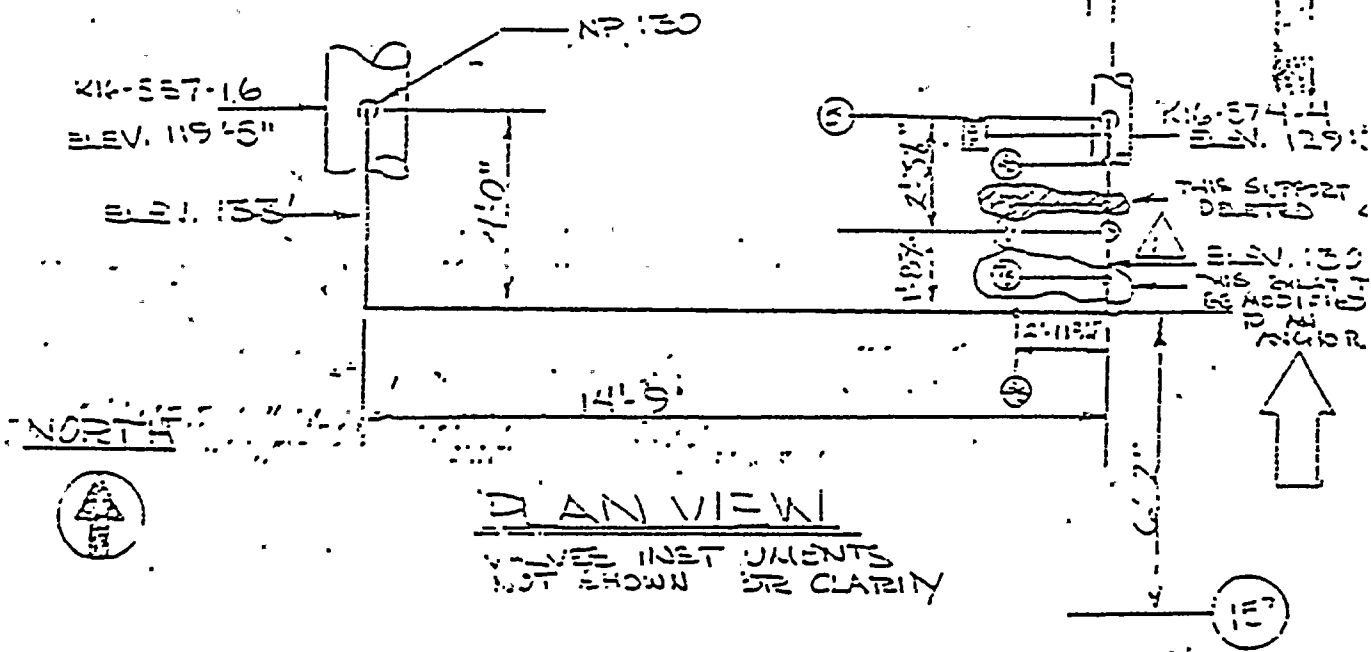
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GENERAL DATA

THESE DATA ARE BEING FOR TO BE USED IN CONNECTION WITH THE DATA TO THE FIG. NO. OF SUBMITTAL FOR SEWER MAINS IN THE TOWN OF ...

PLAN VIEW

THESE DATA ARE BEING FOR TO BE USED IN CONNECTION WITH THE DATA TO THE FIG. NO. OF SUBMITTAL FOR SEWER MAINS IN THE TOWN OF ... THE TOWN OF ... IS BEING ACCORDANCE WITH ... SUPPORTED ... TO 130 IS INCLUDED TO ILLUSTRATE THE OVERALL SCOPE OF THE ... RIDING BETWEEN NODES TO A 130 WILL NOT BE QUALIFIED. ESPECIALLY.



COPY

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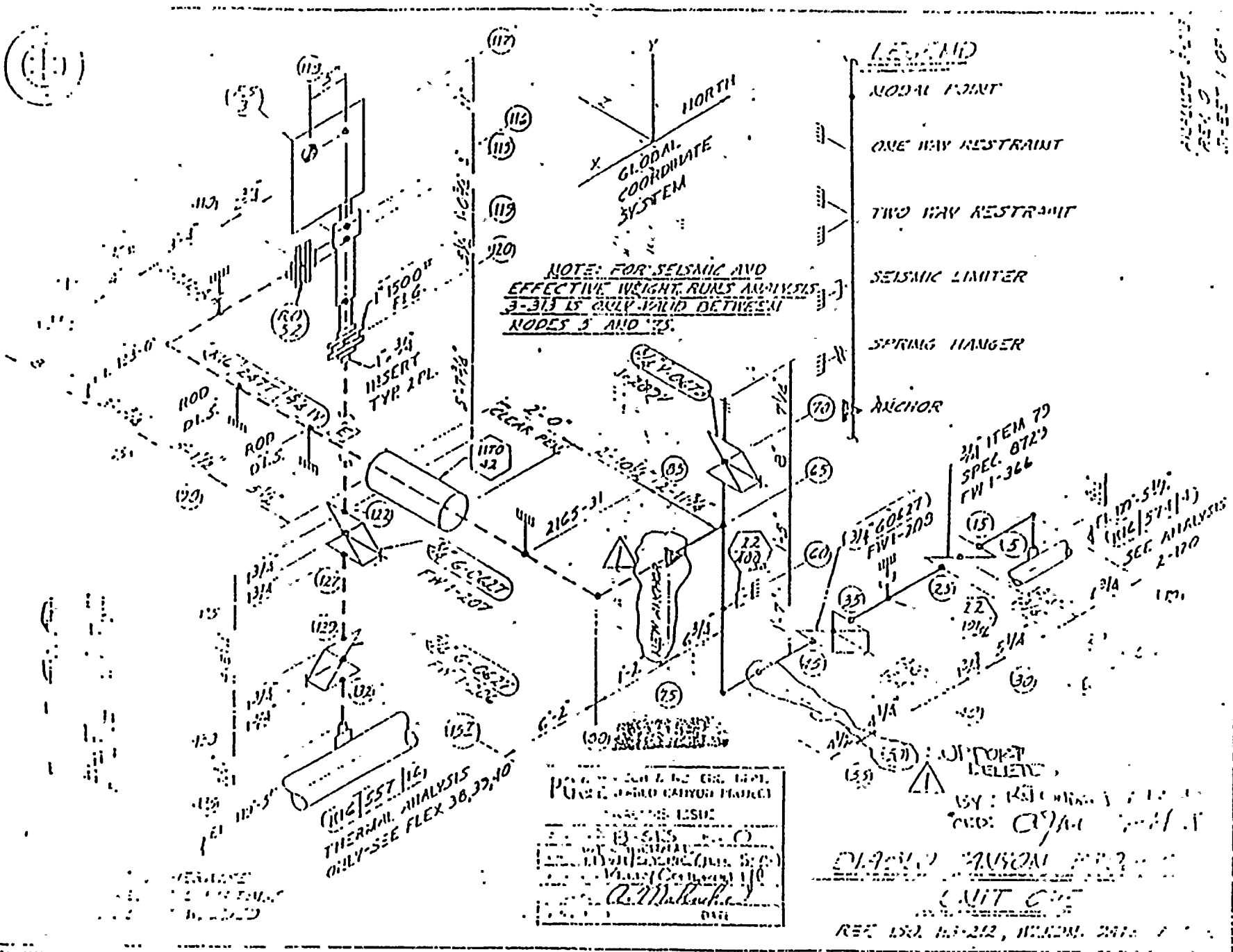
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LINE NO.	PIPE SIZE	SUPPORT DATA Y.D.	DATA POINT	DIRECTION OF RESTRAINT (CONT. INT. AUX. ETC.)	ATTACH. STRUCTURE & EL.
2477	3/4" Ø 5/8"	L-BORE LINE 574	5	ANCHOR	NOT APPLICABLE
		22-40151	30	Y	CUTTING PERFORATION WALL CEILING @ ELEV. 137' AREA 65'
		22-40151	30	Y	CUTTING PERFORATION WALL @ ELEV. 137'
		22-40094	60	X	CUTTING PERFORATION WALL @ ELEV. 131.7'
		NEW ANCHOR 22-40094	75	ANCHOR	CUTTING PERFORATION WALL @ ELEV. 133' AREA 125'
		2165-31	85	Y	CUTTING PERFORATION CEILING @ ELEV. 137'
	↓	L-BORE LINE 557	150	ANCHOR	NOT APPLICABLE No Seismic Analysis

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PROJECT: [Illegible]
 DRAWN BY: [Illegible]
 CHECKED BY: [Illegible]
 DATE: [Illegible]



07-2013-0000

TITLE PAGE 11/10/78
 REVISED 11/10/78
 THIS IS A COST-BEHAVIOR ANALYSIS REQUESTED BY THE U.S. AIR FORCE
 ON 7-5-83 FEEDATER SYSTEMS-LINE NO. 2477
 VERSION PAC-FIX1
 PASSWORD KJC

TIT=ANX FEED FF 3/3 DISC FILL,
 PROBN0=3-313 REV0,
 PROJID=15:20,
 USER=R.G.R.Y.,
 SC=01, SIGMA=8, UNITS=2,
 MODE=30, ER=.025,
 LDCASE=KTRP1(1-4),
 LDCASE=EF T01(1),

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THEM1=START UP/SPLTDOWN
 4218 NODES 5 TO 21
 1155 NODES 22 TO 135

THEM2=BUCK FORCE-ENVELOPES 2ND, 3RD, 4TH, 5TH AND 6TH
 4218 NODES 5 TO 135

LDCASE=THEM1(1),
 LDCASE=THEM2(2),
 LDCASE=SE1SD1(A),
 LDCASE=SE1SD2(A),
 LDCASE=SE1SHX(A),
 LDCASE=SE1SHZ(A),

SAN LOAD CASES ARE NOT REQUIRED SINCE DISPLACEMENTS ARE LESS THAN 1/16"

5 -.165
 5 .174

ROT-Z=-.00158
 ROT-Z=.00161,

15 0-4.7
 25 0-1.8
 35 0-1.8
 40 0-5.2
 45 0-1.1
 40 0-1.7
 45 0-1.7
 55 0-4.3

.001
 3-1-3/4

EXP=-0.231
 EXP=2.99
 CODE=F31S73, DFRESS=15#6,
 SC=15000, SH=15000,
 OD=1.05, TPI=.154, LBS/FT=3.1,
 E=27.9E6,
 SIF=2.1,
 SIF=2.1,
 SIF=2.1,
 TPI=.308, LBS/FT=0, ADDWT=4.2,
 SIF=2.1,
 EXP=0.305
 TPI=.154, LBS/FT=3.1,
 DFRESS=1415,
 DTI=SNB 22-4C1SL,

SIF=2.1,
 TPI=.308, LBS/FT=0, ADDWT=4.2,
 SIF=2.1,
 TPI=.154, LBS/FT=3.1,
 DTI=2145-15.



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CIFEN 1,
DTIC 166-1, ACOUNT=39.5

THE MASS OF 3 SPAN LENGTHS WAS LEASED AT NODE 107. SPAN LENGTHS WERE USED FOR THE MASS CONTRIBUTION EFFECTS OF THE CLASSIFICATION. IT IS NOTED THAT APPROACH DOES NOT ADDRESS INTERNAL EFFECTS FOR CASE.
HOSGRI HORIZ. HOSGRI VERT. SP. HOSGRI. BE VERT.
MASS=2.148 (MA44) MA37 DANC=1.3017 (A20) 220
MASS=1.381 (MA57) MA38 DANC=1.3017 (A20)
MASS=2.148 (MA50) MA39
MASS=1.193 (MA54)

- 1. CRITERIA FOR UTILIZATION OF DESIGN EARTHQUAKE RESPONSE SPECTRA FOR STRUCTURES, SYSTEMS AND COMPONENTS DIESEL CANYON POWER PLANT, UNIT 1 AND 2. DOM. NO. C-28, REV. 2
- 2. CRITERIA FOR UTILIZATION OF HOSGRI 7.5 P EARTHQUAKE RESPONSE SPECTRA FOR STRUCTURES, SYSTEMS AND COMPONENTS DIESEL CANYON POWER PLANT, UNIT 1 AND 2. DOM. NO. C-37, REV. 7

TITLE PAGE NO. 3-51

AGE
LOC
FOR
AGE

.0145	.4018	.0171	.5757	.207	.57
.0232	.5825	.0253	.7650	.310	.76
.0332	.7598	.0340	.9334	.448	.93
.0441	.9313	.0450	.1107	.500	.77
.0557	.1107	.0560	.7116	.505	.93
.0641	1.5292	.0675	1.9483	.501	1.55
.0707	2.3145	.0837	4.4615	.801	4.45
.1100	3.6442	.1125	2.5586	.822	2.47
.1375	2.4433	.1335	1.7412	.8467	1.85
.1571	1.5551	.1572	1.7743	.800	1.35
.1630	1.4958	.1515	1.7743	.800	1.77
.2340	1.7696	.2713	1.4934	.841	1.11
.3097	1.5179	.3078	1.1548	.815	1.07
.3333	1.3533	.3467	1.3533	.827	1.15
.4231	1.5232	.4513	1.1548	.800	.87
.5000	.3018	.4331	.5074	.875	.74
.7053	.7192	.7273	.6787	.827	.87
.9182	.4856	1.0016	.6556	1.000	.87
1.2222	.4857	1.3750	.2045	1.000	.87
1.5333	.2609	2.2000	.1245	2.000	.87
3.0667	.0661	5.5000	.0283		
.0617	.1330	.0576	.1510	.600	.87
.0800	.0670	.0980	.0570	.800	.87
.1207	.0775	.1500	.0571	.800	.87

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