REPORT OF SOILS EXPLORATION ZION SOLUTIONS ZION NUCLEAR POWER PLANT INDEPENDENT SPENT FUEL STORAGE INSTALLATION

PREPARED FOR: SARGENT & LUNDY LLC 55 EAST MONROE STREET CHICAGO, ILLINOIS 60603

PREPARED BY:
TESTING SERVICE CORPORATION
457 EAST GUNDERSEN DRIVE
CAROL STREAM, ILLINOIS 60188
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REPORT OF SOILS EXPLORATION ZION SOLUTIONS ZION NUCLEAR POWER PLANT INDEPENDENT SPENT FUEL STORAGE INSTALLATION

1.0 INTRODUCTION

Presented in this report are the results of the soils investigation performed to determine the general subsurface conditions for the proposed construction of an Independent Spent Fuel Storage facility. The proposed structure will be constructed at the Zion Nuclear Power Plant in Zion, IL.

This work was performed in general accordance with Technical Specification Subsurface Investigation Specification Z-5701 prepared by Sargent and Lundy and dated July 29, 2008.

2.0 FIELD WORK

A total of two(2) soil borings (I-20 and I-21) were performed for this project with each of them being extended to a depth of 100.0 feet. Boring I-20 was subsequently converted into a seismic cross hole survey source hole. Additionally, two (2) boreholes were drilled and completed with 3" PVC casing for cross hole seismic testing.

Upon completion of the drilling and testing, ground surface elevations as well as Northing and Easting coordinates were determined for the drilling locations using a Trimble R8 GPS. The vertical and horizontal data shown on the Boring Location Plan was retrieved from a National Geodetic Survey data base located approximately 1.2 miles south of Zion, Illinois.



The boreholes were initially advanced by 4 1/4" hollow stem auger methods to the ground water table and then completed by rotary wash methods using bentonite and potable water in accordance with ASTM D 5783. Upon completion, each borehole was sealed with cement-bentonite grout placed by tremie methods.

Soil samples were collected at I-20 on 2.5 foot intervals for the entire 100 foot depth. At location I-20 the sampling interval was 2.5 feet for the first 10 feet and 5.0 feet thereafter. Standard Penetration Tests (SPT) - N Values (ASTM D 1586), were determined using an automatic hammer which allows a 140 pound weight to free fall from a 30 inch drop to drive the sampler. The samples were collected using standard split spoon procedures during SPT since granular soils and/or very stiff clays prevented the use of Shelby tube samplers. Liners were not used with the split spoon samplers.

Dynamic energy measurements have not been made on this automatic hammer. However, Pile Driving Analyzer's (PDA) have been used to process dynamic measurements of strain and acceleration between the drill string and similar hammers made by the same manufacturer (Boart Longyear). The Average Energy Transfer Ratio is accepted as 75% for this model hammer.

The soil types were logged in the field by the drill crew and representative portions of each sample were contained in glass jars per ASTM D 4220 and brought back to our laboratory. Boring Logs defining the soil lithology and ground water observations is included in the Appendix of this report.

Ground water depth was determined during hollow stem auger drilling only. The borings were completed using wet rotary methods and therefore, the water levels inside the boreholes at the completion of the drilling were not representative. PVC casing was installed shortly after the completion of the drilling and reaming at location I-20. Boring I-21 was sealed with grout immediately following the drilling and sampling to prevent an open hole hazard. Therefore, 24 hour water level readings were not obtained. The ground water depth measured during the hollow stem auger drilling is considered the most accurate indication of the undisturbed ground water levels at the site.



3.0 GEOPHYSICAL TESTING

Geophysical testing consisting of a Cross Hole Seismic Investigation, was performed at location I-20. The original bore hole (I-20) was reamed to a depth of 100 feet using a 6" tri cone roller bit and then cased with 3" PVC casing sealed in place with cement-bentonite grout. Two additional holes (I-20A and I-20B) were drilled using a 6" tri cone roller bit to a depth of 100 feet and were located in a straight line at approximate distances of 10 and 20 feet from I-20. These bore holes were also completed with the installation of 3" PVC casing sealed in place with cement-bentonite grout.

The Cross Hole testing was performed by Geotechnology, Inc. In general conformance with Section 4.31 of ASTM D 4428. A copy of the report prepared by Geotechnology, Inc. summarizing the test results is included in the Appendix of this report. Following the testing each of the PVC casings were sealed with bentonite chips which were hydrated upon placement. In addition the PVC was cut off near the ground surface and topsoil was place over the holes.

4.0 LABORATORY TESTING

The soil samples obtained from the borings were examined in our laboratory and visually classified according to the Unified Soils Classification System (ASTM D 2488). Cohesive soils were measured for moisture content (ASTM D 2216) and unconfined compressive strength (ASTM D 2166). Additional testing including grain size distribution (ASTM D 422, D 1140), Atterberg Limits (ASTM D 4318), pH (ASTM G 51) and dry unit weight determinations (EM 1110-2-1906) were performed on samples selected by Sargent and Lundy. The results of this testing has been summarized and incorporated into the Soil Test data Sheets included in the Appendix.

Two (2) soil samples selected by the client were analyzed for soluble chlorides (Method 4500CL,C) and soluble sulfates (Method 4500SO4, E) by First Environmental Laboratories. A copy of their report is Appended.



5.0 CLOSURE

The data and results presented in this report are based upon the conditions encountered at the two (2) locations investigated as indicated on the Soil Boring Location Plan. This report does not reflect any variations that may occur between these locations, the nature and extent of which may not become evident until the construction phase of the project.

We are appreciative of the opportunity to assist you with this project and look forward to working with you in the future. Please call if you have any questions or are in need of further information.

Respectfully Submitted,

Michael V. Machalinski

Vice President

Registered Professional Engineer

Illinois No. 062-038559

Prepared By,.

Darin Delaney Project Geologist

APPENDIX

UNIFIED CLASSIFICATION CHART

LEGEND FOR BORING LOGS

TWO (2) BORING LOGS (6 pages)

BORING LOCATION PLAN (1 page)

GEOPHYSICAL DATA REPORT Geotechnology, Inc. (16 pages)

SOIL DATA SHEETS (12 pages)

TWO (2) ANALYTICAL REPORTS First Environmental Laboratories, Inc. (5 pages)

TESTING SERVICE CORPORATION UNIFIED CLASSIFICATION CHART

	CRITERI	A FOR A	SSIGNING GR	OUP SYMBOLS AND	S	OIL CLASSIFICATION	
	GROUP	NAMES U	SING LABO	DRATORY TESTS ^a	GROUP SYMBOL	GROUP NAME b	
200	GRAVELS More than 50%	re than 50% CLEAN GRAV coarse Less than 5% fines c		$C_{u \ge 4 \text{ and } 1 \le C_c \le 3 e}$	G W	Well graded gravel ^f	
o	of coarse fraction retained			$C_u < 4$ and/or $I > C_c > 3$ e	GP	Poorly graded gravel f	
d on N	on No. 4 sieve	GRAVELS		Fines clossify as ML or MH	GM	Silty grovel f,g,h	
aine raine		12% fin	More than es ^c	Fines classify as CL or CH	GC	Clayey gravel f,g,h	
siens	SANDS	CLEAN		C _u ≥ 6 and l ≤ C _c ≤3 ^e	sw	Well-graded sand	
COARSE-GRAINED S more than 50 % retained sieve	50 % or more of coarse froction passes No. 4 sieve	Less that	on 5 %	$C_U \le 6$ and/or $I > C_C > 3^e$	SP	Poorly graded sand I	
		CANDS WITH FINI		Fines classify as ML or MH	SM	Silty sand g,h,f	
Ë		more in	esd esd	Fines classify as CL or CH	sc	Clayey sand g,h,f	
0	SILTS & CLAYS Liquid limit less than 50%			>7 and plots on or above "A" line j	CL	Lean clay k,I,m	
No. 200			Inorganic	PI<	4 or plots below "A" line j	ML	Silf ^k ,I,m
ore possed the No		Organic	<u>Liqu</u> Liqu	id limit - oven dried < 0.75	OL	Organic clay ^{k,l,m} ,n Organic silt ^{k,l,m} ,o	
ore po	SILTS & CLAYS Liquid limit 50 % or more	SILTS & CLAYS	Inorganic	PIpl	ots on or above "A" line	сн	Fot clay ^{k,l,m}
FINE - G % or more		inor yanic	PI p	lots below "A" line	мн	Elostic silt ^{k,l,m}	
50		Organic	<u>Liqui</u> Liqui	d limit - oven dried <0.75	он	Organic clay k,l,m,p Organic sill k,l,m,q	
ighly	organic soils	Primarily	organic matt	er,dark in color, and organic odor	PT	Peat	

 $c_u = D_{60}/D_{10}$ $c_c = \frac{(D_{30})^2}{D_{10} \times D_{60}}$

1). If Atterberg Limits plot in hotched area, soil is a CL-ML, silly clay.

k. If soil contains 15 to 29 % plus No. 200, add "with sand" or "with gravel" whichever is predominant.

1. If soil contains \(\sum_{0.000} 30 \% \) plus No. 200, predominantly sand, add "sandy" to group name.

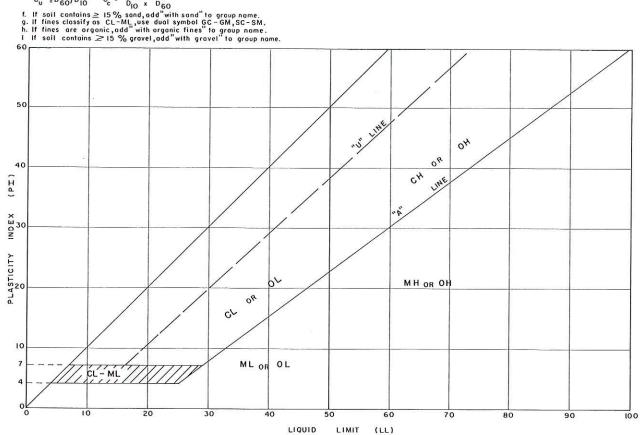
m. If soil contains \(\sum_{0.000} 30 \% \) plus No. 200, predominantly gravel, add "gravelly" to group name.

n. PI \(\sum_{0.000} 4 \) are plots on or above "A" line.

p. PI \(\sum_{0.000} 4 \) or plots below "A" line.

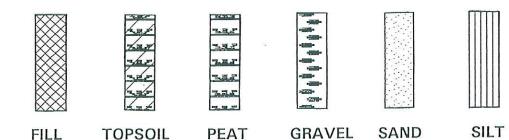
q. PI \(\sum_{0.0000} 1 \) plots below "A" line.

q. PI \(\sum_{0.00000} 1 \) plots below "A" line.



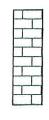
TESTING SERVICE CORPORATION

LEGEND FOR BORING LOGS





CLAY



DOLOMITE

SAMPLE TYPE:

SS = Split Spoon ST = Thin-Walled Tube

A = Auger

FIELD AND LABORATORY TEST DATA:

N = Standard Penetration Resistance in Blows per Foot

Wc = In-Situ Water Content

Qu = Unconfined Compressive Strength in Tons per Square Foot

* Pocket Penetrometer Measurement; Maximum Reading = 4.5 tsf

yD = Dry Unit Weight in Pounds per Cubic Foot

WATER LEVELS:

V While Drilling∇ End of BoringV 24 Hours

SOIL DESCRIPTION:

PARTICLE SIZE RANGE **MATERIAL** Over 12 inches **BOULDER** 12 inches to 3 inches COBBLE 3 inches to 34 inch Coarse GRAVEL 3/4 inch to No. 4 Sieve Small GRAVEL Coarse SAND No. 4 Sieve to No. 10 Sieve No. 10 Sieve to No. 40 Sieve Medium SAND No. 40 Sieve to No. 200 Sieve Fine SAND Passing No. 200 Sieve SILT and CLAY

COHESIVE SOILS

COHESIONLESS SOILS

CONSISTENCY	<u> </u>	RELATIVE DENSITY	N
Very Soft	Less than 0.3	Very Loose	0 - 4
Soft	0.3 to 0.6	Loose	4 - 10
Stiff	0.6 to 1.0	Firm	10 - 30
Tough	1.0 to 2.0	Dense	30 - 50
Very Tough	2.0 to 4.0	Very Dense	50 and over
Hard	4.0 and over		

MODIFYING TERM	PERCENT BY WEIGHT
Trace	1 - 10
Little	10 - 20
Some	20 - 35

PROJECT ISFSI Zion Nuclear Power Plant CLIENT Sargent & Lundy, LLC 1-20**BORING** 8-12-08 DATE STARTED 8-14-08 DATE COMPLETED **JOB ELEVATIONS** WATER LEVEL OBSERVATIONS **GROUND SURFACE** 587.2 ▼ WHILE DRILLING 3.5' **END OF BORING** 487.2 AT END OF BORING Mud Rotary Wash LENGTH RECOVERY 24 HOURS (PVC Pipe Installed) SAMPLE YDRY DEPTH ELEV. WC Qu SOIL DESCRIPTIONS NO. TYPE 0 Firm brown and gray fine to medium SAND, SS 5trace gravel, damp (SP) 11-13 3.0 584.2 Loose brown and gray fine to medium SAND. trace gravel, saturated (SP) 2 SS 2-2-4 S-2: % Graval = 8 5.5 581.7 % Sand = 86 SS 3 2-% Silt and Clay = 6 1-1 Loose dark gray fine to medium SAND, trace gravel, saturated (SP) SS 10 2-3 10.5 576.7 SS 8-10-12 Firm gray fine to medium SAND, trace gravel, saturated (SP) DISTANCE BELOW SURFACE IN FEET SS 7-8-10 15 15.5 571.7 S-7: % Gravel = 5 % Sand = 91 SS 9-% Silt and Clay = 4 23-22 SS 10-20 17-19 Dense gray fine to medium SAND, trace gravel, saturated (SP) SS 13-14-21 23.0 564.2 Dense gray fine SAND, trace silt, saturated 10 SS 14-(SP-SM) 25 30-29 25.5 561.7 SS 8-26.6 Firm gray sandy SILT, saturated (ML) 5-5 28.0 559.2 S-12: (LL, PL, PI) = 22, 11, 11 % Gravel = 3 SS 8-11.0 4.5+* 129.9 % Sand = 24 10-11 % Silt = 52 % Clay = 21 SS 8-11.2 6.73 128.4 9-13 SS 20.5 4.73 108.5 7-Very tough to hard gray silty CLAY, some sand, 7-10 35 trace gravel, moist (CL) SS 10-12.8 6.90 125.2 14-15 38.0 549.2 31-Very dense gray clayey SILT, very moist (ML) SS 16 14.8 120.4 50/5"

DRILL RIG NO. 315

72070.GPJ TSC_ALL.GDT 10/2/08

SC

Division lines between deposits represent approximate boundaries between soil types: in-situ, the transition may be gradual.

PROJECT ISFSI Zion Nuclear Power Plant Sargent & Lundy, LLC CLIENT 1 - 20**BORING** 8-12-08 8-14-08 DATE STARTED DATE COMPLETED **JOB ELEVATIONS** WATER LEVEL OBSERVATIONS **GROUND SURFACE** 587.2 WHILE DRILLING 3.5 ' 487.2 **END OF BORING** Mud Rotary Wash AT END OF BORING (PVC Pipe Installed) 24 HOURS LENGTH RECOVERY SAMPLE YDRY DEPTH ELEV. WC Qu SOIL DESCRIPTIONS NO. TYPE 40 Description on Page 1 40.5 546.7 SS 11-9.6 10.71 133.8 17 14-14 SS 10-10.6 5.90 131.0 Very tough to hard gray silty CLAY, little sand, 10-11 trace gravel, moist (CL) 19 SS 11-10.5 9.38 131.3 13-13 SS 8-12.9 5.12 124.4 12-15 50 SS 9-13.8 5.06 122.7 10-13 S-22: (LL, PL, PI) = 31, 12, 19 22 SS 8-13.4 5.32 123.7 % Gravel = 1 55 12-14 % Sand = 15 % Silt = 50 % Clay = 34 23 SS 12-13.4 6.23 123.7 16-17 SS 12-15.7 5.12 118.3 15-15 60 25 SS 10-20.3 4.07 108.8 11-13 26 SS 8-20.1 3.93 106.9 65 9-11 SS 8-13.8 4.53 117.7 8-8 68.0 519.2 Very tough to hard gray silty CLAY, little sand, SS 28 10-17.4 3.01 114.6 trace gravel, occasional Cobbles, moist (CL) 50/6" Boulder Zone 69.5 to 72.5' 29 SS 72.5 514.7 SS 25-7.6 14.57 113.8 34-42 Very tough to hard gray silty CLAY, little sand, trace gravel, occasional silt seams, moist (CL) SS 21-11.1 4.5+* 129.6 31-36 78.0 509.2 Very dense gray clayey SILT, little sand, moist 37-(CL-ML) SS 10.7 130.7 50/5"

DRILL RIG NO. 315

DISTANCE BELOW SURFACE IN FEET

72070.GPJ TSC_ALL.GDT 10/2/08

Division lines between deposits represent approximate boundaries between soil types. in-situ, the transition may be gradual.

PROJECT ISFSI Zion Nuclear Power Plant Sargent & Lundy, LLC CLIENT 1-20 8-12-08 **BORING** DATE STARTED DATE COMPLETED 8-14-08 **JOB ELEVATIONS** WATER LEVEL OBSERVATIONS 587.2 WHILE DRILLING 3.5' GROUND SURFACE 487.2 **Mud Rotary Wash** END OF BORING AT END OF BORING (PVC Pipe Installed) 24 HOURS LENGTH SAMPLE YDRY DEPTH ELEV. WC N Qu SOIL DESCRIPTIONS NO. TYPE 80 S-32: (LL, PL, PI) = 17, 10, 7 % Gravel = 0 % Sand = 27 SS 12.0 127.3 33 43-% Silt and Clay = 73 50/4" SS 44-12.1 50/4" 85 Very dense gray clayey SILT, little sand, moist (CL-ML) 86.0 501.2 13.2 Firm gray clayey SAND, saturated (SC) SS 9-87.0 500.2 8-9 15.1 2.55 119.7 SS 18.0 3.21 36 7-113.4 90 8-11 Very tough to hard gray very silty CLAY, little 16.0 2.48 115.4 sand, occasional silt seams, moist (CL-ML) SS 8-37 В 12-13 16.2 4.0* 117.2 38 SS 8-17.3 3.0* 114.8 12-13 95.5 491.7 Dense gray fine to medium SAND, saturated 12.1 26-SS 39 97.0 490.2 27-15 122.5 В 13.9 2.0 Very tough gray very silty CLAY, little sand, occasional silt seams, moist (CL-ML) 15-SS 11.9 2.0* 127.5 21-28 100 End of Boring at 100.0' * Approximate unconfined compressive strength based on measurements with a calibrated pocket penetrometer. 105 Diedrich D-120 Truck Rig (#315) CME Automatic Hammer 110 115 120 Division lines between deposits represent

DRILL RIG NO. 315

BELOW SURFACE IN FEET

DISTANCE

72070, GPJ TSC_ALL, GDT 10/2/08

Division lines between deposits represent approximate boundaries between soil types. in-situ, the transition may be gradual

PROJECT ISFSI Zion Nuclear Power Plant Sargent & Lundy, LLC CLIENT 1-21 **BORING** DATE STARTED 8-18-08 DATE COMPLETED 8-18-08 **JOB ELEVATIONS** WATER LEVEL OBSERVATIONS 587.1 WHILE DRILLING 5.5' **GROUND SURFACE** 487.1 Rotary Wash END OF BORING AT END OF BORING LENGTH RECOVERY **Grouted Upon Completion** 24 HOURS SAMPLE γ_{DRY} DEPTH ELEV. WC N Qu SOIL DESCRIPTIONS NO. TYPE Firm brown and gray clayey SAND and SS 6-GRAVEL, damp (SĆ-GĆ) 8-9 3.0 584.1 Firm brown and gray SAND, trace gravel, damp SS 4-7-10 ♣ S-2: % Gravel = 1 581.6 5.5 % Sand = 95 SS % Silt and Clay = 4 5-9 SS 10-4 10 13-15 S-5: % Gravel = 24 % Sand = 72 DISTANCE BELOW SURFACE IN FEET % Silt and Clay = 4 SS 8-13-14 Firm gray SAND, some gravel, occasional Cobbles, saturated (SP) 6 SS 11-13-13 20 22.0 565.1 SS 10-Firm gray SAND, saturated (SP) 15-12 25 27.0 560.1 Firm gray silty fine to medium SAND, saturated SS 8 6-(SM) 30 8-11 32.0 555.1 S-9: (LL, PL, PI) = 28, 14, 14 % Gravel = 1 % Sand = 8 SS 122.7 16-12.7 5.41 % Silt = 59 16-21 % Clay = 32 Very tough to hard gray silty CLAY, trace sand, trace gravel, moist (CL) 549.1 38.0 Very tough to hard gray very silty CLAY, little 36sand, trace gravel, occasional silt seams, damp 10 SS 12.2 4.5+ 50/4" (CL-ML) Division lines between deposits represent

DRILL RIG NO. 315

72070.GPJ TSC_ALL.GDT 10/2/08

approximate boundaries between soil types; in-situ, the transition may be gradual

PROJECT ISFSI Zion Nuclear Power Plant Sargent & Lundy, LLC CLIENT 1-218-18-08 8-18-08 **BORING** DATE STARTED DATE COMPLETED **JOB ELEVATIONS** WATER LEVEL OBSERVATIONS GROUND SURFACE 587.1 5.5' WHILE DRILLING 487.1 END OF BORING AT END OF BORING Rotary Wash LENGTH 24 HOURS **Grouted Upon Completion** SAMPLE γ_{DRY} DEPTH ELEV. WC SOIL DESCRIPTIONS N Qu NO. TYPE 40 Very tough to hard gray very silty CLAY, little sand, trace gravel, occasional silt seams, damp 42.0 545.1 S-11: (LL, PL, PI) = 23, 12, 11 SS 16-13.0 4.0* 124.7 % Gravel = 5 22-25 % Sand = 20 % Silt = 51 % Clay = 24SS 9.22 15-9.7 133.5 Very tough to hard gray silty CLAY, little sand, 31-34 50 trace gravel, moist (CL) 535.1 52.0 DISTANCE BELOW SURFACE IN FEET 13 SS 26-9.9 3.5* Very tough to hard gray very silty CLAY, little 50/3" 55 sand, trace gravel, moist (CL) 529.1 58.0 14 SS 17-29 50/1" 60 Dense to very dense gray SAND and GRAVEL, occasional Cobbles and Boulders, saturated (SP-GP) SS 27-15 65 24-24 68.0 519.1 SS 14-14.5 7.73 121.1 16 Very tough to hard gray silty CLAY, little sand, 17-20 trace gravel, moist (CL) SS 14-15.7 2.5* 118.3 S-17: (LL, PL, PI) = 25, 12, 13 17-23 % Gravel = 3 % Sand = 19 % Silt = 51 % Clay = 27 16-SS 13.3 6.23 124.0 16-18

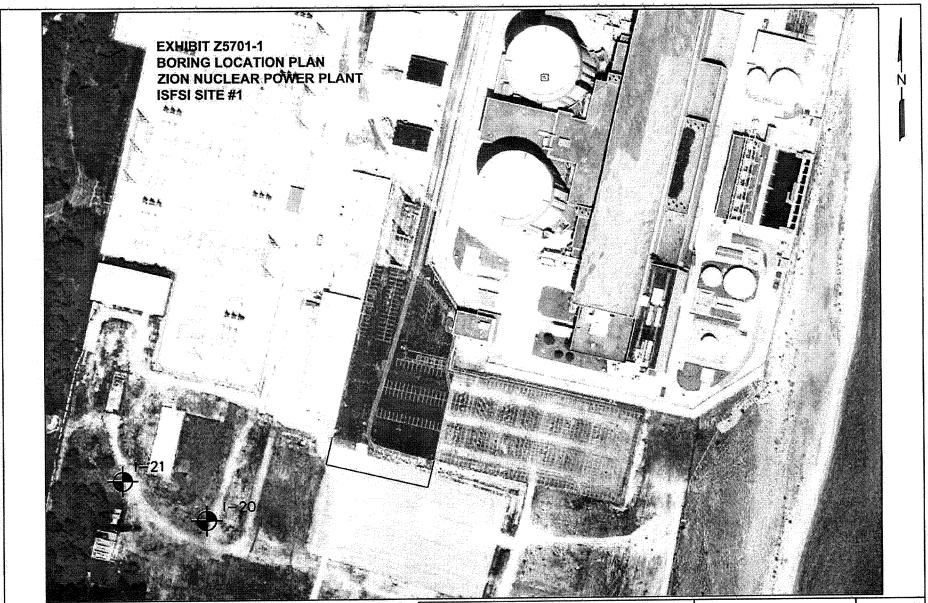
DRILL RIG NO. 315

72070,GPJ TSC_ALL.GDT 10/2/08

Division lines between deposits represent approximate boundaries between soil types; in-situ, the transition may be gradual.

PROJECT ISFSI Zion Nuclear Power Plant CLIENT Sargent & Lundy, LLC **BORING** 1-21 8-18-08 DATE COMPLETED 8-18-08 DATE STARTED **JOB ELEVATIONS** WATER LEVEL OBSERVATIONS 587.1 WHILE DRILLING 5.5' **GROUND SURFACE** END OF BORING 487.1 AT END OF BORING Rotary Wash 24 HOURS **Grouted Upon Completion** SAMPLE YDRY DEPTH ELEV. WC SOIL DESCRIPTIONS NO. TYPE 80 Very tough to hard gray silty CLAY, little sand, trace gravel, moist (CL) 19 SS 4.5+* 10-13.4 18-21 500.1 87.0 SS 11-17.5 3.5* 22-26 Very tough to hard gray very silty CLAY, little DISTANCE BELOW SURFACE IN FEET sand, occasional silt seams, moist (CL-ML) SS 15-13.2 4.5+* 20-25 95 20-SS 10.3 4.5+* 29-37 100 End of Boring at 100.0' * Approximate unconfined compressive strength based on measurements with a calibrated pocket penetrometer. 105 Diedrich D-120 Truck Rig (#315) CME Automatic Hammer 110 115 Division lines between deposits represent

72070, GPJ TSC_ALL. GDT 10/2/08



Barina	Survey Co	ordinates	Elevation (ft)
Boring	Northing (ft)	Easting (ft)	Lievauon (it)
1-20	2,104,754	1,126,708	587.2
I-21	2,104,826	1,126,498	587.1



TESTING SERVICE CORPORATION 457 EAST GUNDERSEN DRIVE CAROL STREAM, ILLINOIS 60188

DRAWN BY: TRP
CHECKED BY: DPD
JOB NO.: L-72,070
DATE: October 2008

1 OF 1

PAGE NO.

GEOPHYSICAL DATA REPORT CROSSHOLE SEISMIC RESULTS ZION NUCLEAR POWER PLANT ZION, ILLINOIS

Prepared for:

TESTING SERVICE CORPORATION
Carol Stream, Illinois

Prepared by:

GEOTECHNOLOGY, INC. St. Louis, Missouri

Geotechnology, Inc. Report No. 0979802.85IS

September 15, 2008

September 15, 2007

0979802.85IS

Mr. Darin Delaney Testing Service Corporation 457 East Gundersen Drive Carol Stream, Illinois 60188

Reference:

Geophysical Data Report

Crosshole Seismic Results Zion Nuclear Power Plant

Zion, Illinois

Dear Mr. Delaney:

Presented herein are the results of the crosshole seismic test for the referenced site. This work was conducted in general accordance with proposal P14676.00.85IS dated July 31, 2008. Presented in this report is a description of the geophysical method and data plots.

It is a pleasure to be of service to you on this project. If you have any questions or comments, please contact the undersigned at (314) 997-7440.

Very truly yours,

GEOTECHNOLOGY, INC.

Glen L. Adams Senior Geophysicist

GLA/DWL:gla/sjc

Copies Submitted: (3)

Douglas W. Lambert, P.G.

Senior Project Manager-Geophysics



0979802.85IS

GEOPHYSICAL DATA REPORT CROSSHOLE SEISMIC RESULTS ZION POWER STATION ZION, ILLINOIS

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Plan of Relativ Shear (Compr Summa Source Geopho	Site and the Borel (S) Wavessional (S) Wavessional (S) Boring (S) Boring (S)	and Topography d Boring Locations lole Locations e Velocity and Shear Modulus vs. Depth (P) Wave Velocity, Poisson's Ratio, and Young's Modulus vs. Depth le – Crosshole Results Deviation ing 1 Deviation ing 2 Deviation APPENDIX	2 4 5 6 7
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GEOPHYSICAL DATA REPORT CROSSHOLE SEISMIC RESULTS ZION POWER STATION ZION, ILLINOIS

1.0 INTRODUCTION

1.1 Site Description. The site consists of an existing nuclear power plant near Zion, Illinois. A site location map is presented on Plate 1. The crosshole borings were located approximately 900 feet south of the switch yard within the plant perimeter fence, as shown on the site plan on Plate 2.

1.2 Scope of Work. The scope of work included mobilizing geophysical equipment and personnel to the site, conducting a crosshole seismic test, and processing/interpreting the data. The seismic data were processed, and shear and compressional wave velocities were calculated to determine dynamic soil properties. The seismic data were plotted and the results are presented in this report. Engineering analysis or recommendations based on this data are outside our scope of work.

2.0 GEOPHYSICAL METHODOLOGY

The crosshole seismic testing method involves generating compressional (P) and shear (S) wave seismic energy at periodic depth intervals within a borehole and measuring the seismic wave travel-times at geophones situated at the source depth between two adjacent boreholes (per ASTM Standard D4428). The seismic wave arrivals at the geophones are transmitted through cables to the seismograph for digital recording. The data are interpreted by analyzing the differences in elapsed travel-time from the source to geophones. P- and S-wave velocities can be calculated for each depth interval by analyzing the travel-time data. The distance between boreholes may vary with depth; therefore borehole deviation logging is conducted to adjust the borehole separation distance with depth for the velocity calculations.

3.0 RESULTS

Crosshole seismic testing was conducted using a source boring and two adjacent receiver (geophone) borings, indicated as Borings I-20 (source), I-20A (Geophone 1), and I-20B (Geophone 2), respectively, as shown on Plate 3. Crosshole data were recorded on 5-foot intervals throughout the depth of the borings. Due to a physical limitation of the crosshole equipment, data could only be collected to a depth of 97-feet from the surface. The borings were cased to

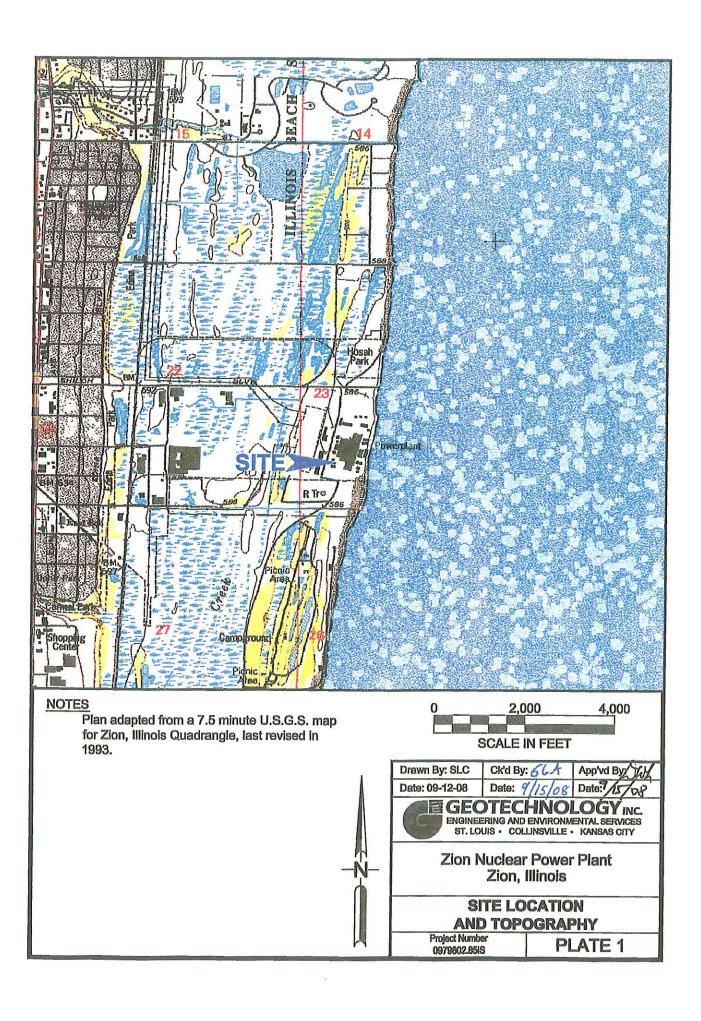


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Testing Service Corporation September 15, 2008 Page 2

approximately 99-feet and the downhole equipment requires approximately 2-feet from the bottom of the casing to collect data.

Seismic velocities calculated between the source boring and the two geophone borings are presented on Plates 4 and 5. A table of all tabulated values and velocities is presented on Plate 6. Plates 7 through 9 are plots of the deviation from vertical for the three borings used for the crosshole test. Due to the inherent noise in the earth, and lateral variations in soil velocities between the test borings, some variations in velocities were calculated between the two receiver holes. An average velocity is plotted on the crosshole data plots that allows a smoother fit to the data. Densities used for the moduli calculations were based on lab values provided by the client as wet unit weights.





NOTES

1. Plan adapted from an aerial photograph courtesy of Google Earth.

LEGEND

Boring Location

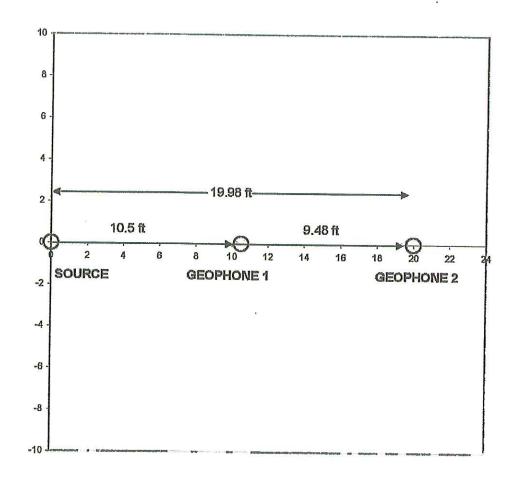


Ck'd By: 6 LA	
Date: 9/15/08	Date 9/15/62
	Ck'd By: 5 LA Date: 9/15/08



Zion Nuclear Power Plant Zion, Illinois

PLAN OF SITE AND BORING LOCATIONS
Project Number
0979802.85is
PLA

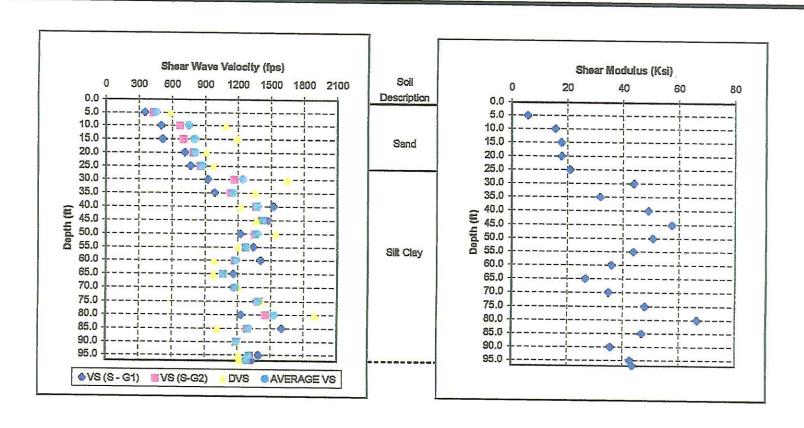




Drawn By: SLC	Ck'd By: GLA	App'vd By
Date: 09-15-08	Date: 9/15/08	Date: 7/5/02
ENGINE	OTECHNO EERING AND ENVIRO OUIS • COLLINSVILL	DLOGY INC. NMENTAL SERVICES E · KANSAS CITY

Zion Nuclear Power Plant Zion, Illinois

RELATIVE BOREHOLE LOCATIONS
Project Number
0979802.85IS

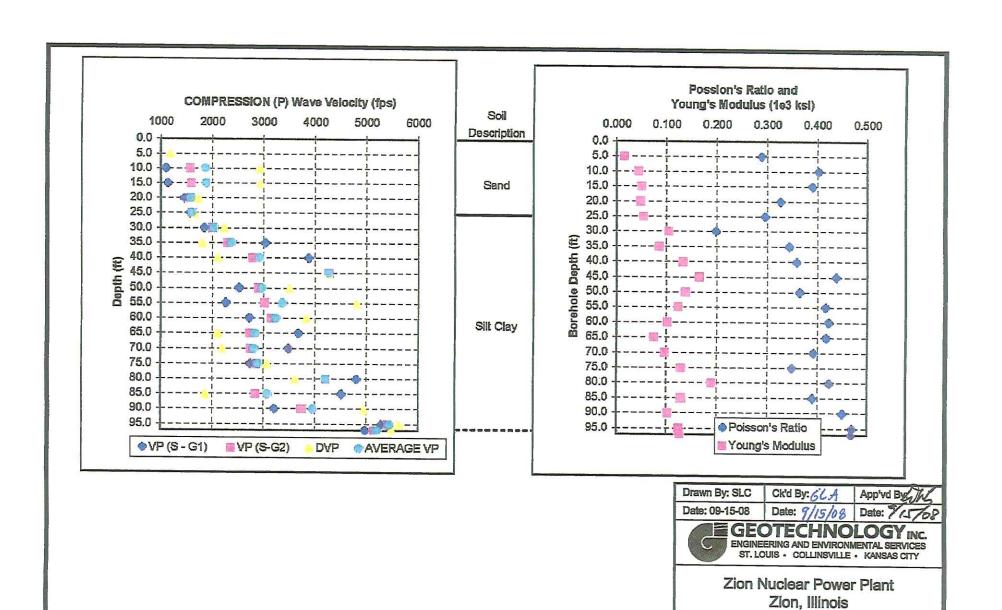




Zion Nuclear Power Plant Zion, Illinois

SHEAR (S) WAVE VELOCITY AND SHEAR MODULUS VS DEPTH

Project Number 0979802.85IS



COMPRESSIONAL (P) WAVE VELOCITY, POISSON'S RATIO, AND YOUNG'S MODULUS VS DEPTH

PLATE 5

Project Number 0979802.85IS

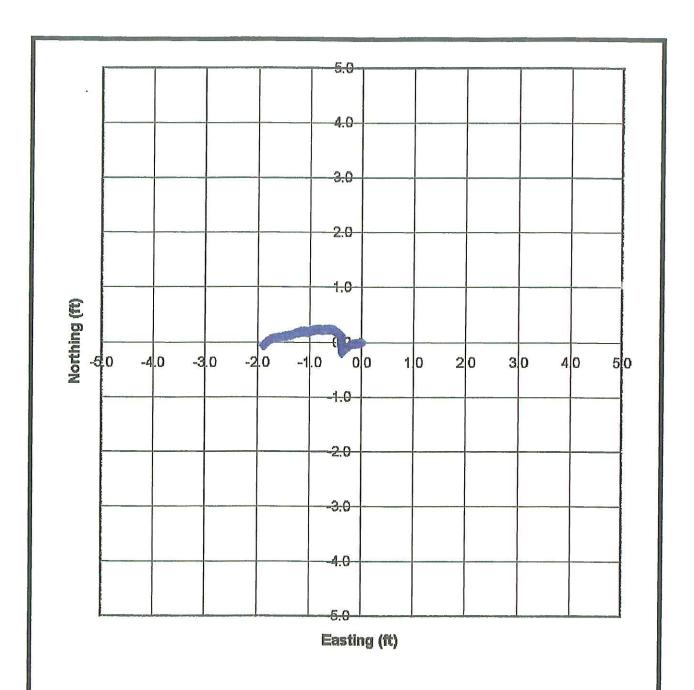
DEPTH (FT)	SHEAR VELOCITY	P-WAVE VELOCITY	UNIT WEIGHT	SHEAR MODULUS	POISSON RATIO	YOUNG'S MODULUS
	(AVERAGE, FT/SEC)	(AVERAGE FT/SEC)	(LB/CU FT)	(KSI)		(KSI)
5	455	836	128	5.75	0 29	14.83
10	752	1865	128	15.73	0.40	44.13
15	802	1890	128	17.88	0.39	49 71
20	807	1590	128	17.98	0.33	47.71
25	866	1609	128	20.84	0.30	54.01
30	1246	2030	130	43.81	0.20	104.91
35	1159	2379	109	31.75	0.34	85.38
40	1371	2921	120	48 95	0.36	133 02
45	1420	4263	131	57 36	0.44	164.92
50	1371	2970	124	50.59	0.36	138.06
55	1272	3360	124	43.52	0.42	123.29
60	1184	3230	118	35 90	0.42	102.12
65	1066	2839	107	26.39	0.42	74.85
70	1181	2808	115	34 78	0 39	96 88
75	1390	2890	114	47.78	0.35	128 98
80	1528	4208	131	66.37	0.42	189.04
85	1302	3073	127	46.70	0.39	
90	1193	3963	115	35.54	0.39	129.88 103.06
95	1304	5448	115	42 46	0.47	124 81
97	1279	5200	122	43.34	0.47	127 23

Drawn By: SLC Ck'd By: 66A App'vd By Ju Date: 09-15-08

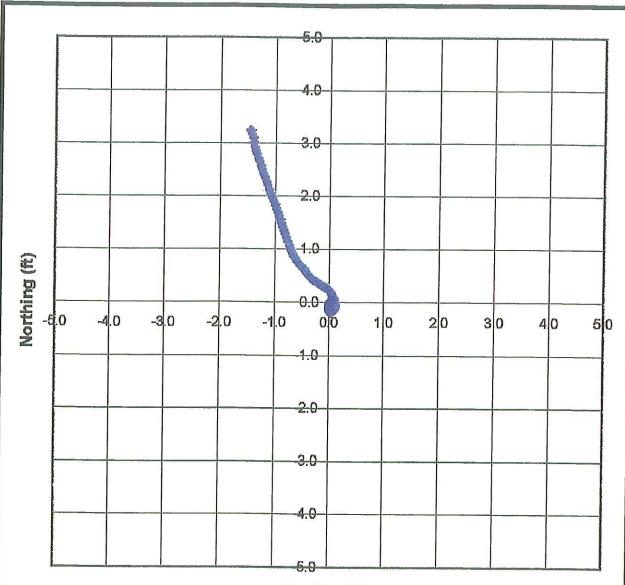


Zion Nuclear Power Plant Zion, Illinois

SUMMARY TABLE -CROSSHOLE RESULTS
Project Number
0979802.85(S)
PLAT

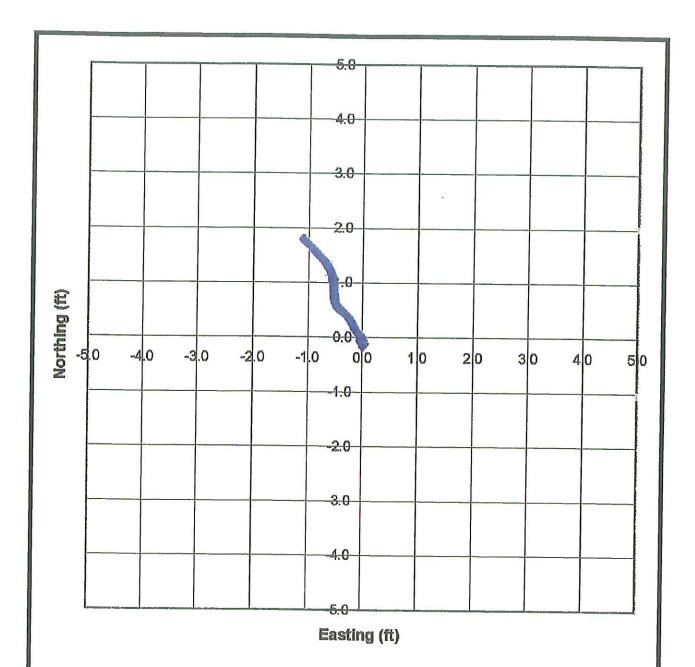






Easting (ft)







APPENDIX A LIMITATIONS OF REPORT

GEOPHYSICAL SERVICES LIMITATIONS OF REPORT

- 1. This report was prepared for the exclusive use of the owner, architect, and engineer for evaluating the project as it relates to the technical aspects discussed herein. It can be made available to prospective contractors for information on factual data only and not as a warranty of subsurface conditions included in this report. Unless other contractual agreements were made, the services described in this report were carried out in accordance with the Terms for Geotechnology's Services which were attached to the proposal.
- 2. Geotechnology endeavored to perform the crosshole seismic geophysical survey in accordance with generally accepted practices of other consultants undertaking similar studies at the same time and in the same geographical area. The findings and conclusions stated herein must be considered not as scientific certainties, but rather as professional opinions concerning the significance of the limited data gathered during the course of the survey. No warranty, express or implied, is made.
- 3. The geophysical analyses and conclusions contained in this report are based on the site conditions, project layout, sampling interval, geophysical data, and interpretive procedures described herein. Geotechnology can make no interpretation of underground conditions beyond the test location. Geophysical exploration methods are indirect and potentially influenced by a variety of natural or man-made conditions. The resulting interpretations are based on the quality of the recorded data as limited by site conditions

Revised 03/09/04

Stdpar\9798crosshole.doc

TESTING SERVICE CORPORATION

457 East Gundersen Drive Carol Stream, Illinois

TSC Job No. L - 72,070 September 9, 2008

CLIENT:

Sargent & Lundy, LLC 55 East Monroe Street Chicago, IL

PROJECT:

ISFSI Project Zion Nuclear power Plant

Zion, IL

SOIL TEST DATA

LOCATION						
BORING NUI	MBER	1-20	I-20	I-20	1-20	1-20
SAMPLE NUI	MBER	2	7	12	22	32
DEPTH IN FE	EET	3.5 - 5.0	16.0 - 17.5	28.5 - 30.0	53.5 - 55.0	78.5 - 80.0
HRB CLASSI	FICATION & GROUP INDEX					
UNIFIED CLA	ASSIFICATION	SP	SP	CL	CL	CL
GRAIN SIZE	CLASSIFICATION					
GRADATION	I - PASSING 1" SIEVE %					
GRADATION	I - PASSING 3/4" SIEVE %	100	100	100	100	
GRADATION	I - PASSING 3/8" SIEVE %	95	99	99	99	
GRADATION	I - PASSING # 4 SIEVE %	92	95	97	99	
GRADATION	I - PASSING # 10 SIEVE %	88	91	95	97	100
GRADATION	I - PASSING # 40 SIEVE %	79	71	85	92	98
GRADATION	I - PASSING # 100 SIEVE %	14	8	77	88	88
GRADATION	I - PASSING # 200 SIEVE %	6	4	73	84	73
SAND AND O	GRAVEL%	94	96	27	16	27
SILT OR SIL	T AND CLAY COMBINED %	6	4	52	50	73
CLAY % (<0.	002 MM)			21	34	
LIQUID LIMIT	Г%			22	31	17
PLASTIC LIM	IIT %			11	12	10
PLASTICITY	INDEX %			11	19	7
NATURAL M	OISTURE CONTENT %			11.0	13.4	10.7
DRY UNIT W	/EIGHT			129.9	123.7	130.7
рН		9.26				
SOLUBLE CH	LORIDES	<50				
SOLUBLE SUI	LFATES	<150				
ORGANIC	L-O-I %					
CONTENT	WET COMBUSTION %					

TESTING SERVICE CORPORATION

457 East Gundersen Drive Carol Stream, Illinois

> TSC Job No. L - 72,070 September 9, 2008

CLIENT:

Sargent & Lundy, LLC 55 East Monroe Street

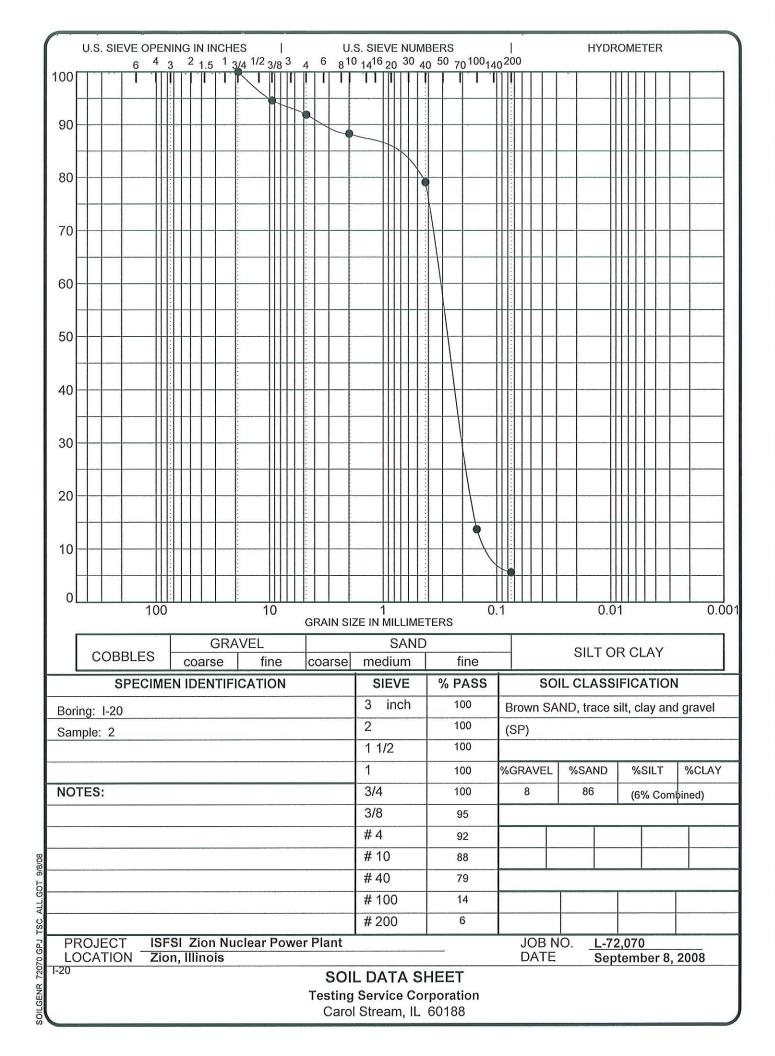
Chicago, IL

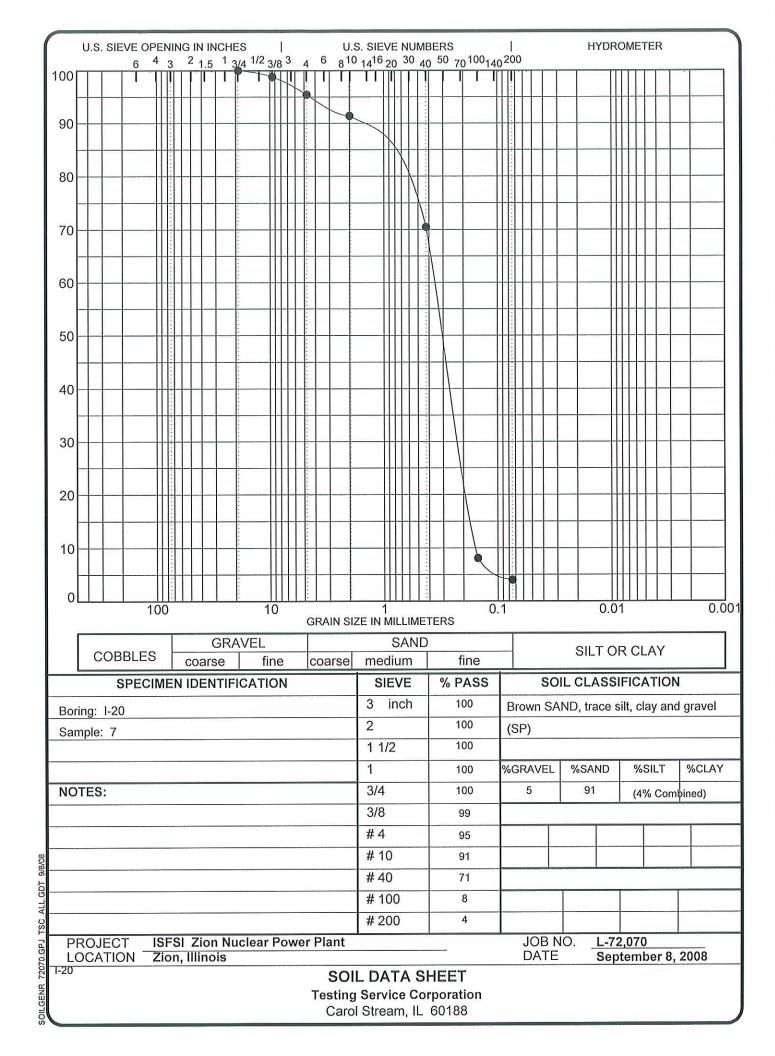
PROJECT: ISFSI Project
Zion Nuclear power Plant

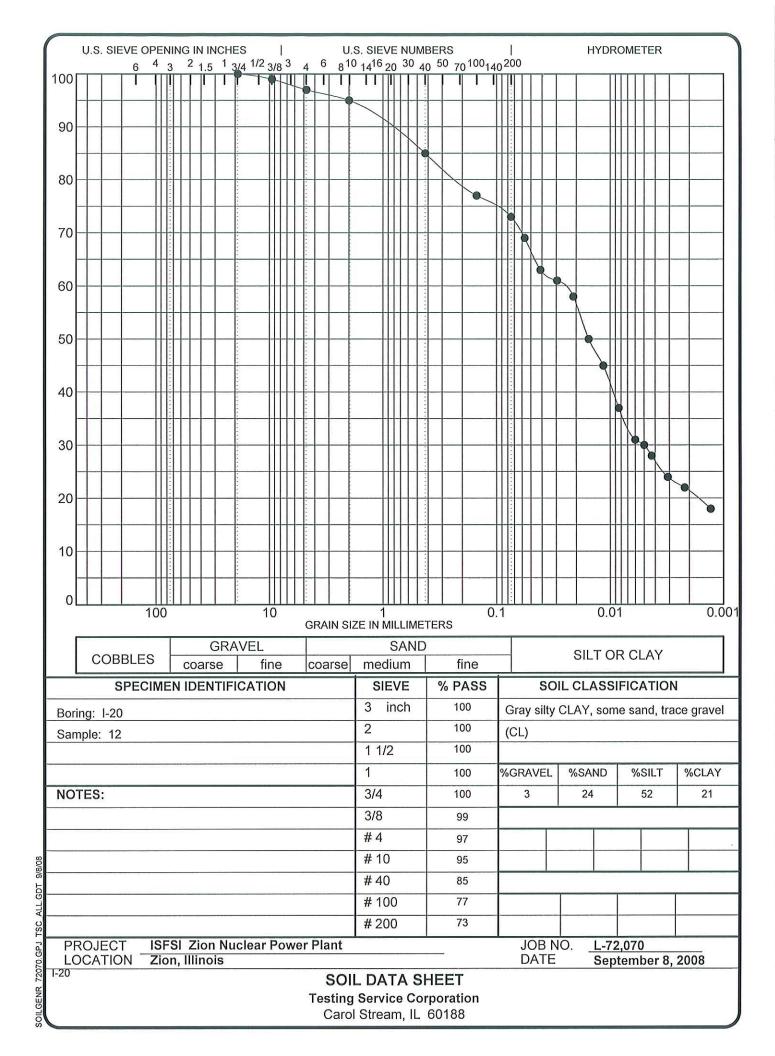
Zion, IL

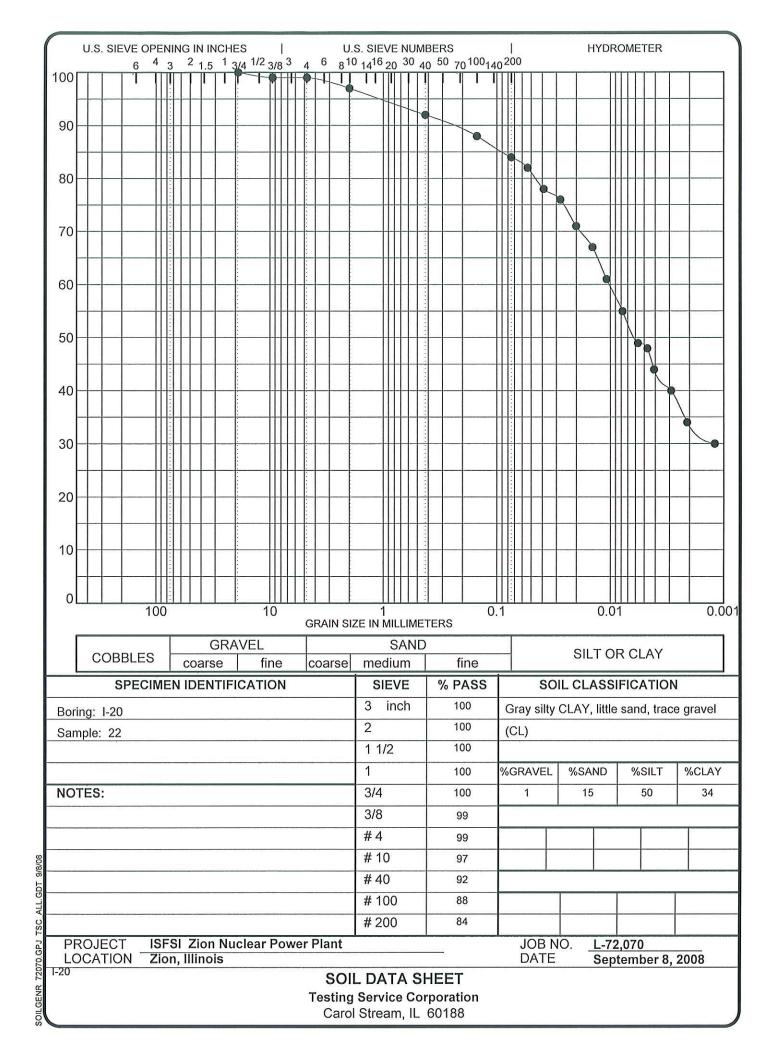
SOIL TEST DATA

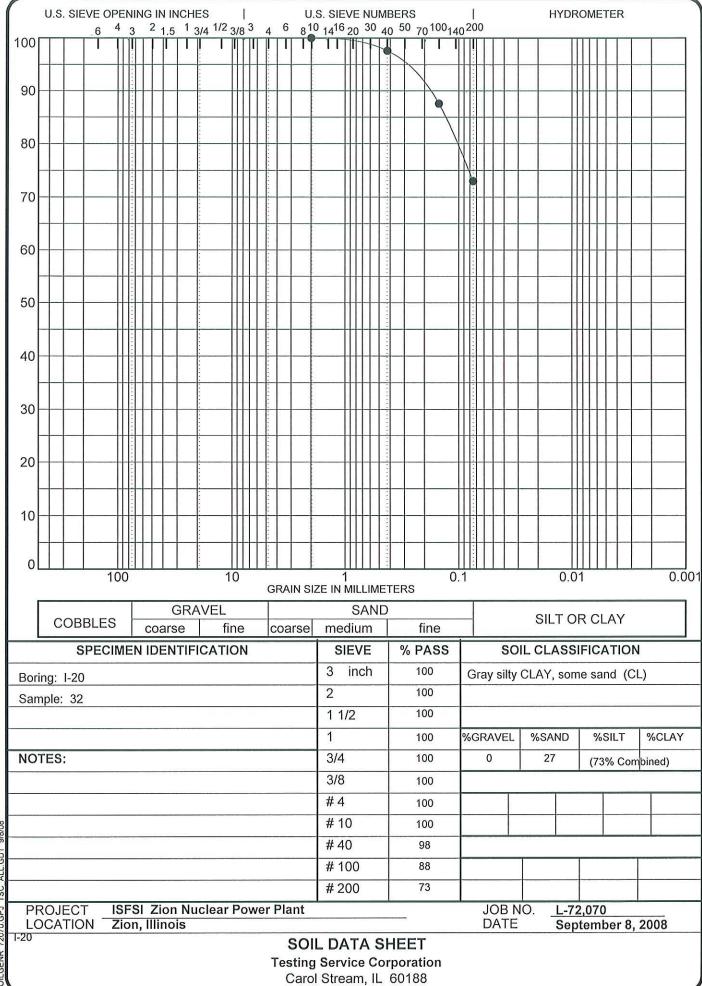
LOCATION					
BORING NUMBER	I-21	I-21	I-21	I-21	I-21
SAMPLE NUMBER	2	5	9	11	17
DEPTH IN FEET					
HRB CLASSIFICATION & GROUP INDEX					
UNIFIED CLASSIFICATION	SP	SP	CL	CL	CL
UNCONFINED COMPRESSIVE STRENGTH (TSF)			5.41	NP	NP
GRADATION - PASSING 1" SIEVE %		86			
GRADATION - PASSING 3/4" SIEVE %		86		100	100
GRADATION - PASSING 3/8" SIEVE %	100	79	100	98	98
GRADATION - PASSING #4 SIEVE %	99	76	99	95	97
GRADATION - PASSING # 10 SIEVE %	99	71	98	93	96
GRADATION - PASSING # 40 SIEVE %	96	62	95	85	89
GRADATION - PASSING # 100 SIEVE %	9	7	92	79	83
GRADATION - PASSING # 200 SIEVE %	4	4	91	76	78
SAND AND GRAVEL%	96	96	9	25	22
SILT OR SILT AND CLAY COMBINED%	4	4	59	51	51
CLAY % (<0.002 MM)			32	24	27
LIQUID LIMIT %			28	23	25
PLASTIC LIMIT %			14	12	12
PLASTICITY INDEX %			14	11	113
NATURAL MOISTURE CONTENT %			12.7	13.0	15.7
DRY UNIT WEIGHT			122.7	124.7	118.3
pH	9.11				
SOLUBLE CHLORIDES	<50				
SOLUBLE SULFATES	<150				
ORGANIC L-O-I %					
CONTENT WET COMBUSTION %					

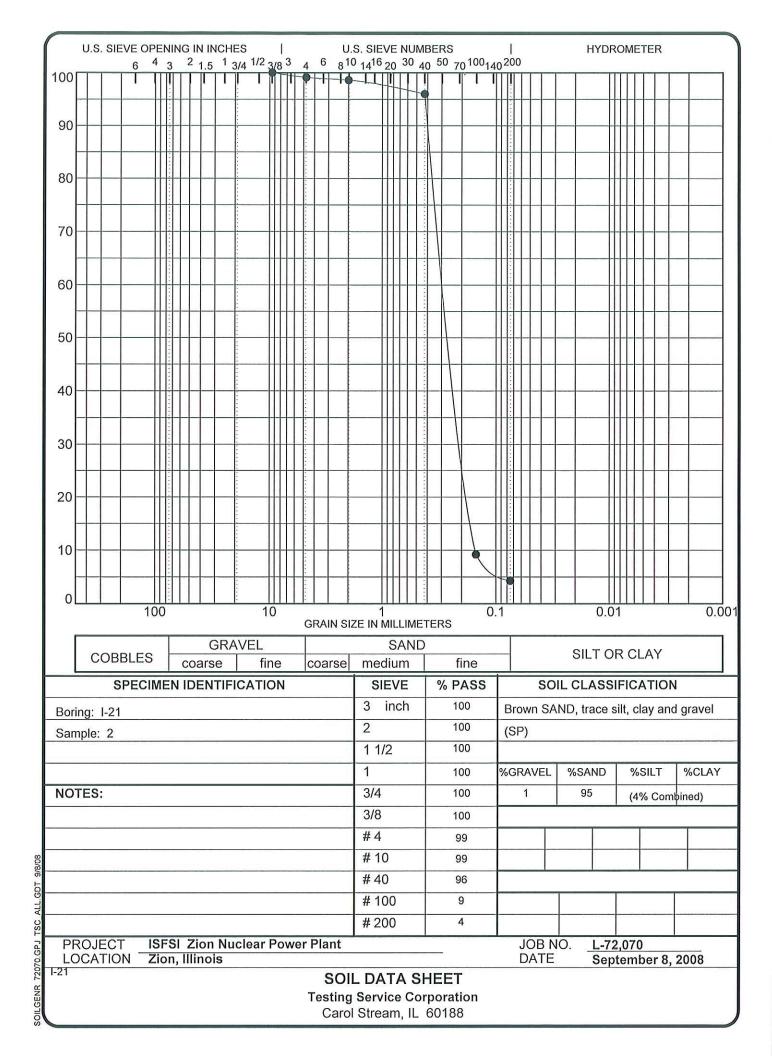


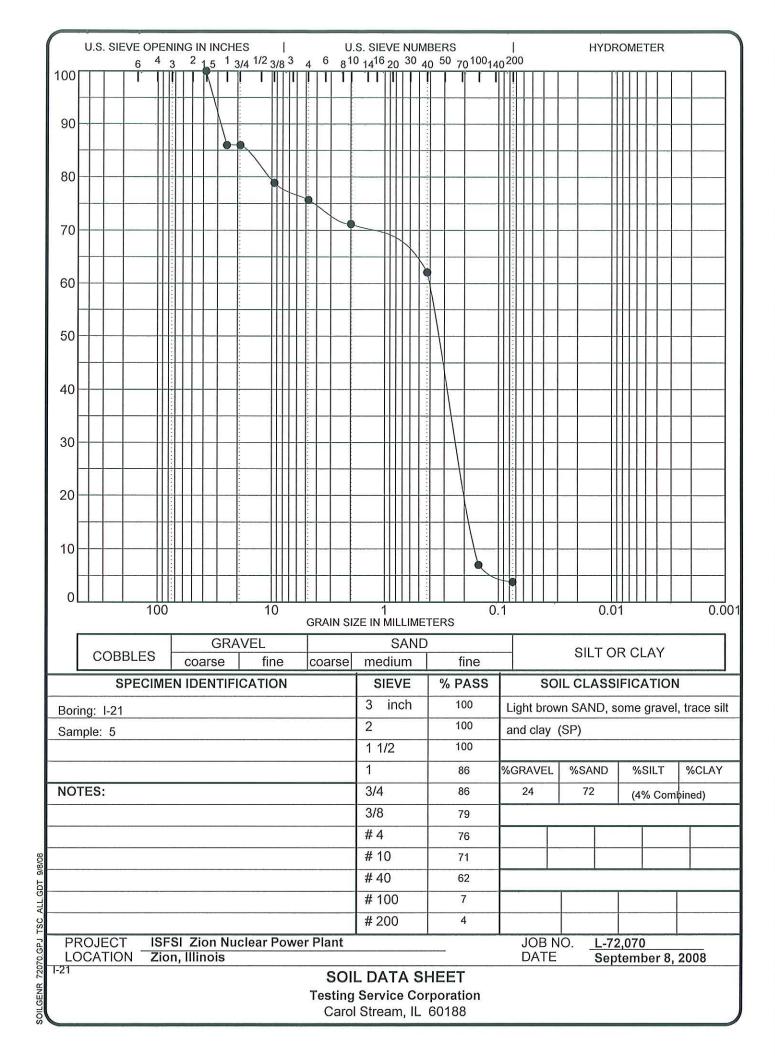


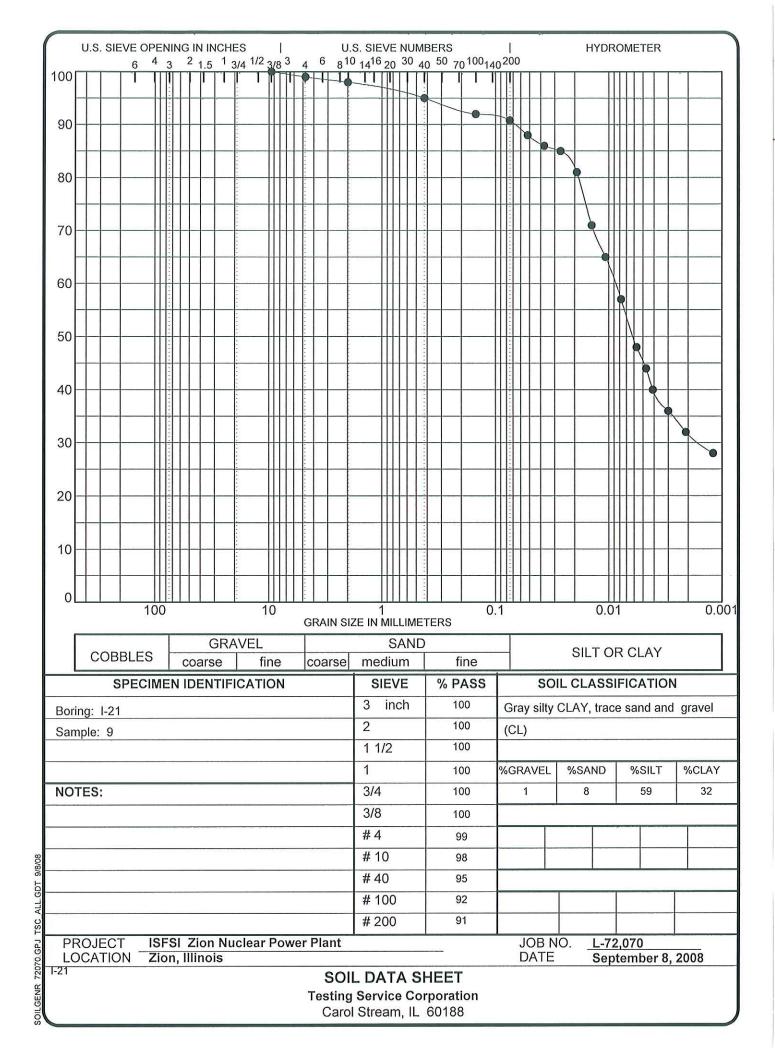


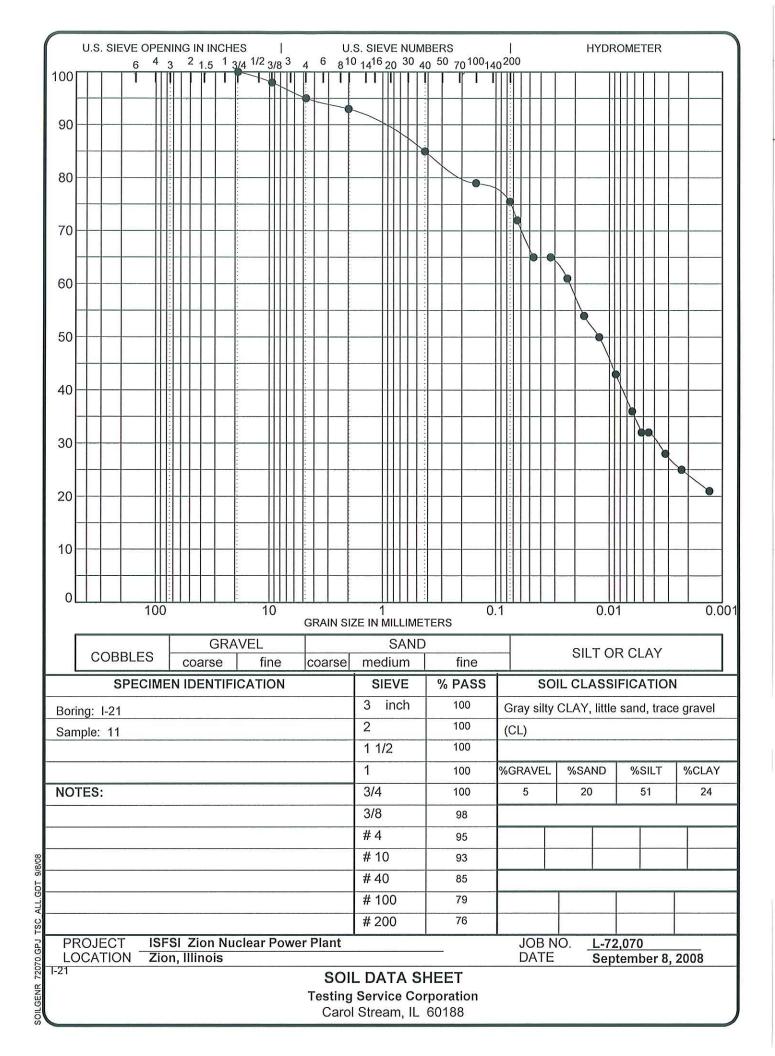


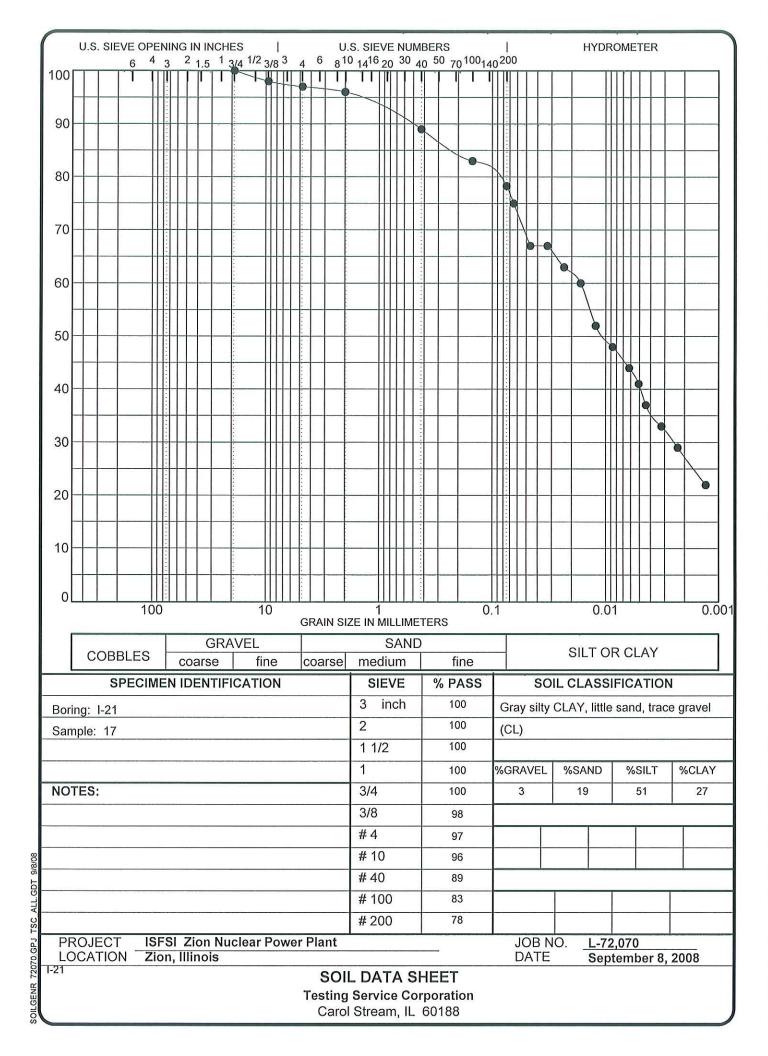












1600 Shore Road • Naperville, Illinois 60563 • Phone (630) 778-1200 • Fax (630) 778-1233

September 03, 2008

Mr. Darin Delaney
TESTING SERVICE CORP.

360 So. Main Place Carol Stream, IL 60188

Project ID: 72070

First Environmental File ID: 8-3902 Date Received: August 27, 2008

Dear Mr. Darin Delaney:

The above referenced project was analyzed as directed on the enclosed chain of custody record.

All Quality Control criteria as outlined in the methods and current IL ELAP/NELAP have been met unless otherwise noted. QA/QC documentation and raw data will remain on file for future reference. Our accreditation number is 100292 and our current certificate is number 002045: effective 05/14/08 through 02/28/09.

I thank you for the opportunity to be of service to you and look forward to working with you again in the future. Should you have any questions regarding any of the enclosed analytical data or need additional information, please contact me at (630) 778-1200.

Sincerely.

Stan Zaworski Project Manager

1600 Shore Road • Naperville, Illinois 60563 • Phone (630) 778-1200 • Fax (630) 778-1233

Case Narrative

TESTING SERVICE CORP.

Project ID:

72070

First Environmental File ID: 8-3902

Date Received:

August 27, 2008

Flag	Description	Flag	Description
<	Analyte not detected at or above the reporting limit.	L+	LCS recovery outside control limits; high bias.
В	Analyte detected in associated method blank.	L-	LCS recovery outside control limits; low bias.
C	Identification confirmed by GC/MS.	M	MS recovery outside control limits; LCS acceptable.
D	Surrogates diluted out; recovery not available.	M+	MS recovery outside control limits high bias; LCS acceptable.
E	Estimated result; concentration exceeds calibration range.	M-	MS recovery outside control limits low bias; LCS acceptable.
F	Field measurement.	N	Analyte is not part of our NELAC accreditation.
		ND	Analyte was not detected using a library search routine; No calibration standard was analyzed.
G	Surrogate recovery outside control limits; matrix effect.	P	Chemical preservation pH adjusted in lab.
Н	Analysis or extraction holding time exceeded.	Q	The analyte was determined by a GC/MS database search.
J	Estimated result; concentration is less than calib range.	S	Analyte was sub-contracted to another laboratory for analysis.
K	RPD outside control limits.	T	Sample temperature upon receipt exceeded 0-6°C
RL	Routine Reporting Limit (Lowest amount that can be detected when routine weights/volumes are used without dilution.)	W	Reporting limit elevated due to sample matrix.

All quality control criteria, as outlined in the methods, have been met except as noted below or on the following analytical report.

Sample Batch Comments:

Sample acceptance criteria were met.



1600 Shore Road • Naperville, Illinois 60563 • Phone (630) 778-1200 • Fax (630) 778-1233

Analytical Report

Client:

TESTING SERVICE CORP.

72070

Project ID: Sample ID:

I-20 S-2

Sample No:

8-3902-001

Date Collected: 08/27/08

Time Collected: 10:00

Date Received: Date Reported:

08/27/08 09/03/08

Analyte	Result	R.L.	Units	Flags
Chloride, Soluble Analysis Date: 09/03/08	Method: 4500CL,C			
Chloride, Soluble	< 50	50	mg/kg	
Sulfate, Soluble Analysis Date: 09/03/08	Method: 4500SO4,E			
Sulfate, Soluble	< 150	150	mg/kg	
pH @ 25°C, 1:10 Analysis Date: 08/28/08 13:30	Method: 4500H+B			
pH @ 25°C, 1:10	9.26		Units	



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Analytical Report

Client:

TESTING SERVICE CORP.

Date Collected:

08/27/08

Project ID:

72070

Time Collected: 10:00

Date Received:

08/27/08

Sample ID: Sample No: I-21 S-2 8-3902-002

Date Reported:

09/03/08

Analyte	Result	R.L.	Units	Flags
Chloride, Soluble Analysis Date: 09/03/08	Method: 4500CL,C			
Chloride, Soluble	< 50	50	mg/kg	
Sulfate, Soluble Analysis Date: 09/03/08	Method: 4500SO4,E			
Sulfate, Soluble	< 150	150	mg/kg	
pH @ 25°C, 1:10 Analysis Date: 08/28/08 13:30	Method: 4500H+B			
рН @ 25°C, 1:10	9.11		Units	

First Environmental Laboratories, Inc.

CHAIN OF CUSTODY RECORD

Page____ of ____ pgs

Labo	ratories, Inc.		Comp	any Na	me:	150	,						
First Environment			Street	Addres	ss:								
600 Shore Road, Suite	City:								State: Zip:				
Naperville, Illinois 6056	Phone:					Fax:				e-mail:	e-mail		
Phone: (630) 778-1200 • E-mail: firstinfo@firste	Send Report To:				Via: Fax								
EPA Certification #100			-	led By:		rin		elav	180				
		Analyses											
	070												
1.0. π			/	ζ,	1/	3/3	7						
				Ž,	./.								
	Soil W = Water O = Other		/ (1	y in	2/							
Date/Time Taken	Sample Description	Matrix		υ							Comments	Lab I.D.	
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8/27/08010:04	I-21 5-2	5							ļ			00	2
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FOR LAB USE ONLY:	I-6°C Yes_ Nø 25.6°C Sa of collection: Re												
Cooler Temperature: 0.1	I-6°C Yes_ Nø C Sa	mple Refrige frigerator Te	erated:	Yes	No	Co	ntaine	rs Rece	eived Pre	eserved:	Yes No		
Ice Present: Yes No	50	35 Vials Fro											
	Fre	eezer Tempe											
Notes and Special Ins	structions:												
Ì	11.	A ->						17/	//	7	/	8/27/04	2 20
Relinquished By:&	Date/T	ime						1-	-0		Date/Time	2/27/04	230
Relinquished By:	Date/T	īme			Rec	eived B	y:				Date/Time	,	
Rev. 4/06													