

APPENDIX A
SUMMARY OF EXISTING GEOTECHNICAL DATA



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BUILDING A BETTER WORLD

TECHNICAL MEMORANDUM

TO: *Mr. Lance Hauer, GE* DATE: *August 12, 2013*
FROM: *Jason Cumbers, PE, MWH* REFERENCE: *1012376*
CC: *Toby Leeson, MWH*
SUBJECT: *Church Rock Mill Site Repository - Summary of Relevant Geotechnical Data*

Background

NRC and DOE comments on the MWH Draft Data Gaps Report (MWH, 2012) recommended a site geotechnical investigation of the foundation materials for the new repository. MWH evaluated existing geotechnical, geological and hydraulic data for the tailings and underlying materials in the vicinity of the proposed repository. Specifically, this information was evaluated relevant to the placement of mine spoils on top of the reclaimed tailings impoundment, in order to develop a focused geotechnical investigation program to obtain information to supplement existing information on the tailings, the underlying alluvium, and the Zone 3 Sandstone.

This memorandum provides a summary of existing data from the materials described above. This summary of data is focused on the area around the conceptual repository layouts and the specific data for the tailings, in the North and Central Cells. Pertinent geotechnical, geological and hydraulic information is summarized herein, and references, where specific test results and borehole information can be found, are presented. Figure 1 shows the locations of pertinent historic borings, wells, and geologic cross sections in the vicinity of the North and Central Cells. Figure 2 shows the locations of the currently existing wells at the site, as well as the locations of aquifer tests performed within the sandstone and alluvium, and permeability tests conducted at soil boring locations. Information on the borehole permeability tests is included in this summary and the existing information from the aquifer tests is further discussed in the documents included in Appendix B.

Table 1 provides a description of the existing relevant geotechnical data by study date and material type. This data summary includes available geotechnical data pertinent to loading of the tailings with mine spoils, and generally does not include data collected in the South Cell. Information in Table 1 is compiled from reports listed in the References section of this memorandum. Table 1 is not a comprehensive summary of all available geotechnical data from the North and Central Cells.

Tailings

Geotechnical data on the tailings in the North and Central Cells are available from several sources. Additional tailings data also exist for the tailings in the South Cell. During the preliminary geotechnical investigation for the impoundment in 1974 (SHB, 1974), a bulk sample

of the cycloned tailings sands was tested for gradation, permeability, and shear strength. The 1978 geotechnical investigation (SHB, 1978a) included drilling borings through the impoundment and through the tailings within the impoundment. Tailings samples were tested for Atterberg Limits, gradations, shear strength, and relative density. In 1979, UNC conducted a stability and integrity assessment of the dam (SHB, 1979). This assessment included a series of seven borings on the interior dikes (Northern Cross Dike and Southern Cross Dike) which separate the three cells. These borings were drilled through the existing tailings and provide thickness information and standard penetration test (SPT) data on the tailings. This investigation also includes data collected in the south cell.

A series of borings (658, 659, 660, and 662) were drilled through the tailings impoundment in 1985. Borings 658, 659, and 660 were located, west to east, across the Central Cell with 660 located within Borrow Pit No. 1. Boring 662 was drilled in the South Cell. These boring logs are included as attachments to a UNC memo (UNC, 1986) and provide tailings thickness information, as well as depths and thickness of fine-grained material within the profile. Testing on the samples collected from these boreholes include specific gravity, water content, dry density, and consolidation. This data was partially summarized in the impoundment Reclamation Plan (Canonie, 1991).

In 1992, a series of shallow borings were drilled in the tailings of the Central Cell, to aid in evaluation of the radon modeling (UNC, 1993). Geotechnical samples collected from shallow depths (less than 8 feet deep) were tested for specific gravity, water content, dry density, and gradation. No boring logs for this program were available for review. Interim stabilization of the Central Cell was completed in 1991, and tailings samples were collected in November 1992. Ground surface elevations are not provided; however, the borings appear to have been drilled from the interim cover surface. This tailings data was included in Appendix B of the Central Cell Final Reclamation As-Built Report (Canonie, 1995).

Based on the data for the North and Central Cells, the average specific gravity of the tailings samples is 2.71 and in general the samples are non-plastic. Fine-grained tailings samples have about twice the percentages (average by weight) passing the No. 100 (46 percent) and No. 200 (31 percent) sieves, and 6 percent finer than 0.001 mm, as compared with the coarse-grained samples. With the exception of one sample, both the coarse and fine fractions of the tailings samples were finer than the No. 10 sieve.

The water contents of the samples tested range from 4 to 60 percent with dry densities ranging from 73 to 118 pounds per cubic foot (pcf). Average water content of the coarser samples tested is 15 percent with an average dry density of 101 pcf. The average water content of the finer samples tested is 21 percent with an average dry density of 96 pcf. The reported coefficient of consolidation (C_c) results range from 0.018 to 1.00 for the tailings samples and the friction angles (from direct shear testing) range from 30° to 39°, with some results showing cohesion.

Embankment (North and Central Cells)

A series of borings (78a-15, 17, 18, 19, 20, 21) were drilled through the tailings embankment (SHB, 1978a), presumably to provide information on the materials and construction of the embankment. Borings 15, 17 and 18 were drilled adjacent to the Central Cell. Borings 19, 20, and 21 were drilled through the embankment on the north side of the North Cell. Data from these borings include SPT, torvane shear strength, Atterberg limits, gradations, water contents, dry density, triaxial and direct shear, and laboratory permeability.

The 33 samples from the embankment adjacent to the North and Central Cells are generally classified as low plasticity clay (CL). Atterberg limits for the embankment soils indicate the liquid limits range from 23 to 42 percent and the plasticity indices range from 8 to 22 percent. The average of the plastic Atterberg limits results is a liquid limit of 31 percent and a plasticity index of 13 percent. The percentage passing the No. 200 sieve (fines) ranges from 50 to 77 percent (by weight) and the percentage passing the No. 4 sieve (sand) ranges from 98 to 100 percent. The average of the embankment soils tested indicate 66 percent fines by weight and 100 percent sand size particles, or smaller, by weight. The water content for the embankment samples ranges from 5 to 24 percent with dry densities ranging from 107 to 126 pcf. The average water content is 13 percent and the average dry density is 114 pcf. Direct shear test results on the embankment materials include phi angles of 7°, 49°, and 38° with cohesions of 1.45 kips per square foot (ksf), 1.69 ksf and 0.31 ksf, respectively.

Alluvium

Many of the site drilling programs previously performed in the area of the North and Central Cells include information from both drilling and laboratory testing on the alluvium underlying the tailings, the embankment and the existing cover at the site. Extensive data was collected on the alluvium including field and laboratory permeabilities, Atterberg Limits, gradations, water contents, consolidation tests, Proctor compaction tests, and shear strength. The SHB investigations (SHB, 1974, 1976, 1978a, 1978b, 1979) include laboratory tests on more than 200 alluvium samples taken from the vicinity of the North and Central Cells. While portions of the alluvium were excavated for construction at the site, several of the borings appear to extend below the current depths of tailings in the area of interest and provide geotechnical data on materials still in-place below the impoundment. Borings with geotechnical data below the estimated tailings depths include SHB-74-04, SHB-76-08, 11, SHB-78a-76, 77, and SHB-78b-07.

The alluvium samples from the North and Central Cells are generally classified as low plasticity clay (CL), but also include plastic and non-plastic silts, as well as silty and clayey sands. Atterberg limits for the plastic alluvial soils indicate the liquid limits of the alluvium range from 20 to 67 percent. The plasticity indices range from 4 to 45 percent. The average of the plastic Atterberg limits results is a liquid limit of 36 percent and a plasticity index of 18 percent, which corresponds to a low plasticity clay. The percentage passing the No. 200 sieve (fines) ranges from 0 to 94 percent (by weight) and the percentage passing the No. 4 sieve (sand) ranges from 1 to 100 percent. The average of the alluvium results indicate 41 percent fines by weight and 80 percent sand size particles, or smaller, by weight. The water content for the alluvium samples ranges from 2 to 31 percent with dry densities ranging from 94 to 106 pcf.

Zone 3 Sandstone

More than 50 geotechnical borings were identified that extend into the Zone 3 Sandstone in the vicinity of the proposed repository (SHB, 1974, 1976, 1978a, 1978b, 1979 and CSI, 1980). The bulk of the sandstone data include SPT data, water contents, and the contact elevations. Geotechnical laboratory data includes gradations, water content and Atterberg limits. Field permeability tests were also performed in the sandstone. Laboratory data on samples from the Zone 3 Sandstone is limited; however, water content results range from 5 to 19 percent, and two Atterberg limits tests indicate the sandstone is non-plastic. Two gradation results indicate 27 percent and 25 percent (by weight) passing the no. 200 sieve and 68 percent and 54 percent passing the No. 4 sieve.

Proposed Borrow Areas

MWH identified nineteen borings previously drilled in/or near the proposed East and West Borrow Areas. These include ten borings (SHB78b-18,19,20,28,30,31,32,33,34 and DH-1,3) in the East Borrow (SHB, 1978b and CSI, 1980) and eight borings (SHB78a-52,53,54,55,56 and DH-6,7,8) in or near the West Borrow (SHB, 1978a and CSI, 1980). After reviewing surface elevations to account for previous borrow operations in these areas, the existing data on the remaining subsurface profile includes depth to rock, SPT, gradations, Atterberg limits, and water contents. Depth of alluvium above the sandstone or siltstone appears to vary between about 0 and 25 feet in the proposed West Borrow and between about 0 and 14 feet in the proposed East Borrow Area. The alluvium in the borrow areas is generally classified as silty clay, sandy clay, silty sand, clayey sand, or clayey silt.

Laboratory data on samples from the lower alluvium includes, water content results ranging from 4 to 9 percent, dry densities ranging from 82 to 103 pcf, and two Atterberg limits tests indicate CL or CL-ML classification (liquid limits of 29 and 25 percent, plasticity indices of 14 and 6 percent). Gradation results indicate between 37 percent and 61 percent (by weight) passing the no. 200 sieve. Strength and consolidation testing was conducted on alluvium samples from DH-1 and DH-3 (CSI, 1980), located in the proposed East Borrow. Visual classification of the alluvium and SPT data are included on the referenced boring logs.

Conclusions

Based on the available data, and the geologic mapping of impoundment Area (Appendix B), the sampling plan has been developed to collect additional stratigraphy data on the thickness of the tailings in areas, where data is limited, specifically Borrow Pit No. 1 and the center of the Central Cell. The laboratory data previously collected provides index properties for the tailings, the embankment, and the underlying alluvium. The objectives of the proposed investigation will be to confirm these index properties and collect additional data on the consolidation and strength properties of the tailings, the strength properties of the alluvium and the embankment, and hydraulic properties (conductivity and soil water characteristic curves (SWCC)) on the tailings, the alluvium, and the Zone 3 sandstone.

Attachments:

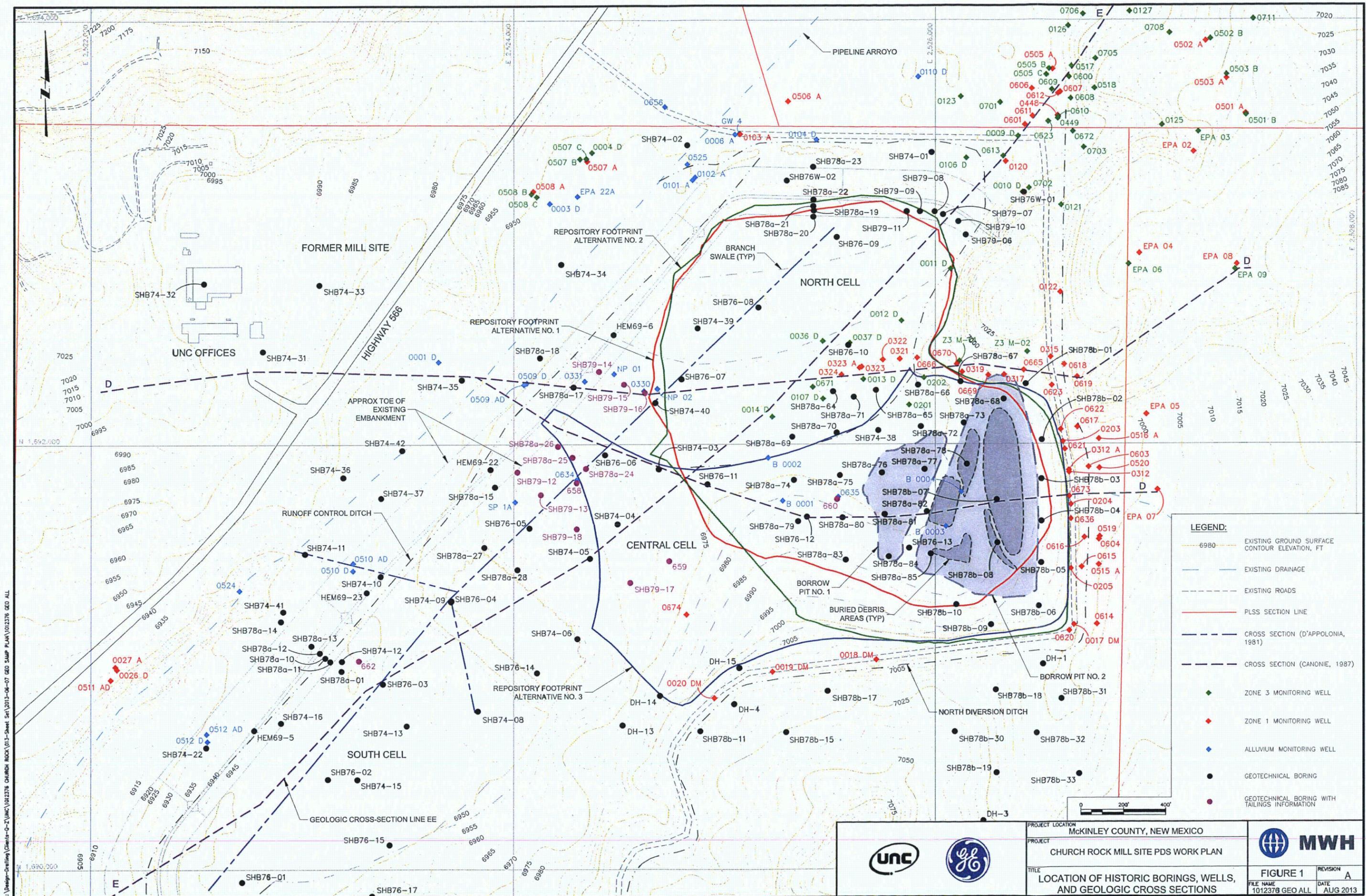
Figure 1 – Location of Historic Borings, Wells, and Geologic Cross Sections

Figure 2 – Location of Existing Wells, and Hydraulic Field Test Data

Table 1 – Church Rock Mill Site Impoundment - Summary of Relevant Existing Geotechnical Data for the North and Central Cells

References:

- Canonie Environmental. 1991. *Tailings Reclamation Plan As Approved by NRC March 1, 1991, License No. SUA-1475, Church Rock Site, Gallup, New Mexico*. 3 Volumes. August.
- Canonie Environmental. 1995. *As-Built Report, Central Cell Final Reclamation, Church Rock Site, Gallup, New Mexico*. June.
- Civil Systems Inc. (CSI). 1980. *Final Design Report Southeast Evaporation Ponds, for United Nuclear Corporation Church Rock Facility, Gallup, New Mexico*. August.
- Hemphill Corporation. 1969. *Report of Soils and Foundation Investigation Church Rock Uranium Mill – United Nuclear Corporation, Gallup, New Mexico, for Kaiser Engineers*. June 30.
- MWH. 2012. *Draft Supplemental Data Needs Evaluation and Work Plans For Removal Design, Northeast Church Rock Mine Site Removal Action*. November 9.
- Sergent, Hauskins & Beckwith (SHB). 1974. *Preliminary Geotechnical Investigation Report, Tailings Dam, Church Rock Uranium Mill, United Nuclear Corporation*. Church Rock, New Mexico. October.
- Sergent, Hauskins & Beckwith (SHB). 1976. *Geotechnical Investigation Report, Tailings Dam and Ponds, Church Rock Uranium Mill, United Nuclear Corporation*. Church Rock, New Mexico. May.
- Sergent, Hauskins & Beckwith (SHB). 1978a. *Geotechnical and Design Development Investigation Report, Tailings Dam and Ponds, Church Rock Uranium Mill, United Nuclear Corporation*. Church Rock, New Mexico. July.
- Sergent, Hauskins & Beckwith (SHB). 1978b. *Engineering Analysis Report – Embankment Volumes-Borrow Quantities, Tailings Disposal Systems Analysis, UNC Church Rock Mill Site*. Church Rock, New Mexico. October.
- Sergent, Hauskins & Beckwith (SHB). 1979. *Geotechnical Investigation Report, Stability and Integrity Assessment, Church Rock Uranium Mill, United Nuclear Corporation*. Church Rock, New Mexico. Volume 1. July.
- UNC Mining and Milling. 1986. *Letter to Canonie Environmental Services Corporation, Re: Previous Geotechnical Data – Tailings & NECR*. October 14.
- United Nuclear Corporation. 1993. *Letter to Canonie Environmental Services Corporation*. September 28.



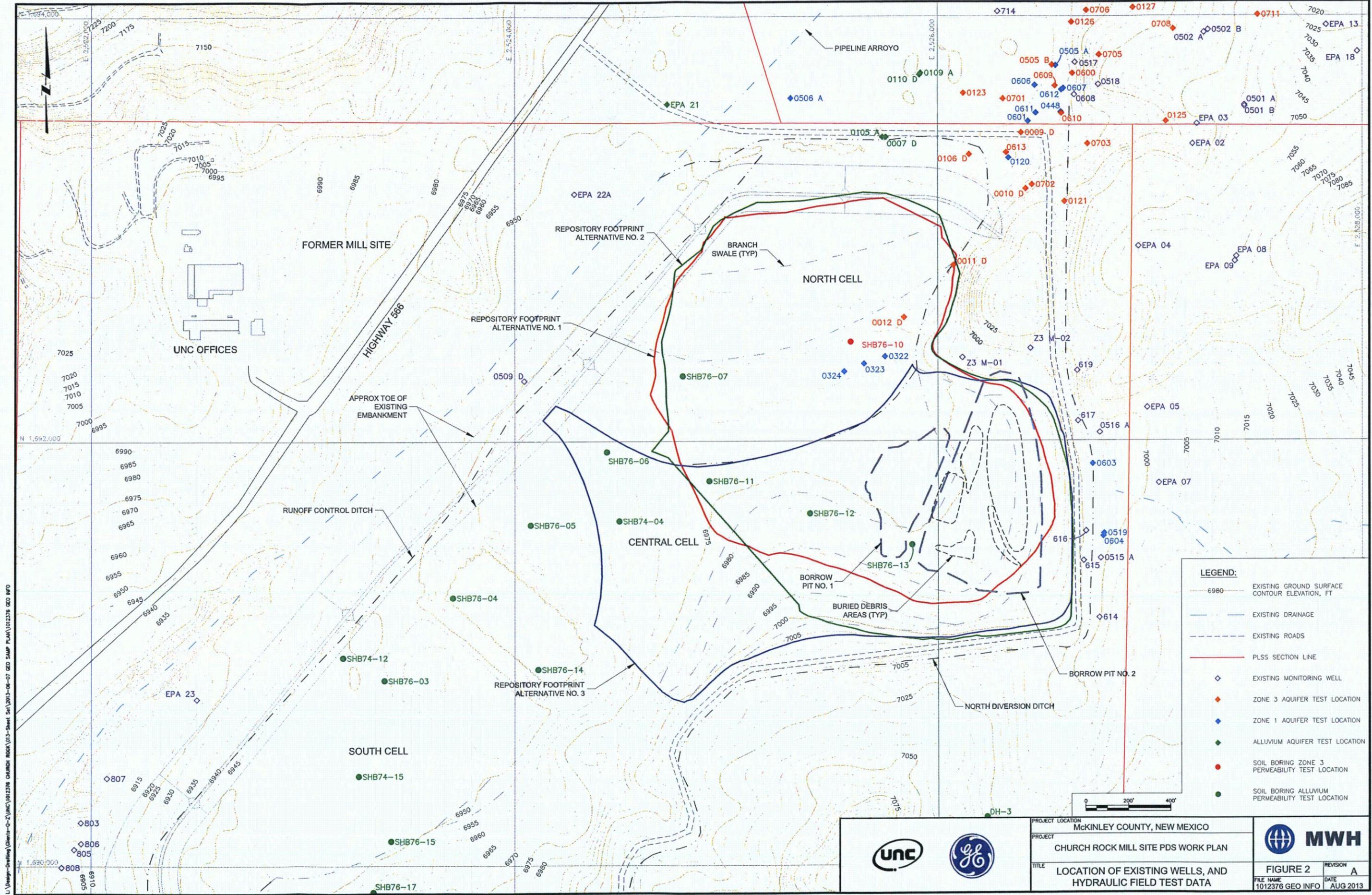
LEGEND:

- 6980 — EXISTING GROUND SURFACE CONTOUR ELEVATION, FT
- EXISTING DRAINAGE
- - - EXISTING ROADS
- PLSS SECTION LINE
- - - CROSS SECTION (D'APPOLONIA, 1981)
- - - CROSS SECTION (CANONIE, 1987)
- ◆ ZONE 3 MONITORING WELL
- ◆ ZONE 1 MONITORING WELL
- ◆ ALLUVIUM MONITORING WELL
- GEOTECHNICAL BORING
- GEOTECHNICAL BORING WITH TAILINGS INFORMATION

PROJECT LOCATION McKINLEY COUNTY, NEW MEXICO		
PROJECT CHURCH ROCK MILL SITE PDS WORK PLAN		
TITLE LOCATION OF HISTORIC BORINGS, WELLS, AND GEOLOGIC CROSS SECTIONS		FIGURE 1 REVISION A FILE NAME 1012376 GEO ALL DATE AUG 2013



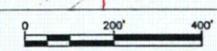
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LEGEND:

- 6980 EXISTING GROUND SURFACE CONTOUR ELEVATION, FT
- EXISTING DRAINAGE
- - - EXISTING ROADS
- PLS SECTION LINE
- ◇ EXISTING MONITORING WELL
- ◇ ZONE 3 AQUIFER TEST LOCATION
- ◇ ZONE 1 AQUIFER TEST LOCATION
- ◇ ALLUVIUM AQUIFER TEST LOCATION
- SOIL BORING ZONE 3 PERMEABILITY TEST LOCATION
- SOIL BORING ALLUVIUM PERMEABILITY TEST LOCATION



	PROJECT LOCATION MCKINLEY COUNTY, NEW MEXICO	
	PROJECT CHURCH ROCK MILL SITE PDS WORK PLAN	
TITLE LOCATION OF EXISTING WELLS, AND HYDRAULIC FIELD TEST DATA		FIGURE 2 REVISION A
FILE NAME 1012376 GEO INFO		DATE AUG 2013

Table 1 - Church Rock Mill Site Impoundment - Summary of Relevant Existing Geotechnical Data for the North and Central Cells

Report Reference	Boring no.	Surf. Elev. (ft)	Sample Depth (ft)	Formation	Material Type	USCS	Field Data							Lab Data										Location				
							SPT (bpf)	Torvane (tsf)	Perm (ft/year)	SG	LL (%)	PI (%)	(%) p.001 mm	(%) p.200	(%) p.100	(%) p.No.10	(%) p.No.4	w.c. (%)	Dry Density (pcf)	Consol	Std. Proctor	rel. density (pcf) min-max	Triax.		Dir. Shear (phi, c (ksf))	Perm (ft/year)(1)		
	DH-2	7082	5	alluvium	Sandy Silt	ML	6										5.3	78									S. Central	
	DH-2	7082	6	alluvium	Sandy Silt	ML											4.5	93				UU					S. Central	
	DH-2	7082	8	alluvium	Sandy Silt	ML	8										4.6	88									S. Central	
	DH-2	7082	11	alluvium	Sandy Silt	ML	17										4.9	105									S. Central	
	DH-2	7082	15	alluvium	Sandy Silt	ML	17										7.8	88									S. Central	
	DH-2	7082	22	sandstone	Silty Sand	-	50/1"																				S. Central	
	DH-2	7082	25	sandstone	-	-	50/1"																				S. Central	
	DH-2	7082	30	sandstone	-	-	50/1"																				S. Central	
	DH-3	7043	2	alluvium		SM-ML							37	76		100	11.7	105	C								E. Borrow	
	DH-3	7043	5	alluvium	Sandy Clayey Silt to Clayey Silty Sand	SM-ML	12		2.66								6.5	97	S								E. Borrow	
	DH-3	7043	6	alluvium		SM-ML	12																					E. Borrow
	DH-3	7043	8	alluvium		SM-ML	13																					E. Borrow
	DH-3	7043	12	alluvium	Sandy Clayey Silt	ML	9					24	87	96		99	9.4	90									E. Borrow	
	DH-3	7043	13	alluvium	Sandy Clayey Silt	ML											7.1	83	C								E. Borrow	
	DH-3	7043	16	alluvium	Sandy Clayey Silt	ML	10										5.8	94									E. Borrow	
	DH-3	7043	20	alluvium	Sandy Clayey Silt	ML	11						37	66		90	5.5	103				CU		416			E. Borrow	
	DH-3	7043	25	alluvium	Sandy Clayey Silt	ML	14										6.1	87									E. Borrow	
	DH-3	7043	26	alluvium	Sandy Clayey Silt	ML											5.4	88				UU					E. Borrow	
	DH-3	7043	29	alluvium	Sandy Clayey Silt	ML						14	55	91		100											E. Borrow	
	DH-3	7043	31	alluvium	Sandy Clayey Silt	ML	12										6.5	90									E. Borrow	
	DH-3	7043	35	alluvium	Sandy Clayey Silt	ML	16																				E. Borrow	
	DH-3	7043	46	total depth	Sandy Silt to Silty Sand	SM-ML	46																				E. Borrow	
	DH-4	7019	4	alluvium	Silty Sand to Sandy Silt	SM-ML	50																				S. Central	
	DH-4	7019	6	siltstone	-	-	50																				S. Central	
	DH-4	7019	8	siltstone	-	-	50																				S. Central	
	DH-4	7019	10	siltstone	-	-	100																				S. Central	
	DH-4	7019	15	siltstone	-	-	50																				S. Central	
	DH-4	7019	20	siltstone	-	-	50																				S. Central	
	DH-4	7019	25	siltstone	-	-	50																				S. Central	
	DH-4	7019	29	sandstone	-	-	100																				S. Central	
	DH-4	7019	40	total depth	-	-	100																				S. Central	
	DH-6	7012	4	alluvium	Sandy Clayey Silt	ML	17																				W. Borrow	
	DH-6	7012	6	alluvium	Sandy Clayey Silt	ML	50																				W. Borrow	
	DH-6	7012	8	alluvium	Sandy Clayey Silt	ML	50																				W. Borrow	
	DH-6	7012	10	alluvium	Sandy Clayey Silt	ML	50																				W. Borrow	
	DH-6	7012	15	siltstone	-	-	50																				W. Borrow	
	DH-6	7012	20	siltstone	-	-	50																				W. Borrow	
	DH-6	7012	25	siltstone	-	-	100																				W. Borrow	
	DH-6	7012	30	siltstone	-	-	100																				W. Borrow	
	DH-6	7012	40	total depth	-	-	100																				W. Borrow	
	DH-7	7022	4	alluvium	Clayey Silt to Sandy Clayey Silt	ML	50																				W. Borrow	
	DH-7	7022	6	siltstone	-	-	50																				W. Borrow	
	DH-7	7022	9	siltstone	-	-	50																				W. Borrow	
	DH-7	7022	11	siltstone	-	-	50																				W. Borrow	
	DH-7	7022	15	siltstone	-	-	50																				W. Borrow	
	DH-7	7022	20	sandstone	-	-	50																				W. Borrow	
	DH-7	7022	25	siltstone	-	-	50																				W. Borrow	
	DH-7	7022	30	siltstone	-	-	50																				W. Borrow	
	DH-7	7022	40	siltstone	-	-	50																				W. Borrow	
	DH-7	7022	50	total depth	-	-	50																				W. Borrow	
	DH-8	6998	4	alluvium	Clayey Silt	ML-CL	50																				W. Borrow	

