PMSTPCOL PEmails

From: Tai, Tom
Sent: Monday, November 07, 2011 4:53 PM
To: Steingass, Timothy
Cc: Norato, Michael; STPCOL
Subject: FW: Section 3.7 & 3.8 Actions - Updated November 3, 2011
Attachments: 03.08.01-11_Draft.pdf; 3.7 & 3.8 Punch List 110311.pdf

Tim,

Attached for your preview is the draft proposed response to the containment ISI program. Please be ready to provide any feedback to NINA/S&L on the Wednesday (Nov 9) 2 pm telephone conference.

Thanks

Tom Tai
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From: Price, John E [mailto:jeprice@STPEGS.COM]
Sent: Monday, November 07, 2011 4:31 PM
To: Tai, Tom
Cc: Thomas, Brian; Chakrabarti, Samir; Chakravorty, Manas; Thomas, Steven; Head, Scott; Mookhoek, William; Dujka, David
Subject: Section 3.7 & 3.8 Actions - Updated November 3, 2011

Tom,

The attached document, *3.7 & 3.8 Punch List 110311*, provides scope and status for all known actions required for FSAR Sections 3.7 and 3.8. This document will be used on our weekly telephone conference. The next call is scheduled for Wednesday, November 9, 2011.

This version reflects the results of the discussion of II/I issues during the weekly telephone conference with NRC on 11/02/11 (Punch List Item 129).

During the conference call, NRC tentatively scheduled a "Final" Audit for COLA Chapters 3.7 and 3.8 for the week of January 9, 2012 in S&L's Chicago offices.

The attachment contains the following:

- Sheets 1 through 15, dated November 3, 2011, show the status of actions for FSAR Sections 3.7 and 3.8.
- Sheets 16 through 27, dated October 7, 2011, address Clarification Issues provided by NRC.

Also, please see attached a file *03.08.01-11 Draft* that provides an advanced copy of the Containment ISI Program RAI response with COLA markup. This is currently in a project technical review and we would welcome any feedback or comments from this information during our phone call. Please do not hesitate to contact me with any questions or clarifications. Regards,

John E. Price

Licensing Engineer - STP Units 3 & 4

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Subject: FW: Section 3.7 & 3.8 Actions - Updated November 3, 2011
Sent Date: 11/7/2011 4:53:27 PM
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From: Tai, Tom

Created By: Tom.Tai@nrc.gov

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"Norato, Michael" <Michael.Norato@nrc.gov>
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Files	**Size**	**Date & Time**
MESSAGE	1895	11/7/2011 4:53:30 PM
03.08.01-11_Draft.pdf	37694	
3.7 & 3.8 Punch List 110311.pdf	136640	

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RAI 03.08.01-11

QUESTION:

10 CFR 50.55a(b)(2)(vi) states that licensees may use either the 1992 Edition with the 1992 Addenda or the 1995 Edition with the 1996 Addenda of Subsection IWE and Subsection IWL as modified and supplemented by the requirements in paragraphs (b)(2)(viii) and (b)(2)(ix) of this section when implementing the initial 120-month inspection interval for the containment inservice inspection requirements of this section. In addition, Regulatory Guide 1.206, Section C.III.1, Chapter 5, Section C.I.5.2.4.1 defines the Inservice Inspection (ISI) Program as an operational program, as described in SECY-05-0197, such that the program and its implementation milestones should be fully described in terms of the scope and level of detail to allow for a reasonable assurance finding of acceptability.

The ABWR DCD states that the Containment ISI requirements are in Section 3.8.1.7, however none are addressed in this section. In addition, Section 6.6 of the DCD does not address ISI of the containment structure in any manner. Similarly, the staff noted that the program describing Containment ISI is not addressed in either of the STP RCOLA Sections 3.8.2 or 6.6. Please discuss the Containment ISI Program in either of these sections in sufficient detail for the staff to obtain a reasonable assurance of the acceptability of the ISI inspections for this structure.

RESPONSE:

The Preservice and Inservice Inspection requirements for the Containment Structure including Class MC and metallic shell and penetration liners of Class CC pressure retaining components, and the Class CC reinforced concrete structure are provided in the attached COLA mark-up.

3.8.1.7.3 Preservice and Inservice Inspection

3.8.1.7.3.1 Scope

This subsection describes the Preservice and Inservice Inspection (ISI) Program requirements for ASME B&PV Code, Class CC and MC pressure retaining components of the Containment Structure and their integral attachments. It describes those programs implementing the requirements of the ASME B&PV Code Section XI (ASME Section XI). Subsection IWE of ASME Section XI applies to Class MC components and metallic shell and penetration liners of Class CC pressure retaining components and their integral attachments. Subsection IWL of ASME Section XI applies to the Class CC reinforced concrete structure.

The Preservice and ISI program plans are based on ASME Section XI, Edition and Addenda per the requirements of 10 CFR 50, Section 50.55a. The Containment Structure is designed to provide access for the examinations required by ASME Section XI, IWE-2500 and IWL-2500. The actual Edition of ASME Section XI to be used is specified based on the procurement date of the component per 10 CFR 50, Section 50.55a. The ASME Code requirements discussed in this section are provided for information and are based on the 2004 Edition of ASME Section XI, and the supplemental requirements provided in 10 CFR 50.55a for the 2004 Edition of the ASME Code.

3.8.1.7.3.2 Components Exempted From Examination

During the detailed design phase, the goal is to minimize the number of inaccessible areas and thus reduce the number of ISI exclusion areas. Furthermore, remote tooling will be used in high radiation areas where feasible.

Portions of the Containment Structure are excluded from Preservice and ISI examination requirements of ASME Section XI, Subsections IWE and IWL as follows:

(1) For Class MC components and metallic shell and penetration liners of Class CC components and their integral attachments:

a. Vessels, parts, and appurtenances outside the boundaries of the containment system as defined in the Design Specifications;

b. Embedded or inaccessible portions of containment vessels parts, and appurtenances that meet the requirements of the Edition and Addenda of ASME Section III used for construction;

c. Portions of containment vessels, parts and appurtenances that become embedded or inaccessible as a result of vessel repair/replacement activities if the prerequisites for exemption of inaccessible surface areas under ASME Section XI, IWE-1232 and IWE-5220 are satisfied;

d. Piping, pumps, and valves that are part of the containment system, or which penetrate or are attached to the containment vessel. These components are examined in accordance with the requirements of ASME Section XI, Subsection IWB or IWC, applicable to their classification as defined in the associated Design Specification.

(2) For Class CC reinforced concrete, those portions of the concrete surface that are covered by the liner, foundation material, or backfill, or are otherwise obstructed by adjacent structures, components, parts, or appurtenances.

3.8.1.7.3.3 Accessibility for Examination

For the entire life of the plant, the following portions of Class MC Containment vessels, parts and appurtenances, and Class CC metallic shell and penetration liners shall remain accessible for direct or remote visual examination in accordance with IWE-1230:

(1) Openings and penetrations;
(2) Structural discontinuities;
(3) 80% of the pressure retaining boundary, excluding attachments, structural reinforcement, and areas made inaccessible during construction;
(4) Surface areas requiring augmented examination as identified in IWE-1241.

3.8.1.7.3.4 Preservice Examination

The preservice examinations are performed prior to plant startup but after performance of the Containment Structural Integrity Test. Visual examinations are performed after the application of any required protective coatings. The preservice examinations include those examinations listed in ASME Section XI, Table IWE-2500-1, IWL-2510 and Table IWL-2500-1, per Articles IWE-2200 and IWL-2200.

Per Table IWE-2500-1, examinations for Class MC Components are general visual examinations of accessible surface areas of the pressure retaining boundary including wetted surfaces, accessible portions of Containment Vent System, and moisture barriers such as caulking, flashing and other sealants used to prevent intrusion of moisture into inaccessible areas. Containment surface areas requiring augmented examination are those listed in IWE-1241.

Table IWL-2500-1 for examinations of Class CC concrete requires general visual examination of accessible concrete surface areas and detailed visual examination of suspect surface areas.

3.8.1.7.3.5 Visual Examination Methodology

Visual examination types VT-1 and VT-3 shall be conducted in accordance with ASME Section XI, IWA-2200. When performing IWE examinations remotely, the maximum direct examination distance specified in Table IWA–2210–1 may be extended and the minimum illumination requirements specified in Table IWA–2210–1 may be decreased provided that the

conditions or indications for which the visual examination is performed can be detected at the chosen distance and illumination. The "owner defined" visual examination provisions in IWE-2310(a) shall not be used for VT-1 and VT-3 examinations.

3.8.1.7.3.6 Visual Examination of Surfaces

Visual examinations shall be performed in accordance with IWE-2500 and Table-IWE-2500-1. General visual examinations shall be in accordance with Examination Category E-A. Detailed visual examinations shall be in accordance with Examination Category E-C and performed to assess the initial condition of surfaces requiring augmented examinations and to determine the magnitude of deterioration and extent of any deteriorated or distressed surfaces. The VT-3 examination method shall be used to conduct the examinations in Items E1.12 and E1.20 of Table IWE-2500-1, and the VT-1 examination method shall be used to conduct the examination in Item E4.11 of Table IWE-2500-1. Visual examinations of concrete surfaces shall be performed in accordance with IWL-2300. Visual examinations shall be performed directly or remotely, with adequate illumination, by personnel with visual acuity sufficient to detect evidence of degradation.

3.8.1.7.3.7 Visual Examination of Bolted Connections

A general visual examination of the pressure-retaining bolted connections that are identified as part of Accessible Surface Areas in Item E1.11 of Table IWE-2500-1, Examination category E-A, shall be conducted using the VT-3 examination method once during each interval as defined by IWE-2412 in ASME Section XI. This includes the containment bolted connections that are disassembled during the scheduled performance of the examinations in Item E1.11 of Table IWE-2500-1. As an alternative to performing the VT-3 examinations of containment bolted connections that are disassembled during the scheduled performance of Item E1.11, the VT-3 examinations of containment bolted connections may be conducted whenever the bolted connections are disassembled for any reason. Flaws or degradation identified during the performance of a VT-3 examination must be examined in accordance with the VT-1 examination method. The criteria given in the material specification or in ASME Section XI, IWB-3517.1, shall be used to evaluate containment bolting flaws or degradation.

3.8.1.7.3.8 Ultrasonic Examination

The ultrasonic thickness measurements used for surfaces requiring augmented examination in accordance with ASME Section XI, Table IWE-2500-1, Examination Category E-C, Item E4.12, are conducted using a technique demonstrated on a calibration standard. Methods such as those described in ASTM E 797, Standard Practice for Measuring Thickness by Manual Ultrasonic Pulse-Echo Contact Method, are acceptable. The ultrasonic thickness measurements are performed for both Class MC Components and metallic shell and penetration liners of Class CC components if augmented examination is necessary under the provisions of ASME Section XI, IWE-1240.

3.8.1.7.3.9 Alternative Examination Techniques

As provided by ASME Section XI, IWA-2240, "Alternative Examinations", a combination of methods, or newly developed techniques may be substituted for methods specified for a given item, provided that they are demonstrated to be equivalent or superior to the specified method. This provision allows for the use of newly developed examination methods, techniques, etc., which may result in improvements in examination reliability and reductions in personnel exposure.

3.8.1.7.3.10 Qualification of Examination Personnel and Systems for Ultrasonic Examination

Personnel performing preservice and inservice examinations of the containment system are qualified in accordance with the applicable requirements of ASME Section XI. Personnel shall conduct VT–1 and VT–3 examinations in accordance with IWA–2200 and shall be qualified in accordance with IWA-2300. Personnel performing detailed visual examination and general visual examination of concrete surfaces are qualified in accordance with IWA-2300 to perform examinations as described in IWL-2300. The "owner defined" personnel qualification provisions in IWE-2330(a) and IWL-2310(d) are not approved for use. Ultrasonic examination systems shall be qualified in accordance with an industry accepted program for implementation of ASME Section XI, Appendix VIII, "Performance Demonstration for Ultrasonic Examination Systems".

3.8.1.7.3.11 Inservice Inspection Schedule

The ISI interval for Class MC Components and metallic shell and penetration liners of Class CC components and their supports shall conform to the 10 year inspection interval of Inspection Program B as described in ASME Section XI, IWE-2412 and Table IWE-2412-1. Except where deferral is permitted until the end of an inspection interval as specified in Table IWE-2500-1, the percentages of minimum examinations completed and maximum examinations credited within each period of the interval shall correspond to Table IWE-2412-1.

The ISI of Class CC reinforced concrete is per Section IWL-2400. It shall be performed at 1, 3, and 5 years following the completion of the Containment Structural Integrity Test and every 5 years thereafter in accordance with ASME Section XI, IWL-2510 and Table IWL-2500-1.

3.8.1.7.3.12 Acceptance Criteria and Evaluation of Examination Results

For Preservice and ISI examinations, the requirements of IWE-3000 for ASME Class MC Components and metallic liners and IWL-3000 for ASME Class CC concrete components are used for evaluation of examination results. The ultrasonic acceptance standard of IWE-3511.3 for ASME Class MC Components is applied to metallic liners of Class CC components for containment surfaces requiring augmented examination. The criteria for the evaluation of

containment bolting flaws or degradation are in accordance with IWB-3517.1 or the material specification.

3.8.1.7.3.13 System Pressure Tests

System pressure tests of the Containment Structure are conducted in accordance with ASME Section XI, IWE-5000 and IWL-5000. Per IWE-5221, except as noted in IWE-5222, repair/replacement activities performed on the pressure retaining boundary of the Class MC or Class CC components shall be subjected to a pneumatic leakage test in accordance with provisions of 10 CFR 50, Appendix J. In addition to the Class CC requirements stated in IWL-5000 pertaining to pressure tests, IWL-4000 provides requirements for repair/replacement activities on the concrete containment structure.

3.8.1.7.3.14 Evaluation of Inaccessible Areas

During operation, areas inaccessible for examination for acceptability are evaluated if conditions exist in accessible areas that indicate the presence of or result in the degradation of the inaccessible areas. For each such area identified, the following information is included in the Inservice Inspection Summary report required by ASME Section XI, IWA-6000:

(1) A description of the type and estimated extent of degradation, and the conditions that led to the degradation.

(2) An evaluation of each area and the result of the evaluation.

(3) A description of necessary corrective actions.

Punch List for South Texas Project Units 3 & 4
Sections 3.7 & 3.8

NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
1			Mesh sensitivity Reviewed during March Audit		**03.07.02-25, R1** **03.07.02-29, R1** **03.07.02-22**	3/15/11	May Audit 3.7 - D.
2					**03.07.02-28**	3/15/11	May Audit 3.7 - D.
3	N/A		Provide ITAAC to close 10CFR21 issue related to dynamic analysis of the Turbine Building		**03.07.02-31**	3/28/11	May Audit 3.7 - D.
4			Site Specific Seismic Analyses of Radwaste Building • Amplified input motion considering the effect of nearby Reactor Building • Fixed base seismic analysis for determination of seismic SSE forces for II/I design Not Audited by NRC NRC Complete Audit		**N/A**	N/A	May Audit 3.8 - C. 13.1 3.8 - C. 13.2
5	3.7-1		NRC would like to have an ITAAC for II/I design in addition to the current discussions in COLA, RAI 03.07.02-20		**03.07.02-20 S1**	4/13/11	May Audit 3.7 - D.
6	3.7-3		Check RSW Piping Tunnel SSI analysis description to see if it fully describes how the motion at various points of tunnel were addressed and/or amplified		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - D.
7	3.7-4		Check DGFOS VAULT SSI analysis description to see if it fully describes how the motion at various points of vault were addressed/amplified Provide additional description in COLA, as needed. See Item 3.7-22		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - D.
8		9	In DGFOSV calcuation for SSI analysis, U7-CALC-DSN-6001, Rev. B, the refined model spectra values are 14.6% higher at 3.8 Hz. than those in the base model. The applicant is requested to provide justification for reconciling this difference. Clarification Issue 9, No further action, answered on a previous RAI		**03.07.01-27 S1 R1**	3/7/11	May Audit 3.7 - D.
9	3.7-5		Check DGFOS TUNNEL SSI analysis description to see if it fully describes how the motion at various points of tunnel were addressed and/or amplified. Provide additional description in COLA, as needed. See Item 3.7-22		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - D.
10	3.7-6	2	Identify soil cases used for all SSSI analysis, and provide basis. As a minimum for All SSSI anlaysis upperbound and upperbound backfill should be considered. Review COLA markup for consistency with soil cases analyzed.		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - F. f.
11	3.7-8	2	Why for SSSI of RSW Tunnel was UB in situ used vs UB backfill soil As a minimum for All SSSI anlaysis upperbound and upperbound backfill should be considered. Review COLA markup for consistency with soil cases analyzed.		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - F. f.
12	3.7-36	2	Confirm that as a minimum for All SSSI anlaysis upperbound in-situ and upperbound backfill should be considered. Provide additional information in RAI response as appropriate		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - F. f.

Punch List for South Texas Project Units 3 & 4
Sections 3.7 & 3.8

NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
13	3.7-7	1	Revise Appendix 3A and 3H.6 to reconcile ground water elevation with Chapter 2 Demonstrate that soil springs values due to groundwater elevation reconciliation do not affect foundation design Demonstrate that calculated gap values are not affected by due to groundwater elevation reconciliation for settlement analysis		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - F. g.
14		3	Clearly describing in the FSAR how seismic demand for non-seismic II/I structures for stability evaluation is determined	S&L Javad Moslemian	**03.07.02-13 S3**	7/28/11	
15	3.7-10		Clarify SSSI soil pressures in COLA figures to indicate that they represent envelope of all soil cases analyzed		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - D.
16	3.7-11		Include in FSAR the following information for the time histories developed for 0.3g, 1.60 Reg. Guide spectrum: 1) Development method by reference to DCD; 2) Plots of three acceleration time histories; 3) Comparison of 5% response spectra		**03.07.01-2 S2**	4/13/11	May Audit 3.7 - D.
17	3.7-15		Frequency/acceleration evaluation for UHS columns with and without hydrodynamic mass Determine column accelerations for column mass and hydrodynamic mass based on column frequency and spectra at top and bottom of the columns and revise RAI 03.08.04-30 supplement 1 to report new information	S&L Javad Moslemian	**03.08.04-30 S4**	6/30/11	
18	3.7-18		Effect of structural mesh refinement on maximum acceleration for design In the manual calculation for design of PH Roof slab, increase the vertical seismc load for PH Roof based on examination of structural mesh sensitivity results	S&L Javad Moslemian	**03.08.04-30 S4**	6/30/11	
19	3.7-2		Determine if FSAR (Appendix 3C) revision is required for the use of computer programs used by Westinghouse Will provide information in COLA		**03.07.01-2 S2**	4/13/11	May Audit 3.7 - D.
20		7	Justify comparison of SAP2000 to ANSYS		**03.07.02-29 S1**	5/10/11	May Audit 3.7 - E.
21	3.7-28	6	SASSI Validation - Run aspect ratio problem with reduced shear wave velocity in vertical direction and get transfer function at the center of the slab S&L to revise the SASSI2000 test problem for two way slab action to match that by SGH. Add a cautionary note to SASSI2000 release memo for users to examine transfer functions for any sign of instability		**03.07.02-29 S1**	5/10/11	May Audit 3.7 - E.
22	3.7-19		Provide a figure in COLA showing 0.3g Reg. Guide 1.60 spectra envelopes amplified motions for all three storage vaults		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - D.
23	3.7-22		Include amplified site-specific spectra for RSW Piping Tunnel, DGFOSV, and DGFO Tunnel in FSAR		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - D.
24	3.7-26		Confirm that DGFO tank rigidty requirement is included in the procurement specification		**N/A**	N/A	May Audit 3.7 - F. h.

Punch List for South Texas Project Units 3 & 4
Sections 3.7 & 3.8

NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
25	3.7-27		Consolidate all 03.07.01-27 responses. Also include the spectra comparisons for what was done in the audit for cracked concrete cases. With reader's guide provided, there is no need to consolidate 3.7.1-27. Spectra comparisons will be provided.		**03.07.01-27 S3**	5/17/11	May Audit 3.7 - D.
26	3.7-28	6	SASSI Validation - Run aspect ratio problem with reduced shear wave velocity in vertical direction and get transfer function at the center of the slab		**03.07.02-29 S1**	5/10/11	May Audit 3.7 - E.
27	3.7-33	8	For Es use the SSSI pressure diagram as the driving force in the stability evaluation and using passive on the resisting side for FOS Vault, RSW Piping Tunnel, FOS Tunnel Provide requested information in RAI response	S&L Javad Moslemian	**03.07.02-13 S4**	11/23/11	
28	3.7-35	8	Confirm for FOS Vault that E' is more than the inertial force for amplified site-specific SSI analysis in the stability evaluation Provide requested information in RAI response	S&L Javad Moslemian	**03.07.02-13 S4**	11/23/11	
29			Perform sensitivity analysis related to Poisson's Ratio	NRC	**N/A**	N/A	May Audit 3.7
30		13	Clarification Issue 13 For the Radwaste Building, provide additional information and/or justification for the following: 1. Design and Analysis (3H.3.5.1 & 2) 2. II/I Analysis and design (3H.3.5.3) 3. Load Combination for Shear (Steel) (3H.3.4.3.4.2) 4. Ultimate soil bearing capacity (3H.3.4.2.1) 5. Tornado Parameters (3H.3.B & 3H.3.4.3.3.1) 6. Flood Design (3H.3.4.2.3)		**N/A**	N/A	May Audit 3.8 - C. 13. Punch List 92
31			Reviewed during March Audit		**03.08.04-18**	3/15/11	May Audit 3.8 - C. 13.2 Punch List 92
32			Reviewed during March Audit		**03.08.04-18 R1 S2**	3/15/11	May Audit 3.8 - C. 13.4 Punch List 92
33			Design of DGFOT including design considerations for seismic wave propagation • Design inputs • Structural analysis model • Design of walls, floors, foundations, and other structural components • Stability evaluation Not Audited by NRC in March Audit		**03.08.04-30 S1**	3/15/11	May Audit 3.8 - H.

Punch List for South Texas Project Units 3 & 4
Sections 3.7 & 3.8

NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
34			Design of UHS/RSW Pumphouse • Design inputs • Structural analysis model • Design of walls, floors, foundations, and other structural components • Stability evaluation Not Audited by NRC in March Audit		**03.08.04-30 S1**	3/15/11	May Audit 3.8 - A.
35			During the audit held on October 18 through 22, 2010, the staff noted that the applicant did not include foundation bearing pressure calculations for wind and tornado loadings for UHS/PH The applicant agreed to include the bearing pressure evaluations for wind and tornado in its calculations (CALC-EORA-020, "Dynamic Bearing Capacity Analysis for the UHS Basin and RSW Pump House under Wind and Tornado Loading", Revision 0) for the UHS/PH accordingly.	MACTEC Rob Smith	**CALC-EORA-020, Revision 0**	3/12/11	May Audit 3.8 - A.
36		14	Clarification Issue 14 For all Site-Specific Category I Structures, provide additional information and/or justification for extreme wind and tornado loading (3H.6.4.3.2)		**N/A**	N/A	May Audit 3.8 - C. 14. Punch List 74
37	3.8-1		**RAI 03.08.01-9** +/- 25% of the gap: How does this compare to long term settlement values? Clarify if 25% movement envelopes long term settlement. The 25% movement envelops the expected long term settlement at the RB/CB interface, below flood level Revise response to RAI 03.04.02-6 & 03.08.01-9 to require the testing to be the maximum of +/- 25% or long term settlement.		**03.08.01-9 S1**	4/13/11	May Audit 3.8 - E.
38	3.8-2		For Section 3H.6-7 in RAI 03.08.04-17, Supplement 1, provide clarification for last line of the first paragraph. (*Clarify that envelop of SSSI pressure and ASCE 4-98 is used.)* Revise response to RAI 03.08.04-17 Supplement 1 to clarify that the envelop is used.		**03.08.04-17 S1**	4/26/11	May Audit 3.8 - E.
39	3.8-3	11	RAI 03.08.04-28 Bullet 4: Show the basis for reductions in the dynamic resistance coefficients (Part C of question). Explain in revised response to RAI 03.08.04-28 how values were determined (from MACTEC calculation) Clarification Issue 11 Applicant is requested to provide a comparison between the strain ranges assumed in the above reference and the strain values expected beneath the foundation of Cat I buildings of Units 3&4.		**03.08.04-28 S1**	4/26/11	May Audit 3.8 - E.

Punch List for South Texas Project Units 3 & 4
Sections 3.7 & 3.8

NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
40	3.8-5		**RAI 03.08.04-32** NRC to compare response to other applicant responses.		**N/A**	N/A	Per 6/1/11 Telecon
41	3.8-6		**RAI 03.08.01-7, Rev. 2** Review response - should the pressure loading be based on a flood height of 8' vs. 7'? The pressure loading should be based on a height of 8', but this does not change the result. Revise response to RAI 03.08.01-7 and related responses to clarify this.		**03.08.01-7 S1**	4/13/11	May Audit 3.8 - E.
42	3.8-7		Docket the design parameters table (Agenda Item A)		**N/A**	4/18/11	May Audit 3.8 - B. 1. Punch List 75
43	3.8-8	4 5	Concrete and concrete to waterproofing friction coefficient of 0.6 is based on static, but soil coefficient of 0.47 is based on dynamic. Revise RAI 03.08.04-28 or 03.08.04-19 to show revised required static coefficient of friction of concrete and membrane increased to ≥ 0.75 Clarification Issue 4 Designate whether the friction coefficients reported in Table 3H.6-14 are static or dynamic coefficient of friction values. (Revise response to RAI 3.7.2-13 S1) Clarification Issue 5 (No action required)		**03.08.04-19 S1** **03.07.02-13 S2**	4/26/11	May Audit 3.8 - B. 2. 3.8 - C. 4.
44		16	Clarification Issue 16 Provide additional information and/or justification for specified minimum static coefficient of friction at the various interfaces, specifically at the waterproofing membrane. Also, the requirement of intentionally roughening the concrete surface should be included in the FSAR. Refer to Action Item 3.8-29			N/A	May Audit 3.8 - C. 16. Punch List 77
45	3.8-9		S&L to evaluate method of reconciliation for soil pressure from equivalent pressure method for bearing pressure evaluation and soil pressure from finite element analysis. Formally provide the proposed reconciliation method in response to RAI 3.8.4-35.		**03.08.04-35**	4/11/11	May Audit 3.8 - B. 3.
46	3.8-9		S&L to perform the reconciliation evaluation Formally provide the proposed reconciliation method in response to RAI 3.8.4-35. Refer to Action Item 3.8-38			N/A	May Audit 3.8 - B. 3. 3.8 - C. 15. Punch List 93

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NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
47		15	Clarification Issue 15 Provide additional information and/or justification for foundation bearing pressures reconciliation.			N/A	May Audit 3.8 - B. 3. 3.8 - C. 15. Punch List 93
48	3.8-11		Review the wind loading used for design and stabilty calculations for the vault. NRC to review the justification for the importance factor for wind pressure calculation provided in response to RAI 03.08.04-30 Supplement 1		**N/A**	N/A	May Audit 3.8 - B. 4. Punch List 74
49	3.8-12		Discuss with John Price delivery of the Reviewer's Guide to NRC		**N/A**	4/8/11	May Audit 3.8 - E.
50		17	Clarification Issue 17 Miscellaneous Questions from Audit and Reviewers Guide			N/A	May Audit 3.8 - C. 17. Punch List 75, 78, 79, 80 & 81
51	3.8-15		DGFOSV design calculation: Confirm the tornado wind load used in design, pg. 26 Revise design parameters table (See Item 3.8-7).		**N/A**	N/A	May Audit 3.8 - E.
52	3.8-16		DGFOSV design calculation: Verify acceleration values on pg. 113 with Attachment B Determine the latest revision of the SSI calculation. Ref. 7.19 (design) vs 7.30 (stability). B is the correct revision; reference in stability calculation will be revised in the calculation. (PIP 2011-0365)		**N/A**	N/A	May Audit 3.8 - E.
53		10	As both, the SASSI and the SAP models, should be based on the same geometry and total weight and are subject to the same absolute accelerations, it is not apparent why an additional, relatively large amplification would be needed in vertical direction to obtain comparable total base seismic loads. Therefore, the applicant is requested to provide a justification regarding the different behavior of both structural models.		**03.08.04-30 S2**	5/17/11	May Audit 3.8 - C. 10.
54	3.8-20		Use of newer version of ACI-349 & ASME Section III, Division 2 Codes, Respond to RAI 03.08.04-36		**03.08.04-36**	4/11/11	May Audit 3.8 - E.
55	3.8-21		Beam shear discussion Commitment to incorporate NRC feedback will be provided in an RAI Response		**03.08.04-34**	4/11/11	May Audit 3.8 - E.
56	3.8-21		Beam shear discussion Calculations will be revised and FSAR tables will be updated as a Confirmatory Action COLA updates reflecting the results of the hydrodynamic mass effect on the columns will be included in this response (Punch List No. 17)	S&L Javad Moslemian	**03.08.04-34 S1** **(All but RWB & DGFOSV)**	9/12/11	
57	3.8-22		Attach Supplement to the release memo for SAFE (regarding shear)		**N/A**	N/A	May Audit 3.8 - B. 6.

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NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
58			Audit Report 3.8 K. Computer Software Validation and Verification, PCACOLUMN, Version 4.10 ACI 349-97 used in STP 3&4 design is not covered by this V&V, however it was assured that results from PCAColumn are evaluated in the calcs against the special code requirements in ACI 349-97. The staff needs to review in further detail how this assurance is implemented. This will be included in the next audit.		**N/A**	N/A	May Audit 3.8
59	3.8-23		Verify that 0.21 g used in the basic design of RSW Tunnel envelopes the seismic accelerations Provided SGH calculation showing where the enveloping accelerations came from		**N/A**	N/A	May Audit 3.8 - B. 7. 3.8 - C. 12. Punch List 82
60	3.8-24		RSW Tunnel: Confirm that soil pressures consider additional wave propagation effect Calculation will be revised		**N/A**	N/A	May Audit 3.8 - B. 7. 3.8 - C. 12. Punch List 82
61		12	Applicant is requested to explain the apparent discrepancy with the 3,000 ft/sec sheer wave velocity denoted in the COLA and a value of 6,600 ft/sec used for the wave propogation effect in the calculationscalculation. Calculation will be clarified or revised		**N/A**	N/A	May Audit 3.8 - B. 7. 3.8 - C. 12. Punch List 82
62			Not Used			N/A	N/A
63			Audit report 3.8 I 8. Calculation no. U7-RSW-S-CALC-DESN-6001, Revision D, "Basic Structural Design of Reactor Service Water (RSW) Tunnel". The staff noted that several areas of design of the tunnel were not complete at time of the audit, e.g., global tornado missile impact evaluation for the access region of the tunnel, tunnel walls in the corner region, access covers, etc.		**N/A**	N/A	May Audit 3.8 Punch List 102
64	3.8-25		Revise Control Building Annex stability calculation to eliminate statement regarding design being applicable to DCD Standard Plant Calculation will be revised		**N/A**	N/A	May Audit 3.8 - B. 8.
65		18	Comments on Response to RAI 03.07.02-22 On the COLA Markup, Section 3H.6.6.2.1 UHS basin, UHS Cooling Tower Enclosure, and RSW Pump House, provide an explanation of the pressure distribution for the calculated seismic soil pressures on the RSW Pump House North Wall shown in Figure 3H.6-219 from 2D SSSI.			N/A	May Audit 3.7 - F
66		19	Comments on Response to RAI 03.07.02-24, Supplement 1, Revision 1 On the COLA Markup, Section 3H.6.5.3 Seismic Analysis of RSW Piping Tunnel, in the 4th bullet on the following page (Page 11 of 27), please clarify for which soil case the cracked concrete and soil separation analysis was performed. This information is not provided in the COLA markup. Refer to Action Item 3.7-48			N/A	May Audit 3.8 Punch List 98
67		20	Comments on Response to RAI 03.07.01-27, Supplement 1, Revision 1 On the COLA Markup, Section 3H.6.7 Diesel Generator Fuel Oil Storage Vault (DGFOSV), provide further clarifications why two different input motions are developed and used in separate SSI analyses of DGFOSV			N/A	May Audit 3.7 - F

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NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
68		21	Comments on Response to RAI 03.07.02-29, Supplement 1 Post Audit Clarification Issue #6 submitted as part of the response to RAI 03.07.02-29, Supplement 1. Because the shape of the response, transfer functions are strongly dependent on the a_o value (e.g. as shown in Fig. 03.07.02-29 S1.6 provided with this response), the applicant is requested to extend the results of the above test problems in terms of the foundation response transfer functions (including both compliance and scattering functions) calculated using the subtraction method to a_o values up to at least 8. Issue a revision to the SASSI Release Memo.	S&L Javad Moslemian	**Update Release Memo**	6/30/11	
69		22	DNFSB Issues Related to the SASSI Subtraction Method The Defense Nuclear Facilities Safety Board (DNFSB) issued a letter on April 8th, 2011 requesting the Department of Energy (DOE) to address technical and software quality assurance issues related to potentially erroneous seismic analyses performed using the SASSI Subtraction method. SASSI subtraction method has been used for STP application for embedded structures. As such, the applicant is requested to review these concerns and assess the potential impact on STP's seismic analysis. Refer to Action Items 3.7-37 & 3.7-38			N/A	May Audit 3.8 Punch List 71 & 72
70			Audit Report 3.8 D2 5. "The accelerations are taken from the SSI analysis of the vault. A reference is given in the report from where the accelerations were taken. As this information was not verified during the audit, the staff plans to verify the source and the acceleration values during the next audit."		**N/A**	N/A	May Audit 3.8
71	3.7-37	22	Provide in an RAI response a plan to resolve the issues related to subtraction method in SASSI, as described in the DOE letter. (Clarification Issue 22)	S&L Javad Moslemian	**03.07.01-29**	6/20/11	
72	3.7-38	22	Subsequent to Action Item 3.7-37, revise the results presented in COLA as necessary. (Clarification Issue 22)	S&L Javad Moslemian	**03.07.01-29 S1 Draft**	9/1/11	
73	3.7-39		Revise RWB stability calculation considering amplified motion at ground surface and revise COLA as necessary.	S&L Javad Moslemian	**03.07.02-13 S3**	7/28/11	
74	3.8-26	14	Revise the response for Importance Factor used for wind loading (RAI 03.08.04-30 S1) to state that our position is deviating from SRP 3.3 guidance based on the justification provided in the RAI response. Also revise the Importance Factor from 1.15 to 1.0. (Clarification Issue 14)	S&L P. K. Agrawal	**03.08.04-30 S3**	6/20/11	
75	3.8-27	13.6 17.6	Update the design parameters table with damping values for Radwaste Building Design, and include the table in Section 3.8 of COLA. (Clarification Issues 13.6 and 17.6)	S&L P. K. Agrawal	**03.08.04-30 S3**	6/20/11	
76	3.8-28		Examine amplified input motion used for Service Building stability evaluation. Refer to Action Item 3.8-27	S&L Javad Moslemian		N/A	May Audit 3.8 Punch List 75
77	3.8-29	16	Clarify response to RAI 03.08.04-19 to state that the static coefficient of friction at the interface of mudmat and soil will not exceed 0.75. Also include the requirement for roughening the concrete surfaces in the COLA. (Clarification Issue 16)	S&L P. K. Agrawal	**03.08.04-19 S2**	6/20/11	
78	3.8-30	17.3	Provide more legible Figures 3H.6-138 and 139 in COLA. (Clarification Issue 17.3)	S&L P. K. Agrawal	**03.08.04-30 S3**	6/20/11	
79	3.8-31	17.4	Add in the FSAR that the Large Equipment Access Building (LEAB) foundation will be designed such that the surcharge on the DGFOSV located directly east of the LEAB will be negligible. (Clarification Issue 17.4)	S&L P. K. Agrawal	**03.07.01-27 S4**	7/11/11	

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Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
80	3.8-32	17.1	The COLA will be revised to refer to Figures 3H.6-212 through 3H.6-217 for RSW Piping Tunnel design. (Clarification Issue 17.1)	S&L Javad Moslemian	**03.07.01-27 S4**	7/11/11	
81	3.8-33	17.5	Clarify title for Figure 3H.6-137 to specify that it is applicable to Category I site-specific structures (Clarification Issue 17.5)	S&L Javad Moslemian	**03.07.02-13 S3**	7/28/11	
82	3.8-34	12	Revise the wave propagation calculations for the tunnels using the following: 1) an apparent wave velocity of 3000 ft/s 2) maximum ground velocity based on site-specific SSE maximum ground acceleration of 0.13g 3) a triangular pressure distribution limited by the maximum passive pressure Revise COLA to reflect these changes also. (Clarification Issue 12, AI 3.8-23 and 3.8-24)	S&L Javad Moslemian	**03.08.04-30 S5**	7/15/11	
83	3.8-35	3 8	Provide in COLA the discussion of stability evaluation for each structure including a discussion of the input motion. (Clarification Issues 3 and 8)	S&L Javad Moslemian	**03.07.02-13 S4**	11/23/11	
84	3.8-36		Verify equivalent static loading for RSW Piping Tunnel and Fuel Oil Tunnel exceed that from SSI	S&L Javad Moslemian	**03.07.02-13 S4**	11/23/11	
85	3.7-40		Provide response to questions on lateral pressure diagrams Refer to Action Items 3.7-42 & 3.7-44	S&L Javad Moslemian	**N/A**	N/A	May Audit 3.7 Punch List 87 & 89
86	3.7-41		Discuss the effects of groundwater change on UHS responses Refer to Action Item 3.7-46	S&L Javad Moslemian	**N/A**	N/A	May Audit 3.7 Punch List 91
87	3.7-42		Revise Figure 3H.6-219 to remove 2D 'Alone' soil pressure profile. Also revise the figure to provide seismic pressure profile used for design in lieu of static + dynamic. Also check other figures for the same issues.	S&L Javad Moslemian	**03.07.01-27 S4**	7/11/11	
88	3.7-43		Add the factor of safety for floatation in Table 3H.6-14	S&L Javad Moslemian	**03.07.02-13 S3**	7/28/11	
89	3.7-44		Verify that SGH's validation for SASSI included verification for soil pressures and provide validation for NRC review	S&L Javad Moslemian		N/A	May Audit 3.7
90	3.7-45		Turbine Building Seismic Calculation, Fluor calculation number U3-TB-S-CALC-DESN-2100 Rev B, should be revised for the following: 1) Assumption 1 on sheet 9 of 288 should be clarified to clearly describe how mass and stiffnesses were derived from different Turbine Building models 2) On sheet 8 of 288, correct Reference 3 document number from U3-TB-S-CALC-DESN-2001 to U7-TB-S-CALC-DESN-2001 3) Revise Section 5.15 to clearly describe how the seismic demand was determined. For example, clarify the statement in the last paragraph of sheet 250 "after running the RSA static analysis is then run with the RSA results to determine the base shear". Also describe how the RISA anlaysis was done in 3 directions and how the 3 directional responses were combined.	TANE Jim Fisicaro	**03.07.02-13 S3**	7/28/11	
91	3.7-46		For groundwater resolution, in addition to the RAI response provided, perform the following: 1) Use 28 ft groundwater level in the analysis being performed for DOE letter (AI 3.7-37) 2) Revise the COLA markup provided in the previous RAI response 03.07.01-27 S3 3) Revise the SSI calculations to add a clarification for groundwater level	S&L Javad Moslemian	**03.07.01-29 S1 Draft**	9/1/11	

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NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
92	3.8-37		For Radwaste Building, revise the COLA for the following: -- Add clarification in Section 3H.3.5.1, 2 and 3 for seismic analysis models (stick model for stability, FEM model for design and II/I) [Clarification Issue 13.1] -- Delete vertical accelerations from Table 3H.3-1 [Clarification Issue 13.1] -- Revise Section 3H.3.4.3.4.2 to include N690 provision for shear allowable [Clarification Issue 13.3] -- Revise Section 3H.3.4.2.1 to state that bearing pressure capacity is determined using methodology described in COLA Section 2.5S.4 [Clarification Issue 13.4]	S&L Javad Moslemian	**03.08.04-18 S3**	7/28/11	
93	3.8-38		Revise response to RAI 03.08.04-35 to explain design in lieu of further analysis for equivalent bearing pressure.	S&L Javad Moslemian	**03.08.04-35 R1**	6/30/11	
94	3.8-39		Provide questions on Turbine Building and Service Building stability calculations by about 10 a.m. on 5/26 Refer to Action Item 3.7-45	NRC Chakrabarti	**N/A**	N/A	May Audit 3.8 Punch List 90
95	3.8-40		Discuss additional information to be included in COLA for II/I evaluations of non-seismic Category I structures with OGC	NRC Tom Tai	**N/A**	N/A	7/22/11
96	3.8-41		Revise UHS design calculation to use N690-1994 Supplement 2	S&L Javad Moslemian	**U7-UHS-S-CALC-DESN-6002** **U7-YARD-S-CALC-DESN-6001**	6/30/11	
97	3.7-47		For Service Building stability calculation (U3-SB-S-CALC-DESN-2100 Rev. B), expand Section 5.14 to fully describe how the seismic analysis was performed and specifically address the following: 1) How the stick model mass and stiffness were calculated or provide a copy of Ref. No. 23, U3-SB-CALC-DESN-2001 Rev. 0, "Calculation of Service Building Mass and Stiffness Model" for review. 2) RSA details: - 1. modal combination method - 2. combination of 3 directional responses Attachment 01 sheets 4 and 5 - 1. What are modes 5 and 6 with frequency > 3.0x108 Hz? Write Condition Report to document response and track calculation revision. See Action Item 3.8-40.	TANE Jim Fisicaro	**03.07.02-13 S3**	7/28/11	
98	3.7-48		Clarify COLA Section 3H.6.5.3 to specify that UB Backfill soil case was used for cracked and separated cases (Clarification Issue 19)	S&L Javad Moslemian	**03.08.04-30 S3**	6/20/11	
99	3.7-49		Revise calculation U7-UHS-C-CALC-DESN-6007 to delete Table 3 (Reference March 2011 audit report)	S&L Javad Moslemian	**U7-UHS-C-CALC-DESN-6007**	9/1/11	
100			Not Used			N/A	N/A
101	3.8-42		Revise the Control Building Annex stability evaluation to use ASCE 7-05 instead of ASCE 7-88. Check for two cases, (1) with live load for both the stabilizing force and the driving force and (2) with no live load for either the stabilizing force or the driving force	S&L Javad Moslemian	**03.07.02-13 S3**	7/28/11	

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NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
102	3.8-43		Audit Report Section 3.8 I 8. Calculation no. U7-RSW-S-CALC-DESN-6001, Revision D, "Basic Structural Design of Reactor Service Water (RSW) Tunnel". The staff noted that several areas of design of the tunnel were not complete at time of the audit, e.g., global tornado missile impact evaluation for the access region of the tunnel, tunnel walls in the corner region, access covers, etc. Include the closure in the May audit report	NRC Chakrabarti	**N/A**	N/A	
103			Evaluate DNFSB Issues Related to the SASSI Subtraction Method with the model used to perform sensitivity analysis related to Poisson's Ratio (Punch List Item 29)	NRC	**N/A**	N/A	Discussed at September Audit
104			Provide legible prints of Table 3H.8-1 contained in the June 16 response in Letter U7-C-NINA-NRC-110081	S&L P. K. Agrawal	**N/A**	6/29/11	
105			Provide a plan for providing information that will allow the Staff to make a determination that II/I for the AFIPH is adequately addressed in the FSAR Will be addressed under RAI 03.07.02-32 (Punch List Item 109)	S&L P. K. Agrawal	**N/A**	7/22/11	
106			Provide applicant with an update on what actions, if any, are required as a result of hurricane wind load consideration in Chapters 2 and 3 - no action required by NINA	NRC Chakrabarti	**N/A**	N/A	7/27/2011
107		23	Staff has a concern that ACI 349-97 Appendix B Steel Embedments, referenced in Response to RAI 03.08.04-33 (page 3 of 7), is not endorsed by NRC. Applicant will provide clarification. Clarification Issue #23	S&L P. K. Agrawal	**03.08.04-33 S1**	7/28/11	
108			Staff requests additional description of foundations in FSAR 3.8.5. Applicant will provide a brief description of the foundations, foundation analysis, and differential settlement determination, including consideration of construction sequence.	S&L Javad Moslemian	**03.07.02-13 S3**	7/28/11	
109			Develop a response and ITAAC to address II/I issues associted with 3.7.2.8 in response to an upcoming RAI Refer to Punch List Item 95	S&L P. K. Agrawal	**03.07.02-32**	9/1/11	
110			From the July 12, 2011 submittal in NINA letter 110099 (RAI 03.08.04-30): (a) Table 3H.6-6, "Results of RSW Piping Tunnel Design", discuss the small differences between Required vs. Provided Reinforcement (0.7 versus 0.79 or 0.97 versus 1.00) or non-existent (1.56 for both). (b) Check see if Note 3 in Table 3H.6-6 is clear as written. (c) Determine if there is a sketch defining the reinforcing zones in the RSWPT (d) In Figure 3H.6-247, is the passive pressure shown applicable to the entire RSWPT design?	S&L Javad Moslemian	**03.08.04-30 S6** **(All but RWB & DGFOSV)**	9/12/11	
111			Provide the required material for verification of the response to Punch List Item 18	S&L Javad Moslemian	**N/A**	8/12/11	
112	3.7-50		Identify RAI responses that justify exceedances seen in soil pressure plots	S&L Javad Moslemian	**03.07.01-29 S1, R1**	11/23/11	`
113	3.7-51		Identify RAI response that discusses seismic gap joint material	S&L Javad Moslemian	**N/A**	8/16/11	
114	3.7-52	22	In RAI response, provide explanation of "modified subtraction model" -- state that all surface nodes are included	S&L Javad Moslemian	**03.07.01-29 S1 Draft**	9/1/11	

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Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
115	3.7-53	22	In RAI response, when describing soil cases for Section 6 SSSI analysis, justify considering only UB Backfill and LB In-situ soil cases	S&L Javad Moslemian	**03.07.01-29 S1 Draft**	9/1/11	
116	3.7-54	22	In RAI response, provide reasoning as to why for the section east of Section 7 where no SSSI analysis is performed, the DGFOSV is adequate.	S&L Javad Moslemian	**03.07.01-29 S1 Draft**	9/1/11	
117	3.7-55	22	In RAI responses, where "engineering judgement" is used, provide supporting technical justification	S&L Javad Moslemian	**03.07.01-29 S1 Draft**	9/1/11	
118		22	Provide results for Punch List Items 72, 114, 115, 117, 123, and 124 (Refer to RAI 03.07.01-29 S1 Draft)	S&L Javad Moslemian	**03.07.01-29 S1**	11/14/11	`
119	3.8-21		Beam shear discussion Calculations will be revised and FSAR tables will be updated as a Confirmatory Action Follow up to Punch List Item 56	S&L Javad Moslemian	**03.08.04-34 S2 (RWB & DGFOSV)**	11/14/11	`
120	3.7-57	8	Follow up to Punch List Items 27, 28, 83 & 84 related to stability evaluations	S&L Javad Moslemian	**03.07.02-13 S4**	11/23/11	
121		22	Provide results for Punch List Items 91 & 116 related to groundwater elevation and DOE letter resolution Include results in COLA Mark-up	S&L Javad Moslemian	**03.07.01-29 S1**	11/14/11	`
122		22	Provide SSI soil pressures for the RSW Tunnel	S&L Javad Moslemian	**03.08.04-30 S6 (All but RWB & DGFOSV)**	9/12/11	
123		22	Show why soil pressures obtained from MSM SSI analysis for the UHS/RSW Pumphouse and DGFOSV are conservatively bounded by the actual design pressures.	S&L Javad Moslemian	**03.07.01-29 S1 Draft**	9/1/11	
124		22	During the South Texas Project, Units 3&4, Audit of FSAR Section 3.7, RAI 03.07.01-29, SASSI Issues Raised by the DNFSB Letter to DOE, NINA presented Table 6.2 – Comparison of Direct and Modified Subtraction Method Beam Forces in the Y-Direction. NINA is requested to clarify Table 6.2.	S&L Javad Moslemian	**03.07.01-29 S1 Draft**	9/1/11	
125		24	Describe where in the FSAR the basis for the Cassification IIb for the RWB is justified.	S&L P. K. Agrawal	**03.08.04-37**	9/12/11	
126			Describe the Radwaste Tunnel in the FSAR in sufficient detail for Staff to conclude its potential effect on adjacent structures or describe where the information may be found in existing documentation	S&L P. K. Agrawal	**Audit Discussion**	N/A	Was discussed during audit; no related RAI.
127		25	Issues identified by NRC that need further consideration to resolve DOE SASSI issues	S&L Javad Moslemian	**03.07.01-30**	11/23/11	`
128			Correct a typo in FSAR Table 3.2-1 on Page 3.2-5 under Component P8, by revising the parenthetical text from U16 to U20.	S&L P. K. Agrawal	**09.02.05-1 R1**	9/15/11	
129			Provide clarification regarding how the design of non-Category I structures is based on IBC-2006 result in "the margin of safety of the structure is equivalent to that of the Category I structures" for II/I. Response to RAI 03.07.2-32 (eRAI 5919) on the acceptance criteria of ITAAC Tables 3.0-21 to 3.0-25 for the turbine building, service building, radwaste building, control building annex, and the stack on the reactor building roof, respectively.	S&L Javad Moslemian	**03.07.02-32 R1**	NINA will resubmit draft to NRC incorporating clarifications discussed during 11/02 telephone conference.	Draft revision for ITAAC was discussed with NRC. NRC provided feedback on 11/02.
130			Discuss Containment ISI in FSAR	S&L R. W. Hooks	**03.08.01-11**	11/17/11	

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Sections 3.7 & 3.8

NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
131	3.7-58		RAI 03.08.04-30 S5, clarify why the wave propagation for DGFOST is based on site-specific SSE. Design parameters table will be updated per Action Item 3.7-58.	S&L Javad Moslemian	**03.08.04-30 S7**	11/14/11	`
132			RAI 03.08.04-30 S6 The noted 11% margin for RSW piping tunnels is not applicable to shear, please provide justification For RSW piping tunnels it is stated that passive pressure controls the design, please clarify.	S&L Javad Moslemian	**Audit Discussion**	N/A	Was discussed during audit; no related RAI.
133			RAI 03.08.04-34 S1, Figure 3H.7-10a referenced in the response does not exist.	S&L Javad Moslemian	**Audit Discussion**	N/A	Was discussed during audit; no related RAI.
134	3.7-58		COLA Rev. 6, Figures 3H.7-31 and 3H.7-32 are not legible. Table 3H.9-1, the Table heading is noted as Table 3H.8-1. COLA Rev 6 figures 3H.7-31 and 32 will be replaced. Design parameters table will be updated per Action Item 3.7-58.	S&L Javad Moslemian	**03.08.04-30 S7**	11/14/11	`
135			RAI 03.08.04-18 S3, clarify why all accelerations for the RWB were deleted.	S&L Javad Moslemian	**Audit Discussion**	N/A	Was discussed during audit; no related RAI.
136	3.7-56 3.7-57		Clarify if there is any SSI analysis for Service Building (RAI 03.07.02-13 Supplement 3) and also clarify response for Turbine Building and Service Building In response to RAI 03.07.02-13 S4 describe in the FSAR the source for E' and Es and discuss the comparison of E' and Es versus what is obtained from SSI or SSSI and add a statement that the conclusions are conservative	S&L Javad Moslemian	**03.07.02-13 S3 R1**	11/23/11	
137	3.7-59		For stability evaluations address all exceedances based on amplified motion	S&L Javad Moslemian	**03.07.01-29 S1, R1**	11/23/11	`
138	3.7-60		For RSW Piping Tunnel, scaling of spectra for all dampings should be calculated at same frequency intervals used for UHS based on the ratio of new amplified motion over the old amplified motion	S&L Javad Moslemian	**03.07.01-29 S1, R1**	11/23/11	`
139	3.7-61		Also address amplified motion for Fuel Oil Storage Vault adjacent to Ultimate Heat Sink for UB soil case using MSM method for both full and empty basin	S&L Javad Moslemian	**03.07.01-29 S1, R1**	11/23/11	`
140	3.7-62		Include flow chart for DOE issue in the response to RAI 03.07.01-29 S1	S&L Javad Moslemian	**03.07.01-29 S1**	11/14/11	`
141	3.7-63		In RAI response 03.07.01-30, also discuss the pressure exerted by the compressible material on the Pump House wall and also why no additional SSSI section is needed for the Fuel Oil Storage Vault adjacent to the RSW Tunnel	S&L Javad Moslemian	**03.07.01-29 S1, R1**	11/23/11	`
142	3.7-64		When addressing amplified motion, clearly describe why Control Building Annex is not affected by DOE issue	S&L Javad Moslemian	**03.07.01-29 S1, R1**	11/23/11	`
143			1. Demonstrate that the amplified seismic input (i.e., amplified input spectra due to presence of the nearby heavy structures), if generated by using the Subtraction Method (SM) for DGFOT, RSWPT, DGFOSV, and any other structures as applicable would be conservative as compared to those obtained using the Modified Subtraction Method (MSM) or the Direct Method (DM). Alternatively, the applicant may use amplified spectra derived from the use of MSM or the DM.	S&L Javad Moslemian	**03.07.01-30**	11/23/11	`

Punch List for South Texas Project Units 3 & 4
Sections 3.7 & 3.8

NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
144			2. While SSI soil pressures obtained using both the SM and the MSM were in general comparable (See Figure 4.13 of July 27 & 28 audit presentation, SASSI Issues Raised by the DNFSB Letter to DOE), the results, presented at July 27, audit, did not fully demonstrate acceptability of the soil pressure distribution obtained from either the MSM or SM in comparison to results obtained from the DM. STP's project specific confirmation of the MSM method (using CB SSI analysis) or the SSSI analysis performed for one model (consisting of RWB, RSW Tunnel, and RB) did not include any comparison of the transfer functions of the soil pressure parameter at the interaction nodes at the exterior walls and the interacting adjacent building walls. The applicant is requested to further demonstrate that the soil pressure distribution obtained from the SM or MSM method is acceptable and is conservative for use in seismic design.	S&L Javad Moslemian	**03.07.01-30**	11/23/11	`
145			3. For SSSI analysis (for soil pressure determination considering interaction of adjacent building), only one model (consisting of RWB, RSW Tunnel, and RB) was evaluated using the DM, SM, and MSM. STP has completed the analysis only for the lower bound soil case (UB case using backfill will also be performed). Preliminary results indicate that absolute soil pressure profile obtained from SM and MSM in some instances (particularly for exterior walls) did not compare well with those obtained from the DM. However, maximum total wall force (obtained from the TH analysis) due to soil pressure in general is within 5% for all three methods (Table 5.1 of July 27 & 28 audit presentation, SASSI Issues Raised by the DNFSB Letter to DOE). Based on this analysis, STP preliminarily concluded that the total soil pressure on the embedded wall obtained from SM is acceptable. However, the applicant is requested to further clarify the entries (including how they were computed) as presented in Table 5.1 provided by STP at July 27 Audit.	S&L Javad Moslemian	**03.07.01-30**	11/23/11	`
146			4. The applicant is requested to reassess the seismic demand for stability evaluation of any applicable Category I and II/I structures in light of the DNFSB issue and confirm acceptability of the factors of safety against stability during an SSE.	S&L Javad Moslemian	**03.07.01-30**	11/23/11	`
147			5. The applicant is requested to review all the Punch List Items and any applicable RAI responses to determine if any of the responses previously provided should be revised as a result of the assessment performed for addressing DNFSB issues.	S&L Javad Moslemian	**03.07.01-30**	11/23/11	`

Punch List for South Texas Project Units 3 & 4
Sections 3.7 & 3.8

NINA Action	NRC Action	Action Closed	Closed after Phone Call

Punch List Item	AI No.	CI I	Action Item Description	Responsible Engineer	RAI	Letter to NRC	NRC Review
148			6. The issue of zero SSSI pressure on portions of the RSWPH North wall (Figure 3H.6-219, letter U7-C-NINA-NRC-110096) was further discussed with STP at July 27, 2011 meeting. It was indicated that there is a gap at these locations between the RSWPT south wall and the RSWPH north wall filled by the compressible material. However, for better clarity and understanding of the analysis model, STP is requested to provide an engineering sketch showing typical sections between the RSWPT and RSWPH including the tunnel entries to the RSWPH. In addition it was noted that Section 7 of Figure 1 (see seismic soil pressure handout of July 27 & 28, 2011 meeting) was cut through RSWPH north wall, inter space between the tunnel entries to the RSWPH north wall, RSW tunnel cross section, and other buildings. However Figure 3H.6-211 (see letter U7-C-NINA-NRC-110042 - 2D SSSI model of RSWPH, RSWPT, DGFOSVs, and RB) indicates that the actual SSSI model section has been cut through the tunnel entries to the RSWPH instead of the inter space between the tunnel entries as depicted in Section 7. While the SSSI model analyzed appears to be consistent with the soil pressure shown in Figure 3H.6-219, the resulting SSSI pressure may not conservatively represent the interaction pressure that could develop on the RSWPH North wall and RSWPT south wall through interaction of soil in the space enclosed by the tunnel entries, RSWPH North wall, and RSWPT south wall. The applicant is requested to address this issue and demonstrate that SSSI interaction pressure used for design is still conservative.	S&L Javad Moslemian	**03.07.01-30**	11/23/11	`
149		26	4. Acceptance of section cut forces obtained from SASSI soil structure interaction (SSI) analysis for structural design (RSW piping tunnel, diesel generator fuel oil tunnel, diesel generator fuel oil storage vault, and UHS/RSW pump house). 5. Refinement of SSI models to extract section cut forces for structural evaluation and design.	S&L Javad Moslemian	**03.07.01-29 S2**	12/21/11	`

South Texas Project Units 3 & 4
Clarifications from NRC

Issue 1 [Received 04/04/2011] (Audit Action 3.7-7): Revise Appendix 3A and 3H.6 to reconcile ground water elevation with Chapter 2

Inconsistencies were noted among various Sections of the FSAR concerning the specified design ground water level and the ground water level used in the seismic analysis for Category I structures. For example in FSAR Section 2.4S.12.5, it is stated that *"In summary, based on measured groundwater levels in observation wells and modeled post-construction groundwater levels, the maximum post-construction groundwater elevation at the STP Units 3 and 4 site is estimated to be 28 ft MSL, as reflected in Table 2.0-2. The nominal finished plant grade in the power block area is approximately 34 ft MSL,* ***six feet*** *higher than the site characteristic maximum groundwater level."* Appendix 3H.6.4.2.2, Design Ground Water Level, also specified ground water level at 28 MSL establishing the depth of water table at **six feet** below the grade.

However, in Section 3A.15, Site Conditions, it is stated that *"Based on the site groundwater conditions described in FSAR Subsection 2.4S.12, the groundwater elevation of approximately* ***eight feet*** *below grade was used in the analysis to determine the soil properties."* It is also noted that SSI model for the seismic analysis of Category I structures considered the ground water table to be approximately at **eight feet** below the grade elevation. As such, the applicant is requested to address these inconsistencies among various sections of the FSAR and the seismic analysis model and revise the applicable FSAR sections on groundwater level and structural design criteria. The applicant is specifically requested to demonstrate that the FIRS, GMRS, and the results of seismic analysis of the Category I structures including the results of stability calculations as currently established in the COLA are not adversely affected and include this justification in the applicable FSAR sections.

Issue 2 [Received 04/04/2011] (Audit Action 3.7-6/3.7-8/3.7-36)**:** Why for SSSI of RSW Tunnel was UB in situ used vs. UB backfill soil?

Review of responses to RAI 03.07.01-27, Supplements 1, Revision 1 indicates that two 2D SSSI models (East-West and North-South Sections) are analyzed to evaluate the effects of nearby structures on the three DGFOSVs and calculate the seismic soil pressures. In the East-West direction 2-D SSSI DGFOSV model (DGFOT 1C + DGFOSV 1A + CFRW), five cases of soil and backfill properties are considered to evaluate the effects of the soil and backfill properties variation. Also response to RAI 03.07.01-27, Supplement 2, Revision 2 indicates that in the East-West direction for DGFOT 1A (RB + DGFOT 1A + CFRW), five cases are run with various combinations of soil and backfill properties. However, in the North-South direction **(UHS/RSWPH + RSW Tunnel + DGFOSV 1B + DGFOSV 1C + RB),** only one case was run with UB soil properties.

Also as discussed in response to RAI 03.07.02-24, Supplement 1, Revision 1, in the East-West direction for dynamic soil pressure evaluation for RSW Tunnel and RWB walls, the 2 D SSSI model **(RB + RSW Tunnel + RWB)** included only the UB in situ soil without any evaluation for backfill properties. As such, the applicant is requested to demonstrate that consideration of only UB soil profile instead of using a combination of UB soil and backfill parameters for the cited cases will still be conservative for the wall design of all site specific Category I and RWB structures (UHS/RSWPH, DGFOSV, RSW Tunnel, UHS/and RWB). The applicant is also requested to include this evaluation in the applicable sections of the FSAR.

Issue 3: [Received 04/04/2011; New Action Item] Clearly describing in the FSAR how seismic demand for non-seismic II/I structures for stability evaluation is

It is not clear from the descriptions in the FSAR as to how the seismic demand is determined for the site-specific stability evaluation of the non-seismic II/I structures considering the influence of nearby heavy structures during SSE. As such, the applicant is requested to describe in sufficient detail in the FSAR the following information:

1. How the input motion at the foundation level of the non-seismic II/I structures are determined using the site specific SSE input spectra applied at the ground surface
2. How the effects of nearby heavy structures are included in the input response spectra for various soil and backfill conditions
3. Demonstrate that horizontal input response spectra at the foundation level of non-seismic II/I structures is broad band spectra with peak acceleration greater than 0.1g and envelops FIRS
4. Types of seismic analysis performed to determine seismic demand for stability evaluation (i.e., fixed base analysis, equivalent static or dynamic analysis, or SSI analysis) and how the input is specified in the analysis
5. Include 5% damped input response spectra (vertical and horizontal) as amplified by the presence of nearby heavy structures used in the seismic demand evaluation for site-specific stability analysis for non-seismic II/I structures

Discuss or refer to appropriate FSAR section of any differences in the method of stability evaluation from that of Category 1 structures

South Texas Project Units 3 & 4
Clarifications from NRC

Issue 4: [Received 04/04/2011] (Audit Action Item 3.8-8) Sliding Friction Coefficients reported in Table 3H.6-14

Designate whether the friction coefficients reported in Table 3H.6-14 are static or dynamic coefficient of friction values.

Issue 5: [Received 04/04/2011] (Audit Action Item 3.8-8) Referencing Table 3H.6-5

Section 3H.6.6.5 refers to Table 3H.6-5 for factors of safety against sliding and overturning for RSW piping tunnel. However, Table 3H.6-5 provides values for RSW pump house. Please verify the acceptability of this reference Table.

Issue 6: [Received 04/04/2011] (Audit Action Item 3.7-28) SASSI2000 Subtraction Method Validation

In reviewing the SGH SASSI2000 V&V of the subtraction method, it was found that the test problems (SAS-3C, SAS-4C and SAS-8) are not analyzed to sufficiently high enough frequencies that can validate the stability and accuracy of the subtraction method for passing frequency of $V_s/5h$, where V_s is the lowest shear wave velocity of the foundation media and h is the largest element size in the soil model.

In the case of S&L SASSI2000 V&V Test problems 4 and 8 used in validation of the subtraction method, the analyses were carried out to sufficiently high frequencies to cover the passing frequency requirement of $V_s/5h$. However, the conclusion does not address the stability and accuracy of the subtraction method used in the context of the calculated results.

In both cases, the applicant is requested to revisit the test problems and provide validation that adequately addresses the stability and accuracy of the subtraction method in relation to the acceptable passing frequency of $V_s/5h$ used in the STP 3&4 design.

Issue 7: [Received 04/04/2011; New Action Item] SAP2000 V&V

The response to RAI.03.07.02-29, Rev. 1, documents additional validation problems for SAP 2000. The validations include:

1) Section cuts
2) Thick shell out-of-plane response
3) Time history modal superposition with shell elements
4) Spectra calculation using thick shell elements

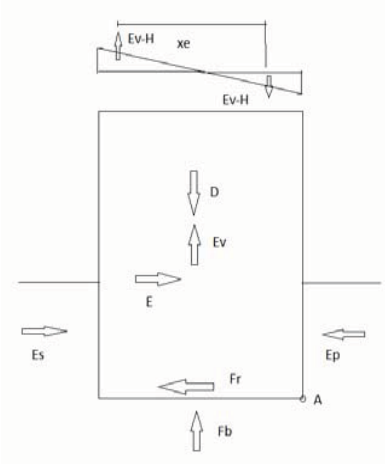
Validation of item “1” was done by hand calculation. For items “2”, “3” and “4”, the benchmark solutions for validation were developed with ANSYS utilizing “SHELL43 “element, which is well suited to model linear, warped, moderately-thick shell structures according to ANSYS User’s Manual. For items “2”, “3” and “4”, the following acceptance criteria were used: 5% for frequencies, 10% for forces, and 15% for spectra. The 5% difference is considered acceptable within the engineering accuracy, while the 10% and 15% criteria may be excessive. The applicant is requested to assess the impact of the above acceptance criteria on the STP 3&4 design.

South Texas Project Units 3 & 4
Clarifications from NRC

Issue 8: [Received 04/04/2011] (Audit Action Item 3.7-33 and 35) Stability Analysis Procedure

A brief presentation of the stability analysis procedure was made by the applicant during the March 14, 2011 audit. The presentation included a rigid body diagram showing the forces acting on the structure for stability calculations. The applicant indicated that the total seismic demand is calculated by summing all the inertia forces and moments acting on the structure. These inertia forces are calculated by multiplying the mass of each node by the absolute value of the maximum acceleration response at that node obtained from the SSI analysis.

The calculation of the seismic overturning moment is based on the horizontal seismic load E and vertical seismic load Ev about tipping point A, as shown in the figure below. The moment due to horizontal earthquake does not include Ev-H (vertical force due to horizontal input) which also produces overturning moment about A. The applicant is requested to provide justification for not including the vertical component of response due to horizontal seismic input in calculating the foundation overturning moment.



E – Horizontal seismic force

E_v – Vertical seismic force

E_s – Soil active force

E_p – Soil passive resistance

D – 0.9 of dead load

F_b – Buoyancy force

F_r – Frictional resistance

Issue 9: [Received 04/13/2011; New Action Item] DGFOSV Calculation U7-YARD-C-CALC-DESN-6001, Rev. B

In DGFOSV calcuation for SSI analysis, U7-YARD-C-CALC-DESN-6001, Rev. B, the refined model spectra values are 14.6% higher at 3.8 Hz. than those in the base model. The applicant is requested to provide justification for reconciling this difference.

South Texas Project Units 3 & 4
Clarifications from NRC

Issue 10: [Received 04/13/2011; New Action Item] (Issue 1 (New)): Calc Report, U7-YARD-S-CALC-DESN-6001, Rev. E: "Basic Structural Design of Diesel Generator The seismic design of the DGFOSV is performed with equivalent static forces, whereby the inertial forces are computed as products of masses and accelerations. The accelerations are taken from the corresponding SSI analyses as to envelope all soil conditions, design parameters, and seismic input motions. The masses are determined form the building drawings and specific weights for concrete, oil, steel and soil. The SAP structural model is divided into regions, and constant average acceleration values are applied to each region, simulating the absolute acceleration distribution of the SSI model. These inertial forces are applied in global X, Y and Z direction as external static loading to the nodes and elements of the SAP model. The response quantities, i.e. internal forces, displacements, etc., resulting from each earthquake component are combined by the SRSS rule. As a verification of the equivalent static procedure, the resultant base shear, total vertical base force and overturning moments obtained from the SAP model are compared to the corresponding values of the SSI (SASSI) model. If required, adjustment factors are applied to the input acceleration components in SAP as to provide base forces and moments which envelope the corresponding SSI base values. The maximum resultant SSI absolute acceleration values are approximately: X=0.31g; Y=0.31g and Z=0.33g. The equivalent horizontal acceleration values obtained from the SAP analysis are: X=0.33g and Y=0.32g and therefore match closely the SSI accelerations. The equivalent vertical acceleration in the SAP analysis however, had to be amplified by a factor of about 1.27 to yield Z=0.42g, in order to meet the total base forces and moments from the SSI analysis. As both, the SASSI and the SAP models, should be based on the same geometry and total weight and are subject to the same absolute accelerations, it is not apparent why an additional, relatively large amplification would be needed in vertical direction to obtain comparable total base seismic loads. Therefore, the applicant is requested to provide a justification regarding the different behavior of both structural models.
Issue 11: [Received 04/13/2011; New Action Item] MACTEC Presentation on Friction and Cohesion Values used in Stability Evaluation of R/B Lateral sliding stability against seismic loads is based on soil resistance at the foundation-soil interface (friction/ cohesion), and on lateral earth pressures acting on the embedded basement walls. Based on analyses and testing, dynamic friction and cohesion values are typically lower than the corresponding static values. Thus, for the STP units, static cohesion values are reduced 20% to account for dynamic soil behavior. The applicant's justification for this reduction is based on a publication by Makdisi and Seed from July 1978, which states that in most cases cyclic yield strength of clayey material appears to be 80% of the static undrained strength for typical shear strains obtained from earthquake soil response analyses. According to the above reference, the dynamic reduction of cohesion strength is shown to be significantly affected by the strain level present in the soil layers during a seismic event. Therefore, the applicant is requested to provide a comparison between the strain ranges assumed in the above reference and the strain values expected beneath the foundation of Cat I buildings of Units 3&4.
Issue 12: [Received 04/20/2011; New Action Item] (Action Item 3.8-24) RSW Tunnel Calculations Applicant is requested to explain the apparent discrepancy with the 3,000 ft/sec sheer wave velocity denoted in the COLA and a value of 6,600 ft/sec used for the wave propogation effect in the calculations.

South Texas Project Units 3 & 4
Clarifications from NRC

Issue 13: [Received 05/04/2011; New Action Item] Comments on Response to RAI 03.08.04-18 R1 S2

1. Design and Analysis (3H.3.5.1 & 2)

These sections describe the seismic and structural analysis of the RW/B, whereby the seismic response accelerations are obtained from a fixed base stick model subjected to a 0.15 RG 1.60 input ground motion spectrum. These accelerations subsequently are applied to a refined 3D SAP FE model. The applicant is requested to explain why using a fixed base model was considered adequate. Furthermore, it is not clear what type of SAP analysis was performed and how the results from the seismic stick model analysis were applied to the 3D finite element model. Clarification should be given as to how local amplification effects, i.e. of slabs and walls are considered in the design. In a previous response the applicant stated that the influence of nearby heavy buildings, i.e. R/B, on the seismic input motion was considered. The applicant is requested to include in the FSAR how the effect of the adjacent R/B was considered in the input motion for the RW/B, and to include the input ground spectra for the appropriate damping used in design of the RW/B.

FSAR Table 3H.3-1 provides vertical accelerations for the RWB at three floor elevations. It is not clear why horizontal accelerations are not included in the FSAR. Please include the horizontal accelerations for the RWB in the FSAR.

2. II/I Analysis and design (3H.3.5.3)

FSAR Section 3H.3.5.3 states that for II/I design, the structure is conservatively designed to remain elastic. The earthquake input used at the foundation level is the envelope of 0.3g RG 1.60 response spectrum and the induced acceleration response spectrum due to site-specific SSE. However, the FSAR does not describe how the seismic analysis and design is performed using the seismic input motion. Please describe in the FSAR how seismic analysis and design is performed for the RWB for II/I design.

3. Load Combination for Shear (Steel) (3H.3.4.3.4.2)

FSAR Section 3H.3.4.3.4.2 provides load combinations for structural steel design for RWB. The sixth load combination has a stress coefficient of 1.6. According to ANSI/AISC N690-1994 (R2004), Supplement 2, Table Q1.5.7.1, Note g, the stress limit coefficient in shear shall not exceed 1.4 in members and bolts. The applicant is requested to note the above limitation in the FSAR.

4. Ultimate soil bearing capacity (3H.3.4.2.1)

This section shows static and dynamic ultimate soil bearing capacities for the RW/B. The applicant is requested to state in the FSAR how these values were derived.

5. Tornado Parameters (3H.3.B & 3H.3.4.3.3.1)

These sections show different parameters to define tornado loads which depend on the design goal. The Design Information Table provided at the Mar 2011 Audit Meeting differs from the definitions in the FSAR text and provides specific design parameters for three different situations: main design of the structure, stability analysis and II/I design. The applicant is requested to reconcile the information in the FSAR text with the contents of the Design Information Table, and to make reference to the table if it should be incorporated in the FSAR.

6. Flood Design (3H.3.4.2.3)

This section defines the DBF as 33 ft MSL and is taken to correspond to the DCD design parameter for flooding (one ft below grade). The Design Information Table differs from the definitions in the FSAR text and provides specific design parameters for three different situations: main design of the structure, stability analysis and II/I design. The applicant is requested to reconcile the information in the FSAR text with the contents of the Design Information Table, and to make reference to the table if it should be incorporated in the FSAR.

South Texas Project Units 3 & 4
Clarifications from NRC

Issue 14: [Received 05/04/2011; New Action Item] Comments on Response to RAI 03.08.04-30 S1

7. Extreme wind and tornado loading (3H.6.4.3.2)

In its response to this RAI the applicant uses Criterion 4 of SRP 3.3.1 to justify 50 year wind velocity in the wind pressure equation. However the staff notes that Criterion 4 just means that using table C6-7 of ASCE 7 is acceptable to convert 50 year into 100 year MRI and does not refer to the importance factor, and that therefore the procedure is not consistent with SRP 3.3.1, which requires a 100 year wind in addition to the importance factor of 1.15. This approach was also confirmed during the review of FSAR section 3.3.1. In light of recorded hurricane velocities exceeding 134 mph at the site, the staff needs to further evaluate this deviation from the SRP requirements. The staff also noted that the SRP 3.3.2 design requirements are met for tornado loadings (including the importance factor of 1.15), and that the tornado design thus follows the SRP recommendations.

Issue 15: [Received 05/04/2011; New Action Item] Comments on Response to RAI 03.08.04-35

8. Foundation bearing pressures reconciliation

In its response to this RAI the applicant states that the slab edges have to be fixed in order to perform the verification analysis. The applicant is requested to explain this approach, especially if boundary conditions which do not reflect real conditions are going to be introduced in the analysis. All possible load eccentricities (plus / minus components) need to be verified. Justification is needed to base the verification on only one load combination (with the lowest safety factor).

Issue 16: [Received 05/09/2011; New Action Item] Comments on Response to RAI 03.08.04-19, Supplement 1

9. Coefficient of Friction

In its response RAI 03.08.04-19, Supplement 1, dated April 25, 2011, the applicant addressed the issue of coefficient of friction raised in audit item 3.8-8. The applicant revised the site-specific ITAAC to require the waterproofing membrane to have a minimum static coefficient of friction of 0.75, and also provided a Table that specifies that the concrete to concrete interface joints will be intentionally roughened to have a minimum static coefficient of friction of 0.75. However, the applicant did not explain how the specified minimum static coefficient of friction at the various interfaces, specifically at the waterproofing membrane, was compared with the dynamic coefficient of friction of 0.47 assumed for soil, and considered acceptable. Also, the requirement of intentionally roughening the concrete surface should be included in the FSAR.

Issue 17 [Received 05/09/2011; New Action Item] Miscellaneous Questions from Audit and Reviewers Guide

1. FSAR Section 3H.6.4.3.3.3 refers to Figure 3H.6-44 and Figures 3H.6-245 through 3H.6-247 for lateral soil pressure for design of RSW Piping Tunnels. It is not clear why Figures 3H.6-212 through 3H.6-217, which provides lateral soil pressure on RSW Tunnels considering effects from adjacent RB and RWB, are not included here.
2. Figure 3H.6-44 in the Reviewer's Guide did not include the coordinate axes.
3. Figures 3H.6-138 and 139 are not clear, and, at least, the ZPA values should be annotated in the Figures.
4. Please describe if static surcharge pressures from adjacent buildings were considered in design.
5. For clarity, Figure 3H.6-137 should be appropriately annotated regarding its applicability.
6. Please clarify if the design table provided to us is going to be included in the FSAR. Because of the complexity of the various loadings used for the same building for various types of analysis, it is strongly recommended that this table be included in the FSAR.

Issue 18: [Received 05/19/2011; New Action Item] Comments on Response to RAI 03.07.02-22

COLA Markup, Section 3H.6.6.2.1 UHS basin, UHS Cooling Tower Enclosure, and RSW Pump House

The calculated seismic soil pressures on the RSW Pump House North Wall shown in Figure 3H.6-219 from 2D SSSI analysis with other buildings show a significant spike at depths of about 10 to 13 ft below grade with the pressures falling to zero from about 13 to 44 ft below grade corresponding to the bottom of RSW piping tunnel. Please provide explanation for this pressure distribution.

Issue 19: [Received 05/19/2011; New Action Item] Comments on Response to RAI 03.07.02-24,

COLA Markup, Section 3H.6.5.3 Seismic Analysis of RSW Piping Tunnel

In the 4th bullet on the following page (Page 11 of 27), please clarify for which soil case the cracked concrete and soil separation analysis was performed. This information is not provided in the COLA markup.

South Texas Project Units 3 & 4
Clarifications from NRC

Issue 20: [Received 05/19/2011; New Action Item] Comments on Response to RAI 03.07.01-27, COLA Markup, Section 3H.6.7 Diesel Generator Fuel Oil Storage Vault (DGFOSV)

The second and third paragraphs (Page 38 of 54) discuss two different types of input motions being developed for the seismic analysis and design of DGFOSV. Please provide further clarifications why two different input motions are developed and used in separate SSI analyses of DGFOSV, as discussed in the first paragraph on the following page (Page 39 of 54).

Issue 21: [Received 05/19/2011; New Action Item] Comments on Response to RAI 03.07.02-29, Post Audit Clarification Issue # 6

In response to Post Audit Clarification Issue #6 submitted as part of the response to RAI 03.07.02-29, Supplement 1 (submitted with letter U7-C-NINA-NRC-110075), the applicant has provided comparisons of the response transfer functions of a circular embedded foundation over elastic half space to theoretical solutions to validate the accuracy and stability of the SASSI2000 subtraction method. The response comparisons cover a frequency range corresponding to ao values less than 3, where a_o is a dimensionless parameter equal to $2\pi fr/V_s$, f is the frequency of analysis, r is the radius of foundation and V_s is the shear wave velocity of foundation media. The applicant has stated that comparisons of the response transfer functions for $a_o > 3$ is not performed because theoretical solutions obtained for similar problems by Apsel & Luco, 1986 are in agreement with the results of Day, 1978 for ao less than about 3, but the results start deviating at ao of higher than 3.

The site-specific Seismic Category I structures at the STP 3 and 4 sites, such as UHS Basin/RSW Pump House, are generally large structures with significant foundation footprint. For example, UHS Basin has a foundation footprint of approximately 275 ft x 140 ft. With an equivalent foundation radius of 110 ft, and assuming an average shear wave velocity of about 1000 ft/sec and a cut-off frequency of about 22Hz used in the SSI analysis, the corresponding ao value for this structure is calculated to be on the order of about 15 while the test problems are carried out to ao values less than 3. Because the shape of the response transfer functions are strongly dependent on the a_o value (e.g. as shown in Fig. 03.07.02-29 S1.6 provided with this response), the applicant is requested to extend the results of the above test problems in terms of the foundation response transfer functions (including both compliance and scattering functions) calculated using the subtraction method to a_o values up to at least 8. The inspection of the results is necessary to ensure the

accuracy and stability of the subtraction method used for analysis of large embedded structures. Theoretical closed-form solutions are available for comparison to a_o values of 6 to 8.

Issue 22: [Received 05/19/2011; New Action Item] DNFSB Issues Related to the SASSI Subtraction Method

The Defense Nuclear Facilities Safety Board (DNFSB) issued a letter on April 8th, 2011 requesting the Department of Energy (DOE) to address technical and software quality assurance issues related to potentially erroneous seismic analyses performed using the SASSI Subtraction method. SASSI subtraction method has been used for STP application for embedded structures. As such, the applicant is requested to review these concerns and assess the potential impact on STP's seismic analysis.

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Issue 23: [Received 06/29/2011; New Action Item] Follow-up Question to RAI 03.08.04-33 Regarding Concrete Embedment Design Codes

In its response to RAI 03.08.04-33 with letter U7-C-STP-NRC-100208 Attachment 21 Page 3 of 7, the applicant stated that: “Appendix B - Steel Embedments, includes changes in ACI 349-97 based on later research. The changes in Appendix B are for the local design of embedment plates and do not affect the design of the major concrete elements. Additionally, the supplemental requirements defined in the Staff Positions in DCD Table 3.8 -10 will have a larger impact on the embedment design.”

It appears that the applicant is comparing the provisions of ACI 349-97, Appendix B, with those of the ABWR DCD Table 3.8-10. However, ACI 349-97, Appendix B was not endorsed by the NRC.

In its evaluation the staff noted that according to FSAR, Rev. 04, Table 1.8-20, RG 1.142 is changed to Revision 2, Nov 2001, which endorses ACI 349-97 with the exception of Appendix B (Anchoring to Concrete): “This regulatory guide delineates the extent to which ACI 349-97, except for Appendix B, "Steel Embedments," is acceptable to the NRC staff. In a separate action, the staff intends to endorse Appendix B of ACI 349-97 in a regulatory guide that is being developed to address anchoring components and structural supports in concrete.”

SRP 3.8.3, II.2, Rev 2, March 2007 lists under applicable codes ACI 349 as supplemented with additional guidance by RG 1.142 and 1.199.

SRP 3.8.4, II.4 (A), Rev 2, March 2007 lists as acceptance criteria: “For concrete structures, the procedures are in accordance with ACI 349, as supplemented by RG 1.142. The design and analysis of anchors (steel embedments) used for component and structural supports on concrete structures are acceptable if found in accordance with Appendix B to ACI 349, as supplemented by RG 1.199.”

RG 1.199, Nov. 2003, under C. Regulatory Position, states: “1. The procedures and standards of Appendix B to ACI 349-01 are acceptable to the NRC staff as described and supplemented below. The recommendations are applicable to the types of anchors discussed in Section B.1, "Definitions," and B.2, “Scope,” of Appendix B to ACI 349-01.”

Since RG 1.199 is not listed in FSAR, Rev. 4, Table 1.8-20 as an applicable guidance to STP 3&4, the applicant is requested to clarify which provisions are being used for steel embedments for both the standard plant structures and the site-specific structures.

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Issue 24: [Received 08/18/2011; New Action Item] Follow-up Discussion to RAI 03.08.04-37 Regarding Radwaste Building Classiification

Based on excerpts from documents U7-RWB-S-GDD-6001 and 2008-09734, we have the following preliminary comments. RAI 5991 has been issued in Letter 384 on the same subject (provide background document to justify RWB classification of IIb in accordance with RG 1.143) for NINA action.

Comments:

1. The use of an SRP 11.2, BTP 11-6 consequence analysis of radwaste tank failures is not appropriate since the assessment addresses offsite impacts for someone assumed to consume/use ground or surface water, either directly (drinking) or indirectly (ingredient in food products or for livestock watering). BTP 11-6 is clear on this and as well as the type of radiological analysis that need to be performed.

2. RG 1.143 addresses itself to doses for someone located in the unprotected area and for plant workers. RG 1.143, Reg. Position C.5 and Fig. 1 are clear on this.

3. The acceptance criteria are different: BTP 11-6 applies Part 20, App. B liquid ECLs for members of the public. RG 1.143 applies a dose of 500 mrem for someone located in the unprotected area and 5 rem for workers.

4. The development of the radioactive source terms are different: BTP 11-6 focuses on a single tank with the development of the maximum expected radio

5. With respect to RG 1.206, Part I, C.I.3 and SRP Sections 3.2.2 and 3.8.4:

RG 1.206, Section 3.2.2 states:

"The applicant should indicate the extent to which it has followed the recommendations of RG 1.26, RG 1.143, and RG 1.151. The applicant should identify any differences between the recommendations and its application and justify each proposed quality group classification in terms of the reliance placed on those systems that perform any of the following functions:

(1) prevent or mitigate the consequences of accidents and malfunctions originating within the RCPB
(2) permit reactor shutdown and maintenance in the safe shutdown condition
(3) contain radioactive material

For such systems, the applicant should specify the proposed design features and measures that it would apply to attain a quality level equivalent to the level of the RG 1.26, RG 1.143 and RG 1.151 classifications (as applicable), including the QA programs that would be implemented."

6. SRP Section 3.8.4 identifies RG 1.143 as an applicable acceptance criteria in evaluating other structures and not SRP 11.2, BTP 11-6.

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Issue 25: [Received 08/18/2011; New Action Item] Follow-up Question to RAI 03.07.01-29 - DNFSB Issues Related to the SASSI Subtraction Method

1. CB/RB departure assessment used Direct Method (DM). What was the method of analysis for RB/CB SSSI departure assessment (SM or DM)? If SM was used, need some justification that use of SM for the SSSI departure assessment is acceptable.

2. SSI analysis of DGFOT used SASSI DM. DGFOT analysis is being repeated using DM to extract seismic SSI soil pressure. This is acceptable for determination of SSI pressure, structural response quantities, and ISRS. However, the applicant should demonstrate that the seismic input (i.e., amplified input spectra due to presence of the nearby heavy structures) used for SSI analysis of DGFOT is not affected when modified subtraction method (MSM) is used for the reanalysis (to address DNFSB issues) of the nearby heavy structures.

3. SSI analysis for the RSWPT used DM. This is acceptable for determination of SSI pressure (need to verify whether SSI soil pressure was extracted from the DM), structural response quantities, and ISRS. However, the applicant should demonstrate that the seismic input (i.e., amplified input spectra due to presence of the nearby heavy structure) used for SSI analysis of RSWPT is not affected when MSM is used for the reanalysis (to address DNFSB issues) of the nearby heavy structures.

4. For UHS/RSWPH, SSI analysis is being repeated using MSM. ISRS based on SM will be modified as a result of the new analysis results based on MSM. Structural design based on SM remains unchanged. Assessment of the Impact of MSM on the design of beams and column within the basin is not complete. STP did not perform a direct evaluation of the design of the selected panels of UHS/RSW Pump House using the modified average accelerations obtained from the MSM. Instead, STP used a margin analysis based on the SSI TH analysis and the equivalent static method used in the design. Staff indicated that while equivalent static analysis generally yields conservative results, it will further assess the use of this approach from design standpoint for acceptability.

5. While SSI soil pressures obtained using both the SM and the MSM were in general comparable (See Figure 4.13 of July 27 presentation), the results, presented at July 27, audit, did not fully demonstrate acceptability of the soil pressure obtained from either the MSM or SM in comparison to results obtained from the DM. STP's project specific confirmation of the MSM method (using CB SSI analysis) or the SSSI analysis performed for one model (consisting of RWB, RSW Tunnel, and RB) did not include any comparison of the transfer functions of the soil pressure parameter at the interaction nodes at the exterior walls and the interacting adjacent building walls. STP indicated that potential impact on design of any changes in soil distribution due to method of analysis will be demonstrated by engineering judgment. The staff will further review and discuss this issue with STP.

6. DGFOSV SSI analysis is being revised using MSM. Forces and accelerations and ISRS were generated. Design is not expected to change. This is acceptable to the staff. However, the applicant should demonstrate that the seismic input (i.e., amplified input spectra due to presence of the nearby heavy structure) used for SSI analysis of DGFOSV is not affected when MSM is used for the reanalysis (to address DNFSB issues) of the nearby heavy structures.

7. For SSSI analysis (for soil pressure determination considering interaction of adjacent building) only one model (consisting of RWB, RSW Tunnel, and RB) was evaluated using the DM, SM, and MSM. STP has completed the analysis only for the lower bound soil case (UB case using backfill will also be performed). Preliminary results indicate that absolute soil pressure profile obtained from SM and MSM in some instances (particularly for exterior walls) did not compare well with those obtained from the DM. However, maximum total wall force (obtained from the TH analysis) due to soil pressure in general is within 5% for all three methods. Based on this analysis, STP preliminarily concluded that the soil pressure obtained from SM is acceptable. Further clarifications regarding the entries (including how they were computed) presented in Table 5.1 provided by STP at July 27 Audit are needed. The potential impact on design of any changes in soil pressure distribution due to method of analysis will be addressed in item 5 above.

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8. STP should reevaluate Clarification Issue 3 for establishing seismic demand of II/I and applicable Category I structures in light of DNFSB issue and initiate appropriate actions including any revision of the amplified input spectra for stability evaluation and FSAR.

9. STP should review all the Punch List Items and any applicable RAI responses to determine if any of the responses previously provided should be revised as a result of the assessment performed for addressing DNFSB issues.

10. The issue of zero SSSI pressure on portions of the RSWPH North wall (Figure 3H.6-219, letter NRC-110096) was further discussed with STP at July 27, 2011 meeting. It was indicated that there is a gap at these locations between the RSWPT south wall and the RSWPH north wall filled by the compressible material. However, for better clarity and understanding of the analysis model, STP is requested to provide an engineering sketch showing typical sections between the RSWPT and RSWPH including the tunnel entries to the RSWPH.

In addition it was noted that Section 7 of Figure 1 (see seismic soil pressure handout of July 27 meeting) was cut through RSWPH north wall, inter space between the tunnel entries to the RSWPH north wall, RSW tunnel cross section, and other buildings. However Figure 3H.6-211 (see letter NRC-110042 - 2D SSSI model of RSWPH, RSWPT, DGFOSVs, and RB) indicates that the actual SSSI model section has been cut through the tunnel entries to the RSWPH instead of the inter space between the tunnel entries as depicted in Section 7. While the SSSI model analyzed appears to be consistent with the soil pressure shown in Figure 3H.6-219, the resulting SSSI pressure may not conservatively represent the interaction pressure that could develop on RSWPH North wall and RSWPT south wall through interaction of soil in the space enclosed by the tunnel entries, RSWPH North wall, and RSWPT south wall. The applicant is requested to address this issue and demonstrate that SSSI interaction pressure used for design is still conservative.

11. Tentative. For UHS/RSWPH, RSW Tunnel (SSSI pressure), and DGFOSV, need further demonstration of soil pressure comparability at the interaction nodes at the exterior and interior wall between DM and SM/MSM for justifying the basis of SSI/SSSI soil pressure. Following or other options may be considered:

1. Comparison of SSI soil pressure transfer function obtained from CB analysis by DM and SM/MSM at selected exterior wall nodes as part of project specific confirmation (Poisson's ratio 0.495)
2. Comparison of soil pressure transfer functions (for the SSSI model consisting of RWB, RSW Tunnel, and RB) at representative interaction nodes for both exterior and interior walls for the three methods

For the model discussed in item 2 above, comparison of soil pressure distribution obtained from the three methods at the corresponding time at which the maximum total pressure had occurred.

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Issue 26: [Discussed 09/30/2011; New Action Item] As discussed at the Audit on 09/30/11

The NRC has reviewed NINA's response to the DOE issue with respect to seismic design loads on the Ultimate Heat Sink / Pump House structure. NINA demonstrated that changes in demand attributable to the Modified Subtraction Method (MSM), as opposed to the original Subtraction Method (SM) analysis, are adequately covered by the existing margins due to conservative applied seismic loads, and hence the change to MSM does not affect the design. Herein, "margins" refers to the amount by which the applied seismic design loads, as calculated using a SAP2000 equivalent static model, exceed those same loads as calculated from the SSI analyses. . The NRC has raised two concerns regarding quantification of the margins:

1. The NRC is asking for further verification, beyond what has been demonstrated in the V&V process, that section cut forces based on element forces from SSI analysis using SASSI2000 are correct.
2. The NRC is also asking for further verification that the so-called coarse mesh used in the SSI analyses is sufficiently refined for determination of section cut forces.

Proposed Response:
NINA proposes the following two independent analyses to address the two NRC concerns:

1. Benchmark: To confirm that the section cut forces from SSI analysis using SASSI2000 are calculated correctly, a dynamic analysis performed in SASSI2000 will be repeated using SAP2000 with an identical model and input. The models will be identical to the so-called coarse mesh model used for SSI analysis of UHS/RSW PH, but will be run as fixed base. Input ground motions will be the site-specific SSE, the results from the three seismic components will be combined using SRSS, and only the full basin case will be considered. Comparison will include all section cut forces from the same 19 locations listed in Tables 03.07.01-29 S1.16 and 03.07.01-29 S1.17 of the draft RAI response 03.07.01-29 S1. If section cut forces from the two analyses are within approximately 10%, the section cut forces from the SSI analysis using SASSI2000 will be considered correct.

2. Mesh Refinement: To confirm that the coarse mesh model of the SSI analysis of the UHS/RSW PH using Modified Subtraction Method is sufficiently refined for determination of section cut forces, a dynamic analysis performed in SASSI2000 will be repeated using a mesh that has been modified to best approximate that used in the SAP2000 design model using equivalent static method. The models and input motions will be identical except for this mesh modification. Both dynamic analyses will be run using fixed base boundary conditions subject to site-specific SSE ground motions considering both full and empty basin cases. The results from the three seismic components will be combined using SRSS. Comparison will include all section cut forces from the same 19 locations listed in Tables 03.07.01-29 S1.16 and 03.07.01-29 S1.17 of the draft RAI response 03.07.01-29 S1. If section cut forces are within 10%, the SSI coarse mesh will be considered adequately refined for determination of section cut forces from MSM SSI analysis. For any section where the section cut forces from the modified mesh are higher by
more than 10%, the corresponding section cut forces from the MSM SSI analysis will be increased by the same % increase prior to comparison with the section cut forces from the SAP2000 design model for demonstrating adequacy of the existing design.