

PMSTPCOL PEmails

From: STPCOL
Sent: Wednesday, September 28, 2011 4:50 PM
To: PMSTPCOL PEmails
Subject: FW: Transmittal of Letter U7-C-NINA-NRC-110116
Attachments: U7-C-NINA-NRC-110116.pdf

From: Tai, Tom
Sent: Tuesday, September 13, 2011 9:06 AM
To: Chakrabarti, Samir
Cc: Chakravorty, Manas; Thomas, Brian; STPCOL; Wunder, George
Subject: FW: Transmittal of Letter U7-C-NINA-NRC-110116

Samir,

Attached for your review are supplemental responses to RAI 03.08.04-30 (Supplement 6) and 03.08.04-34 (Supplement 1), including FSAR markup.

03.08.04-30 S6 addresses punch-list items 110 and 122, with markup to Table 3H.6-6 and Figures 3H.6-212, -213, and -248.

03.08.04-34 S1 addresses punch-list item 56. FSAR markup include:

Sections (3H.6.6.3.1 and 3H.7.5.3.1);
Tables (3H.6-7, 3H.6-8, 3H.6-9, and 3H.7-1), and
Figures (3H.6-51, 3H.6-54 to 3H.6-74 and -74A, 3H.6-75 to 3H.6-78 and -78A, 3H.6-116 and -116A, 3H.6-117 to 3H.6-120 and -120A, 3H.6-121 to 3H.6-122 and -122A, 3H.6-123 to 3H.6-126 and 126A, 3H.6-127 to 3H.6-134 and 134A, 3H.6-135 to 3H.6-136A, -136B, and -136C) in the FSAR are also revised.

Regards

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From: Elton, Loree [<mailto:leelton@STPEGS.COM>]
Sent: Tuesday, September 13, 2011 8:20 AM
To: Muniz, Adrian; Casto, Chuck; Wunder, George; Tonacci, Mark; Eudy, Michael; Anand, Raj; Foster, Rocky; Joseph, Stacy; Govan, Tekia; Tai, Tom
Subject: Transmittal of Letter U7-C-NINA-NRC-110116

Please find attached a courtesy copy of letter number U7-C-NINA-NRC-110116 which provides supplemental responses to NRC Request for Additional Information (RAI) Questions 03.08.04-30 and 03.08.04-34 related to the Combined License Application (COLA) Part 2, Tier 2, Section 3.8.

Note that the attachment has been compressed in .zip format in order to meet NRC incoming e-mail size limitations.

The official version of this correspondence will be placed in the mail. Please call John Price at 972-754-8221 if you have any questions concerning this letter.

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Hearing Identifier: SouthTexas34Public_EX
Email Number: 3082

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Nuclear Innovation
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September 12, 2011
U7-C-NINA-NRC-110116

U. S. Nuclear Regulatory Commission
Attention: Document Control Desk
One White Flint North
11555 Rockville Pike
Rockville MD 20852-2738

South Texas Project
Units 3 and 4
Docket Nos. 52-012 and 52-013
Supplemental Response to Request for Additional Information

Attached are Nuclear Innovation North America LLC (NINA) supplemental responses to NRC Request for Additional Information (RAI) Questions 03.08.04-30 and 03.08.04-34 related to the Combined License Application (COLA) Part 2, Tier 2, Section 3.8.

Where there are COLA markups, they will be made at the first routine COLA update following NRC acceptance of the RAI response.

There are no commitments in this letter.

If you have any questions regarding these responses, please contact me at (361) 972-7136 or Bill Mookhoek at (361) 972-7274.

I declare under penalty of perjury that the foregoing is true and correct.

Executed on 9/12/11


Scott Head
Manager, Regulatory Affairs
South Texas Project Units 3 & 4

jep

Attachments:

1. RAI 03.08.04-30, Supplement 6
2. RAI 03.08.04-34, Supplement 1

cc: w/o attachment except*
(paper copy)

(electronic copy)

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RAI 03.08.04-30, Supplement 6**QUESTION:****Follow-up to Question 03.08.04-23**

In response to staff question requesting additional information (Letter U7-C-STP-NRC-100036, dated February 10, 2010) about how various steel and concrete elements of site-specific structures are designed, and the design results, the applicant provided some analysis and design information. The applicant also referred to the Supplement 2 response to Question 03.07.01-13 (Letter U7-C-STP-NRC-090230, dated 12/30/09) for pertinent design summary information. In order for the staff to conclude that the design of site-specific structures meet the requirements of GDC 2 by meeting the guidance provided in SRP 3.8.4 and 3.8.5, or otherwise, the applicant is requested to provide the following additional information:

1. The applicant states in the response that a three dimensional finite element analysis (FEA) is used for structural analysis and design of the UHS/RSW Pump House. FSAR Section 3H.6.6.1 states that analysis for the seismic loads was performed using equivalent static loads and the induced forces due to X, Y, and Z seismic excitations were combined using the SRSS method of combination. However, the applicant did not describe how the equivalent static loads due to seismic excitation were determined and applied to the static FEA model from the results of soil structure interaction (SSI) analysis used for determination of seismic response. Therefore, the applicant is requested to provide details of how seismic response analysis results from dynamic SSI analysis were transferred to the static FEA model, including how the effects of accidental torsion were included in the analysis and design of UHS/RSW Pump house. Please also update FSAR with the information, as appropriate.
2. The applicant stated in its response that the modulus of subgrade reaction for static loading was calculated as the average of the local values at nine locations under the foundation. The applicant is requested to provide these nine values, and explain why it is considered appropriate to use the average value. Please also explain how the foundation subgrade modulus was used for calculating nodal springs for the FEA model, and how the effect due to coupling of soil springs was considered in the analysis.
3. For seismic loading, the applicant has outlined a hand-calculated procedure that utilizes published formulas and charts to estimate the foundation spring constants. According to this procedure, the equivalent modulus and Poisson's ratio of a layered soil system are first estimated using the cumulative strain energy method. The resulting values are then used in the equations for computation of the spring constants for a rigid foundation of an arbitrary shape embedded in a uniform half-space. The shear moduli used for individual layers are strain compatible values, and include the mean, upper bound, and lower bound soil cases. The approximate procedure outlined above for developing the foundation spring constants does not take into account the pressure distribution under the base slab. Furthermore, this procedure does not account for the

frequency dependence of these springs. As such, the applicant is requested to provide a justification for not considering the effects of pressure distribution and system frequency in developing the foundation dynamic springs including describing the impact on the calculated results.

4. The applicant's response does not provide details as to how the soil springs calculated under static and seismic loadings are inputted to the 3-D static FEA model to calculate the design stresses. Therefore, the applicant is requested to describe in detail how the static and seismic soil springs are inputted into the FEA model, and how the results are obtained for stress evaluations. Specifically, the applicant is requested to explain if the two sets of springs were used in a single model, and how the two sets were combined to a single set of springs. Otherwise, if the two sets of springs were applied to separate FEA models, describe how the load combinations were performed. The applicant is also requested to provide sufficient detail to assist staff in understanding how static and seismic soil springs are used in the FEA model and results combined for stress evaluations.
5. In the FSAR mark-up of Sections 3H.6.6.3.1 and 3H.6.6.3.2 provided with the response, the applicant identifies the method used by the applicant for combining forces and moments. In this method, for each reinforcing zone, the maximum force or moment is coupled with the corresponding moment or force for design for the same load combination. It is not clear if this method of combining forces and moments for design will envelop the worst combination of forces and moments for all elements in a reinforcing zone. Therefore, the applicant is requested to describe the method of combining forces and moments used by the applicant with a typical example of a reinforcing zone, and demonstrate that this method of combination will yield the worst combination of forces and moments that should be considered for design.
6. The staff notes that in the FSAR mark-up of Section 3H.6.6.3.1 provided with the response, the reported values of soil springs for the RSW Pump House are significantly larger than those for the UHS basin. The applicant is requested to confirm these values, and explain the reason for the large difference.
7. The response did not include any information about the maximum static and dynamic bearing pressures under the foundations of UHS/RSW Pump House. The applicant is requested to provide the maximum static and dynamic bearing pressure under the foundations of UHS/RSW Pump House, compare these values with the maximum allowable static and dynamic bearing pressures, and include this information in the FSAR.
8. In its response to Question 03.07.01-19 (letter U7-C-STP-NRC-100129, dated June 7, 2010), the applicant provided analysis and design information for the seismic category I Diesel Generator Fuel Oil Storage Vault (DGFOSV) which was not previously included in the FSAR. The information included in the response does not describe how structural analysis and design of the structure was performed. Also, reference is made to FSAR

Section 3H.6.4 for design loads. FSAR Section 3H.6.4 has been updated several times in various responses, and it is not clear where this information can be found. Therefore, the applicant is requested to provide complete structural analysis and design information for the DGFOSV to ensure it meets acceptance criteria 1 through 7 of SRP 3.8.4 and 3.8.5. The staff needs this information to conclude that the DGFOSV is designed to withstand seismic loads and meet GDC 2. Include in the response an updated version of Appendix 3H where structural analysis and design information for all seismic category I structures can be found.

9. While reviewing this response, and other responses referenced in this response, the staff noted that the applicant has used different values of coefficient of friction for sliding stability evaluation; e.g., the value 0.3 was used for the RSW Pump House, 0.4 was used for UHS basin, 0.58 was used DGFOSV, and for the Reactor Building (RB) and the Control Building (CB), it was stated to be more than 0.47. It is not clear if these values are the required coefficient of friction, or the minimum coefficient of friction available. The applicant is requested to clearly specify the minimum coefficient of friction at various locations of the site, if they are different, and explain how these values were determined. Please also clarify this information in the FSAR.
10. The staff noted references to Diesel Generator Fuel Oil Tunnel (DGFOT) in several RAI responses. Please confirm that DGFOT is not a seismic category I structure, and if it is seismic category I, include the analysis and design information to show how the design of the DGFOT meets the acceptance criteria 1 through 7 in the SRP 3.8.4 and 3.8.5 in the FSAR.

SUPPLEMENTAL RESPONSE:

The Supplement 5 response to this RAI was submitted with Nuclear Innovation North America (NINA) letter U7-C-NINA-NRC-110099, dated July 12, 2011. This supplement provides the response to the following action items discussed in the NRC audits performed during the weeks of May 23, 2011 and July 25, 2011.

Punch List Item 110

From the July 12, 2011 submittal in NINA letter 110099 (RAI 03.08.04-30):

- (a) Table 3H.6-6, "Results of RSW Piping Tunnel Design", discuss the small differences between Required vs. Provided Reinforcement (0.7 versus 0.79 or 0.97 versus 1.00) or non-existent (1.56 for both).*
- (b) Check see if Note 3 in Table 3H.6-6 is clear as written.*
- (c) Determine if there is a sketch defining the reinforcing zones in the RSWPT*
- (d) In Figure 3H.6-247, is the passive pressure shown applicable to the entire RSWPT design?*

Punch List Item 122

Provide SSI soil pressures for the RSW Tunnel

Response to Punch List Item 110:

- Part a) In all cases the provided reinforcement equals or exceeds the required reinforcement. This meets the code requirements. Also, in determining the required reinforcement for RSW Piping Tunnel shown in Table 3H.6-6, in order to provide additional margin, the calculated moments and axial forces were divided by 0.9 (i.e. increased by $1/0.9 = 1.11$). Therefore, as a minimum a margin of 11% is available.
- Part b) The notes of Table 3H.6-6 will be revised to further clarify this table (see Enclosure).
- Part c) Figure 3H.6-248 (see Enclosure) provides the RSW Piping Tunnel layout and reinforcement zones.
- Part d) The passive pressure shown in Figure 3H.6-247 is applicable to the entire RSW Piping Tunnel design (i.e. main tunnel segments, north portion, and access regions 1, 2, and 3)

Response to Punch List Item 122:

Figures 03.08.04-30 S6.1 and 03.08.04-30 S6.2 provide SSI and SSSI soil pressures on RSW Piping Tunnel east and west walls, respectively. Also shown in these figures are the seismic soil pressures used for the design of these walls. As shown in these figures, the seismic soil pressures used for design envelope all SSI and SSSI soil pressures except for a small portion along the wall height where SSI soil pressure from separated soil case is higher. The tunnel wall design was controlled by passive pressure. Based on the following this local exceedance is found acceptable.

Due to total out-of-plane load including SSI soil pressure from separated soil case:

Maximum calculated out-of-plane shear, $V1 = 28.07$ kips/ft

Maximum calculated out-of-plane moment, $M1 = 124.53$ kips-ft/ft

Due to controlling total out-of-plane load:

Calculated out-of-plane shear, $V2 = 36.52$ kips/ft

Calculated out-of-plane moment, $M2 = 226.78$ kips-ft/ft

As shown above, $V1 = 28.07$ kips/ft is less than $V2 = 36.52$ kips/ft and $M1 = 124.53$ kips-ft/ft is less than $M2 = 226.78$ kips-ft/ft. Thus, the design is conservative for separated soil case.

As a result of this response COLA Part 2, Tier 2, Appendix 3H will be revised as shown in the Enclosure.

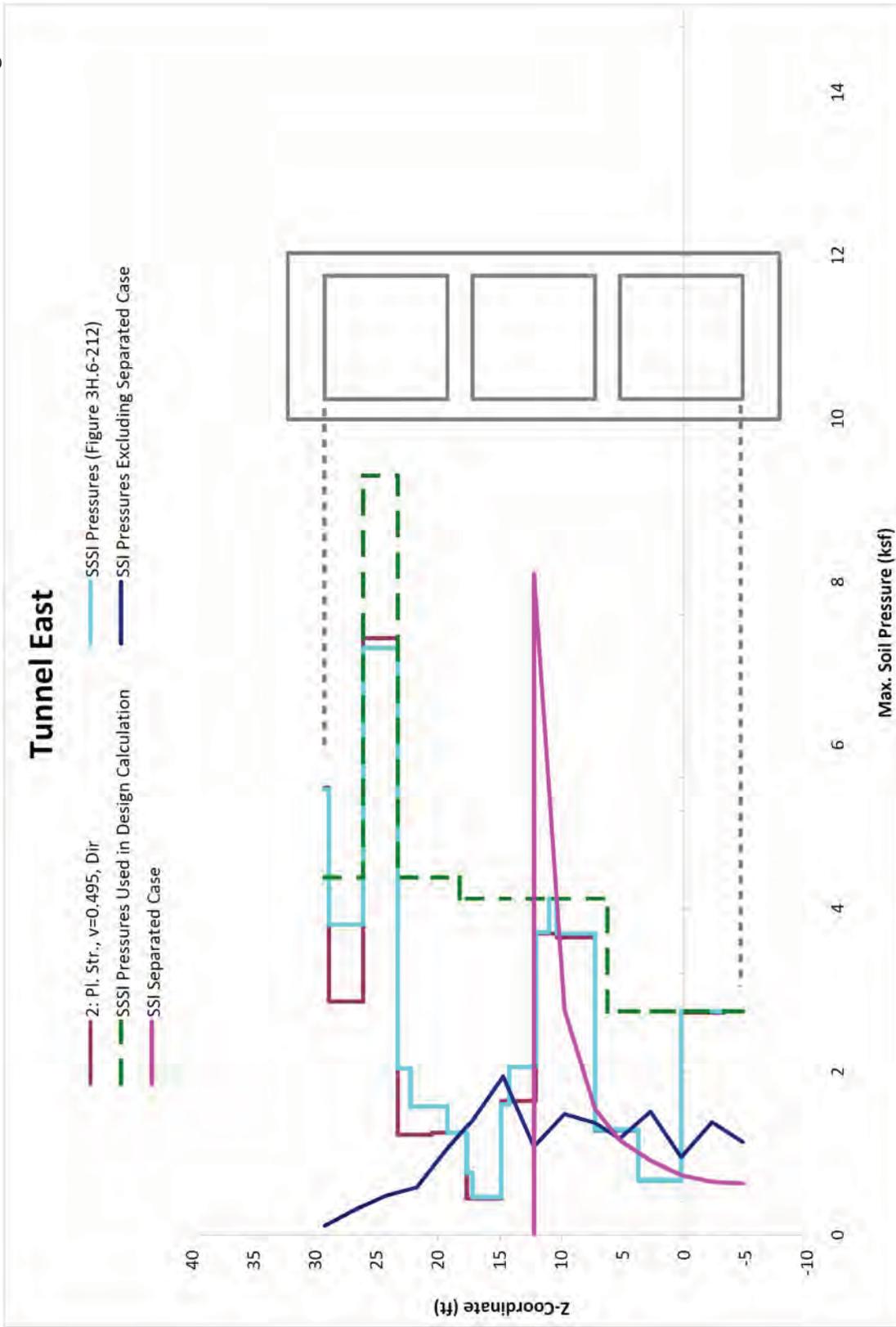


Figure 03.08.04-30 S6.1

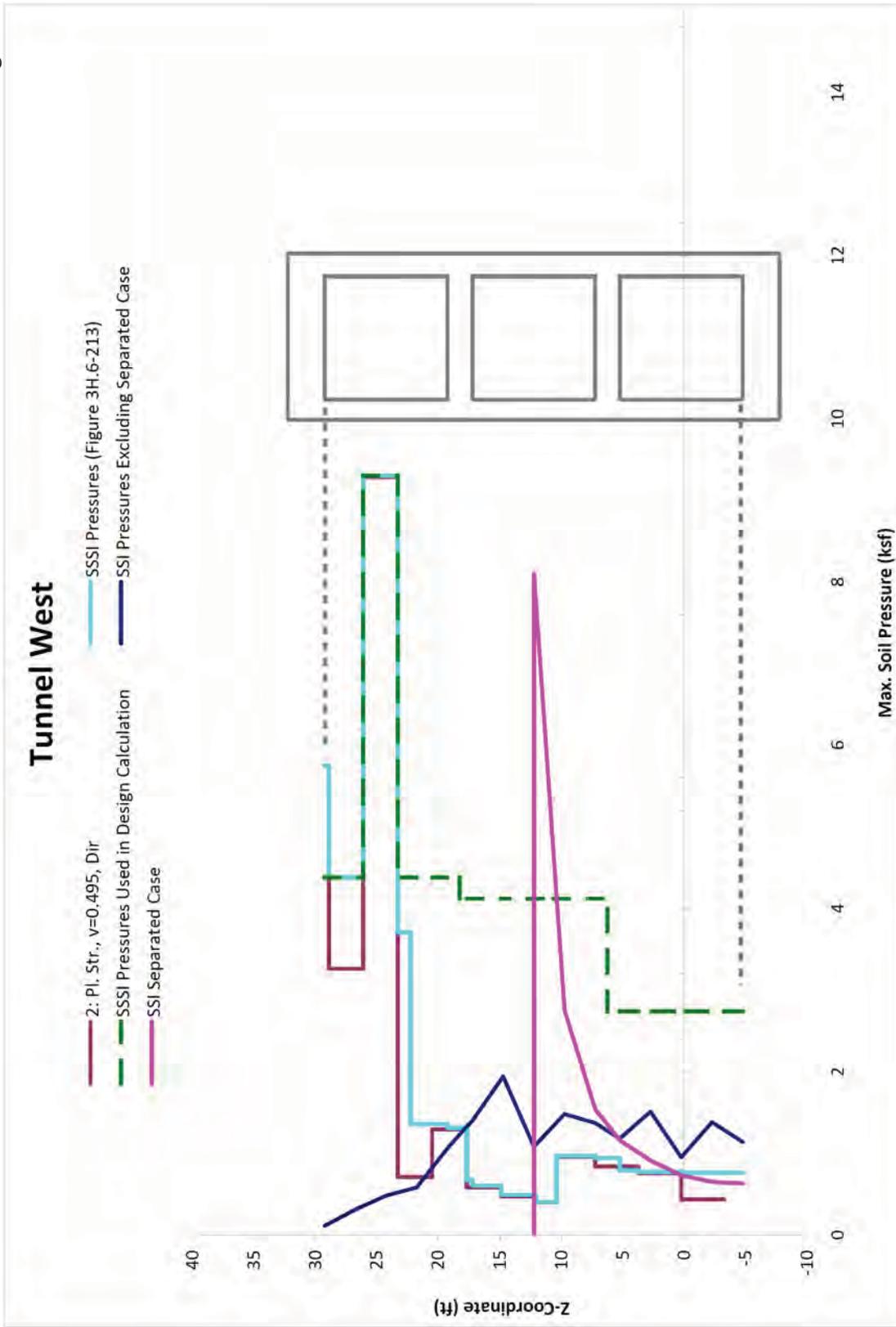


Figure 03.08.04-30 S6.2

Enclosure 1
COLA MARK-UPS

Table 3H.6-6: Results of RSW Piping Tunnel Design

Location Location ⁽⁴⁾	Item	Thickness (ft)	Governing Load Combination	Design Moment (kip-ft/ft)	Design Shear (kip/ft)	Area of Reinforcement (in ² /ft)			
						Moment Reinforcement ⁽¹⁾		Shear Reinforcement	
						Required	Provided (both faces)	Required	Provided
Main Tunnel	Exterior Wall	3'-0"	D+Lo+F+H'+E'	226.78	36.52	1.56 (vertical)	1.56 (vertical)	None	None
	Roof Slab	3'-0"	1.4D+1.7L+1.4F+1.7H	55.90	11.29	0.7 (east-west)	0.79 (east-west)	None	None
	Interior Slab	2'-0"	D+Lo+F+H'+E' (2)	95.22	13.16	1.13 (east-west)	1.27 (east-west)	None	None
	Basemat	3'-0"	D+Lo+F+H'+E' (2)	123.94	19.10	0.97 (east-west)	1.00 (east-west)	None	None
North End of Main Tunnel (near Control Building) (West of Control Building)	Exterior Wall	3'-0"	D+Lo+F+H'+E'	543.34	59.39	4.27 (east-west)	4.68 (east-west)	0.19	0.20
	Interior Wall	2'-0"	D+Lo+F+H'+E' (2)	152.15	19.96	1.69 (east-west)	2.25 (east-west)	None	None
	Roof Slab	3'-0"	1.4D+1.7L+1.4F+1.7H	86.64	15.29	0.70 (east-west)	0.79 (east-west)	None	None
	Interior Slab	2'-0"	D+Lo+F+H'+E' (2)	136.30	18.03	1.49 (east-west)	2.25 (east-west)	None	None
Main Tunnel (near Access Region 1) (In Access Region 1)	Basemat	3'-0"	1.4D+1.7L+1.4F+1.7H	70.42	28.27	0.36 (north-south)	0.79 (north-south)	None	None
	Basemat	3'-0"	1.4D+1.7L+1.4F+1.7H	155.74	36.39	1.16 (east-west)	1.27 (east-west)	None	None
Basemat	3'-0"	1.4D+1.7L+1.4F+1.7H	46.60	20.54	0.70 (north-south)	0.79 (north-south)	None	None	

Table 3H.6-6: Results of RSW Piping Tunnel Design (Continued)

Location Location ⁽⁴⁾	Item	Thickness (ft)	Governing Load Combination	Design Moment (kip-ft/ft)	Design Shear (kip/ft)	Area of Reinforcement (in ² /ft)			
						Moment Reinforcement ⁽¹⁾		Shear Reinforcement	
						Required	Provided (both faces)	Required	Provided
Main Tunnel (near Access Region 2) (in Access Region 2)	Exterior Wall	3'-0"	D+Lo+F+H+E'	321.96	29.22	2.21 (vertical)	2.25 (vertical)	None	None
	Basemat	6'-0"	D+Lo+F+H+E' (2) 1.4D+1.7L+1.4F+1.7H / D+Lo+F+H+E' (2)	214.84	29.22	1.40 (horizontal)	1.56 (horizontal)	None	None
Main Tunnel (near Access Region 3) (in Access Region 3) (North of Pump House)	Exterior Wall	3'-0"	D+Lo+F+H+E'	245.29	36.52	1.76 (vertical)	3.12 (vertical)	None	None
	Roof Slab	3'-0"	1.4D+1.7L+1.4F+1.7H	344.53	37.20	2.56 (north-south)	4.68 (north-south)	None	None
	Interior Slab	2'-0"	D+Lo+F+H+E' (2)	150.97	19.29	1.70 (north-south)	3.12 (north-south)	None	None
	Basemat	3'-0"	1.4D+1.7L+1.4F+1.7H	236.52	38.12	1.74 (north-south)	3.12 (north-south)	0.18	0.20

Table 3H.6-6: Results of RSW Piping Tunnel Design (Continued)

Notes:

- 1) Unless noted otherwise, the required reinforcement in the direction not reported in the table is controlled by the minimum required reinforcement. The minimum required reinforcement for 2'-0" thick and 3'-0" thick elements is 0.36 in²/ft and 0.54 in²/ft. For such cases the provided reinforcement is 0.79 in²/ft.
- 2) The loading also includes loads due to internal flooding.

<p>3) The following additional reinforcement is required due to SSE Wave Propagation: For the Main Tunnel, #8 bars at 12" o.c. in the longitudinal direction of the Main Tunnel for 84'-0" (measured north from the centerline of the Main Tunnel and Access Region 3) Access Region 3) For Access Region 3 from 0'-0" to 56'-0" (measured east from the centerline of the intersection of the Main Tunnel and Access Region 3) (i) Second layer of #11 bars at 12" o.c. in the transverse direction applied to both faces of the roof (ii) Second layer of #11 bars at 12" o.c. in the transverse direction applied to both faces of the interior slab (iii) Second layer of #11 bars at 12" o.c. in the transverse direction applied to both faces of the basement For Access Region 3 from 56'-0" to 103'-0" (measured east from the centerline of the intersection of the Main Tunnel and Access Region 3) (i) Second layer of #11 bars at 12" o.c. in the transverse direction applied to both faces of the roof (ii) Second layer of #11 bars at 12" o.c. in the transverse direction applied to both faces of the basement</p>
<p>3) In addition to the reinforcement shown within this table, the following reinforcement is required due to SSE Wave Propagation: - For the Main Tunnel, 0.79 in²/ft (applied to both faces of the walls and slabs) in the north-south direction of the Main Tunnel for 84'-0" (measured north from the centerline of the intersection of the Main Tunnel and Access Region 3) - For Access Region 3 from 0'-0" to 56'-0" (measured east from the centerline of the intersection of the Main Tunnel and Access Region 3), 1.56 in²/ft (applied to both faces of the roof, interior slab, and basement) in the north-south direction - For Access Region 3 from 56'-0" to 103'-0" (measured east from the centerline of the intersection of the Main Tunnel and Access Region 3), 1.56 in²/ft (applied to both faces of the roof and basement) in the north-south direction</p>

4) Refer to Figure 3H.6-248 for plan view of the RSW Tunnel

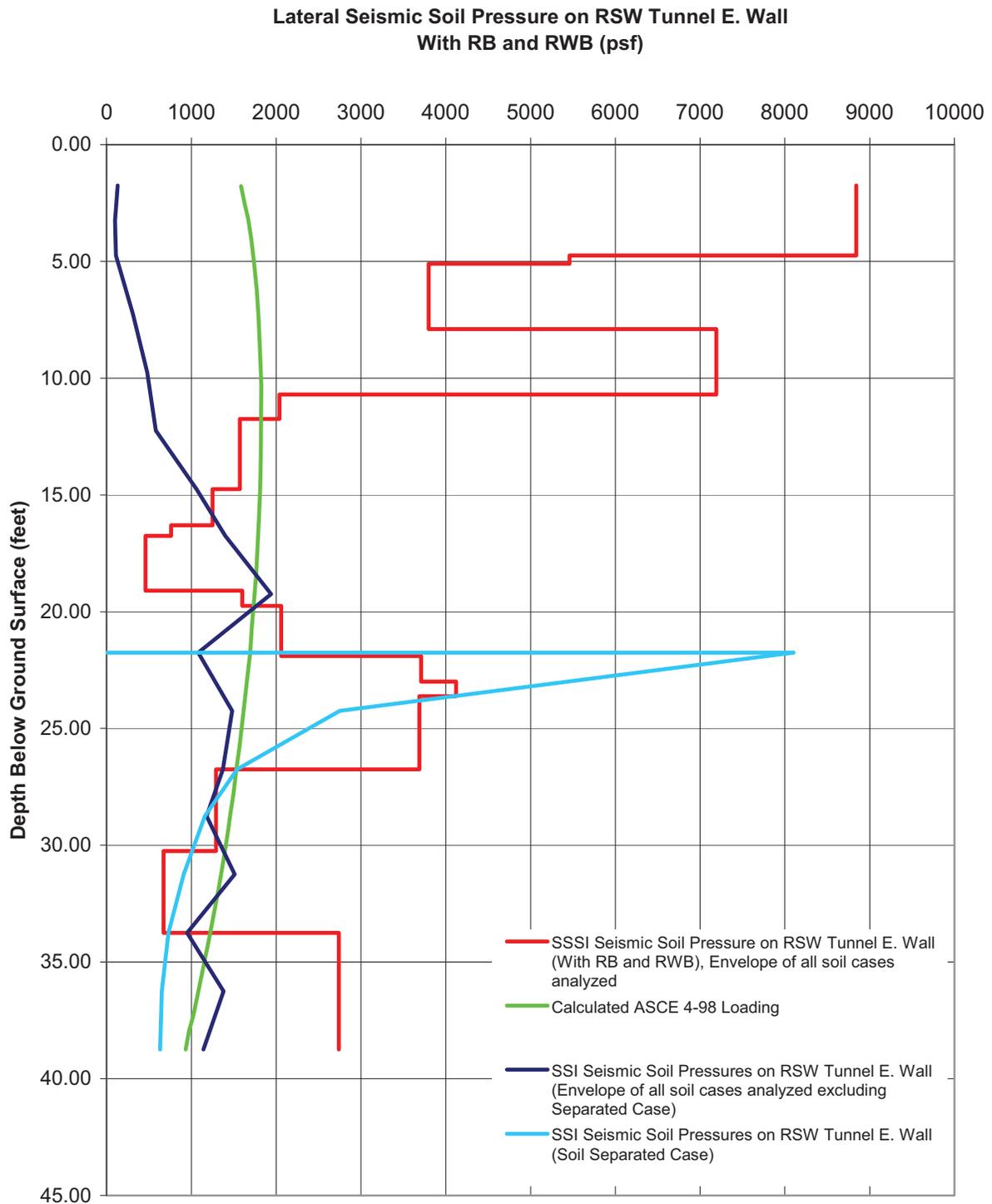


Figure 3H.6-212: Lateral Seismic Soil Pressure (psf) on RSW Piping Tunnel East Wall (Main Cross Section of RSW Piping Tunnel)

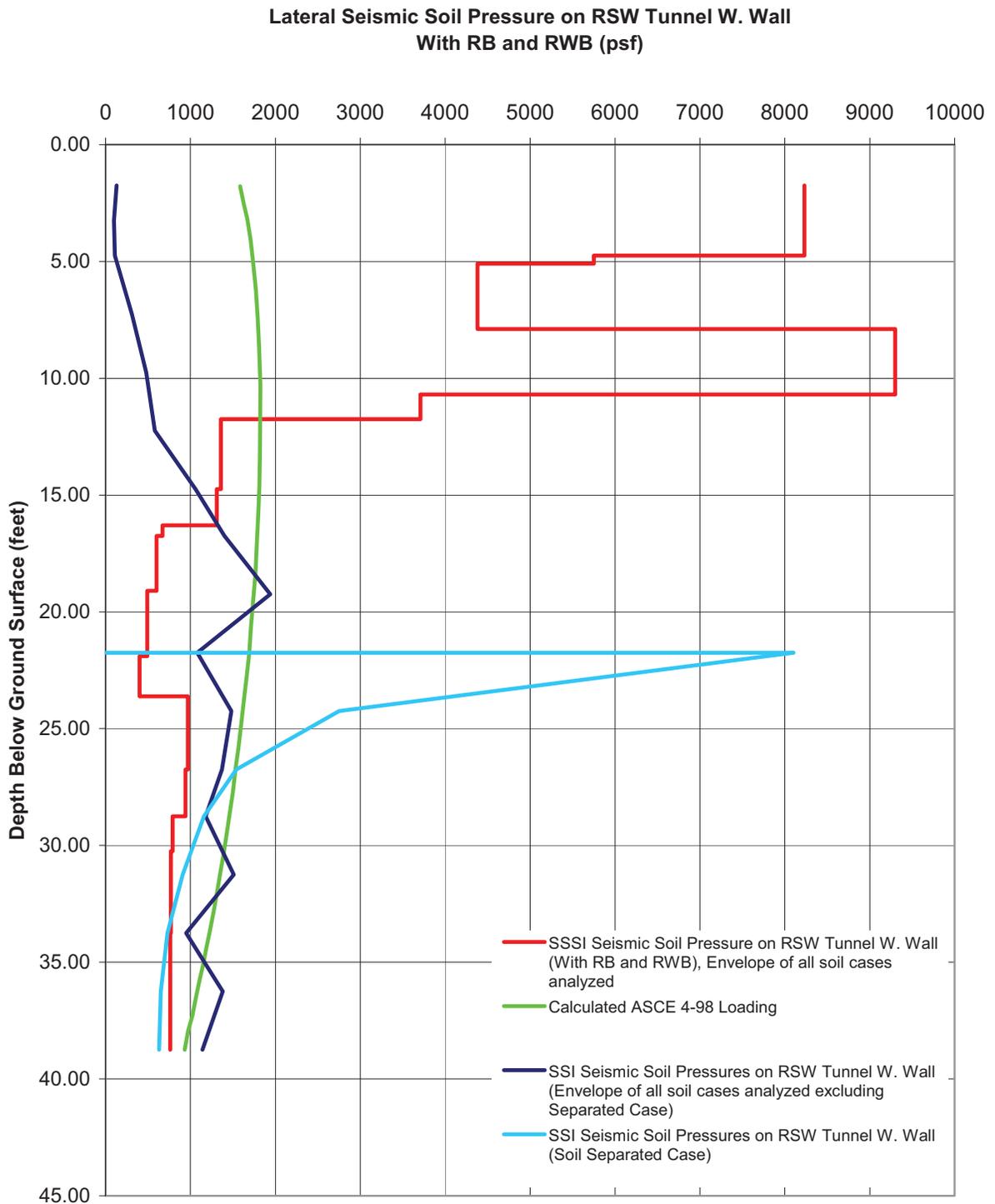


Figure 3H.6-213: Lateral Seismic Soil Pressure (psf) on RSW Piping Tunnel West Wall (Main Cross Section of RSW Piping Tunnel)

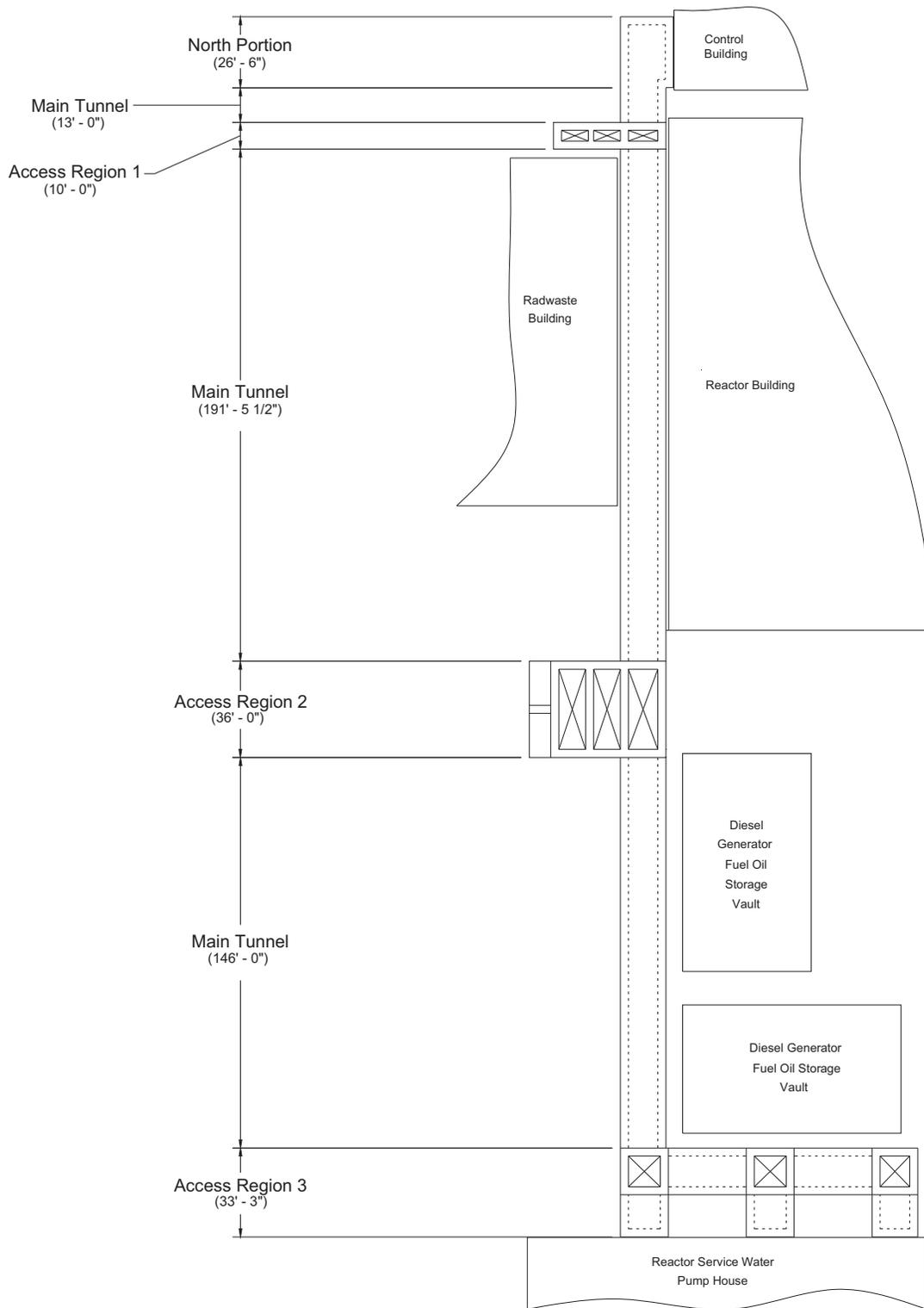


Figure 3H.6-248: RSW Tunnel Plan View

RAI 03.08.04-34, Supplement 1**QUESTION:**

10 CFR 50, Appendix A, GDC 2, requires that structures important to safety shall be designed to withstand the effects of natural phenomena with appropriate combination of the effects of normal and accident conditions. To meet this requirement, all seismic category I structures must be designed for required strength at all locations in the structure. During the October 2010 Audit the applicant presented the procedures to verify the concrete sections of the UHS/PH structural members resulting from the code-required load combinations. The internal forces (i.e. shear, moment, axial force, torsion, etc.) used to determine the required strength of the structural members (i.e. walls, slabs, beam, columns, etc.) of the UHS/PH building are generated by the applicant with the help of SAP2000 models simulating the building's static and dynamic behavior. These element forces are subsequently processed by the applicant with a number of in-house developed programs for design of concrete sections. It was noted that concrete slabs and walls were designed for out-of-plane shear by averaging the element shear forces across cut lines that extended along the entire width of the walls and slabs. The staff considers that averaging of out of plane shear along the entire cut line of a slab or wall could lead to unconservative estimate of shear stress in slabs. The subject was discussed with the applicant during the audit. Although the applicant explained the procedure by referencing to ACI 349-97, Section 11.12, "Special provisions for slabs and footings," it did not provide the staff with a sufficient interpretation of the provision of the ACI code, which appears to be intended for shear strength of slabs and footings in the vicinity of columns, concentrated loads, or reactions, to close this issue. ACI 349-97, Section 13.3.1, states that a slab system may be designed by any procedure satisfying conditions of equilibrium and geometric compatibility, if shown that the design strength at every section is at least equal to the required strength. Averaging of out-of-plane shear across the entire width of a slab may not show that the design strength at every section is at least equal to the required strength. Therefore, in order for the staff to conclude that the site-specific structures are adequately designed for out-of plane shear, the staff requests STP to demonstrate that use of average shear force across the entire width of slab, instead of the shear force demand at every section obtained from analysis may be considered acceptable by any or more of the following:

- Obtain clarification from the ACI regarding validity of use of Section 11.12 of ACI 349-87 for the situations where the provisions of the code were used,
- Provide examples of any precedence where similar methodology was accepted by the staff,
- Provide detailed justification using industry accepted standards, technical references, experimental results, etc., to justify redistribution of the shear forces obtained from finite element analysis.

The applicant is also requested to update the FSAR as necessary.

SUPPLEMENTAL RESPONSE:

The original response to this RAI was submitted with Nuclear Innovation North America (NINA) letter U7-C-NINA-NRC-110050, dated April 5, 2011. This supplement provides the response to the following action item discussed in the NRC audits performed during the weeks of May 23, 2011 and July 25, 2011.

Beam shear discussion (Audit Action Item 3.8-21, Punch List Item 56):

Calculations will be revised and FSAR tables will be updated as a Confirmatory Action

COLA updates reflecting the results of the hydrodynamic mass effect on the columns will be included in this response (Punch List No. 17)

Finite element analysis was used for design of the following structures:

- Ultimate Heat Sink (UHS)/Reactor Service Water (RSW) Pump House
- Diesel Generator Fuel Oil Tunnels (DGFOT)
- Diesel Generator Fuel Oil Storage Vaults (DGFOSV)
- Radwate Building (RWB)

As noted in the original response, unless noted otherwise, design of these structures for out-of-plane shear have been conservatively revised based on finite element analysis results for each element without averaging the shear over several elements. This supplemental response provides the summary of results for UHS/RSW Pump House and DGFOT. The summary of results for DGFOSV and RWB will be provided in RAI 03.08.04-34, Supplement 2 currently scheduled for submittal in October 2011.

UHS/RSW Pump House:

No averaging has been used for out-of-plane shear design except at four locations within the UHS basin basemat at the junction of basin buttresses with the basin basemat. At these four locations, the out-of-plane shear was averaged over a total length of twice the basemat thickness (i.e. $2 \times 10 = 20$ ft, basemat thickness = 10 ft). This averaging of shear is justified for the following reasons:

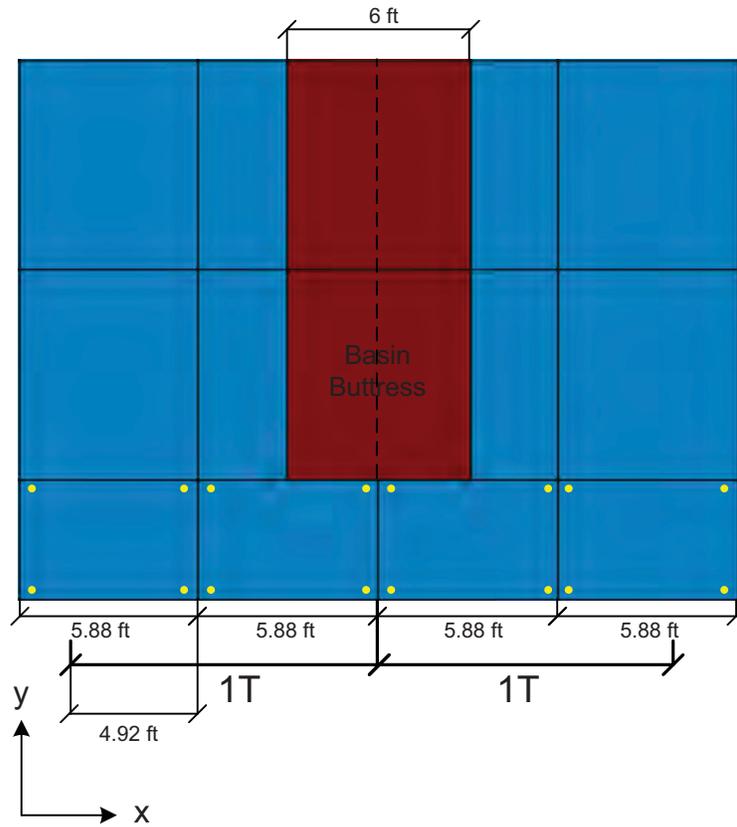
- The local shear concentration is due to excessively refined mesh for a 10 ft thick mat. As can be seen from Figure 03.08.04-34 S1.1, the width of the basemat elements is only 5.88 ft.
- The basin buttresses are 6 ft thick walls. Considering a load distribution of 45° through the basemat thickness will yield an effective strip of 26 ft (i.e. $6 \text{ ft} + 2 \times 10 \text{ ft} = 26 \text{ ft}$) which exceeds the 20 ft used for shear averaging.

Tables 3H.6-7 through 3H.6-9 and Figures 3H.6-51 through 3H.6-74, 3H.6-74A, 3H.6-75 through 3H.6-78, 3H.6-78A, 3H.6-79 through 3H.6-116, 3H.6-116A, 3H.6-117 through 3H.6-120, 3H.6-120A, 3H.6-121 through 3H.6-122, 3H.6-122A, 3H.6-123 through 3H.6-126, 3H.6-126A, 3H.6-127 through 3H.6-134, 3H.6-134A, 3H.6-135, and 3H.6-136A through 3H.6-136C (see enclosure) have been revised to reflect the results of this analysis. In addition, these revised tables also reflect the results of the hydrodynamic mass effect on the UHS basin columns and beams considering the scaled column accelerations discussed in RAI 03.08.04-30, Supplement 4.

DGFOT:

No averaging has been used for out-of-plane shear design. Tables 3H.7-1 and Figures 3H.7-10A, 3H.7-14A, 3H.7-17, and 3H.7-19A (see Enclosure) have been revised to reflect the results of this analysis.

As a result of this response COLA Part 2, Tier 2, Appendix 3H will be revised as shown in the Enclosure.



**Figure 03.08.04-34 S1.1: Partial Plan View of UHS Basin Basemat Mesh
Out-of-plane Shear Averaged over 2T
T = Basemat Thickness, 10 ft**

ENCLOSURE

COLA MARK-UPS

3H.6.6.3.1 UHS Basin/UHS Cooling Tower/RSW Pump House Concrete Wall and Slab Design

For each reinforcement zone, the following in-plane and transverse shears with the corresponding load combination are reported in Tables 3H.6-7 and 3H.6-8. The in-plane shear is the maximum average in-plane shear along a plane that crosses the longitudinal reinforcement zone.

The shell forces from every element for every load combination in the finite element model were evaluated to determine the required transverse reinforcement. The transverse shear and axial force reported in Tables 3H.6-7 and 3H.6-8 correspond to the maximum required transverse reinforcement for an element within that transverse reinforcement zone.

- The in-plane shear is the maximum average in-plane shear along a plane that crosses the longitudinal reinforcement zone.
- The transverse shear is the maximum average transverse shear along a plane in that transverse reinforcement zone.

3H.7.5.3.1 Reinforced Concrete Elements

For each surface, the following in-plane and transverse shears with the corresponding load combination are reported in Table 3H.7-1 when the governing forces, moments and reinforcement is from the SAP2000 models. The in-plane shear is the maximum average in-plane shear along a plane that crosses the longitudinal reinforcement zone.

The shell forces from every element for every load combination in the finite element model were evaluated to determine the required transverse reinforcement. The transverse shear and axial force reported in Table 3H.7-1 correspond to the maximum required transverse reinforcement for an element within that transverse reinforcement zone.

- The in-plane shear is the maximum average in-plane shear along a plane that crosses the longitudinal reinforcement zone.

The transverse shear is the maximum average transverse shear along a plane in that transverse reinforcement zone.

Table 3H.6-7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement Layout	Drawing Number (1)	Reinforcement Zone Number (2)	Maximum Forces (3)	Element	Longitudinal Reinforcement Design Loads				Longitudinal Reinforcement (ft ² /ft)	Transverse Shear Design Loads				Remarks	
									Axial and Flexure Loads		In-Plane Shear Loads			Load Combination	Transverse Shear Force (kips/ft)	Compressive Axial Force (kips/ft)	Vertical Section		Compressive Axial Force (kips/ft)
									Axial (kips/ft)	Moment (kips-ft)	In-Plane Shear (kips/ft)	Load Combination							
Pump House East Wall (Cont'd)	9	East (outside)	Vertical	3H.6.7				MTCM 8228	D-L-F-H-T-E	275	-124	D-L-F-H-T-E	247	-	-	-	-	-	
								MCCM 8653	D-L-F-H-T-E	-251	-182								
								MMAT 8654	D-L-F-H-T-E	62	-531								
								MMAC 8654	D-L-F-H-T-E	-389	-842								
								MTCM 8628	D-L-F-H-T-E	76	-59								
								MCCM 8630	D-L-F-H-T-E	-306	-41								
								MMAT 3097	D-L-F-H-T-E	36	-299								
								MMAC 8491	D-L-F-H-T-E	-112	-344								
								MTCM 8698	1.00D+1.3L+0.6F+1.3M+1.2T+1.3W	198	-67								
								MCCM 8628	D-L-F-H-T-E	-294	-62								
								MMAT 8686	D-L-F-H-T-E	109	-229								
								MMAC 8647	D-L-F-H-T-E	-40	-221								
MTCM 8628	1.40+1.2L+1.3T+1.3M+1.3W	242	-411																
MCCM 8346	D-L-F-H-T-E	-440	-653																
MMAT 8118	D-L-F-H-T-E	9	-336																
MMAC 8869	D-L-F-H-T-E	-251	-484																
MTCM 8222	D-L-F-H-T-E	609	40																
MCCM 8222	D-L-F-H-T-E	-814	668																
MMAT 8222	D-L-F-H-T-E	189	868																
MMAC 8222	D-L-F-H-T-E	-314	868																
MTCM 8988	D-L-F-H-T-E	262	129																
MCCM 8988	D-L-F-H-T-E	-301	46																
MMAT 3100	D-L-F-H-T-E	27	357																
MMAC 3100	D-L-F-H-T-E	-62	357																
MTCM 8884	D-L-F-H-T-E	108	179																
MCCM 8824	D-L-F-H-T-E	-514	552																
MMAT 8822	D-L-F-H-T-E	57	415																
MMAC 8822	D-L-F-H-T-E	-469	562																
MTCM 8827	1.40+1.6F+1.3M+1.3W	62	65																
MCCM 8827	D-L-F-H-T-E	-465	204																
MMAT 8851	D-L-F-H-T-E	6	817																
MMAC 8851	D-L-F-H-T-E	-410	862																
MTCM 8222	D-L-F-H-T-E	649	146																
MCCM 8825	D-L-F-H-T-E	-684	1232																
MMAT 8825	D-L-F-H-T-E	-	-																
MMAC 8825	D-L-F-H-T-E	-626	1815																
MTCM 8226	D-L-F-H-T-E	199	51																
MCCM 8853	D-L-F-H-T-E	-435	833																
MMAT 8854	D-L-F-H-T-E	2	1176																
MMAC 8853	D-L-F-H-T-E	-491	1604																
MTCM 8241	D-L-F-H-T-E	69	49																
MCCM 8900	D-L-F-H-T-E	-267	62																
MMAT 8397	D-L-F-H-T-E	1	590																
MMAC 8888	D-L-F-H-T-E	-284	691																

(1) In-Plane Shear Reinforcement (ft²/ft)
(2) Transverse Shear Reinforcement (ft²/ft)
(3) Compressive Axial Force (kips/ft)

Table 3H.6-7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement Layout	Drawing Number (1)	Reinforcement Zone Number (2)	Maximum Forces (3)	Element	Longitudinal Reinforcement Design Loads				Longitudinal Reinforcement (ft ² /ft)	Transverse Shear Design Loads				Transverse Shear Reinforcement (ft ² /ft)	Remarks									
									Asix and Flexure Loads	In-Plane Shear Loads	Longitudinal Reinforcement (ft ² /ft)	Transverse Shear Force (kip/ft)		Horizontal Section	Vertical Section	Compressive Axial Force (kip/ft)												
									Asix (ft ² /ft)	Flexure (ft ² /ft)	In-Plane Shear Load Combination	Reinforcement (ft ² /ft)	Transverse Shear Force (kip/ft)	Transverse Shear Force (kip/ft)	Vertical Section	Compressive Axial Force (kip/ft)												
Pump House East Wall (Cont'd)	9	West (outside)	Vertical	3H.6.6.9	14L	3	MTCM	6444	D-L-F-H-T-E	46	202	D-L-F-H-T-E	175	-	-	-	-	-										
									MCCM	6050	D-L-F-H-T-E	320	20	-	-	-	-	-	-									
									MMAT	6056	D-L-F-H-T-E	1	533	-	-	-	-	-	-									
									MMAC	3077	D-L-F-H-T-E	-86	561	-	-	-	-	-	-									
									MTCM	6058	D-L-F-H-T-E	76	36	-	-	-	-	-	-									
									MCCM	6052	D-L-F-H-T-E	244	217	-	-	-	-	-	-									
									MMAT	6055	D-L-F-H-T-E	4	308	-	-	-	-	-	-									
									MMAC	3105	D-L-F-H-T-E	-46	321	-	-	-	-	-	-									
									MTCM	6059	D-L-F-H-T-E	211	116	-	-	-	-	-	-									
									MCCM	6050	D-L-F-H-T-E	300	164	-	-	-	-	-	-									
									MMAT	6050	D-L-F-H-T-E	2	222	-	-	-	-	-	-									
									MMAC	6020	D-L-F-H-T-E	-239	228	-	-	-	-	-	-									
Pump House East Wall (Cont'd)	9	West (outside)	Vertical	3H.6.6.6	14L	3	MTCM	6444	D-L-F-H-T-E	41	34	D-L-F-H-T-E	41	164	542	0.00	-											
									MCCM	6050	D-L-F-H-T-E	130	-200	-	-	-	-	-	-									
									MMAT	6056	D-L-F-H-T-E	49	76	-	-	-	-	-	-									
									MMAC	3105	D-L-F-H-T-E	43	32	-	-	-	-	-	-									
									MTCM	6059	D-L-F-H-T-E	327	110	-	-	-	-	-	-									
									MCCM	6050	D-L-F-H-T-E	507	530	-	-	-	-	-	-									
									MMAT	6050	D-L-F-H-T-E	2	2	-	-	-	-	-	-									
									MMAC	6020	D-L-F-H-T-E	-58	93	-	-	-	-	-	-									
									Pump House South Wall	9	South (outside)	Horizontal	3H.6.6.3	14L	3	MTCM	6444	D-L-F-H-T-E	249	43	D-L-F-H-T-E	235	-	-	-	-		
																		MCCM	6051	D-L-F-H-T-E	-115	-117	-	-	-	-	-	-
																		MMAT	6194	D-L-F-H-T-E	6	-639	-	-	-	-	-	-
																		MMAC	6194	D-L-F-H-T-E	-69	-639	-	-	-	-	-	-
MTCM	6194	D-L-F-H-T-E	149	-102	-	-	-	-										-	-									
MCCM	6067	D-L-F-H-T-E	397	-298	-	-	-	-										-	-									
MMAT	6193	D-L-F-H-T-E	0	-462	-	-	-	-										-	-									
MMAC	6193	D-L-F-H-T-E	-239	-463	-	-	-	-										-	-									
MTCM	6196	D-L-F-H-T-E	243	-411	-	-	-	-										-	-									
MCCM	6069	D-L-F-H-T-E	1095	-601	-	-	-	-										-	-									
MMAT	6196	D-L-F-H-T-E	130	-1204	-	-	-	-										-	-									
MMAC	6196	D-L-F-H-T-E	-605	-1401	-	-	-	-										-	-									
Pump House South Wall	9	South (outside)	Horizontal	3H.6.6.3	14L	3	MTCM	6444	D-L-F-H-T-E	97	209	D-L-F-H-T-E	235	-	-	-	-											
									MCCM	6069	D-L-F-H-T-E	620	192	-	-	-	-	-	-									
									MMAT	6194	D-L-F-H-T-E	25	712	-	-	-	-	-	-									
									MMAC	6194	D-L-F-H-T-E	-163	785	-	-	-	-	-	-									
									MTCM	6067	D-L-F-H-T-E	164	186	-	-	-	-	-	-									
									MCCM	6067	D-L-F-H-T-E	722	17	-	-	-	-	-	-									
									MMAT	6194	D-L-F-H-T-E	0	519	-	-	-	-	-	-									
									MMAC	6197	D-L-F-H-T-E	-261	1158	-	-	-	-	-	-									
									Pump House South Wall	9	South (outside)	Horizontal	3H.6.6.6	14L	3	MTCM	6444	D-L-F-H-T-E	42	178	D-L-F-H-T-E	42	140	91	0.31			
																		MCCM	6050	D-L-F-H-T-E	13	-140	-	-	-	-	-	-
																		MMAT	6229	D-L-F-H-T-E	252	-58	-	-	-	-	-	-
																		MMAC	3098	D-L-F-H-T-E	69	-407	-	-	-	-	-	-
MTCM	6067	D-L-F-H-T-E	102	-204	-	-	-	-										-	-									
MCCM	6069	D-L-F-H-T-E	462	-106	-	-	-	-										-	-									
MMAT	6229	D-L-F-H-T-E	252	-58	-	-	-	-										-	-									
MMAC	3098	D-L-F-H-T-E	69	-407	-	-	-	-										-	-									
Pump House South Wall	9	West (outside)	Horizontal	3H.6.6.6	14L	3	MTCM	6444										D-L-F-H-T-E	13	140	D-L-F-H-T-E	13	100	46	0.20			
																		MCCM	6050	D-L-F-H-T-E	42	-140	-	-	-	-	-	-
																		MMAT	6229	D-L-F-H-T-E	252	-58	-	-	-	-	-	-
																		MMAC	3098	D-L-F-H-T-E	69	-407	-	-	-	-	-	-
									MTCM	6067	D-L-F-H-T-E	102	-204	-	-	-	-	-	-									
									MCCM	6069	D-L-F-H-T-E	462	-106	-	-	-	-	-	-									
									MMAT	6229	D-L-F-H-T-E	252	-58	-	-	-	-	-	-									
									MMAC	3098	D-L-F-H-T-E	69	-407	-	-	-	-	-	-									

Table 3H.6-7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement (ft)	Driving Number (ft)	Zone Number (ft)	Maximum Forces (ft)	Element	Longitudinal Reinforcement Design Loads				Transverse Reinforcement Design Loads				Longitudinal Reinforcement (ft ² /ft)	Results
									Axial and Flexure Loads		In-Plane Shear Loads		Horizontal Section		Vertical Section			
									Load Combination	Axial (ft) (kips/ft)	Moment (ft) (kips-ft)	Load Combination	Force (ft) (kips/ft)	Shear (ft) (kips/ft)	Transverse Shear Force (kips/ft)	Compressive Axial Force (kips/ft)		
Pump House Baffles	0	Horizontal	Vertical	3H-63	3H-63	14L	MOM	1330	1050 + 1.3L + 1.09F + 1.3W + 1.2T + 1.3W	220	9	D-L-F-H-T-E	218	4.68	-	-	-	-
								MOM	1345	D-L-F-H-T-E	275	53	-	-	-	-	-	-
								MAT	1346	D-L-F-H-T-E	69	198	-	-	-	-	-	-
								MWAC	1349	D-L-F-H-T-E	-50	142	-	-	-	-	-	-
								MIDM	1350	D-L-F-H-T-E	188	-69	-	-	-	-	-	-
								MCOM	1340	D-L-F-H-T-E	-261	-69	-	-	-	-	-	-
								MWAT	13414	D-L-F-H-T-E	103	145	-	-	-	-	-	-
								MWAC	13414	D-L-F-H-T-E	-48	143	-	-	-	-	-	-
								MIDM	13410	D-L-F-H-T-E	471	32	-	-	-	-	-	-
								MCOM	13407	1.6D + 1.3L + 1.09F + 1.3W + 1.7W	-321	288	-	-	-	-	-	-
								MWAT	13407	D-L-F-H-T-E	7	475	-	-	-	-	-	-
								MWAC	13407	D-L-F-H-T-E	-127	477	-	-	-	-	-	-
								MIDM	13410	D-L-F-H-T-E	-	-	-	-	-	-	-	-
								MCOM	13407	D-L-F-H-T-E	1055	-246	-	-	-	-	-	-
UHS Baffle North Wall	6	North (Outside)	Vertical	3H-63	3H-63	14L	MOM	1077	D-L-F-H-T-E	1055	-246	D-L-F-H-T-E	42	12.48	-	-	-	-
								MCOM	1075	D-L-F-H-T-E	-244	-499	-	-	-	-	-	-
								MWAT	1081	D-L-F-H-T-E	57	-1311	-	-	-	-	-	-
								MWAC	1081	D-L-F-H-T-E	-133	-1311	-	-	-	-	-	-
								MIDM	1085	1.6D + 1.3F + 1.3W + 1.4T	648	-159	-	-	-	-	-	-
								MCOM	1079	1.05D + 1.3L + 1.09F + 1.3W + 1.2T + 1.3W	-612	-182	-	-	-	-	-	-
								MWAT	1089	D-L-F-H-T-E	39	-668	-	-	-	-	-	-
								MWAC	1089	D-L-F-H-T-E	-149	-1036	-	-	-	-	-	-
								MIDM	1076	1.6D + 1.3F + 1.3W + 1.4T	282	-338	-	-	-	-	-	-
								MCOM	1080	D-L-F-H-T-E	468	-46	-	-	-	-	-	-
								MWAT	1075	1.6D + 1.3L + 1.09F + 1.3W + 1.7W + 1.7W	66	-323	-	-	-	-	-	-
								MWAC	1074	1.6D + 1.3F + 1.3W + 1.4T	-48	-477	-	-	-	-	-	-
								MIDM	1077	D-L-F-H-T-E	248	-159	-	-	-	-	-	-
								MCOM	1078	D-L-F-H-T-E	-334	-101	-	-	-	-	-	-
MWAT	1078	D-L-F-H-T-E	26	-664	-	-	-	-	-	-								
MWAC	1078	D-L-F-H-T-E	-269	-664	-	-	-	-	-	-								
UHS Baffle North Wall	6	North (Outside)	Vertical	3H-63	3H-63	24L	MOM	1080	D-L-F-H-T-E	259	-190	D-L-F-H-T-E	159	4.68	-	-	-	-
								MCOM	1079	D-L-F-H-T-E	-320	-41	-	-	-	-	-	-
								MWAT	1078	D-L-F-H-T-E	0	-713	-	-	-	-	-	-
								MWAC	1078	D-L-F-H-T-E	-144	-713	-	-	-	-	-	-
								MIDM	1080	D-L-F-H-T-E	313	-184	-	-	-	-	-	-
								MCOM	1076	D-L-F-H-T-E	-332	-149	-	-	-	-	-	-
								MWAT	1076	D-L-F-H-T-E	76	-236	-	-	-	-	-	-
								MWAC	1076	D-L-F-H-T-E	-188	-236	-	-	-	-	-	-
								MIDM	1077	D-L-F-H-T-E	473	-599	-	-	-	-	-	-
								MCOM	1088	D-L-F-H-T-E	-607	-206	-	-	-	-	-	-
								MWAT	1074	D-L-F-H-T-E	133	-600	-	-	-	-	-	-
								MWAC	1074	D-L-F-H-T-E	-49	-600	-	-	-	-	-	-
								MIDM	1080	D-L-F-H-T-E	281	-59	-	-	-	-	-	-
								MCOM	1080	D-L-F-H-T-E	-284	-61	-	-	-	-	-	-
MWAT	1410	1.05D + 1.3L + 1.09F + 1.3W + 1.2T + 1.3W	153	-372	-	-	-	-	-	-								
MWAC	1410	D-L-F-H-T-E	-5	-350	-	-	-	-	-	-								

Table 3H.6-7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement (ft)	Reinforcement (ft)	Zone Number	Maximum Forces	Element	Longitudinal Reinforcement Design Loads				Transverse Shear Design Loads				Longitudinal Reinforcement (ft ² /ft)	Results		
									Axial and Flexure Loads		In-Plane Shear Loads		Horizontal Section		Vertical Section					
									Load Combination	Axial (ft)	Moment (ft-kip)	Load Combination	Shear (ft)	Transverse Shear Force (kip/ft)	Constr. Shear Force (kip/ft)	Transverse Shear Force (kip/ft)			Constr. Shear Force (kip/ft)	
UHS Base in North Wall (Cont'd)	8	North (outside)	Vertical	3H-63					600C	D+L+FH+T+E	375	-744	D+L+FH+T+E	-	-	-	-	-	-	
									600M	D+L+FH+T+E	462	-582	D+L+FH+T+E	222	9.36	-	-	-	-	-
									600S	D+L+FH+T+E	375	-744	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	-189	-744	D+L+FH+T+E	-	-	-	-	-	-	-
									600W	1.4D+1.3F+1.3M+1.4T+0	143	-152	D+L+FH+T+E	222	6.24	-	-	-	-	-
									600N	D+L+FH+T+E	658	-157	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	3	-589	D+L+FH+T+E	-	-	-	-	-	-	-
									600W	D+L+FH+T+E	272	-587	D+L+FH+T+E	-	-	-	-	-	-	-
									600N	1.4D+1.3F+1.3M+1.4T+0	300	336	D+L+FH+T+E	-	-	-	-	-	-	-
									600M	D+L+FH+T+E	-265	368	D+L+FH+T+E	113	9.36	-	-	-	-	-
									600S	1.4D+1.3F+1.3M+1.4T+0	172	1113	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	-3	1062	D+L+FH+T+E	-	-	-	-	-	-	-
UHS Base in North Wall (Cont'd)	8	Horizontal							600C	1.6D+1.3L+1.09F+1.3M+1.2T+1.3W	1029	209	D+L+FH+T+E	42	14.04	-	-	-	-	
									600M	D+L+FH+T+E	-294	193	D+L+FH+T+E	-	-	-	-	-	-	-
									600S	D+L+FH+T+E	108	1279	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	-77	1279	D+L+FH+T+E	-	-	-	-	-	-	-
									600W	1.4D+1.3L+1.09F+1.3M+1.2T+1.3W	629	417	D+L+FH+T+E	-	-	-	-	-	-	-
									600M	D+L+FH+T+E	-344	210	D+L+FH+T+E	63	9.36	-	-	-	-	-
									600S	1.4D+1.3L+1.09F+1.3M+1.2T+1.3W	224	900	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	-4	895	D+L+FH+T+E	-	-	-	-	-	-	-
									600W	1.4D+1.3L+1.09F+1.3M+1.2T+1.3W	179	227	D+L+FH+T+E	149	6.24	-	-	-	-	-
									600M	D+L+FH+T+E	-658	152	D+L+FH+T+E	-	-	-	-	-	-	-
									600S	1.4D+1.3L+1.09F+1.3M+1.2T+1.3W	58	943	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	-129	975	D+L+FH+T+E	-	-	-	-	-	-	-
UHS Base in North Wall (Cont'd)	8	South (inside)							600C	1.4D+1.3F+1.3M+1.4T+0	664	777	D+L+FH+T+E	-	-	-	-	-		
									600M	1.6D+1.3L+1.09F+1.3M+1.2T+1.3W	-496	99	D+L+FH+T+E	176	12.48	-	-	-	-	-
									600S	D+L+FH+T+E	127	1461	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	-24	1347	D+L+FH+T+E	-	-	-	-	-	-	-
									600W	1.4D+1.3F+1.3M+1.4T+0	522	61	D+L+FH+T+E	-	-	-	-	-	-	-
									600M	D+L+FH+T+E	-384	293	D+L+FH+T+E	176	12.48	-	-	-	-	-
									600S	1.4D+1.3L+1.09F+1.3M+1.2T+1.3W	149	1296	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	-9	1193	D+L+FH+T+E	-	-	-	-	-	-	-
									600W	D+L+FH+T+E	248	53	D+L+FH+T+E	139	4.68	-	-	-	-	-
									600M	D+L+FH+T+E	-268	141	D+L+FH+T+E	-	-	-	-	-	-	-
									600S	D+L+FH+T+E	28	341	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	1.4D+1.3L+1.09F+1.3M+1.2T+1.3W	-47	358	D+L+FH+T+E	-	-	-	-	-	-	-
UHS Base in North Wall (Cont'd)	8	Vertical	3H-65						600C	D+L+FH+T+E	309	35	D+L+FH+T+E	-	-	-	-	-		
									600M	D+L+FH+T+E	-269	183	D+L+FH+T+E	211	6.24	-	-	-	-	-
									600S	D+L+FH+T+E	23	423	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	1.4D+1.3L+1.09F+1.3M+1.2T+1.3W	-97	476	D+L+FH+T+E	-	-	-	-	-	-	-
									600W	D+L+FH+T+E	475	411	D+L+FH+T+E	-	-	-	-	-	-	-
									600M	D+L+FH+T+E	-507	713	D+L+FH+T+E	-	-	-	-	-	-	-
									600S	D+L+FH+T+E	39	713	D+L+FH+T+E	-	-	-	-	-	-	-
									600E	D+L+FH+T+E	-507	713	D+L+FH+T+E	-	-	-	-	-	-	-
									600W	1.4D+1.3L+1.09F+1.3M+1.2T+1.3W	607	713	D+L+FH+T+E	-	-	-	-	-	-	-
									600M	D+L+FH+T+E	-607	713	D+L+FH+T+E	-	-	-	-	-	-	-
									600S	D+L+FH+T+E	607	713	D+L+FH+T+E	-	-	-	-	-	-	-

Table 3H.6.7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement (ft)	Reinforcement (ft)	Zone Number	Maximum Forces	Element	Longitudinal Reinforcement Design Loads				Transverse Shear Design Loads				Results		
									Axial and Flexure Loads		In-Plane Shear Loads		Horizontal Section		Vertical Section			Transverse Shear Reinforcement (ft/ft)	Results
									Load Combination	Axial (k) (kips/ft)	Moment (k) (kips-ft)	Load Combination	In-plane (k) (kips/ft)	Transverse Shear Force (k/ft)	Compressive Axial Force (k/ft)	Transverse Shear Force (k/ft)			
UHS Basin South Wall (Cont'd)	8	North (inside)	Horizontal	3H.6.0	Vertical	3H.6.0			MTDM 2182	D+L+FF+H+T+E	217	-103	D+L+FF+H+T+E	-	-	-	-	-	-
									MCOM 1973	D+L+FF+H+T+E	-358	-38	D+L+FF+H+T+E	-	-	-	-	-	-
									MWAT 1972	D+L+FF+H+T+E	7	-537	D+L+FF+H+T+E	-	-	-	-	-	-
									MWAC 1868	D+L+FF+H+T+E	-175	-601	D+L+FF+H+T+E	-	-	-	-	-	-
									MTDM 1880	D+L+FF+H+T+E	227	-508	D+L+FF+H+T+E	-	-	-	-	-	-
									MCOM 1880	D+L+FF+H+T+E	-237	-125	D+L+FF+H+T+E	-	-	-	-	-	-
									MWAT 1880	1.4D+1.3L+1.3F+1.7W+1.7W	155	-370	D+L+FF+H+T+E	-	-	-	-	-	-
									MWAC 1880	D+L+FF+H+T+E	-52	-395	D+L+FF+H+T+E	-	-	-	-	-	-
									MTDM 2032	1.4D+1.3F+1.3W+1.4T0	351	424	D+L+FF+H+T+E	-	-	-	-	-	-
									MCOM 2032	D+L+FF+H+T+E	-249	428	D+L+FF+H+T+E	-	-	-	-	-	-
									MWAT 4318	D+L+FF+H+T+E	198	1608	D+L+FF+H+T+E	-	-	-	-	-	-
									MWAC 4318	D+L+FF+H+T+E	-79	1608	D+L+FF+H+T+E	-	-	-	-	-	-
MTDM 4475	D+L+FF+H+T+E	607	384	D+L+FF+H+T+E	-	-	-	-	-	-									
MCOM 4382	D+L+FF+H+T+E	-329	339	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAT 4497	D+L+FF+H+T+E	70	698	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAC 4497	D+L+FF+H+T+E	-99	698	D+L+FF+H+T+E	-	-	-	-	-	-									
MTDM 3975	D+L+FF+H+T+E	275	280	D+L+FF+H+T+E	-	-	-	-	-	-									
MCOM 3507	D+L+FF+H+T+E	-382	193	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAT 4438	1.4D+1.3L+1.3F+1.7W+1.7W	88	713	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAC 4432	1.4D+1.3L+1.3F+1.7W+1.7W	-37	729	D+L+FF+H+T+E	-	-	-	-	-	-									
MTDM 2198	1.4D+1.3F+1.3W+1.4T0	309	154	D+L+FF+H+T+E	-	-	-	-	-	-									
MCOM 2118	D+L+FF+H+T+E	-191	671	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAT 2140	1.4D+1.3F+1.3W+1.4T0	288	848	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAC 2092	1.4D+1.3L+1.3F+1.7W+1.7W	-21	852	D+L+FF+H+T+E	-	-	-	-	-	-									
MTDM 1792	1.4D+1.3F+1.3W+1.4T0	232	69	D+L+FF+H+T+E	-	-	-	-	-	-									
MCOM 1096	1.05D+1.3L+1.05F+1.3W+1.3T1+1.3W	-244	214	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAT 1887	1.4D+1.3L+1.3F+1.7W+1.7W	64	720	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAC 1897	1.4D+1.3F+1.3W+1.4T0	-43	728	D+L+FF+H+T+E	-	-	-	-	-	-									
MTDM 2244	1.4D+1.3F+1.3W+1.4T0	386	588	D+L+FF+H+T+E	-	-	-	-	-	-									
MCOM 3836	1.05D+1.3L+1.05F+1.3W+1.3T1+1.3W	-246	38	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAT 4505	D+L+FF+H+T+E	111	1548	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAC 4505	D+L+FF+H+T+E	-76	1548	D+L+FF+H+T+E	-	-	-	-	-	-									
MTDM 3500	1.05D+1.3L+1.05F+1.3W+1.3T1+1.3W	197	42	D+L+FF+H+T+E	-	-	-	-	-	-									
MCOM 1014	D+L+FF+H+T+E	-273	120	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAT 4317	D+L+FF+H+T+E	12	328	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAC 1119	1.4D+1.3F+1.3W+1.4T0	-127	451	D+L+FF+H+T+E	-	-	-	-	-	-									
MTDM 3507	1.05D+1.3L+1.05F+1.3W+1.3T1+1.3W	251	15	D+L+FF+H+T+E	-	-	-	-	-	-									
MCOM 1197	D+L+FF+H+T+E	-250	142	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAT 4575	D+L+FF+H+T+E	24	255	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAC 1197	1.4D+1.3L+1.3F+1.7W+1.7W	-259	308	D+L+FF+H+T+E	-	-	-	-	-	-									
MTDM 2199	D+L+FF+H+T+E	238	25	D+L+FF+H+T+E	-	-	-	-	-	-									
MCOM 1036	D+L+FF+H+T+E	-324	170	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAT 1880	D+L+FF+H+T+E	6	344	D+L+FF+H+T+E	-	-	-	-	-	-									
MWAC 1291	1.4D+1.3F+1.3W+1.4T0	-128	447	D+L+FF+H+T+E	-	-	-	-	-	-									

Table 3H.6.7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement (Drawing Layer)	Reinforcement (Zone Number)	Maximum Forces (k)	Eminent	Longitudinal Reinforcement Design Loads				Transverse Shear Design Loads				Transverse Shear Reinforcement (ft ² /ft)	Remarks					
								Axial and Flexure Loads		In-Plane Shear Loads		Horizontal Section		Vertical Section								
								Load Combination	Axial (k) (kips/ft)	Flexure (k) (kips-ft/ft)	Load Combination	In-plane (k) (kips/ft)	Transverse Shear Force (k/ft)	Compressive Axial Force (k/ft)	Transverse Shear Force (k/ft)			Compressive Axial Force (k/ft)				
UHS Brain South Wall (Cont'd)	9	North (panels)	Vertical	3H.6.91	14-VL	MDCM	242C	D+L+FH+T+E	250	67	D+L+FH+T+E	-	-	-	-	-	-					
							MDCM	165D	D+L+FH+T+E	-520	193	D+L+FH+T+E	174	0.24	-	-	-	-	-	-		
							MMAI	155D	D+L+FH+T+E	5	203	D+L+FH+T+E	-	-	-	-	-	-	-	-		
							MWAC	100D	D+L+FH+T+E	-311	203	D+L+FH+T+E	-	-	-	-	-	-	-	-		
							MDCM	210D	D+L+FH+T+E	217	32	D+L+FH+T+E	-	-	-	-	-	-	-	-		
							MDCM	170D	D+L+FH+T+E	-209	137	D+L+FH+T+E	148	4.68	-	-	-	-	-	-		
							MMAI	404A	D+L+FH+T+E	14	375	D+L+FH+T+E	-	-	-	-	-	-	-	-		
							MWAC	388B	D+L+FH+T+E	-75	402	D+L+FH+T+E	-	-	-	-	-	-	-	-		
							MDCM	188B	D+L+FH+T+E	222	38	D+L+FH+T+E	-	-	-	-	-	-	-	-		
							MDCM	185E	D+L+FH+T+E	-388	588	D+L+FH+T+E	174	7.8	-	-	-	-	-	-		
							MMAI	189E	D+L+FH+T+E	27	837	D+L+FH+T+E	-	-	-	-	-	-	-	-		
							MWAC	1791	1.4D+1.3F+1.3M+1.4T0	-159	1307	D+L+FH+T+E	-	-	-	-	-	-	-	-		
							3H.6.91	3H.6.91	-	-	-	-	-	-	-	-	-	-	-	-	-	-
							2.1	2.1	-	-	-	-	-	-	-	-	-	-	-	-	-	-
							UHS Brain East Wall	9	East (panels)	Horizontal	3H.6.92	24-HL	MDCM	324	1.0D+1.3L+1.09F+1.3M+1.27+1.3W	410	-66	D+L+FH+T+E	-	-	-	-
MDCM	525D	D+L+FH+T+E	-311	-1019	D+L+FH+T+E	40								12.48	-	-	-	-	-			
MMAI	5241	D+L+FH+T+E	64	-2078	D+L+FH+T+E	-								-	-	-	-	-	-			
MWAC	5241	D+L+FH+T+E	-222	-2130	D+L+FH+T+E	-								-	-	-	-	-	-			
MDCM	2611	1.4D+1.3F+1.3M+1.4T0	216	-508	D+L+FH+T+E	71								6.24	-	-	-	-	-	-		
MDCM	3064	1.0D+1.3L+1.09F+1.3M+1.27+1.3W	-346	-25	D+L+FH+T+E	-								-	-	-	-	-	-			
MMAI	3062	D+L+FH+T+E	37	-668	D+L+FH+T+E	-								-	-	-	-	-	-			
MWAC	3056	D+L+FH+T+E	-156	-1033	D+L+FH+T+E	-								-	-	-	-	-	-			
MDCM	2000	1.4D+1.3F+1.3M+1.4T0	383	-216	D+L+FH+T+E	78								7.8	-	-	-	-	-			
MDCM	2622	1.0D+1.3L+1.09F+1.3M+1.27+1.3W	-250	-156	D+L+FH+T+E	-								-	-	-	-	-	-			
MMAI	195E	1.4D+1.3F+1.3M+1.4T0	103	-568	D+L+FH+T+E	-								-	-	-	-	-	-			
MWAC	1996	D+L+FH+T+E	-21	-539	D+L+FH+T+E	-								-	-	-	-	-	-			
MDCM	2449	1.4D+1.3F+1.3M+1.4T0	275	-248	D+L+FH+T+E	106								6.24	-	-	-	-	-			
MDCM	2620	1.0D+1.3L+1.09F+1.3M+1.27+1.3W	-182	-111	D+L+FH+T+E	-								-	-	-	-	-	-			
MMAI	2640	1.4D+1.3F+1.3M+1.4T0	162	-566	D+L+FH+T+E	-								-	-	-	-	-	-			
MWAC	2627	D+L+FH+T+E	-101	-489	D+L+FH+T+E	-	-	-	-	-	-	-										
UHS Brain East Wall	9	Vertical	3H.6.93	14-VL	MDCM	237E	D+L+FH+T+E	266	-222	D+L+FH+T+E	-	-	-	-	-	-						
						MDCM	293C	D+L+FH+T+E	-460	-157	D+L+FH+T+E	129	6.24	-	-	-	-					
						MMAI	429E	D+L+FH+T+E	0	-683	D+L+FH+T+E	-	-	-	-	-	-					
						MWAC	5244	D+L+FH+T+E	-283	-1073	D+L+FH+T+E	-	-	-	-	-	-					
						MDCM	420E	1.0D+1.3L+1.09F+1.3M+1.27+1.3W	410	107	D+L+FH+T+E	40	16.6	-	-	-	-	-				
						MDCM	523D	D+L+FH+T+E	-311	471	D+L+FH+T+E	-	-	-	-	-	-	-				
						MMAI	523E	D+L+FH+T+E	209	2186	D+L+FH+T+E	-	-	-	-	-	-	-				
						MWAC	523E	D+L+FH+T+E	-67	2124	D+L+FH+T+E	-	-	-	-	-	-	-				
						MDCM	252F	1.4D+1.3F+1.3M+1.4T0	386	546	D+L+FH+T+E	106	10.92	-	-	-	-	-				
						MDCM	399D	1.0D+1.3L+1.09F+1.3M+1.27+1.3W	-260	96	D+L+FH+T+E	-	-	-	-	-	-	-				
						MMAI	399D	D+L+FH+T+E	138	1469	D+L+FH+T+E	-	-	-	-	-	-	-				
						MWAC	3890	D+L+FH+T+E	-7	1413	D+L+FH+T+E	-	-	-	-	-	-	-				
						MDCM	2628	1.4D+1.3F+1.3M+1.4T0	204	101	D+L+FH+T+E	71	6.24	-	-	-	-	-				
						MDCM	3097	1.0D+1.3L+1.09F+1.3M+1.27+1.3W	-346	-20	D+L+FH+T+E	-	-	-	-	-	-	-				
						MMAI	244	1.4D+1.3F+1.3M+1.4T0	81	891	D+L+FH+T+E	-	-	-	-	-	-	-				
MWAC	523E	D+L+FH+T+E	-99	756	D+L+FH+T+E	-	-	-	-	-	-	-										

Table 3H.6.7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement (ft)	Drawing Number (1)	Reinforcement Layout	Zone Number (2)	Maximum Forces (3)	Element	Longitudinal Reinforcement Design Loads				Transverse Shear Design Loads				Transverse Shear Reinforcement (ft ² /ft)	Results				
										Axial and Flexure Loads		In-Plane Shear Loads		Horizontal Section		Vertical Section				Transverse Shear Force (kip/ft)	Compressive Axial Force (kip/ft)	Transverse Shear Force (kip/ft)	Compressive Axial Force (kip/ft)
										Load Combination	Axial (R) (kip/ft)	Moment (R) (kip-ft)	Load Combination	In-plane (R) (kip/ft)	Transverse Shear Force (kip/ft)	Compressive Axial Force (kip/ft)	Transverse Shear Force (kip/ft)						
UHS Basin Wall (Vt Core)	8	East (exterior)	Horizontal	3H.6.7	3H.6.7	44-L	M10M	M10M	2315	140+1.3F+1.3M+1.4R	666	-380	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	2314	140+1.3F+1.3M+1.4R	271	-527	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	2314	140+1.3F+1.3M+1.4R	3	-514	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	2314	140+1.3F+1.3M+1.4R	-40	-614	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	2302	140+1.3F+1.3M+1.4R	200	-296	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	2405	140+1.3F+1.3M+1.4R	-214	-44	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	1903	140+1.3F+1.3M+1.4R	72	-514	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	1903	140+1.3F+1.3M+1.4R	-49	-481	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	2239	140+1.3F+1.3M+1.4R	617	-67	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	2096	140+1.3F+1.3M+1.4R	-172	-183	D-L-F-H+T+E	141	7.8	-	-	-	-				
								M10M	2096	140+1.3F+1.3M+1.4R	73	-904	D-L-F-H+T+E	141	7.8	-	-	-	-				
								UHS Basin Wall (Vt Core)	8	West (interior)	Horizontal	3H.6.8	3H.6.8	44-L	M10M	2304	140+1.3F+1.3M+1.4R	238	-115	D-L-F-H+T+E	133	6.24	-
M10M	2405	140+1.3F+1.3M+1.4R	-278	-69	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	2004	140+1.3F+1.3M+1.4R	40	-704	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	3900	140+1.3F+1.3M+1.4R	-75	-725	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	2239	140+1.3F+1.3M+1.4R	294	-286	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	2095	140+1.3F+1.3M+1.4R	-379	-150	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	2320	140+1.3F+1.3M+1.4R	75	-791	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	5170	140+1.3F+1.3M+1.4R	-256	-1009	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	2402	140+1.3F+1.3M+1.4R	294	-280	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	2097	140+1.3F+1.3M+1.4R	-453	-63	D-L-F-H+T+E	133	6.24								-	-	-	-					
M10M	4253	140+1.3F+1.3M+1.4R	4	-1011	D-L-F-H+T+E	133	6.24								-	-	-	-					
UHS Basin Wall (Vt Core)	8	East (interior)	Horizontal	3H.6.9	3H.6.9	44-L	M10M								2406	140+1.3F+1.3M+1.4R	195	-271	D-L-F-H+T+E	116	4.68	-	-
							M10M	2012	140+1.3F+1.3M+1.4R	-379	-110	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	5184	140+1.3F+1.3M+1.4R	1	-466	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	5176	140+1.3F+1.3M+1.4R	-73	-770	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	4252	140+1.3F+1.3M+1.4R	644	132	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	5171	140+1.3F+1.3M+1.4R	-416	1733	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	5171	140+1.3F+1.3M+1.4R	288	2357	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	5171	140+1.3F+1.3M+1.4R	-109	2283	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	4515	140+1.3F+1.3M+1.4R	228	138	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	3897	140+1.3F+1.3M+1.4R	-303	60	D-L-F-H+T+E	116	4.68	-	-	-	-					
							M10M	3842	140+1.3F+1.3M+1.4R	198	1203	D-L-F-H+T+E	116	4.68	-	-	-	-					
							UHS Basin Wall (Vt Core)	8	East (interior)	Horizontal	3H.6.9	3H.6.9	44-L	M10M	3898	140+1.3F+1.3M+1.4R	-73	1233	D-L-F-H+T+E	61	7.8	-	-
M10M	2220	140+1.3F+1.3M+1.4R	668	1136	D-L-F-H+T+E	61								7.8	-	-	-	-					
M10M	2314	140+1.3F+1.3M+1.4R	-271	402	D-L-F-H+T+E	61								7.8	-	-	-	-					
M10M	2320	140+1.3F+1.3M+1.4R	732	1286	D-L-F-H+T+E	61								7.8	-	-	-	-					
M10M	2320	140+1.3F+1.3M+1.4R	-330	1169	D-L-F-H+T+E	61								7.8	-	-	-	-					
M10M	2226	140+1.3F+1.3M+1.4R	921	332	D-L-F-H+T+E	61								7.8	-	-	-	-					
M10M	2183	140+1.3F+1.3M+1.4R	-226	276	D-L-F-H+T+E	61								7.8	-	-	-	-					
M10M	2250	140+1.3F+1.3M+1.4R	183	1221	D-L-F-H+T+E	61								7.8	-	-	-	-					
M10M	2291	140+1.3F+1.3M+1.4R	-116	864	D-L-F-H+T+E	61								7.8	-	-	-	-					

Table 3H.6.7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement Drawing Number ⁽¹⁾	Reinforcement Layer	Zone Number ⁽²⁾	Maximum Forces ⁽³⁾	Element	Longitudinal Reinforcement Design Loads				Transverse Shear Design Loads				Transverse Shear Reinforcement (ft ² /ft)	Results
									Axial and Flexure Loads		In-Plane Shear Loads		Horizontal Section		Vertical Section			
									Load Combination	Axial (k) (kips) (ft)	Moment (k) (kips-ft)	Load Combination	Shear (k) (kips)	Transverse Shear Force (k) (kips)	Compressive Axial Force (k) (kips)	Transverse Shear Force (k) (kips)		
Cooking Tower East Wall (Cont'd)	8	East (outside)	Horizontal	3H.6.113	2-4LL	MTCM	MTCM 229	D+L+FH+T+E	143	-481	D+L+FH+T+E	37	9.36	-	-	-	-	-
							MCCM 231	D+L+FH+T+E	-146	-744	D+L+FH+T+E	37	9.36	-	-	-	-	-
							MMAT 297	D+L+FH+T+E	26	-1246	D+L+FH+T+E	37	9.36	-	-	-	-	-
							MMAC 297	D+L+FH+T+E	-103	-1267	D+L+FH+T+E	37	9.36	-	-	-	-	-
							MTCM 291	D+L+FH+T+E	31	-171	D+L+FH+T+E	118	3.12	-	-	-	-	-
							MCCM 291	D+L+FH+T+E	-115	-74	D+L+FH+T+E	118	3.12	-	-	-	-	-
							MMAT 295	D+L+FH+T+E	7	-195	D+L+FH+T+E	118	3.12	-	-	-	-	-
							MMAC 275	D+L+FH+T+E	-42	-197	D+L+FH+T+E	118	3.12	-	-	-	-	-
							MTCM 289	D+L+FH+T+E	121	-766	D+L+FH+T+E	118	6.24	-	-	-	-	-
							MCCM 225	D+L+FH+T+E	-297	-152	D+L+FH+T+E	118	6.24	-	-	-	-	-
	MTCM 290	D+L+FH+T+E	36	189	D+L+FH+T+E	33	3.12	-	-	-	-	-						
	MCCM 233	D+L+FH+T+E	-42	296	D+L+FH+T+E	33	3.12	-	-	-	-	-						
	MMAT 289	D+L+FH+T+E	3	295	D+L+FH+T+E	33	3.12	-	-	-	-	-						
	MMAC 289	D+L+FH+T+E	-41	295	D+L+FH+T+E	33	3.12	-	-	-	-	-						
	MTCM 239	D+L+FH+T+E	145	343	D+L+FH+T+E	37	9.36	-	-	-	-	-						
	MCCM 231	D+L+FH+T+E	-146	239	D+L+FH+T+E	37	9.36	-	-	-	-	-						
	MMAT 231	D+L+FH+T+E	126	1397	D+L+FH+T+E	118	3.12	-	-	-	-	-						
	MMAC 231	D+L+FH+T+E	-9	1394	D+L+FH+T+E	118	3.12	-	-	-	-	-						
	MTCM 291	D+L+FH+T+E	37	191	D+L+FH+T+E	118	3.12	-	-	-	-	-						
	MCCM 255	D+L+FH+T+E	-120	71	D+L+FH+T+E	118	3.12	-	-	-	-	-						
MMAT 283	D+L+FH+T+E	3	243	D+L+FH+T+E	118	3.12	-	-	-	-	-							
MMAC 275	D+L+FH+T+E	-35	288	D+L+FH+T+E	118	3.12	-	-	-	-	-							
MTCM 289	D+L+FH+T+E	121	486	D+L+FH+T+E	118	3.12	-	-	-	-	-							
MCCM 233	D+L+FH+T+E	-297	309	D+L+FH+T+E	118	3.12	-	-	-	-	-							
MMAT 231	D+L+FH+T+E	6	1173	D+L+FH+T+E	118	3.12	-	-	-	-	-							
MMAC 232	D+L+FH+T+E	-69	1212	D+L+FH+T+E	118	3.12	-	-	-	-	-							
Cooking Tower West Wall	9	West (outside)	Horizontal	3H.6.117	2-4LL	MTCM	MTCM 210	D+L+FH+T+E	133	-280	D+L+FH+T+E	35	7.8	-	-	-	-	
							MCCM 209	D+L+FH+T+E	-172	-296	D+L+FH+T+E	35	7.8	-	-	-	-	-
							MMAT 218	D+L+FH+T+E	10	-1296	D+L+FH+T+E	35	7.8	-	-	-	-	-
							MMAC 206	D+L+FH+T+E	-117	-1306	D+L+FH+T+E	35	7.8	-	-	-	-	-
							MTCM 222	D+L+FH+T+E	35	-173	D+L+FH+T+E	112	3.12	-	-	-	-	-
							MCCM 222	D+L+FH+T+E	-116	-55	D+L+FH+T+E	112	3.12	-	-	-	-	-
							MMAT 214	D+L+FH+T+E	7	-198	D+L+FH+T+E	112	3.12	-	-	-	-	-
							MMAC 206	D+L+FH+T+E	-46	-200	D+L+FH+T+E	112	3.12	-	-	-	-	-
							MTCM 209	D+L+FH+T+E	123	-770	D+L+FH+T+E	112	3.12	-	-	-	-	-
							MCCM 220	D+L+FH+T+E	-265	-148	D+L+FH+T+E	112	3.12	-	-	-	-	-
MMAT 218	D+L+FH+T+E	8	-1083	D+L+FH+T+E	112	3.12	-	-	-	-	-							
MMAC 218	D+L+FH+T+E	-193	-1094	D+L+FH+T+E	112	3.12	-	-	-	-	-							
Cooking Tower East Wall (Cont'd)	8	East (outside)	Horizontal	3H.6.116	2-3	MTCM	MTCM 183	D+L+FH+T+E	42	-266	D+L+FH+T+E	177	125	239	5	660		
							MCCM 226	D+L+FH+T+E	-49	-23	D+L+FH+T+E	177	125	239	5	660		
MTCM 204	D+L+FH+T+E	6	-388	D+L+FH+T+E	177	125	239	5	660									
MMAC 204	D+L+FH+T+E	-49	-391	D+L+FH+T+E	177	125	239	5	660									
MTCM 210	D+L+FH+T+E	133	-280	D+L+FH+T+E	177	125	239	5	660									
MCCM 209	D+L+FH+T+E	-172	-296	D+L+FH+T+E	177	125	239	5	660									
MMAT 218	D+L+FH+T+E	10	-1296	D+L+FH+T+E	177	125	239	5	660									
MMAC 206	D+L+FH+T+E	-117	-1306	D+L+FH+T+E	177	125	239	5	660									
MTCM 222	D+L+FH+T+E	35	-173	D+L+FH+T+E	177	125	239	5	660									
MCCM 222	D+L+FH+T+E	-116	-55	D+L+FH+T+E	177	125	239	5	660									
MMAT 214	D+L+FH+T+E	7	-198	D+L+FH+T+E	177	125	239	5	660									
MMAC 206	D+L+FH+T+E	-46	-200	D+L+FH+T+E	177	125	239	5	660									
MTCM 209	D+L+FH+T+E	123	-770	D+L+FH+T+E	177	125	239	5	660									
MCCM 220	D+L+FH+T+E	-265	-148	D+L+FH+T+E	177	125	239	5	660									
MMAT 218	D+L+FH+T+E	8	-1083	D+L+FH+T+E	177	125	239	5	660									
MMAC 218	D+L+FH+T+E	-193	-1094	D+L+FH+T+E	177	125	239	5	660									

Table 3H.6.7: Results of UHS/RSW Pump House Concrete Wall Design (Continued)

Location	Thickness (ft)	Face	Direction	Reference (1)	Driving Number (1)	Reference Number (1)	Zone Number (1)	Maximum Forces (3)	Eminent	Longitudinal Reinforcement Design Loads				Transverse Shear Design Loads				Transverse Shear Reinforcement (ft ² /ft)	Results																			
										Asilt and Flexure Loads	In-Plane Shear Loads	Longitudinal Reinforcement	Horizontal Section	Vertical Section																								
										Asilt (kips/ft)	Flexure (kips/ft)	In-plane Shear (kips/ft)	Transverse Shear Force (kips/ft)	Compressive Axial Force (kips/ft)	Transverse Shear Force (kips/ft)	Compressive Axial Force (kips/ft)																						
Cooking Tower West Wall (Core)	9	East	Vertical	3H.6.23A	1.4L	MTCM	155	D-L-F-H-T-E	42	228	D-L-F-H-T-E	31	3.12	-	-	-	-	-	-																			
																				MCCM	220	D-L-F-H-T-E	42	228	D-L-F-H-T-E	31	3.12	-	-	-	-	-	-	-	-			
																				MMAT	220	D-L-F-H-T-E	3	299	D-L-F-H-T-E	3	299	-	-	-	-	-	-	-	-	-	-	
																				MMAC	220	D-L-F-H-T-E	-62	209	D-L-F-H-T-E	-62	209	-	-	-	-	-	-	-	-	-	-	-
																				MTCM	210	D-L-F-H-T-E	153	139	D-L-F-H-T-E	153	139	-	-	-	-	-	-	-	-	-	-	-
																				MCCM	29	D-L-F-H-T-E	-172	979	D-L-F-H-T-E	-172	979	-	-	-	-	-	-	-	-	-	-	-
																				MMAT	29	D-L-F-H-T-E	94	1484	D-L-F-H-T-E	94	1484	-	-	-	-	-	-	-	-	-	-	-
																				MMAC	29	D-L-F-H-T-E	-16	1464	D-L-F-H-T-E	-16	1464	-	-	-	-	-	-	-	-	-	-	-
																				MTCM	222	D-L-F-H-T-E	36	164	D-L-F-H-T-E	36	164	-	-	-	-	-	-	-	-	-	-	-
																				MCCM	35	D-L-F-H-T-E	-119	56	D-L-F-H-T-E	-119	56	-	-	-	-	-	-	-	-	-	-	-
																				MMAT	214	D-L-F-H-T-E	3	248	D-L-F-H-T-E	3	248	-	-	-	-	-	-	-	-	-	-	-
																				MMAC	206	D-L-F-H-T-E	-37	260	D-L-F-H-T-E	-37	260	-	-	-	-	-	-	-	-	-	-	-
MTCM	200	D-L-F-H-T-E	120	544	D-L-F-H-T-E	120	544	-	-	-	-	-	-	-	-	-	-	-																				
MCCM	220	D-L-F-H-T-E	-206	421	D-L-F-H-T-E	-206	421	-	-	-	-	-	-	-	-	-	-	-																				
MMAT	29	D-L-F-H-T-E	7	1187	D-L-F-H-T-E	7	1187	-	-	-	-	-	-	-	-	-	-	-																				
MMAC	30	D-L-F-H-T-E	-166	1201	D-L-F-H-T-E	-166	1201	-	-	-	-	-	-	-	-	-	-	-																				
Cooking Tower Inland Wall	2	East and West	Vertical	3H.6.23B	1.4L	MTCM	247	D-L-F-H-T-E	83	-116	D-L-F-H-T-E	30	3.12	-	-	-	-	-	-																			
																				MCCM	1897	D-L-F-H-T-E	-117	-11	D-L-F-H-T-E	30	3.12	-	-	-	-	-	-	-	-			
																				MMAT	247	D-L-F-H-T-E	19	-120	D-L-F-H-T-E	19	-120	-	-	-	-	-	-	-	-	-	-	
																				MMAC	247	D-L-F-H-T-E	-9	-139	D-L-F-H-T-E	-9	-139	-	-	-	-	-	-	-	-	-	-	-
																				MTCM	2033	D-L-F-H-T-E	378	89	D-L-F-H-T-E	378	89	-	-	-	-	-	-	-	-	-	-	-
																				MCCM	2033	D-L-F-H-T-E	-252	-60	D-L-F-H-T-E	-252	-60	-	-	-	-	-	-	-	-	-	-	-
																				MMAT	2450	D-L-F-H-T-E	61	-125	D-L-F-H-T-E	61	-125	-	-	-	-	-	-	-	-	-	-	-
																				MMAC	2426	D-L-F-H-T-E	-4	-125	D-L-F-H-T-E	-4	-125	-	-	-	-	-	-	-	-	-	-	-
																				MTCM	2426	D-L-F-H-T-E	31	23	D-L-F-H-T-E	31	23	-	-	-	-	-	-	-	-	-	-	-
																				MCCM	2426	D-L-F-H-T-E	-67	-20	D-L-F-H-T-E	-67	-20	-	-	-	-	-	-	-	-	-	-	-
																				MMAT	2451	D-L-F-H-T-E	2	-64	D-L-F-H-T-E	2	-64	-	-	-	-	-	-	-	-	-	-	-
																				MMAC	1959	D-L-F-H-T-E	-49	-66	D-L-F-H-T-E	-49	-66	-	-	-	-	-	-	-	-	-	-	-
MTCM	2597	1.65D+1.3L+0.65E+1.3K+1.2T+1.3W	211	2	D-L-F-H-T-E	211	2	-	-	-	-	-	-	-	-	-	-	-																				
MCCM	2033	D-L-F-H-T-E	-220	-54	D-L-F-H-T-E	-220	-54	-	-	-	-	-	-	-	-	-	-	-																				
MMAT	1500	D-L-F-H-T-E	31	-147	D-L-F-H-T-E	31	-147	-	-	-	-	-	-	-	-	-	-	-																				
MMAC	1020	D-L-F-H-T-E	-63	-147	D-L-F-H-T-E	-63	-147	-	-	-	-	-	-	-	-	-	-	-																				
Cooking Tower Inland Wall	2	East and West	Vertical	3H.6.23A	1.4L	MTCM	247	D-L-F-H-T-E	83	-116	D-L-F-H-T-E	30	3.12	-	-	-	-	-	-																			
																				MCCM	1897	D-L-F-H-T-E	-117	-11	D-L-F-H-T-E	30	3.12	-	-	-	-	-	-	-	-			

Notes:

- (1) The reinforcement layout drawings show the various zones used to define the minimum reinforcement that will be provided based on finite element analysis results. Actual provided reinforcement based on final rebar layout may exceed the reported provided reinforcement and the zones with higher reinforcement may be extended beyond their reported boundaries.
- (2) Each reinforcement layout drawing is divided into reinforcement zones. The reinforcement zone naming convention is as follows: "H" = horizontal, "V" = vertical, "T" = transverse reinforcement.
- (3) The maximum tension (MTCM) and compression (MCCM) axial forces are provided with the corresponding moment from the same load combination. The maximum moment that has a corresponding tension (MMAT) in the same load combination and the maximum moment that has a corresponding compression (MMAC) in the same load combination are also provided. For zones where either axial tension or axial compression does not occur for any load combination, dashes are input into the corresponding cell.
- (4) Negative axial load is compression and positive axial load is tension. Negative moment applied tension to the top face of the shell element and positive moment applied tension to the bottom face of the shell element.
- (5) The reported in-plane shear is the maximum average in-plane shear along a plane that crosses the longitudinal reinforcement zone.
- (6) NOT USED.
- (7) The Pump House Operating Floor and Roof slab thickness includes the metal decking (2.5 inches).
- (8) For certain areas of the structure, the standard element post-processing methods were too conservative. For such cases, detailed manual design was performed and the design forces determined by the detailed manual design are provided in the table.
- (9) The transverse reinforcement for the UHS Beam and RSW Pump House Beam is spaced with a maximum center-to-center spacing of 4".

Table 3H.6.8: Results of UHS/RSW Pump House Concrete Slab Design (Continued)

Thickness (ft)	Face	Direction	Reference Layout	Reference Number (ft)	Zone Number (ft)	Reinforcement	Element	Axial and Rebar Loads				In-Block Shear Loads				Longitudinal Reinforcement (ft ²)	Transverse Shear Design Loads				Remarks		
								Load Combination	Area (ft ²)	Rebar (ft ²)	Rebar (ft ²)	Load Combination	In-Block Shear (ft ²)	Rebar (ft ²)	Rebar (ft ²)		Horizontal Section	Vertical Section	Horizontal Section	Vertical Section			
10	Bottom	North-South	3H4-229	344	74L	MCON	13052	1.8D+1.3L+1.3W	315	396	D+1.4F+H+T+E	50	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	405	1192	1.8D+1.3L+1.3W+1.7W	199	37	195	1.8D+1.3L+1.3W+1.7W	31	31	0.31					
	Top	East-West	3H4-229	344	74L	MCON	13056	D+1.4F+H+T+E	49	116	-	-	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	4	44	1.8D+1.3L+1.3W+1.7W	3.81	-	-	-	-	-	-					
	Bottom	North-South	3H4-229	344	74L	MCON	13059	D+1.4F+H+T+E	31	3	D+1.4F+H+T+E	87	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	0	32	1.8D+1.3L+1.3W+1.7W	2.54	-	-	-	-	-	-					
	15	Bottom	East-West	3H4-229	344	74L	MCON	13060	D+1.4F+H+T+E	324	4	D+1.4F+H+T+E	98	-	-	-	-	-	-	-	-	-	-
									1.8D+1.3L+1.3W+1.7W	1	18	1.8D+1.3L+1.3W+1.7W	2.54	-	-	-	-	-	-				
		Top	East-West	3H4-229	344	74L	MCON	13062	D+1.4F+H+T+E	31	5	D+1.4F+H+T+E	87	-	-	-	-	-	-	-	-	-	-
									1.8D+1.3L+1.3W+1.7W	103	25	1.8D+1.3L+1.3W+1.7W	2.54	-	-	-	-	-	-				
Bottom		North-South	3H4-229	344	74L	MCON	13069	D+1.4F+H+T+E	381	399	D+1.4F+H+T+E	187	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	40	443	1.8D+1.3L+1.3W+1.7W	8	-	-	-	-	-	-					
Top		East-West	3H4-229	344	74L	MCON	13107	D+1.4F+H+T+E	37	304	D+1.4F+H+T+E	101	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	402	1407	1.8D+1.3L+1.3W+1.7W	16	-	-	-	-	-	-					
10		Bottom	North-South	3H4-229	344	74L	MCON	13051	1.8D+1.3L+1.3W+1.7W	44	356	D+1.4F+H+T+E	63	-	-	-	-	-	-	-	-	-	-
									1.8D+1.3L+1.3W+1.7W	200	480	1.8D+1.3L+1.3W+1.7W	12	-	-	-	-	-	-				
	Top	East-West	3H4-229	344	74L	MCON	13117	D+1.4F+H+T+E	42	338	D+1.4F+H+T+E	101	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	77	305	1.8D+1.3L+1.3W+1.7W	12	-	-	-	-	-	-					
	Bottom	North-South	3H4-229	344	74L	MCON	13124	D+1.4F+H+T+E	42	338	D+1.4F+H+T+E	101	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	77	305	1.8D+1.3L+1.3W+1.7W	12	-	-	-	-	-	-					
	Top	East-West	3H4-229	344	74L	MCON	13137	D+1.4F+H+T+E	44	356	D+1.4F+H+T+E	104	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	200	480	1.8D+1.3L+1.3W+1.7W	16	-	-	-	-	-	-					
	Bottom	North-South	3H4-229	344	74L	MCON	13147	D+1.4F+H+T+E	47	366	D+1.4F+H+T+E	104	-	-	-	-	-	-	-	-	-	-	
								1.8D+1.3L+1.3W+1.7W	200	480	1.8D+1.3L+1.3W+1.7W	16	-	-	-	-	-	-					
Top	East-West	3H4-229	344	74L	MCON	13155	D+1.4F+H+T+E	47	366	D+1.4F+H+T+E	104	-	-	-	-	-	-	-	-	-	-		
							1.8D+1.3L+1.3W+1.7W	200	480	1.8D+1.3L+1.3W+1.7W	16	-	-	-	-	-	-						
Bottom	North-South	3H4-229	344	74L	MCON	13159	D+1.4F+H+T+E	47	366	D+1.4F+H+T+E	104	-	-	-	-	-	-	-	-	-	-		
							1.8D+1.3L+1.3W+1.7W	200	480	1.8D+1.3L+1.3W+1.7W	16	-	-	-	-	-	-						

Table 3H.6.8: Results of UHS/RSW Pump House Concrete Slab Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement Layout	Reinforcement Zone Number	Maximum Forces	Element	Longitudinal Reinforcement Design Loads				Available Reinforcement (ft ²)	Transverse Shear Reinforcement (ft ²)	Remarks			
								Abut and Return Loads		In-Pipe Shear Loads					Load Combination	Horizontal Section	Vertical Section
								Adj. (d) (kips/ft)	Phys. (d) (kips/ft)	Load Combination	Reinforcement (ft ²)						
								1.4D + 1.7E + 1.3M + 1.4R _s	D + L + F + H + T + E								
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									
								D + L + F + H + T + E									
								1.4D + 1.7E + 1.3M + 1.4R _s									
								D + L + F + H + T + E									
								1.4D + 1.7L + 1.3E + 1.3H + 1.7W									

Table 3H.6.8: Results of UHS/RSW Pump House Concrete Slab Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement Layout	Reinforcement Zone Number	Maximum Forces	Element	Longitudinal Reinforcement Design Loads				Available Reinforcement (ft ²)	Transverse Shear Design Loads				Transverse Shear Reinforcement Ratio (ft ² /ft)	Remarks
								Axial and Rebar Loads		In-Plane Shear Loads			Load Combination	Horizontal Section		Vertical Section		
								Load	Rebar (ft ²)	Load	Rebar (ft ²)			Transverse Shear Force (kips)	Transverse Shear Force (kips)			
								1.4D + 1.7E + 1.3M + 1.7W	600	-109								
								D + L + F + H + T + E	-574	-56	D + L + F + H + T + E	21						
								1.4D + 1.7E + 1.3M + 1.7W	442	-668								
								1.4D + 1.7E + 1.3M + 1.7W	-100	-1191								
								1.4D + 1.7E + 1.3M + 1.7W	899	-105								
								D + L + F + H + T + E	-760	-487	D + L + F + H + T + E	39						
								D + L + F + H + T + E	129	-1589								
								1.4D + 1.7E + 1.3M + 1.7W	-448	-1640								
								1.4D + 1.7E + 1.3M + 1.7W	213	-40								
								D + L + F + H + T + E	-179	-660	D + L + F + H + T + E	51						
								D + L + F + H + T + E	94	-1651								
								D + L + F + H + T + E	-36	-1236								
								D + L + F + H + T + E	250	-523								
								D + L + F + H + T + E	-536	-749								
								D + L + F + H + T + E	93	-1003								
								D + L + F + H + T + E	-109	-1053								
								1.4D + 1.7E + 1.3M + 1.7W	291	-341								
								D + L + F + H + T + E	-304	-760	D + L + F + H + T + E	158						
								1.4D + 1.7E + 1.3M + 1.7W	98	-1600								
								1.4D + 1.7E + 1.3M + 1.7W	-106	-1652								
								1.4D + 1.7E + 1.3M + 1.7W	582	-2087								
								D + L + F + H + T + E	-450	-459	D + L + F + H + T + E	117						
								D + L + F + H + T + E	262	-2662								
								D + L + F + H + T + E	-322	-2726								
								1.4D + 1.7E + 1.3M + 1.7W	404	-2635								
								D + L + F + H + T + E	-475	-724	D + L + F + H + T + E	158						
								1.4D + 1.7E + 1.3M + 1.7W	398	-3394								
								D + L + F + H + T + E	-6	-3063								
								1.4D + 1.7E + 1.3M + 1.7W	696	-4489								
								D + L + F + H + T + E	-522	-512	D + L + F + H + T + E	149						
								1.4D + 1.7E + 1.3M + 1.7W	696	-4489								
								D + L + F + H + T + E	-3	-3689								
								1.4D + 1.7E + 1.3M + 1.7W	274	-1481								
								D + L + F + H + T + E	-158	-271	D + L + F + H + T + E	60						
								1.4D + 1.7E + 1.3M + 1.7W	99	-2375								
								D + L + F + H + T + E	-2	-3520								
								1.4D + 1.7E + 1.3M + 1.7W	267	-1507								
								D + L + F + H + T + E	-191	-231	D + L + F + H + T + E	61						
								1.4D + 1.7E + 1.3M + 1.7W	133	-3670								
								D + L + F + H + T + E	-44	-3336								
								1.4D + 1.7E + 1.3M + 1.7W	324	-454								
								D + L + F + H + T + E	-211	-36	D + L + F + H + T + E	88						
								1.4D + 1.7E + 1.3M + 1.7W	274	-2127								
								D + L + F + H + T + E	-34	-1371								
								1.4D + 1.7E + 1.3M + 1.7W	520	-2782								
								D + L + F + H + T + E	-282	-560	D + L + F + H + T + E	68						
								1.4D + 1.7E + 1.3M + 1.7W	404	-3295								
								D + L + F + H + T + E	-22	-1088								

UHS/RSW Foundation Max (Cont'd)

Table 3H.6.8: Results of UHS/RSW Pump House Concrete Slab Design (Continued)

Location	Thickness (ft)	Face	Direction	Centerline Elevation (ft)	Zone Number	Maximum Forces	Element	Longitudinal Reinforcement Design Loads				Length of Reinforcement Provided (ft)	Transverse Shear Design Loads			Transverse Shear Reinforcement Ratio (ft ² /ft)	Remarks	
								Axial and Flexure Loads		In-Plane Shear Loads			Horizontal Section	Vertical Section	Transverse Shear Zone			
								Load	Flexure (ft-kip)	Load	Flexure (ft-kip)							
								1400 x 12 x 1.25 x 1.25 x 1.25	261	-246								
								D-L-F-H-T+E	-52	-52								
								D-L-F-H-T+E	65	-65								
								D-L-F-H-T+E	-81	-63								
								1400 x 12 x 1.25 x 1.25 x 1.25	342	-182								
								D-L-F-H-T+E	-173	-432								
								D-L-F-H-T+E	95	-2785								
								D-L-F-H-T+E	-8	-2785								
								1400 x 12 x 1.25 x 1.25 x 1.25	447	-474								
								D-L-F-H-T+E	-231	-43								
								D-L-F-H-T+E	161	-2066								
								D-L-F-H-T+E	-30	-2066								
								1400 x 12 x 1.25 x 1.25 x 1.25	687	-5307								
								D-L-F-H-T+E	-203	-371								
								1400 x 12 x 1.25 x 1.25 x 1.25	675	-5331								
								D-L-F-H-T+E	-4	-3687								
								1400 x 12 x 1.25 x 1.25 x 1.25	300	-343								
								D-L-F-H-T+E	-284	-43								
								D-L-F-H-T+E	76	-662								
								D-L-F-H-T+E	-123	-1085								
								1400 x 12 x 1.25 x 1.25 x 1.25	354	-1316								
								D-L-F-H-T+E	-177	-328								
								1400 x 12 x 1.25 x 1.25 x 1.25	227	-3419								
								D-L-F-H-T+E	-4	-3071								
								1400 x 12 x 1.25 x 1.25 x 1.25	724	-5446								
								D-L-F-H-T+E	-207	-559								
								1400 x 12 x 1.25 x 1.25 x 1.25	720	-5596								
								D-L-F-H-T+E	-3	-4321								
								1400 x 12 x 1.25 x 1.25 x 1.25	361	-686								
								D-L-F-H-T+E	-176	-159								
								1400 x 12 x 1.25 x 1.25 x 1.25	260	-1688								
								D-L-F-H-T+E	-21	-1322								
								1400 x 12 x 1.25 x 1.25 x 1.25	308	-479								
								D-L-F-H-T+E	-124	-648								
								1400 x 12 x 1.25 x 1.25 x 1.25	107	-2552								
								D-L-F-H-T+E	0	-2095								
								1400 x 12 x 1.25 x 1.25 x 1.25	524	-2060								
								D-L-F-H-T+E	-592	-1006								
								D-L-F-H-T+E	308	-2346								
								D-L-F-H-T+E	-16	-3017								
								1400 x 12 x 1.25 x 1.25 x 1.25	192	-864								
								D-L-F-H-T+E	-91	-215								
								1400 x 12 x 1.25 x 1.25 x 1.25	71	-1350								
								D-L-F-H-T+E	-3	-1034								
								1400 x 12 x 1.25 x 1.25 x 1.25	171	-342								
								D-L-F-H-T+E	-158	-452								
								1400 x 12 x 1.25 x 1.25 x 1.25	3	-1516								
								D-L-F-H-T+E	-2	-1482								

UHS/RSW Foundation Max (Cont'd)

Table 3H.6.8: Results of UHS/RSW Pump House Concrete Slab Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement Layout	Reinforcement Zone Number	Maximum Forces	Element	Longitudinal Reinforcement Design Loads			Length of Reinforcement Provided (ft) (N)	Transverse Shear Design Loads			Transverse Shear Reinforcement Ratio (ft ²) (ft ²)	Remarks		
								Adm (k) (kips/ft)	Phy (k) (kips/ft)	In-Plane Shear Loads Load Combination		Adm (k) (kips/ft)	Phy (k) (kips/ft)	In-Plane Shear Loads Load Combination			Adm (k) (kips/ft)	Phy (k) (kips/ft)
Top	10	Bottom	North-South	3M/3S	13-14	MOM	1407	MOM	1407	343	2208	D-L-F-H+T+E	104	16	-	-	-	-
							MOM	1407	-340	3102	D-L-F-H+T+E	104	16	-	-	-	-	-
							MOM	1407	238	3438	D-L-F-H+T+E	104	16	-	-	-	-	-
							MOM	1407	-103	3438	D-L-F-H+T+E	104	16	-	-	-	-	-
							MOM	1094	217	464	1.8D+1.3F+1.3M+1.4E	78	12	-	-	-	-	-
							MOM	1094	-173	1025	D-L-F-H+T+E	78	12	-	-	-	-	-
							MOM	1094	99	1091	D-L-F-H+T+E	78	12	-	-	-	-	-
							MOM	1094	-44	1091	D-L-F-H+T+E	78	12	-	-	-	-	-
							MOM	1245	303	1780	D-L-F-H+T+E	77	16	-	-	-	-	-
							MOM	1245	-310	2019	D-L-F-H+T+E	77	16	-	-	-	-	-
							MOM	1245	212	3412	D-L-F-H+T+E	77	16	-	-	-	-	-
							Top	10	Bottom	North-South	3M/3S	15-16	MOM	1091	MOM	1091	192	543
MOM	1091	-121	461	D-L-F-H+T+E	104	6								-	-	-	-	-
MOM	1091	1	1044	D-L-F-H+T+E	104	6								-	-	-	-	-
MOM	1091	-12	1014	1.8D+1.3F+1.3M+1.4E	104	6								-	-	-	-	-
MOM	1151	233	119	D-L-F-H+T+E	60	6								-	-	-	-	-
MOM	1151	-165	115	D-L-F-H+T+E	60	6								-	-	-	-	-
MOM	1151	42	1791	1.8D+1.3F+1.3M+1.4E	60	6								-	-	-	-	-
MOM	1151	-33	1625	D-L-F-H+T+E	60	6								-	-	-	-	-
MOM	1016	933	496	1.8D+1.3F+1.3M+1.4E	21	16								-	-	-	-	-
MOM	1016	-484	223	D-L-F-H+T+E	21	16								-	-	-	-	-
MOM	1016	21	1827	1.8D+1.3F+1.3M+1.4E	21	16								-	-	-	-	-
Top	10	Bottom	North-South	3M/3S	17-18	MOM								4076	MOM	4076	904	1132
							MOM	4076	-740	124	D-L-F-H+T+E	39	16	-	-	-	-	-
							MOM	4076	9	1062	1.8D+1.3F+1.3M+1.4E	39	16	-	-	-	-	-
							MOM	4076	-23	1048	D-L-F-H+T+E	39	16	-	-	-	-	-
							MOM	1986	219	187	1.8D+1.3F+1.3M+1.4E	51	6	-	-	-	-	-
							MOM	1986	-162	222	D-L-F-H+T+E	51	6	-	-	-	-	-
							MOM	1458	23	1784	1.8D+1.3F+1.3M+1.4E	117	8	-	-	-	-	-
							MOM	1458	-36	1723	D-L-F-H+T+E	117	8	-	-	-	-	-
							MOM	1057	256	38	1.8D+1.3F+1.3M+1.4E	154	12	-	-	-	-	-
							MOM	1057	-354	1522	D-L-F-H+T+E	154	12	-	-	-	-	-
							MOM	1211	28	1099	1.8D+1.3F+1.3M+1.4E	154	12	-	-	-	-	-
							Top	10	Bottom	North-South	3M/3S	19-20	MOM	1324	MOM	1324	101	2223
MOM	1324	-33	2223	D-L-F-H+T+E	154	12								-	-	-	-	-
MOM	1373	202	415	1.8D+1.3F+1.3M+1.4E	96	6								-	-	-	-	-
MOM	1373	-95	537	D-L-F-H+T+E	96	6								-	-	-	-	-
MOM	1328	1	1491	1.8D+1.3F+1.3M+1.4E	96	6								-	-	-	-	-
MOM	1328	-9	1488	D-L-F-H+T+E	96	6								-	-	-	-	-
MOM	1096	304	751	1.8D+1.3F+1.3M+1.4E	88	12								-	-	-	-	-
MOM	1096	-389	1047	D-L-F-H+T+E	88	12								-	-	-	-	-
MOM	1096	68	3289	D-L-F-H+T+E	88	12								-	-	-	-	-
MOM	1096	-28	3289	D-L-F-H+T+E	88	12								-	-	-	-	-

Table 3H.6-8: Results of UHS/RSW Pump House Concrete Slab Design (Continued)

Location	Thickness (ft)	Face	Direction	Reinforcement Layout	Reinforcement Drawing Number (1)	Zone Number (2)	Maximum Forces (3)	Element	Longitudinal Reinforcement Design Loads			Longitudinal Reinforcement Provided (ft ² /ft)	Transverse Shear Design Loads			Transverse Shear Reinforcement Provided (ft ² /ft)	Remarks
									Actual (4) Load Combination	Placed (4) (ft ² /ft)	In-Plane Shear Loads Load Combination		Horizontal Section Transverse Shear Zone (ft ² /ft)	Vertical Section Transverse Shear Zone (ft ² /ft)	Vertical Section Transverse Shear Zone (ft ² /ft)		

- Notes:**
- (1) The reinforcement layout drawings show the minimum reinforcement that will be provided based on finite element analysis results. Actual provided reinforcement based on final layout may exceed the reported provided reinforcement and the zones with higher reinforcement may be extended beyond their reported boundaries.
 - (2) Each reinforcement layout drawing is divided into reinforcement zones. The reinforcement zone naming convention is as follows: "T" = horizontal, "V" = vertical, "X" = longitudinal reinforcement, "T" = transverse reinforcement.
 - (3) The maximum tension (MCTM) and compression (MCCM) load forces are provided with the corresponding moment from the same load combination. The maximum moment that has a corresponding tension (MMAT) in the same load combination and the maximum moment that has a corresponding compression (MMC) in the same load combination are also provided. For zones where either axial tension or axial compression does not occur for any load combination, dashes are input for the corresponding cell.
 - (4) Negative axial load is compression and positive axial load is tension. Negative moment applies tension to the top face of the slab element and positive moment applies tension to the bottom face of the slab element.
 - (5) The reported in-plane shear is the maximum average in-plane shear along a plane that crosses the longitudinal reinforcement zone.
 - (6) NOT USED.
 - (7) The Pump House Operating Floor and Roof slab thickness includes the steel decking (2.5 inches).
 - (8) For certain areas of the structure, the standard element post-processing method were too conservative. For such cases, detailed manual design was performed and the design forces determined by the detailed manual design are provided in the table.
 - (9) The transverse reinforcement for the UHS Basin and RSW Pump House Basins is spaced with a maximum center-to-center spacing of 4".

Table 3H.6-9: Results of UHS/RSW Pump House Beams and Columns Design

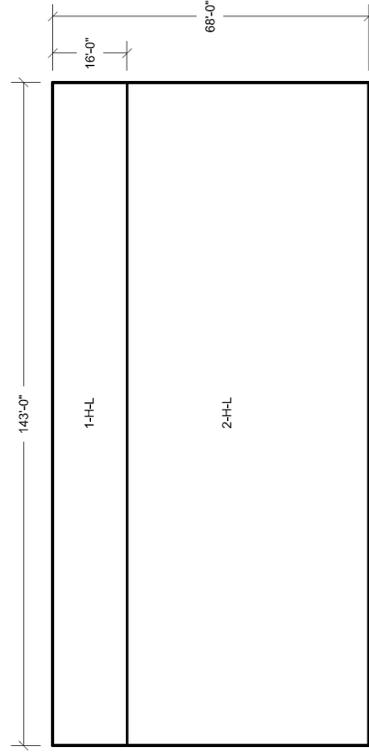
Location	Item	Critical Element Number	Load Combination	Maximum Forces	Design Loads						Reinforcement		Remarks		
					Axial (kips)		Moments (ft-kips)		Shear (kips)		Longitudinal Provided (in ²)	Transverse			
					P	MZ	M3	Torsion	V2	V3		Provided X-direction		Provided Y-direction	
UHS Basin	5' x 5' Columns	516	1.4D+1.7L+1.7F+1.7H+1.7W	Maximum axial compression with corresponding forces	-3587	-1473	904	-	-	-	-	148.5	7#5 @ 4" O.C	7#5 @ 4" O.C	Local Axis definition: 1 = vertical 2 = east-west 3 = north-south Transverse reinforcement includes one closed loop which accounts for two legs in each direction.
		487	D+LoF+H+To+E	Maximum axial tension with corresponding forces	348	1148	465	-	-	-	-	148.5	7#5 @ 4" O.C	7#5 @ 4" O.C	
		510	D+LoF+H+To+E	Maximum M2 moment with corresponding forces	-1066	-9127	1990	-	-	-	-	148.5	7#5 @ 4" O.C	7#5 @ 4" O.C	
		506	D+LoF+H+To+E	Maximum M3 moment with corresponding forces	-630	834	7298	-	-	-	-	148.5	7#5 @ 4" O.C	7#5 @ 4" O.C	
		505	D+LoF+H+To+E	Maximum V2	-	-	-	-	212	-	-	148.5	7#5 @ 4" O.C	7#5 @ 4" O.C	
	5' x 12' Columns	510	D+LoF+H+To+E	Maximum V3	-	-	-	-	-	-278	-	148.5	7#5 @ 4" O.C	7#5 @ 4" O.C	Local Axis definition: 1 = vertical 2 = east-west 3 = north-south Transverse reinforcement includes one closed loop which accounts for two legs in each direction.
		505	D+LoF+H+To+E	Maximum Torsion	-	-	-	-652	-	-	148.5	7#5 @ 4" O.C	7#5 @ 4" O.C		
		518	1.4D+1.7L+1.7F+1.7H+1.7W	Maximum axial compression with corresponding forces	-4746	-2484	822	-	-	-	-	175.5	13#5 @ 4" O.C	7#5 @ 4" O.C	
		487	D+LoF+H+To+E	Maximum axial tension with corresponding forces	645	2639	2900	-	-	-	-	175.5	13#5 @ 4" O.C	7#5 @ 4" O.C	
		496	D+LoF+H+To+E	Maximum M2 moment with corresponding forces	-2509	-10456	-1048	-	-	-	-	175.5	13#5 @ 4" O.C	7#5 @ 4" O.C	
4' x 4' Beams	5' x 12' Columns	518	D+LoF+H+To+E	Maximum M3 moment with corresponding forces	-3435	3346	30990	-	-	-	-	175.5	13#5 @ 4" O.C	7#5 @ 4" O.C	Local Axis definition: 1 = vertical 2 = east-west 3 = north-south Transverse reinforcement includes one closed loop which accounts for two legs in each direction.
		518	D+LoF+H+To+E	Maximum V2	-	-	-	-	453	-	-	175.5	13#5 @ 4" O.C	7#5 @ 4" O.C	
		496	D+LoF+H+To+E	Maximum V3	-	-	-	-	-	-388	-	175.5	13#5 @ 4" O.C	7#5 @ 4" O.C	
		487	D+LoF+H+To+E	Maximum Torsion	-	-	-	-980	-	-	175.5	13#5 @ 4" O.C	7#5 @ 4" O.C		
		16	D+LoF+H+To+E	Maximum axial compression with corresponding forces	-3313	-2668	-3215	-	-	-	-	155.16	8#5 @ 4" O.C	6#5 @ 4" O.C	
	4' x 4' Beams	16	D+LoF+H+To+E	Maximum axial tension with corresponding forces	5159	1054	2155	-	-	-	-	155.16	8#5 @ 4" O.C	6#5 @ 4" O.C	Local Axis definition: 1 = vertical 2 = east-west 3 = north-south Transverse reinforcement includes one closed loop which accounts for two legs in each direction.
		36	D+LoF+H+To+E	Maximum M2 moment with corresponding forces	647	-6596	44	-	-	-	-	155.16	8#5 @ 4" O.C	6#5 @ 4" O.C	
		16	D+LoF+H+To+E	Maximum M3 moment with corresponding forces	-1848	2332	6466	-	-	-	-	155.16	8#5 @ 4" O.C	6#5 @ 4" O.C	
		16	D+LoF+H+To+E	Maximum V2	-	-	-	-	693	-	-	155.16	8#5 @ 4" O.C	6#5 @ 4" O.C	
		36	D+LoF+H+To+E	Maximum V3	-	-	-	-	-	798	-	155.16	8#5 @ 4" O.C	6#5 @ 4" O.C	
403	D+LoF+H+To+E	Maximum Torsion	-	-	-	698	-	-	-	155.16	8#5 @ 4" O.C	6#5 @ 4" O.C			

Table 3H.7-1: Results of DGFOT Concrete Design (Continued)

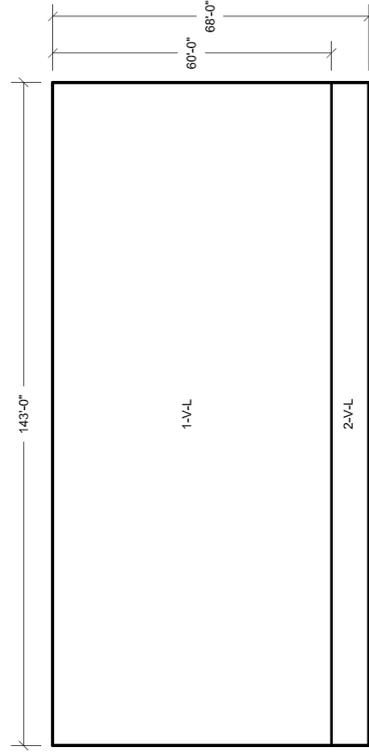
Location	Thickness (ft)	Face	Direction	Element Label	Element Zone Number (2)	Main Forces (3)	Element	Longitudinal Reinforcement Design Loads				Transverse Shear Design Forces				Remarks				
								Axial and Flexure Loads		In-Phase Shear Loads		Longitudinal Reinforcement (kSF/ft)	Load Combination	Horizontal Section	Vertical Section		Transverse Shear Reinforcement Provided (kSF/ft)			
								Loads (1)	Combination	Axis (4)	Flange (4)							Reinforce (5)	Reinforce (5)	
Basement	2	N/W Side	Horizontal	3H7-15	14L	-	-	-	D + L + H + W	See Note (10)	27	3.12	-	-	-					
																Max. Tension w/ corresponding moment	2624	8		
																Max. Compression w/ corresponding moment	309	-17		
																Max. Moment with axial tension	2351	12	21	
																Max. Moment with axial compression	2316	-13	32	
																Max. Tension w/ corresponding moment	2425	20	-40	
																Max. Compression w/ corresponding moment	301	-23	0	
																Max. Moment with axial tension	2433	16	74	
																Max. Moment with axial compression	2624	-2	-72	
																Max. Tension w/ corresponding moment	2315	13	2	
																Max. Compression w/ corresponding moment	309	-35	79	
																Max. Moment with axial tension	2439	1	56	
																Max. Moment with axial compression	2496	-9	87	
																Max. Tension w/ corresponding moment	174	104	-11	
Max. Compression w/ corresponding moment	1703	-117	4																	
Max. Moment with axial tension	1698	53	-38																	
Max. Moment with axial compression	1694	-9	-30																	
Max. Tension w/ corresponding moment	1710	119	7																	
Max. Compression w/ corresponding moment	1703	-117	12																	
Max. Moment with axial tension	1695	10	41																	
Max. Moment with axial compression	1640	-10	53																	
Max. Tension w/ corresponding moment	1684	17	-20																	
Max. Compression w/ corresponding moment	1694	-28	-45																	
Max. Moment with compressive axial tension	1710	0	-65																	
Max. Moment with corresponding axial compression	174	-44	-70																	
Max. Tension w/ corresponding moment	1684	16	9																	
Max. Compression w/ corresponding moment	1636	-23	6																	
Max. Moment with corresponding axial tension	209	2	54																	
Max. Moment with corresponding axial compression	209	-5	54																	
Roof of Tunnel	2	N/W Side	Horizontal	3H7-16	14L	-	-	-	D + L + H + W	-	-	-	-	-	-					
																Max. Tension w/ corresponding moment	174	104	-11	
																Max. Compression w/ corresponding moment	1703	-117	4	
																Max. Moment with axial tension	1698	53	-38	
																Max. Moment with axial compression	1694	-9	-30	
																Max. Tension w/ corresponding moment	1710	119	7	
																Max. Compression w/ corresponding moment	1703	-117	12	
																Max. Moment with axial tension	1695	10	41	
																Max. Moment with axial compression	1640	-10	53	
																Max. Tension w/ corresponding moment	1684	17	-20	
																Max. Compression w/ corresponding moment	1694	-28	-45	
																Max. Moment with compressive axial tension	1710	0	-65	
																Max. Moment with corresponding axial compression	174	-44	-70	
																Max. Tension w/ corresponding moment	1684	16	9	
Max. Compression w/ corresponding moment	1636	-23	6																	
Max. Moment with corresponding axial tension	209	2	54																	
Max. Moment with corresponding axial compression	209	-5	54																	
Roof of Tunnel	2	S/E Side	Vertical	3H7-19	14L	-	-	-	D + L + H + W	-	-	-	-	-	-					
																Max. Tension w/ corresponding moment	174	104	-11	
																Max. Compression w/ corresponding moment	1703	-117	4	
																Max. Moment with axial tension	1698	53	-38	
																Max. Moment with axial compression	1694	-9	-30	
																Max. Tension w/ corresponding moment	1710	119	7	
																Max. Compression w/ corresponding moment	1703	-117	12	
																Max. Moment with axial tension	1695	10	41	
																Max. Moment with axial compression	1640	-10	53	
																Max. Tension w/ corresponding moment	1684	17	-20	
																Max. Compression w/ corresponding moment	1694	-28	-45	
																Max. Moment with compressive axial tension	1710	0	-65	
																Max. Moment with corresponding axial compression	174	-44	-70	
																Max. Tension w/ corresponding moment	1684	16	9	
Max. Compression w/ corresponding moment	1636	-23	6																	
Max. Moment with corresponding axial tension	209	2	54																	
Max. Moment with corresponding axial compression	209	-5	54																	
Roof of Tunnel	2	S/E Side	Vertical	3H7-20	14L	-	-	-	D + L + H + W	-	-	-	-	-	-					
																Max. Tension w/ corresponding moment	174	104	-11	
																Max. Compression w/ corresponding moment	1703	-117	4	
																Max. Moment with axial tension	1698	53	-38	
																Max. Moment with axial compression	1694	-9	-30	
																Max. Tension w/ corresponding moment	1710	119	7	
																Max. Compression w/ corresponding moment	1703	-117	12	
																Max. Moment with axial tension	1695	10	41	
																Max. Moment with axial compression	1640	-10	53	
																Max. Tension w/ corresponding moment	1684	17	-20	
																Max. Compression w/ corresponding moment	1694	-28	-45	
																Max. Moment with compressive axial tension	1710	0	-65	
																Max. Moment with corresponding axial compression	174	-44	-70	
																Max. Tension w/ corresponding moment	1684	16	9	
Max. Compression w/ corresponding moment	1636	-23	6																	
Max. Moment with corresponding axial tension	209	2	54																	
Max. Moment with corresponding axial compression	209	-5	54																	

Notes:

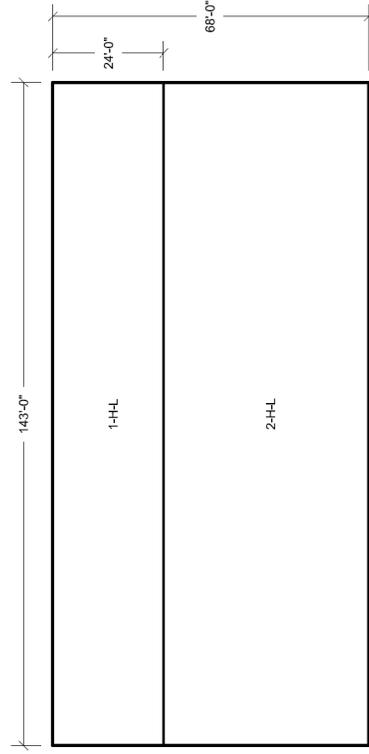
- (1) The reinforcement layout drawings show the various zones used to define the minimum reinforcement that will be provided based on finite element analysis results. Actual provided reinforcement based on final rebar layout and industry development length may exceed the reported provided reinforcement and the zones with higher reinforcement may be extended beyond their reported boundaries. The dimensions in the reinforcement drawings are based on the dimensions of the SAP2000 shell elements, which are modeled at the centerline of the walls and slabs.
- (2) Each reinforcement layout drawing is divided into reinforcement zones. The reinforcement zone naming convention is as follows: "H" = horizontal, "V" = vertical, "L" = longitudinal reinforcement, "T" = transverse reinforcement. For slabs, vertical corresponds to Y-axis and horizontal corresponds to X-axis as shown on Figure 3H.7.1.
- (3) The maximum tension and compression axial forces are provided with the corresponding moment from the same load combination. The maximum moment that has a corresponding tension in the same load combination and the maximum moment that has a corresponding compression in the same load combination are also provided.
- (4) Negative axial loads in compression and positive axial loads in tension. Negative moment applies tension to the top face of the shell element and positive moment applies tension to the bottom face of the shell element. For walls or slabs where the same reinforcement is provided on both faces, the moment is shown as absolute values.
- (5) The reported in-plane shear is the maximum average in-plane shear along a plane that crosses the longitudinal reinforcement zone.
- (6) Not used.
- (7) In areas where horizontal and vertical transverse shear zones overlap, the total transverse shear reinforcement to be supplied in the overlapping area is the sum of the transverse reinforcement required from the horizontal and vertical zones.
- (8) Openings in the Access Regions have not been included in the Reinforcement Layout Drawings.
- (9) The Access Region is governed by the torsional load combination. The outside layer of transverse torsional reinforcement (i.e. 4 near side horizontal) reinforcement is utilized to resist a torsional moment of 1438 kip-ft due to an eccentric torsional moment load. The far side horizontal reinforcement is utilized to resist an axial force of 805 kip due to a concrete torsional moment load as well as a torsional wind pressure of 294 psf. The remaining capacity of the near side vertical longitudinal reinforcement in conjunction with the far side vertical longitudinal reinforcement are utilized to resist a moment of 10076 kip-ft due to a torsional load combination.
- (10) The basement near side horizontal reinforcement is governed by the torsional load combination. The outside layer of transverse torsional reinforcement is composed of near side vertical reinforcement (three walls in Z-section and four and basement in Y-section) in conjunction with the near side horizontal reinforcement (2 tunnel walls, roof and basement in X-section) are utilized to resist a torsional moment of 6985 kip-ft due to torsional load combination.
- (11) The "E" (WPT) designation in the load combination column indicates seismic SSE loading including wave propagation effects.



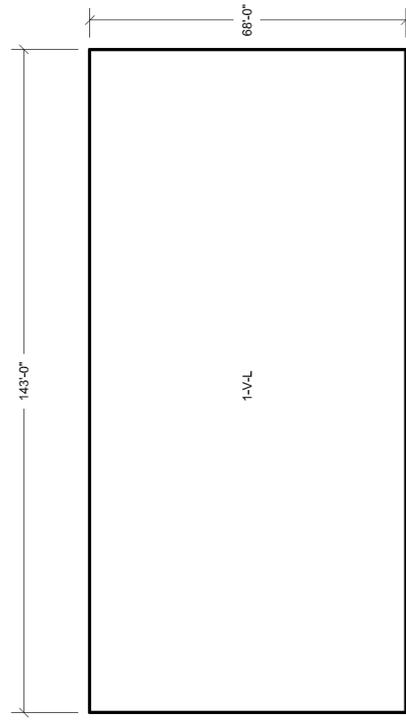
**FIGURE 3H.6.6.1 PUMP HOUSE NORTH WALL LOCKING SOUTH
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE**



**FIGURE 3H.6.52- PUMP HOUSE NORTH WALL LOOKING SOUTH
VERTICAL REBAR PLACEMENT ZONES
NEAR SIDE FACE**



**FIGURE 3H.63H PUMP HOUSE NORTH WALL LOCKING SOUTH
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE**



**FIGURE 3H16.54 PUMP HOUSE NORTH WALL LOOKING SOUTH
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE**

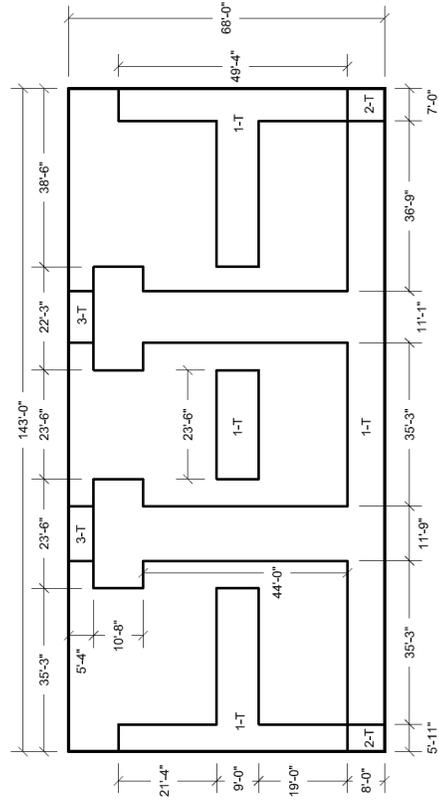
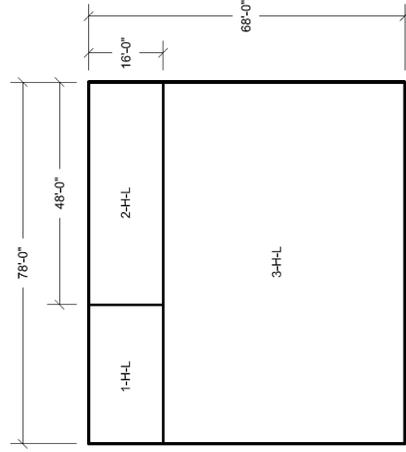


FIGURE 3H16.55: PUMP HOUSE NORTH WALL LOOKING SOUTH
TRANSVERSE REINFORCEMENT ZONES



**FIGURE 216.56: PUMP HOUSE EAST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE**

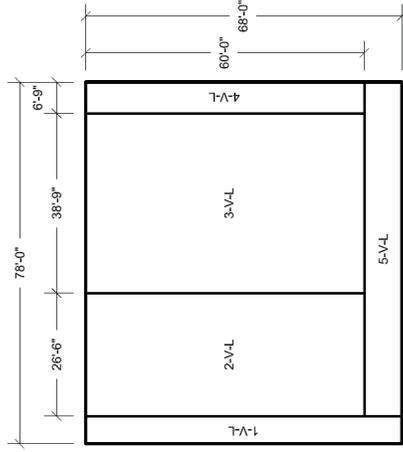


FIGURE 31.6.57: PUMP HOUSE EAST WALL LOOKING WEST
VERTICAL SEGMENT ZONES
NEAR SIDE FACE

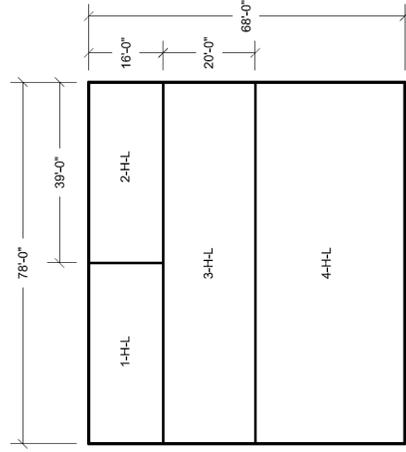


FIGURE 3H.6-58: PUMP HOUSE EAST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE

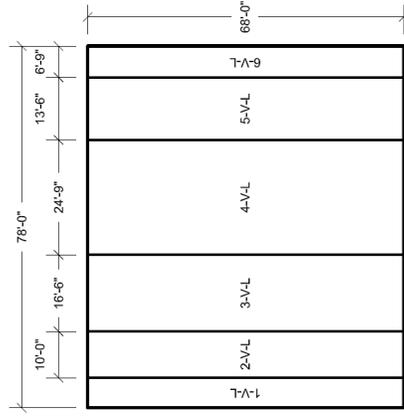


FIGURE 3H.6-59: PUMP HOUSE EAST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE

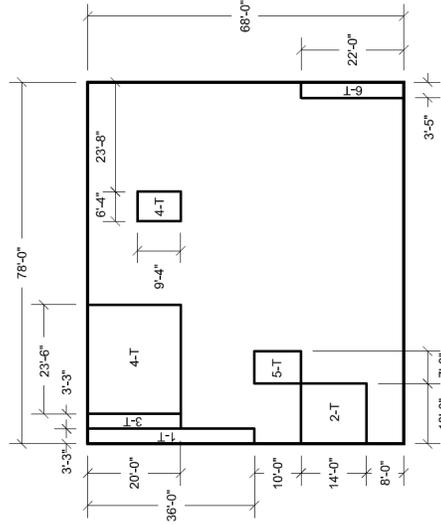


FIGURE 3H.6-60: PUMP HOUSE EAST WALL LOOKING WEST
TRANSVERSE REINFORCEMENT ZONES

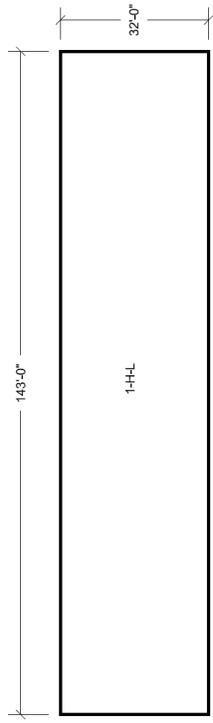


FIGURE 3H.6-61: PUMP HOUSE SOUTH WALL LOOKING SOUTH
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE

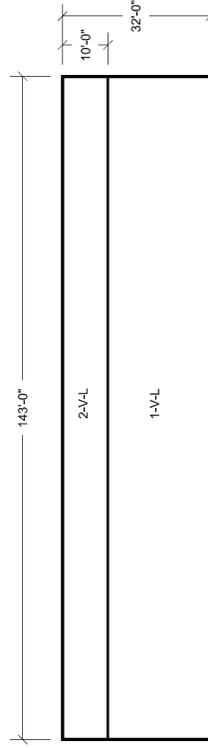


FIGURE 3H.6-6Z: PUMP HOUSE SOUTH WALL LOOKING SOUTH
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

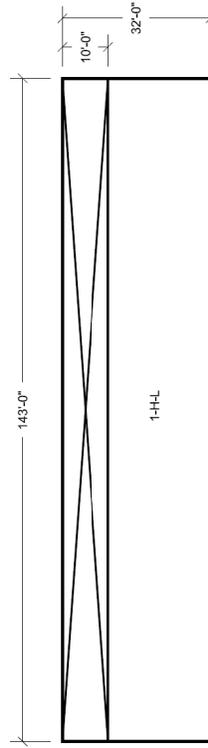


FIGURE 3-H-6-63: PUMP HOUSE SOUTH WALL LOOKING SOUTH
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE

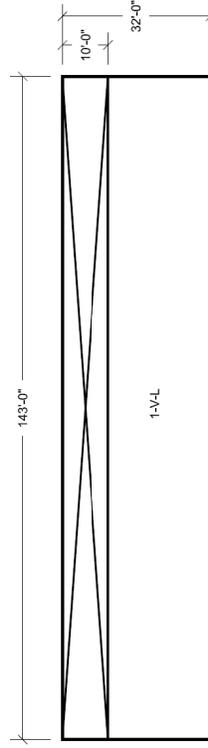


FIGURE 3H.6-64: PUMP HOUSE SOUTH WALL LOOKING SOUTH
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE

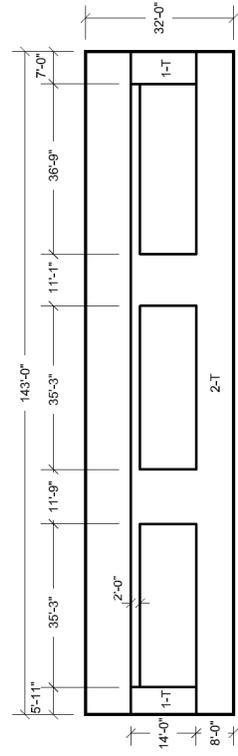


FIGURE 3H.6-555: PUMP HOUSE SOUTH WALL LOOKING SOUTH
TRANSVERSE REINFORCEMENT ZONES

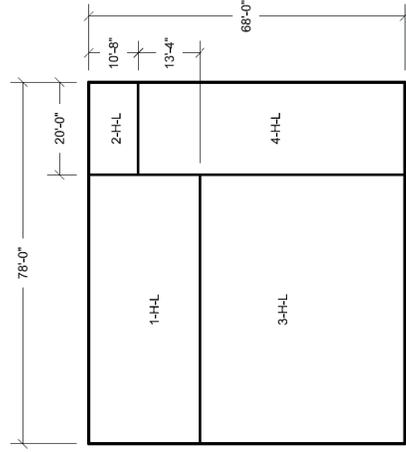
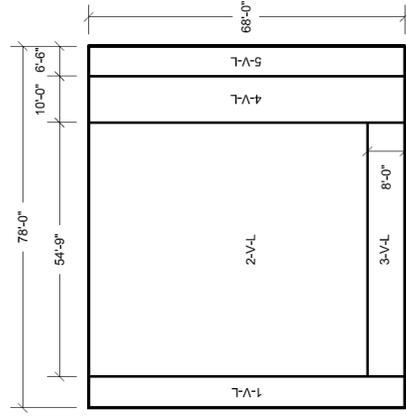
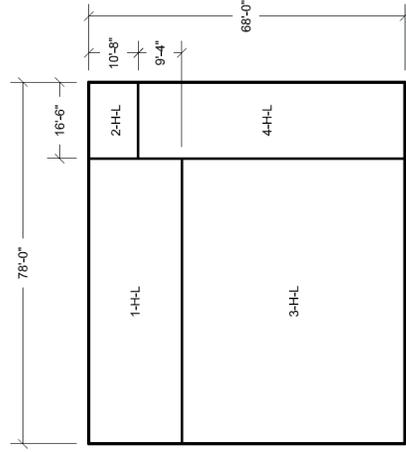


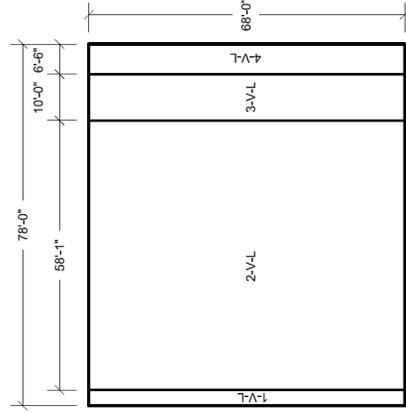
FIGURE 3H.6-66: PUMP HOUSE WEST WALL LOOKING EAST
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE



**FIGURE 3H.6-57: PUMP HOUSE WEST WALL LOOKING EAST
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE**



**FIGURE 3H.6-68: PUMP HOUSE WEST WALL LOOKING EAST
HORIZONTAL REINFORCEMENT ZONES,
EAST SIDE FACE**



**FIGURE 3H.6-69: PUMP HOUSE WEST WALL LOOKING EAST
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE**

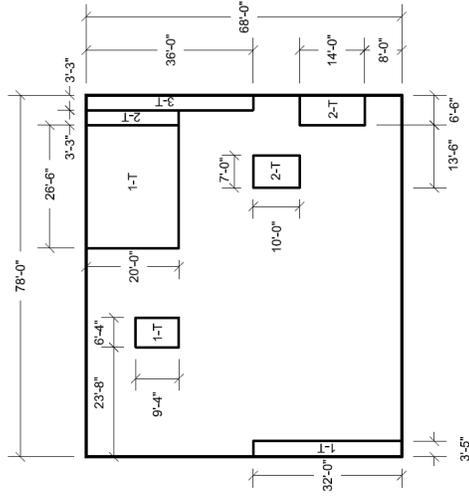


FIGURE 3H.6-70: PUMP HOUSE WEST WALL LOOKING EAST
TRANSVERSE REINFORCEMENT ZONES

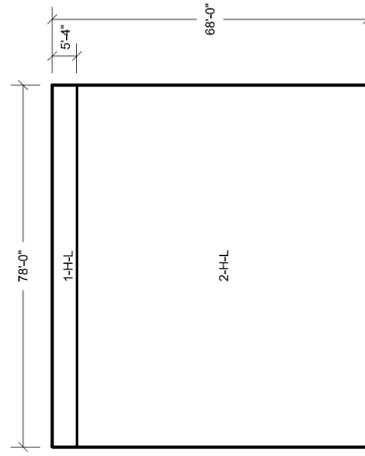


FIGURE 3H.6-71: PUMP HOUSE INTERNAL EAST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE

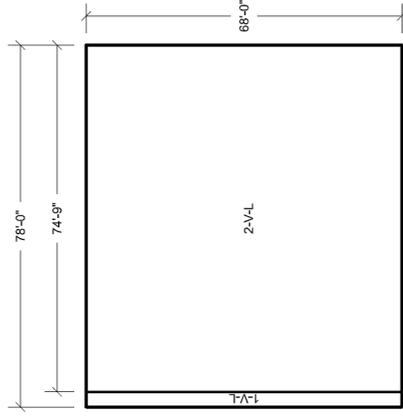


FIGURE 3H.6-72: PUMP HOUSE INTERNAL EAST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

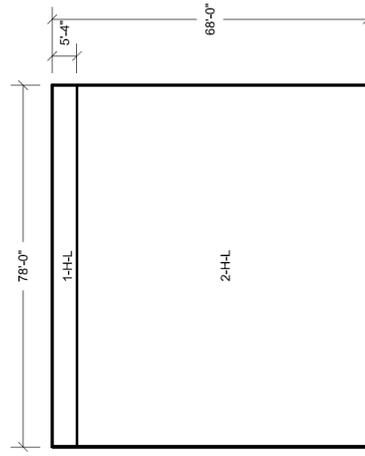


FIGURE 3H.6-73: PUMP HOUSE INTERNAL EAST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE

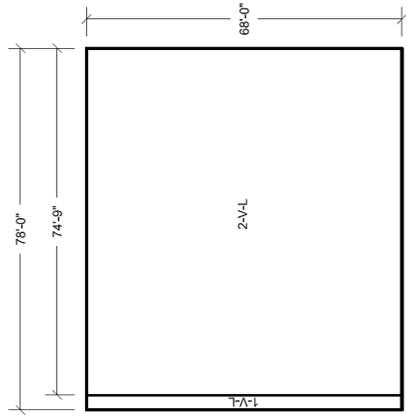


FIGURE 3H.6-74: PUMP HOUSE INTERNAL EAST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE

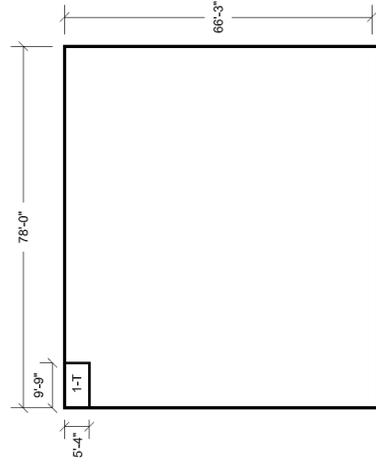


FIGURE 3H.6.74A: PUMP HOUSE INTERNAL EAST WALL LOOKING WEST
TRANSVERSE REINFORCEMENT ZONES

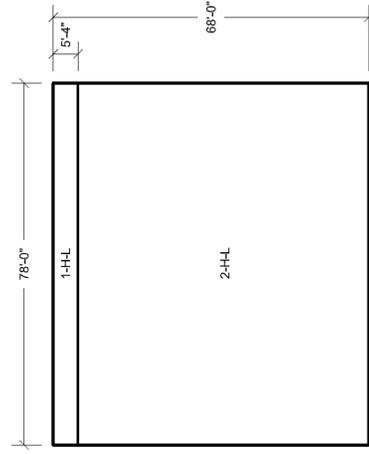


FIGURE 3H.6-5: PUMP HOUSE INTERNAL WEST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE

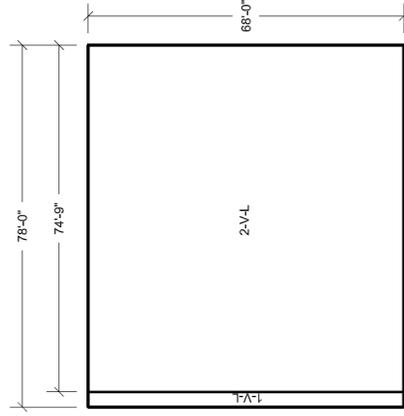


FIGURE 3H.6-76: PUMP HOUSE INTERNAL WEST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

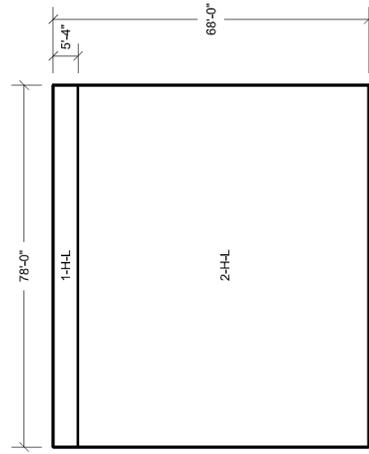


FIGURE 3H.6-77: PUMP HOUSE INTERNAL WEST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE

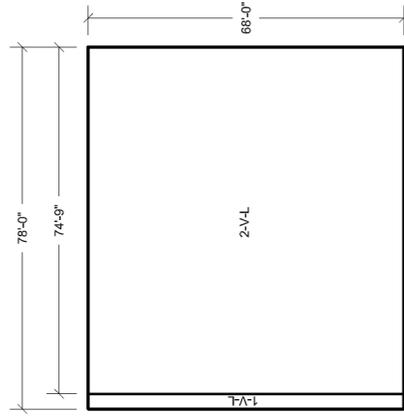


FIGURE 3H.6-78: PUMP HOUSE INTERNAL WEST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE

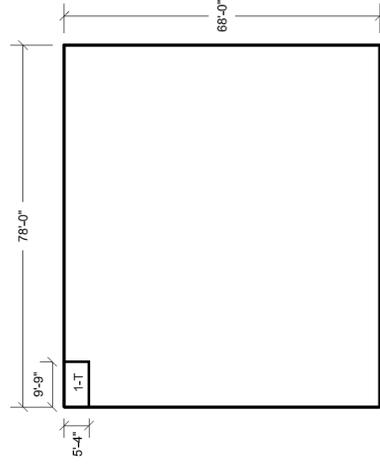


FIGURE 3H.6-78A: PUMP HOUSE INTERNAL WEST WALL LOOKING WEST
TRANSVERSE REINFORCEMENT ZONES

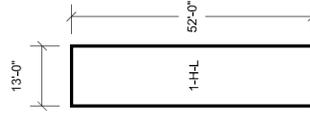


FIGURE 3H.6.79: PUMP HOUSE EAST BUTTRESS LOOKING NORTH & PUMP HOUSE WEST BUTTRESS LOOKING SOUTH
HORIZONTAL REINFORCEMENT ZONES
NEAR AND FAR SIDE FACES

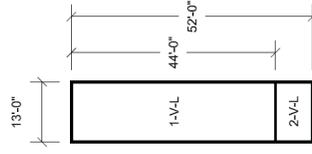


FIGURE 3H.6-80: PUMP HOUSE EAST BUTTRESS LOOKING NORTH & PUMP HOUSE WEST BUTTRESS LOOKING SOUTH
VERTICAL REINFORCEMENT ZONES
NEAR AND FAR SIDE FACES



FIGURE 3H.6-81: PUMP HOUSE EAST BUTTRESS LOOKING NORTH & PUMP HOUSE WEST BUTTRESS LOOKING SOUTH
TRANSVERSE REINFORCEMENT ZONES

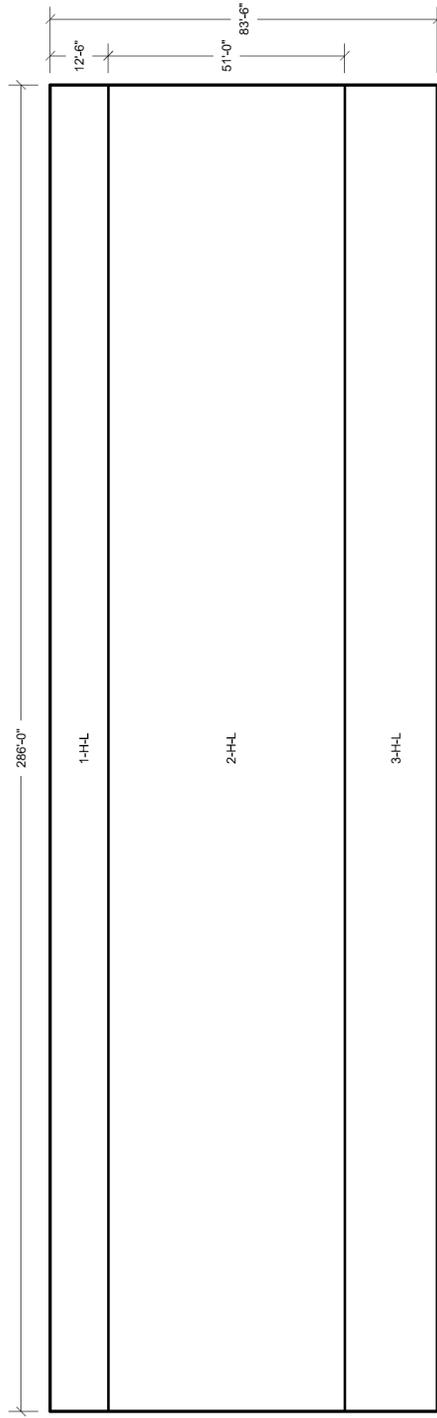


FIGURE 3H.6-32: UHS BASIN NORTH WALL LOOKING SOUTH
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE

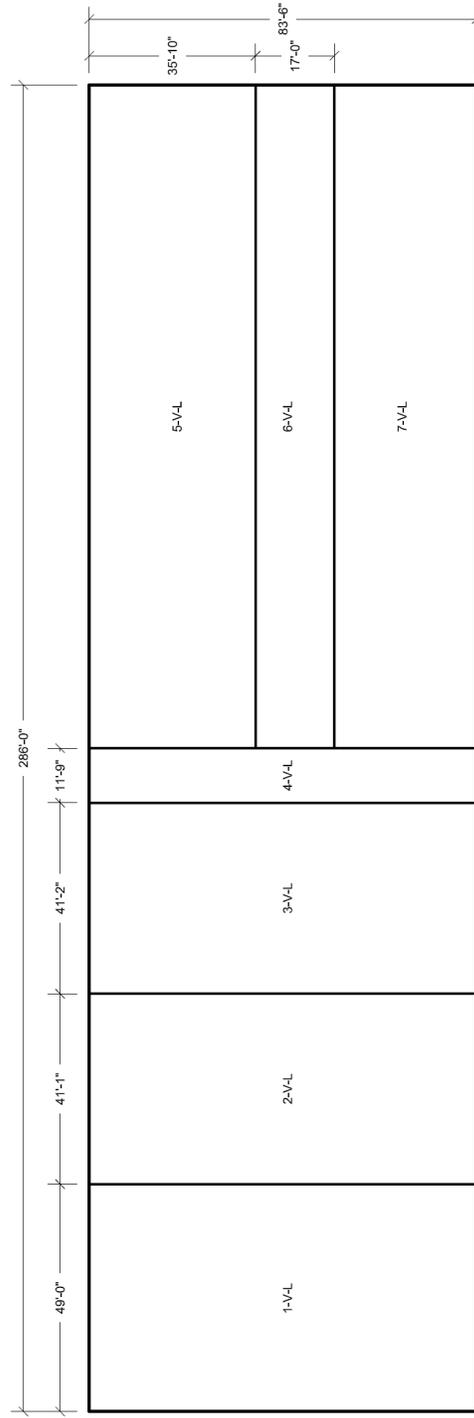


FIGURE 3H.6-83: UHS BASIN NORTH WALL LOOKING SOUTH
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

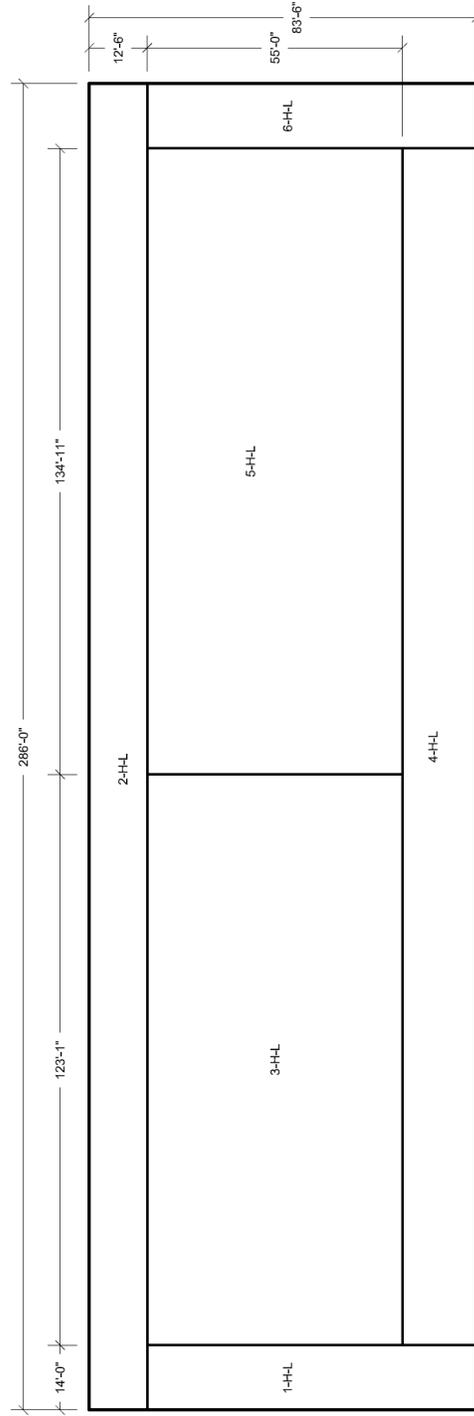


FIGURE 3H.6-34: UHS BASIN NORTH WALL LOOKING SOUTH
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE

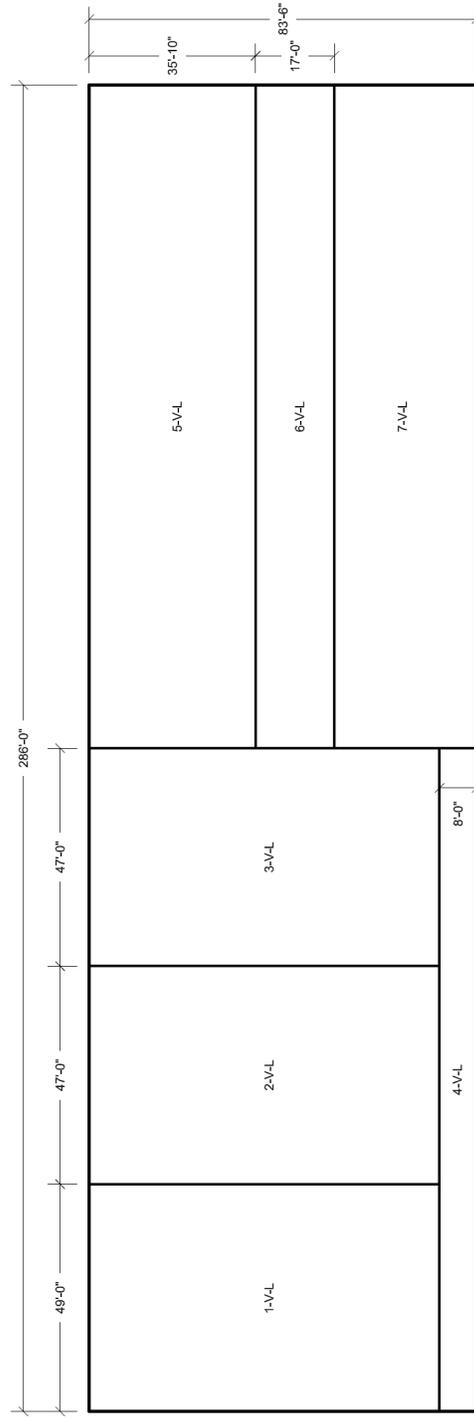
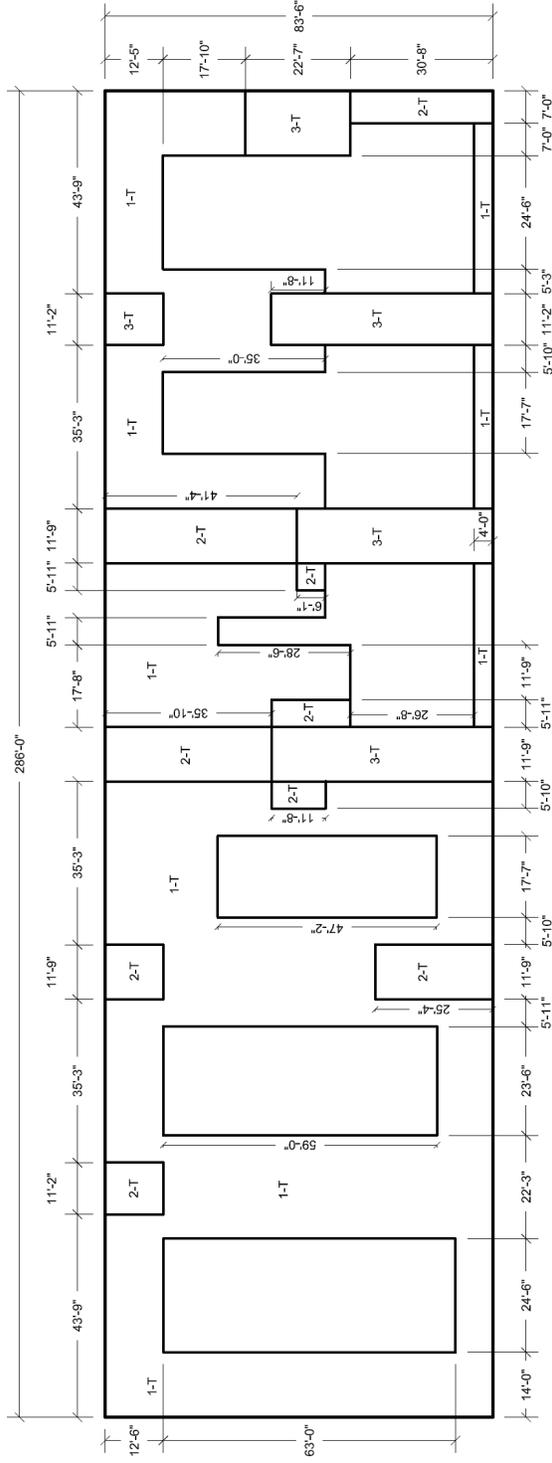


FIGURE 3H.6-35: UHS BASIN NORTH WALL LOOKING SOUTH
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE



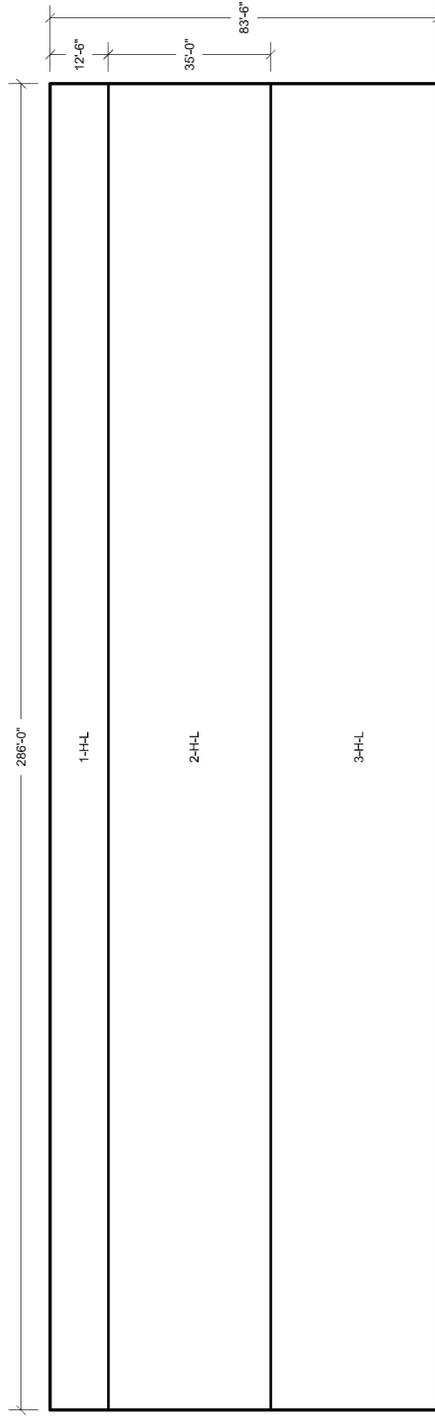


FIGURE 3H.6-87: UHS BASIN SOUTH WALL LOOKING NORTH
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE

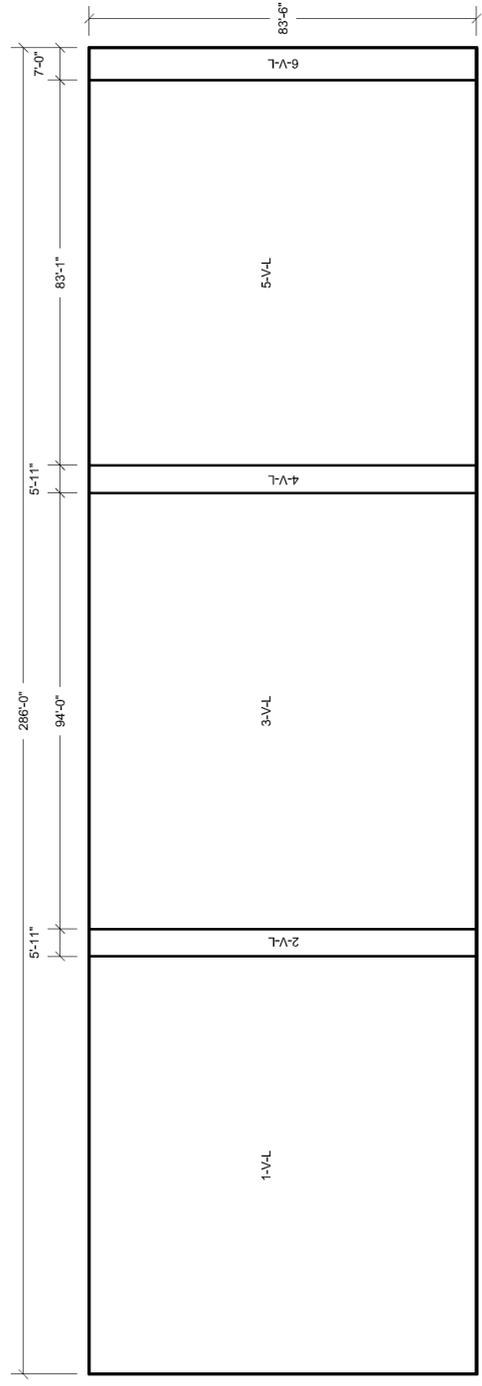


FIGURE 3H.6-38: UHS BASIN SOUTH WALL LOOKING NORTH
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

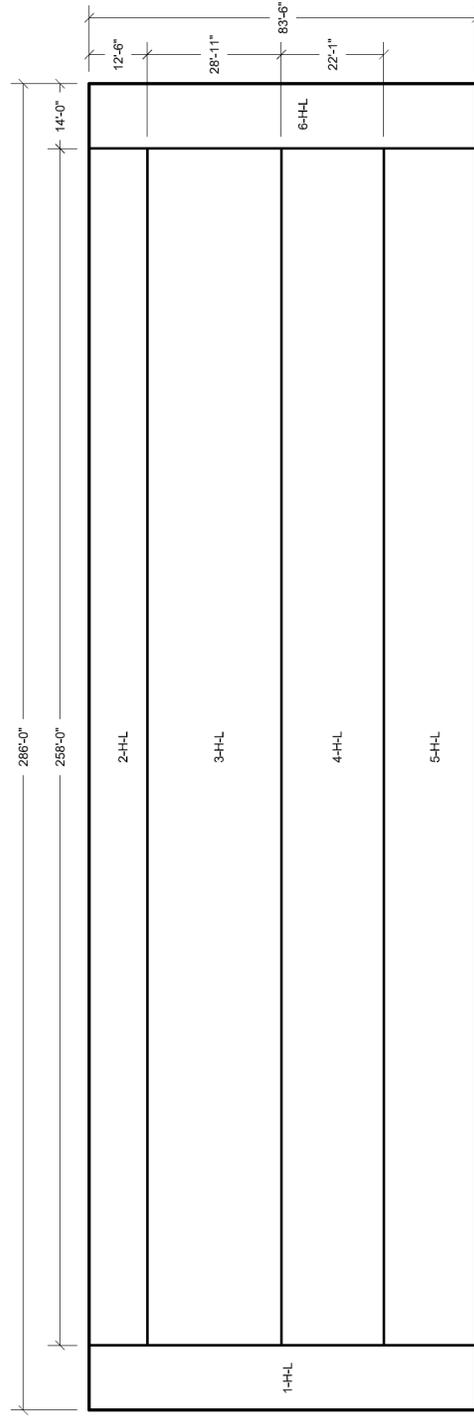


FIGURE 3H.6-38: UHS BASIN SOUTH WALL LOOKING NORTH
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE

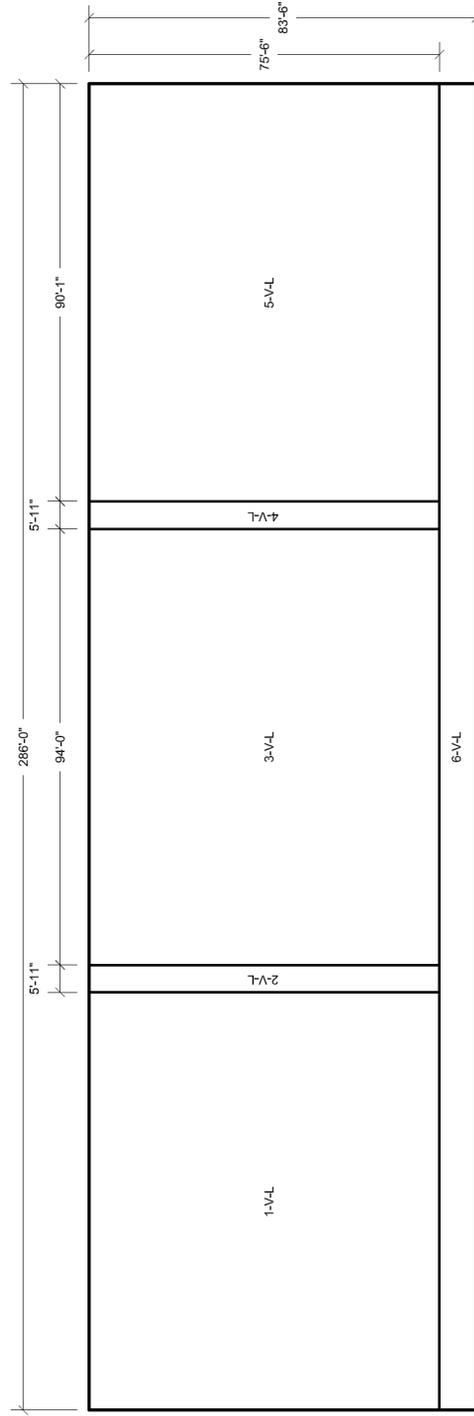


FIGURE 3H.6-80: UHS BASIN SOUTH WALL LOOKING NORTH
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE

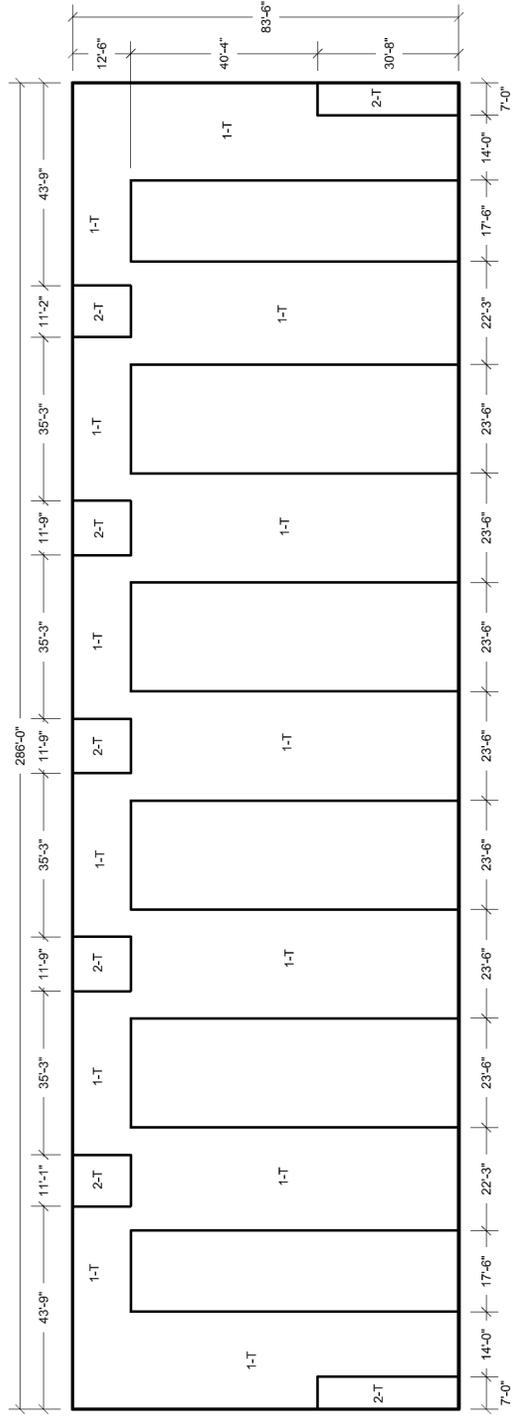


FIGURE 3H.6-81: UHS BASIN SOUTH WALL LOOKING NORTH
TRANSVERSE REINFORCEMENT ZONES

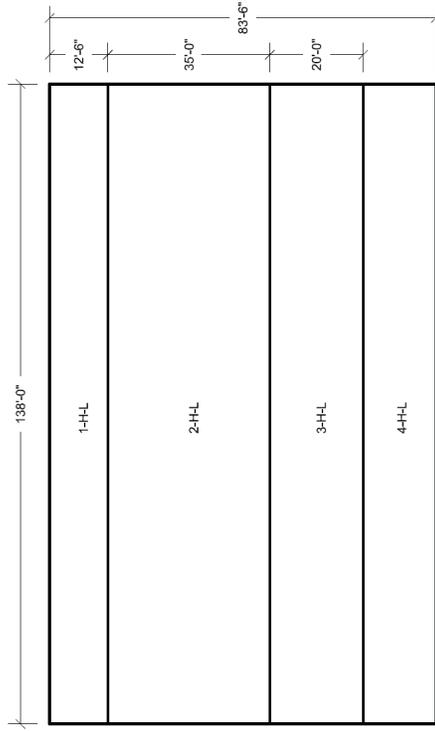


FIGURE 3H.6-9Z: LHS BASIN EAST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE

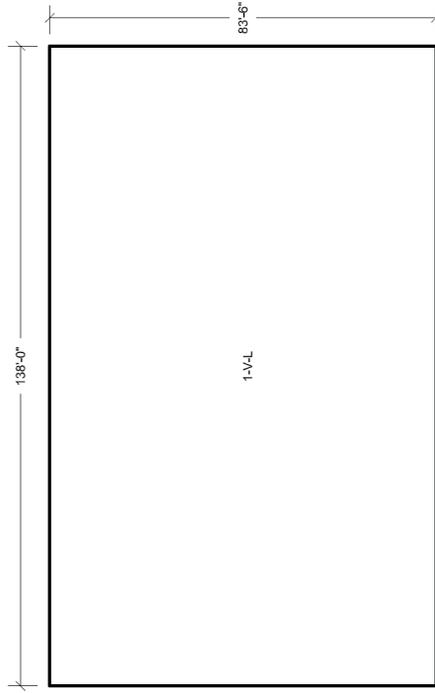


FIGURE 3H.6-98: LHS BASIN EAST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

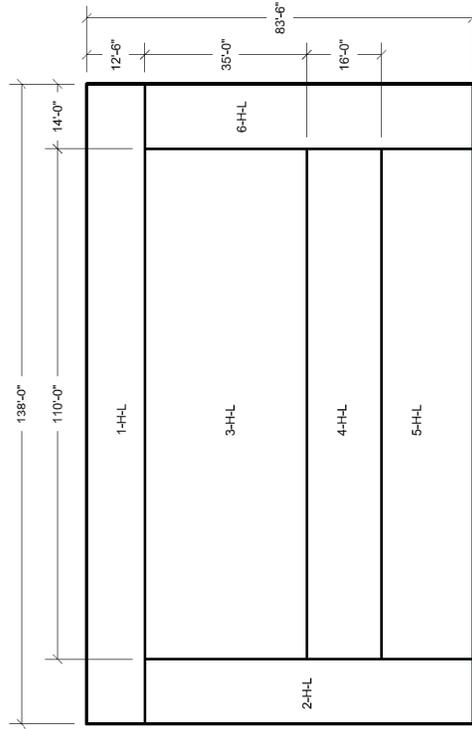


FIGURE 3H.6-94: LHS BASIN EAST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE

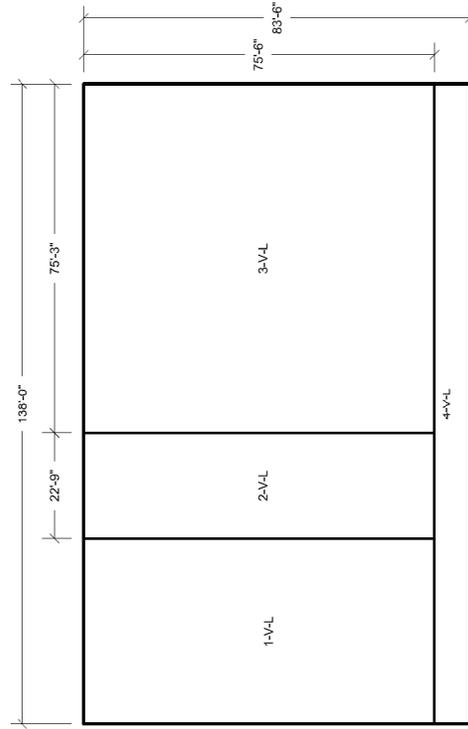


FIGURE 3H.6-96: LHS BASIN EAST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE

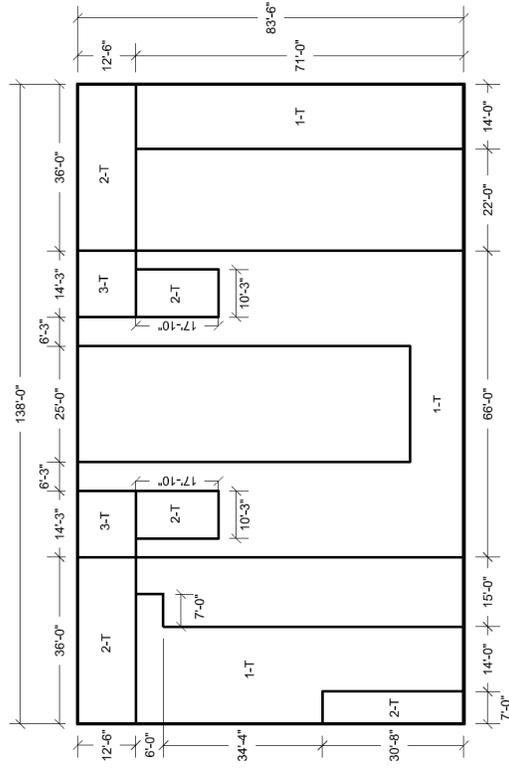
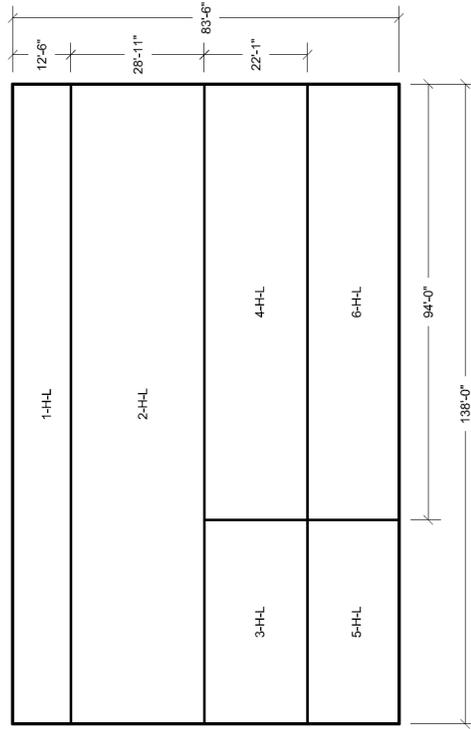


FIGURE 3H.6.96: LHS BASIN EAST WALL LOOKING WEST
TRANSVERSE REINFORCEMENT ZONES



**FIGURE 3H.6-97: LHS BASIN WEST WALL LOOKING EAST
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE**

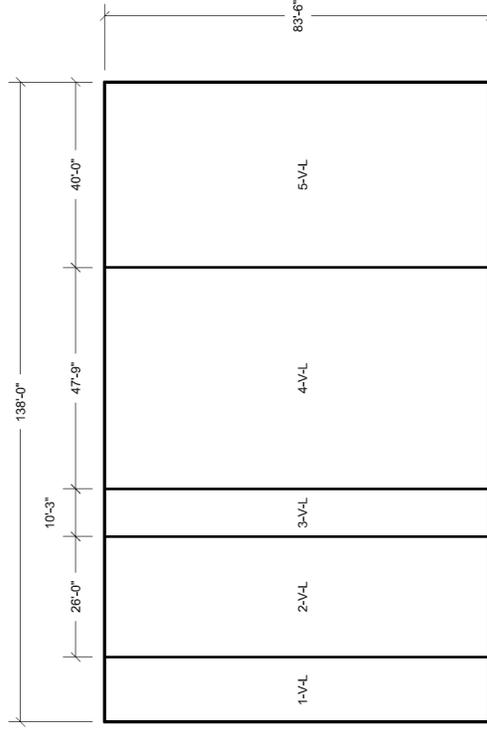


FIGURE 3H.6.98: LHS BASIN WEST WALL LOOKING EAST
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

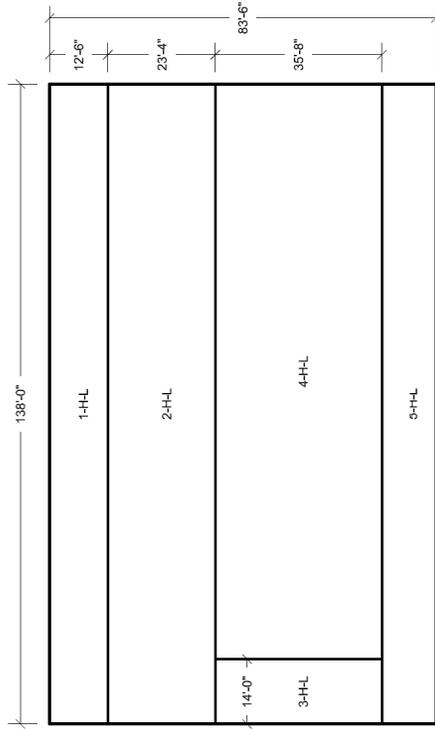
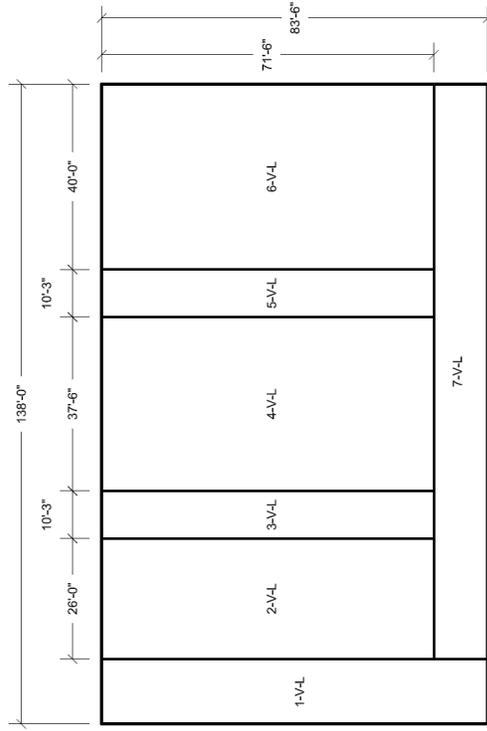
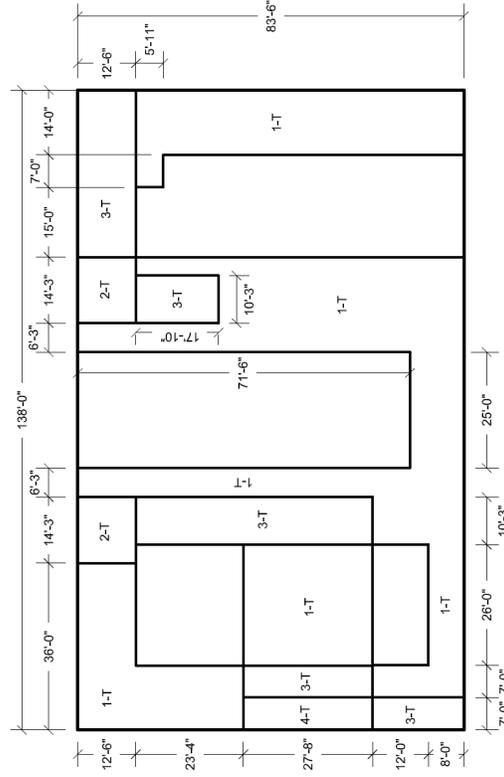


FIGURE 3H.6-98: LHS BASIN WEST WALL LOOKING EAST
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE



**FIGURE 3H.6-100: UHS BASIN WEST WALL LOOKING EAST
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE**



**FIGURE 3H.6-101: UHS BASIN WEST WALL LOOKING EAST
TRANSVERSE REINFORCEMENT ZONES**

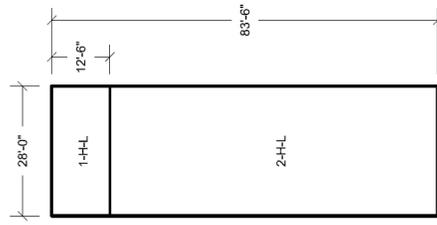


FIGURE 3H.6-102: UHS BASIN NORTH BUTTRESS LOOKING WEST & UHS BASIN SOUTH BUTTRESS LOOKING EAST
HORIZONTAL REINFORCEMENT ZONES
NEAR & FAR SIDE FACES

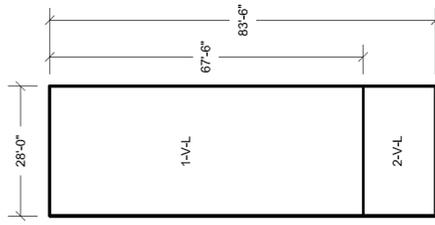


FIGURE 3H.6-103: UHS BASIN NORTH BUTTRESS LOOKING WEST & UHS BASIN SOUTH BUTTRESS LOOKING EAST
VERTICAL REINFORCEMENT ZONES
NEAR & FAR SIDE FACES

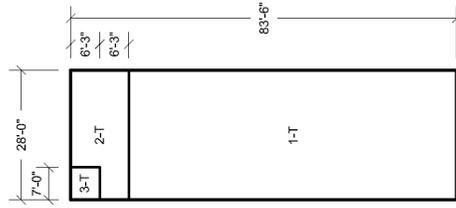


FIGURE 3H.6-104: UHS BASIN NORTH BUTTRESS LOOKING WEST & UHS BASIN SOUTH BUTTRESS LOOKING EAST TRANSVERSE REINFORCEMENT ZONES

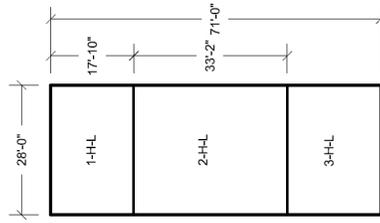


FIGURE 3H.6-105: UHS BASIN EAST BUTTRESS LOOKING NORTH & UHS BASIN WEST BUTTRESS LOOKING SOUTH
HORIZONTAL REINFORCEMENT ZONES
NEAR AND FAR SIDE FACES

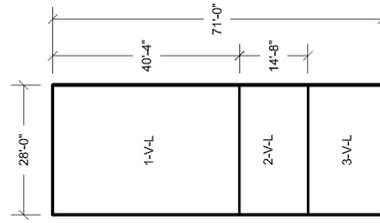


FIGURE 3H.6-106: UHS BASIN EAST BUTTRESS LOOKING NORTH & UHS BASIN WEST BUTTRESS LOOKING SOUTH
VERTICAL REINFORCEMENT ZONES
NEAR AND FAR SIDE FACES

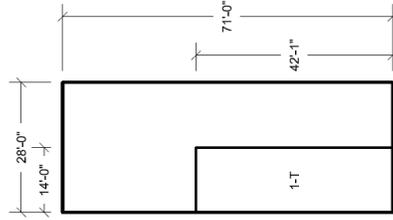


FIGURE 3H.6-107: UHS BASIN EAST BUTTRESS LOOKING NORTH & UHS BASIN WEST BUTTRESS LOOKING SOUTH TRANSVERSE REINFORCEMENT ZONES

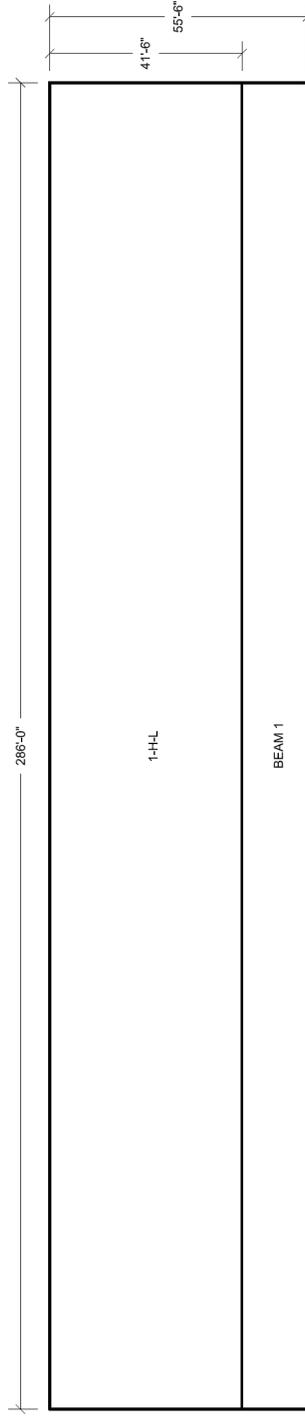


FIGURE 3H.6-108: COOLING TOWER NORTH (AND SOUTH) WALL LOOKING SOUTH (NORTH),
HORIZONTAL REINFORCEMENT ZONES,
NEAR SIDE FACE

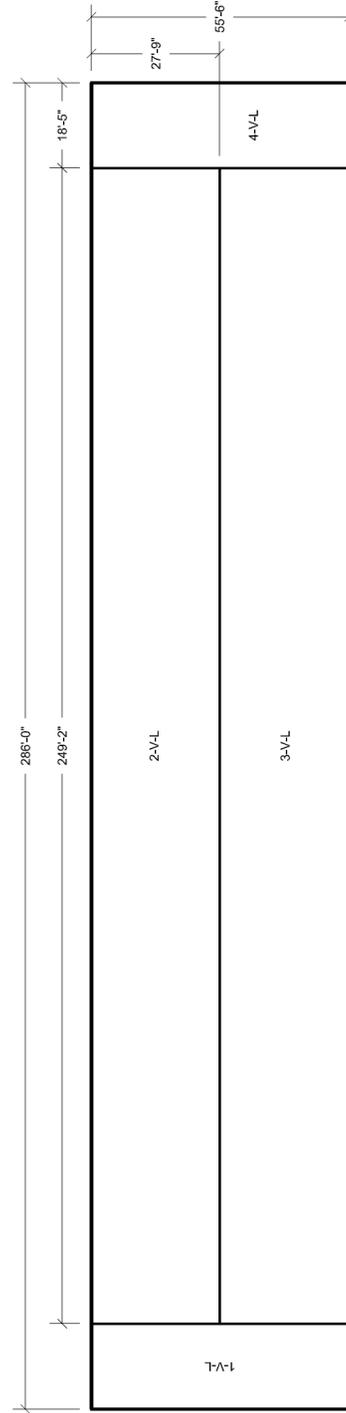


FIGURE 3H.6-108. COOLING TOWER NORTH (AND SOUTH) WALL LOOKING SOUTH (NORTH).
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

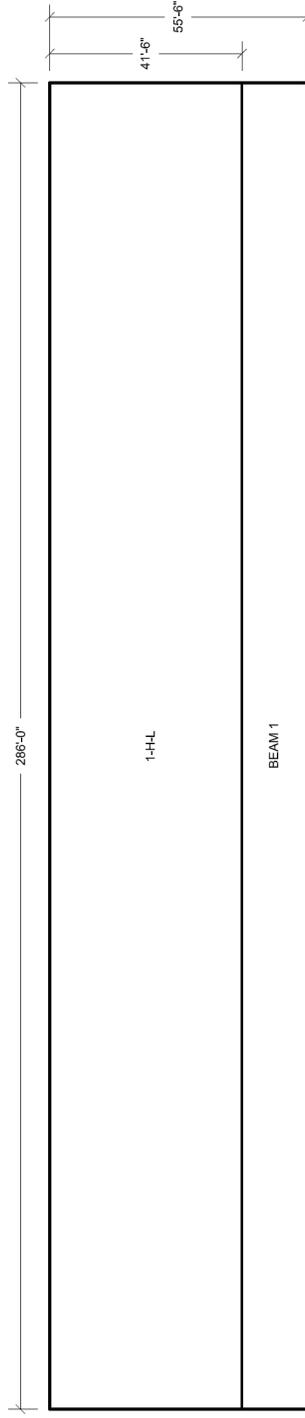


FIGURE 3H.6-110-COOLING TOWER NORTH (AND SOUTH) WALL LOOKING SOUTH (NORTH)
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE

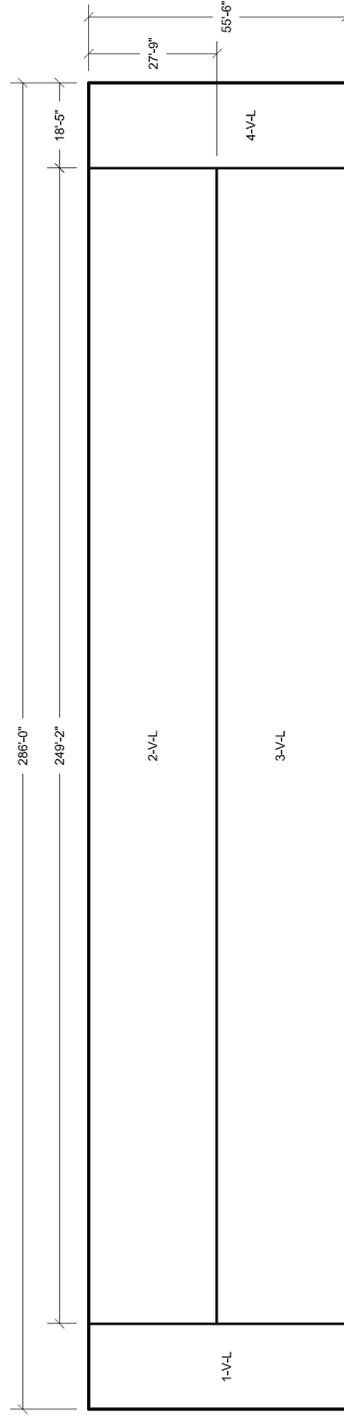


FIGURE 3H.6-111: COOLING TOWER NORTH (AND SOUTH) WALL LOOKING SOUTH (NORTH),
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE

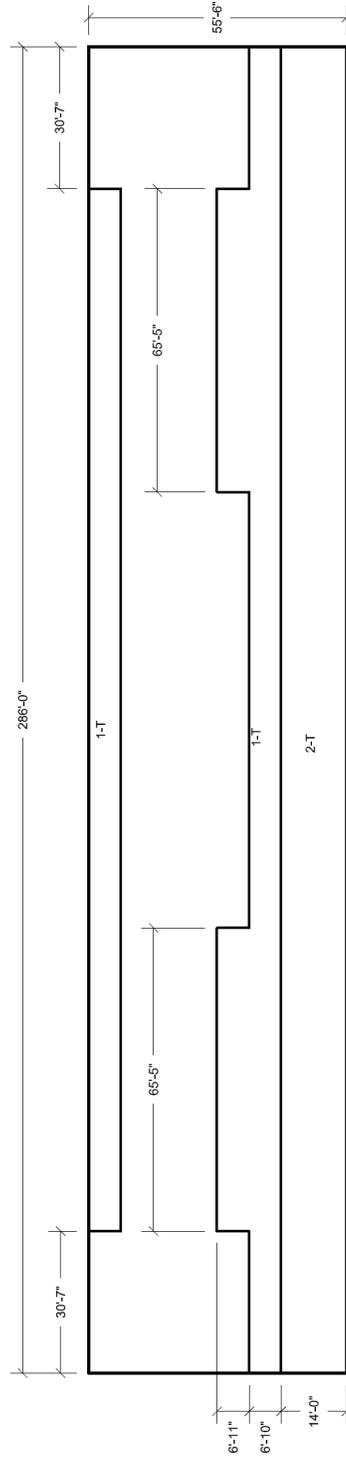


FIGURE 3H.6-112-COOLING TOWER NORTH (AND SOUTH) WALL LOOKING SOUTH (NORTH)
TRANSVERSE REINFORCEMENT ZONES

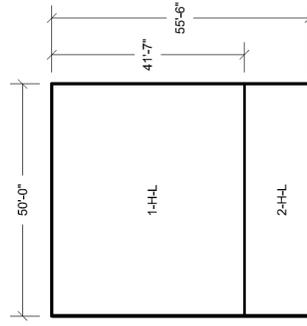


FIGURE 3H.6-113. COOLING TOWER EAST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE

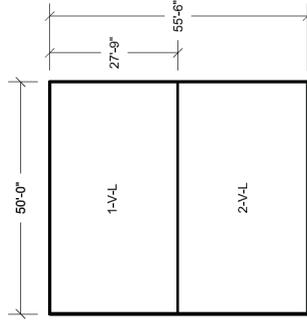


FIGURE 3H.6-114: COOLING TOWER EAST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

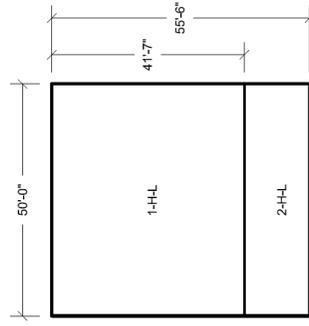
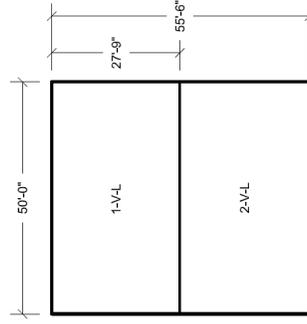
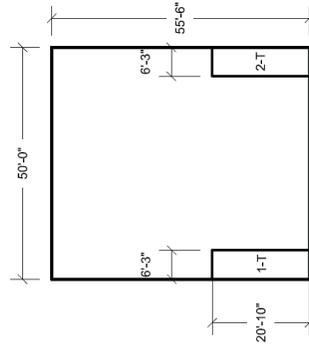


FIGURE 3H.6-118. COOLING TOWER EAST WALL LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES.
FAR SIDE FACE



**FIGURE 3H.6-118. COOLING TOWER EAST WALL LOOKING WEST
VERTICAL REINFORCEMENT ZONES
EAST SIDE FACE**



**FIGURE 3H.6-16A: COOLING TOWER EAST WALL LOOKING WEST
TRANSVERSE REINFORCEMENT ZONES**

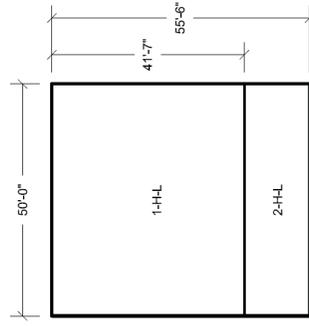


FIGURE 3H.6-117. COOLING TOWER WEST WALL LOOKING EAST
HORIZONTAL REINFORCEMENT ZONES
NEAR SIDE FACE

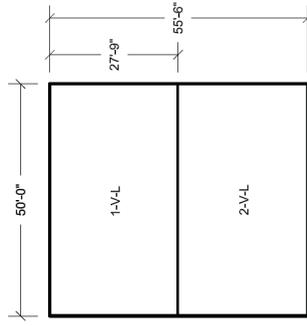


FIGURE 3H.6-118. COOLING TOWER WEST WALL LOOKING EAST
VERTICAL REINFORCEMENT ZONES
NEAR SIDE FACE

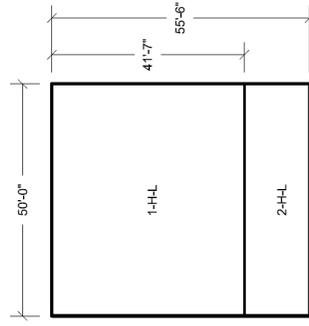
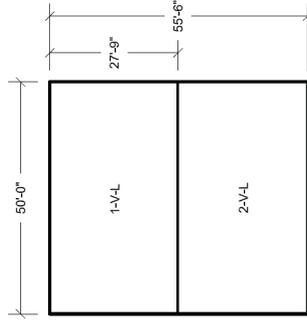
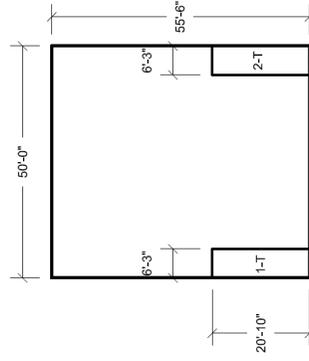


FIGURE 3H.6-118. COOLING TOWER WEST WALL LOOKING EAST
HORIZONTAL REINFORCEMENT ZONES
FAR SIDE FACE



**FIGURE 3H.6-128. COOLING TOWER WEST WALL LOOKING EAST
VERTICAL REINFORCEMENT ZONES
FAR SIDE FACE**



**FIGURE 3H-6-120A: COOLING TOWER WEST WALL LOOKING EAST
TRANSVERSE REINFORCEMENT ZONES**

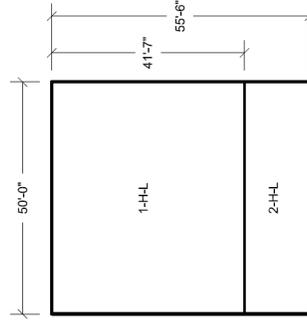
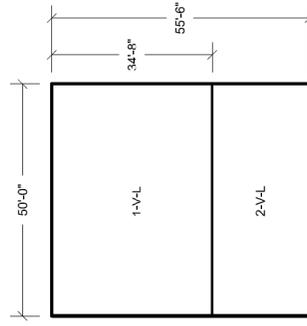


FIGURE 3H.6-121: COOLING TOWER INTERNAL WALLS, LOOKING WEST
HORIZONTAL REINFORCEMENT ZONES
NEAR AND FAR SIDE FACES



**FIGURE 3H.6-122: COOLING TOWER INTERNAL WALLS, LOOKING WEST
VERTICAL REINFORCEMENT ZONES
NEAR AND FAR SIDE FACES**

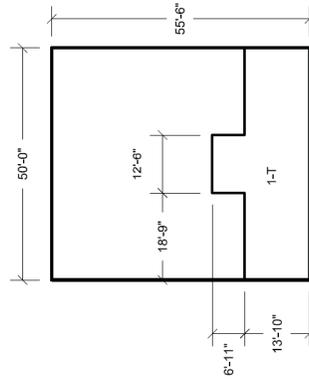


FIGURE 3H-6-122A: COOLING TOWER INTERNAL WALL LOOKING WEST
TRANSVERSE REINFORCEMENT ZONES

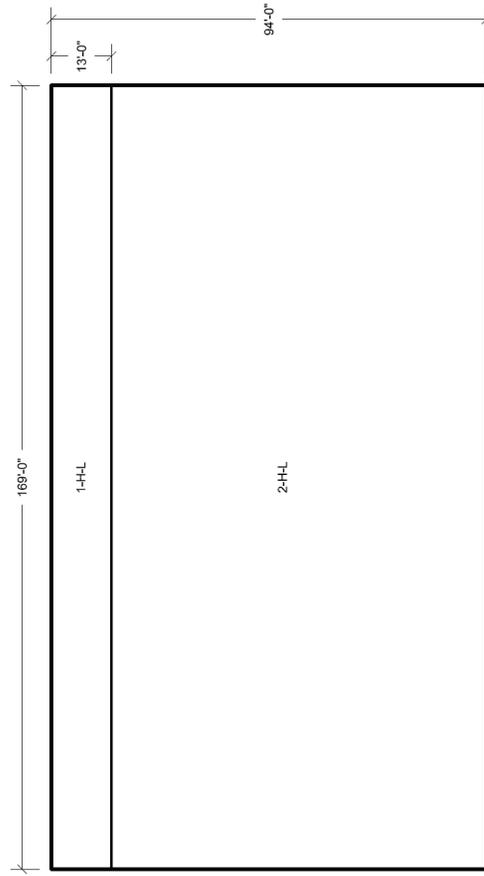


FIGURE 3H.6-123: PUMP HOUSE FOUNDATION MAT
EAST/WEST REINFORCEMENT ZONES
TOP FACE

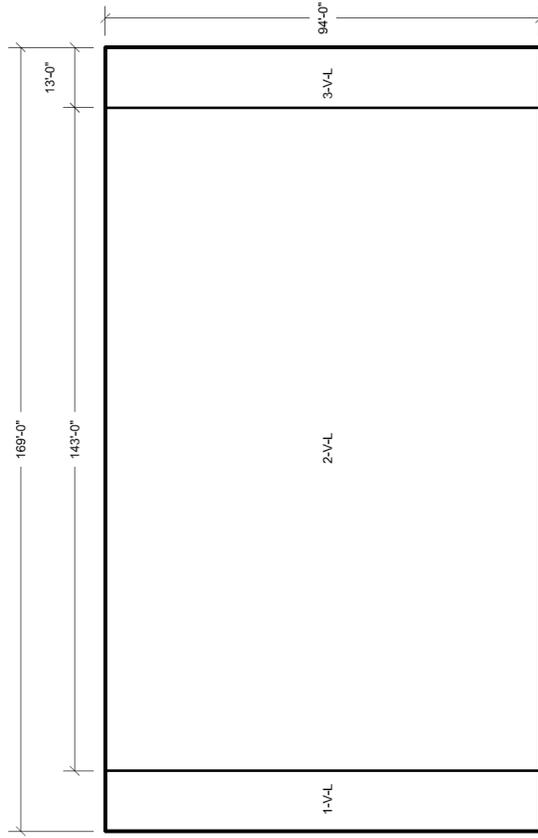


FIGURE 3.H.6-124: PUMP HOUSE FOUNDATION MAT
NORTHSOUTH REINFORCEMENT ZONES
TOP FACE

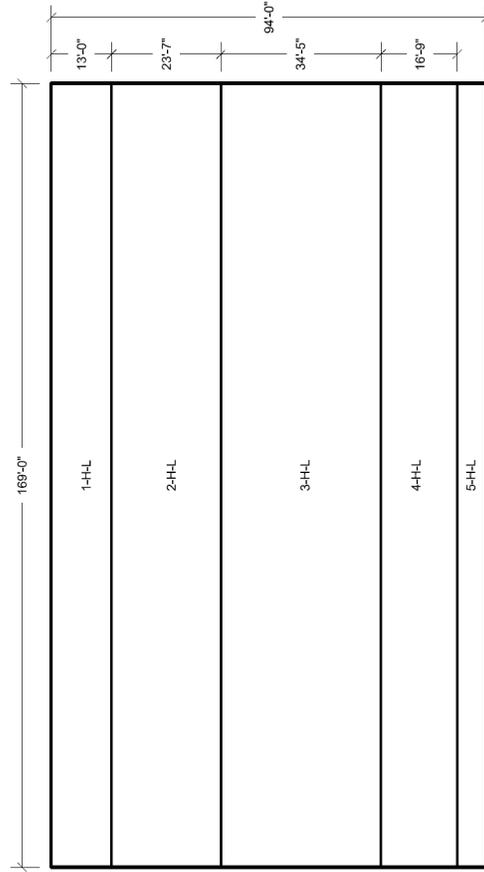


FIGURE 3H.6-12c: PUMP HOUSE FOUNDATION MAT
EAST/WEST REINFORCEMENT ZONES
BOTTOM FACE

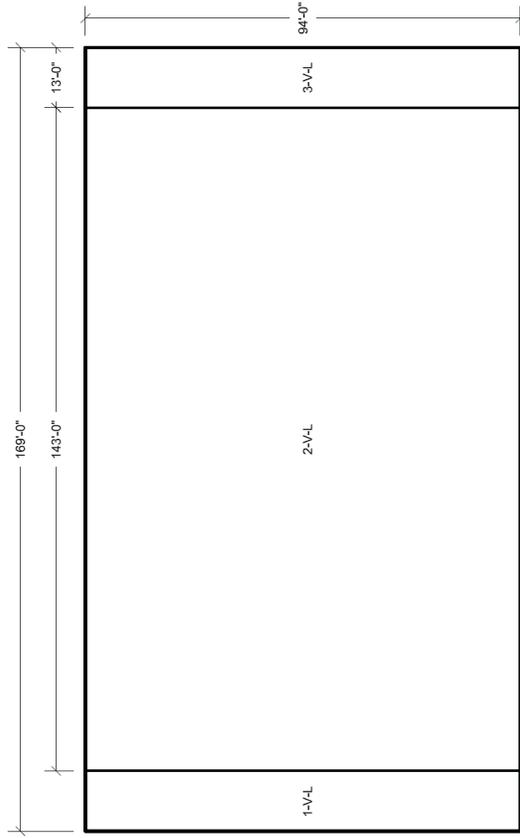


FIGURE 3.H.6-126: PUMP HOUSE FOUNDATION MAT
NORTHSOUTH REINFORCEMENT ZONES
BOTTOM FACE

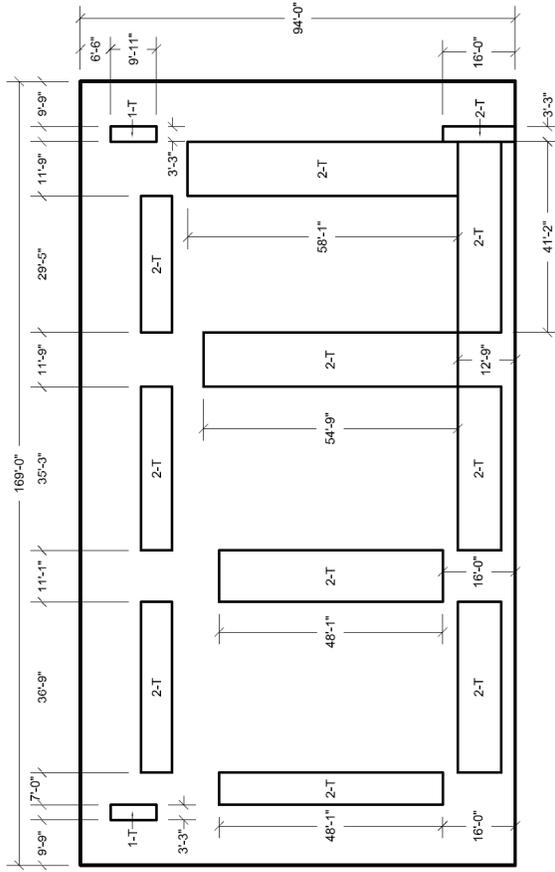


FIGURE 3H.6-126A: PUMP HOUSE FOUNDATION MAT
TRANSVERSE REINFORCEMENT ZONES

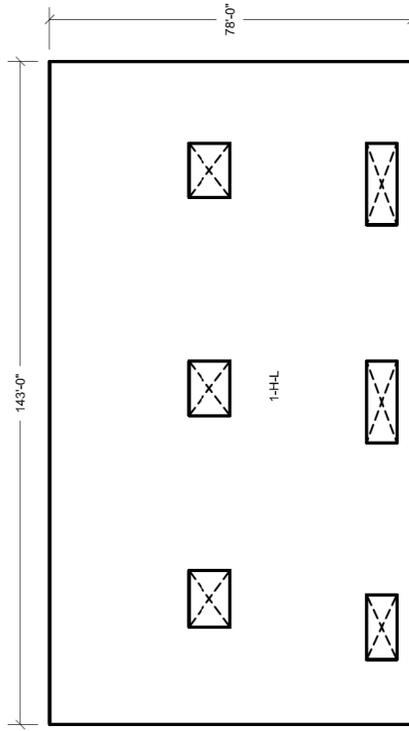


FIGURE 3H.6-127: PUMP HOUSE FLOOR EL. 15'-2"
EAST/WEST REINFORCEMENT ZONES
TOP FACE

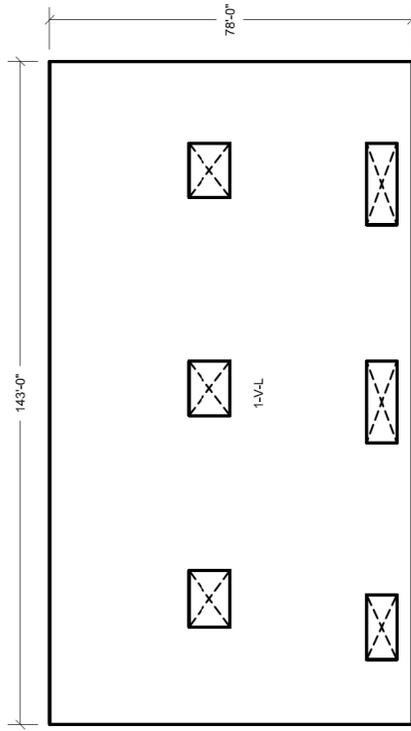


FIGURE 3H.6-128: PUMP HOUSE FLOOR EL. 15'-2"
NORTHSOUTH REINFORCEMENT ZONES
TOP FACE

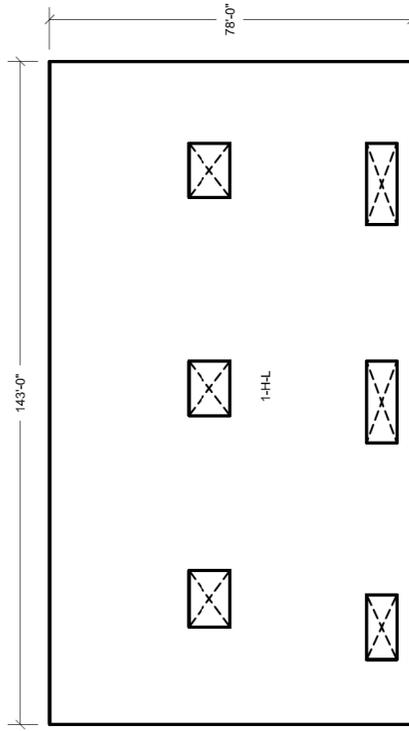


FIGURE 3H.6-128: PUMP HOUSE FLOOR EL. 15'-2"
EAST/WEST REINFORCEMENT ZONES
BOTTOM FACE

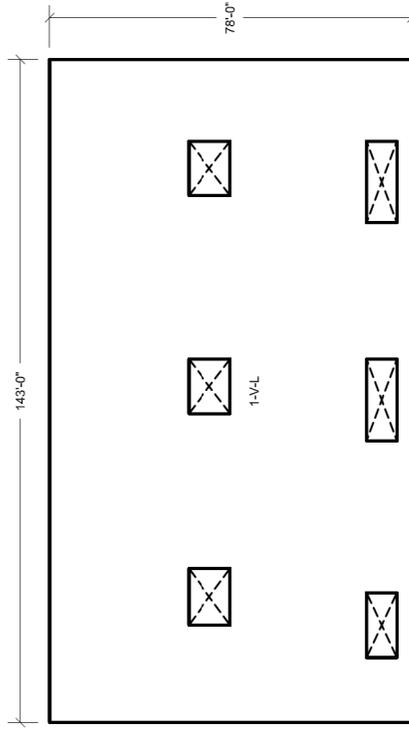


FIGURE SH.6-130: PUMP HOUSE FLOOR EL. 15'-2"
NORTHSOUTH REINFORCEMENT ZONES
BOTTOM FACE

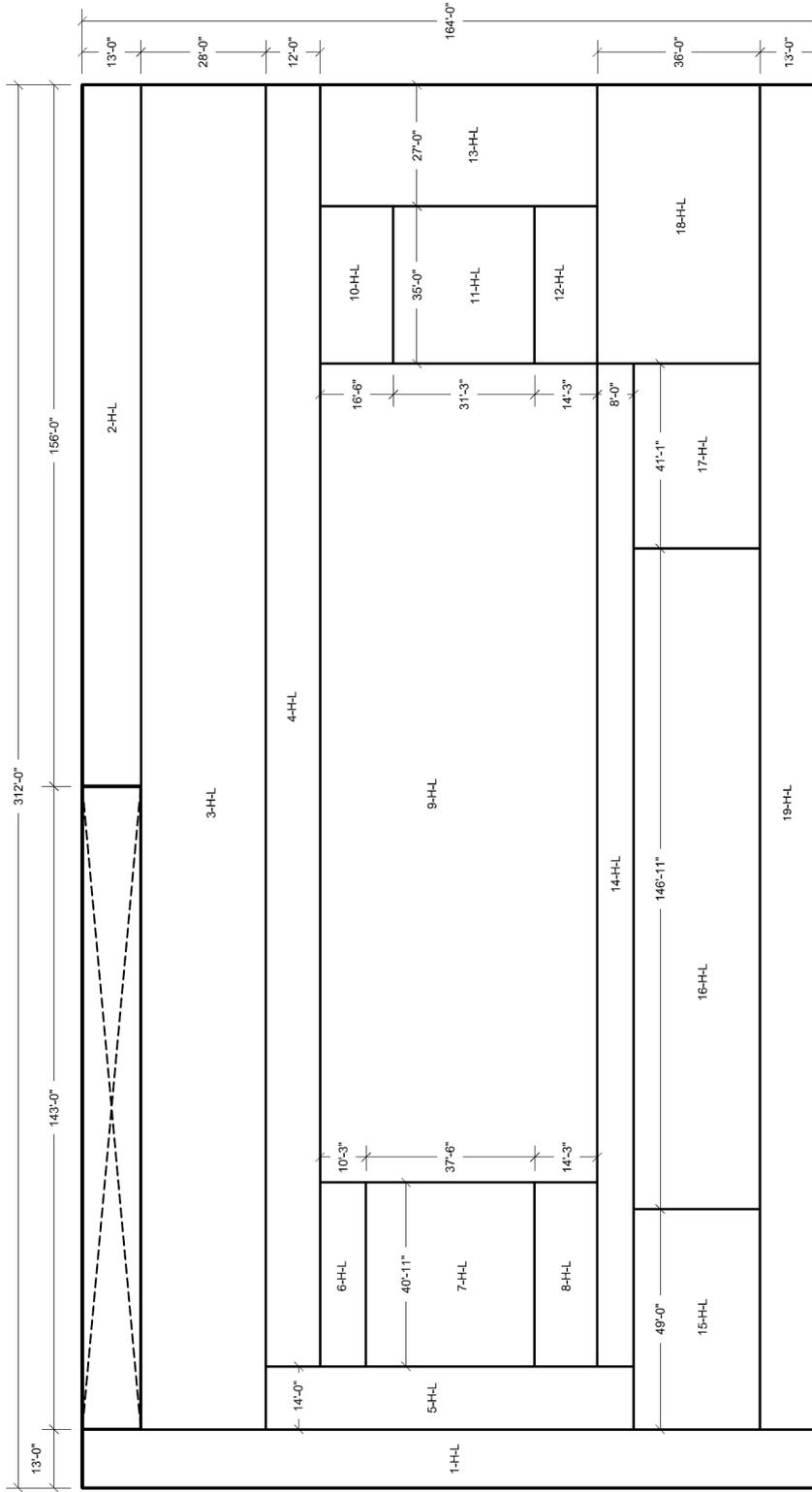


FIGURE 3H.6-431: UHS BASIN FOUNDATION MAT
 EAST/WEST REINFORCEMENT ZONES
 TOP FACE

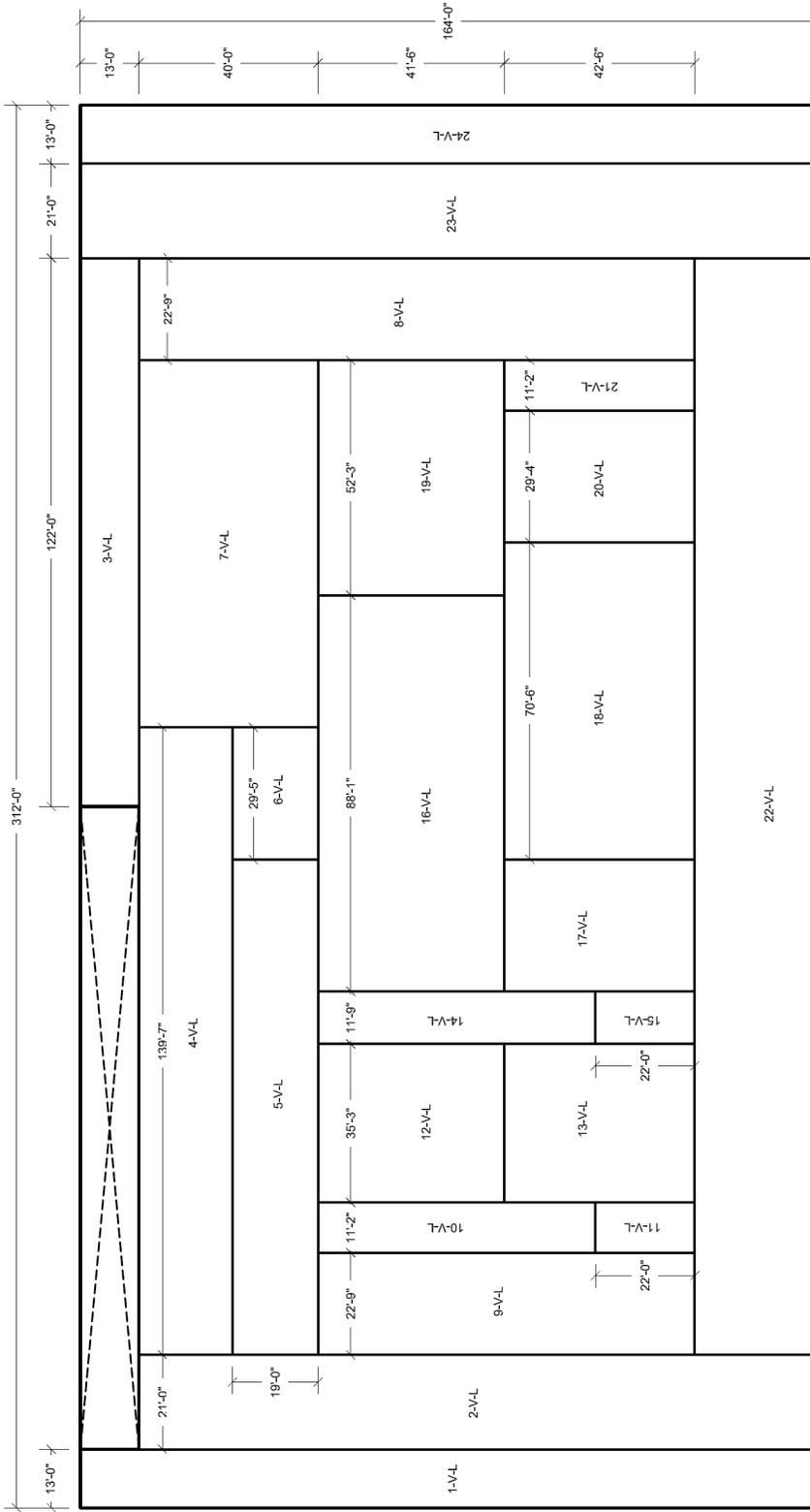


FIGURE 3H.6-132: UHS BASIN FOUNDATION MAT
NORTH/SOUTH REINFORCEMENT ZONES
TOP FACE

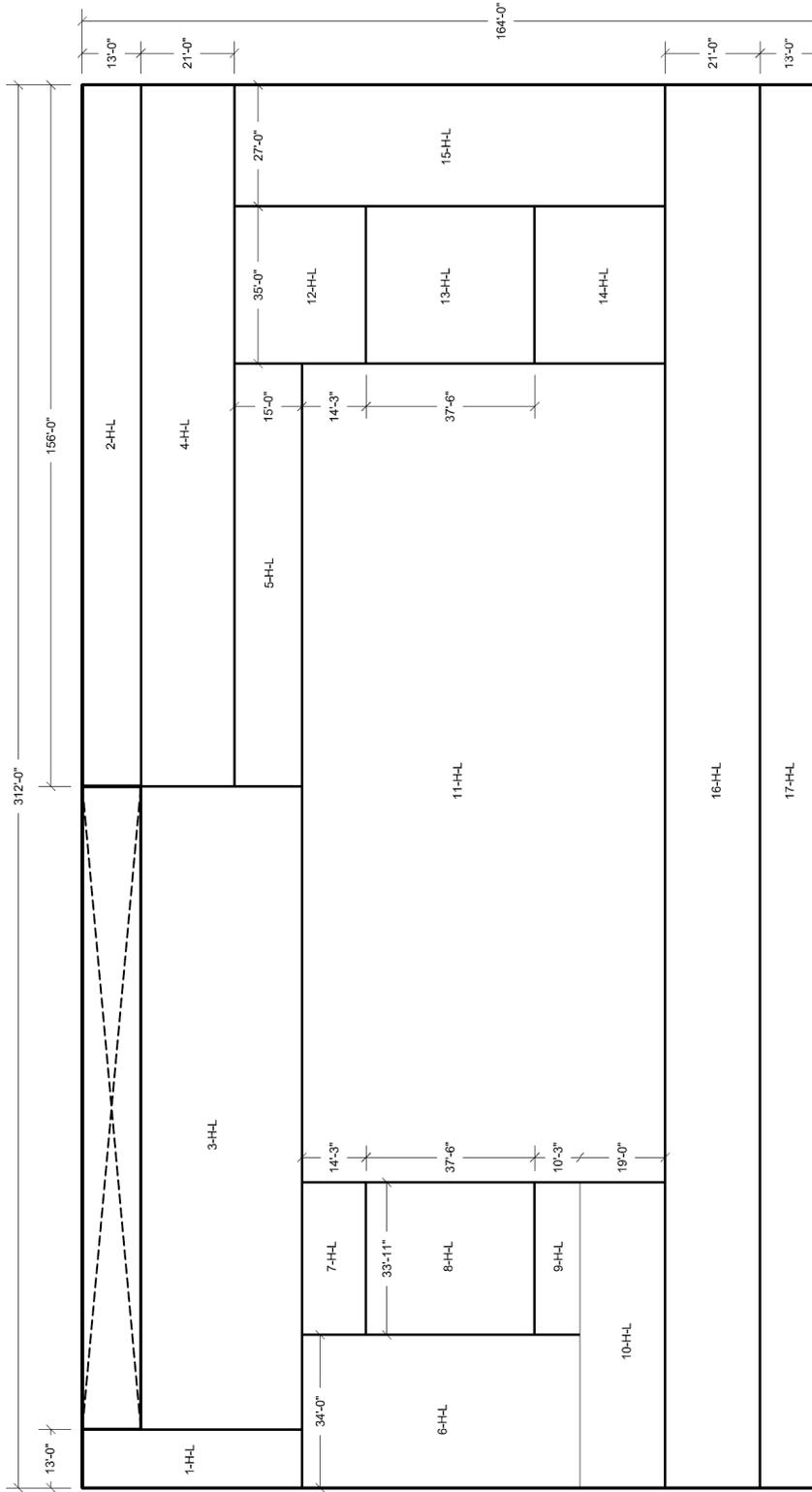


FIGURE 3H.6-433: UHS BASIN FOUNDATION MAT
EAST/WEST REINFORCEMENT ZONES
BOTTOM FACE

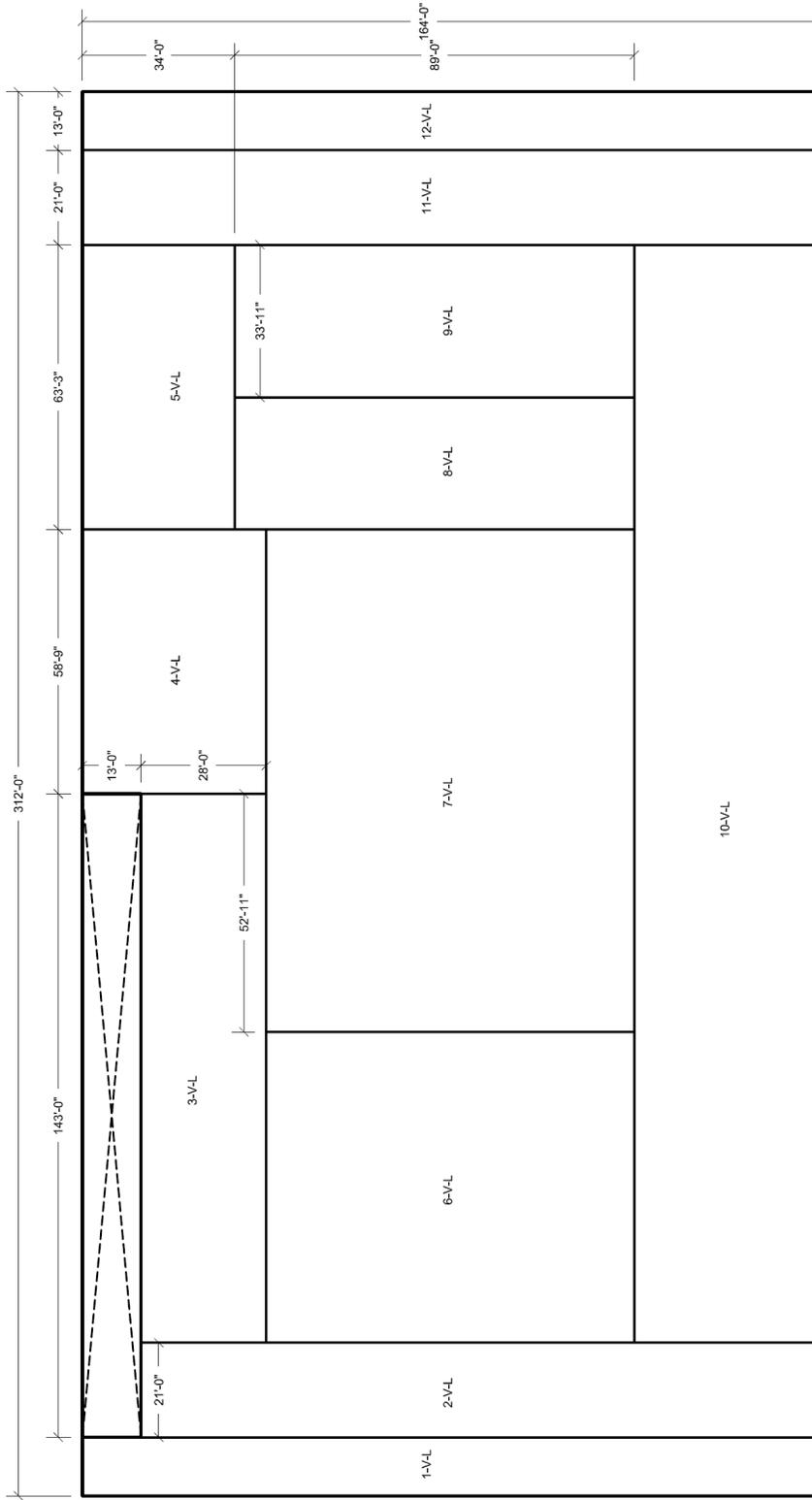


FIGURE 3H.6-134: UHS BASIN FOUNDATION MAT
NORTH/SOUTH REINFORCEMENT ZONES
BOTTOM FACE

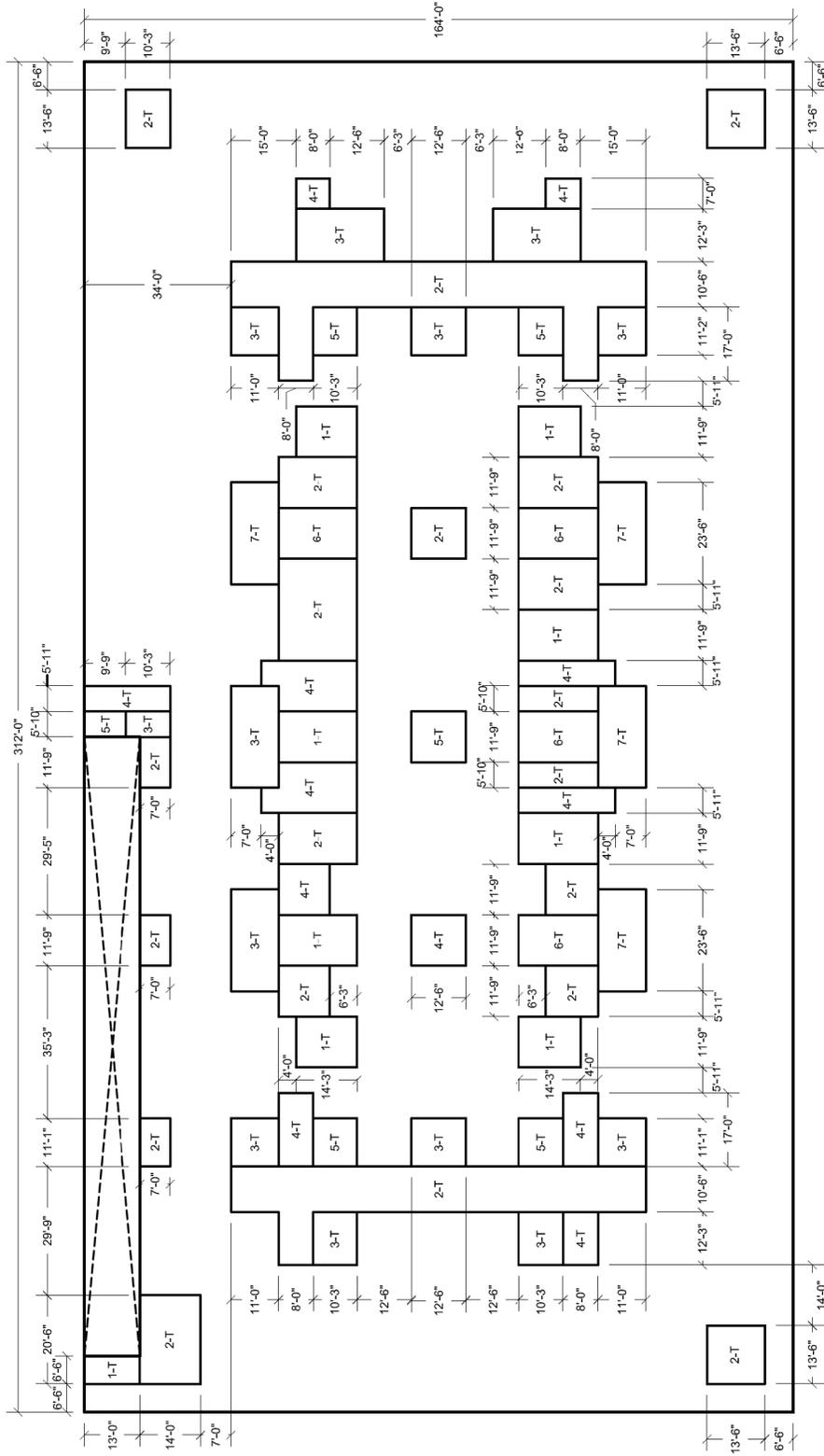


FIGURE 3H.16-134A: UHS BASIN FOUNDATION MAT
TRANSVERSE REINFORCEMENT ZONES

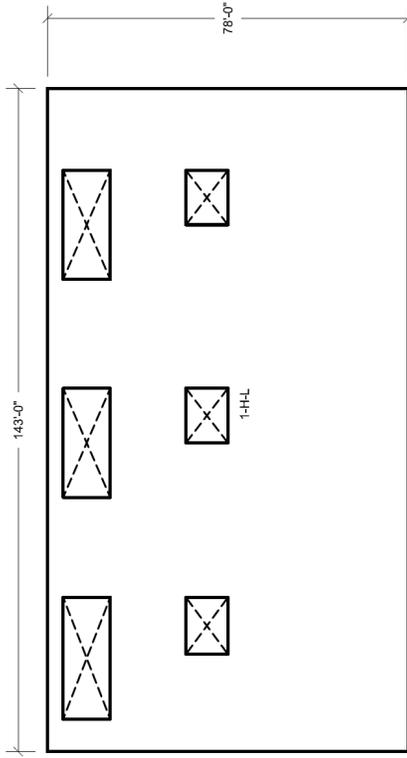
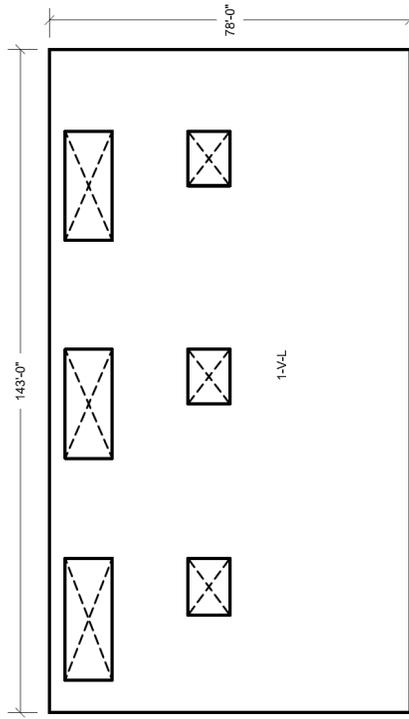
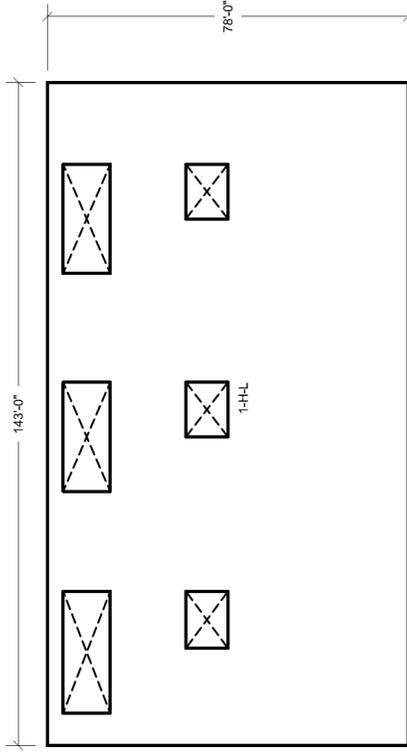


FIGURE 3H.6-135: PUMP HOUSE ROOF
EAST/WEST REINFORCEMENT ZONES
TOP FACE



**FIGURE 3H.6-136A: PUMP HOUSE ROOF
NORTHSOUTH REINFORCEMENT ZONES
TOP FACE**



**FIGURE 3H.6-138B: PUMP HOUSE ROOF
EAST/WEST REINFORCEMENT ZONES
BOTTOM FACE**

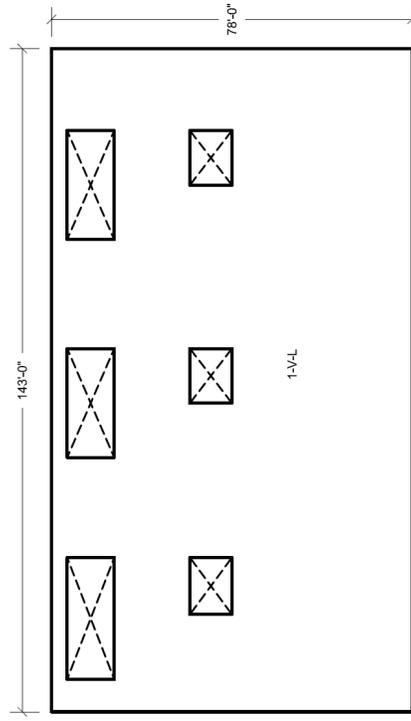


FIGURE 31.6.186C: PUMP HOUSE ROOF
NORTH/SOUTH REINFORCEMENT ZONES
BOTTOM FACE







