

**ADDENDUM 3C:
HANK NUMERICAL GROUNDWATER MODELING**

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Acronyms and Abbreviations

gpd	gallons per day
gpm	gallons per minute
ID	inner (inside) diameter
ISR	In-Situ Recovery
UZF	unsaturated zone flow
WDEQ	Wyoming Department of Environmental Quality
WY	Wyoming

MPH.1 HANK SITE NUMERICAL GROUND-WATER MODELING

Several modeling techniques were employed to evaluate ground-water impacts by the proposed ISR mining operations. The products of this modeling included predictions of operational drawdown, gradient changes, recovery, horizontal wellfield flare, and vertical flare.

The primary modeling approach used a version of the MODFLOW model to evaluate ground-water flow and drawdown resulting from the planned mining operations. The MODFLOW model was developed by the USGS in 1988 and has been updated and revised several times. MODFLOW-2005 (Harbaugh, 2005) was used for modeling of the ground-water system at the Hank Project. MODFLOW-2005 was used for the Hank Project because it has provisions for modeling of unsaturated zone flow (UZF) under unconfined conditions. The names MODFLOW and MODFLOW-2005 are used interchangeably in the remainder of the addendum.

The horizontal flare from an operating ISR wellfield was evaluated with the contaminant transport model MT3DMS (Zheng and Wang, 2006) which utilizes cell by cell flow terms produced by the MODFLOW model. With this coupling to the MODFLOW model, MT3DMS and MODFLOW use a common model domain and configuration to evaluate the transport flare of mining solutions during conveyance between ISR injection and production wells. The use of a convection dispersion equation based numerical transport model allows a fairly sophisticated interpretation of the expected flare that will occur with the proposed injection and collection well operation.

The vertical flare of mining solution was evaluated by compiling multiple runs of an analytical radial well flow model (WTAQ (Barlow and Moench, 1999)) into a spreadsheet based matrix representing a paired ISR injection and extraction well. The WTAQ model incorporates partial penetration of both the injection and extraction wells, allows a large degree of anisotropy in the ratio of vertical to horizontal hydraulic conductivity and utilizes an implementation of the Neuman (1972) solution for unconfined aquifers. Predicted drawdowns from the WTAQ model were then compiled in a spreadsheet, and, using some additional programming to interpret the WTAQ model output, the results were converted to a matrix of heads and velocities for the aquifer interval between the paired wells.

The numerical model was also used to evaluate the potential for retrieval of excursions and the sufficiency of the monitor well spacing. Well stress rates for a local area were adjusted slightly to produce a stronger gradient reversal in simulating the proposed response to a local excursion. The magnitude of the gradient reversal was then compared with baseline simulations to evaluate the effectiveness in retrieval of an excursion.

MPH.1.1 Hank Project Modeling

MODFLOW-2005 was used to model the ground-water flow prior to, during and after operation of the wellfield(s). A model grid was developed to cover the proposed mine area with a relatively fine grid (30 foot by 30 foot cells) and extending the modeled area with increased cell size to encompass approximately 283 square miles. Injection and production wells were included as well stresses within the fine grid area. MODFLOW-2005 has the capability of modeling partially saturated flow through an unsaturated zone flow (UZF) module, and this was

used for the single layer unconfined aquifer Hank model in the area around the active ISR mining. This module allowed incorporation of delayed drainage from the zone above the water table for the aquifer under unconfined conditions.

MPH.1.1.1 Model Configuration

The single layer model utilized an unconfined aquifer type, with a series of general head boundaries on the perimeter of the model grid. The initial potentiometric head in the ore sand was approximated as a uniform gradient across the model grid areas. This surface was developed using the typical gradient of 0.005 feet/feet and the general gradient is from east to west. The base of the aquifer in the immediate mine area was determined from drill hole based structural mapping. Outside of the mine area, the elevation of the base of the aquifer was extrapolated based on typical structural dip from the available structural mapping. The thickness of the aquifer was established as the typical thickness of 90 feet.

On the periphery of the model grid, selected cells were designated as general head boundary cells to stabilize the potentiometric surface. The head in each of the 106 designated general head boundary cells was set at the initial model head and the cell conductance was set at a relatively high level to provide a generally stable regional potentiometric surface.

MPH.1.1.1.1 Model Grid

The model grid consists of 274 rows by 98 columns and is rotated approximately 10.5 degrees counterclockwise from the orthogonal directions. The smallest cell dimension is 30 feet by 30 feet, and the largest cell dimension is 13,500 feet by 13,500 feet as shown in Figure MPH.1-1.

MPH.1.1.1.2 Aquifer Properties

The primary aquifer properties information used in the model included hydraulic conductivity and specific yield. The hydraulic conductivity was set at 1.0 foot/day and an effective specific yield of 0.14. The water level is near the overlying confining layer in some areas of the planned wellfields, and it is likely that a significant portion of the wellfield area will be under unconfined conditions both prior to and during mining. This results in a condition where there is potentially an impact by vertical partially saturated flow from areas where the wellfield bleed causes significant drawdown in the aquifer.

The partially saturated flow conditions require additional definition of hydraulic properties. The UZF module in MODFLOW-2005 utilizes the ratio of vertical to horizontal hydraulic conductivity and a Brooks-Corey function to approximate the hydraulic conductivity under partially saturated conditions. The ratio of vertical to horizontal hydraulic conductivity was estimated at 0.085. The Brooks-Corey function uses an exponent (epsilon) to define the shape of the partially saturated hydraulic conductivity as a function of volumetric moisture content and that was set at 3.5. The effective saturated volumetric moisture content was set at 0.30 and the UZF module uses the specific yield of 0.14 to approximate residual saturation.

MPH.1.1.1.3 Wellfield Configuration

The proposed mining sequence includes two distinct wellfields with an anticipated mining period of 1½ years for each wellfield. Each wellfield consists of a combination of staggered production

and injection wells arranged generally in a line drive layout for the sinuous ore body. The number of wells and well locations are preliminary and will be refined with further definition of the ore body. Because the natural gradient is from east to west, the well arrangement for the typically narrow ore body places the injection wells on the upgradient side of the ore zone with the production wells on the downgradient side of the ore zone. Several model runs were conducted to evaluate horizontal flare, general wellfield operation, and post mining recovery. The model runs and wellfield configuration for the horizontal flare evaluation are described in a following section. Figure MPH.1-2 presents the wellfield #1 production and injection well layout. Figure MPH.1-3 presents the wellfield #2 production and injection well layout.

MPH.1.1.1.4 Operational Parameters

The anticipated production rates from the wellfield #1 wells range from 12.5 to 12.7 gpm. A total of 198 production wells were included in the full wellfield #1 operation. Total production rate was 2,500 gpm. Injection well operational rates ranged from 5.2 to 12.7 gpm with a total of 271 injection wells. Excess production or bleed rate was set at 3% of total production with a resulting injection rate of 2,425 gpm.

The anticipated production rate from the 93 production wells in wellfield #2 is 26.9 gpm with a resulting total production rate of 2,500 gpm. Injection well operational rates ranged from 15.6 to 20 gpm with a total of 119 injection wells. Excess production or bleed rate was set at 3% of total production with a resulting injection rate of 2,425 gpm.

MPH.1.1.1.5 Stress Periods

Numerous stress periods were included to allow comparison of predicted aquifer response to the wellfield operations at several times during the simulation period. A transient simulation also requires very small computational time steps after each significant change in aquifer stresses including startup or shutdown of well operation. This is necessary to prevent a failure to converge in the model computation. The initial stress period and time steps were set at a very small value (0.0001 day with 5 time steps) to produce a model output result that essentially reflects initial head conditions. The stress period lengths were then gradually increased until there was a significant change in model stresses, at which the sequence reverted to a short stress period followed by gradually increasing stress period lengths. A total of 12 stress periods were used in a total simulation period of six years which included 1.5 years of operation of each wellfield followed by a three year period of post-mining recovery.

MPH.1.1.2 Model Results

The MODFLOW model produces output in terms of predicted drawdown or predicted head at selected times within the simulation. The drawdown or water-level rise is calculated as the difference between head at a selected time and the initial head for the aquifer at the start of the simulation. Both results are useful in the interpretation of aquifer response to the mining and are used to evaluate the modeling predictions.

MPH.1.1.2.1 Wellfield #1

The configuration for wellfield #1 is shown in Figure MPH.1-2. The modeled potentiometric surface prior to the start of mining is presented in Figure MPH.1-4. The mining operation of the

production and injection wells is expected to continue for 18 months, after which mining of wellfield #2 begins. Figure MPH.1-5 presents the predicted drawdown contours for wellfield #1 after one year of operation. Figure MPH.1-6 presents the predicted water-level elevation contours for wellfield #1 after one year of operation. The operation of the wellfield at a bleed rate of 3% of the planned 2,500 gpm production rate has resulted in development of a significant cone of depression around the operating wellfield. The area of gradient reversal extends approximately 800 to 1,300 feet to the west of wellfield #1.

MPH.1.1.2.2 Wellfield #2

The configuration for wellfield #2 is shown in Figure MPH.1-3. The operation of wellfield #2 will begin after mining is completed in wellfield #1. Figure MPH.1-7 presents the predicted drawdown contours for the mine area after 18 months of operation of wellfield #1 and 18 months of operation of wellfield #2. The drawdown calculation is based on water level change from the pre-mining potentiometric surface and this drawdown reflects significant residual drawdown from the operation of wellfield #1. The drawdown at the end of mining shown in Figure MPH.1-7 is very similar to drawdown predictions produced by the analytical model. This similarity between the numerical and analytical model results demonstrates the adequacy of analytical modeling with an appropriate configuration. Figure MPH.1-8 presents the predicted water-level elevation contours for the mine area at the end of mining in wellfield #2. Wellfield #2 is planned to be operated at a bleed rate of 3% of the planned 2,500 gpm production rate. The area of gradient reversal extends approximately 1,200 to 1,600 feet to the west of wellfield #2.

MPH.1.1.2.3 End of Mining

The end of mining water level changes are reflected in Figures MPH.1-7 and MPH.1-8 as described in the previous section. The planned Hank area ISR project includes two adjacent wellfields operated in sequence for a period of 18 months per wellfield. Wellfield #1 encompasses a larger area, but the effective stress rate of 75 gpm still produces a significant impact on the potentiometric surface. Following cessation of mining in wellfield #1, the potentiometric surface exhibits some recovery in the northern portion of the mining project. Simultaneously, the operation of wellfield #2 causes drawdown in the southern portion of the project area.

MPH.1.2 Horizontal Flare Evaluation

Horizontal flare around the operating well field was evaluated by modeling transport of a generic solute that was introduced into the injection wells. The MODFLOW-2005 results for a selected ore zone within wellfield #1 were used as a basis for simulating flare of the lixiviant in the operating wellfield.

MPH.1.2.1 MT3DMS Modeling

The MT3DMS model is a convection-dispersion equation (CDE) based model that utilizes ground-water flow output from the MODFLOW model to simulate solute transport. This is accomplished using a routine in MODFLOW that produces a transfer file that includes cell by cell flow terms. This transfer file is then read by MT3DMS, and the solute transport processes are "superimposed" on the ground-water flow. The MT3DMS has features for solute adsorption,

retardation, transformation, degradation, etc., but for this application, the solute was assumed to be conservatively transported and these features were not used.

In order to evaluate the flare, a generic solute was used with an elevated concentration of the lixiviant injectate. The ratio of lixiviant concentration to background concentration was 5, and the background concentration was set at 1.0 for simplicity. The lixiviant concentration was set at 5.0, and the increase in concentration in the area surrounding injection wells was used as the indicator of flare. Because the solute was generic and the magnitude of concentration changes is used to quantify flare, the units of concentration do not affect the evaluation.

MPH.1.2.1.1 Transport Model Configuration

The model grid, dimensions, and layout are the same as those established in the MODFLOW-2005 modeling.

MPH.1.2.1.2 Wellfield Configuration

The wellfield utilized in the MODFLOW-2005/MT3DMS modeling was limited to the lower ore zone of wellfield #1. This subset of wellfield #1 included 88 production wells operating at a rate of 12.5 gpm, and 125 injection wells operating at a rate ranging from 5.2 to 12.7 gpm. There was a 3% bleed in the well field operation with a resulting net extraction stress of approximately 33 gpm. The wells included in the horizontal flare modeling are shown along with the approximate boundary of the identified ore body in Figure MPH.1-9

MPH.1.2.1.3 Stress Periods

Because MT3DMS and MODFLOW-2005 are coupled through a transfer file, the stress periods for MT3DMS are the same as those used in MODFLOW-2005. A modeling period of 120 days was used in the interpretation of horizontal flare. This modeling period was selected as being sufficient to allow establishment of pseudo steady-state solution flow paths and gradients within the operating wellfield, while being a short enough period that the increased gradient reversal with longer operation will not appreciably change or reduce the flare zone. With only a subset of wellfield #1 included in the stress rate, total magnitude of drawdown and corresponding gradient reversal to the wellfield is also conservatively small so there should also be some degree of conservatism in the estimation of flare.

MPH.1.2.1.4 MT3DMS Inputs

The typical aquifer thickness for the MODFLOW-2005 modeling is 90 feet, but the anticipated completion interval for an ore body is roughly 15 feet. A cell thickness of 15 feet was specified in the MT3DMS model to represent the typical anticipated completion thickness. The effective porosity of the ore zone was estimated at 30%. The dispersivity was set at 2 feet, but it is not considered a critical factor because ISR mining is primarily a pseudo steady-state convection dominated process. The diffusion coefficient was set at zero. As discussed previously, the background generic solute concentration was set at one, with a lixiviant injectate concentration of five.

MPH.1.2.2 Model Results

The development of the drawdown around the operating wellfield area with the 120 day simulation period results in gradient reversal to the wellfield. Figure MPH.1-10 presents the predicted potentiometric surface for the horizontal flare wellfield operation. On the west side of the wellfield, the zone of gradient reversal generally extends a few hundred feet after 120 days of operation. Since the ore body is irregularly shaped and consists of two separate zones, the potentiometric surface is complex.

The MT3DMS simulation utilized the ground-water flow predictions from MODFLOW-2005 to simulate the transport of the generic solute from the injection wells to the production wells. The results of this simulation are presented in Figure MPH.1-11 as concentration contours centered around the operating injection wells. The contour interval is 0.5 units, and the outer contour is 1.5 times the natural background concentration of the aquifer. This is interpreted as a concentration change representing the extent of the lixiviant flare. In the model cells containing an active injection well, the concentration approaches the injectate concentration of five.

MPH.1.2.2.1 Flare Evaluation

As shown in Figure MPH.1-11, the combination of radial flow of the lixiviant immediately around the injection wells and the radial capture zone around production wells results in flow paths that extend throughout and slightly beyond the ore body. This horizontal flare is quantified as the ratio of the area contacted by the injectate to the area of the ore body under wellfield pattern (see Figure MPH.1-9). The area contacted by the injectate is represented by the contour line where there is a 0.5 unit concentration increase over the background concentration of 1.0. The ratio of the area within the 1.5 concentration contour to the area of the ore body within the well pattern is 1.39 and this is considered the horizontal flare factor. This flare factor is larger than a more typical estimate of 1.25, and this reflects the relatively narrow linear nature of the ore body and wellfield.

MPH.1.3 Vertical Flare Evaluation

The vertical flare was estimated using a combination of the WTAQ program to calculate heads through a cross section of the aquifer and a spreadsheet for compositing the heads to evaluate the resulting velocity field. The WTAQ program incorporates a two-dimensional analytic solution for axial-symmetric ground-water flow in both confined and unconfined aquifers. The solution allows simulation of partially penetrating wells for an unconfined aquifer, which is directly applicable for the Hank ISR mining project.

The product of the WTAQ model is prediction of observation well drawdown at specified time(s) after pump start and at specified distance(s) from the pumping well. For an injection well, the drawdown predictions are simply inverted to represent water-level rise. The WTAQ program was run multiple times and the results composited to generate a matrix of drawdown predictions with matrix rows representing one foot of vertical thickness and matrix columns representing radial distance from the well in increments of one foot. The matrix dimensions were 90 rows (90 feet aquifer thickness) by 68 columns (69 feet radial distance from well). The matrix was basically mirrored on a vertical axis to provide a matrix for both an operating injection and production well. The resulting matrices were then incorporated into the spreadsheet to represent

a combination of an ISR injection and production well pair at a spacing of 69 feet in a 90 feet thick aquifer.

MPH.1.3.1 WTAQ Modeling

Inputs to the WATQ model define the completion interval for the simulated production and injection wells, and the required aquifer properties for the solution. Both the production and injection wells were located within a 90 foot thick water table aquifer. Horizontal hydraulic conductivity was 1.0 feet per day, and the ratio of vertical to horizontal hydraulic conductivity was 0.085. The aquifer storage properties included a storage coefficient of $2.1E-06$ (ft/ft) and a specific yield of 0.14. The wells were assumed to be completed from a depth of 76 to 85 feet (inclusive) below the top of the aquifer for a ten foot ore body. This represents a likely configuration for a major ore body at the Hank site.

The observation well which represents the general aquifer was assumed to be fully penetrating. Drawdown was simulated for both 10 and 30 days since the start of injection, but only the 30 day simulation was used in the vertical flare analysis. This was considered sufficient time for development of the flow regime. Because the paired well arrangement reduces a typical wellfield arrangement to a simple pair of wells rather than a production well surrounded by multiple injection wells, the anticipated well production rate was reduced to approximately 6.2 gpm to represent the simplified configuration. The multiple runs of the WTAQ program were accomplished with an external shell program that incremented through the depth and distance from the well while compiling the predicted drawdown into the matrix. The matrices were then incorporated into the vertical flare spreadsheet.

MPH.1.3.2 Vertical Flare Spreadsheet

With the product of the WTAQ program in a matrix of predicted drawdown at one foot intervals in both horizontal and vertical dimensions for the hypothetical vertical cross section, an EXCEL spreadsheet with additional Visual Basic programming was used to evaluate the vertical flare. The matrix of drawdown values was inserted into the spreadsheet to represent the propagation of drawdown from a production well after 30 days of operation. A mirror image of the matrix was used to represent the injection well water-level rise. The summation of the drawdown due to the production well and water-level rise represents the head change in each cell representing a square foot of the aquifer between the wells.

An arbitrary water-level elevation value of 90 feet was added to the water-level change in order to produce a "head" matrix for the cross section between the two wells. This head matrix then allowed calculation of both a horizontal and vertical ground-water velocity for each cell using the head in surrounding cells to calculate a gradient. When combined with the horizontal and vertical hydraulic conductivities of 1.0 ft/day and 0.085 ft/day, respectively, the horizontal and vertical Darcy velocities can be calculated.

MPH.1.3.2.1 Velocity Field

The velocity field for the simple well configuration is used to interpret vertical flare. Because the differential between horizontal and vertical hydraulic conductivity is large, the vertical velocity is reduced very quickly with small vertical distance from the completion interval. The

horizontal and vertical ground-water velocity tabulations in the two dimensional field are presented in Figure MPH.1-12. The tabulations are abbreviated to show only the lower portion of the aquifer where there is active injection and production. The vertical and horizontal velocities are presented in units of feet per day. The well completion is shown as the larger diameter interval in the schematic at each end of the section.

The direction of the vertical velocity is indicated by the sign with a positive value indicating upward flow and a negative value indicating downward flow. In close proximity to the injection well, the larger head values within the completion interval produce upward and downward flare. With increasing distance from the injection well, there is a gradual convergence from intervals above and below the completion interval to the completion interval. Near the production well, the vertical convergence to the completion interval becomes stronger.

The horizontal ground-water movement is from the injection well to the production well. The horizontal velocities are greatest near the injection and production wells because of the radial flow representation of the drawdown values produced by WTAQ. The radial flow calculation also results in a variable area represented by each column in the matrix. Each column can be viewed as one-half of a cylinder with a radius of the distance from the nearest of the two wells, and this makes the area proportional to the square of the radius. Hence, the calculation of composite flare is weighted to the square of the radius from the wells.

MPH.1.3.2.2 Flare Evaluation

The vertical flare is calculated as a ratio of the area (or volume) of the aquifer wherein there is a significant vertical velocity away from the completion interval to the actual completion interval. This area or volume is calculated as the thickness of cells in each column where the magnitude of the vertical velocity is significant multiplied by the fraction of the area/volume represented by each column in the matrix. Figure MPH.1-12 presents the vertical velocity matrix with a red boundary line indicating the 10 foot thick ore zone and cells above and below the ore zone where the velocity is 0.05 feet/day or greater away from the ore zone. Horizontal velocity is typically an order of magnitude or more larger than the vertical velocity and the threshold velocity boundary shown for a velocity is 0.50 feet/day or larger. The bounded area for horizontal velocity also includes the entire 10 foot thick ore zone, but does not include horizontal velocity greater than 0.50 feet/day where the vertical flow is convergent to the ore zone.

The proportional area/volume represented by each column increases with distance from the injection well or production well to a maximum at the midpoint between the injection and production wells. The column closest to the injection well represents only 0.043% of the area/volume included in the model, and each of the two columns bridging the midpoint between the wells represents 2.9% of the area/volume.

The number of cells included in each column that were within one or both of the bounded areas shown on Figure MPH.1-12 were summed and then multiplied by the fraction of the area/volume represented by the column. These products of cell counts and fractional area/volume were then summed and divided by the corresponded cell counts for the ore zone only. This ratio represents the estimated vertical flare for the specified configuration, and was calculated as 1.22. This is similar to the industry standard vertical flare of 1.25. Although there are necessary

simplifications and uncertainties involved in this simulation approach, the results are reasonable and consistent with vertical flare estimates from existing ISR operations.

MPH.1.4 Excursion Control and Retrieval

The potential for excursion was considered in a MODFLOW-2005 modeling scenario by adjusting modeling parameters to produce a temporary and local imbalance in wellfield operation. The imbalance involves either insufficient production rate or excess injection rate for a local area such that the local bleed rate is zero or actually negative representing more injection than production. Limiting this condition to a local area of a few wells is considered appropriate because a wider scale imbalance with insufficient bleed is unlikely given continuous monitoring of production and injection rates.

Simulation of retrieval of an excursion is essentially a reversal of the process that created the excursion. Increasing the effective bleed rate for a local area will increase the local drawdown and cause an expansion of the area of gradient reversal. Within this zone of gradient reversal, ground water will be flowing to the production wells and any ground water that has been impacted by mining fluids will be retrieved.

MPH.1.4.1 MODFLOW Modeling Changes

The MODFLOW-2005 modeling configuration described in Section MPH.1.2 was used for the simulation of excursion and retrieval. The model included a wellfield for the lowest ore zone and consisted of 88 production wells operating at a rate of 12.5 gpm, and 125 injection wells operating at a rate ranging from 5.2 to 12.7 gpm. There was a 3% bleed in the well field operation with a resulting net extraction stress of approximately 33 gpm.

In order to simulate a local imbalance, the extraction rate for the four southernmost production wells was adjusted for two separate simulations. The first simulation included operation of the wellfield in a balanced condition for 30 days, followed by 30 days of operation with reduced production rates for the four southernmost production wells to produce a local imbalance. This was in turn followed by a 30 day period with increased production in the four designated wells to affect retrieval and restore gradient reversal. The magnitude of rate changes (both decrease and increase) was 1.04 gpm for each of the four wells. This is approximately an 8% change in the well production rate for the four wells, but only resulted in a wellfield bleed rate range of 2.6 to 3.4% of total wellfield production. The second simulation used the same sequence of balanced, decreased production, and increased production from the wellfield, but utilized a 60 day period for each of the phases.

MPH.1.4.2 30 Day Excursion and Retrieval Simulation

The results of a MODFLOW-2005 simulation of 30 days of normal wellfield operation are presented in Figure MPH.1-13. The cone of depression around the wellfield is expanding, and on the west side of the southern end of the wellfield, the area of gradient reversal extends more than 400 feet from the wellfield. At the end of the initial 30 day period, the production rates were reduced for four wells on the southern end of the wellfield. The potentiometric surface after 30 days of operation with this local imbalance is presented in Figure MPH.1-14. The reduction of production rates for this simulation has resulted in loss of the gradient reversal and a

very flat potentiometric surface west of the southern end of the wellfield. The width of the zone where the gradient reversal is lost is more than 500 feet, and based on the very small ground-water gradient in this area, an excursion is possible but movement rates would be extremely slow. Based on the surface presented in Figure MPH.1-14, the potential excursion of mining fluids would also be spread over a width that is approaching the width of the interval where gradient reversal is lost. Figure MPH.1-15 presents the potentiometric surface after an additional 30 day stress period with increased well production rates. The gradient reversal has been regained and extends approximately 400 feet to the west of the wellfield. This indicates that retrieval will be effective, but the gradient reversal is still relatively mild and the rates of both excursion and retrieval will be slow.

MPH.1.4.3 60 Day Excursion and Retrieval Simulation

The second simulation used a period of 60 days for normal wellfield operation followed by 60 days with a local wellfield imbalance with a subsequent 60 days of overproduction in the affected area. After 60 days of balanced wellfield operation, there is distinct gradient reversal west of the wellfield. After an additional 60 days with local imbalance the potentiometric surface shown in Figure MPH.1-16 indicates that gradient reversal has been lost and that a very flat potentiometric surface extends for approximately 400 feet west of the southern end of the wellfield. When the production rates are increased to retrieve any mining fluid impacted ground water moving to the west of the wellfield, gradient reversal is regained within 60 days as shown in Figure MPH.1-17. The zone of restored gradient reversal extends beyond 500 feet from the edge of the wellfield.

MPH.1.4.4 Discussion of Excursion Model Results

The excursion and retrieval simulations indicate that development of excursion conditions under moderately imbalanced wellfield conditions will be relatively slow, and that regaining gradient reversal will also be a slow process. This is attributed in large part to the expected unconfined conditions for the Hank wellfield areas. The large volume of ground water released or stored with a unit change in head greatly extends the time frame for significant gradient changes. The width of the zone over which gradient reversal is lost is also relatively wide at approximately 500 feet. Mining fluids that are migrating away from the active wellfield will be spread over a width that is approaching the width of the area where gradient reversal is lost, and there will be additional flare as the impacted ground water moves away from the wellfield. This indicates that the anticipated monitoring ring well spacing of 500 feet will be sufficient to detect potential excursions.

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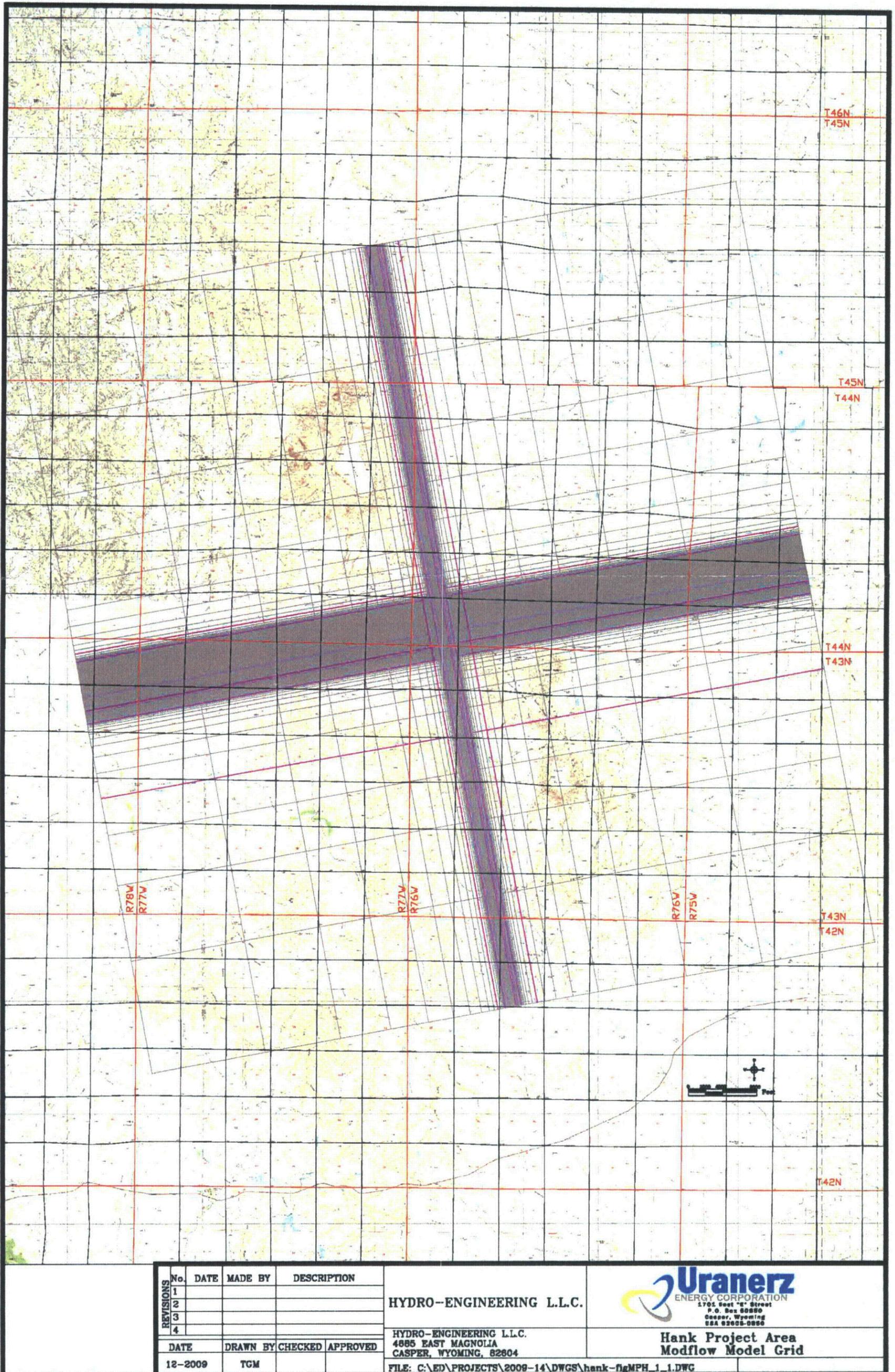
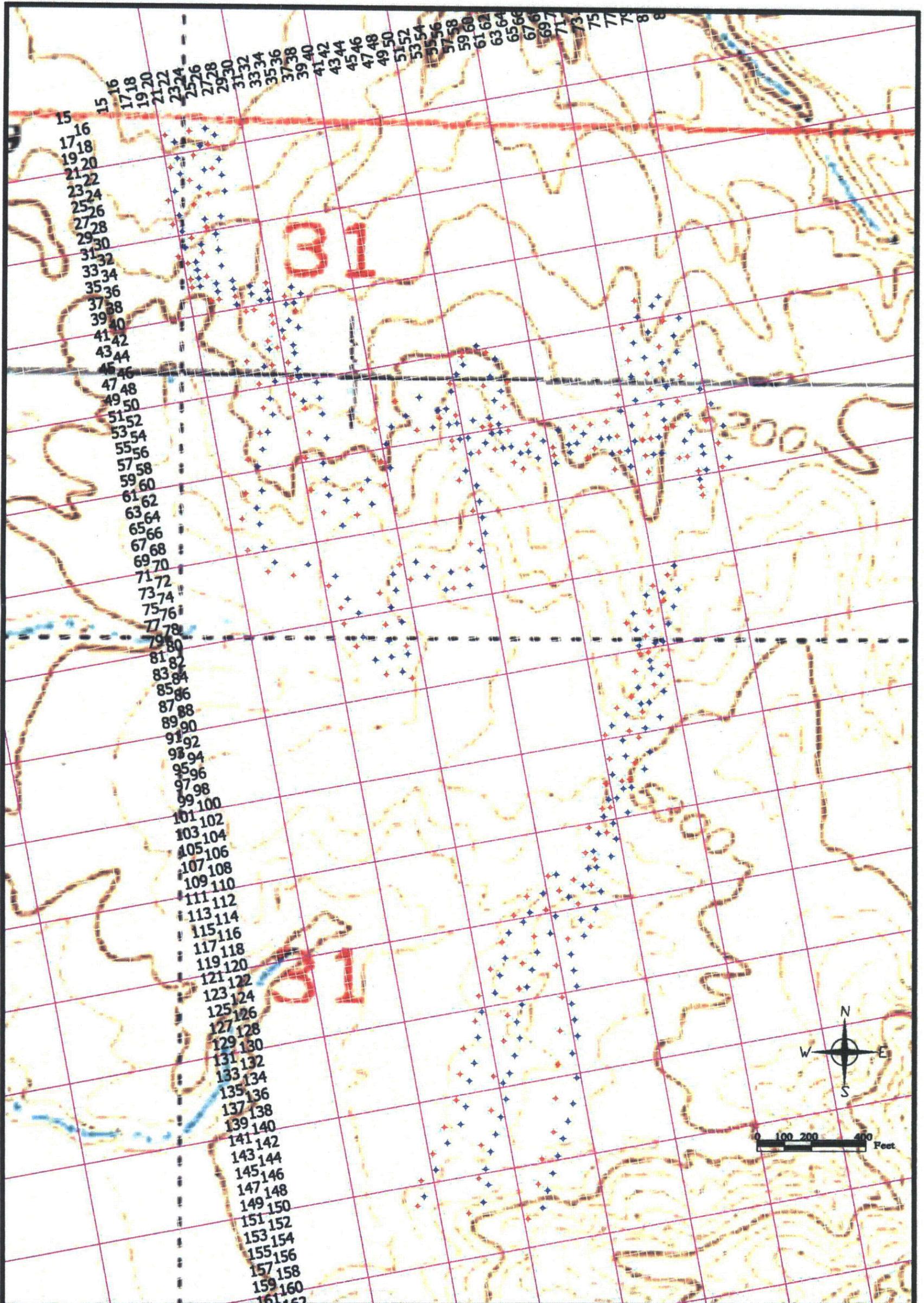
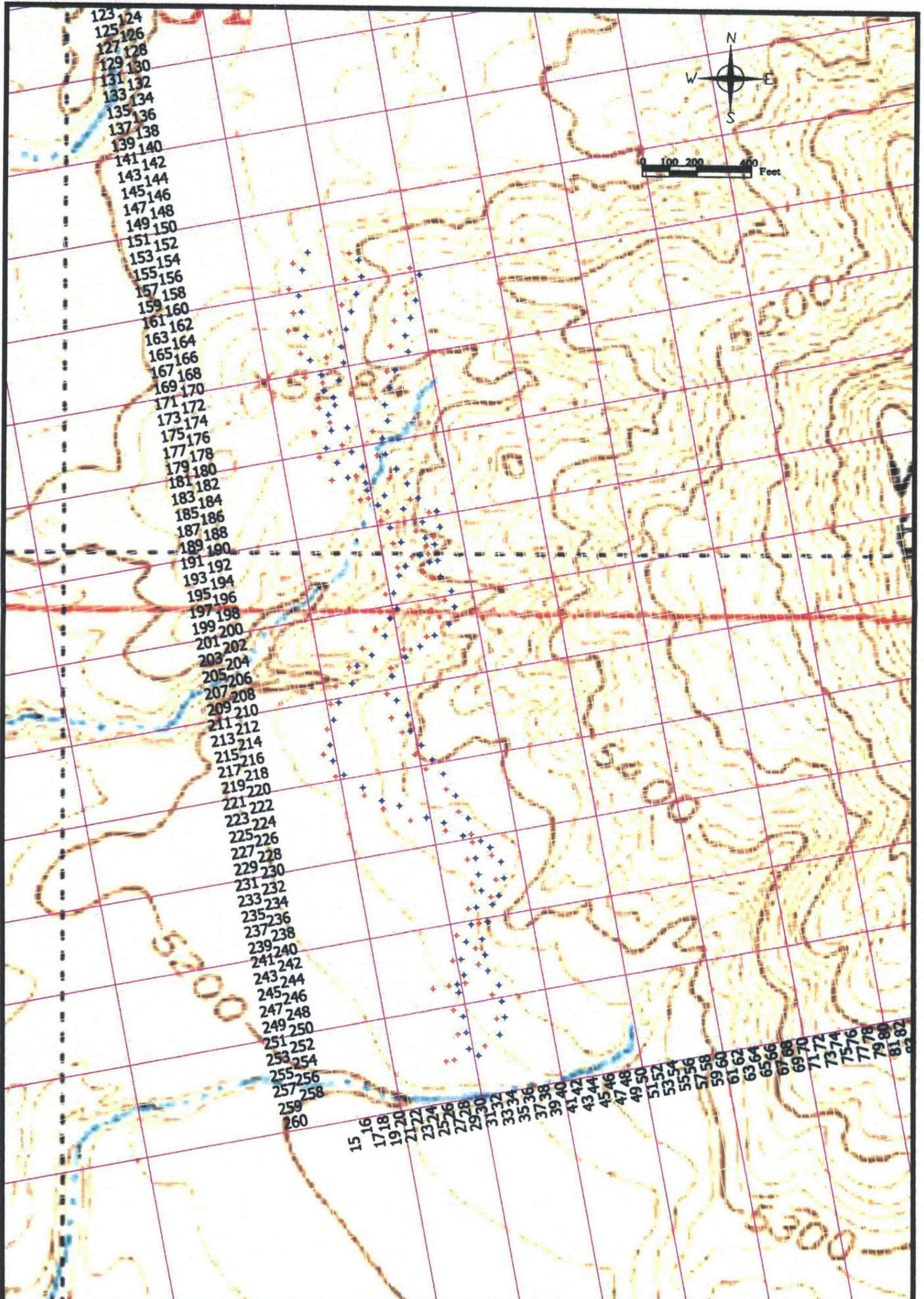


Figure MPH.1-1. Hank Project Area Modflow Model Grid



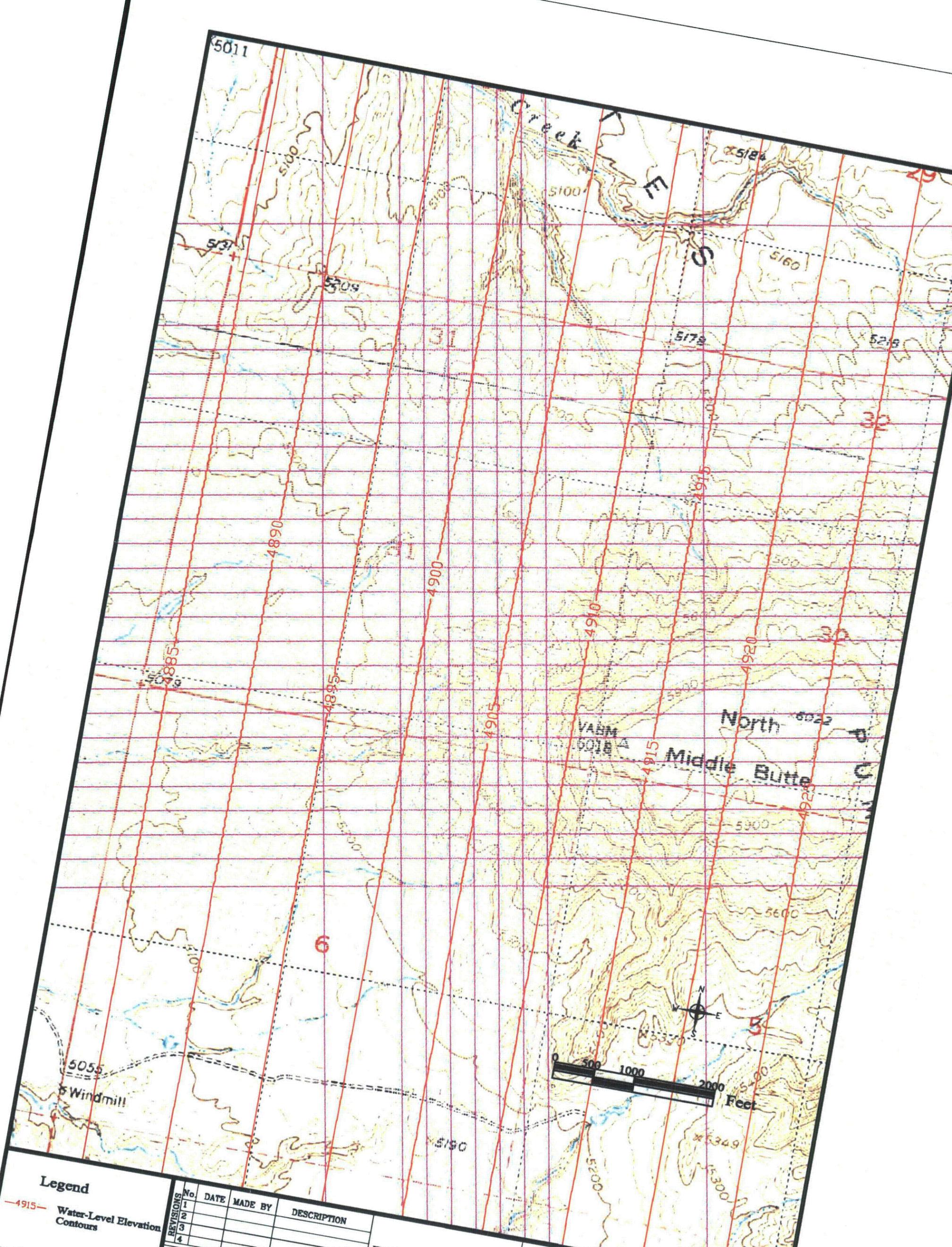
Legend Injection Well Extraction Well	<table border="1"> <thead> <tr> <th>REVISIONS</th> <th>No.</th> <th>DATE</th> <th>MADE BY</th> <th>DESCRIPTION</th> </tr> </thead> <tbody> <tr><td>1</td><td></td><td></td><td></td><td></td></tr> <tr><td>2</td><td></td><td></td><td></td><td></td></tr> <tr><td>3</td><td></td><td></td><td></td><td></td></tr> <tr><td>4</td><td></td><td></td><td></td><td></td></tr> </tbody> </table>	REVISIONS	No.	DATE	MADE BY	DESCRIPTION	1					2					3					4					HYDRO-ENGINEERING L.L.C. HYDRO-ENGINEERING L.L.C. 4885 EAST MAGNOLIA CASPER, WYOMING, 82604 FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG	 Hank Wellfield #1 Model Configuration
	REVISIONS	No.	DATE	MADE BY	DESCRIPTION																							
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<table border="1"> <thead> <tr> <th>DATE</th> <th>DRAWN BY</th> <th>CHECKED</th> <th>APPROVED</th> </tr> </thead> <tbody> <tr> <td>12-2009</td> <td>TGM</td> <td></td> <td></td> </tr> </tbody> </table>	DATE	DRAWN BY	CHECKED	APPROVED	12-2009	TGM																						
DATE	DRAWN BY	CHECKED	APPROVED																									
12-2009	TGM																											

Figure MPH.1-2. Hank Wellfield #1 Model Configuration



Legend ⊕ Injection Well ⊕ Extraction Well	REVISIONS	No.	DATE	MADE BY	DESCRIPTION
	1				
	2				
	3				
	DATE	DRAWN BY	CHECKED	APPROVED	
	12-2009	TGM			
HYDRO-ENGINEERING L.L.C.		 ENERGY CORPORATION 1702 East "E" Street P.O. Box 60880 Casper, Wyoming USA 82902-0880			
HYDRO-ENGINEERING L.L.C. 4685 EAST MAGNOLIA CASPER, WYOMING, 82604		Hank Wellfield #2 Model Configuration			
FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG					

Figure MPH.1-3. Hank Wellfield #2 Model Configuration



Legend

—4915— Water-Level Elevation Contours

REVISIONS	No.	DATE	MADE BY	DESCRIPTION
	1			
	2			
	3			
	4			

DATE	DRAWN BY	CHECKED	APPROVED
12-2009	TGM		

HYDRO-ENGINEERING L.L.C.
 HYDRO-ENGINEERING L.L.C.
 4885 EAST MAGNOLIA
 CASPER, WYOMING, 82804
 FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG



Initial Hank Area Potentiometric Surface

re MPH.1-4. Initial Hank Area Potentiometric Surface

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January 2010

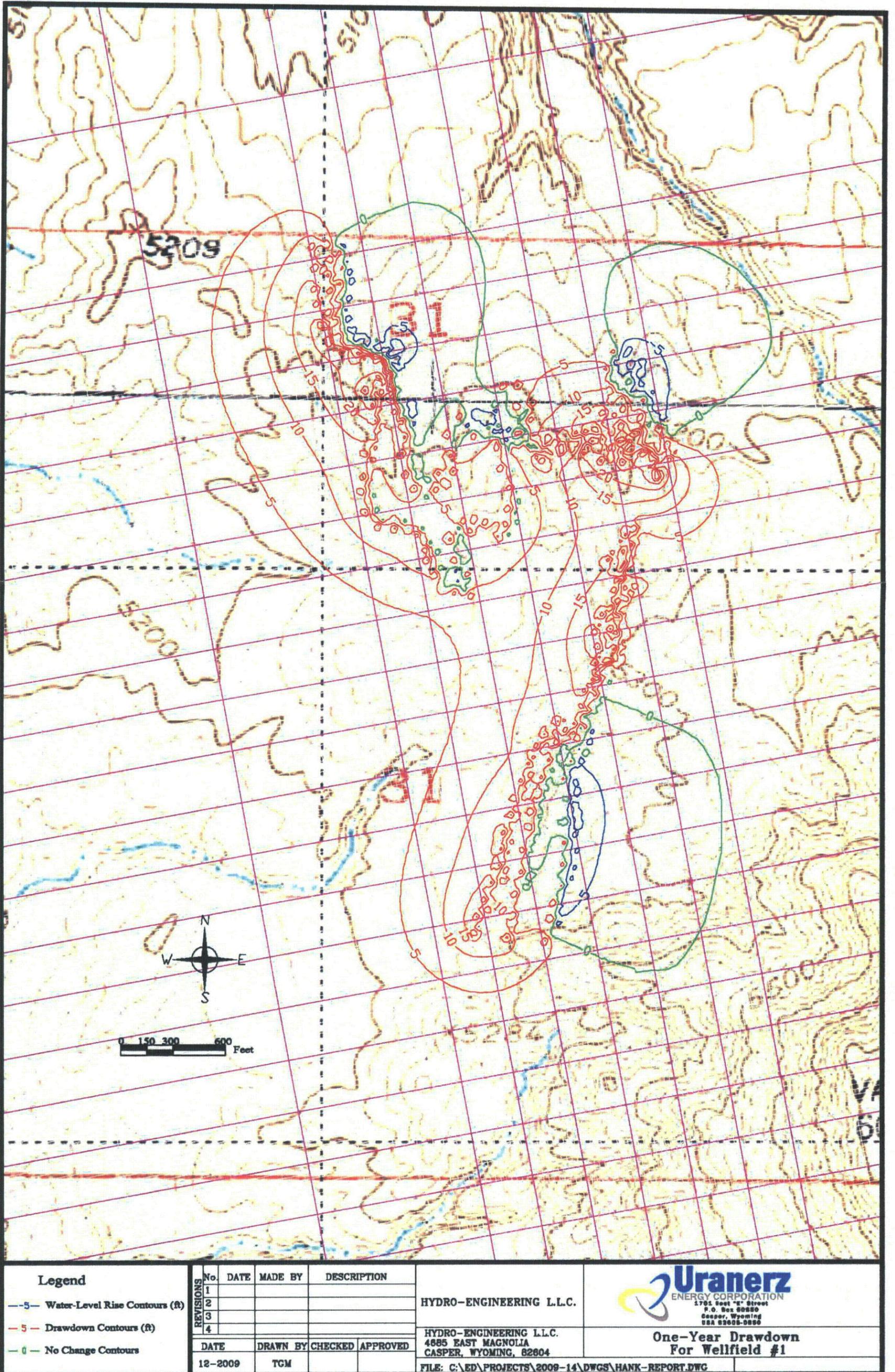
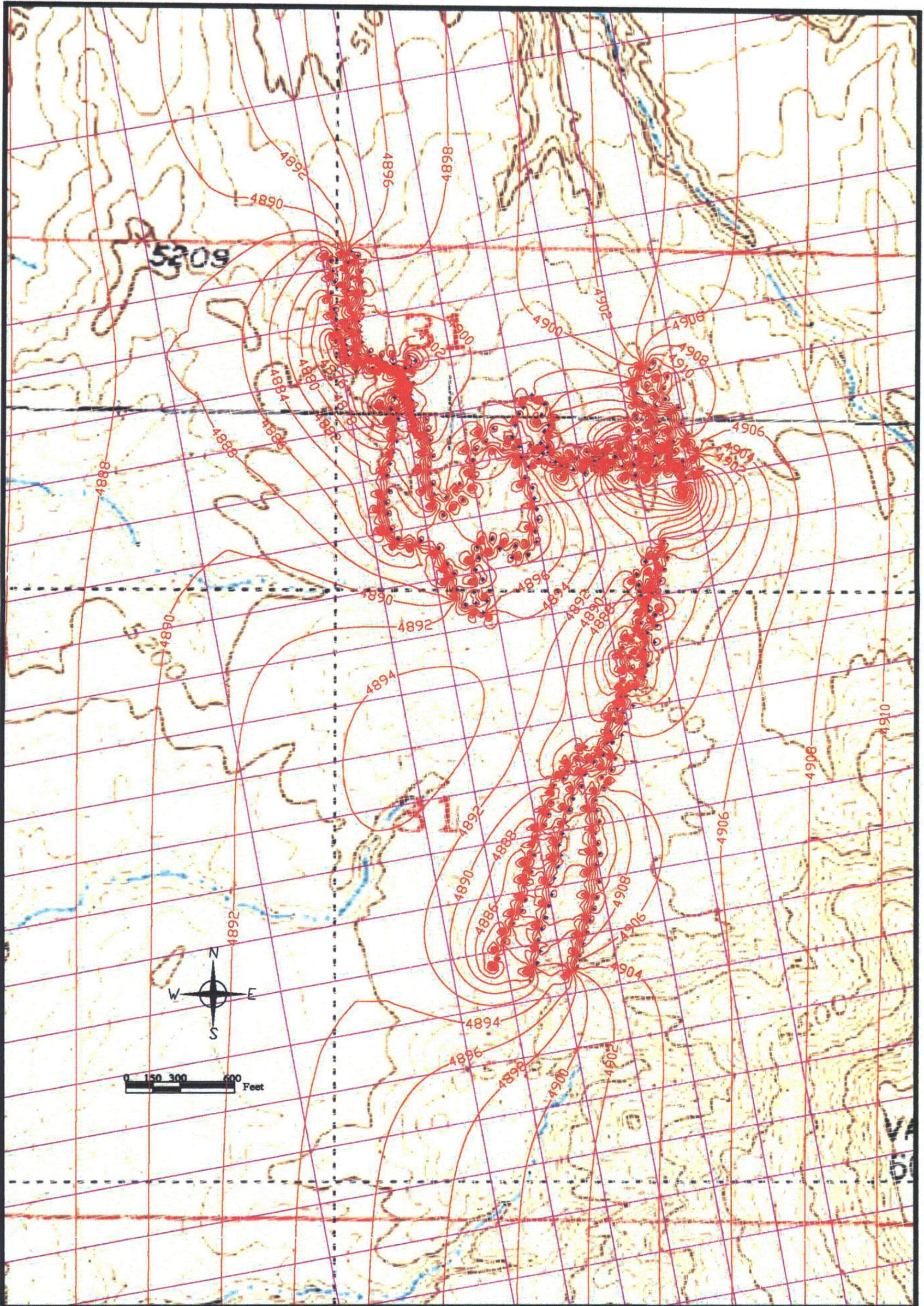
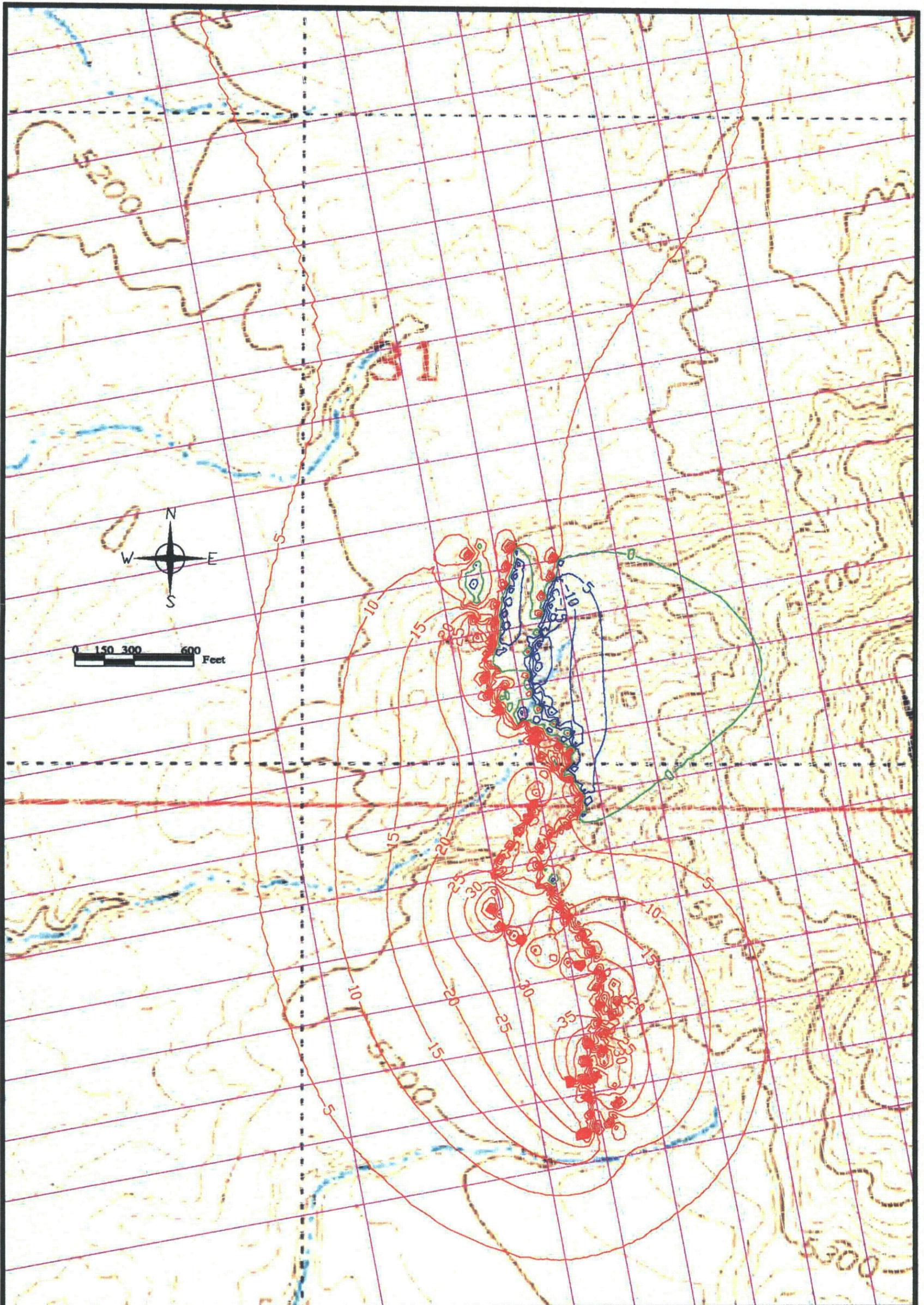


Figure MPH.1-5. One-Year Drawdown for Wellfield #1



Legend —4915— Water-Level Elevation Contours	REVISIONS No. DATE MADE BY DESCRIPTION	HYDRO-ENGINEERING L.L.C. HYDRO-ENGINEERING L.L.C. 4685 EAST MAGNOLIA CASPER, WYOMING, 82804 FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG	 Uranerz ENERGY CORPORATION 1704 East 'G' Street P.O. Box 60860 Casper, Wyoming USA 82608-0860	
	DATE DRAWN BY CHECKED APPROVED			Potentiometric Surface For Wellfield #1 After One Year of Mining
	11-2009 TGM			
	(Empty cells)			

Figure MPH.1-6. Potentiometric Surface for Wellfield #1 After One Year of Mining

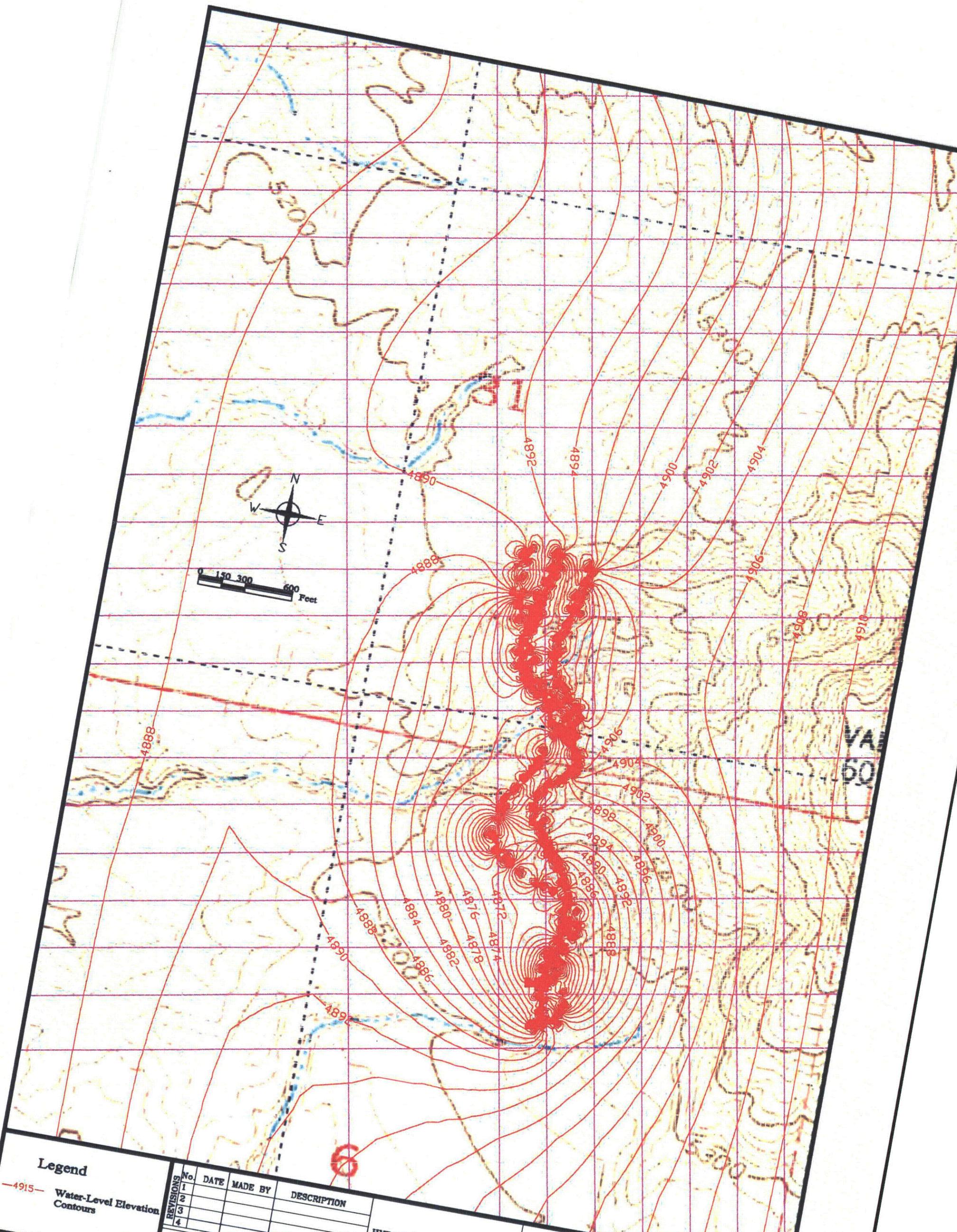


Legend		No.	DATE	MADE BY	DESCRIPTION	
---5---	Water-Level Rise Contours (ft)	1				
---5---	Drawdown Contours (ft)	2				
---	No Change Contours	3				
		4				
			DATE	DRAWN BY	CHECKED	APPROVED
			12-2009	TGM		

HYDRO-ENGINEERING L.L.C.	 Uranerz ENERGY CORPORATION 1705 East "H" Street P.O. Box 50850 Casper, Wyoming USA 82502-0850
HYDRO-ENGINEERING L.L.C. 4885 EAST MAGNOLIA CASPER, WYOMING, 82604	
FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG	

Drawdown at End of Wellfield #2 Operation	
--	--

Figure MPH.1-7. Drawdown at End of Wellfield #2 Operation



Legend

—4915— Water-Level Elevation Contours

REVISIONS	No.	DATE	MADE BY	DESCRIPTION
	1			
	2			
	3			
	4			

DATE	DRAWN BY	CHECKED	APPROVED
12-2009	TGM		

HYDRO-ENGINEERING L.L.C.

HYDRO-ENGINEERING L.L.C.
4885 EAST MAGNOLIA
CASPER, WYOMING, 82604

FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG



Potentiometric Surface At End of Wellfield #2 Operation

MPH.1-8. Potentiometric Surface at End of Wellfield #2 Operation

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January 2010

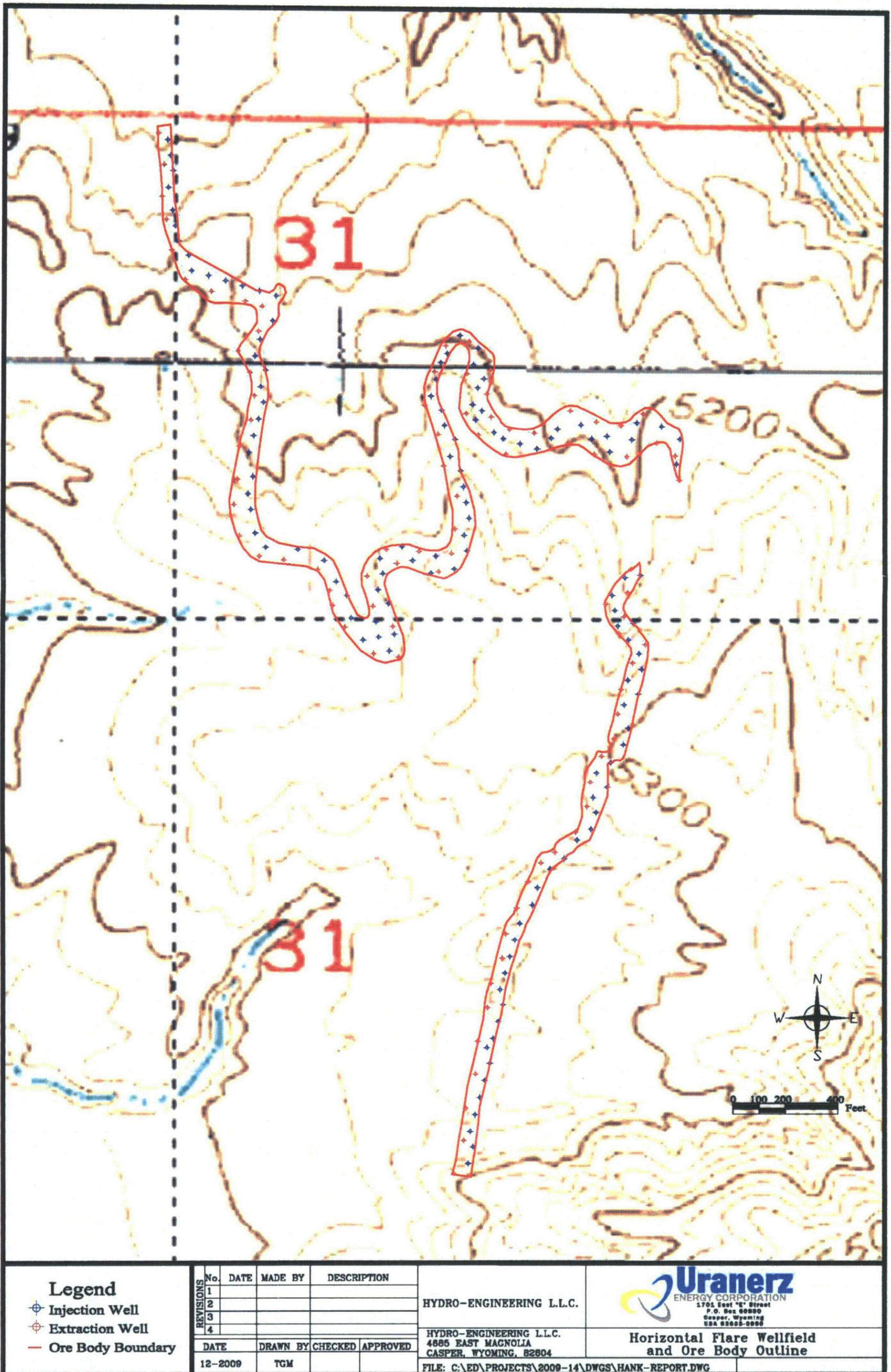
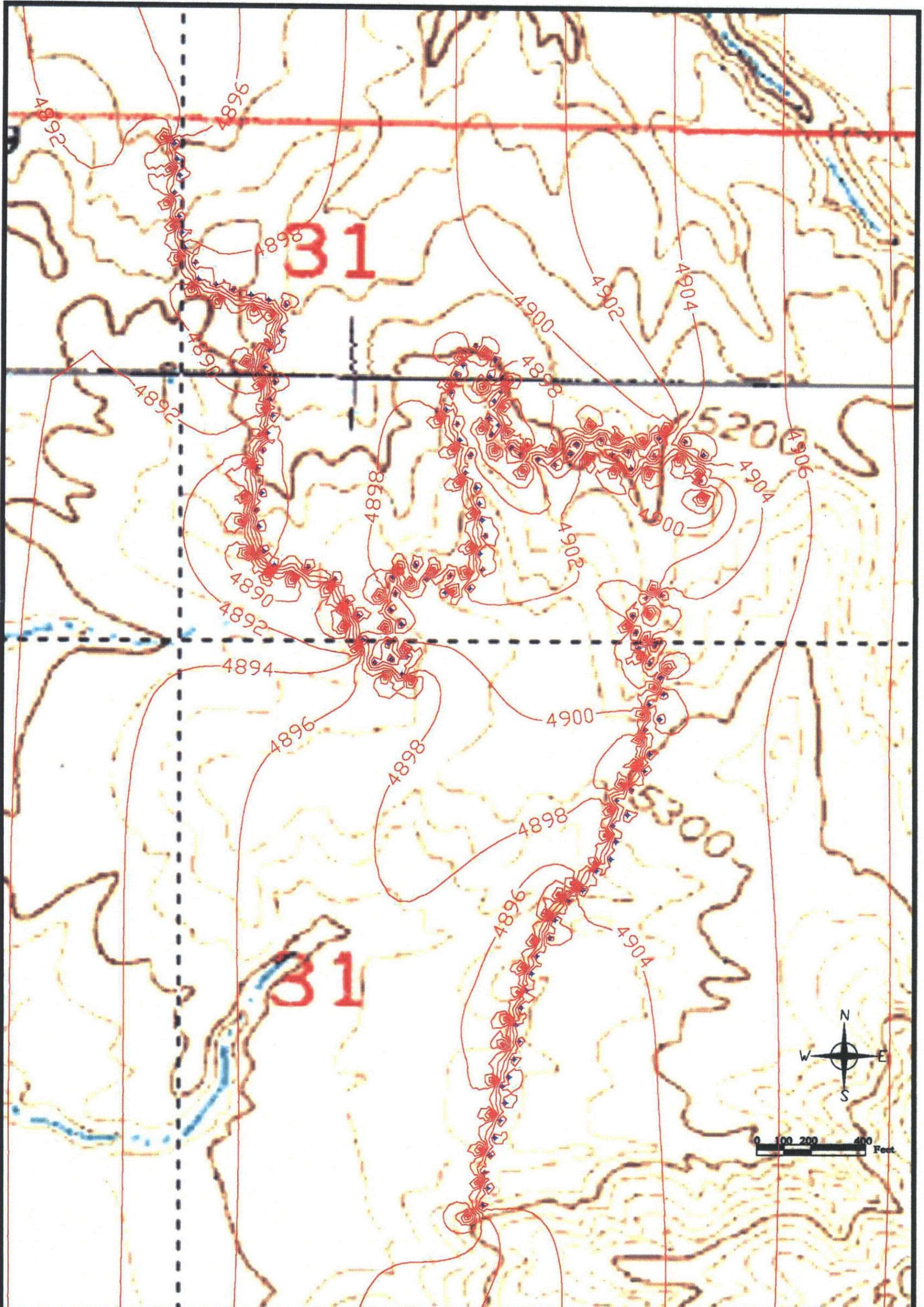


Figure MPH.1-9. Horizontal Flare Wellfield and Ore Body Outline



Legend Injection Well Extraction Well Water-Level Elevation Contours	<table border="1"> <thead> <tr> <th>REVISIONS</th> <th>No.</th> <th>DATE</th> <th>MADE BY</th> <th>DESCRIPTION</th> </tr> </thead> <tbody> <tr><td>1</td><td></td><td></td><td></td><td></td></tr> <tr><td>2</td><td></td><td></td><td></td><td></td></tr> <tr><td>3</td><td></td><td></td><td></td><td></td></tr> <tr><td>4</td><td></td><td></td><td></td><td></td></tr> </tbody> </table>	REVISIONS	No.	DATE	MADE BY	DESCRIPTION	1					2					3					4					<table border="1"> <thead> <tr> <th>DATE</th> <th>DRAWN BY</th> <th>CHECKED</th> <th>APPROVED</th> </tr> </thead> <tbody> <tr> <td>12-2009</td> <td>TCM</td> <td></td> <td></td> </tr> </tbody> </table>	DATE	DRAWN BY	CHECKED	APPROVED	12-2009	TCM			HYDRO-ENGINEERING L.L.C. HYDRO-ENGINEERING L.L.C. 4885 EAST MAGNOLIA CASPER, WYOMING, 82804 FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG	 URANERZ ENERGY CORPORATION 1701 East "G" Street P.O. Box 68890 Casper, Wyoming USA 82608-0890
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DATE	DRAWN BY	CHECKED	APPROVED																																		
12-2009	TCM																																				
			Predicted Potentiometric Surface after 120 Days of Operation																																		

Figure MPH.1-10. Predicted Potentiometric Surface after 120 Days of Operation

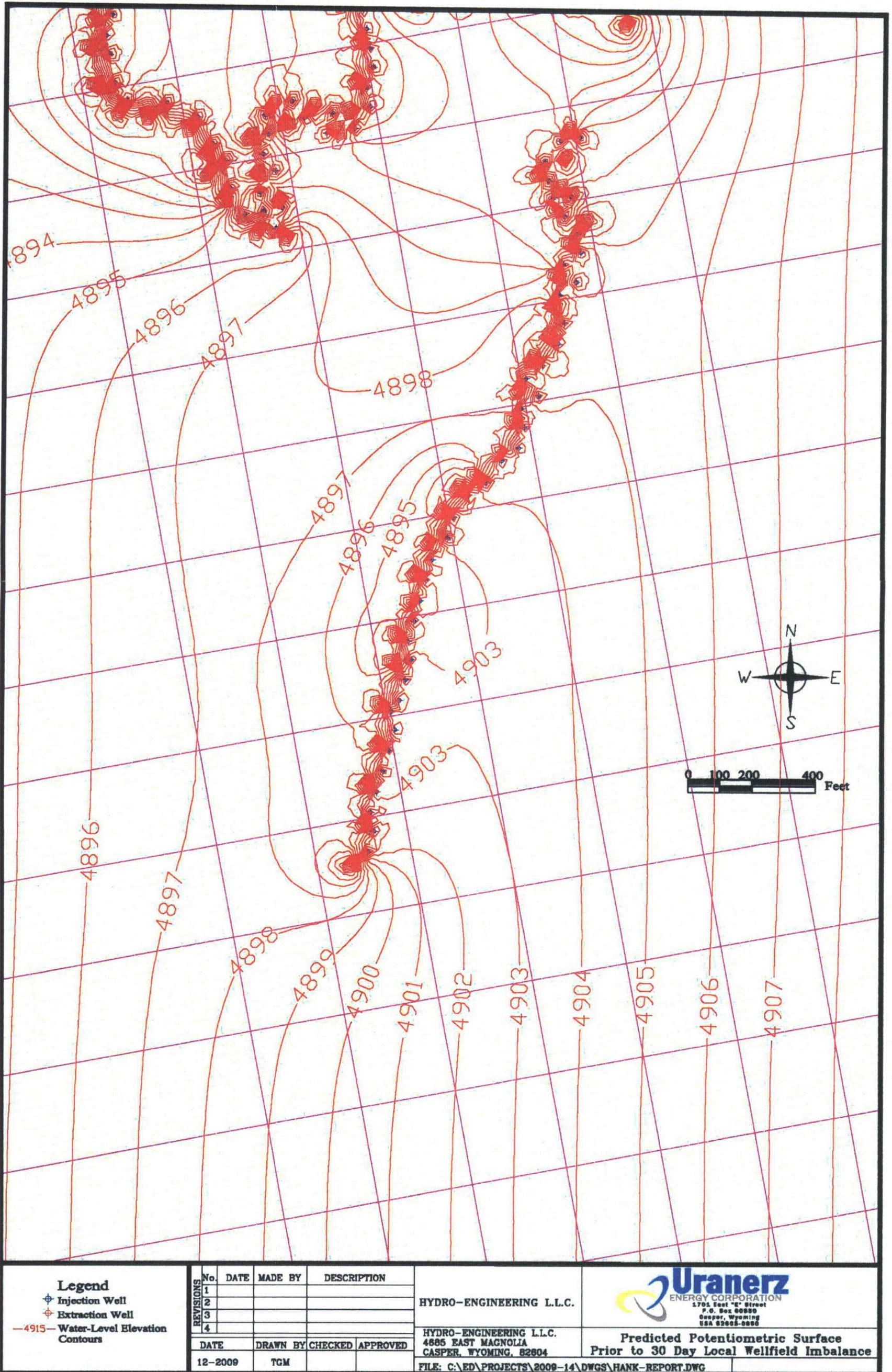
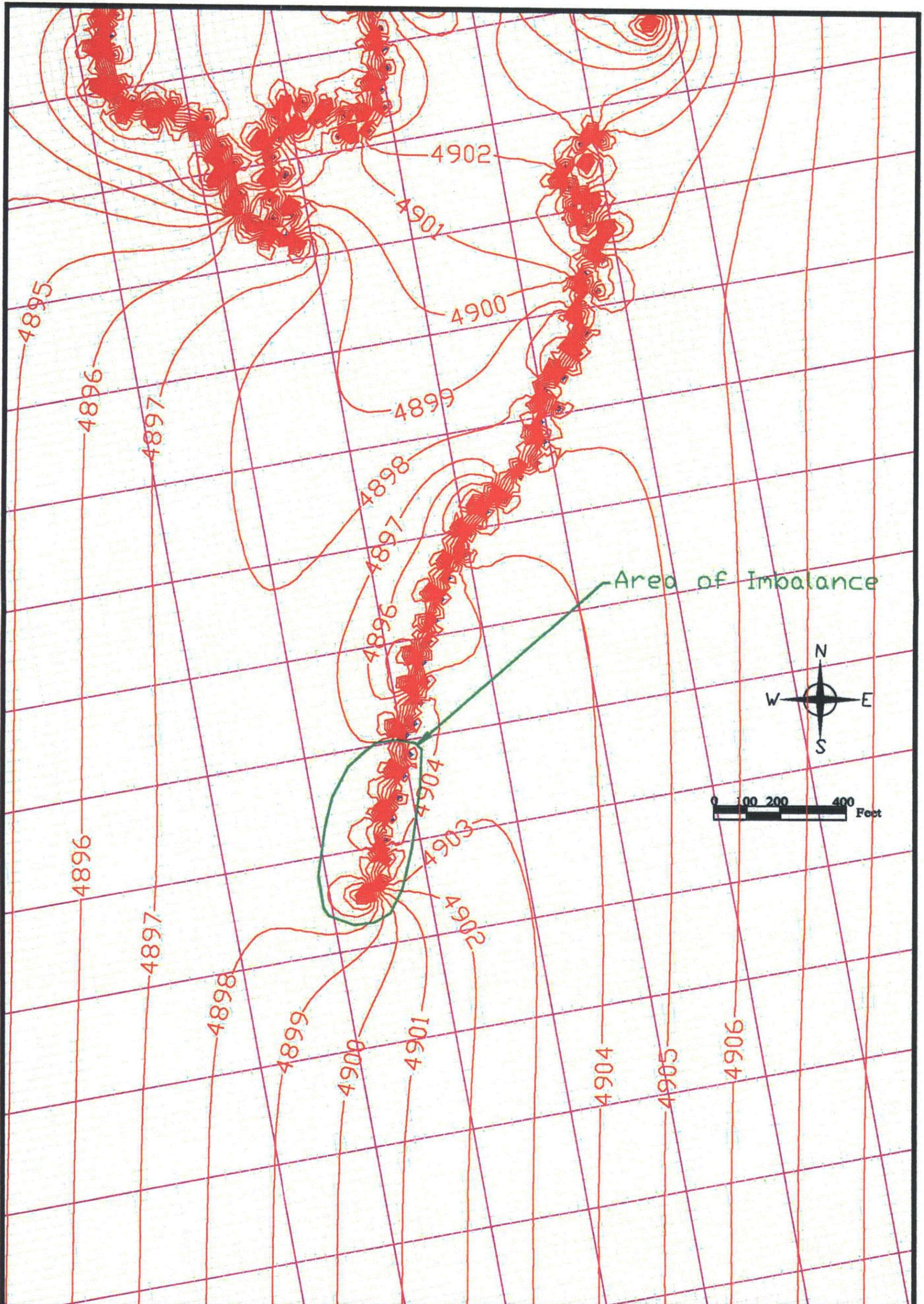
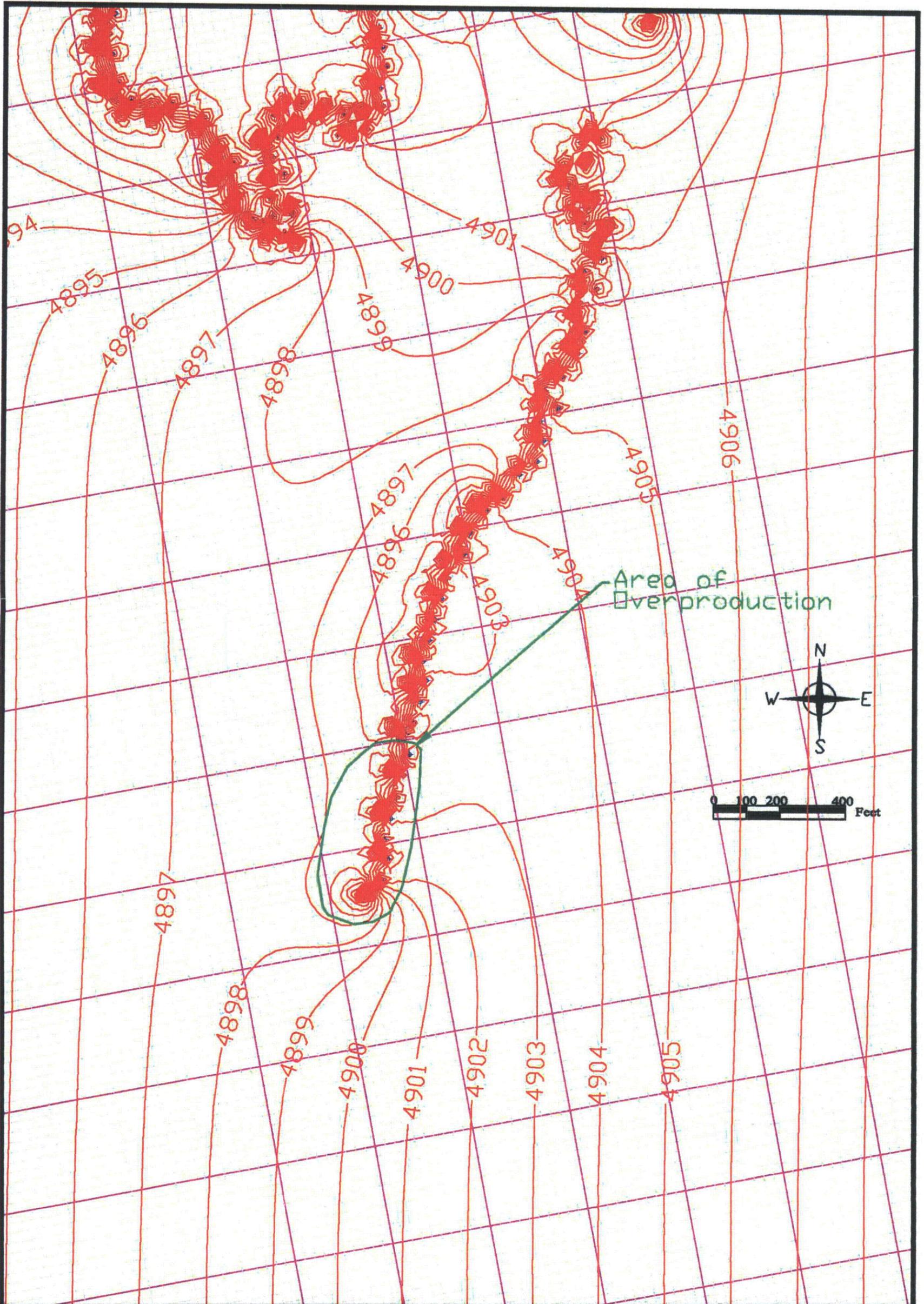


Figure MPH.1-13. Predicted Potentiometric Surface Prior to 30 Day Local Wellfield Imbalance



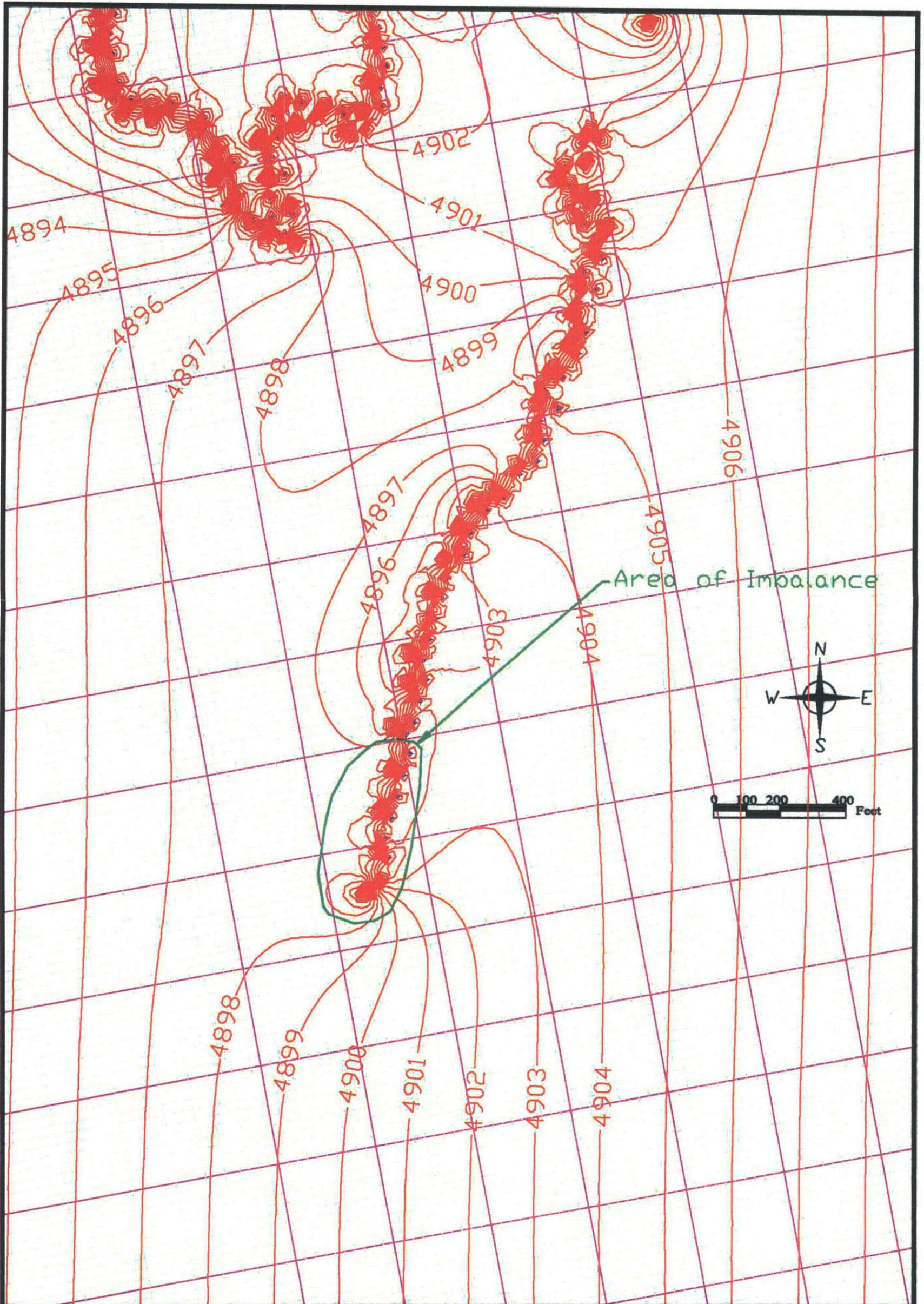
Legend ⊕ Injection Well ⊖ Extraction Well -4915- Water-Level Elevation Contours	REVISIONS	No.	DATE	MADE BY	DESCRIPTION	HYDRO-ENGINEERING L.L.C. HYDRO-ENGINEERING L.L.C. 4885 EAST MAGNOLIA CASPER, WYOMING, 82604 FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG	 Uranerz ENERGY CORPORATION 1701 East "A" Street P.O. Box 60820 Casper, Wyoming USA 82608-0820
	1						
	2						
	3						
DATE	DRAWN BY	CHECKED	APPROVED				
1-2010	TGM						

Figure MPH.1-14. Predicted Potentiometric Surface After 30 Day Local Wellfield Imbalance



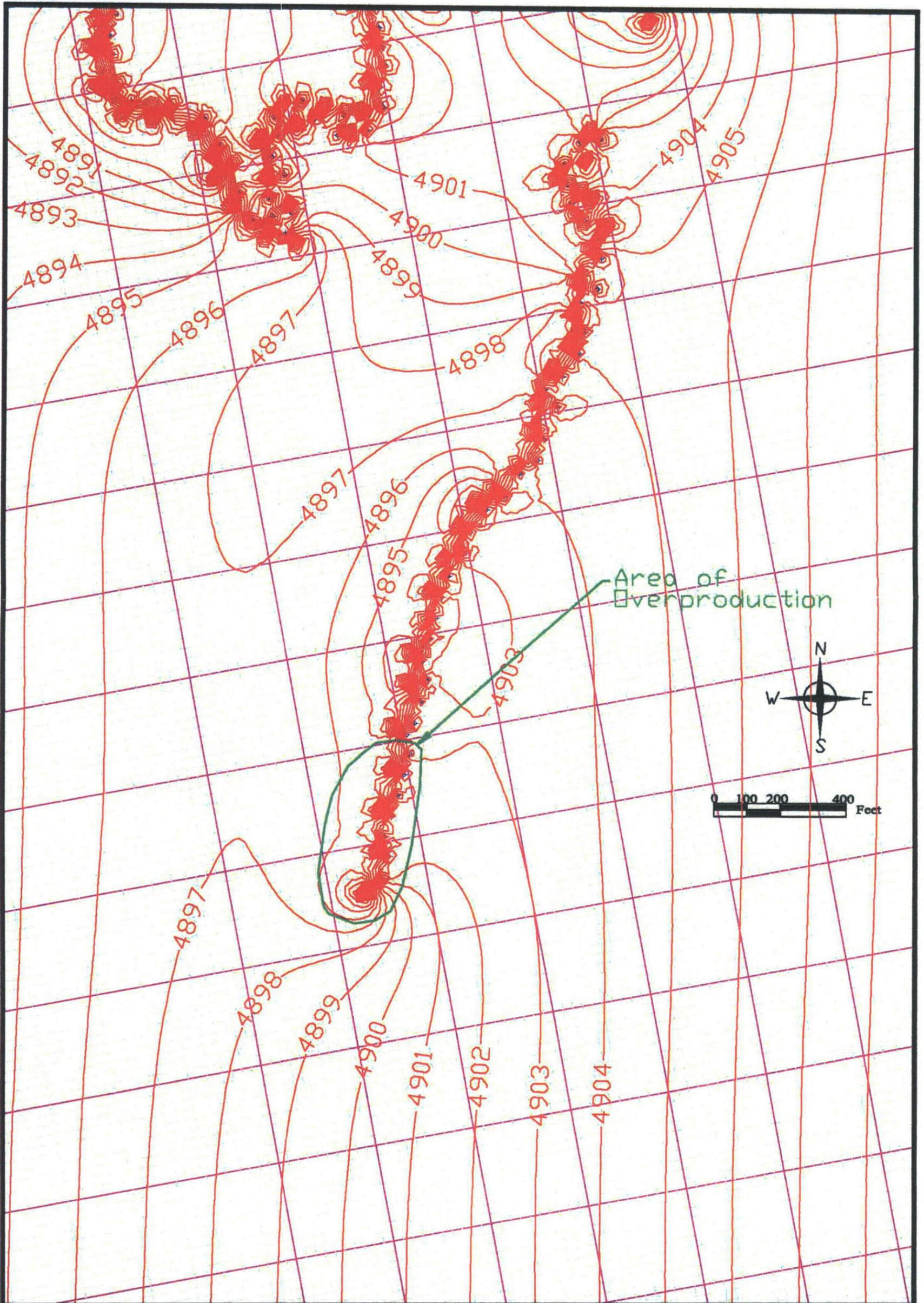
Legend ◆ Injection Well ◇ Extraction Well —4915— Water-Level Elevation Contours	REVISIONS No. 1 DATE MADE BY DESCRIPTION	HYDRO-ENGINEERING L.L.C. HYDRO-ENGINEERING L.L.C. 4685 EAST MAGNOLIA CASPER, WYOMING, 82604 FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG	 ENERGY CORPORATION 1701 East "E" Street P.O. Box 88820 Casper, Wyoming USA 82608-8880
	DATE 1-2010 DRAWN BY TGM CHECKED APPROVED		

Figure MPH.1-15. Predicted Potentiometric Surface After 30 Days with Increased Production Rates



Legend Injection Well Extraction Well 4915 Water-Level Elevation Contours	REVISIONS <table border="1"> <tr> <th>No.</th> <th>DATE</th> <th>MADE BY</th> <th>DESCRIPTION</th> </tr> <tr> <td>1</td> <td></td> <td></td> <td></td> </tr> <tr> <td>2</td> <td></td> <td></td> <td></td> </tr> <tr> <td>3</td> <td></td> <td></td> <td></td> </tr> <tr> <td>4</td> <td></td> <td></td> <td></td> </tr> </table>	No.	DATE	MADE BY	DESCRIPTION	1				2				3				4				HYDRO-ENGINEERING L.L.C. HYDRO-ENGINEERING L.L.C. 4885 EAST MAGNOLIA CASPER, WYOMING, 82804 FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG		 ENERGY CORPORATION 1704 East 1st Street P.O. Box 80880 Casper, Wyoming USA 82608-0880
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<table border="1"> <tr> <th>DATE</th> <th>DRAWN BY</th> <th>CHECKED</th> <th>APPROVED</th> </tr> <tr> <td>1-2010</td> <td>TGM</td> <td></td> <td></td> </tr> </table>	DATE	DRAWN BY	CHECKED	APPROVED	1-2010	TGM			Predicted Potentiometric Surface After 60 Days with Local Wellfield Imbalance															
DATE	DRAWN BY	CHECKED	APPROVED																					
1-2010	TGM																							

Figure MPH.1-16. Predicted Potentiometric Surface After 60 Day Local Wellfield Imbalance



Legend ◆ Injection Well ◊ Extraction Well —4915— Water-Level Elevation Contours	REVISIONS No. DATE MADE BY DESCRIPTION 1 2 3 4	HYDRO-ENGINEERING L.L.C. 4885 EAST MAGNOLIA CASPER, WYOMING, 82604 FILE: C:\ED\PROJECTS\2009-14\DWGS\HANK-REPORT.DWG		 1701 East "B" Street P.O. Box 20820 Casper, Wyoming USA 82602-0820	
	DATE DRAWN BY CHECKED APPROVED 1-2010 TGM	Predicted Potentiometric Surface After 60 Days with Increased Production Rates			

Figure MPH.1-17. Predicted Potentiometric Surface After 60 Days with Increased Production Rates

ADDENDUM 6B:
NICHOLS RANCH ISR PROJECT SURETY ESTIMATE

June 2010



Surety Estimate
First Year of Operation
Nichols Ranch In-Situ Recovery Project
Uranerz Energy Corporation

Total Restoration and Reclamation Cost Estimates

No.	Cost Item	Cost
1	GROUNDWATER RESTORATION COST	\$2,955,240
2a	PLANT EQUIPMENT REMOVAL AND DISPOSAL COST	\$213,150
2b	BUILDING DEMOLITION AND DISPOSAL COST	\$911,292
3	SOIL REMOVAL & DISPOSAL COST	\$234,300
4	TOTAL WELL ABANDONMENT COST	\$319,234
5	WELLFIELD EQUIPMENT REMOVAL & DISPOSAL COST	\$335,643
6	TOPSOIL REPLACEMENT & REVEGETATION COST	\$313,978
7	MISCELLANEOUS RECLAMATION COST	\$3,335
	Subtotal Restoration and Reclamation Cost Estimate	\$5,286,171
	Subtotal	\$5,286,171
	Administration, Overhead and Contingency (25%)	\$1,321,543
	Total	\$6,607,714
	TOTAL CALCULATED IN 2010 DOLLARS	\$6,607,714

US DEPT. of COMMERCE PRODUCER PRICE INDEX ADJUSTMENT 2007 to 2010 **5.78%**
US DEPT. of COMMERCE PRODUCER PRICE INDEX ADJUSTMENT 2009 to 2010 **5.31%**

Note: Unit costs presented in the various worksheets in this estimate originally incorporated 2007 values. Subsequently, where available, unit costs from the latest version (2009) of WDEQ Guideline 12, App K were included. All unit costs, whether 2007 or 2009, were adjusted by the Producer Price Index factor for the respective data. Note that PPI was used rather than Consumer Price Index as identified during WDEQ review, as changes in producer prices are more appropriate for industrial applications than a consumer price index.

**Surety Estimate
First Year of Operation
Nichols Ranch ISR Project
Uranerz Energy Corporation**

**Worksheet 1, No. 1 --
GROUNDWATER RESTORATION**

Cost Item	Mining Unit		Notes
	Nichols #1		
Technical Assumptions			
Wellfield Area (Ft ²)	1,551,650		
Wellfield Area (Acres)	35.62		66.21 Ac at Nichols, 45.56 at Hank per URZ permit
Affected Ore Zone Area (Ft ²)	1,551,650		
Avg Completed Thickness (Ft)	7.27		
Factor for Flare	1.45		
Affected Volume:	16,356,717		
Porosity	0.3		
Gallons per Cubic Foot	7.48		
Gallon per Pore Volume	36,704,474		
Number of Wells in Unit(s)			
Recovery Wells	233		
Injection Wells	259		
Monitor Wells	33		
Average Well Spacing (Ft)	100		
Average Well Depth (Ft)	550		
I Groundwater Sweep			
A. Plant & Office			
Operating Assumptions:			
Flowrate (gpm)	50		
PV's Required	1.00		
Total Gallons for Treatment	36,704,474		
Total Kgals for Treatment	36,704		
Cost Assumptions:			
Power			
Avg Connected Hp	15		
Kwh's/Hp	0.9325		
\$/Kwh	0.05		\$.02 plus demand charges per quote
Gallons per Minute	50		
Gallons per Hour	3000		
Cost per Hour	\$0.74		
Cost per Kgal (\$)	\$0.247		
Chemicals			
Barium Chloride (\$/Kgals)	\$0.043		Costs from operating ISR facility experience (Cogema)
Antiscalant (\$/Kgals)	\$0.000		Costs from operating ISR facility experience (Cogema)
Elution (\$/Kgals)	\$0.105		Costs from operating ISR facility experience (Cogema)
Repair & Maintenance (\$/Kgals)	\$0.065		Costs from operating ISR facility experience (Cogema)
Analysis (\$/Kgals)	\$0.173		Costs from operating ISR facility experience (Cogema)
Total Cost per Kgal	\$0.63		
Total Treatment Cost	\$23,223		
Utilities			
Power (\$/Month)	1,904		
Propane (\$/Month)	846		
Time for Treatment			
Minutes for Treatment	734,089		
Hours for Treatment	12,235		
Days for Treatment	510		
Average Days per Month	30		
Months for Treatment	17.0		
Years for Treatment	1.42		
Utilities Cost (\$)	\$46,735		
TOTAL PLANT & OFFICE COST	\$69,958		
B. WELLFIELD			
Cost Assumptions:			
Power			
Avg Flow/Pump (gpm)	1		
Avg Hp/Pump	1.5		
Avg # of Pumps Required	50		
Avg Connected Hp	75		
Kwh's/Hp	0.9325		
\$/Kwh	0.05289		
Gallons per Minute	50		
Gallons per Hour	3000		
Costs per Hour (\$)	\$3.70		
Costs per Gallon (\$)	\$0.0012		
Costs per Kgal (\$)	\$1.23		
Repair & Maintenance (\$/Kgals)	\$0.017		
Total Cost per Kgal	\$1.250		
TOTAL WELLFIELD COST	\$45,878		
TOTAL GROUNDWATER SWEEP COST	\$115,836		

Surety Estimate
 First Year of Operation
 Nichols Ranch ISR Project
 Uranerz Energy Corporation

Worksheet 1, No. II
 GROUNDWATER RESTORATION

Cost Item	Mining Unit Nichols #1	Notes
II REVERSE OSMOSIS		
A. PLANT & OFFICE		
Operating Assumptions:		
Flowrate (gpm)	50	
PV's Required	6.00	
Total Gallons for Treatment	220,226,842	
Total Kgals for Treatment	220,227	
Feed to RO (gpm)	50	
Permeate Flow (gpm)	40	
Brine Flow (gpm)	10	
Average RO Recovery	80%	
Cost Assumptions:		
Power		
Avg Connected Hp	20	
kWh/Hp	0.9325	
\$/Kwh	0.05289	\$.02 plus demand charges per quote
Gallons per Minute	50	
Gallons per Hour	3000	
Cost per Hour (\$)	\$0.99	
Cost per Gallon (\$)	\$0.0003	
Cost per Kgal (\$)	\$0.33	
Chemicals		
Sulfuric Acid (\$/Kgals)	\$0.080	Costs from operating ISR facility experience (Cogema)
Caustic Soda (\$/Kgals)	\$0.117	Costs from operating ISR facility experience (Cogema)
Hydrochloric Acid (\$/Kgals)	\$0.010	Costs from operating ISR facility experience (Cogema)
Hydrochloric Sulfide (\$/Kgals)	\$0.322	Costs from operating ISR facility experience (Cogema)
Repair & Maintenance (\$/Kgals)	\$0.295	Costs from operating ISR facility experience (Cogema)
Sampling & Analysis (\$/Kgals)	\$0.173	Costs from operating ISR facility experience (Cogema)
Total Cost per Kgal (\$)	\$1.33	
Total Pumping Cost (\$)	\$292,088	
Utilities		
Power (\$/Month)	1,904	
Propane (\$/Month)	846	
Time for Treatment	0	
Minutes for Treatment	4,404,537	
Hours for Treatment	73,409	
Days for Treatment	3,059	
Average Days per Month	30	
Months for Treatment	101	
Utilities Cost (\$)	\$276,720	
TOTAL PLANT & OFFICE COST	\$568,808	
B. WELLFIELD		
Cost Assumptions:		
Power		
Avg Flow/Pump (gpm)	1	
Avg Hp/Pump	1.5	
Avg # of Pumps Required	72.5	
Avg Connected Hp	108.75	
Kwh's/Hp	0.9325	
\$/Kwh	0.053	
Gallons per Minute	72.5	
Gallons per Hour	4350	
Costs per Hour (\$)	\$5.36	
Costs per Gallon (\$)	\$0.0012	
Costs per Kgal (\$)	\$1.23	
Repair & Maintenance (\$/Kgals)	\$0.017	
Total Cost per Kgal	\$1.250	
TOTAL WELLFIELD COST	\$275,267	
TOTAL REVERSE OSMOSIS COST	\$844,075	

Surety Estimate
 First Year of Operation
 Nichols Ranch ISR Project
 Uranerz Energy Corporation

Worksheet 1, No III --
 GROUNDWATER RESTORATION

Cost Item	Mining Unit Nichols #1	Notes
III Deep Disposal Well		
Operating Assumptions:		
Total Disposal Requirement		
RO Brine Total Gallons	44,045,368	
RO Brine Total Kgallons	44,045	
Brine Concentration Factor	1	
Total Concentrated Brine (Gals)	44,045,368	
Months of RO Operation	17.0	
Average Monthly Reqmt (Gallons)	2,592,000	
Average Brine Flow (gpm)	60.0	
Total DDW Disposal (Gallons)	44,045,368	
Total DDW Disposal (Kgallons)	44,045	
Cost Assumptions:		
Avg Connected Hp	20	
Kwh's/Hp	0.9325	
\$/Kwh	0.053	\$.02 plus demand charges per quote
Gallons per Minute	60.0	
Gallons per Hour	3600	
Cost per Hour (\$)	\$0.99	
Cost per Gallon (\$)	\$0.0003	
Cost per Kgal (\$)	\$0.27	
Chemicals		
RO Antiscalent (\$/Kgals)	\$0.203	Costs from operating ISR facility experience (Cogema)
WDW Antiscalent (\$/Kgals)	\$0.239	Costs from operating ISR facility experience (Cogema)
Sulfuric Acid (\$/Kgals)	\$0.296	Costs from operating ISR facility experience (Cogema)
Corrosion Inhibitor	\$0.230	Costs from operating ISR facility experience (Cogema)
Algacide	\$0.085	Costs from operating ISR facility experience (Cogema)
Other	\$0.000	Costs from operating ISR facility experience (Cogema)
Repair & Maint. (\$/Kgals)	\$0.243	Costs from operating ISR facility experience (Cogema)
Total Cost per Kgal	\$1.570	
TOTAL DEEP DISPOSAL WELL COST	\$69,143	

**Surety Estimate
First Year of Operation
Nichols Ranch ISR Project
Uranerz Energy Corporation**

**Worksheet 1, Nos. IV & V --
GROUNDWATER RESTORATION**

Cost Item	Mining Unit		Labor Cost Factors			Notes
	Nichols #1					
IV STABILIZATION MONITORING						
Operating Assumptions:						
Time of Stabilization (mos)		17.0				
Frequency of Analysis (mos)		3				
Total Sets of Analysis		6				
Cost Assumptions:						
Power (\$/Month)		\$0				No add'l power required to sample
Total Power Cost		\$0				
Quantity of Monitoring Wells		12				
Cost per Event		\$349				
Sampling & Analysis (each set)		\$4,189				12 Monitoring Wells @ \$330 per event
Total Sampling & Analysis Cost (\$)		\$25,133				
Utilities (\$/Month)		\$0				No add'l utilities required to sample
Total Utilities Cost (\$)		\$0				
TOTAL STABILIZATION COST		\$25,133				
V LABOR						
Cost Assumptions:						
Crew:	No.	Cost/Hour	Hours/Year	Cost		
1. Supervisor	1	29	2080	\$60,320		
2. Operators	4	22	2080	\$183,040		
3. Maintenance	2	20	2080	\$83,200		
4. Vehicles	2	10	2080	\$41,600		
Cost per Year				\$368,160		
Time Required - Years	5.02					
TOTAL RESTORATION LABOR COST		\$1,848,163				

Surety Estimate
 First Year of Operation
 Nichols Ranch ISR Project
 Uranerz Energy Corporation

Worksheet 1, Nos. VI, VII & Summary --
GROUNDWATER RESTORATION

Cost Item	Mining Unit	Notes
	Nichols #1	
VI RESTORATION CAPITAL REQUIREMENTS		
I Deep Disposal Well(s)	1	
II Plug and Abandon DDW	\$52,890	
III Reverse Osmosis Unit	\$0	Already in Processing Plant
TOTAL RESTORATION CAPITAL REQUIREMENTS	\$52,890	
VII RESTORATION OF EXCURSION WELLS		
I Shallow Sand Well(s)		
Total Wells in Excursion	0	Assume no excursions during Year 1
Cost of Clean-Up	\$0	
Total Shallow Sand Cleanup	\$0	
II Ore Zone Wells		
Total Wells in Excursion	0	
Cost of Clean-Up	\$0	
Total Ore Zone Cleanup	\$0	
III Deep Zone Wells		
Total Wells in Excursion	0	
Cost of Clean-Up	\$0	
Total Deep Zone Cleanup	\$0	
TOTAL WELLFIELD COST		
TOTAL EXCURSION CLEANUP COST	\$0	
SUMMARY:		
I GROUNDWATER SWEEP	\$115,836	
II REVERSE OSMOSIS	\$844,075	
III WASTE DISPOSAL WELL	\$69,143	
IV STABILIZATION	\$25,133	
SUB TOTAL	\$1,054,186	
V LABOR	\$1,848,163	
VI CAPITAL	\$52,890	
VII EXCURSION CLEANUP	\$0	
TOTAL GROUNDWATER RESTORATION COST	\$2,955,240	

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Worksheet 2 a

PLANT EQUIPMENT REMOVAL AND DISPOSAL

Cost Item	Nichols Mine Unit						Sub Total	Notes
	Office & Laboratory	Main Process Building	Maintenance Building	Resin + Sand Filter Media	External Tanks	Header Houses		
Volume (Yds ³)	40	200	45	110	109	240		add 4 add'l 17,000 gal tanks. 84 cy each crushed to 21 cy.
Quantity per Truck Load (Yds ³)	20	20	20	20	20	20		
Number of Truck Loads	2	10	2.25	5.5	5.45	12		
I Decontamination Cost								
Decontamination Cost (\$/Load)	634.68	634.68	634.68	634.68	634.68	634.68		
Percent Requiring Decontamination	20%	100%	20%	0%	50%	100%		
Total Cost	\$254	\$6,347	\$286	\$0	\$1,730	\$7,616		
II Dismantle and Loading Cost								
Cost per Truck Load (\$)	\$846	\$846	\$846	\$846	\$846	\$846		
Total Cost	\$1,692	\$8,462	\$1,904	\$4,654	\$4,612	\$10,155		
III Oversize Charges								
Percent Requiring Permits	40%	40%	40%	0%	50%	40%		
Cost per Truck Load (\$)	\$423	\$423	\$423	\$423	\$423	\$423		
Total Cost	\$338	\$1,692	\$381	\$0	\$1,153	\$2,031		
IV Transportation & Disposal								
A. Landfill								
Percent to be Shipped	90%	80%	90%	0%	100%	80%		
Distance (Miles)	75	75	75	75	75	75		
Transport Cost (\$/Ton-Mile)	\$0.16	\$0.16	\$0.16	\$0.16	\$0.16	\$0.16		
Transportation Cost	\$463	\$2,056	\$521	\$0	\$1,401	\$2,468		
Disposal Fee per Cubic Yard	\$65	\$65	\$65	\$65	\$65	\$65		
Disposal Cost (\$)	\$2,323	\$10,324	\$2,613	\$0	\$7,033	\$12,389		
Total Cost	\$2,786	\$12,380	\$3,134	\$0	\$8,434	\$14,857		
B. Licensed Site								
Percent to be Shipped	10%	20%	10%	100%	0%	20%		
Distance (Miles)	160	160	160	160	160	160		
Transport Cost (\$/Ton-Mile)	\$0.16	\$0.16	\$0.16	\$0.16	\$0.16	\$0.16		
Transport Cost	\$691	\$6,912	\$778	\$19,008	\$0	\$8,294		
Disposal Cost (\$/Ton)	\$370	\$370	\$370	\$370	\$370	\$370		
Quantity per Truck Load (Yds ³)	20	20	20	20	20	20		
Quantity per Truck Load (Tons)	21.6	21.6	21.6	21.6	21.6	21.6		Based on avg 80lbs per cf
Disposal Cost	\$1,599	\$15,994	\$1,799	\$43,983	\$0	\$19,193		
Total Cost	\$2,291	\$22,906	\$2,577	\$62,991	\$0	\$27,487		
Total Cost	\$5,076	\$35,286	\$5,711	\$62,991	\$8,434	\$42,344		
TOTAL COST NICHOLS RANCH MINE	\$7,361	\$51,788	\$8,281	\$67,646	\$15,929	\$62,146	\$213,150	

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Worksheet 2 b --
BUILDING DEMOLITION AND DISPOSAL

Cost Item	Nichols Mine Unit					Notes
	Office & Laboratory	Main Process Building	Maintenance Building	Header Houses	Sub Total	
STRUCTURE DEMOLITION & DISPOSAL						
Structural Character						
Demolition Volume (Ft ³)	90,000	1,188,000	144,000	3,000		
Unit Cost of Demolition (\$/ Ft ³)	\$0.257	\$0.257	\$0.257	\$0.257		Demolition Unit Cost per WDEQ Guideline No.12, App. K (\$/ft3)
Total Demolition Cost	\$23,126	\$305,264	\$37,002	\$771		
Weight of Disposal Material in Tons	41	535	65	1		
Factor for Gutting	0.1	0.3	0.2	0.25		
Cost for Gutting (\$)	\$2,313	\$91,579	\$7,400	\$193		
Quantity per Truck Load (Ton)	21.6	21.6	21.6	21.6		
Number of Truckloads	1.9	24.8	3.0	0.1		
Distance to Landfill	75	75	75	75		
Unit Cost (Ton-Mile)	\$0.16	\$0.16	\$0.16	\$0.16		
Transportation Cost	\$481.96	\$6,361.87	\$771.14	\$16.07		
Disposal Cost (\$/ton)	\$102.89	\$102.89	\$102.89	\$102.89		Demolition Unit Cost per WDEQ Guideline No.12, App. K, Adjusted Cost per Unit
Disposal Cost (\$)	\$4,166.96	\$55,003.86	\$6,667.13	\$138.90		
TOTAL STRUCTURE DEMO & DISPOSAL	\$30,088	\$458,209	\$51,840	\$1,119	\$541,256	
CONCRETE DECONTAMINATION, DEMO & DISPOSAL						
Area	9000	29700	8000	3000		12 header houses @250 sq ft each
Average Thickness (Ft)	0.5	0.5	0.5	0.5		
Volume (Ft ³)	4500	14850	4000	1500		
Weight of Disposal Concrete Assuming 145lbs/cubic foot	652,500	2,153,250	580,000	217,500		
Weight of Disposal in Tons	326	1077	290	109		
Percent Requiring Decontamination	0%	100%	0%	10%		
Volume Decontaminated (Ft ²)	0	14,850	0	150		
Decontamination (\$/Ft ²)	\$0.301	\$0.301	\$0.301	\$0.301		Decontamination by Steam Cleaning (137.5 ft2/hr) ECHOS Unit Cost Book
Decontamination Cost	\$0	\$4,469	\$0	\$45		
Demolition (\$/Ft ²)	\$5.05	\$5.05	\$5.05	\$5.05		Demolition Unit Cost per WDEQ Guideline No.12, App. K, Adjusted Cost per Unit
Demolition Cost	\$45,494	\$150,130	\$40,439	\$15,165		
Transportation & Disposal						
A. Onsite Disposal						
Percent to be Disposed Onsite	100%	75%	100%	100%		
Transportation Cost	\$0	\$0	\$0	\$0		
Disposal Cost per Cubic Yard (\$)	\$8.49	\$8.49	\$8.49	\$8.49		Demolition Unit Cost per WDEQ Guideline No.12, App. K, Adjusted Cost per Unit
Disposal Cost (\$)	\$1,415	\$4,668	\$1,257	\$472		
B. Licensed Site						
Percent to be Shipped	0%	25%	0%	0%		
Distance (Miles)	160	160	160	160		
Unit Cost (Ton-Mile)	\$0.16	\$0.16	\$0.16	\$0.16		
Transportation Cost (\$)	\$0	\$6,833	\$0	\$0		
Disposal Cost (\$/Ton)	\$370	\$370	\$370	\$370		
Disposal Cost (\$)	\$0	\$99,650	\$0	\$0		
TOTAL TRANSPORT & DISPOSAL COST	\$46,909	\$265,750	\$41,697	\$15,681	\$370,037	
TOTAL BUILDING DEMO & DISPOSAL COST	\$76,996	\$723,959	\$93,537	\$16,800	\$911,292	

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Worksheet 3 b --
SOIL REMOVAL & DISPOSAL

Cost Item	Nichols Mine Unit					Notes
	Office & Laboratory	Main Process Building	Maintenance Building	Header Houses	Sub Total	
SOIL EXCAVATION, TRANSPORT & DISPOSAL						
Removal Under Building Footprints						
Excavation, Front End Loader	\$52	\$172	\$46	\$17		\$89.04/hr per WDEQ Guideline 12 and 150 cy/hr
Quantity to be Shipped (Ft ³)	2,250	7,425	2,000	750		Assume removal of 3" of Contaminated Soil under Primary Areas, Disposal at a Licensed facility (ft3)
Weight in Tons	112.5	371.25	100	37.5		
Distance (Miles)	160	160	160	160		
Transportation Unit Cost (Ton/Mile)	\$0.16	\$0.16	\$0.16	\$0.16		
Transportation Cost	\$2,856	\$9,425	\$2,539	\$952		
Disposal Fee (\$/Ton)	\$370	\$370	\$370	\$370		
Disposal Cost (\$)	\$41,651	\$137,448	\$37,023	\$13,884	\$230,005	
Removal NPDES Pts.						
Quantity to be Shipped (Ft ³)	0	0	0	0		Zero discharge facility
Weight in Tons	0	0	0	0		
Distance (Miles)	160	160	160	160		
Transportation Cost Ton/Mile (\$)	\$0.16	\$0.16	\$0.16	\$0.16		
Transportation Cost	\$0	\$0	\$0	\$0		
Disposal Fee (\$/Ton)	\$370	\$370	\$370	\$370		
Disposal Cost (\$)	\$0	\$0	\$0	\$0		
Total NPDES Removal Cost	\$0	\$0	\$0	\$0	\$0	
TOTAL SOILS EXC., TRANSPORT & DISPOSAL	\$41,651	\$137,448	\$37,023	\$13,884	\$230,005	
RADIATION SURVEY						
Area Required (Acres)	0.21	0.68	0.18	0.07		
Survey Cost (\$/Acre)	\$635	\$635	\$635	\$635		
Number of Structures	1	1	1	12		
Cost per Structure (\$)	\$238	\$238	\$238	\$238		
TOTAL RAD SURVEY COST	\$369	\$671	\$355	\$2,900	\$4,294	
TOTAL SOIL REMOVAL & DISPOSAL COST	\$42,020	\$138,119	\$37,378	\$16,783	\$234,300	

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**Worksheet 4 --
 Well Abandonment**

Cost Item	Mining Unit	Notes
	Nichols #1	
Number of Wells	515	Includes injection, recovery and monitor wells.
Average Depth (ft)	550	
Average Diameter (inch)	5	
Area of Annulus (ft ²)	0.1364	
Materials		
Bentonite Chips Required (Ft ³ /Well)	40.9	300 feet of clay above water
Bags of Chips Required/Well	55	
Cost per Bag (\$)	\$6.82	
Cost/Well Bentonite Chips (\$)	\$375	
Gravel Fill Required (Ft ³ /Well)	34.1	Avg depth less 300 feet filled w/ gravel
Cost of Gravel/Yd ³ (\$)	\$21	
Cost/Well Gravel Fill (\$)	\$27	
Cement Cone/Markers Req'd/Well	1	
Cost of Cement Cones Markers (\$)	\$6.35	
Total Materials Cost per Well	\$408	
Labor		
Hours Required per Well	2	
Labor Cost per Hour	\$74	
Total Labor Cost per Well (\$)	148.092	
Equipment Rental		
Hours Required per Well	1	
Backhoe w/Operator Cost/Hr (\$)	\$63	
Total Equipment Cost per Well (\$)	\$63	
Total Cost per Well (\$)	\$620	
TOTAL WELL ABANDONMENT COST (\$)	\$319,234	

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**Worksheet 5, No. 1 --
WELLFIELD EQUIPMENT REMOVAL & DISPOSAL**

Cost Item	Mining Unit	Notes
	Nichols #1	
I Wellfield Piping		
A. Removal		
Total Number of Wells	482	Includes total injection and recovery wells
Feeder lines from HH to Injection wells 1" HDPE (Ft)	71,560	From Preliminary Design
Pregnant solution feeder lines from production wells to HH 1" HDPE (Ft)	50,427	From Preliminary Design
Total Quantity of 1" HDPE Piping (Ft)	121,987	
Plastic Volume (Ft ³)	400.05	Thickness Based on WL Plastics Corp PSI 160 (R1=.05479', R2=.04425')
Chipped Volume Assuming 30% Void Space (Ft ³)	520.07	
Disposal Weight (tons)	20.80	Year 1 buildout only to include Nichols 1
Quantity per Truck Load (Tons)	21.6	Based on 20 cy per truckload and 80lbs per cf
Total Number of Truck Loads	1	
Total Length of Feeder line Trench (ft)	40,765	Includes Shared Trenches
Pipeline Removal Unit Cost (\$/ft of trench)	\$2.38	Quote - Jordan Construction
Total Cost for Trunkline Removal (\$)	\$97,022	
Total Cost - Removal	\$97,022	
B. Survey & Decontamination		
Percent Requiring Decontamination	0	No survey or decon needed. Total volume to low level disposal
Loads for Decontamination	0	
Cost for Decontamination (\$/Load)	\$635	
Cost for Decontamination (\$)	\$0	
C. Transport & Disposal		
1.) Landfill		
a. Transportation		
Percent to be Shipped	0%	
Loads to be Shipped	0	
Distance (Miles)	75	
Transportation Cost (Ton/Mile) (\$)	\$0.16	
Transportation Cost (\$)	\$0	
b. Disposal		
Disposal Fee per Yd ³	\$65	
Yds ³ per Load	20	
Disposal Cost (\$)	\$0	
Total Cost - Landfill	\$0	
2.) Licensed Site		
a. Transportation		
Percent to be Shipped	100%	
Loads to be Shipped	1	
Tons to be Shipped	20.80	
Distance (Miles)	160	
Transportation Ton/Mile (\$)	\$0.16	
Transportation Cost (\$)	\$528	
b. Disposal		
Disposal Fee per ton	\$370	
Disposal Cost (\$)	\$7,702	
Total Cost - Licensed Site	\$8,230	
Total Cost - Transport & Disposal	\$8,230	
Total Cost - WF Piping Removal & Disposal	\$105,251	

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Worksheet 5, No. II

WELLFIELD EQUIPMENT REMOVAL & DISPOSAL

Cost Item	Mining Unit	Notes
	Nichols #1	
II Production Well Pumps		
A. Pump and Tubing Removal		
Number of Production Wells	233	
Cost of Removal (\$/well)	\$42	
Cost of Removal (\$)	\$9,859	
Number of Pumps per Truck Load	180	
Number of Truck Loads (Pumps)	1.29	
Weight of Pumps	21.29	Assume 20 T per truck
B. Survey & Decontamination (Pumps)		
Percent Requiring Decontamination	50%	
Loads for Decontamination	0.65	
Cost for Decontamination (\$/Load)	\$635	
Cost for Decontamination (\$)	\$411	
C. Tubing Volume Reduction & Loading		
Length per Well (Ft)	300	
Total Quantity (Ft ³)	229.2	Thickness Based on WL Plastics Corp PSI 160 (R1=.05479', R2=.04425')
Chipped Volume Assuming 30% Void Space (Ft ³)	298.0	
Cost of Removal (\$/Ft)	\$0.03	
Cost of Removal (\$)	\$9.52	
Quantity per Truck Load (Ft ³)	540	
Number of Truck Loads	0.42	
D. Transport & Disposal		
1.) Landfill		
a. Transportation		
Percent to be Shipped (Pumps)	50%	
Loads to be Shipped	0.6	
Distance (Miles)	75	
Transportation Ton/Mile (\$)	\$0.16	
Transportation Cost (\$)	\$166	
b. Disposal		
Disposal Fee per Yd ³	\$65	
Yds ³ per Load	20	
Disposal Cost (\$)	\$835	
Total Cost - Landfill	\$1,002	
2.) Licensed Site		
a. Transportation		
Percent to be Shipped (Pumps)	50%	
Percent to be Shipped (Tubing)	100%	
Loads to be Shipped	1.07	
Distance (Miles)	160	
Transportation Ton/Mile (\$)	\$0.16	
Transportation Cost (\$)	\$588	
b. Disposal		
Disposal Cost per Yd ³	\$18.51	
Disposal Volume per Load (cy)	20	
Disposal Cost	\$397	
Total Cost - Licensed Site	\$984	
Total Cost - Transport & Disposal	\$1,986	
Total Cost - Pump Removal & Disposal	\$12,265	

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Worksheet 5, No. III

WELLFIELD EQUIPMENT REMOVAL & DISPOSAL

Cost Item	Mining Unit	Notes
	Nichols #1	
III Buried Trunkline		
A. Removal		
Trunk lines from Resin Plant to HH 8" HDPE Pipe (Ft)	38,473	
Pregnant solution trunk lines form HH to Resin Plant 8" HDPE Pipe (Ft)	38,473	
Total Quantity of 8" HDPE Piping (Ft)	76,946	
Plastic Volume (Ft ³)	51,906	Thickness Based on WL Plastics Corp PSI 160 (R1=.7188', R2=.5494')
Chipped Volume Assuming 30% Void Space (Ft ³)	67,478	
Disposal Tons	320	8.315lb/ft per WL Plastics
Quantity per Truck Load (Tons)	21.6	
Total Number of Truck Loads	15	
Total Length of Trunkline Trench (ft)	38,473	
Pipeline Removal Unit Cost (\$/ft of trench)	\$2.38	Quote Jordan Construction
Total Cost for Trunkline Removal (\$)	\$91,568	
B. Survey & Decontamination		
Percent Requiring Decontamination	0	No survey or decon needed. Total volume to low level disposal
Loads for Decontamination	0	
Cost for Decontamination (\$/Load)	\$635	
Cost for Survey & Decontamination (\$)	\$0	
C. Transportation & Disposal		
1.) Landfill		
a. Transportation		
Percent to be Shipped	0%	
Loads to be Shipped	0	
Distance (Miles)	75	
Transportation Cost (Ton/Mile) (\$)	\$0.16	
Transportation Cost (\$)	\$0	
b. Disposal		
Disposal Fee per Yd ³	\$65	
Yds ³ per Load	20	
Disposal Cost (\$)	\$0	
Total Cost - Landfill	\$0	
2.) Licensed Site		
a. Transportation		
Percent to be Shipped	100%	
Loads to be Shipped	15	
Tons to be Shipped	319.90	
Distance (Miles)	160	
Transportation Ton/Mile (\$)	\$0.159	
Transportation Cost (\$)	\$8,121	
b. Disposal		
Disposal Fee per ton	\$370	
Disposal Cost (\$)	\$118,438	
Total Cost - Licensed Site	\$126,559	
Total Cost Transportation & Disposal	\$126,559	
Total Cost - Buried Trunkline Removal & Disposal	\$218,127	
TOTAL WELLFIELD EQUIPMENT REMOVAL & DISPOSAL COST	\$335,643	

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Worksheet 6, No. 1

TOPSOIL REPLACEMENT & REVEGETATION

Cost Item	Mining Unit Nichols #1	Notes
I Process Plant and Office Building		
A. Topsoil Handling & Grading		
Affected Area (Acres)	5.2	Plant site is 475' by 475'
Average Affected Thickness (Ins)	12	
Topsoil Volume (Yds ³)	8,356	
Unit Cost (\$/cy)	\$5	Price from Dragstrip Soil Cover Project MT
Sub Total - Topsoil	\$44,197	
B. Radiation Survey & Soil Analysis		
Unit Cost (\$/Ac)	\$635	
Sub Total - Survey & Analysis	\$3,287	
C. Revegation		
Fertilizer (\$/Ac)	\$245.41	Price from Dragstrip Soil Cover Project MT
Seeding Prep & Seeding (\$/Ac)	\$240.12	Price from Dragstrip Soil Cover Project MT
Mulching & Crimping (\$/Ac)	\$105.78	Price from Dragstrip Soil Cover Project MT
Sub Total Cost/Acre	\$591.31	
Sub Total Revegation	\$3,063	
TOTAL PLANT AND OFFICE BUILDING		
TOPSOIL REPLACEMENT & REVEG COST	\$50,548	

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Worksheet 6, Nos. II & III

TOPSOIL REPLACEMENT & REVEGETATION

Cost Item	Mining Unit	Notes
	Nichols #1	
II Wellfields		
A. Topsoil Handling & Grading		
Affected Area (Acres)	22	Equals trench length times 12 feet wide
Average Affected Thickness (Inch)	12	
Topsoil Volume (Yds ³)	35,217	
Unit Cost - Haul/Place/Grading (\$/cy)	\$5.29	Price from Dragstrip Soil Cover Project MT
Sub Total - Topsoil	\$186,261	
B. Radiation Survey & Soil Analysis		
Unit Cost (\$/Ac)	\$635	
Sub Total - Survey & Analysis	\$13,854	
C. Spill Cleanup		
Affected Area (Acres)	0	
Affected Area (Ft ²)	0	
Affected Area Thickness (Ft)	0.25	
Affected Volume (Ft ³)	0	
Quantity per Truckload (Ft ³)	540	
Quantity to be Shipped (Loads)	0	
Distance (Miles)	160	
Transportation Cost (Ton/Mile) (\$)	\$0.16	
Transportation Cost (\$)	\$0	
Handling Cost (\$/Load)	\$212	
Handling Cost (\$)	\$0	
Disposal Fee (\$/Ton)	\$370	
Disposal Cost (\$)	\$0	
Sub Total - Spill Cleanup	\$0	
D. Revegation		
Fertilizer (\$/Ac)	\$245.41	Price from Dragstrip Soil Cover Project MT
Seeding Prep & Seeding (\$/Ac)	\$240.12	Price from Dragstrip Soil Cover Project MT
Mulching & Crimping (\$/Ac)	\$105.78	Price from Dragstrip Soil Cover Project MT
Sub Total Cost/Acre	\$591.31	
Sub Total Revegation	\$12,907	
Sub Total - Wellfields	\$213,023	
TOTAL WELLFIELDS COST	\$213,023	
III Roads		
A. Topsoil Handling & Grading		
Affected Area (Acres)	5.17	3750 feet by 60 feet wide
Average Affected Thickness (Ins)	12	
Topsoil Volume (Yds ³)	8,333	
Unit Cost - Haul/Place/Grading (\$/cy)	\$5.29	Price from Dragstrip Soil Cover Project MT
Sub Total - Topsoil	\$44,075	
B. Radiation Survey & Soil Analysis		
Unit Cost (\$/Ac)	\$635	
Sub Total - Survey & Analysis	\$3,278	
C. Revegation		
Fertilizer (\$/Ac)	\$245	Price from Dragstrip Soil Cover Project MT
Seeding Prep & Seeding (\$/Ac)	\$240	Price from Dragstrip Soil Cover Project MT
Mulching & Crimping (\$/Ac)	\$106	Price from Dragstrip Soil Cover Project MT
Sub Total Cost/Acre	\$591	
Sub Total Revegation	\$3,054	
Sub Total - Roads	\$50,408	
TOTAL ROADS COST	\$50,407.59	

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Worksheet 6, Nos IV & V

TOPSOIL REPLACEMENT & REVEGETATION

Cost Item	Mining Unit	Notes
	Nichols #1	
IV Other		
A. Topsoil Handling & Grading		
Affected Area (Acres)	0	
Average Affected Thickness (Ins)	3	
Topsoil Volume (Yds ³)	0	
Unit Cost - Haul/Place/Grading (\$/Ac)	\$5.29	Price from Dragstrip Soil Cover Project MT
Sub Total - Topsoil	\$0	
B. Radiation Survey & Soil Analysis		
Unit Cost (\$/Ac)	\$635	
Sub Total - Survey & Analysis	\$0	
C. Revegation		
Fertilizer (\$/Ac)	\$245.41	Price from Dragstrip Soil Cover Project MT
Seeding Prep & Seeding (\$/Ac)	\$240.12	Price from Dragstrip Soil Cover Project MT
Mulching & Crimping (\$/Ac)	\$105.78	Price from Dragstrip Soil Cover Project MT
Sub Total Cost/Acre	\$591.31	
Sub Total Revegation	\$0	
Sub Total - Other	\$0	
TOTAL OTHER COST	\$0	
V Remedial Action		
A. Topsoil Handling & Grading		
Affected Area (Acres)	0	Assume no excursions/spills
Average Affected Thickness (Ins)	3	
Topsoil Volume (Yds ³)	0	
Unit Cost - Haul/Place/Grading (\$/cy)	\$5.29	Price from Dragstrip Soil Cover Project MT
Sub Total - Topsoil	\$0	
B. Radiation Survey & Soil Analysis		
Unit Cost (\$/Ac)	\$635	
Sub Total - Survey & Analysis	\$0	
C. Revegation		
Fertilizer (\$/Ac)	\$245.41	Price from Dragstrip Soil Cover Project MT
Seeding Prep & Seeding (\$/Ac)	\$240.12	Price from Dragstrip Soil Cover Project MT
Mulching & Crimping (\$/Ac)	\$105.78	Price from Dragstrip Soil Cover Project MT
Sub Total Cost/Acre	\$591.31	
Sub Total Revegation	\$0	
TOTAL REMEDIAL ACTION	\$0	
TOTAL TOPSOIL REPLACEMENT & REVEGETATION COST (Total of 7I through 7V)	\$313,978	

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Worksheet 7, Nos I - VII

MISCELLANEOUS RECLAMATION

	Cost Item	Mining Unit	Notes
		Nichols #1	
I	Fence Removal & Disposal		
	Quantity (Ft)	8,558	
	Cost of Removal/Disposal (\$/Ft)	\$0.39	Demolition Unit Cost per WDEQ Guideline No.12, App. H
	Cost of Removal/Disposal (\$)	\$3,335	
II	Powerline Removal & Disposal		
	Quantity (Ft)	160,460	Power to Wells, header houses. Other power already in place by CBM companies
	Cost of Removal/Disposal (\$/Ft)	\$0	Lines buried in pipe trenches. Excavation costs covered on Sheets 6I and 6III. Assume salvage of wire at no cost.
	Cost of Removal/Disposal (\$)	\$0	
III	Powerpole Removal & Disposal		
	Quantity	0	Overhead powerpoles and lines will remain in place for future gas production
	Cost of Removal/Disposal (\$/Each)	0	
	Cost of Removal/Disposal (\$)	\$0.00	
IV	Transformer Removal & Disposal		
	Quantity	0	Tri-County Electric will remove at no cost, WDEQ Guideline No.12, App. H
	Cost of Removal/Disposal (\$/Each)	0	
	Cost of Removal/Disposal (\$)	0	
V	Culvert Removal & Disposal		
	Quantity (Ft)	0	None
	Cost of Removal/Disposal (\$/Ft)	\$5.33	(\$101.21/20') WDEQ Guideline No.12, App. J
	Cost of Removal/Disposal (\$)	\$0.00	
VI	Guardrail Removal		
	Quantity (Ft)	0	None
	Cost of Removal/Disposal (\$/Ft)	\$6.88	
	Cost of Removal/Disposal (\$)	\$0	
VII	Low Water Stream Crossing		
	Quantity	0	None
	Cost of Removal/Disposal (\$/Each)	\$8,462	
	Cost of Removal/Disposal (\$)	\$0	
	TOTAL MISCELLANEOUS COST	\$3,335	