

South Texas Project Electric Generating Station P.O. Box 289 Wadsworth, Texas 77483

July 21, 2010 U7-C-STP-NRC-100161

U. S. Nuclear Regulatory Commission Attention: Document Control Desk One White Flint North 11555 Rockville Pike Rockville, MD 20852-2738

# South Texas Project Units 3 and 4 Docket Nos. 52-012 and 52-013 Supplemental Response to Request for Additional Information

Reference: Letter, Scott Head to Document Control Desk, "Supplemental Response to Request for Additional Information," dated April 14, 2010, U7-C-STP-NRC-100083 (ML101090143)

Attached is a revised supplemental response to an NRC staff question included in Request for Additional Information (RAI) letter number 302 related to Combined License Application (COLA) Part 2, Tier 2, Section 3.7. The attachment revises the supplemental response provided in the referenced letter to RAI question 03.07.01-24.

No COLA changes are required as a result of this response.

There are no commitments in this letter.

If you have any questions regarding this response, please contact me at (361) 972-7136, or Bill Mookhoek at (361) 972-7274.

I declare under penalty of perjury that the foregoing is true and correct.

Executed on 7/21/10

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Scott Head Manager, Regulatory Affairs South Texas Project Units 3 & 4

ccc

Attachment:

RAI 03.07.01-24, Supplement 1, Revision 1

STI 32703108

cc: w/o attachment except\* (paper copy)

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## RAI 03.07.01-24

#### **QUESTION:**

#### (Follow-up Question to RAI 03.07.01-14)

1. In the response to RAI 03.07.01-14, Item 1, the applicant cited DCD Appendix 3A in concluding that "... *the potential effect of structure-to-structure interaction was relatively small.*" However, DCD Section 3A.9.7, "Effect of Adjacent Buildings" also concluded that seismic soil pressure in between the RB and CB increased due to structure-to-structure interaction (SSSI) effect. As such the applicant is requested to discuss how the potential effects of increase in the seismic soil pressure in between the Category 1 structures and the retaining wall due to the SSSI effect has been addressed and bounded by the certified design.

2. In the response to RAI 03.07.01-14, Item 2, the applicant stated in the second bullet that "In comparison to the Reactor, Control and Turbine Buildings, the retaining wall is a light structure and a lighter structure will have less influence on the seismic behavior of the heavy adjacent structures." While the inertia of the RC retaining wall is not expected to affect the seismic response of the adjacent seismic Category I structures, the stiff retaining wall can act as a barrier to reflect the seismic waves due to kinematic interaction with surrounding soil and could affect the seismic input to the adjacent structures. As such, the applicant is requested to provide a quantitative assessment of the effect of RC retaining wall on the SSI analysis of adjacent Reactor and Control Buildings.

#### **REVISED SUPPLEMENTAL RESPONSE:**

The original supplemental response to this RAI was submitted with STPNOC Letter No. U7-C-STP-NRC-100083, dated April 14, 2010. This revised supplemental response is being submitted because of a discrepancy identified in the Reactor Building model used for the soil-structure interaction analysis from which the results for the previously submitted supplemental response were obtained. This revised supplemental response completely supersedes the original supplemental response.

In order to address the above two questions, a soil-structure interaction (SSI) analysis of the Reactor Building (R/B) and Control Building (C/B), with and without the crane wall, was performed for the site-specific conditions, including site-specific safe shutdown earthquake (SSE) and soil properties. These analyses were performed using two-dimensional (2D) models of the R/B and C/B. The SSI analyses were performed using the SASSI2000 program. Summaries of the SSI analyses results for the mean soil case are presented below. Similar results are obtained for lower and upper bound soil cases.

Summary of Results for R/B:

- Table 03.07.01-24a compares the maximum forces and moments, at key locations of the R/B, with and without the crane wall. As can be seen, the crane wall has a negligible effect on the resulting maximum forces and moments.
- Figures 03.07.01-24a through 03.07.01-24d provide comparisons of response spectra at several locations with and without the crane wall. As can be seen, the crane wall has a negligible effect on the resulting spectra.
- Figure 03.07.01-24e provides the comparison of the resulting seismic soil pressures from the SSI analyses with and without the crane wall. As expected, these seismic lateral soil pressures are significantly bounded by the design seismic soil pressure per DCD Tier 2, Figure 3H.1-11 and pressure obtained from the alternate modified Ostadan method described in COLA Part 2, Tier 2, Section 2.5S.4.10.5.2.

Summary of Results for C/B:

- Table 03.07.01-24b compares the maximum force and moment at grade of the C/B with and without the crane wall. As can be seen, the crane wall has a negligible effect on the resulting maximum force and moment.
- Figures 03.07.01-24f and 03.07.01-24g provide comparisons of response spectra at top of basemat and top of C/B with and without the crane wall. As can be seen, the crane wall has a negligible effect on the resulting spectra.
- Figure 03.07.01-24h provides the comparison of the resulting seismic soil pressures from the SSI analyses with and without the crane wall. As expected, these seismic lateral soil pressures are significantly bounded by the design seismic soil pressure per DCD Tier 2, Figure 3H.2-14 and pressure obtained from the alternate modified Ostadan method described in COLA Part 2, Tier 2, Section 2.5S.4.10.5.2.

No COLA change is required for this response.

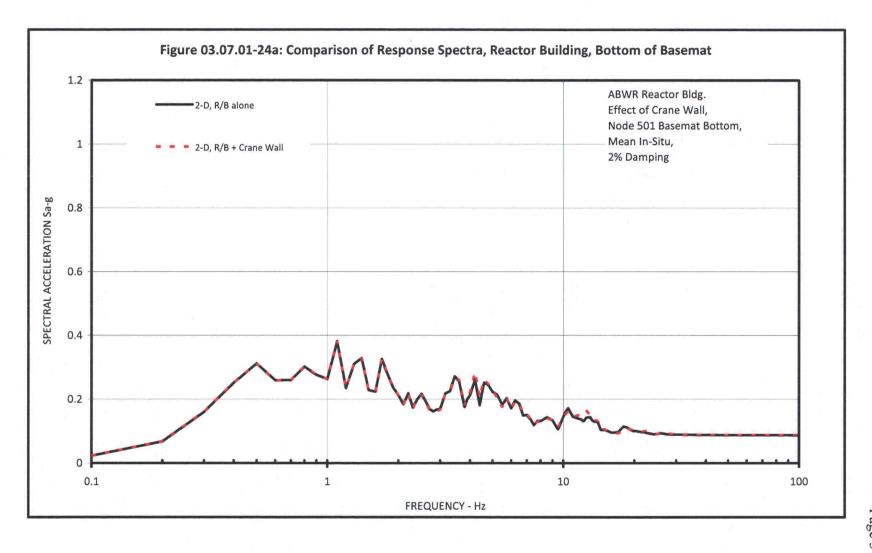
# Table 03.07.01-24aReactor Building Force Comparison

Effect Of Crane Wall on Maximum Forces, Mean Soil							
Beam Element	Location	Response Type	Model in SSI Analysis				
			2-D Reactor Building (alone)	2-D Reactor Building + Crane Wall			
28	Shroud Support	Shear	101	98			
		Moment	1,993	1,948			
69	RPV Skirt	Shear	373	373			
		Moment	6,500	6,420			
78	RSW Base	Shear	299	313			
		Moment	4,464	4,689			
86	Pedestal Base	Shear	1,939	1,943			
		Moment	118,771	119,905			
89	RCCV at Grade	Shear	5,847	5,985			
		Moment	319,708	329,289			
<b>99</b> ·	R/B at Grade	Shear	12,941	13,117			
		Moment	874,650	898,702			

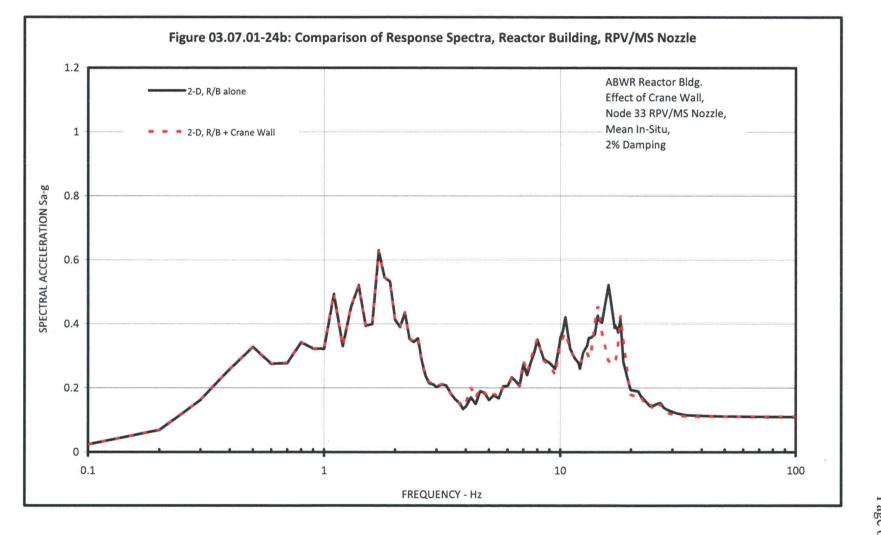
Units: Shear in kip; Moment in kip-ft

Effect of Crane Wall on Maximum Forces, Mean Soil						
			Model in SSI Analysis			
Beam Element	Location	Response Type	2-D Control Building (alone)	2-D Control Building + Crane Wall		
6	C/B at Grade	Shear (kip)	3,068	3,124		
		Moment (kip-ft)	111,181	110,472		

# Table 03.07.01-24bControl Building Force Comparison

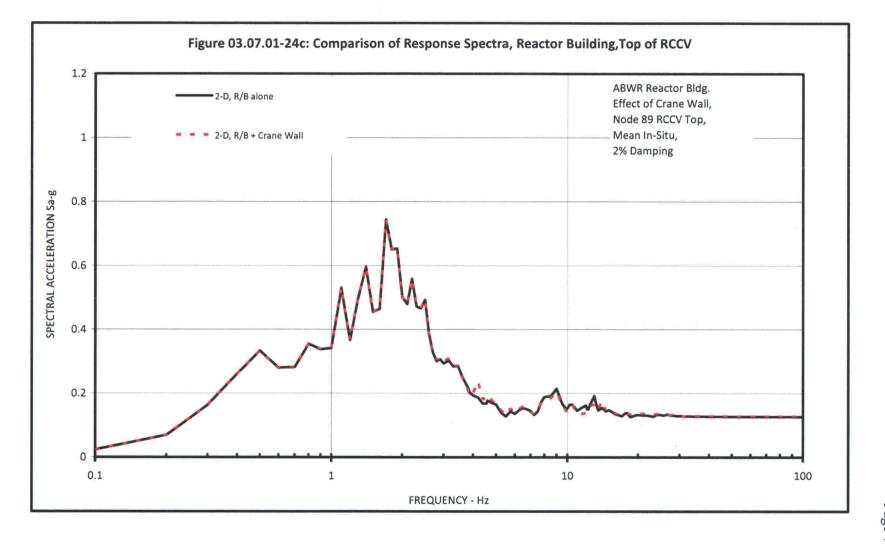


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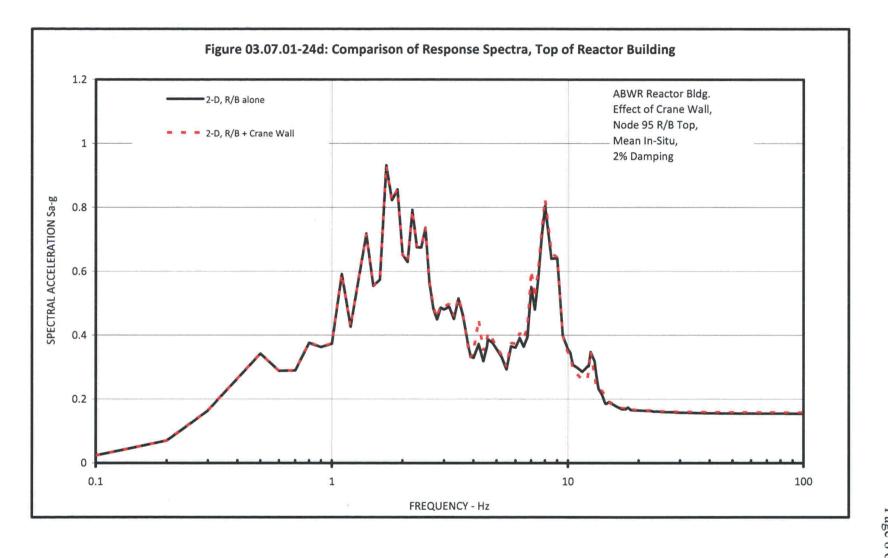


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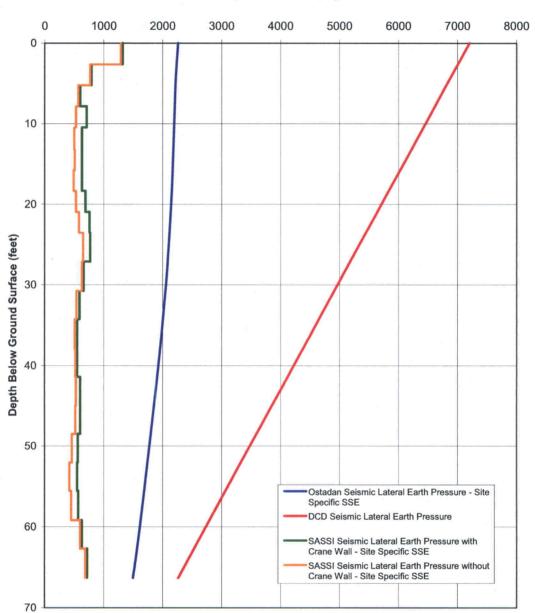


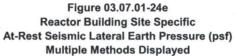
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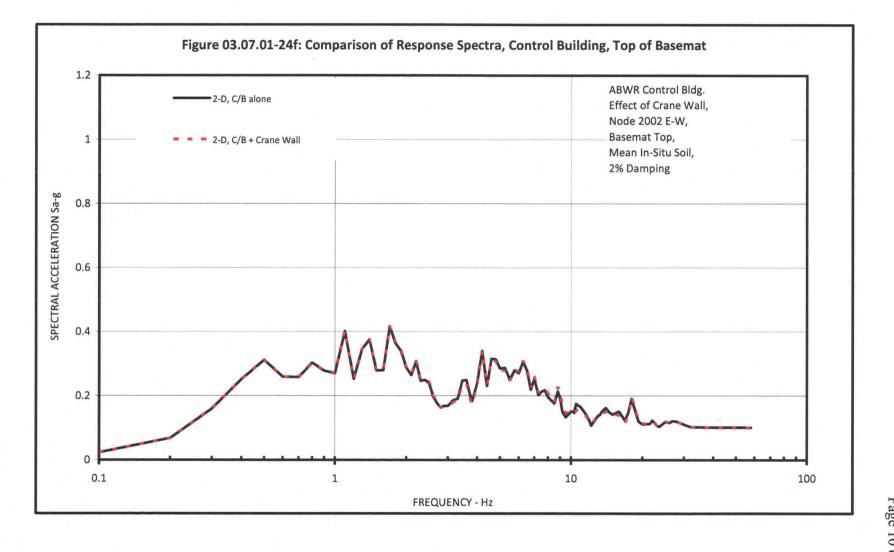


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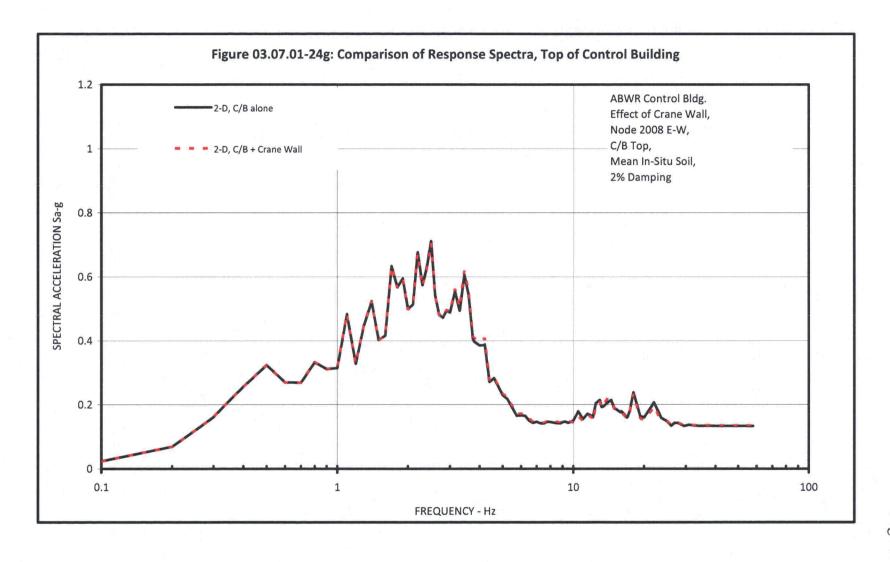
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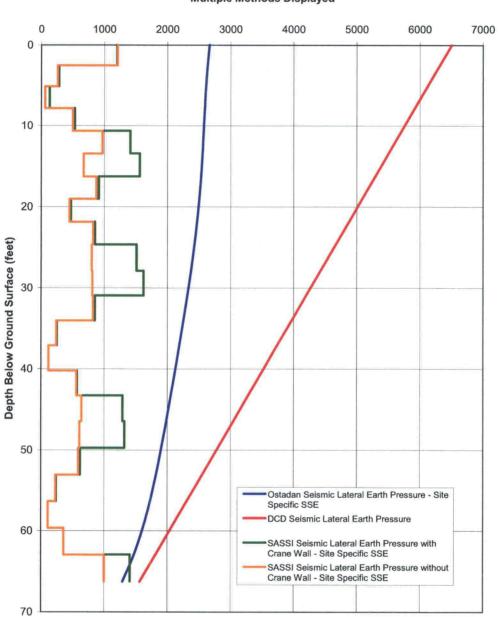




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#### Figure 03.07.01-24h Control Building Site Specific At-Rest Seismic Lateral Earth Pressure (psf) Multiple Methods Displayed