ATTACHMENT (1)

NUH32P+.0203, REVISION 0, 32P+ TRANSFER CASK IMPACT ONTO

THE CONCRETE PAD LS-DYNA ANALYSIS (80 INCH END DROP)

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Revision No.: 0

REVISION SUMMARY

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I **PURPOSE**

This calculation analyzes the rigid body acceleration time histories for the 32P+ cask in the end drop with a drop height of 80". A dynamic finite element analysis program is used to determine the time histories. Rigid body time histories of the cask body and cask bottom plates/resin are extracted from the results.

2 REFERENCES

- 2.1. LS-DYNA Keyword User's Manual, Volumes 1 & 2, Version Is971s R4.2, Livermore Software Technology Corporation.
- 2.2. U.S. Nuclear Regulatory Commision NUREG/CR-6608, "Summary and Evaluation of Low-Velocity Impact Tests of Solid Steel Billet Onto Concrete Pads", February 1998.
- 2.3. ASME Boiler and Pressure Vessel Code, Section II, "Materials Specifications," Parts A, B, C and **D,** 1998 edition with all addenda up to and including 1999 Addenda.
- 2.4. TN Calculation No. 1095-1, Rev. 1, "NUHOMS 32P Weight Calculation of DSC/TC System".
- 2.5. TN Calculation 10494-66, Rev. 0, "NUHOMS-32PTH, OS187H Transfer Cask Dynamic Impact Analysis".
- 2.6. Structural Design of Concrete Storage Pads for Spent Fuel Casks, Electric Power Research Institute, EPRI NP-7551, RP 2813-28, April 1993.
- 2.7. BNL-NUREG-71196-2003-CP, "Impact Analysis of Spent Fuel Dry Casks Under Accidental Drop Scenarios," Brookhaven National Laboratory, 2003.

3 ASSUMPTIONS

3.1 NUH32P+ DSC design is identical to NUH32P DSC design. NUH32P weight properties are used for NUH32P+ weight.

-3.2-Static-and dynamic-coefficient of friction-of-0.25 is assumed-between-all-sliding-surfaces.

3.3 Strain rate effects on all material properties are neglected.

3.4 Mass of DSC is evenly distributed as a homogenous solid.

3.5 A uniform temperature of 350° F is used for the end drop analysis.

4 METHODOLOGY

LS-DYNA, a dynamic finite element analysis program, is used to determine the rigid body acceleration time history of the NUH32P+ cask caused by a hypothetical accident end drop condition. Because of the complexity of the analysis, a simplified model of the cask and DSC is necessary. The cask model does not include trunnions and other details; however, the mass of these unmodeled items is accounted for. The DSC structure is modeled as an isotropic elastic material with properties approximately equivalent to that of the structure as a whole. This is the same method used in Reference [2.2].

The model consists of the cask, the simplified DSC structure, a concrete impact pad, and the subgrade. soil. Only ½ of the cask, DSC structure, concrete and soil are modeled as the entire arrangement is symmetric about the X-Y plane. The section of concrete modeled is 16'-8" long, 6'-8" wide, and 3' thick. The soil section is 66'-8" long, 18'-9" wide, and 39'-2" deep. The concrete and soil dimensions are based on the dimensions used in Reference [2.2]. All lower faces of the soil are fixed except for the symmetry plane. All elements are modeled with fully integrated S/R solid elements.

The finite element model is developed with ANSYS Rev. 11.0 and transferred to LS-DYNA. Modifications were made to the LS-DYNA input files to add the material definitions, non-reflecting boundaries and initial conditions into LS-DYNA, since these input variables are not available through ANSYS. The end drop is analyzed at 350°F. The 32P+ Cask finite element model is shown in Figures 5-1.

5 COMPUTATIONS

5.1 MATERIAL PROPERTIES

The following tables, Table 5-1 through Table 5-3, list stainless steel or carbon .steel material properties available in the model material database. The material properties are based on ASME BPV Code, Section II, 1992 [2.3].

Stainless Steel SA 240 Type 304 (18cr-8ni) - ASME 1992 COM											
Temperature	I°FI		70	200	300	400	500	600	700		
Sy	[psi]	30000	30000	25000	22500	20700	19400	18200	17700		
Su	[psi]	75000	75000	71000	66000	64400	63500	63500	63500		
Sm	[psi]	20000	20000	20000	20000	18700	17500	16400	16000		
	[psi]	2.87E+07	2.83E+07	2.76E+07	2.70E+07	2.65E+07	2.58E+07	$2.53E+07$	2.48E+07		

Table **5-1** Material Properties of Stainless Steel SA 240 Type 304

Table 5-2 Material Properties of Stainless Steel SA 182 Type F304N

Stainless Steel SA 182 Type F304N (18cr-8ni-n)-ASME 1992										
Temperature	r∘Fl		70	200	300	400	500	600	700	
Sy	[psi]	35000	35000	28700	25000	22500	20900	19800	19100	
Su	[psi]	80000	80000	80000	75900	73200	71200	69700	68600	
Sm	[psi]	23300	23300	23300	22500	20300	18800	17800	17200	
Е	[psi]	2.87E+07	2.83E+07	2.76E+07	2.70E+07	2.65E+07	2.58E+07	2.53E+07	2.48E+07	

Table 5-3 Material Properties of Carbon Steel SA 516 Type 70

5.1.1 CASK MATERIAL

The cask material properties are the same at those used in Section 5.1 except the outer shell density is adjusted to account for unmodeled cask parts. The cask weight is calibrated to the weight computed in Reference [2.4]. All cask materials are modeled as elastic.

Table 5-4: Cask Material Properties

The modeled weight of the empty cask is 61,099 lbs since it is a half model, therefore the total modeled weight is 122,198 lbs. The total calculated empty cask weight (121,458 Ibs) in Reference [2.4]. The percentage difference in calculated weight and the modeled weight is 0.58%

5.1.2 **DSC** STRUCTURE MATERIAL

The DSC structure material properties are the same as those used in Reference [2.2] except for the density. The density of the DSC structure is adjusted to calibrate the overall weight of the canister, basket, and fuel assembly [2.4]. The DSC structure is modeled as elastic.

 $E = 2.8 \times 10^6$ psi *v=0.3* $p = 4.0062 \times 10^{-4}$ lb sec²/in⁴

Total modeled weight of the DSC structure is 47,390 lbs since it is a half model. Therefore the total modeled weight is 94,780 lbs. Total actual weight of the DSC per Reference [2.4] is 90,976 lbs.

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Table 5-5: Effective Plastic Strain vs. Scale Factor for Concrete Material

The maximum principal stress tensile failure cutoff is set at 870 psi [2.2]. Strain rate effects are neglected in the analysis.

The pressure-volume behavior of the concrete is modeled with the following tabulated pressure versus volumetric strain relationship shown in Table 5-3 using the equation of state feature in LS-DYNA [Ref. 2.2].

Table 5-6: Tabulated pressures vs. volumetric strain for concrete material

An unloading bulk modulus of 700,000 psi is assumed to be constant at any volumetric strain, as was assumed in Reference [2.2].

One percent deformation is assumed in the concrete pad to account for the pad reinforcement. The one percent reinforcement is also used in the analyses presented in EPRI [2.6].

The material properties used for the reinforcing bar are as follows.

 $E = 30 \times 10^6$ psi $v = 0.3$ *Sy* = 30,000 psi Tangent Modulus, $E_T = 30 \times 10^4$ psi

5.2 BOUNDARY CONDITIONS

Only **1/2** of the cask is modeled with symmetry boundary conditions used to simulate the full structure. Nonreflecting boundaries are applied to the bottom and sides of the modeled soil not aligned with the plane of symmetry (bottom, left side, right side, and back) to prevent artificial stress waves from reflecting back into the model. Both dilatation and shear waves are damped as described in the LS-DYNA *BOUNDARY command [Ref. 2.1].

An automatic surface to surface (contact automatic single surface) contact definition is applied between all parts except the soil. The contact definition has a 0.5 penalty stiffness scale factor to prevent excessive contact stiffness leading to unrealistic part accelerations. A surface to surface (contact surface to surface) contact definition is applied between the concrete and the soil. Both contact definitions have soft contact option 2 as this is necessary for contact between materials that have very different material stiffness. A conservatively low coefficient of friction (static and kinetic) of 0.25 is applied between all contact surfaces. It is conservative to use a low value for the coefficient of friction because

less energy is absorbed due to friction resulting in greater impact acceleration forces.

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6 DATA REDUCTION

The analyses are run for a duration of 0.08 seconds. The time step was set at **1** .17x10-6 which resulted in a negligible weight increase of about 9 lbs.

6.1 CASK NODAL ACCELERATION SECTIONS EVALUATED

The resulting rigid body acceleration time histories are computed by LS-DYNA. The rigid body accelerations are computed for the bottom plates **+** resin and the circumferential shell. The parts can be seen in Figure 6-1.

6.2 RAW DATA FILTERING

LS-DYNA reports the nodal accelerations at 100 usec intervals. Therefore, by the Nyquist theorem, the frequency content of the nodal acceleration data, refined by LS-DYNA, ranges from zero Hz, up to the following maximum frequency, f_{max} .

$$
f_{\text{max}} = \frac{1}{2} \frac{1}{100 \times 10^{-6} \text{ sec}} = 5 \text{ kHz}
$$

The natural frequencies of the 32P+ cask model, which can be excited by an impact event, are much lower than this. These natural modes of the cask involve small displacements (and therefore low stresses) at frequencies higher than that of the rigid body motion of the cask. These high frequency accelerations mask the true rigid body motion of the cask, because both the low frequency rigid body acceleration and the high frequency natural vibration accelerations superimpose. The net acceleration is contained in the raw data computed by LS-DYNA. Therefore, filtering is necessary to remove these high frequency accelerations.

The rigid body acceleration for each part is filtered using an $8th$ order low pass Butterworth filter forwards and backwards with a cutoff frequency of 180Hz. This frequency is based on Fourier spectral analyses shown in Figures 7-1 through 7-4. The figures show that the 180Hz cutoff will still conservatively include some of the cask's natural modes. The impact durations are all over 0.03 seconds, so the minimum ...
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frequency to capture rigid body motion would be $2 \times \frac{1}{\sqrt{2}} = 66.67$ Hz. 0.03

7 RESULTS

Table 7-1 lists the peak filtered accelerations and corresponding time history plot for different parts of the $32P+$ cask. All results are filtered with an $8th$ order low pass butterworth filter with a 180Hz cutoff frequency. Figure 8-1 through 8-2 shows the filtered acceleration time histories. Figure 8-3 through 8-4 shows the Fourier spectral analyses of the acceleration time histories before and after filtering.

Table 7-1: Filtered Results Summary

8 LISTING OF ANSYS COMPUTER FILES

Below is a listing of all files used in LS-DYNA, all Analysis performed on Computer **HEA0105A,** Dual Intel Xeon 3.2GHz, Windows XP SP2, LS-DYNA ver. Is971s R4.2 Revision 50638.

Note: Date & time (EST) for main runs are from the listing at the end of the output file. For other files (e.g., .db files), dates & times are reported by the OS on the report issue date, these values may be changed by Windows depending on time of the year (e.g., daylight savings time) and time zones.

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9 APPENDIX A - BUTTERWORTH FILTER VERIFICATION

The butterworth filter used in this analysis is based on the program Octave, an open source code program used for solving linear and nonlinear problems. An 8th order low pass butterworth filter is run forward and used for solving linear and nonlinear problems. An 8th order low pass butterworth filter is run forward and backward as described in NUREG/CR-6608. To verify the filter functions properly, the filtered and unfiltered fourier spectrum plot of the NUREG/CR-6608 is compared to the fourier spectrum plots of this analysis. Figure **Al** shows a NUREG/CR-6608 data set which was filtered at 450Hz. The plots of this analysis show good correlation to Figure **Al.** In addition, Figure A2 shows the frequency response of the 8th order butterworth filter at 450Hz run forward and backward in this analysis. It can be seen that the frequency response coincides with the attenuation pattern seen in the NUREG/CR-6608 data set.

Figure A1 - Impulse Response of 8th Order Butterworth Filter Forward and Backward

Figure A2 -Frequency Response of 8th Order Butterworth Filter Forward and Backward

ENCLOSURE (1)

The File Listing for Three DVDs Containing LS-DYNA Files for NUH32P+.0203

ATTACHMENT (2)

NUH32P+.0204, REVISION 0, FUEL END DROP ANALYSIS FOR

NUH32P+ USING LS-DYNA, NON-PROPRIETARY VERSION

Non-PROPRIETARY Version

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6.0 Analysis

6.1 Model Geometry and Details

Figure 2 illustrates the finite element model, which is composed of a single fuel rod, a lumped cask mass, springs representing the spacer grids, contact surfaces representing the basket compartment wall, and a spring representing the target stiffness.

Several views of the actual finite element mesh are shown in Figure **3,** In this figure, the views shown are: (a) the entire model, (b) top of rod with basket compartment walls and spacer grid spring, (c) top of rod with fuel pellet springs, and (d) bottom of rod with nodes representing the cask and target (concrete).

6.2 Fuel cladding

The fuel cladding geometry and other physical properties are presented in Table **I** and Figure 1.

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7.0 Results

The analysis results show that the maximum principal strain of fuel rod is 0.89 %, which is less than the yield strain of 0.92%. The maximum principal strain time-history is shown in Figure 9; and correspondingly the maximum principal strain profile is shown in Figure 10.

In addition, the fuel rod and cask velocity and deceleration time histories are shown in Figure 11 and Figure 12, respectively.

8.0 Conclusions

From the above results, the maximum principal strain for the fuel cladding is 0.89%, which is less than the elastic strain of 0.92%. Hence, based on all of the conservatisms in the current analysis

u it can be concluded that there is no plastic deformation in the tuel cladding and the fuel cladding maintains its structural integrity during the 80 inch end drop event

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 $\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right) ^{2}$

 $\label{eq:2} \frac{1}{2} \int_{\mathbb{R}^3} \left| \frac{d\mathbf{x}}{d\mathbf{x}} \right|^2 \, d\mathbf{x} \, d\mathbf{x}$

 $\label{eq:2.1} \frac{1}{2}\sum_{i=1}^n\frac{1}{2}\sum_{i=1}^n\frac{1}{2}\sum_{j=1}^n\frac{1}{2}\sum_{i=1}^n\frac{1}{2}\sum_{i=1}^n\frac{1}{2}\sum_{j=1}^n\frac{1}{2}\sum_{i=1}^n\frac{1}{2}\sum_{j=1}^n\frac{1}{2}\sum_{i=1}^n\frac{1}{2}\sum_{i=1}^n\frac{1}{2}\sum_{j=1}^n\frac{1}{2}\sum_{j=1}^n\frac{1}{2}\sum_{i=1}^n\frac{1}{2}\sum_{i=1}^n\$

 $\frac{1}{2}$

 $\label{eq:2.1} \frac{1}{2} \int_{\mathbb{R}^3} \frac{1}{\sqrt{2}} \, \frac{1}{\sqrt{2}} \,$

 $\label{eq:2} \frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1$

 $\label{eq:2} \frac{1}{\sqrt{2}}\sum_{i=1}^n\frac{1}{\sqrt{2}}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^n\frac{1}{j!}\sum_{j=1}^$

 $\mathcal{A}^{(1)}$

 $\mathcal{F}_{\mathcal{A}}$

 $\label{eq:2.1} \mathcal{L}_{\text{max}}(\mathbf{r}) = \mathcal{L}_{\text{max}}(\mathbf{r}) \mathcal{L}_{\text{max}}(\mathbf{r})$

 $\mathcal{O}(10^{-10})$. The contract of the contract

 $\label{eq:2} \frac{1}{\sqrt{2}}\int_0^1\frac{dx}{y^2}dx$

 $\mathcal{O}(\mathcal{O}_\mathcal{O})$. The contract of the co

 $\label{eq:2} \frac{1}{\sqrt{2}}\int_{0}^{\pi}\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^{2}d\theta\,d\theta.$

 $\label{eq:2.1} \frac{1}{\sqrt{2\pi}}\sum_{i=1}^n\frac{1}{\sqrt{2\pi}}\left(\frac{1}{\sqrt{2\pi}}\right)^2\left(\frac{1}{\sqrt{2\pi}}\right)^2\left(\frac{1}{\sqrt{2\pi}}\right)^2.$

 $\label{eq:2.1} \begin{split} \mathcal{L}(\mathbf{z},\mathbf{w},\mathbf{z})&=\mathcal{L}(\mathbf{z},\mathbf{w})\mathcal{L}(\mathbf{z},\mathbf{w})\\ &=\mathcal{L}(\mathbf{z},\mathbf{w})\mathcal{L}(\mathbf{z},\mathbf{w})\mathcal{L}(\mathbf{z},\mathbf{w})\mathcal{L}(\mathbf{z},\mathbf{w})\mathcal{L}(\mathbf{z},\mathbf{w})\mathcal{L}(\mathbf{z},\mathbf{w})\mathcal{L}(\mathbf{z},\mathbf{w})\mathcal{L}(\mathbf{z},\mathbf{w$

 $\label{eq:2.1} \frac{1}{\sqrt{2}}\int_{\mathbb{R}^3}\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2\frac{1}{\sqrt{2}}\left(\frac{1}{\sqrt{2}}\right)^2.$