### ATTACHMENT 5

### CALCULATION OF BREAK FLOW DURING POSTULATED ALTERNATE HIGH-HEAD RECIRCULATION

CONSOLIDATED EDISON COMPANY OF NEW YORK, INDIAN POINT UNIT NO. 2 DOCKET NO. 50-247 JANUARY, 1997

9701070063 970103 PDR ADDCK 05000247 P PDR

#### **CON EDISON CALCULATION / ANALYSIS COVER SHEET**

Calculation Number: FPX-00139-00 Entry Date: 12/24/96

Type: CA34 PIPE FLOW

> **Modification:** NONE Scanned: N Revision:

Title MAXIMUM FLOW THROUGH LINE #183 (HIGH HEAD RECIRC)

Drawings 9321-2580

Component Description PIPE

**Project Number: NONE** 

Document Page : 8

Old Calculation:

Component Style Style Description PIPE PIPE OR PIPING

System Description SIS SAFETY INJECTION

Structure Description PRIMARY AUXILIARY BUILDING PAB

Preparer: KIMB Update Date Reviewer: SHALABI Signature: 12/27/96 Signature/Date: Approval/Date Confirm. Required?-

Concurrence (If Required)

12/27/96



Туре PIPE

# Page 2 of 8

## CON EDISON CALCULATION/ANALYSIS

1

. . .

# Description of Change Sheet

culation No: FP	Description of Change	Reason for Change
60	ORIGINAL ISSUE	
• •		
		· · · · · · · · · · · · · · · · · · ·

CON EDISON CALCULATION/	CALCULATION NO	REV	PAGE	OF
ANALYSIS SUMMARY SHEET	FPX-00139-00	00	3	8
PREPARERODATE 1. Kimball /2/30/90	REVIEWER/DATS K. Shalabi	130/96	CLASS	
SUBJECTITLE Maximum Flow Rate Through Line #183.			PROJECT NO	
			MOD NO N/A	REV

#### **OBJECTIVE OF CALCULATION**

The objective of this calculation is to determine the maximum flow rate through line #183 for the following scenario:

A total loss of all the weld around the first elbow on line #183 off line #155 for the high head recirc mode.

#### CALCULATION METHOD/ASSUMPTIONS

An analytical flow vs. head loss calculation is made using <u>Cameron Hydraulic Data</u> methods.

- Because of the low flow calculated for the small leak area, only the losses associated with the flow within the elbow/pipe joint will be considered.
- 2. The calculation assumes a conservative value for pressure at the elbow by not including any system friction losses .
- 3. The differences in density and viscosity of pure water and 2000 ppm borated water are assumed to have no effect on the flow.

#### DESIGN BASIS AND REFERENCES

The design basis is not applicable for this calculation.

- L. Cameron Hydraulic Data, 1977.
- 2. Piping Handbook, Crocker & King, 5th Edition.

#### CONCLUSIONS

The calculated flow for loss of all weld is 0.75 gpm.

•	CON EDISON CALCULATION/	CALCULATION NO FPX:00139-00	REV 00	PAGE	OF
	ANALYSIS SHEET PREPARER/DATE I Kumball Manual 15/20/21	REVIEWER/DATEL	12/20/96	CLASS	<u> </u>
	SUBJEC PHILE Maximum Flow Rate Through Line #183.		1-12-14	PROJECT NO	
				MOD NO N/A	REV

## Calculate maximum flow rate through 2" line #183 for loss of all weld around elbow.

(See Figure #1. page 8)

Use  $h_1 = K V^2/2g$  for the areas along the flow path (i.e. at the Ref. 1, p. 3–102 entrance, the "reducing elbow," and exit) and  $h_1 = 0.002083L (100/C)^{1.85}q^{1.85}/d^{4.8655}$  for the friction loss along Ref. 1, p. 3-7 the area inside of the elbow,

where,

 $h_1 = head loss, in feet,$ 

K = friction coefficient for losses at piping anomolies,

V = velocity of flow, in ft/sec,

 $g = acceleration of gravity, in ft/sec^2$ 

L = length of pipe, in feet

C = roughness constant for type of pipe

q = flow, in gpm

d = diameter of pipe.

So, for our case, in the direction of the flow,  $h_{t} = K_{cat} V_{cat}^{2}/2g + K_{eib} V_{eat}^{2}/2g + K_{red} V_{eat}^{2}/2g + 0.002083L (100/C)^{1.85} q^{1.85}/d^{4.8655} + K_{eat} V_{eat}^{2}/2g.$ 

Combining,

 $h_1 = (K_{ent} + K_{elb} + K_{red}) V_{ent}^2/2g + 0.002083L (100/C)^{1.85} q^{1.85}/d^{4.8655} + K_{exit} V_{exit}^2/2g. (Eq. 1)$ 

Use,  $K_{ent} = 0.5$ ,  $K_{elb} = 0.81$ ,  $K_{red} = 0.5(1 - d_{H1}^2/d_{H2}^2)$ , )(Eq. 2) and,  $K_{red} = 1$ . Ref. 1, p. 3-104 Ref. 1, p. 3-109 Ref. 1, p. 3-108



CON EDISON CALCULATION/	CALCULATION NO	REV	PAGE	OF
ANALYSIS SHEET	FPX-00139-00	00	5	8
PREPARER/DATE I Kimball A finital 1/30/96	REVIEWER/DATE K. Shalabi	12/30/96	CLASS	
SUBJECT/TITLE Maximum Flow Rate Through Line #183.	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4		PROJECT NO	
			MOD NO N/A	REV

Since we are actually dealing with an annular space essentially between to different diameter pipes, we want to use the hydraulic diameter,  $D_h$ , to calculate the equivalent pipe diameter for the loss of weld area (i.e., d, from Eq. 1). And since we know that,

 $D_{t_1} = 4 R_{t_1},$ and  $R_{t_1} = Area/Wetted Perimeter$  $= \pi/4 (d_2^2 - d_1^2)/\pi (d_2 + d_1)$  $= (d_2 + d_1)(d_2 - d_1)/4 (d_2 + d_1)$  $= (d_2 - d_1)/4,$ 

where,  $d_1 = 2.416$ " for the inside diameter of the 2" elbow, and  $d_1 = 2.375$ " for the outside diameter of the 2" pipe.

 $D_{11} = 4 R_{11}$ = 4 (d<sub>2</sub> - d<sub>1</sub>)/4 = d<sub>2</sub> - d<sub>1</sub> = 2.416 - 2.375" = 0.041", or 0.00342 ft

 $- d = d_{H1}$ 

So, the hydraulic diameter,

Ref. 2, p. 7-48,49

This value will be used for d in Eq. 1 for the friction loss of the flow between the pipe and elbow.

For Eq. 2, since this is effectively a sudden contraction from the annulus area between the end of the 2" pipe and the butt of the elbow insert (defined by welding procedures as 1/16") to the annulus between the pipe OD and elbow ID, we can use the two hydraulic diameters for this calculation. Although the 1/16" gap is not oriented properly to be able to use a large and small radius/diameter, it can be closely approximated by using the value similar to that just calculated. That is, (see Fig. #1) since the space between the pipe OD and elbow ID and the gap are similar,

then we can say,  $(d_2 - d_1)/2 = 1/16^{"}$ , or  $d_2 - d_1 = 1/8^{"}$   $= 0.125^{"}$ , or 0.01042 ft  $- d_{1/2}$ 

€ 4.5	CON EDISON CALCULATION/	CALCULATION NO	REV	PAGE	OF
	ANALYSIS SHEET	<b>FPX-00139-00</b>	00	6	8
	PREPARER/DATE	REVIEWER/DATE		CLASS	
		K. Shalabi	12/30/96	A	
	SUBJECTITITLE	V		PROJECT NO	
	Maximum Flow Rate Through Line #183.				
				MOD NO	REV
ĺ				N/A	

So, for Eq. 2,  $K_{ted} = 0.5(1 - d_{H1}^2/d_{H2}^2)$   $= 0.5(1 - 0.041^2/.125^2)$  = 0.5(1 - 0.1076) = 0.5(0.8924)= 0.4462.

Which gives us for Eq. 1,

 $h_{t} = (K_{ent} + K_{elb} + K_{red}) V_{ent}^{2}/2g + 0.002083L (100/C)^{1.85}q^{1.85}/d_{H1}^{4.8655} + K_{exit} V_{exit}^{2}/2g$  $h_{t} = (0.5 + 0.81 + 0.4462) V_{ent}^{2}/2g + 0.002083L (100/C)^{1.85}q^{1.85}/d_{H1}^{4.8655} + (1) V_{exit}^{2}/2g,$  $1.7562 V_{ent}^{2}/2g + 0.002083L (100/C)^{1.85}q^{1.85}/d_{H1}^{4.8655} + V_{exit}^{2}/2g. (Eq. 3)$ 

Now, for the velocity of the flow at the entrance and exit, we can use  $V = 0.4085 q/d_{H}^2$  (Eq. 4)

where.

 $d_{\rm H}$  = the hydraulic diameter for that portion of the "pipe."

So, by substituting Eq. 4 into Eq. 3 for each V, we get,

$$\begin{split} h_1 &= 1.7562 \ (0.4085q/d_{H2}^2)^2/2g + 0.002083L \ (100/C)^{1.85}q^{1.85}/d_{H1}^{4.8655} + (0.4085q/d_{H1}^2)^2/2g \\ &= 1.7562 \ [(0.4085)^2q^2/d_{H2}^4]/2g + 0.002083L \ (100/C)^{1.85}q^{1.85}/d_{H1}^{4.8655} + [(0.4085)^2q^2/d_{H1}^4]/2g \\ &= 1.7562 \ (0.1669)q^2/[(0.125^4)2(32.174)] + 0.002083L \ (100/C)^{1.85}q^{1.85}/d_{H1}^{4.8655} + \\ &\quad (0.1669)q^2/[(0.041^4)2(32.174)] \\ &= 1.8576 \ q^2 + 0.002083L \ (100/C)^{1.85}q^{1.85}/d_{H1}^{4.8655} + 923 \ q^2. \end{split}$$

From Fig. #1,

1. = (5/8" - 1/16")/12= (0.625 - .0625)/12= (0.5625)/12 = 0.04688'.

Since this is not new piping, use C = 100.

Ref. 1, p. 3-8

So,

$$\begin{array}{l} h_1 &= 18.76 \; q^2 + 0.002083 L \; (100/C)^{1.85} q^{1.85} / \; d_{H1} \overset{4.8655}{=} + 923 \; q^2, \\ &= 18.76 \; q^2 + 0.002083 (0.04688) q^{1.85} / \; (0.041)^{4.8655} + 923 \; q^2 \\ &= 18.76 \; q^2 + 548.5 \; q^{1.85} + 923 \; q^2 \\ &= 941.8 \; q^2 + 548.5 \; q^{1.85}. \end{array}$$

CON EDISON CALCULATION/	CALCULATION NO	REV	PAGE	OF.
ANALYSIS SHEET	FPX-00139-00	00	7	8
PREPARER/BATE. I. Kimball 12/30/96	REVIEWER/DATE K. Shalabi	12/30/46	CLASS A	
SUBJECTIVITLE Maximum Flow Rate Through Line #183.			PROJECT NO	
			MOD NO N/A	REV

Since the shut-off head for the RHR Pumps is 150 lbs,

assume pressure inside the VC is 50 lbs, and

because the pipe is located higher than the full sump (77' - 51'),

we can conservatively say  $h_L = RHR$  Pump discharge pressure + Maximum VC Pressure + Elevation differences,

or  $h_L = 150 \text{ lbs} + 52 \text{ lbs} + (51' - 77')$ = 202 lbs - 26' = 202 (2.46) -26

=497 - 26 = 471

Which gives us the following:

$$471 = 941.8 q^2 + 548.5 q^{1.85}$$

However, the Hazen and Williams formula for friction loss assumes water at 60°F. As noted in the reference, the friction for water can decrease as much as 40% between 32 and 212°F. Thus, to account for the increased temperature of the water in our case (260°F), Ref. 1, p. 3-7

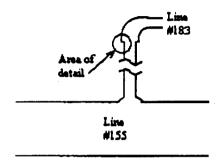
 $(260\ 60)/(212-32) \times 40\% =$ 200/180 x 0.4 = 0.444, say 44.4%.

So.  $466 = (1.00-0.444) (941.8 q^2 + 548.5 q^{1.85})$  $466 = 523.64 q^2 + 304.97 q^{1.85}$ 

Substituting for q by trial and error, we get

q = 0.7388 gpm, say 0.75 gpm.

1, <sup>14</sup> i	CON EDISON CALCULATION/	CALCULATION NO	REV	PAGE	OF
	ANALYSIS SHEET	FPX-90139-00	00	8	8
	PREPARER/DATE / /	REVIEWER/DATE	( )	CLASS	
	I. Kmball Muschell 12/30/96	K. Shalabi	12 3096	Α	
	SUBJECT/TITLE	- • • • •	· - y · 1 ·	PROJECT NO	1
	Maximum Flow Rate Through Line #183.				
				MOD NO	REV
				N/A	



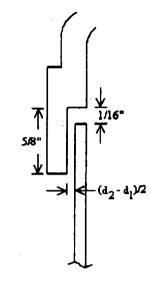


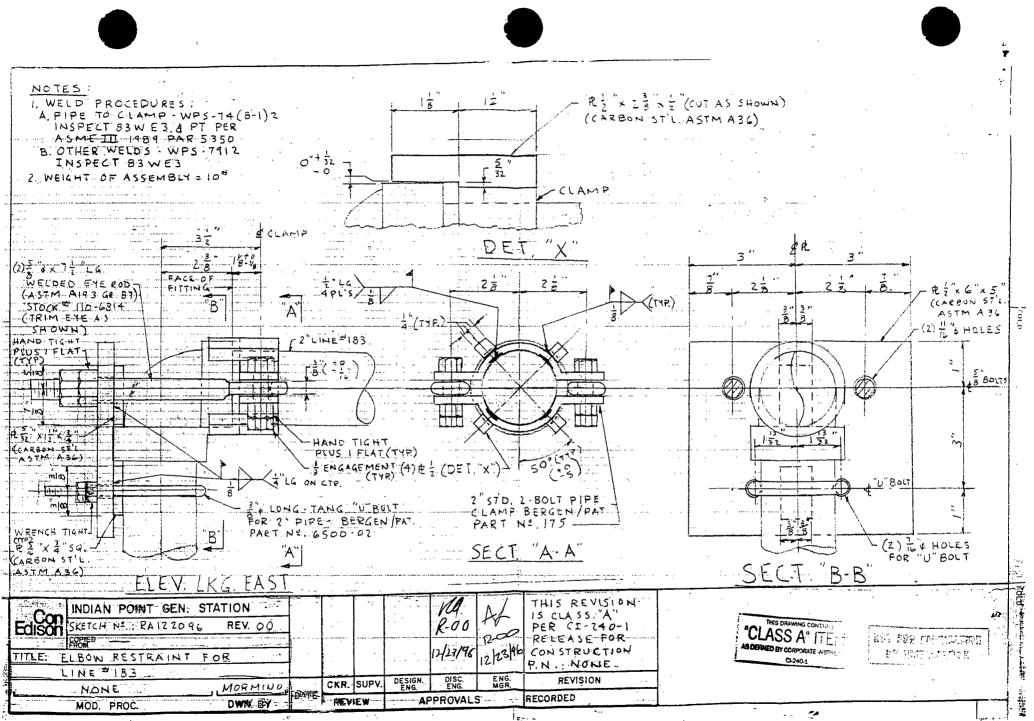
Figure #1

#### ATTACHMENT 6

.

#### ELBOW RESTRAINT

CONSOLIDATED EDISON COMPANY OF NEW YORK, INDIAN POINT UNIT NO. 2 DOCKET NO. 50-247 JANUARY, 1997



and a second second

4 • • • •