Figure 2.5-115—{Plasticity Chart}

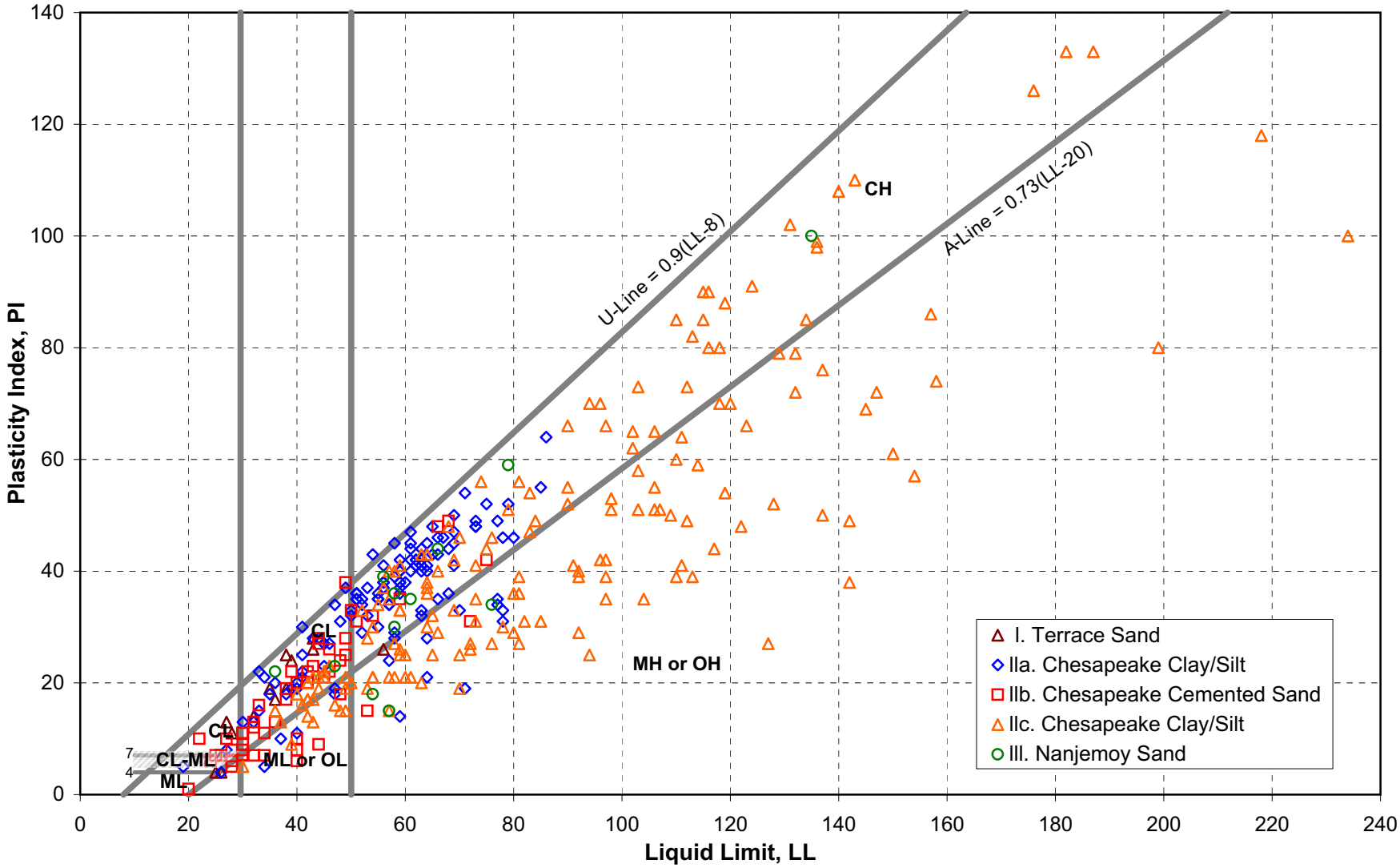


Figure 2.5-116—{Friction Angle Interpretation from CPT Results}

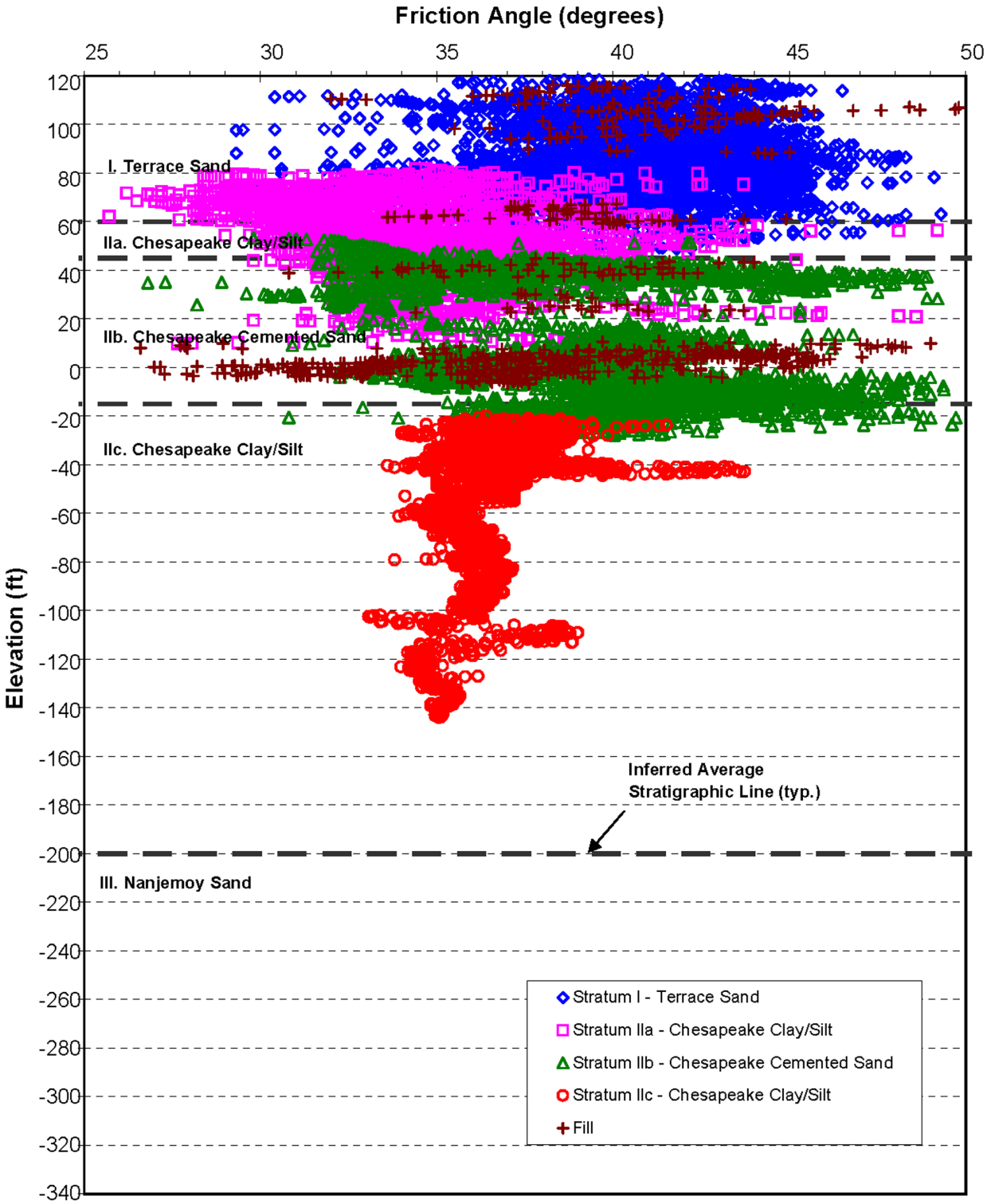


Figure 2.5-117—{Preconsolidation Pressure from Laboratory Testing}

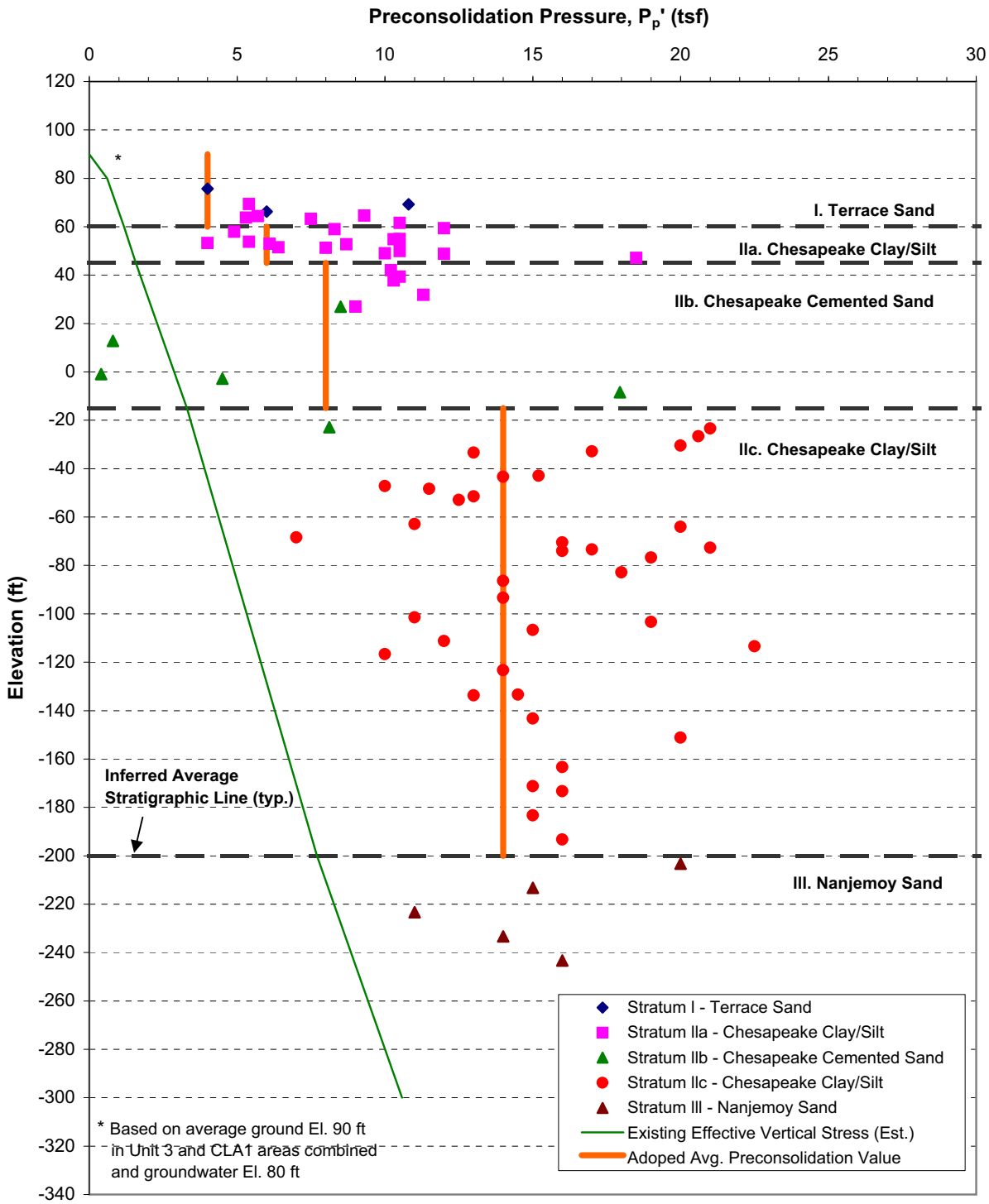


Figure 2.5-118—{OCR Interpretation from CPT Results}

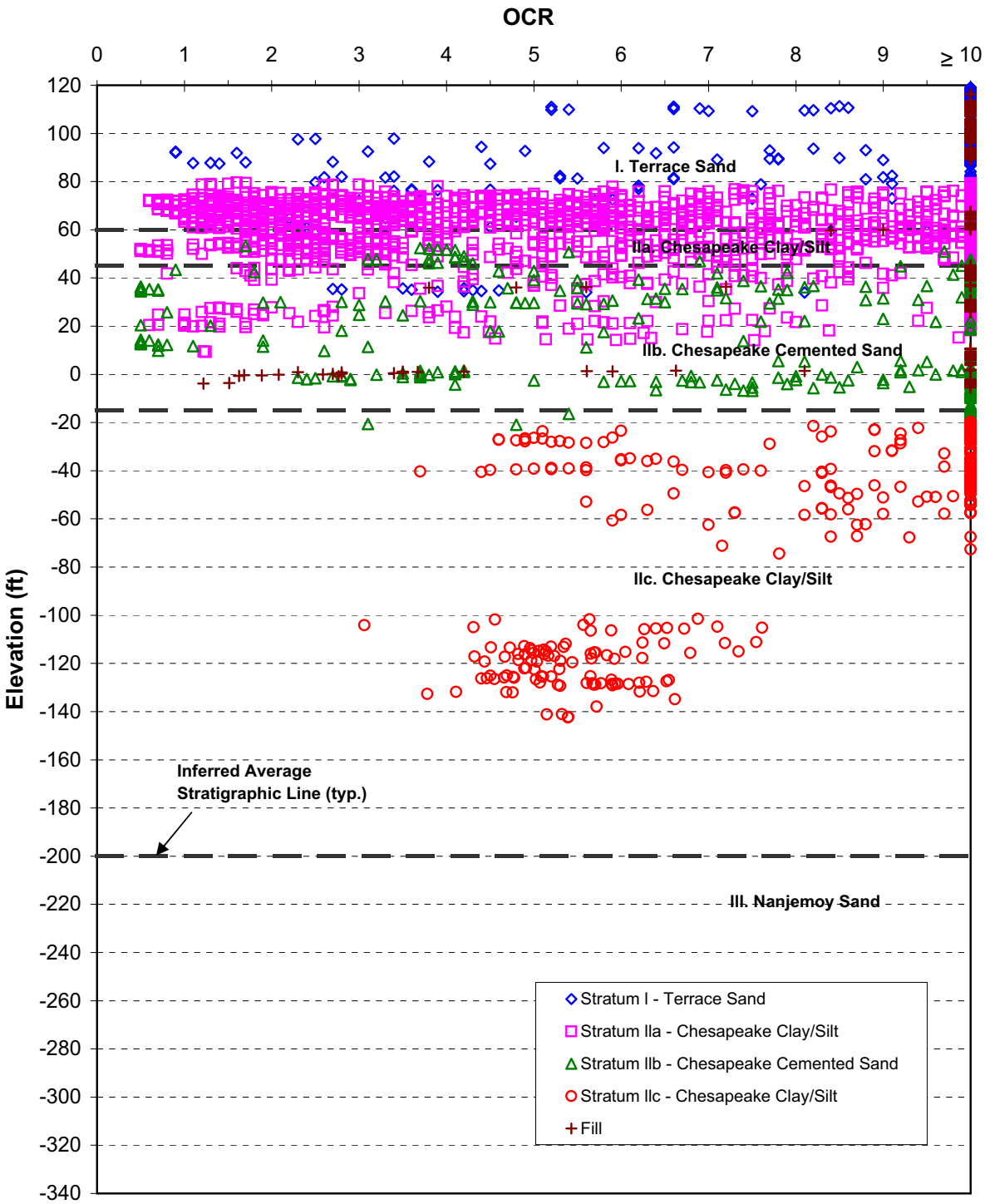


Figure 2.5-119—{Undrained Shear Strength from Laboratory Tests}

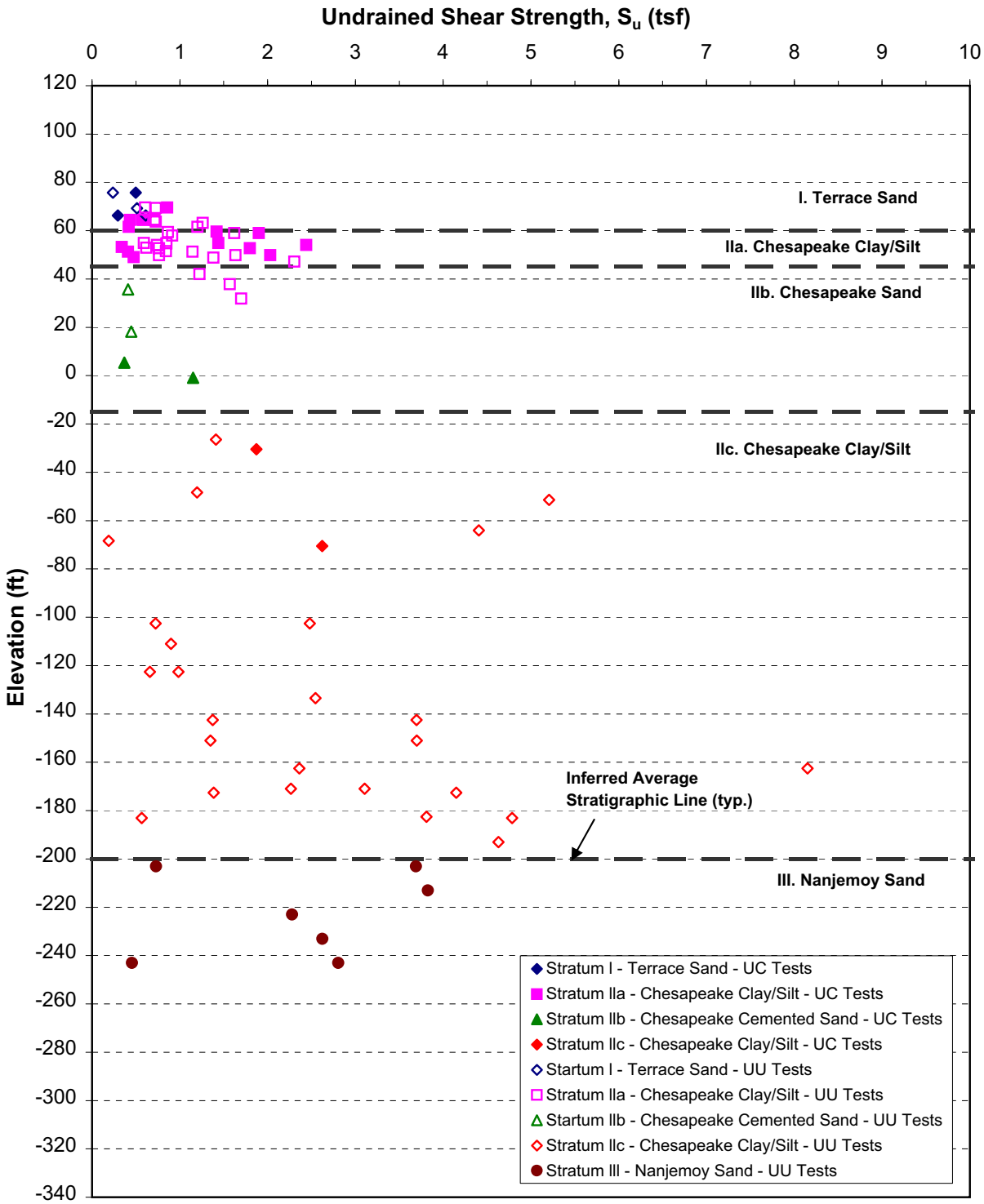


Figure 2.5-120—{Undrained Shear Strength Interpreted from CPT Data}

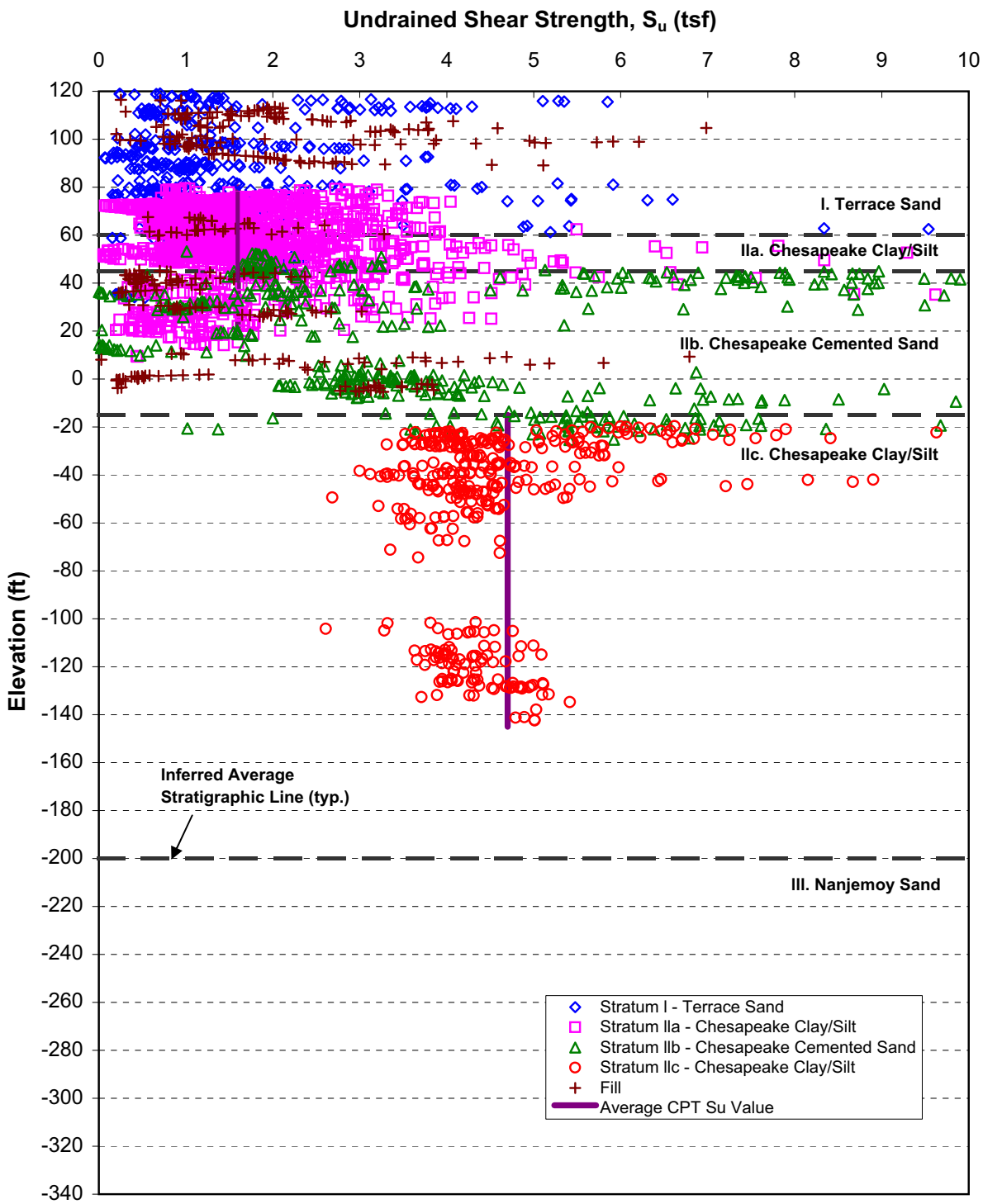


Figure 2.5-121—{Uphole Seismic Survey Results from UFSAR}

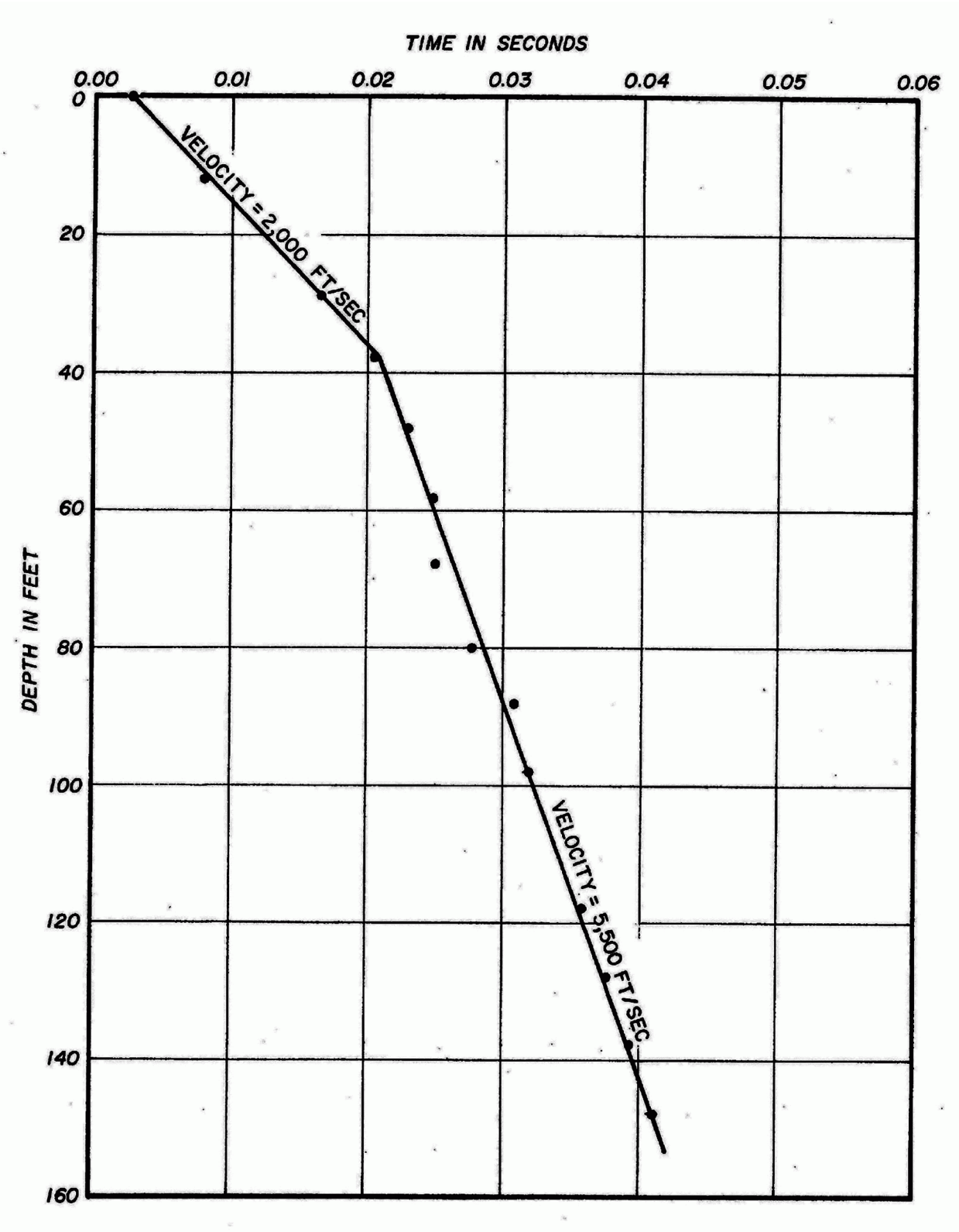


Figure 2.5-122—{Geophysical Model of the Site for Units 1 and 2 from UFSAR}

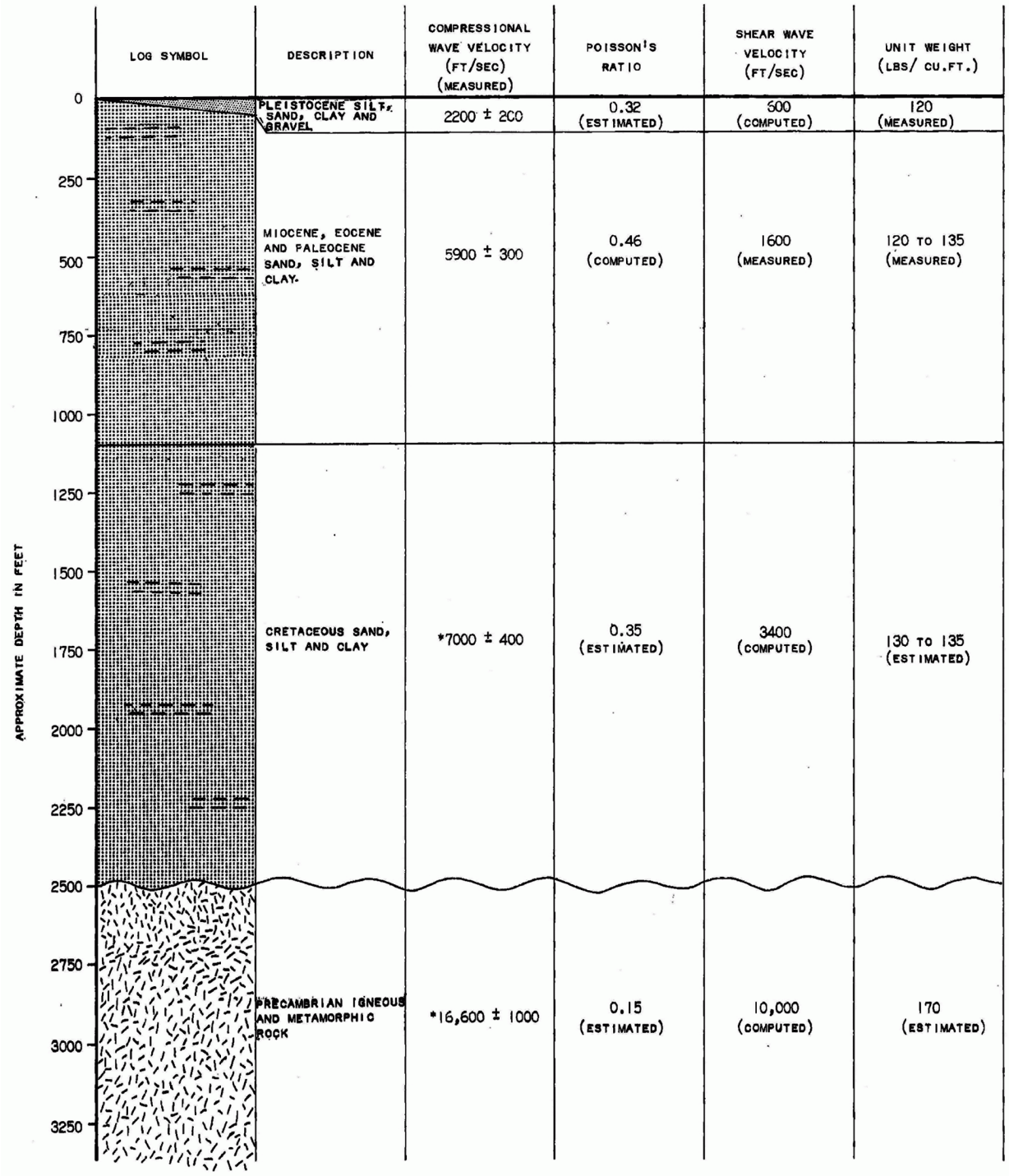


Figure 2.5-123—{V_P Measurements from Suspension P-S Velocity Logging}

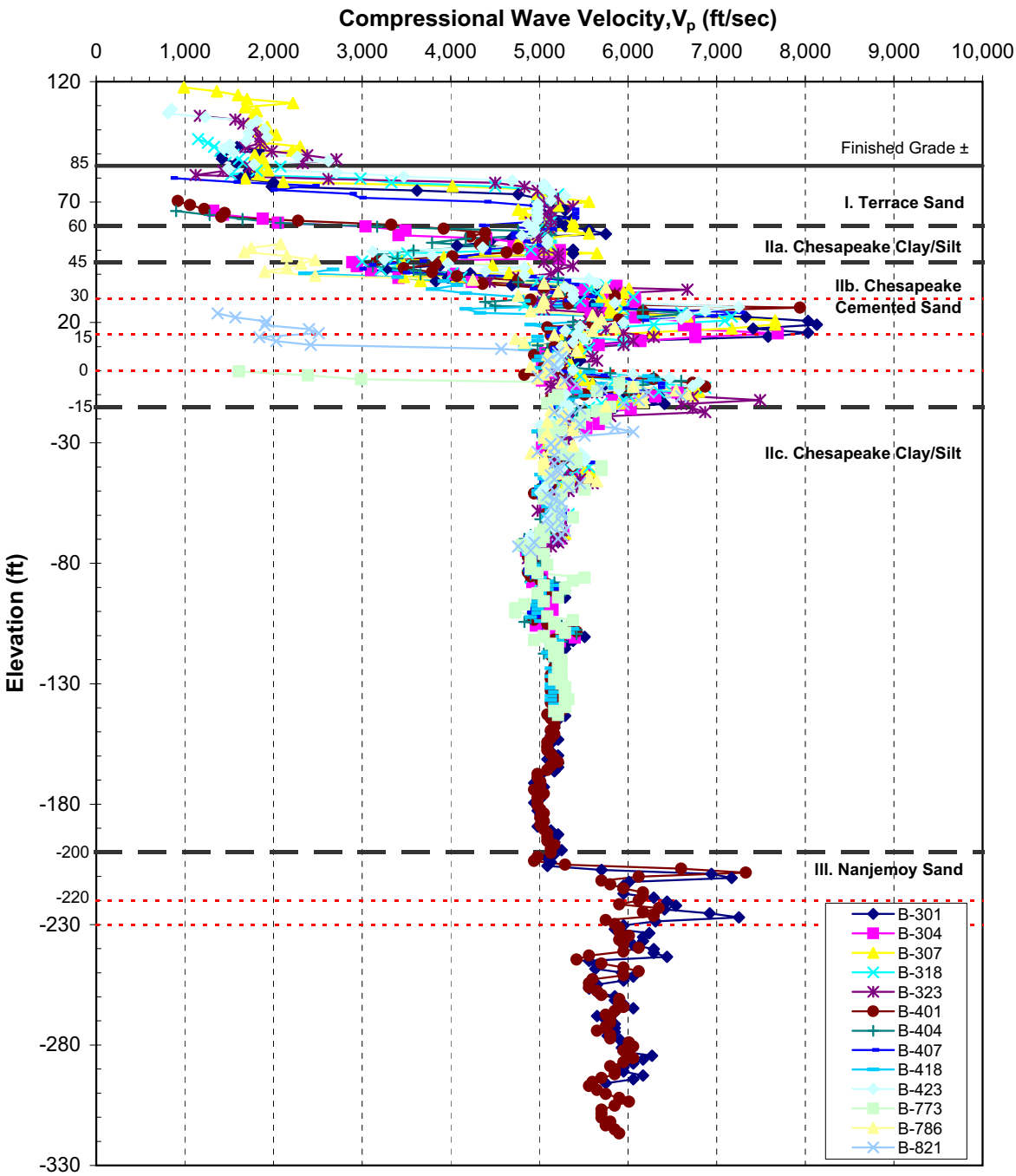


Figure 2.5-124—{V_S Measurements from Suspension P-S Velocity Logging}

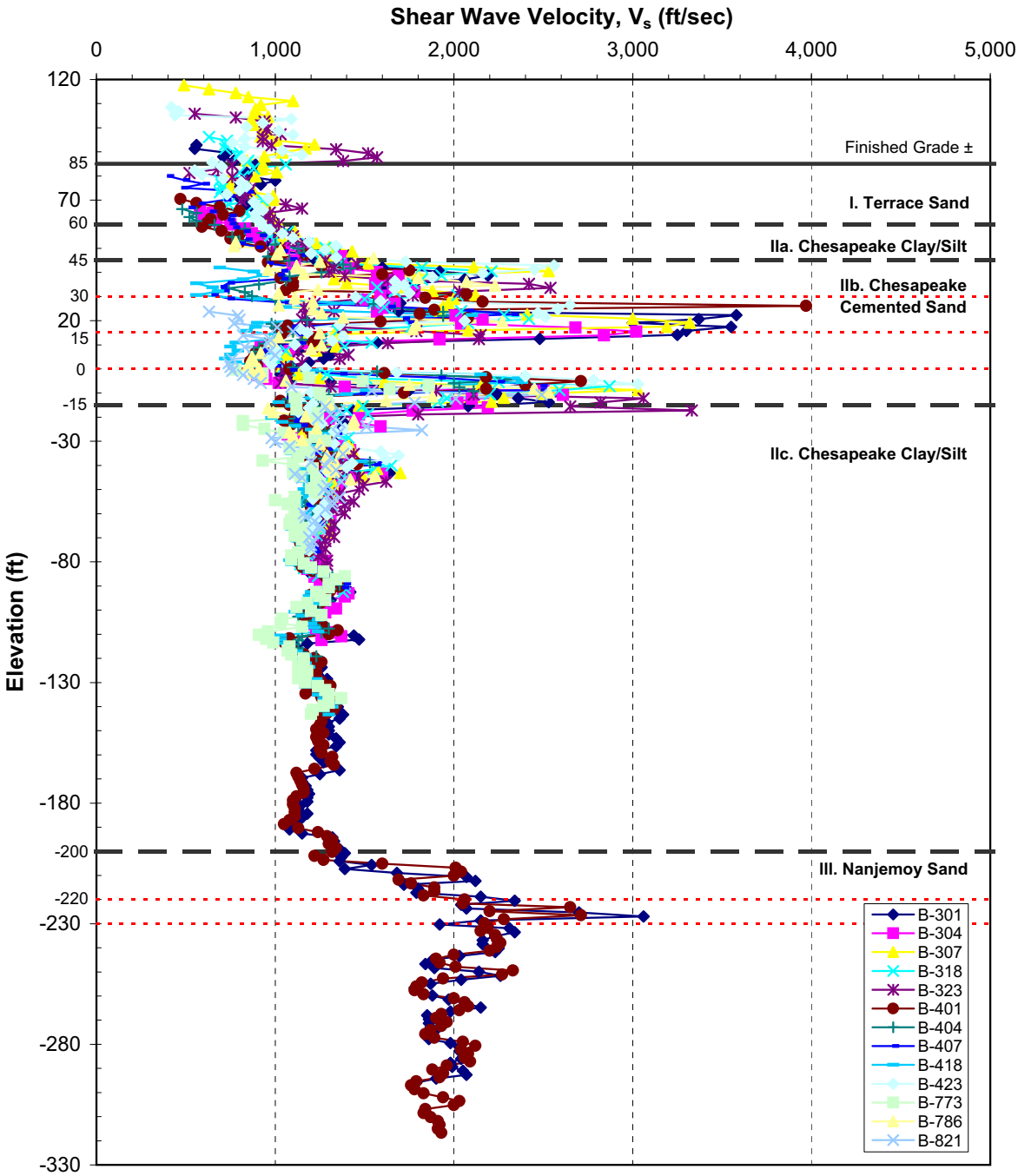


Figure 2.5-125—{Average V_S Measurements from Suspension P-S Velocity Logging}

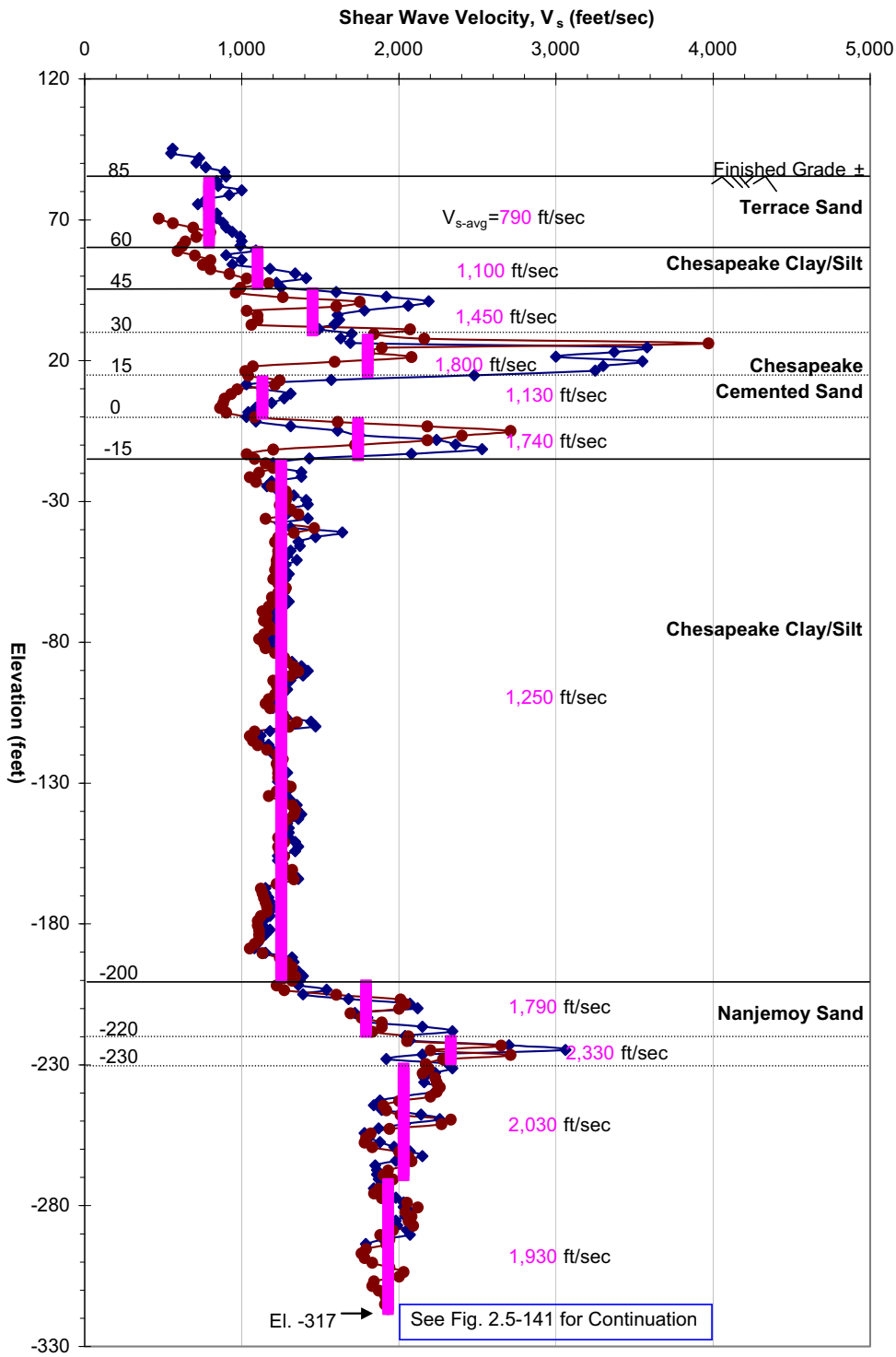


Figure 2.5-126—{Poisson's Ratio from Suspension P-S Velocity Logging}

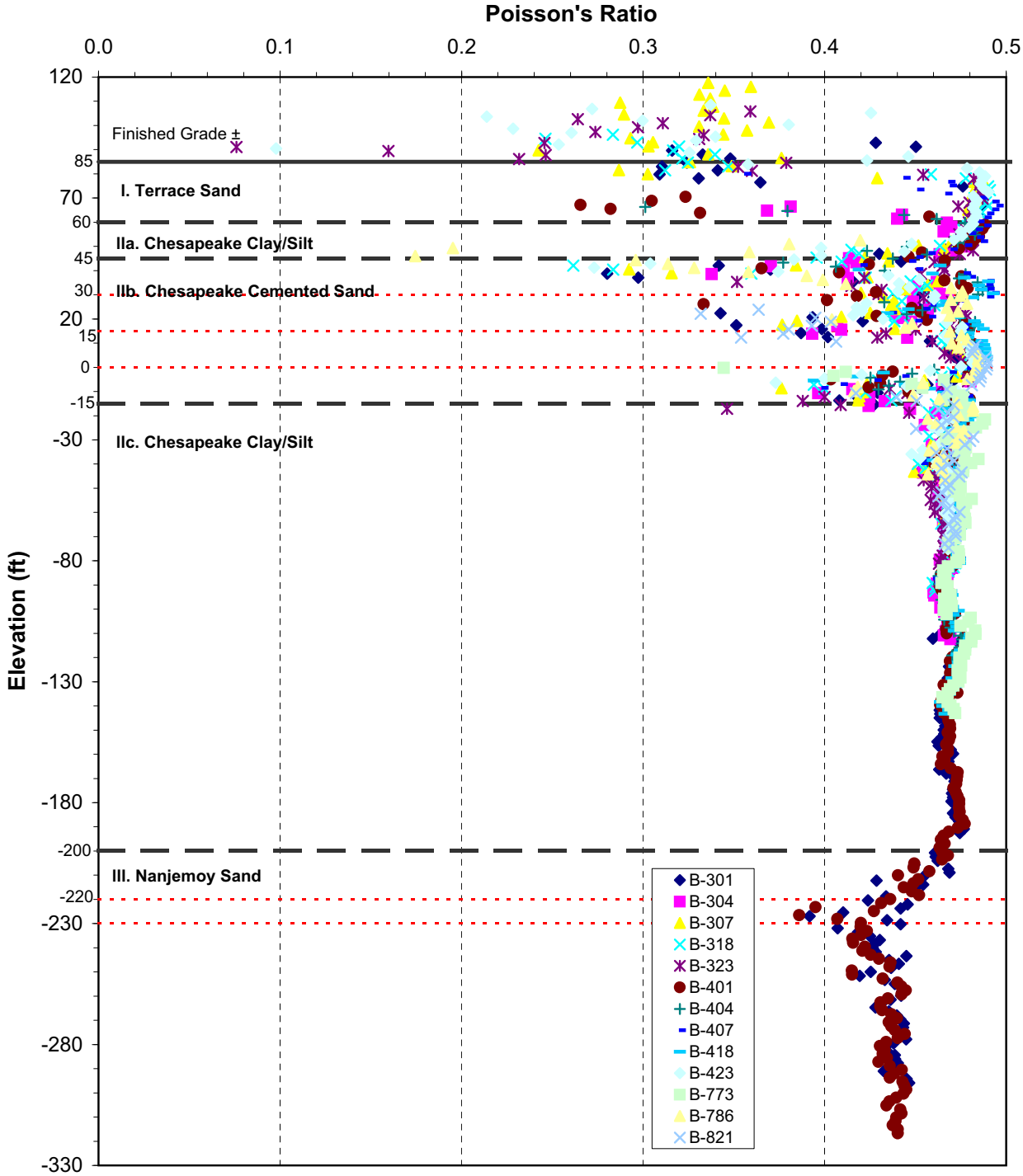


Figure 2.5-127—{Average Poisson's Ratio from Suspension P-S Velocity Logging}

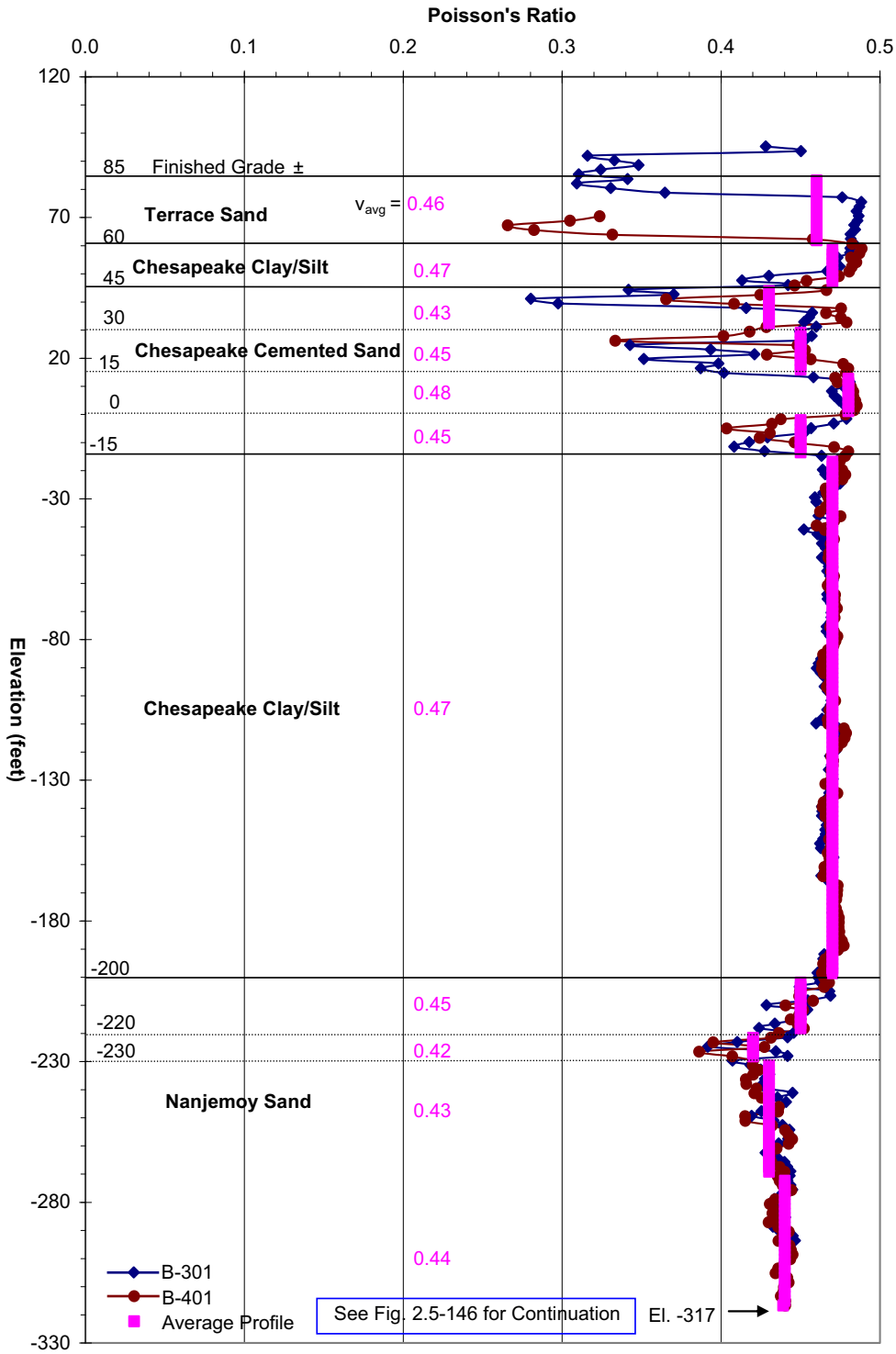


Figure 2.5-128—{Comparison of V_s Measurements from Suspension P-S Velocity Logging and CPT Seismic Tests}

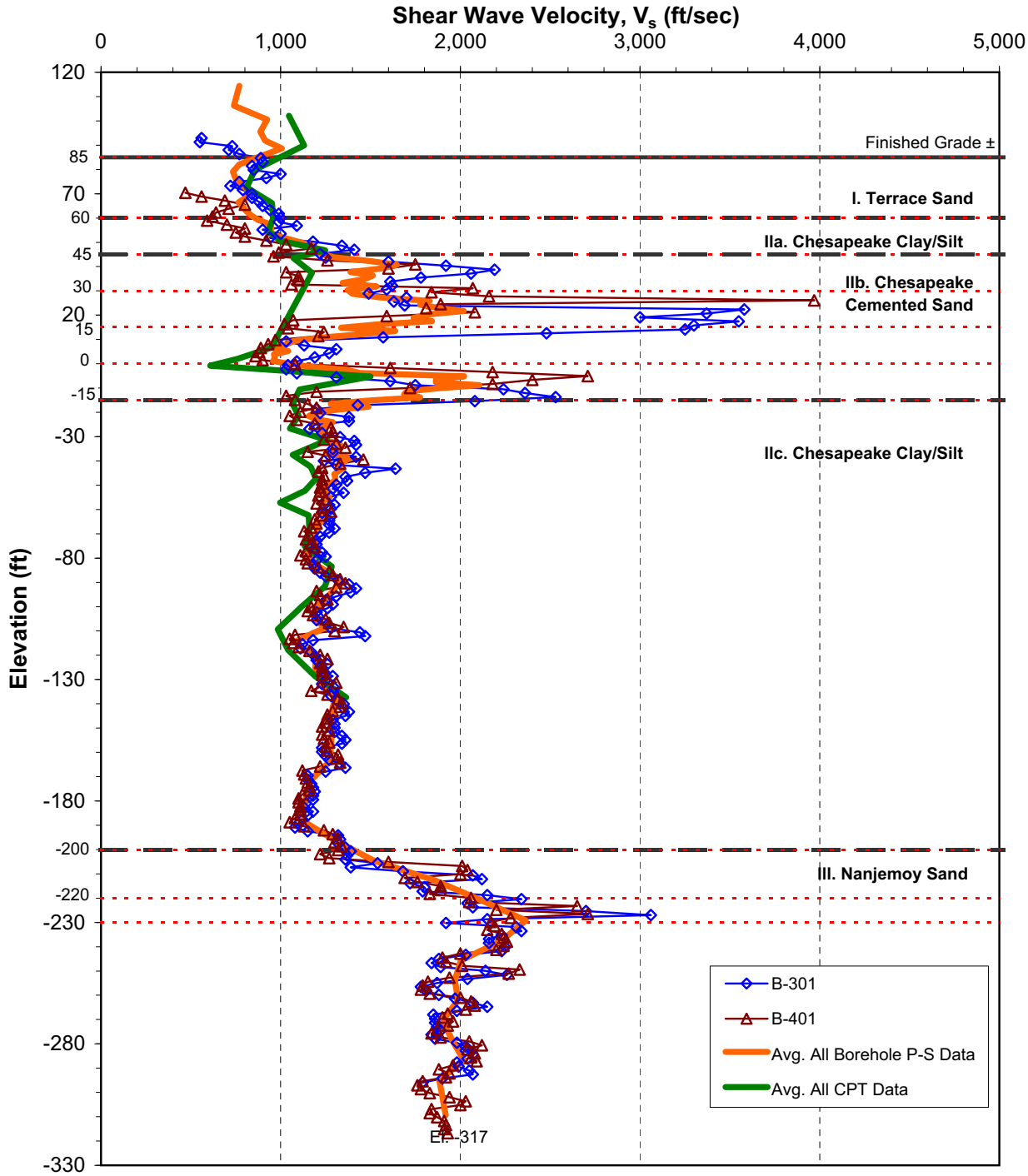


Figure 2.5-129—{Site Grading Plan}

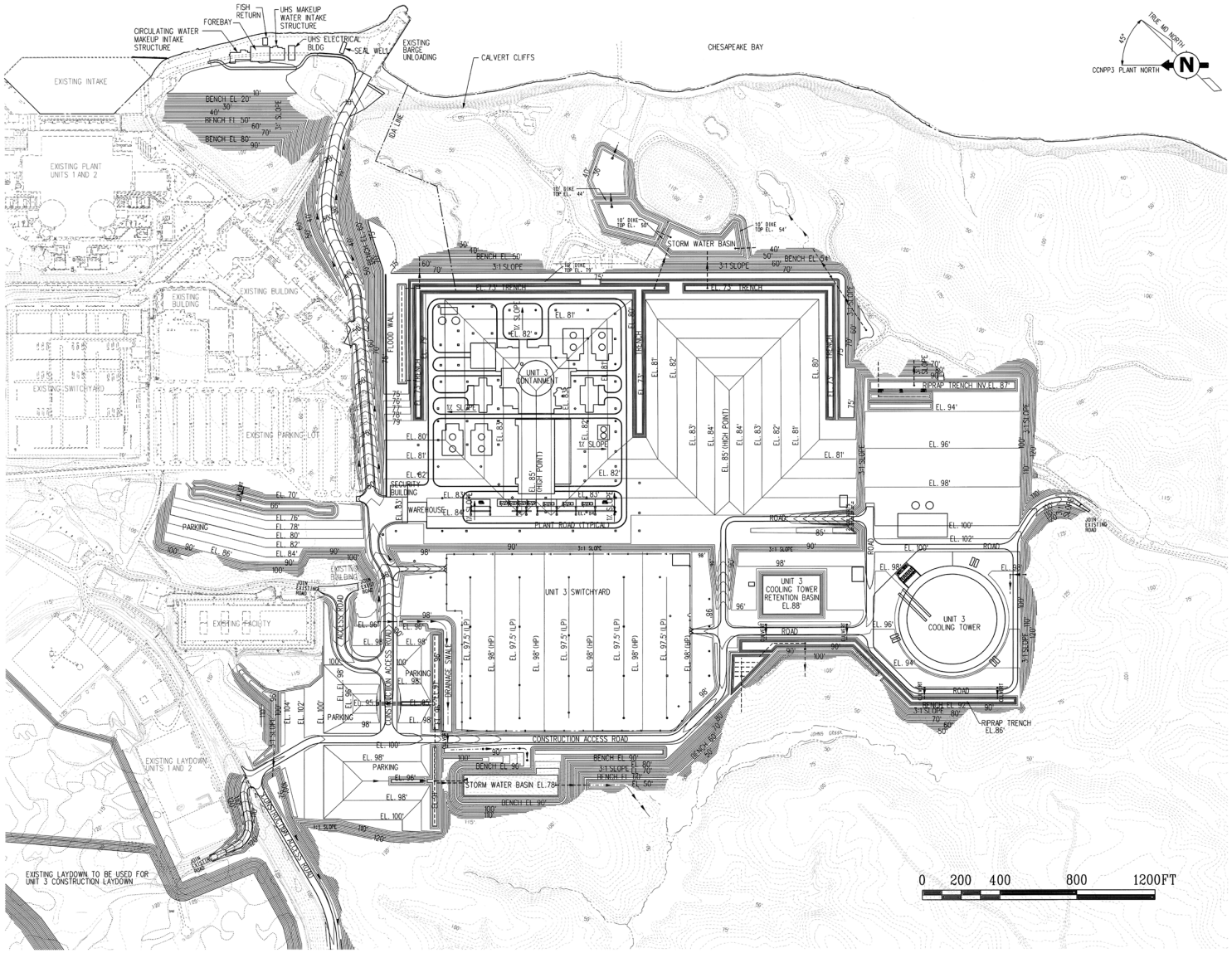


Figure 2.5-130—{Excavation Profile NS-1}

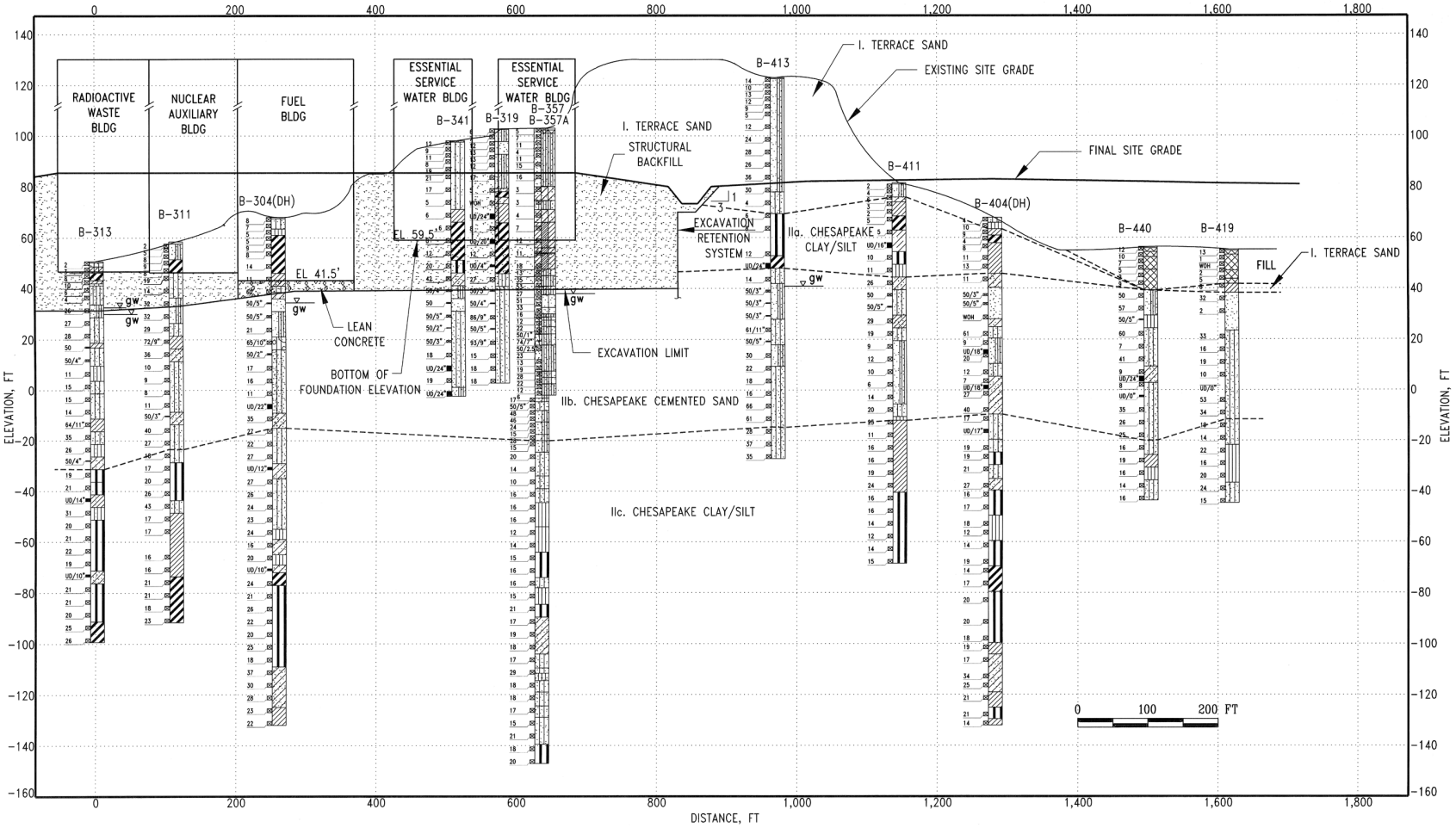


Figure 2.5-131—{Excavation Profile NS-2}

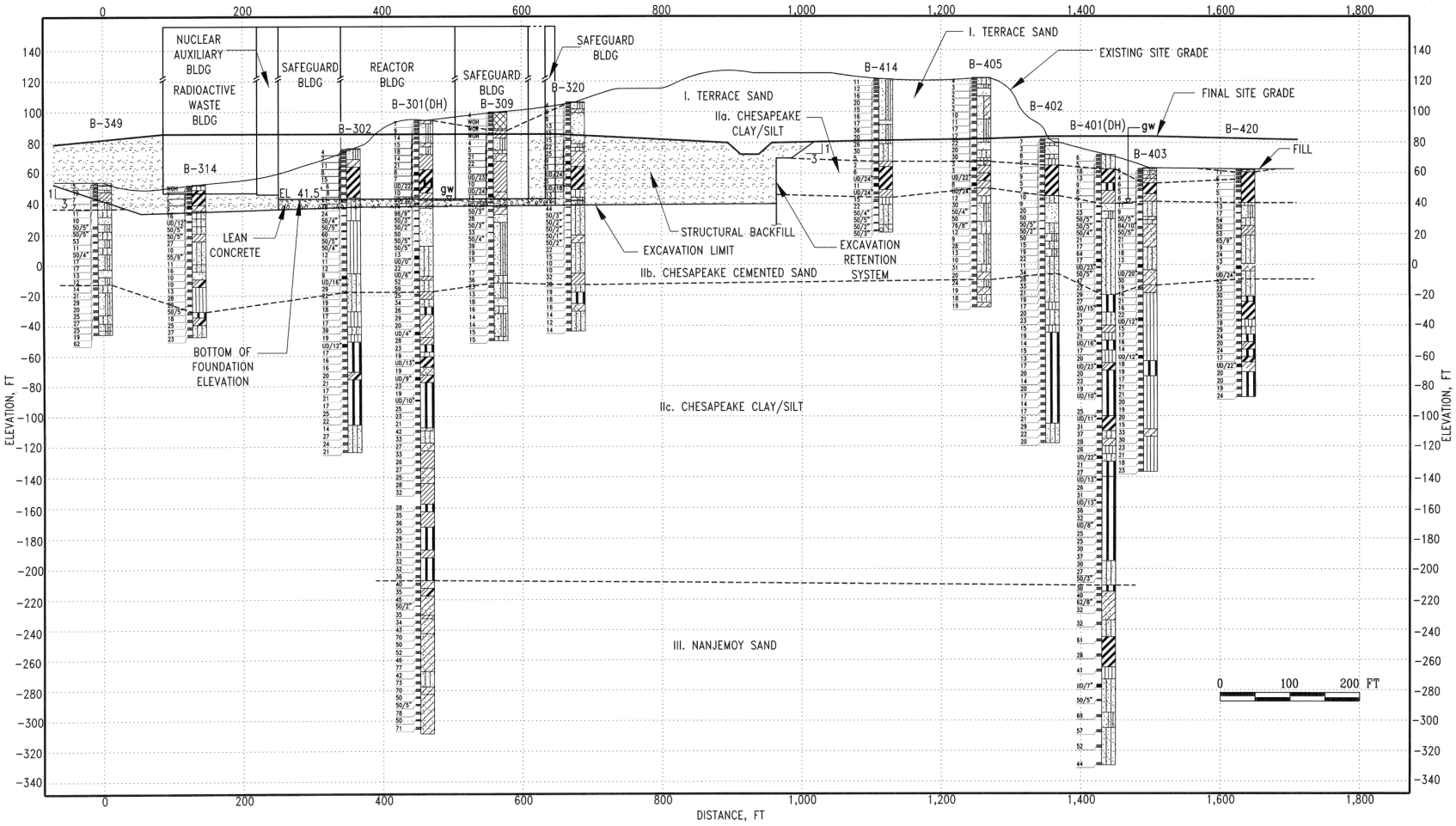


Figure 2.5-132—{Excavation Profile NS-3}

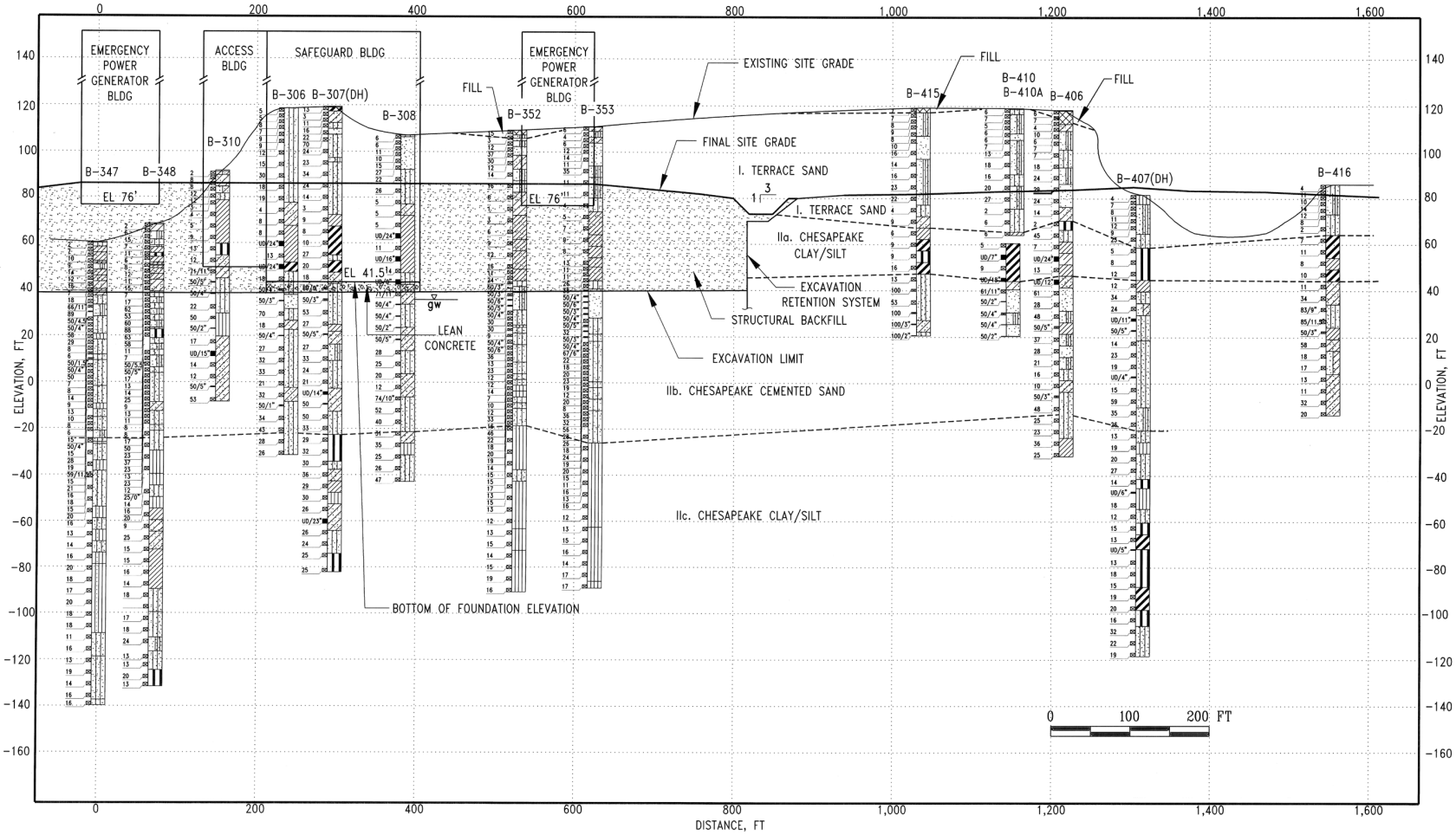


Figure 2.5-133—{Excavation Profile NS-4}

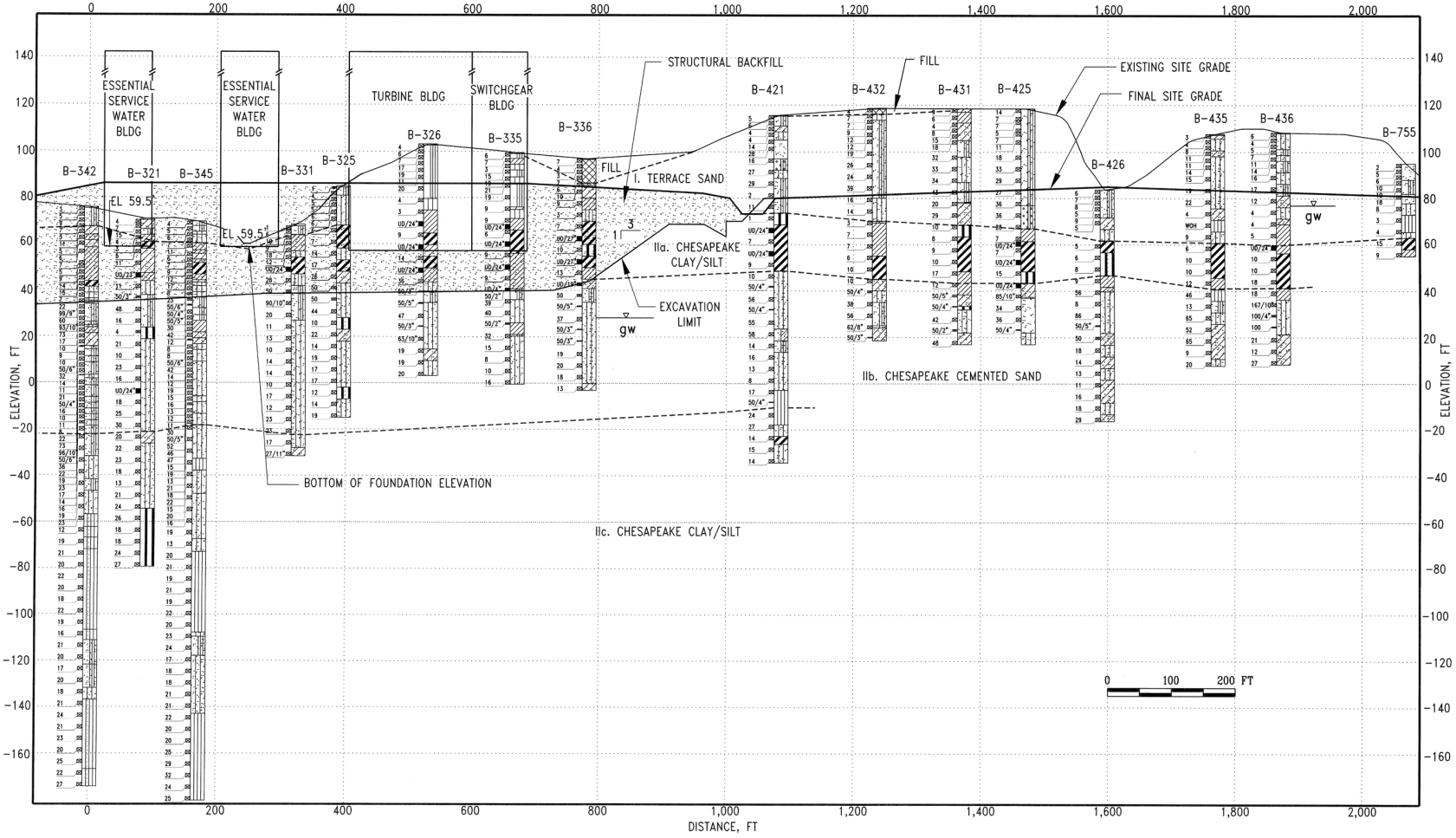


Figure 2.5-134—{Excavation Profile IDP1}

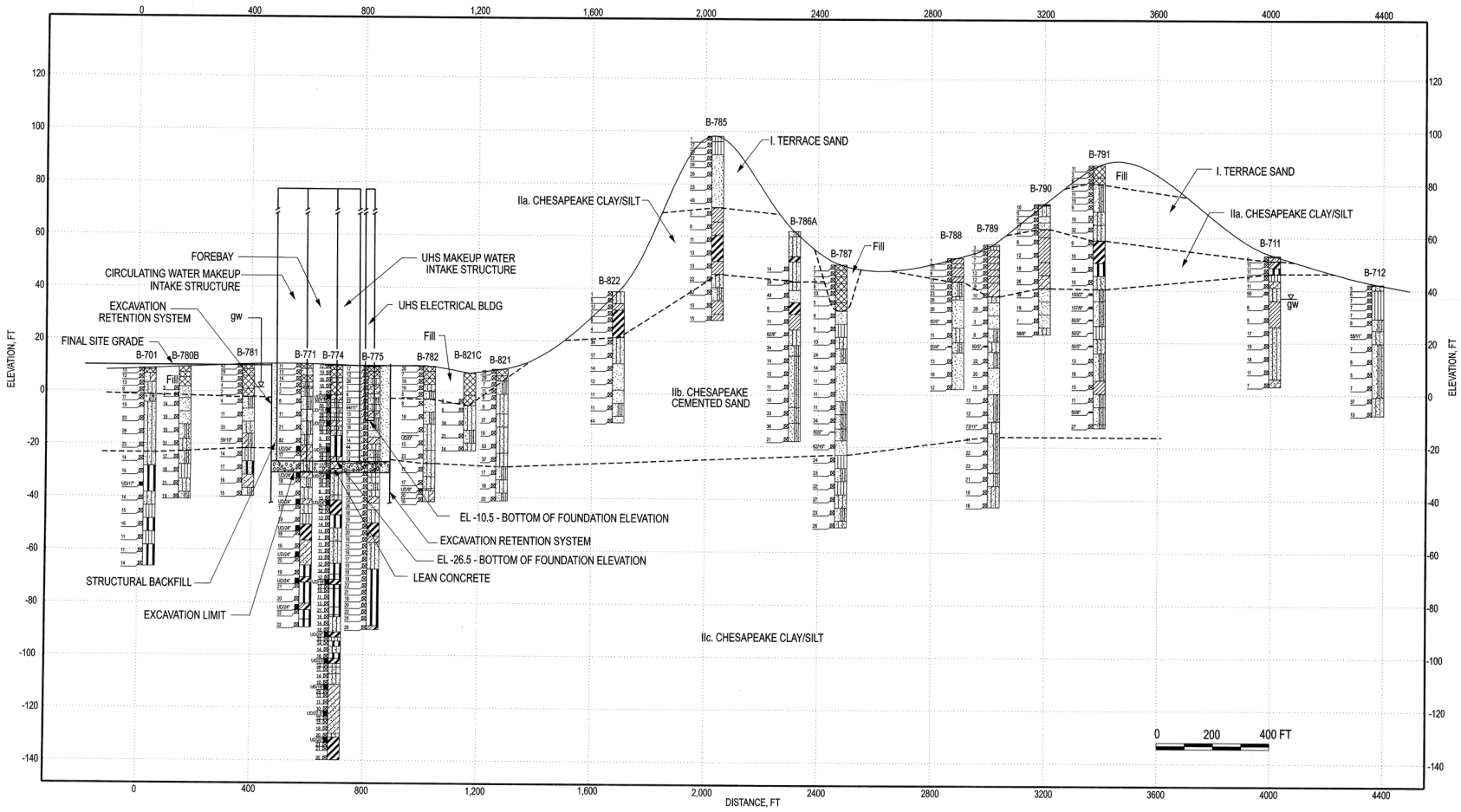
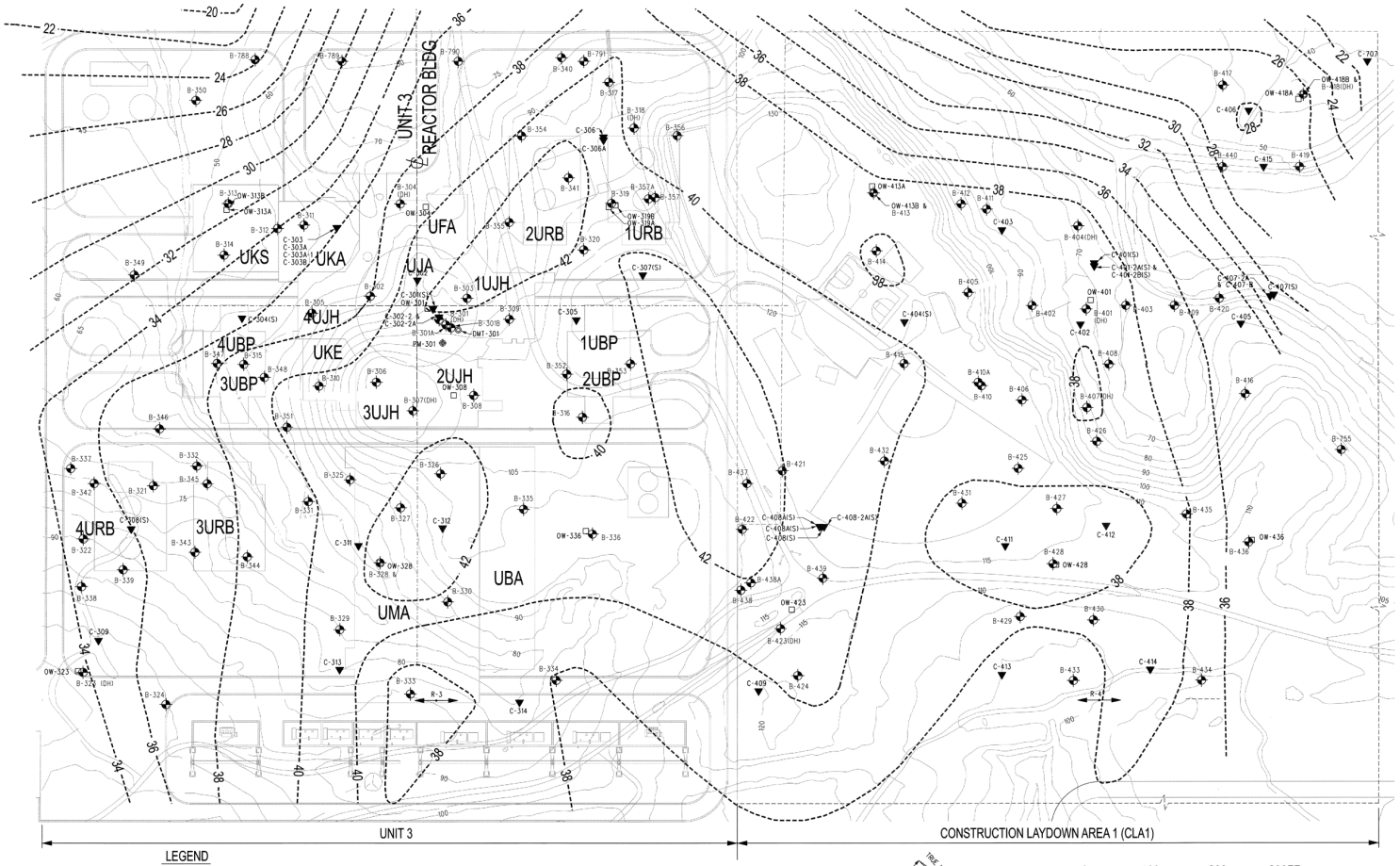


Figure 2.5-135—{Elevation Contours of Top of Stratum IIb Cemented Sand}



LEGEND

1-4 UBP EMERGENCY POWER GENERATING BUILDINGS
UFA FUEL BUILDING
UJA REACTOR BUILDING
1-4 UJH SAFEGUARD BUILDINGS MECHANICAL/ELECTRICAL
UKA NUCLEAR AUXILIARY BUILDING
UKE ACCESS BUILDING
UKS RADIOACTIVE WASTE PROCESSING BUILDING
UMA TURBINE BUILDING
UBA SWITCHGEAR BUILDING
1-4 URB ESSENTIAL SERVICE WATER BUILDING

B-302 BORING NUMBER
B-307(DH) BORING WITH DOWNHOLE GEOPHYSICAL MEASUREMENTS
C-303 CPT NUMBER
C-307(S) CPT WITH SEISMIC TESTING
PM-301 PRESSUREMETER NUMBER
DMT-301 DILATOMETER NUMBER
OW-428 OBSERVATION WELL (A&B INDICATE WELL PAIR)
R-1 FIELD ELECTRICAL RESISTIVITY TEST
38 TOP OF STRATUM IIb ELEVATION CONTOUR

NOTES:
1. ELEVATION CONTOURS OF TOP OF STRATUM IIb CEMENTED SAND ARE APPROXIMATE AND BASED ON INTERPRETATION OF SAMPLE INTERVALS.
2. FINAL ELEVATIONS OF TOP OF CEMENTED SAND TO BE CONFIRMED DURING CONSTRUCTION.

Figure 2.5-136—{Proximity of Chester and Lexington Park Sites to CCNPP}

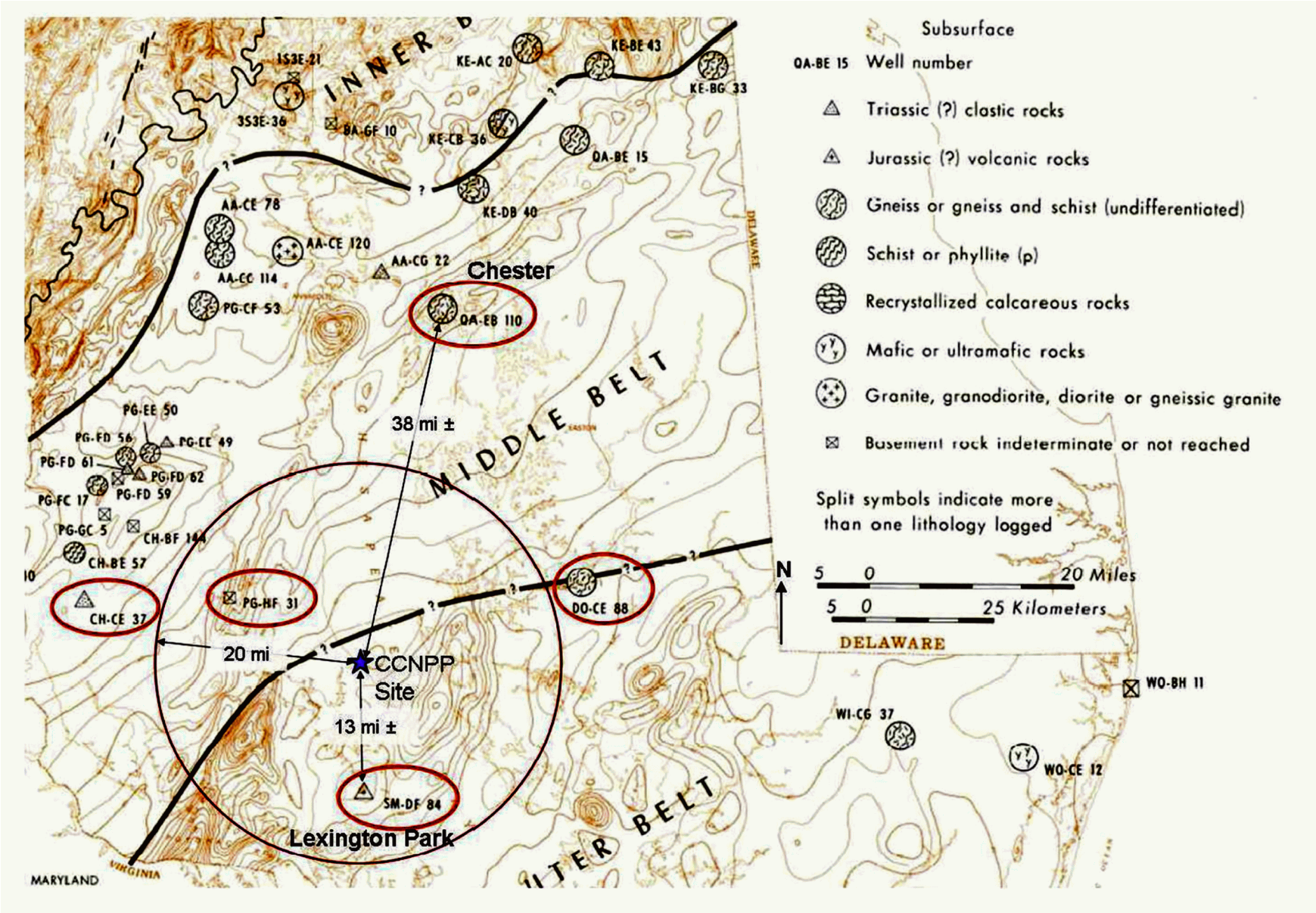


Figure 2.5-137—{V_S Based on Chester (Kent Island) Measurements}

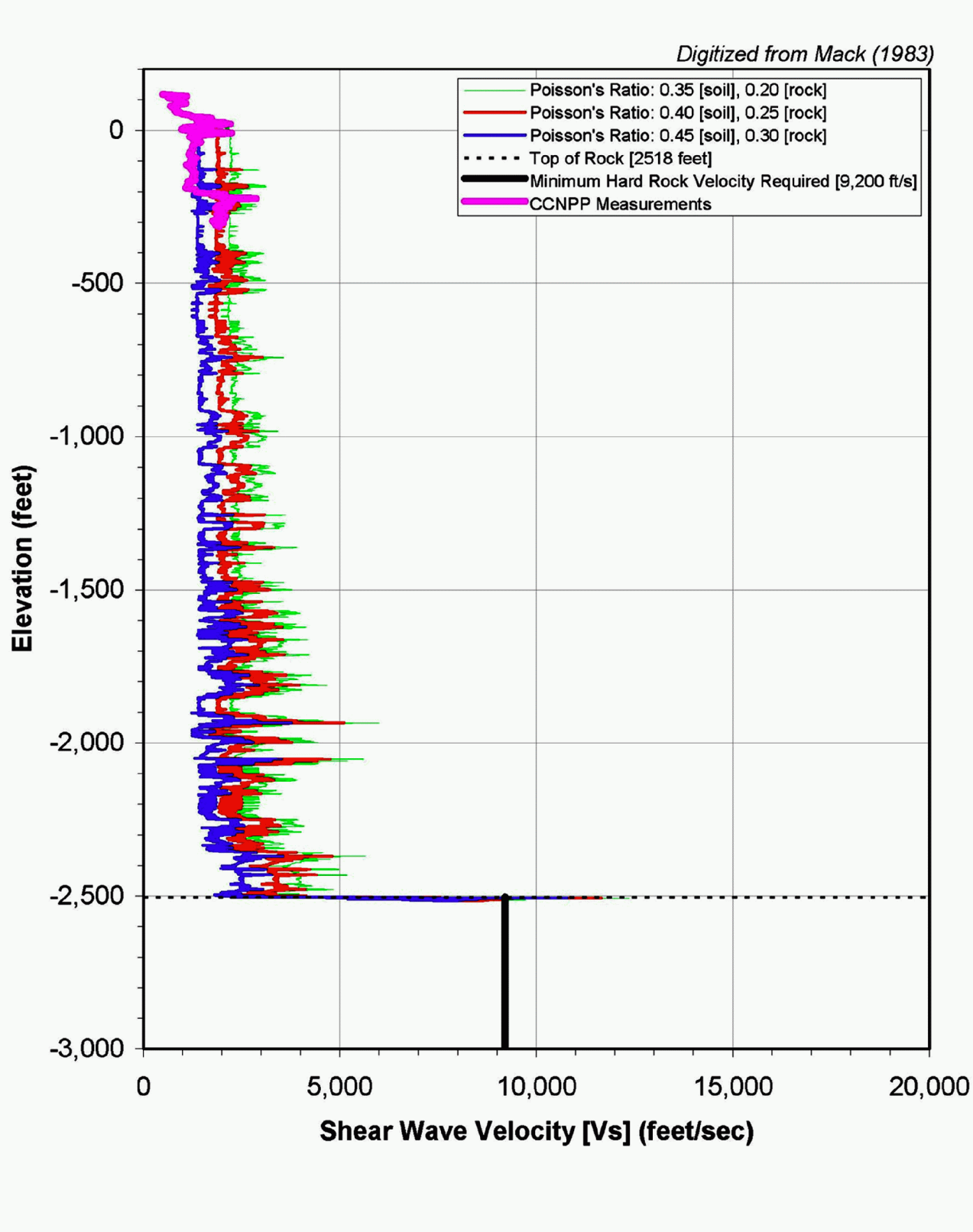


Figure 2.5-138—{V_S Log Based on Lexington Park Measurements}

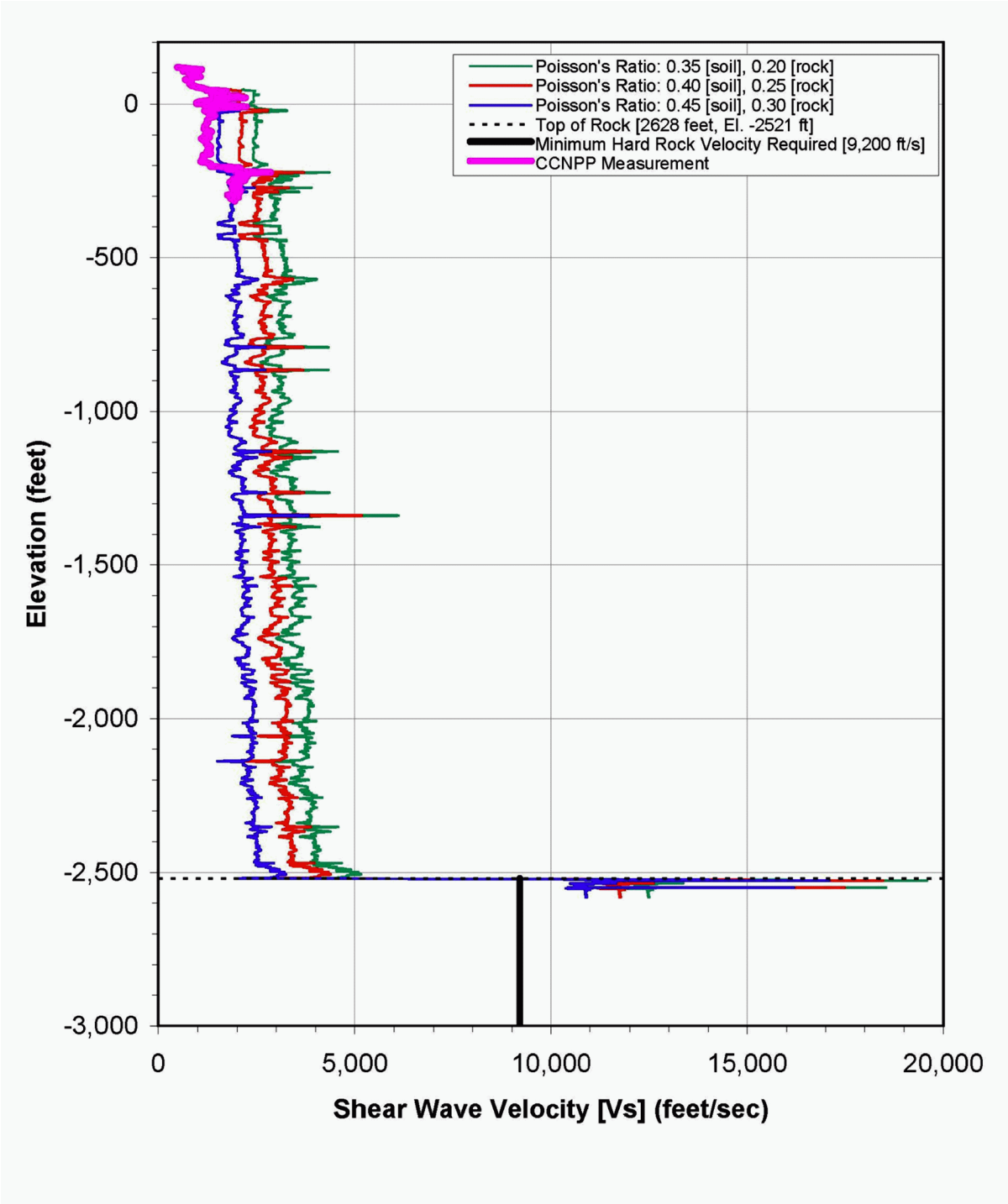


Figure 2.5-139—{Smoothed and Averaged V_S Log for Chester and Lexington Park Measurements}

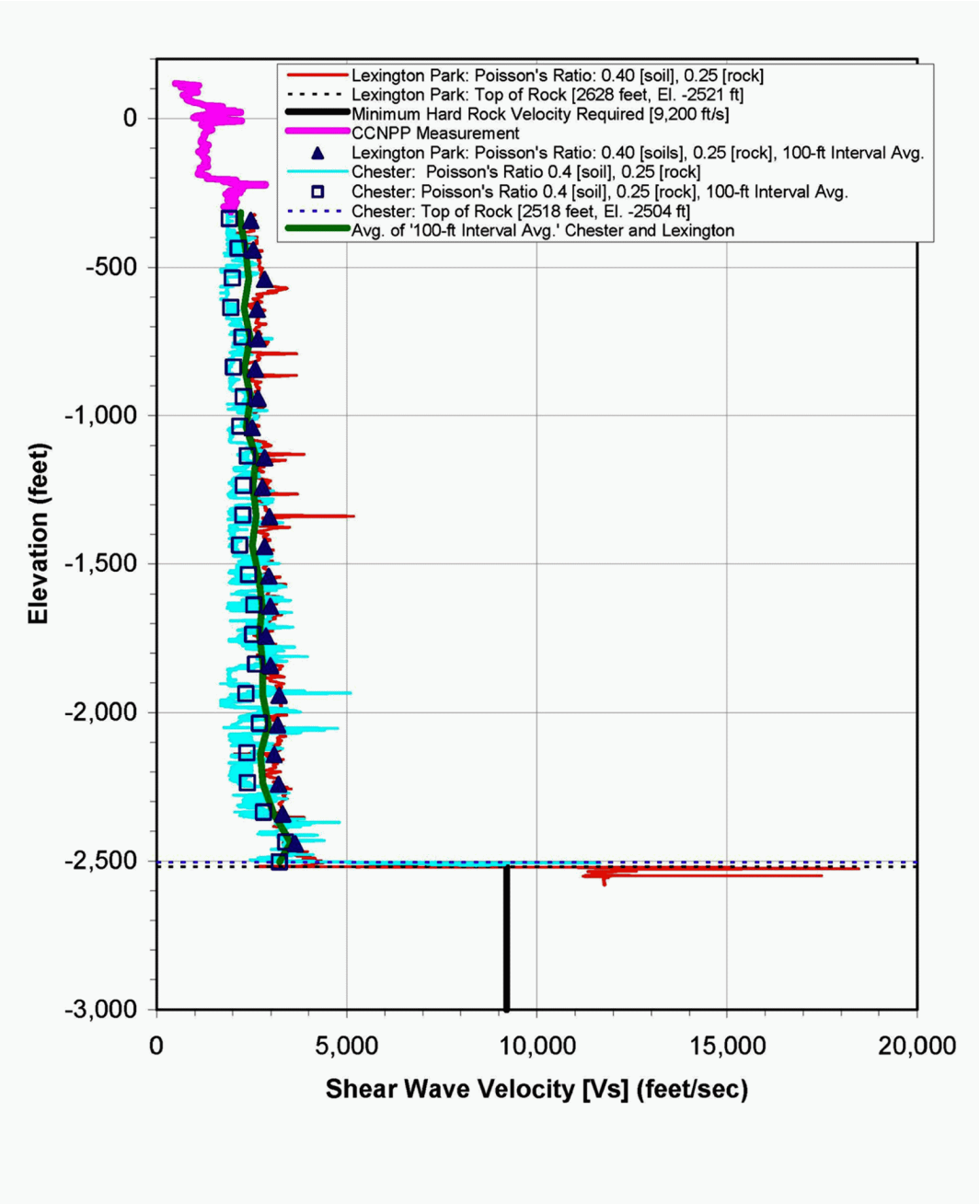


Figure 2.5-140—{Comparison of Average V_S for Chester, Lexington Park, Maryland and Deep measurements in Coastal Plain Soils}

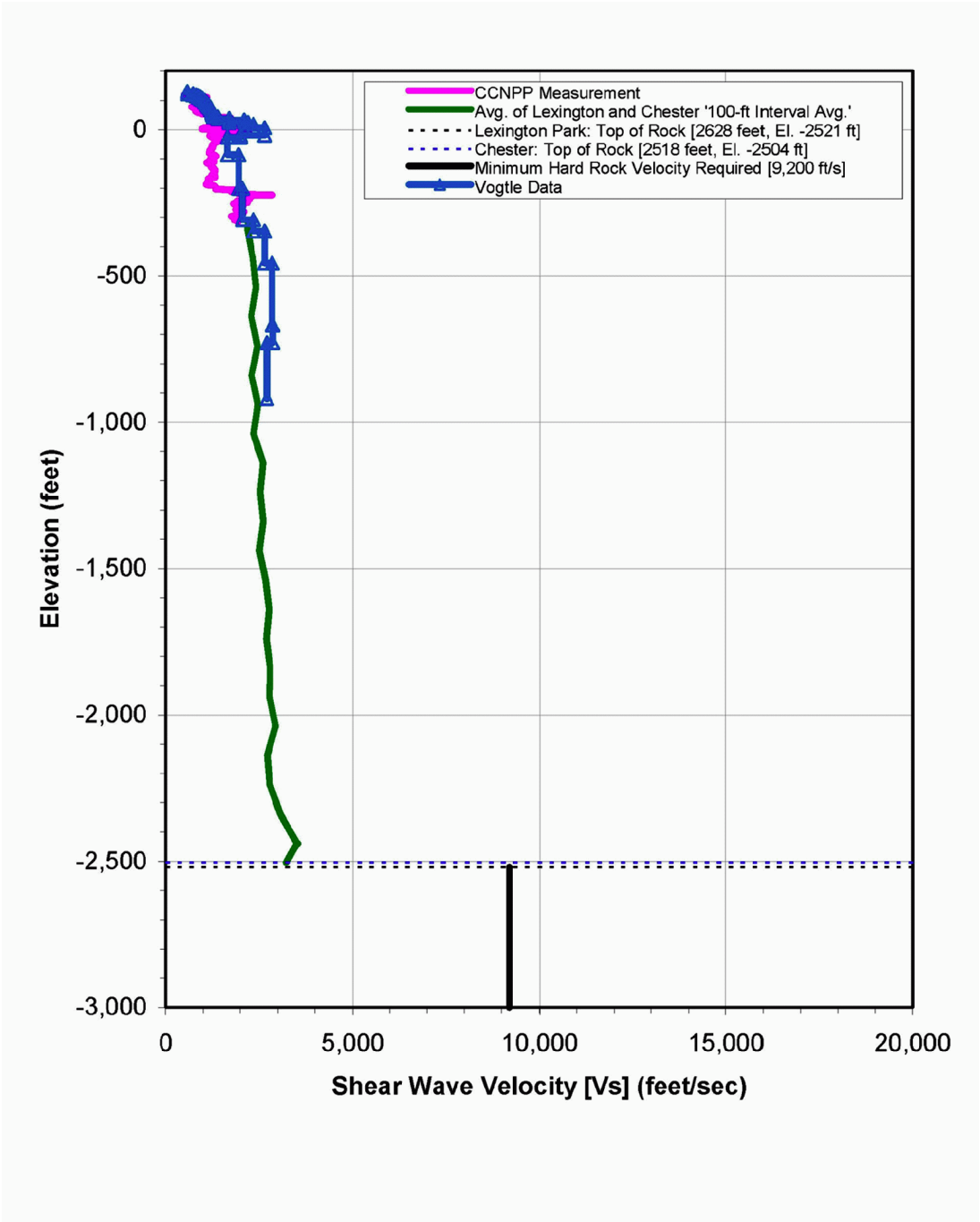


Figure 2.5-141—{Recommended V_s Soil Profile at CCNPP Site}

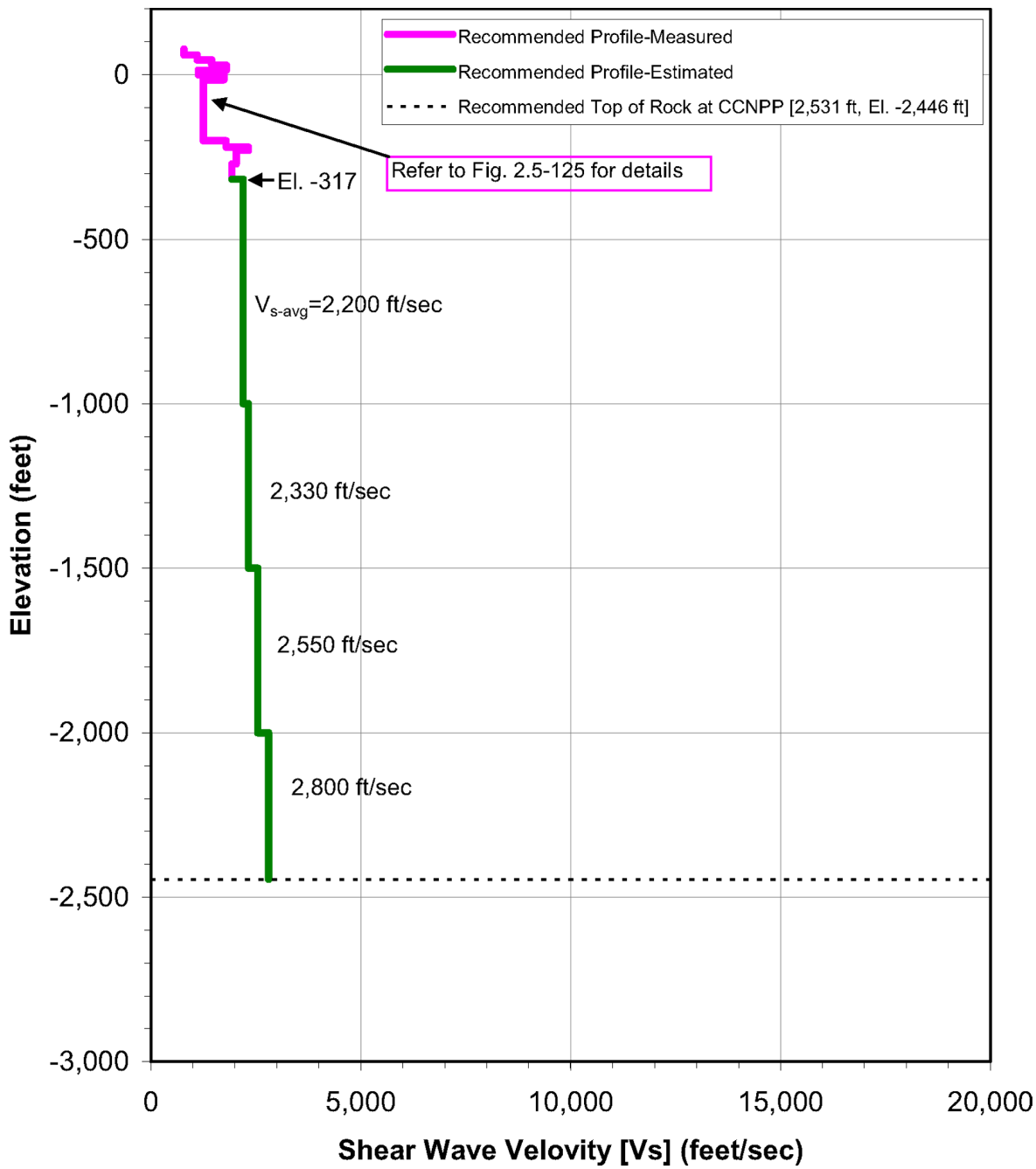


Figure 2.5-142—{Bedrock V_S Log for Chester (Kent Island), Maryland}

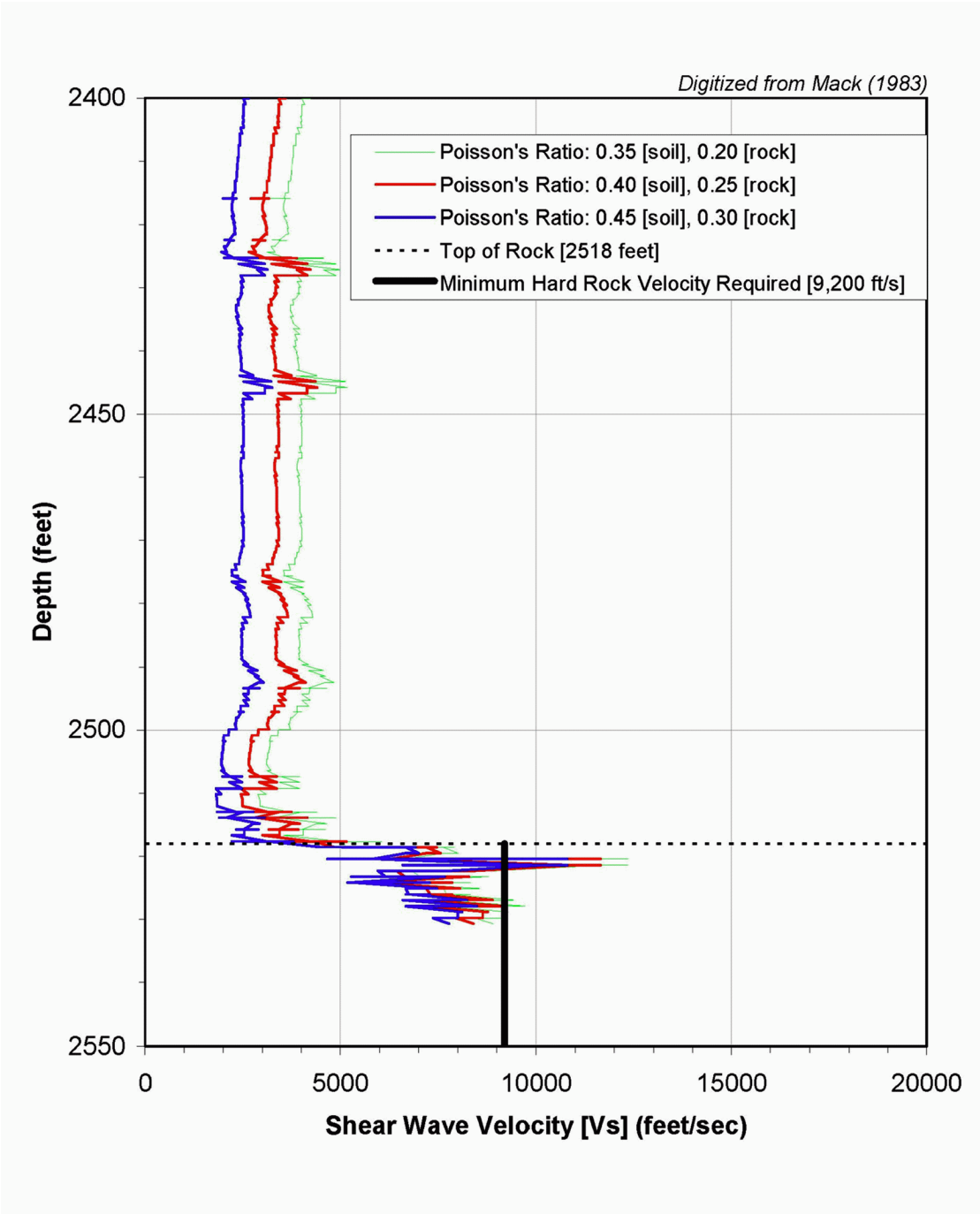


Figure 2.5-143—{Bedrock V_S Log for Lexington Park, Maryland}

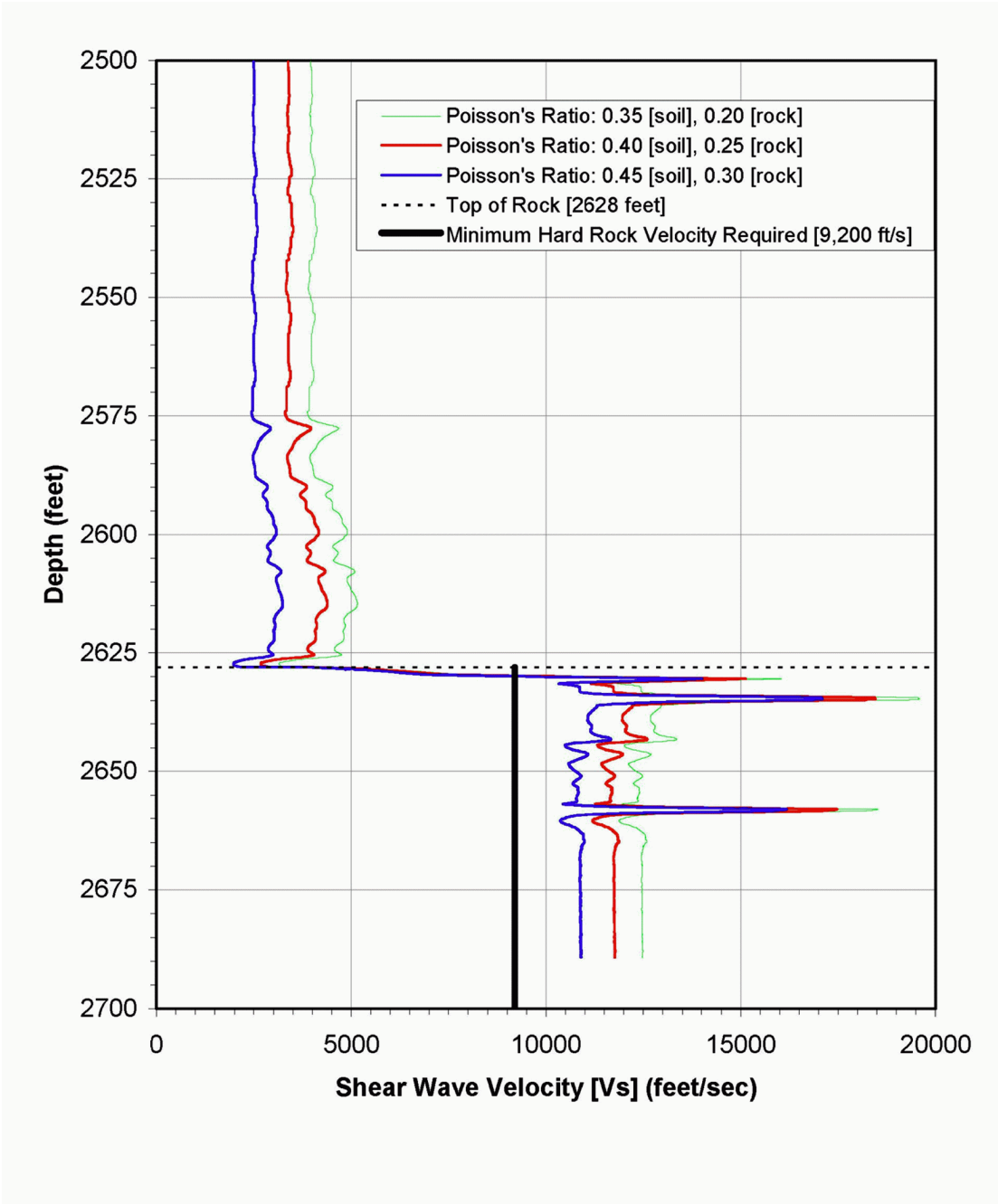


Figure 2.5-144—{Interpretation of Bedrock Velocity Gradient for Chester Measurement}

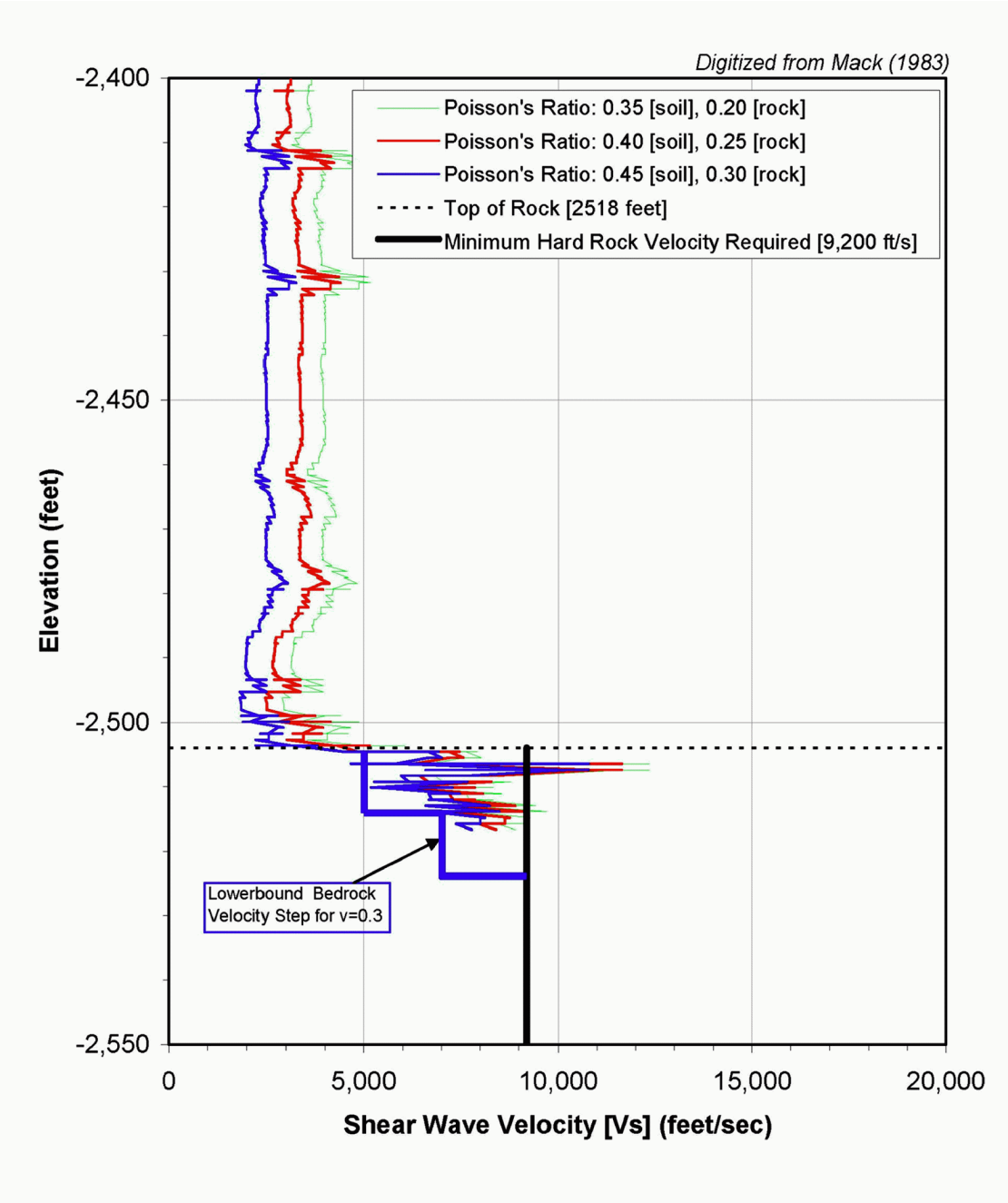


Figure 2.5-145—{Recommended Vs Profile for the CCNPP Site}

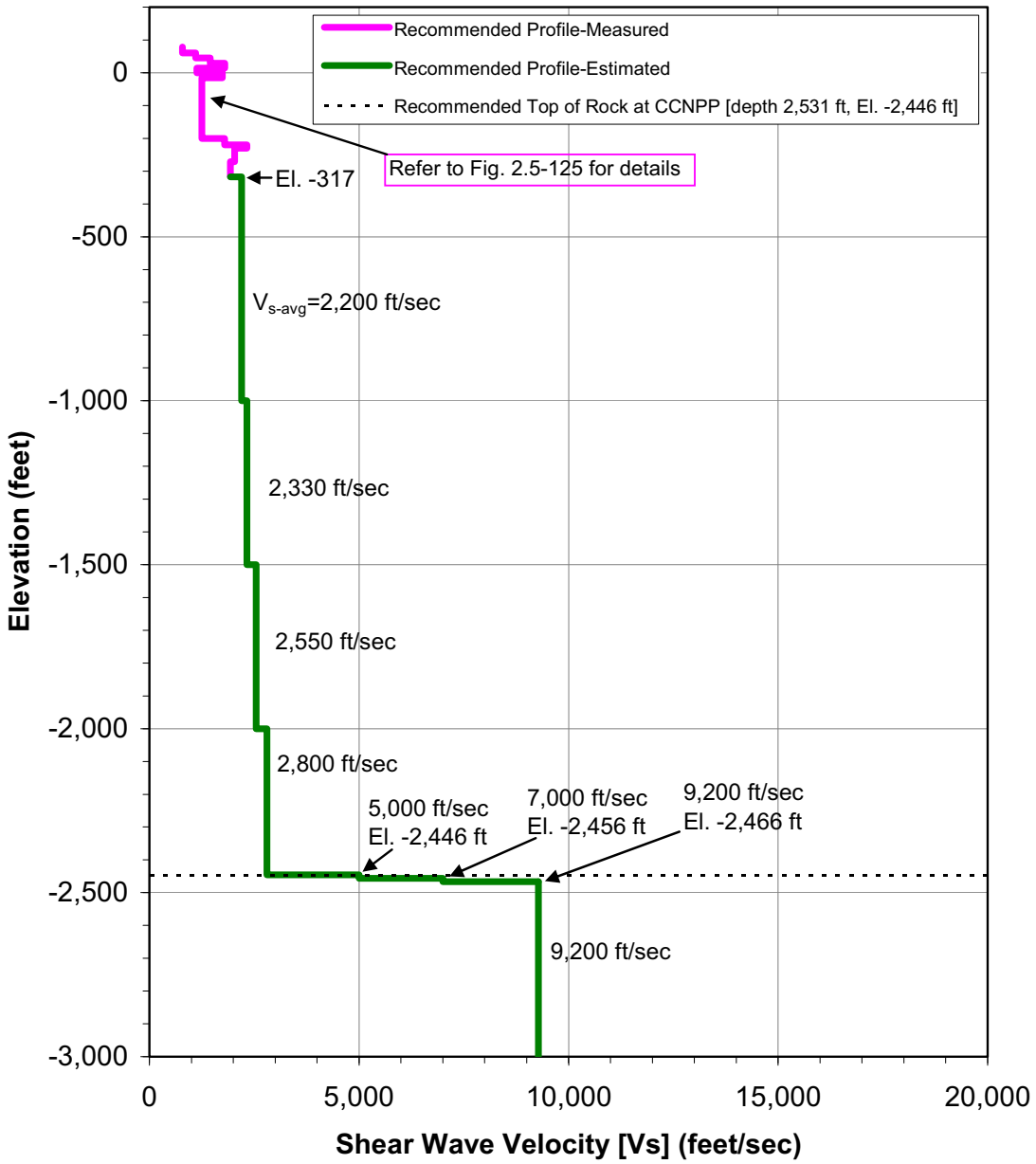


Figure 2.5-146—{Recommended Poisson's Ratio Profile for the CCNPP Site}

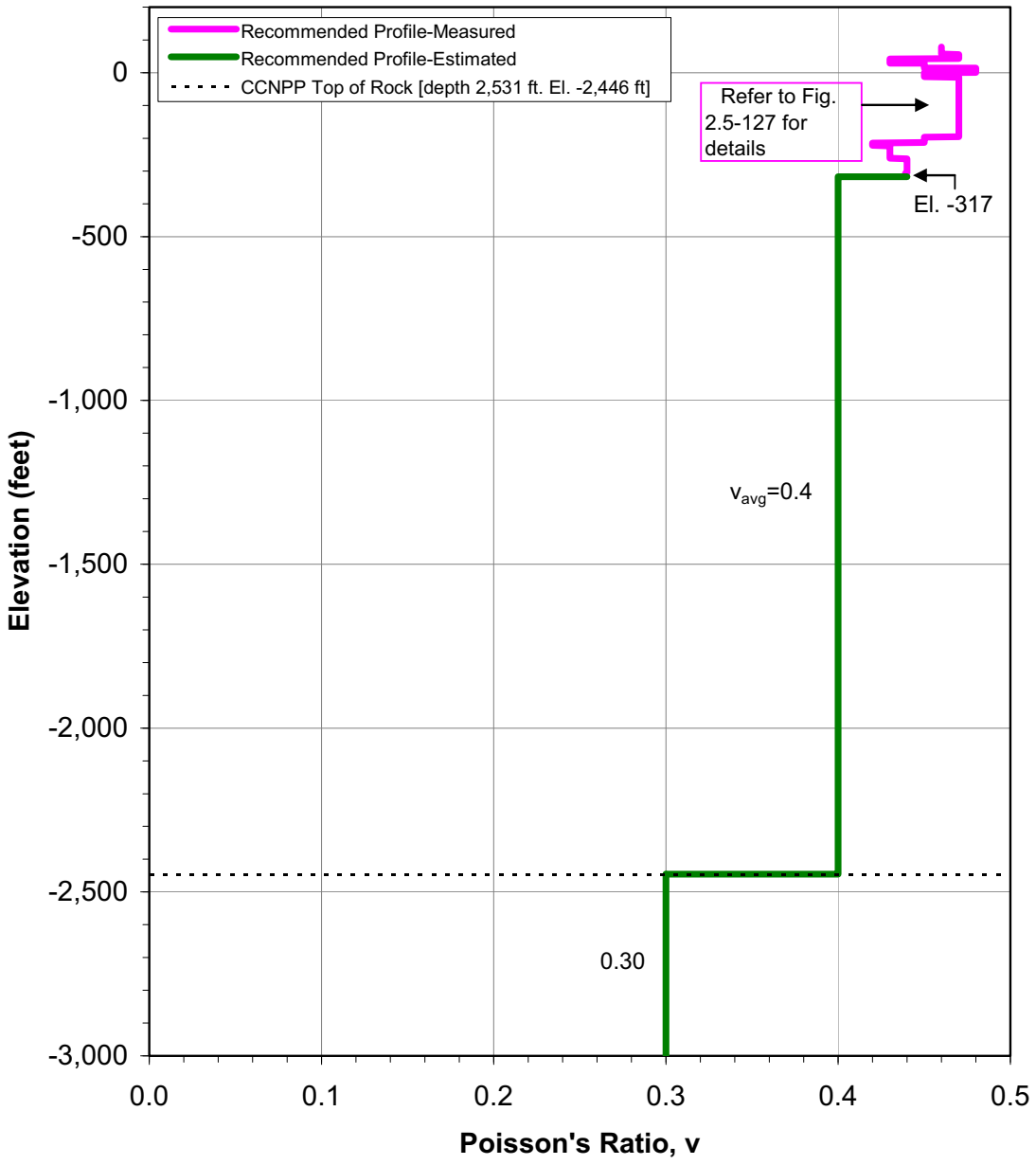


Figure 2.5-147—{Recommended Modulus and Damping Ratio Curves for the CCNPP Site}

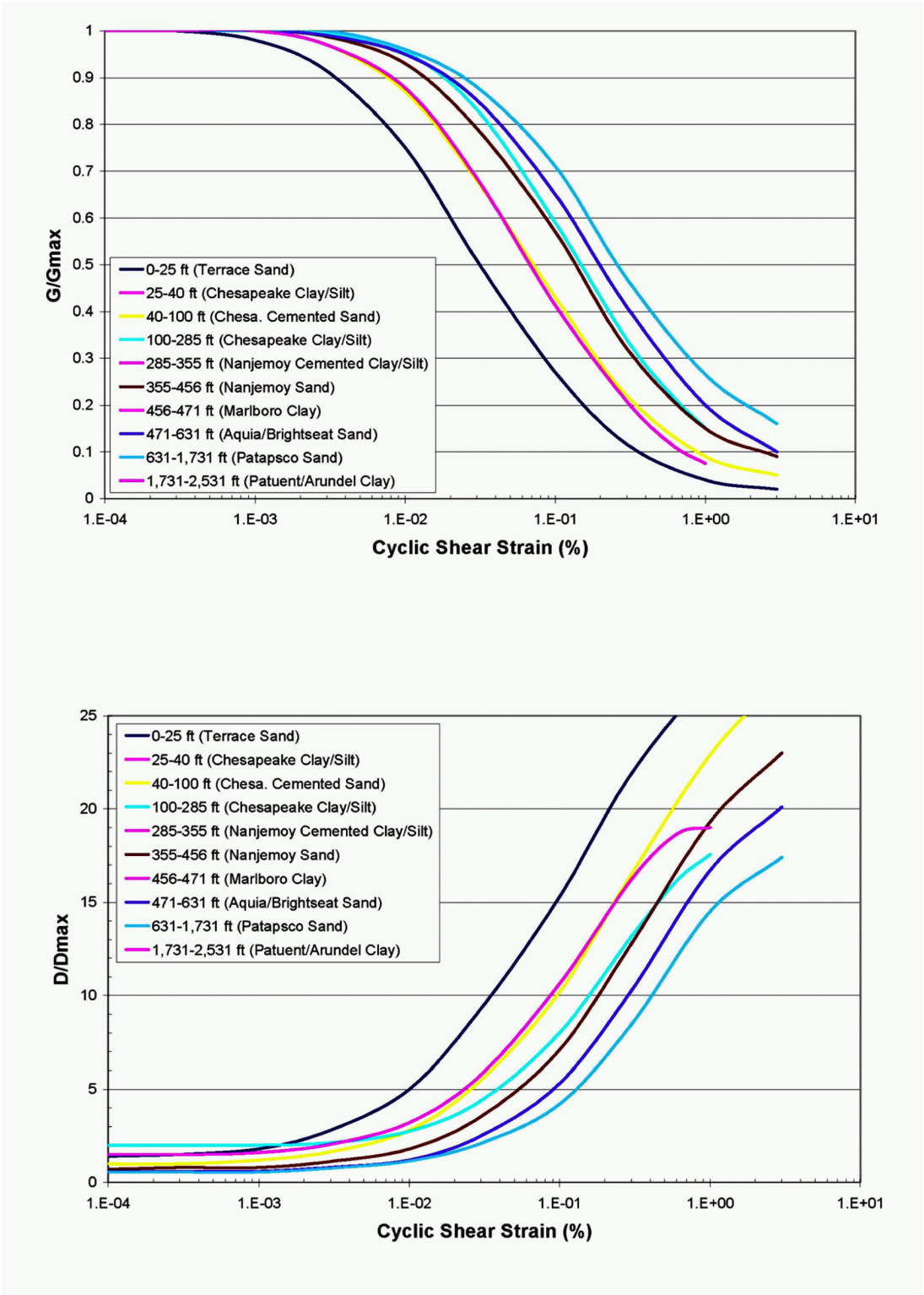


Figure 2.5-148—{Measured SPT N-Values (Uncorrected)}

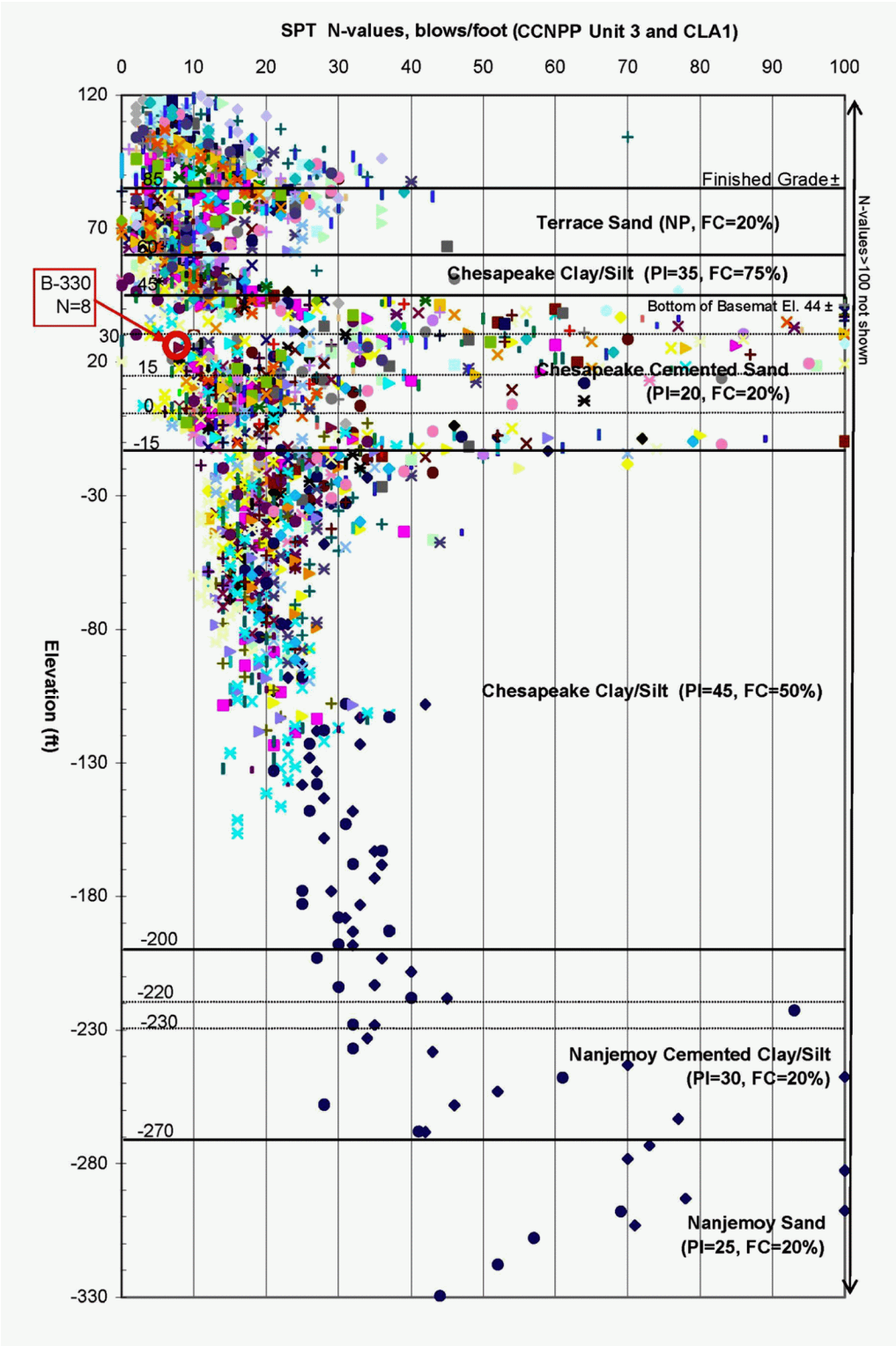


Figure 2.5-149—{Calculated FOS Based on SPT N-Values}

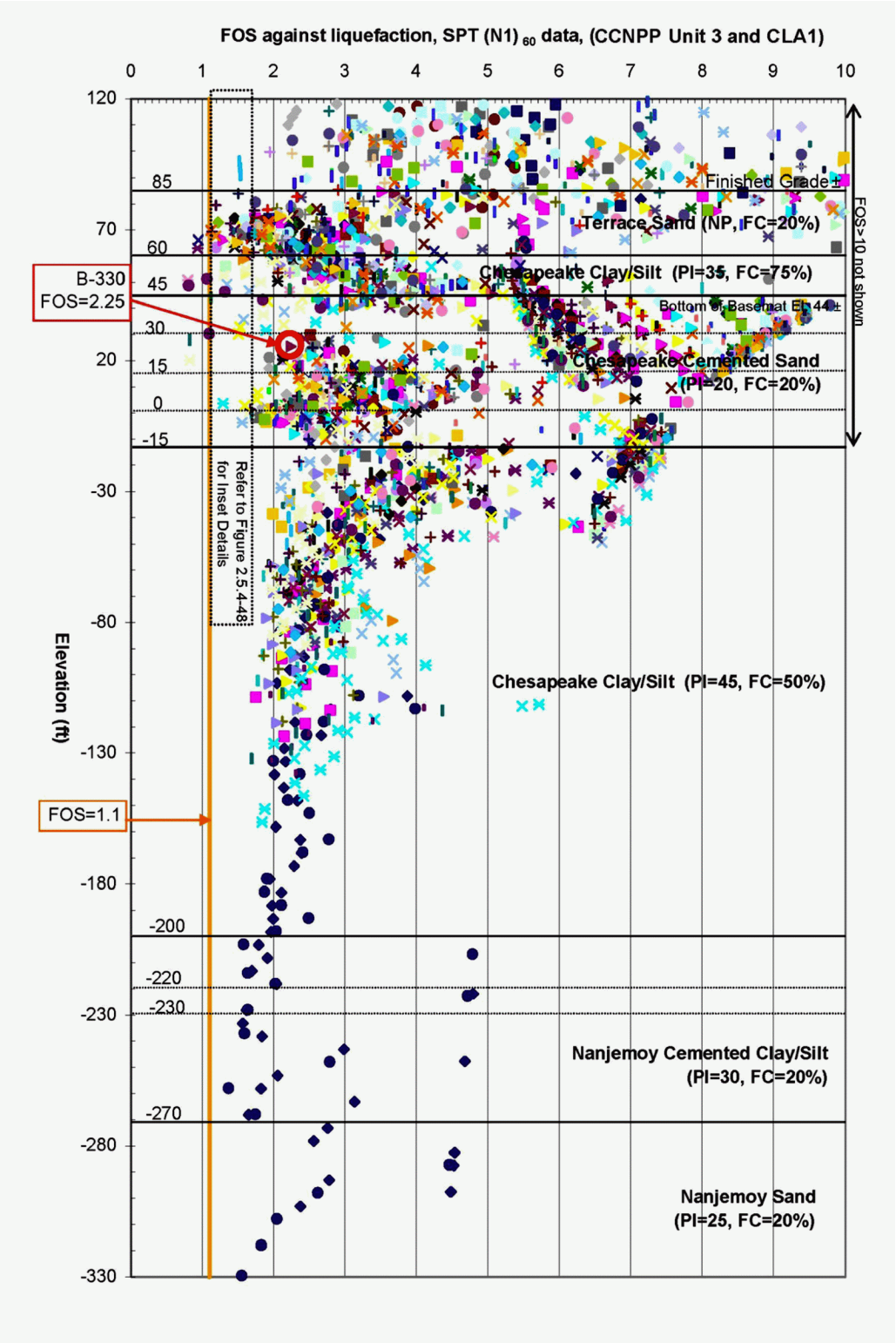


Figure 2.5-150—{Calculated FOS <1.1 Based on SPT N-Values}

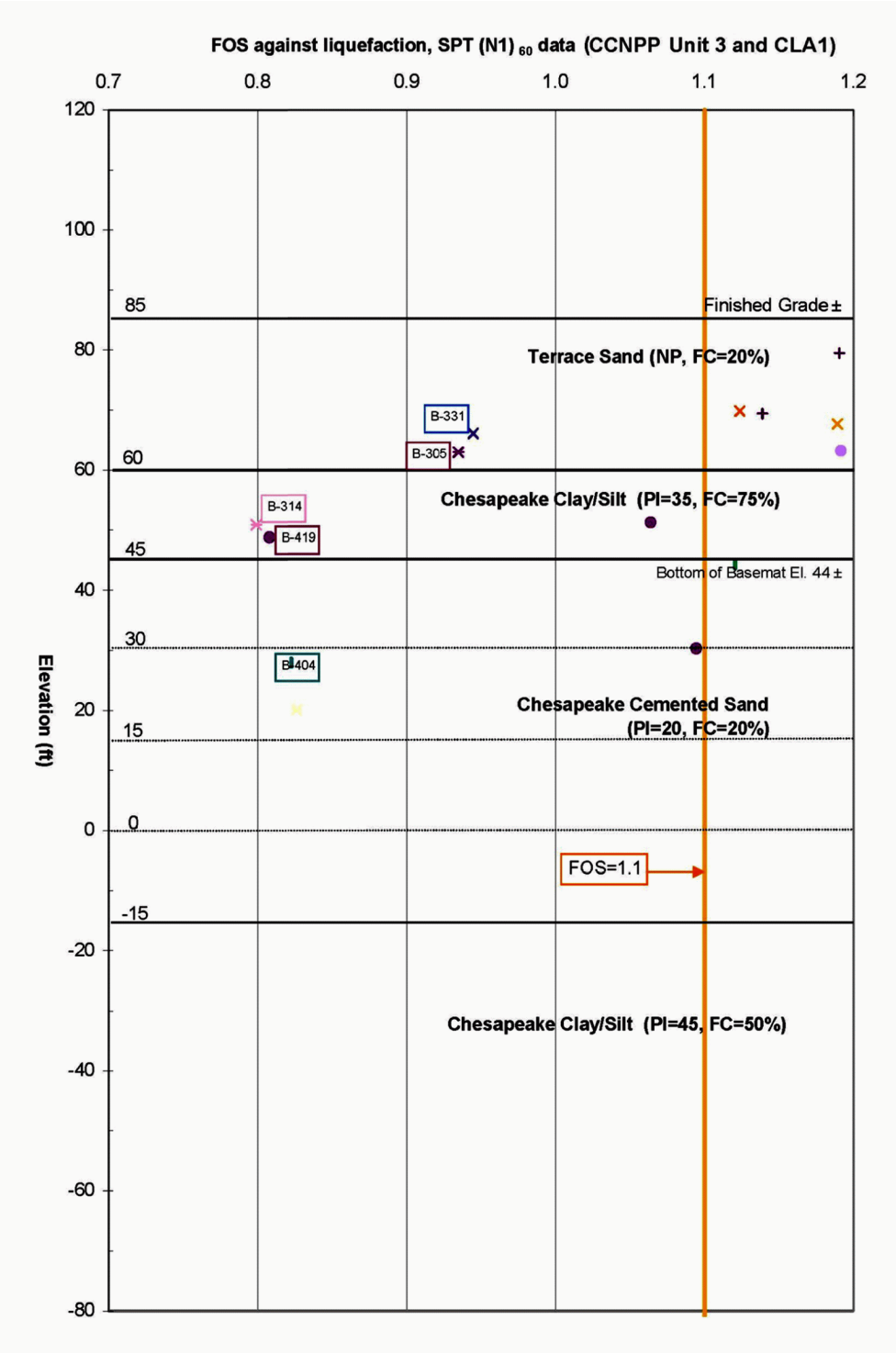


Figure 2.5-151—{Measured Suspension P-S Velocity Logging V_S Data (Uncorrected)}

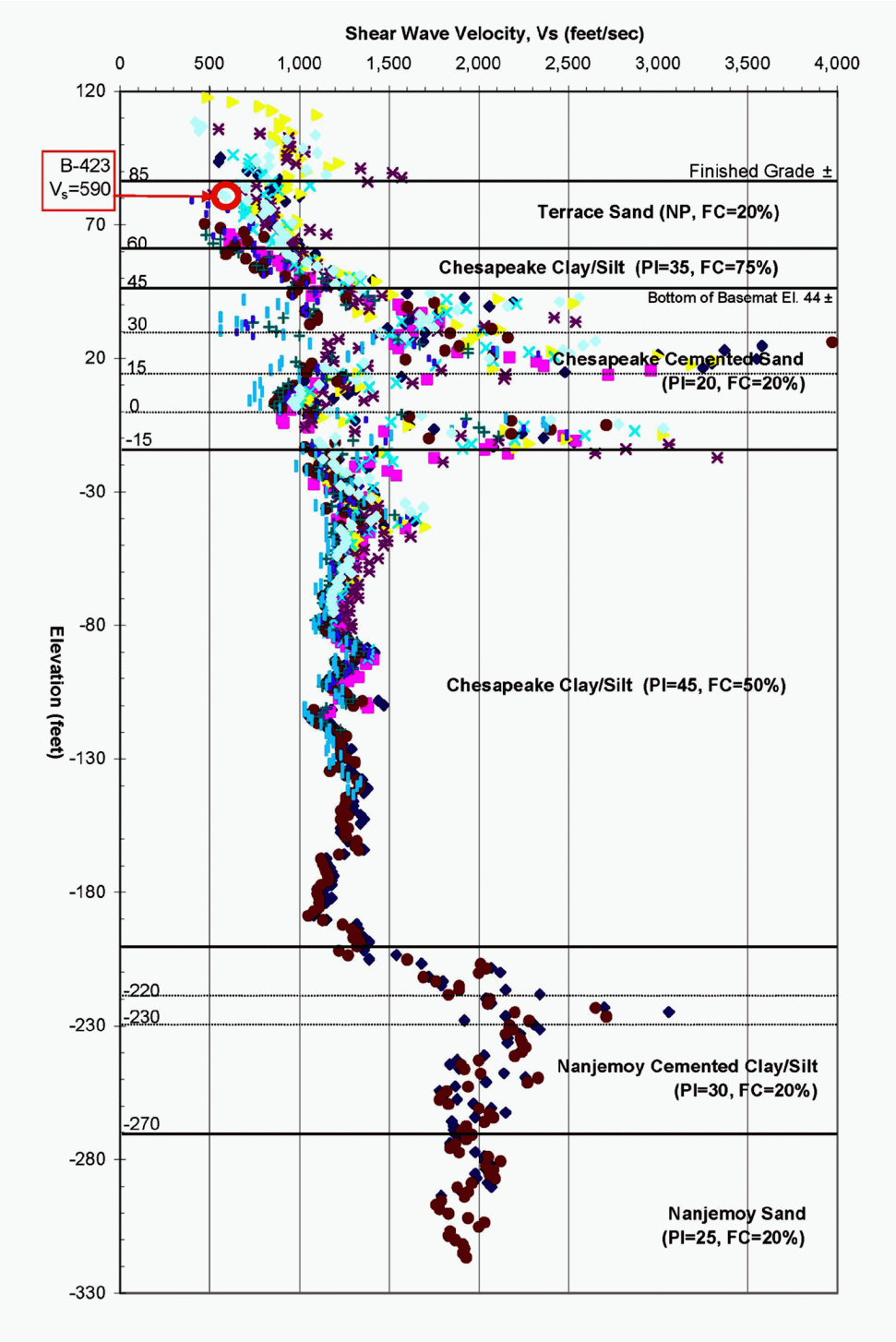


Figure 2.5-152—{Calculated FOS Based on P-S Logging V_S Data}

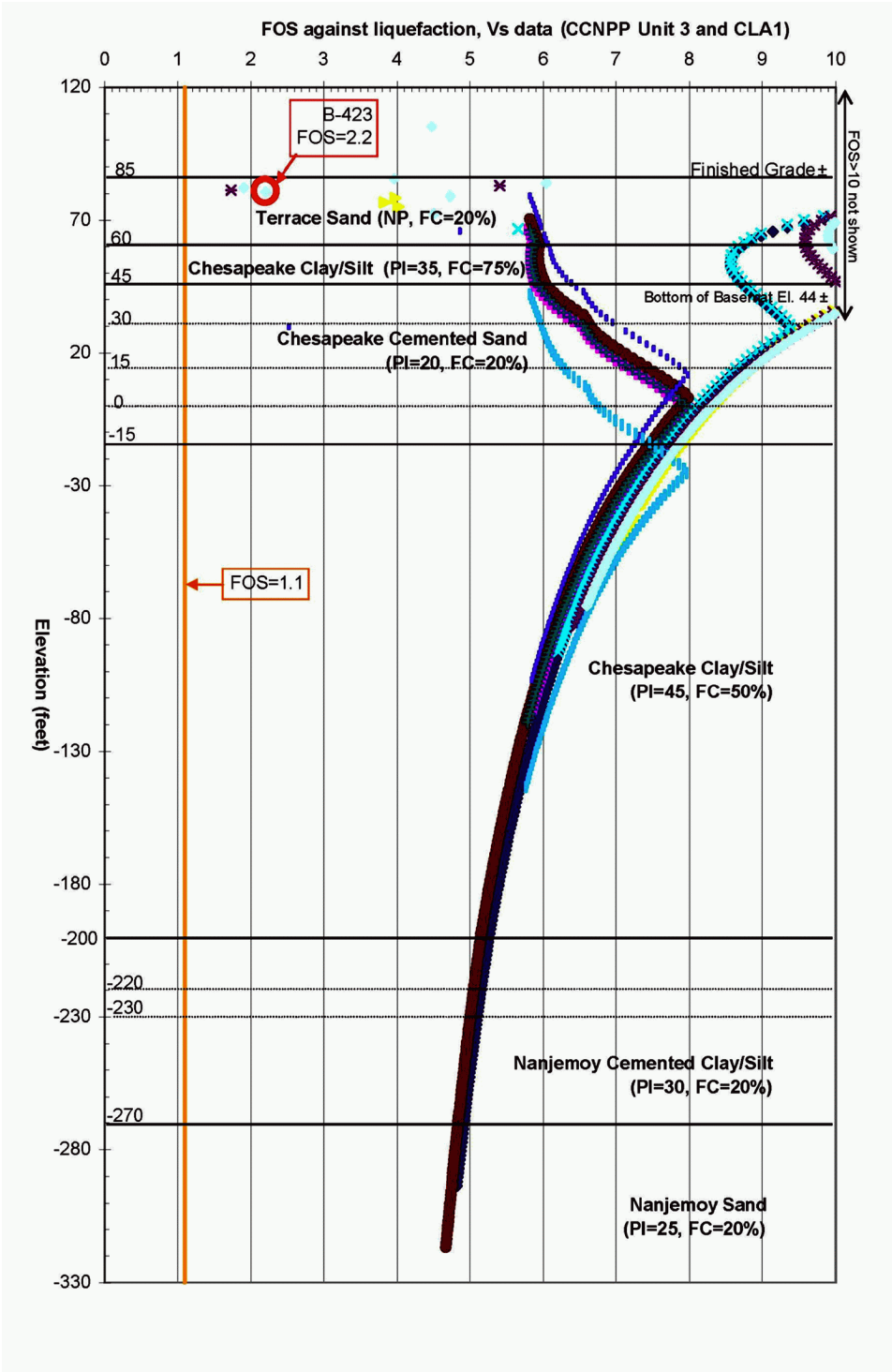


Figure 2.5-153—{Measured CPT Tip Resistance Values}

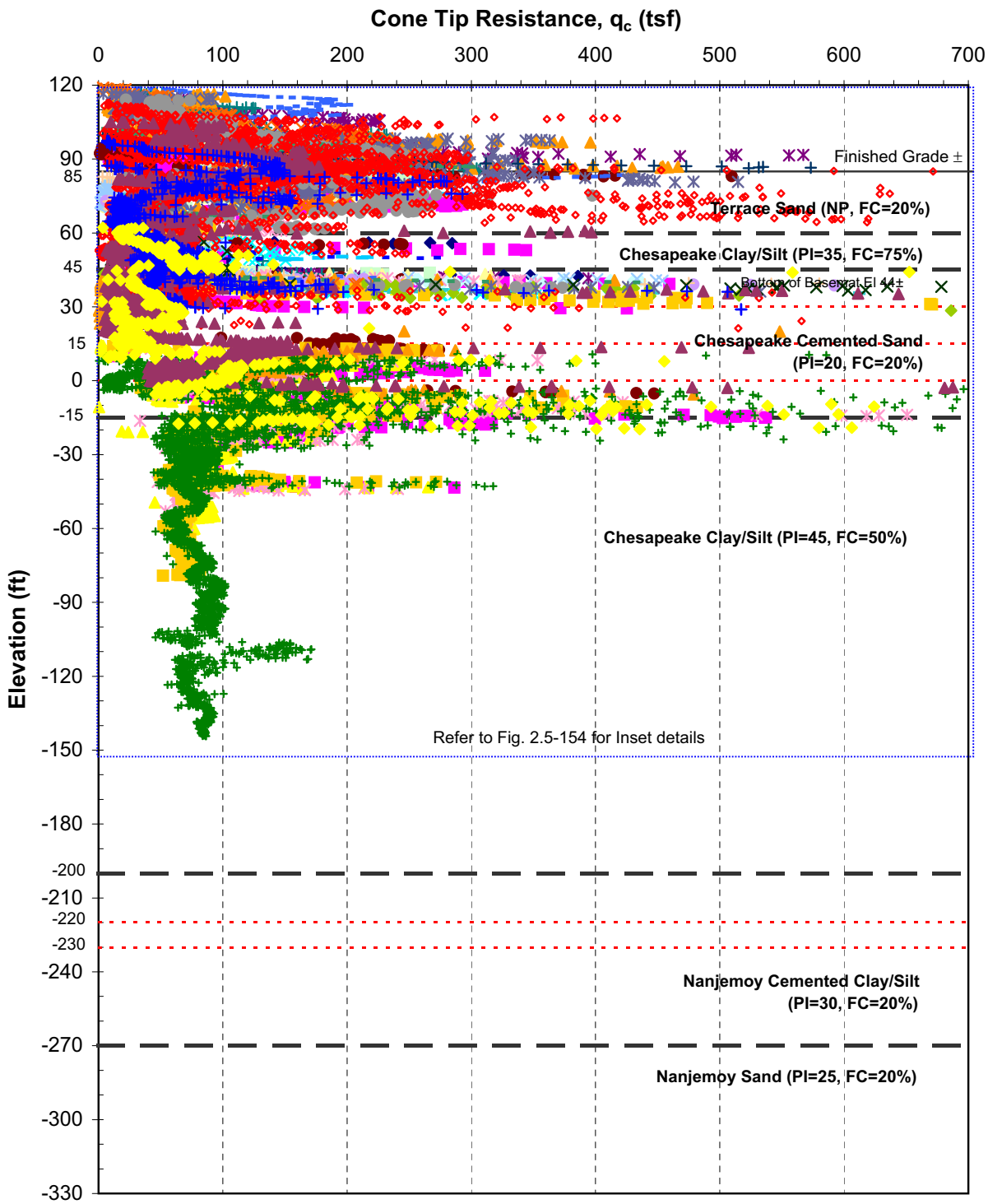


Figure 2.5-154—{Measured CPT Tip Resistance Values}

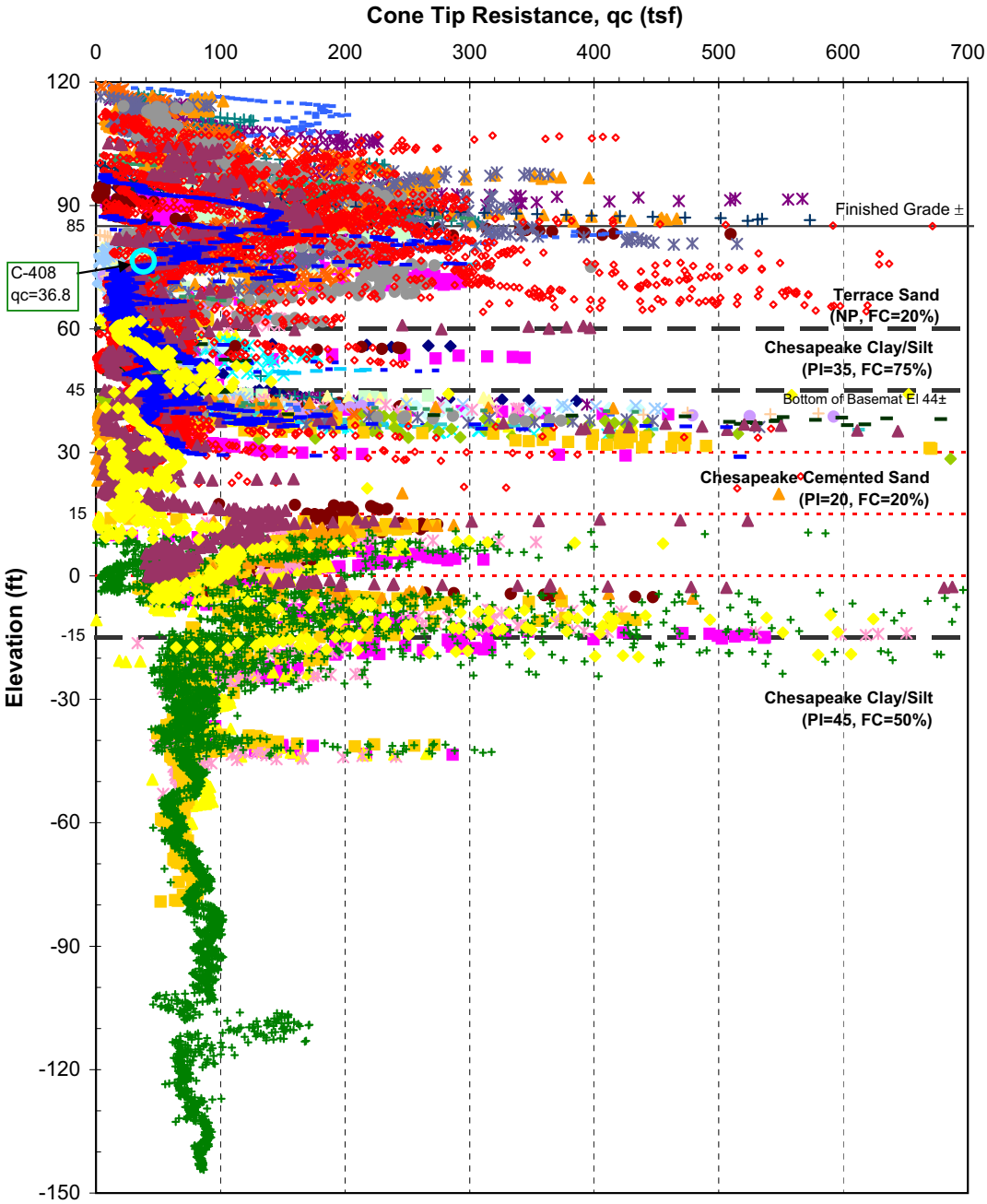


Figure 2.5-155—{Calculated FOS Based on CPT Tip Resistance Data}

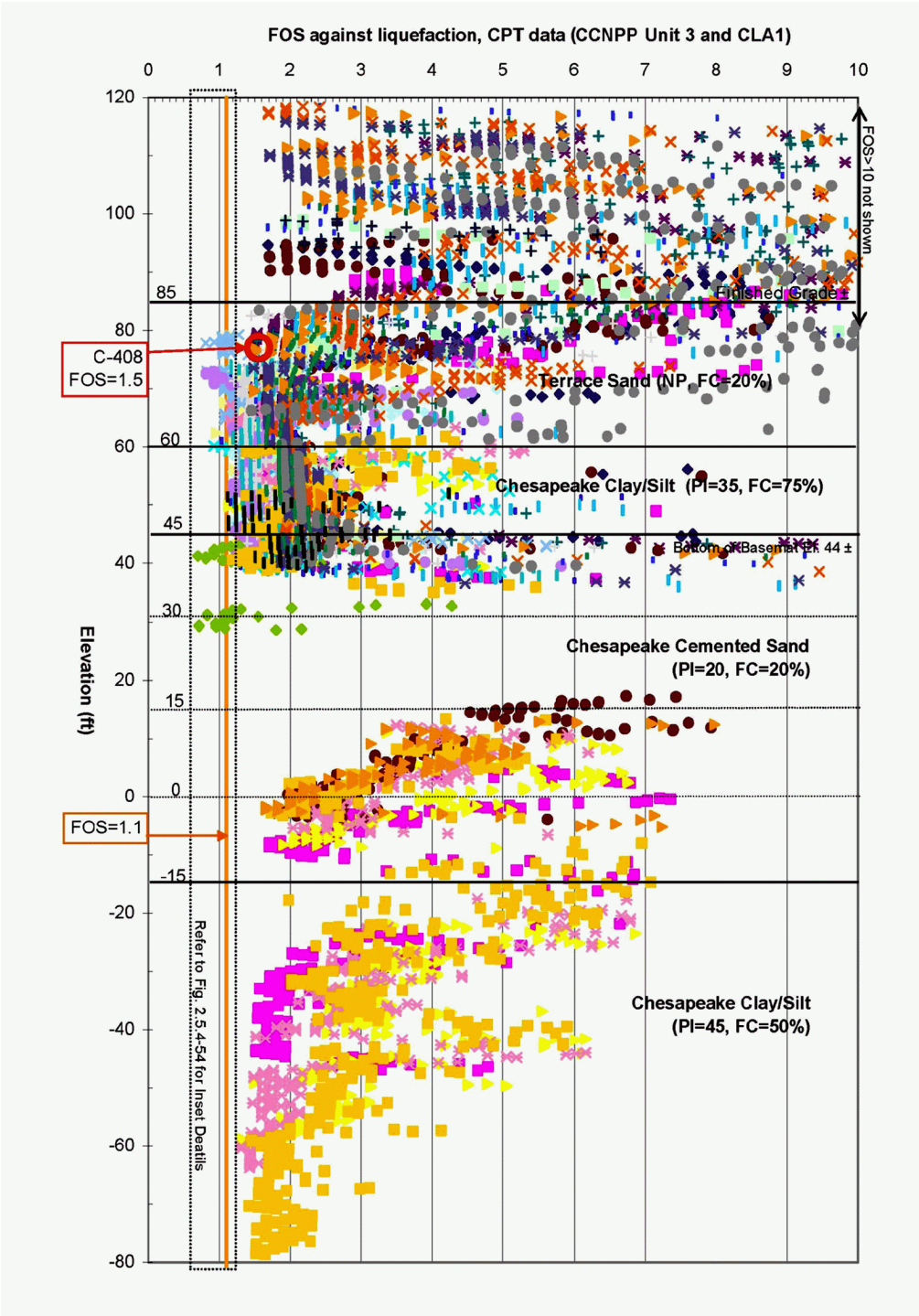


Figure 2.5-156—{Calculated FOS <1.1 Based on CPT Tip Resistance Data}

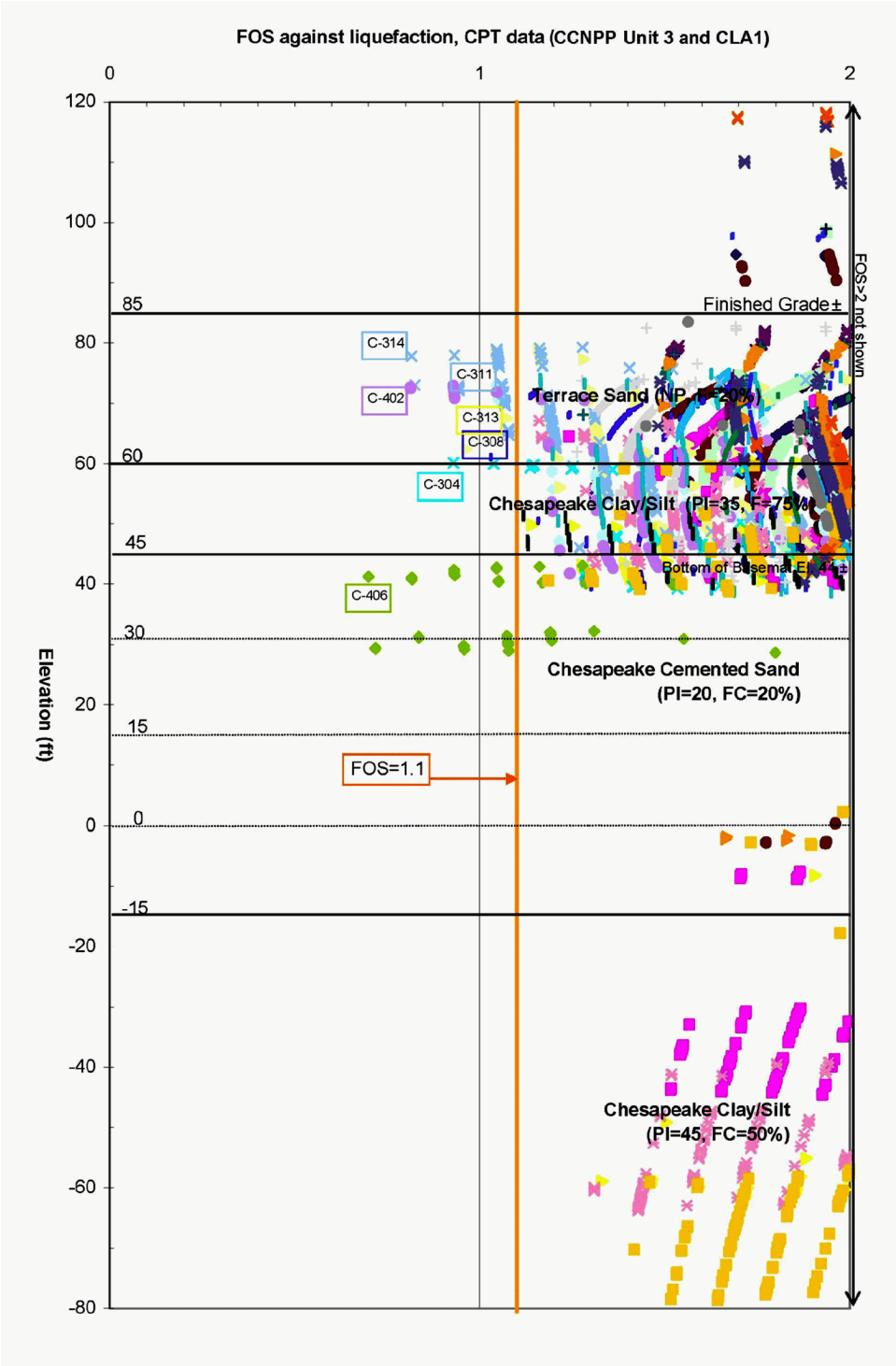


Figure 2.5-157—{Sample Active Lateral Earth Pressure Diagrams}

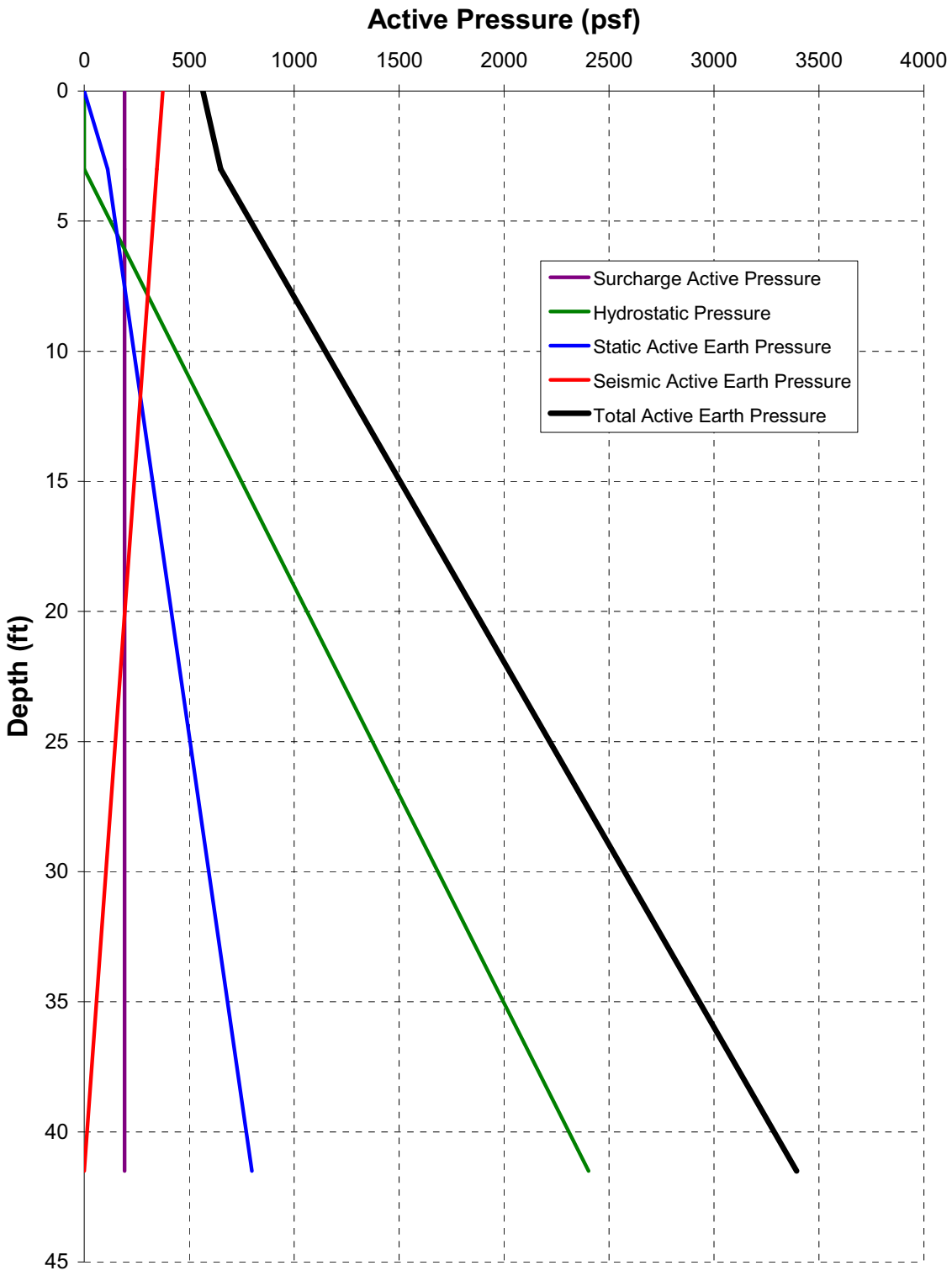


Figure 2.5-158—{Sample At-Rest Lateral Earth Pressure Diagrams}

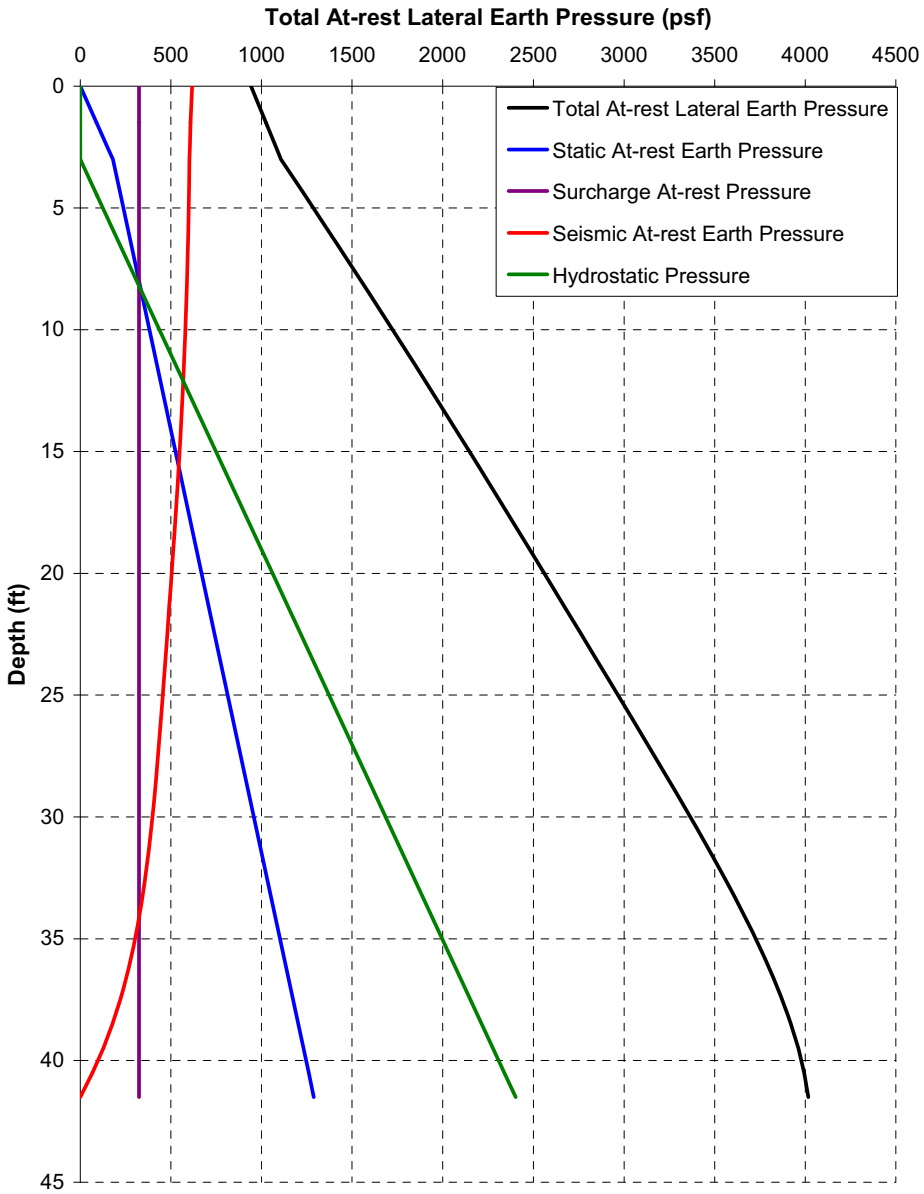


Figure 2.5-159—{Site Grading Plan}

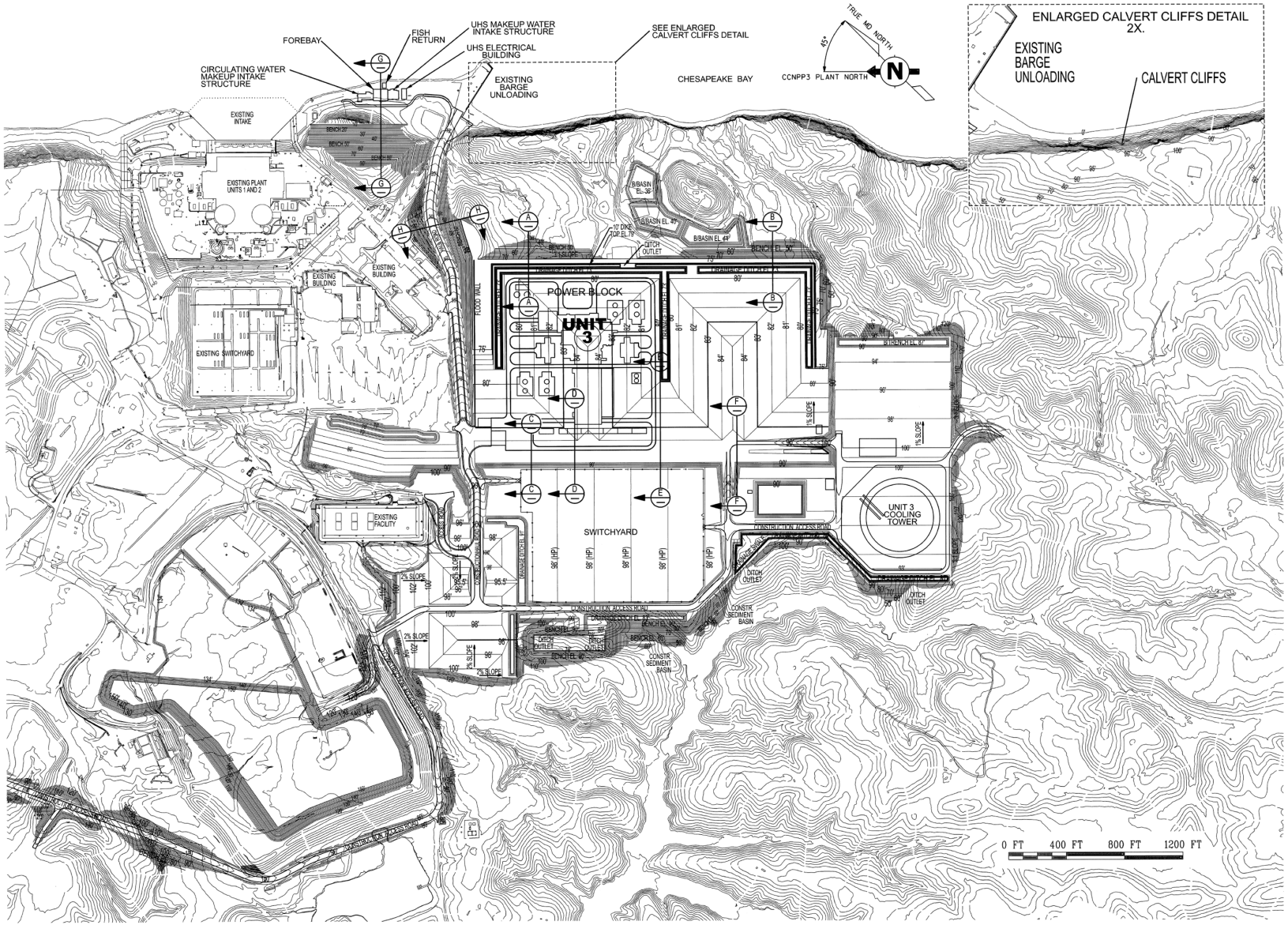


Figure 2.5-160—{Cross-Sections in Powerblock Area}

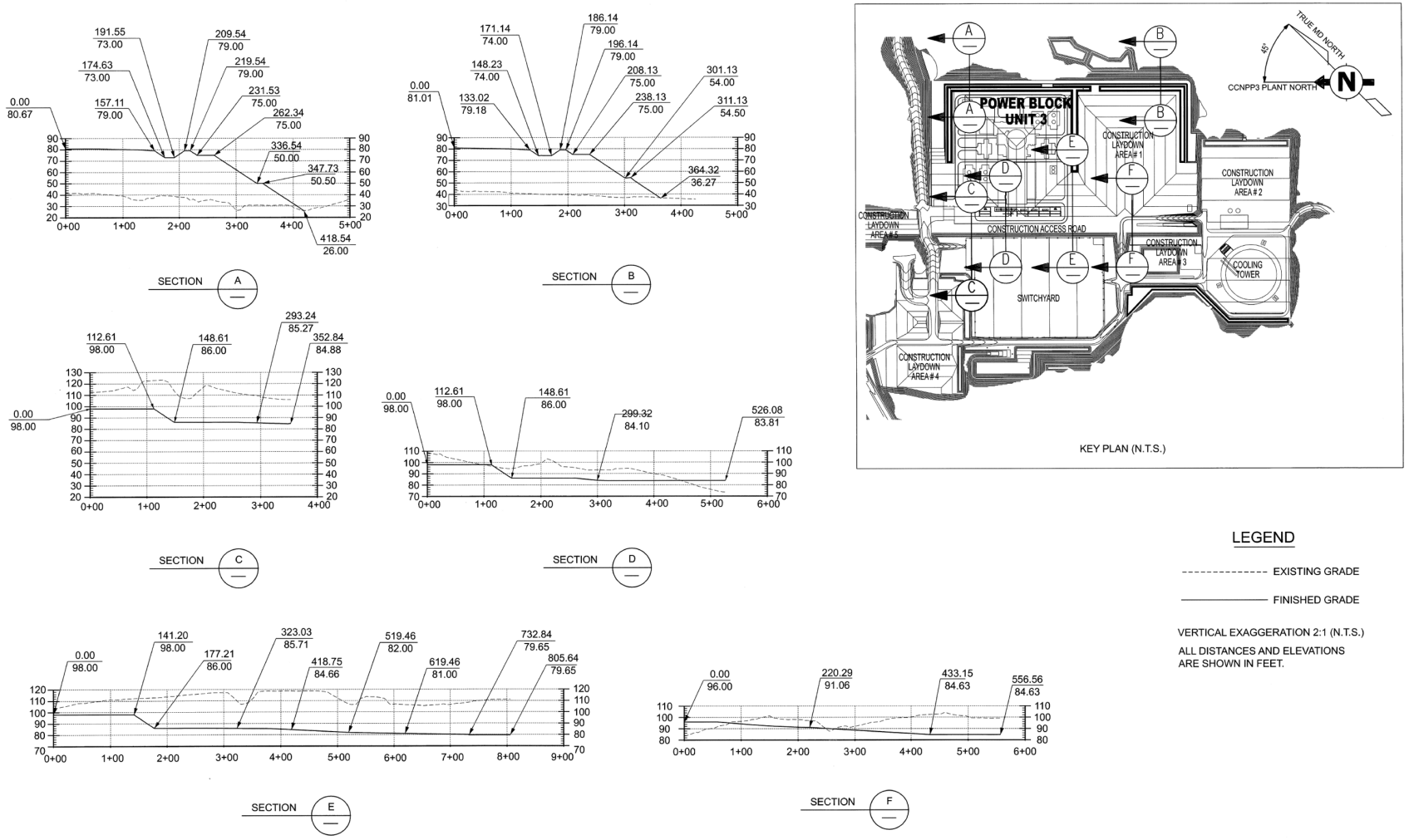
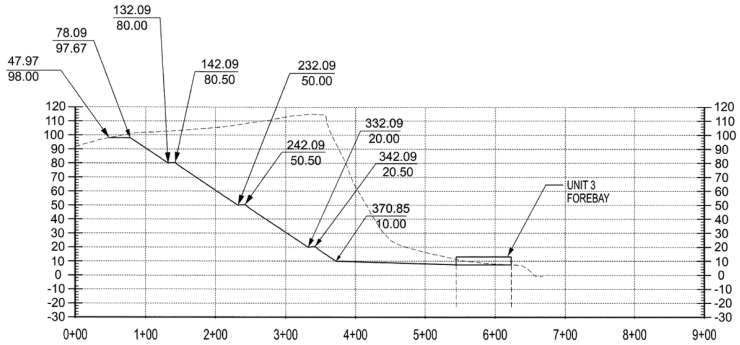
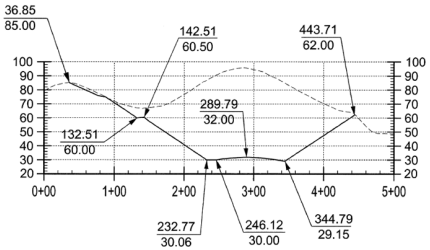


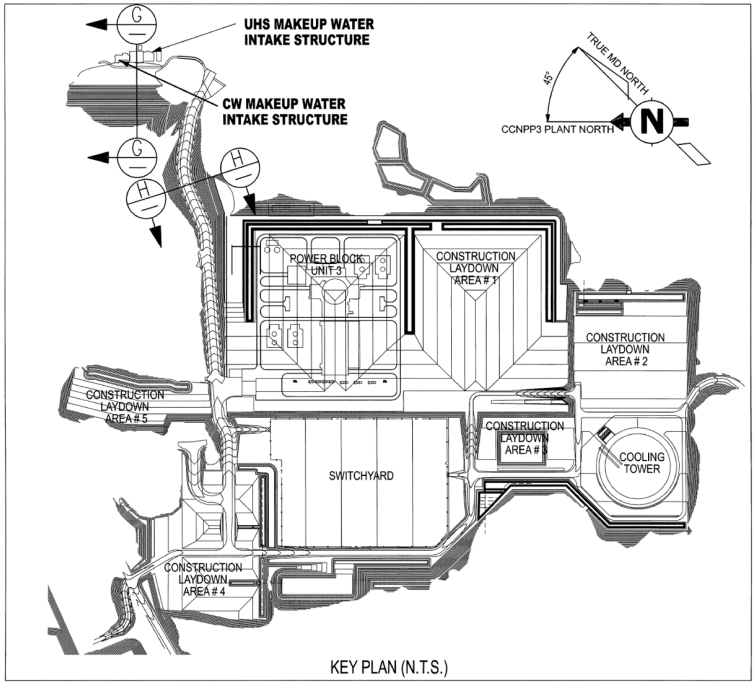
Figure 2.5-161—{Cross-Sections in Intake Area Slope and Utility Corridor}



SECTION G



SECTION H



LEGEND

- - - - - - - - EXISTING GRADE

———————— FINISHED GRADE

VERTICAL EXAGGERATION 2:1 (N.T.S.)

ALL DISTANCES AND ELEVATIONS
ARE SHOWN IN FEET.

Figure 2.5-162—{Schematic Section of Calvert Cliffs}

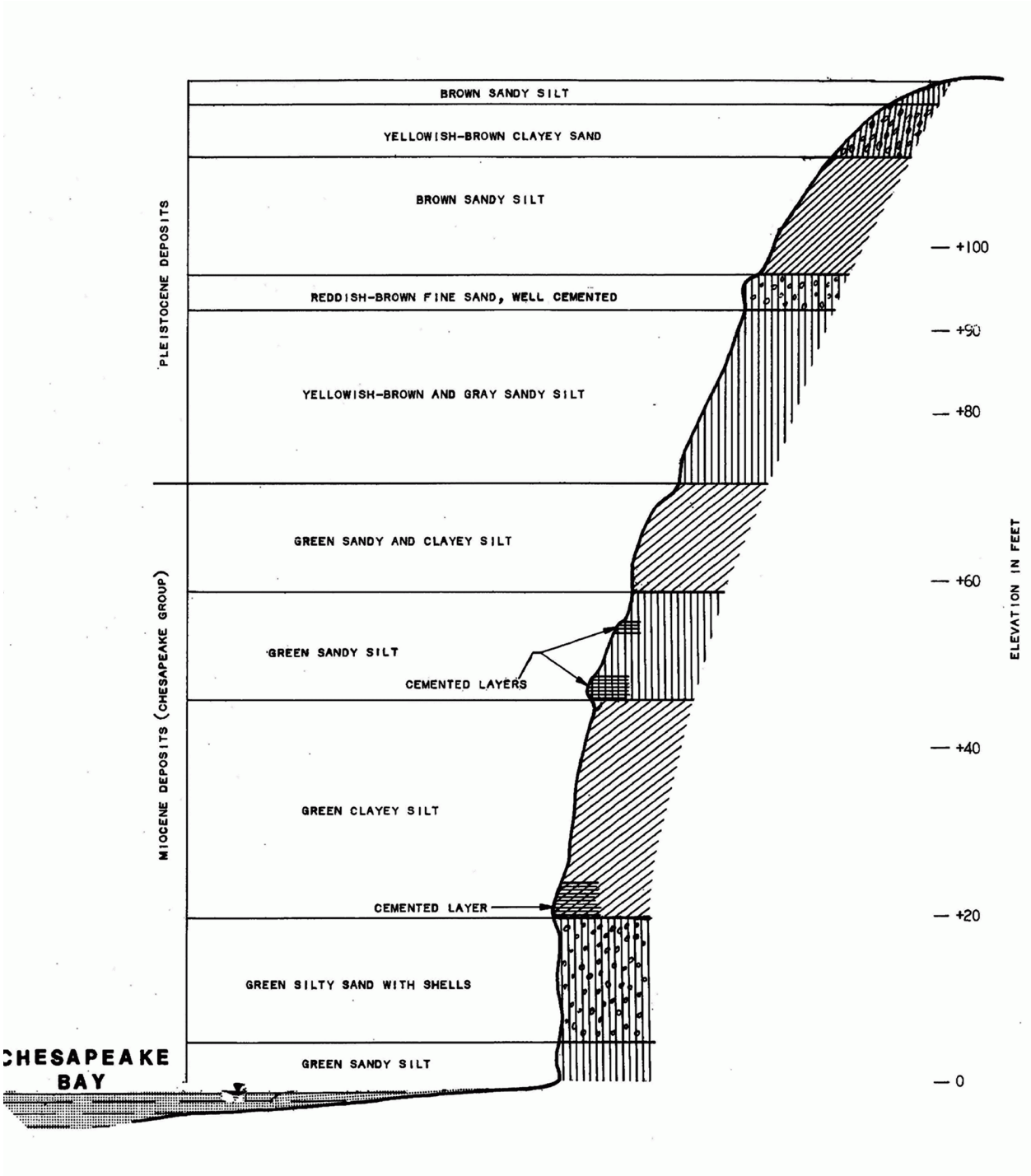


Figure 2.5-163—{Soil Stratigraphy and Properties for Slope Stability Analysis}

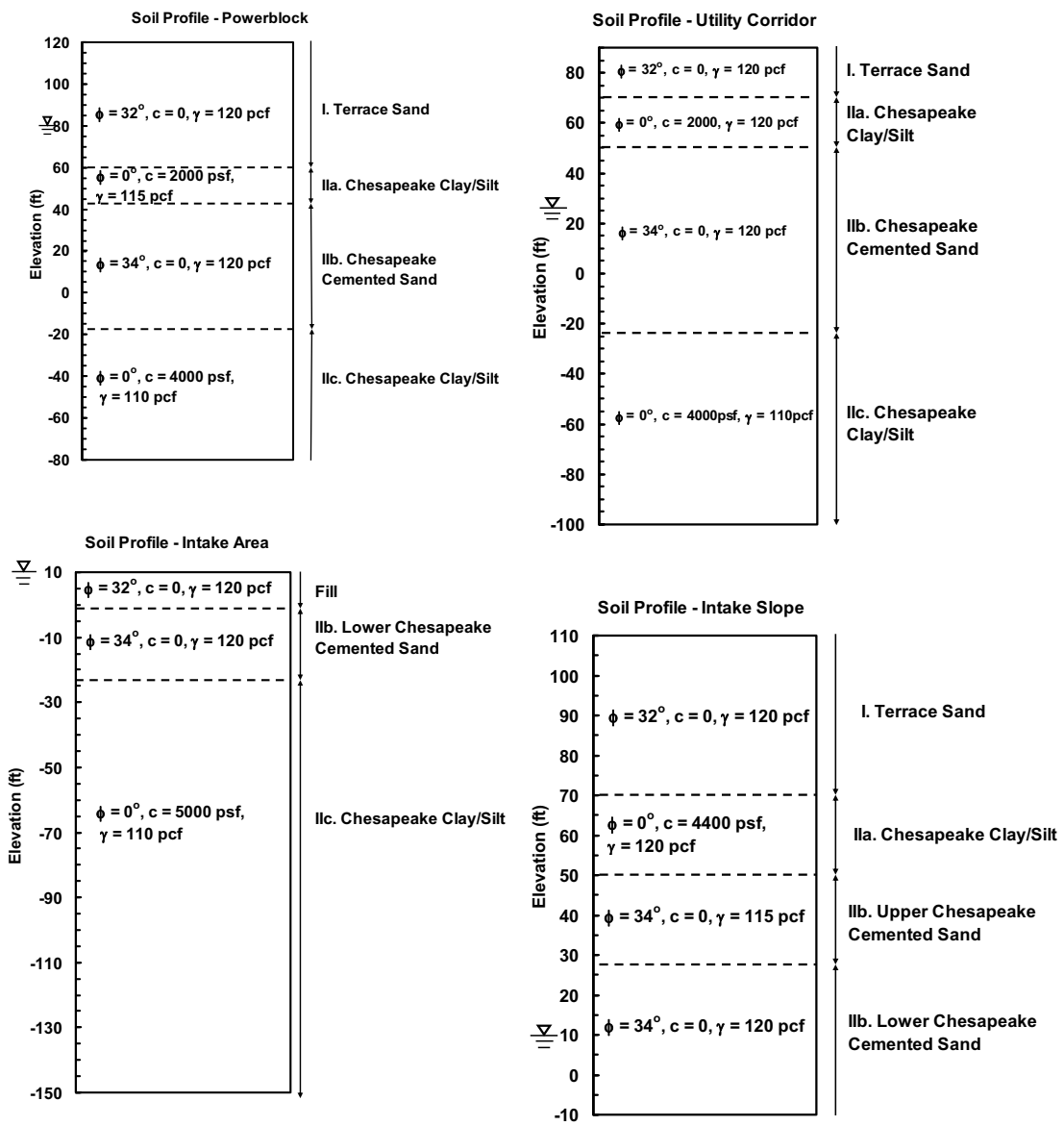
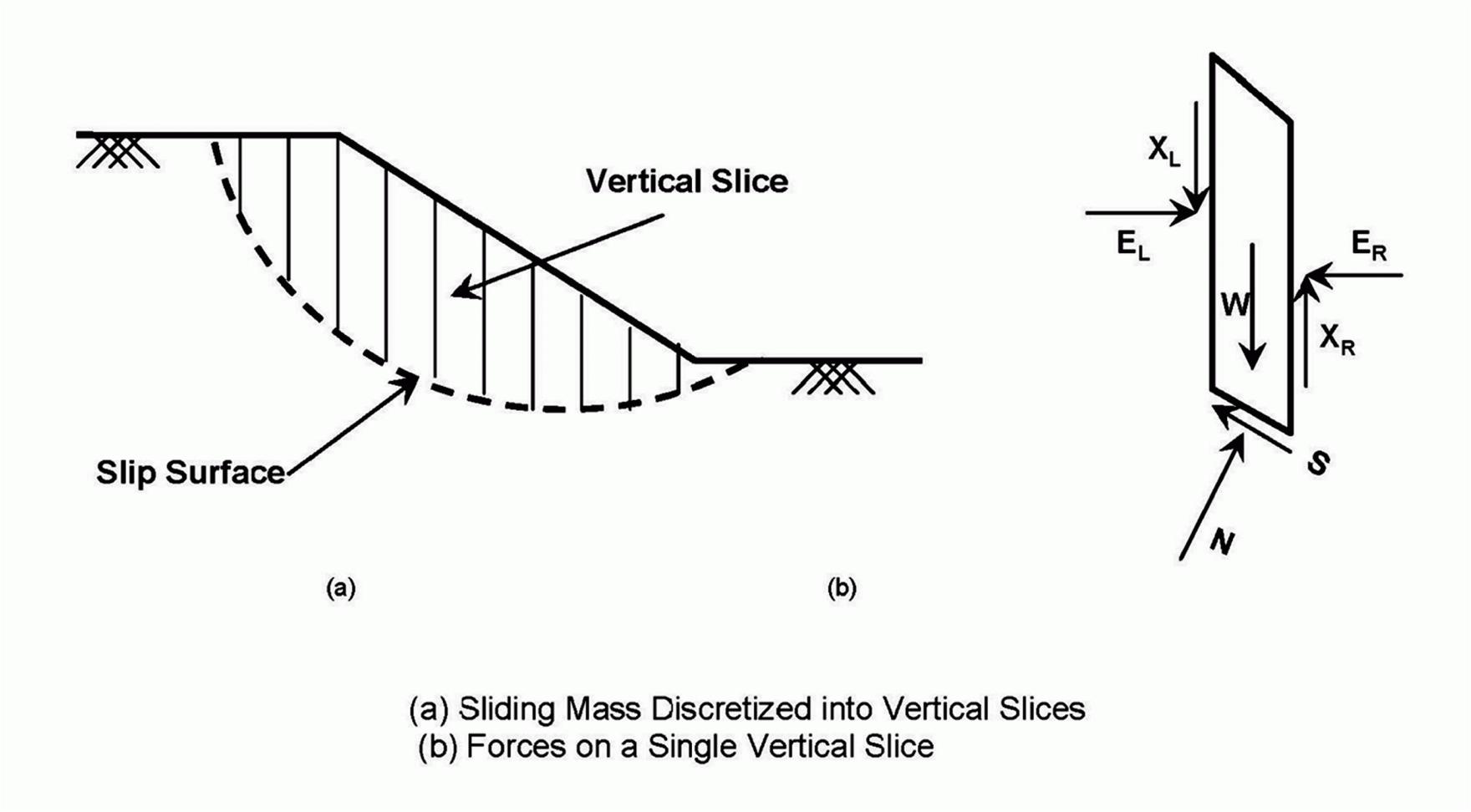


Figure 2.5-164—{Limit Equilibrium Slope Stability Analysis}



(a) Sliding Mass Discretized into Vertical Slices
(b) Forces on a Single Vertical Slice

Figure 2.5-165—{Static and Pseudo-Static Stability Analyes of Slope Section A}

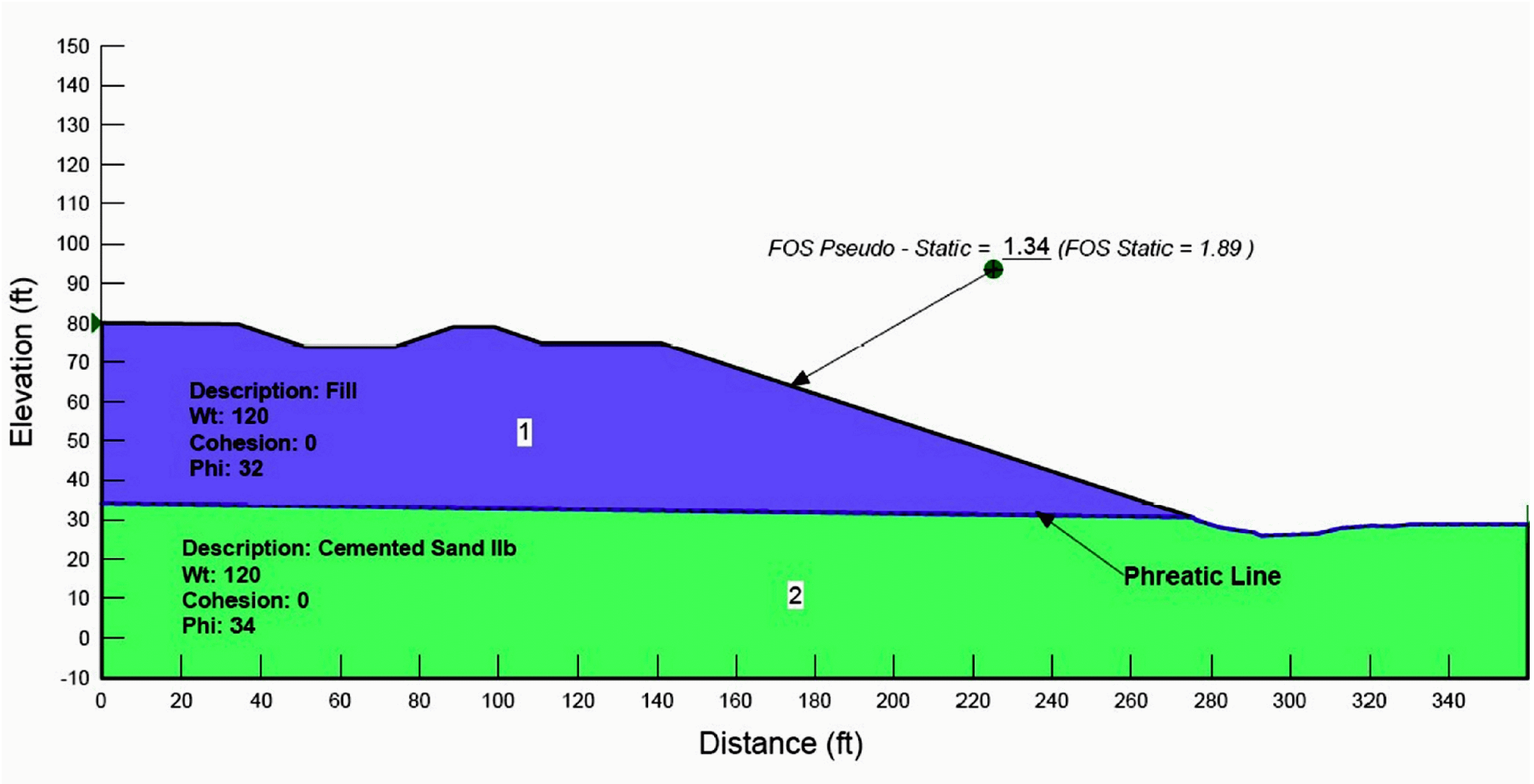


Figure 2.5-166—{Static and Pseudo-Static Stability Analyses of Slope Section B}

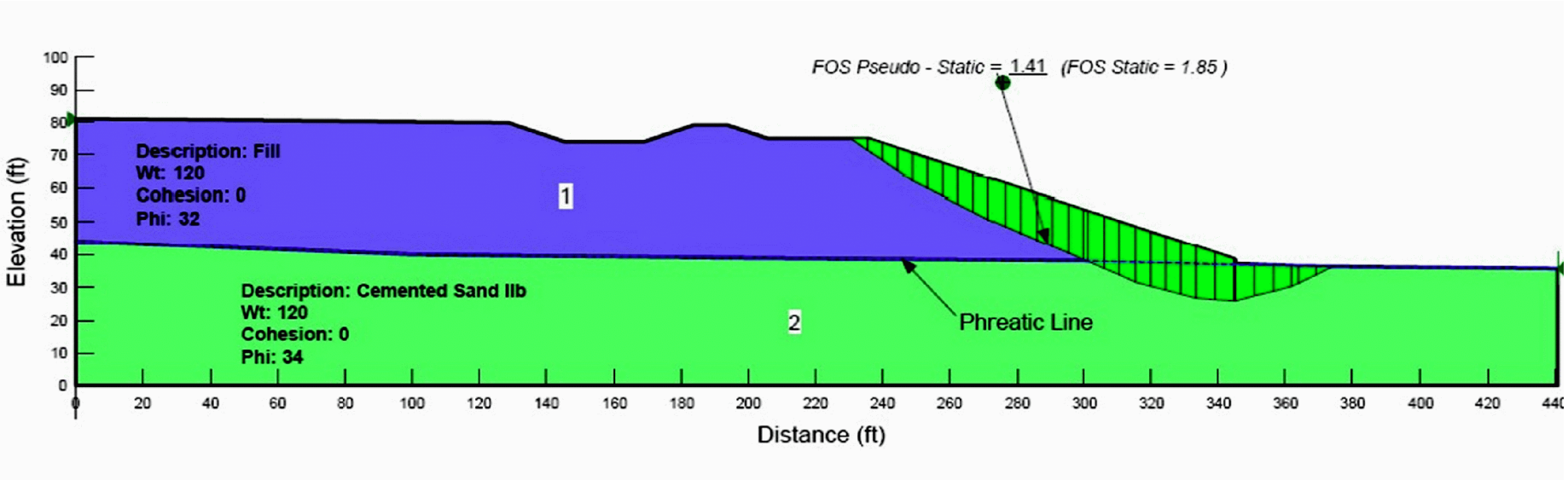


Figure 2.5-167—{Static and Pseudo-Static Stability Analyses of Slope Section C}

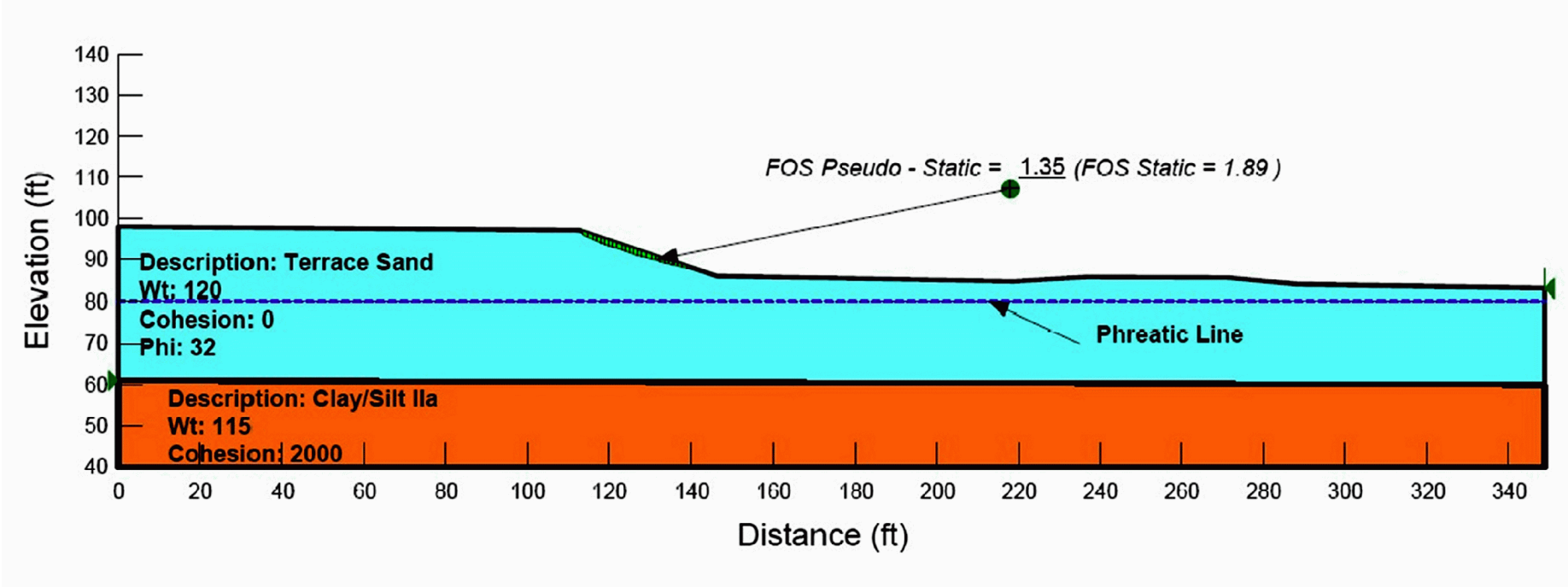


Figure 2.5-168—{Static and Pseudo-Static Stability Analyses of Slope Section D}

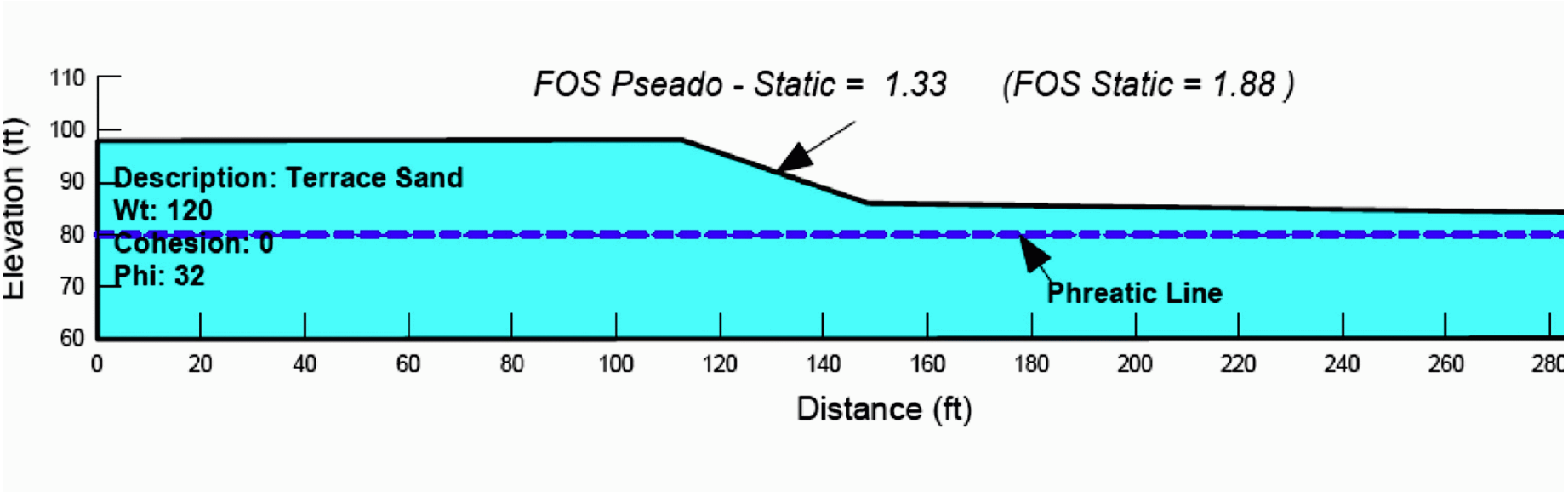


Figure 2.5-169—{Static and Pseudo-Static Stability Analyses of Slope Section E}

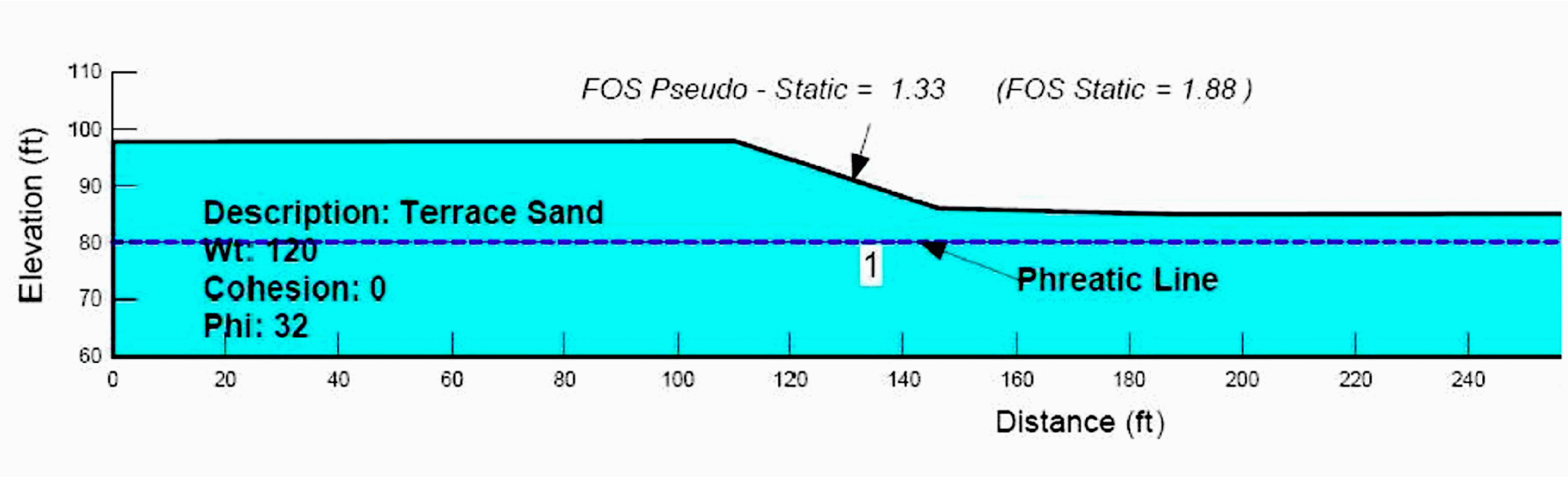


Figure 2.5-170—{Static and Pseudo-Static Stability Analyses of Slope Section F}

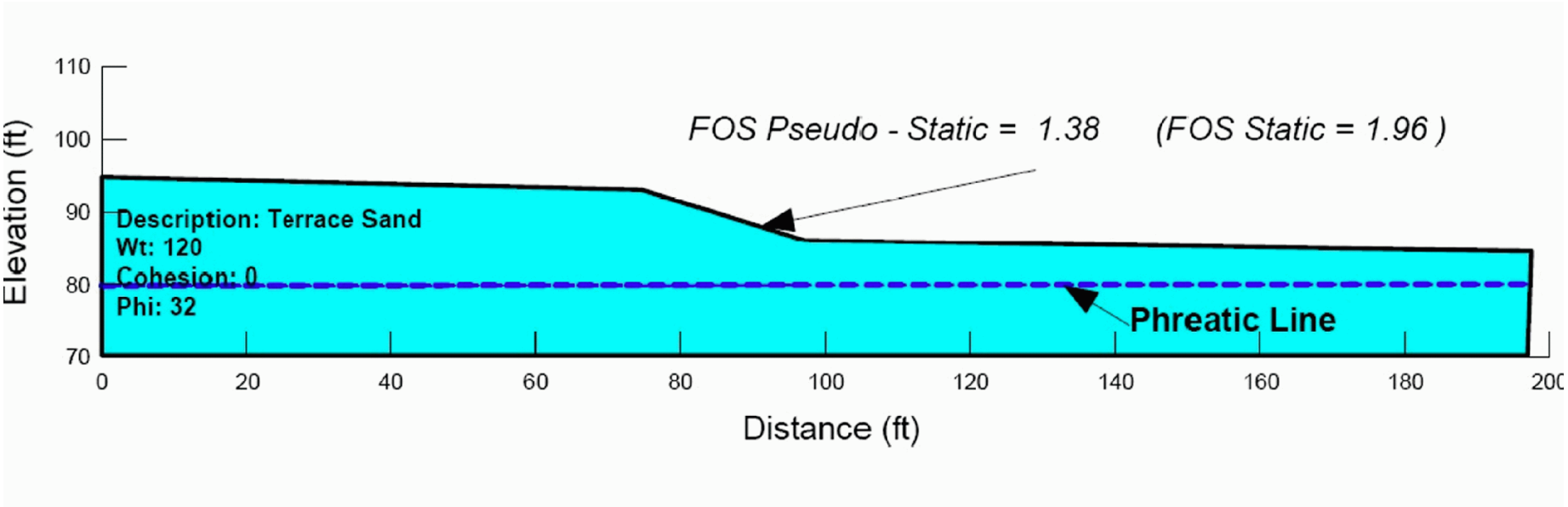


Figure 2.5-171—{Static and Pseudo-Static Stability Analyses of Slope Section G}

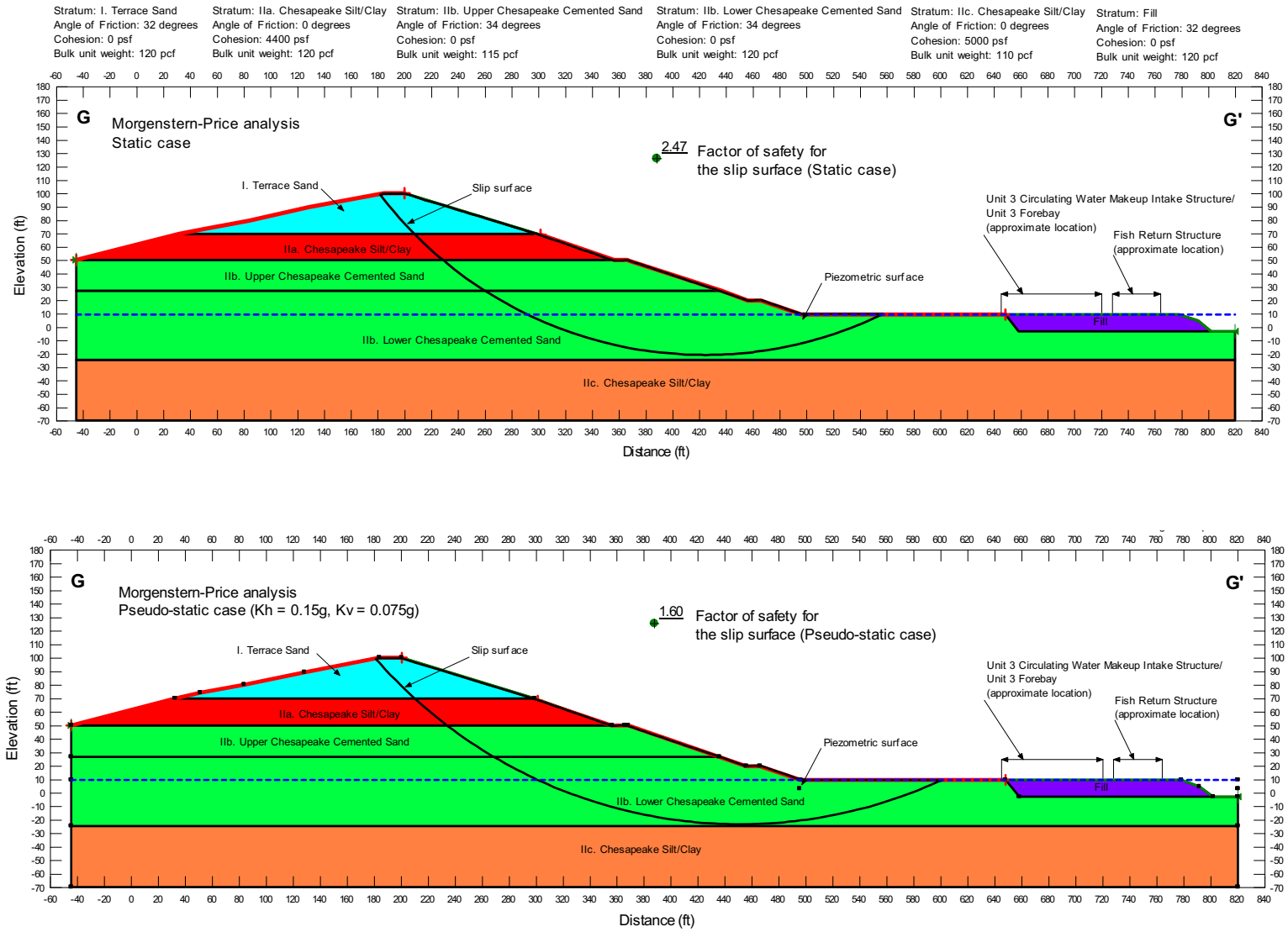


Figure 2.5-172—{Static and Pseudo-Static Stability Analyses of Slope Section A (Forced Deeper Surface)}

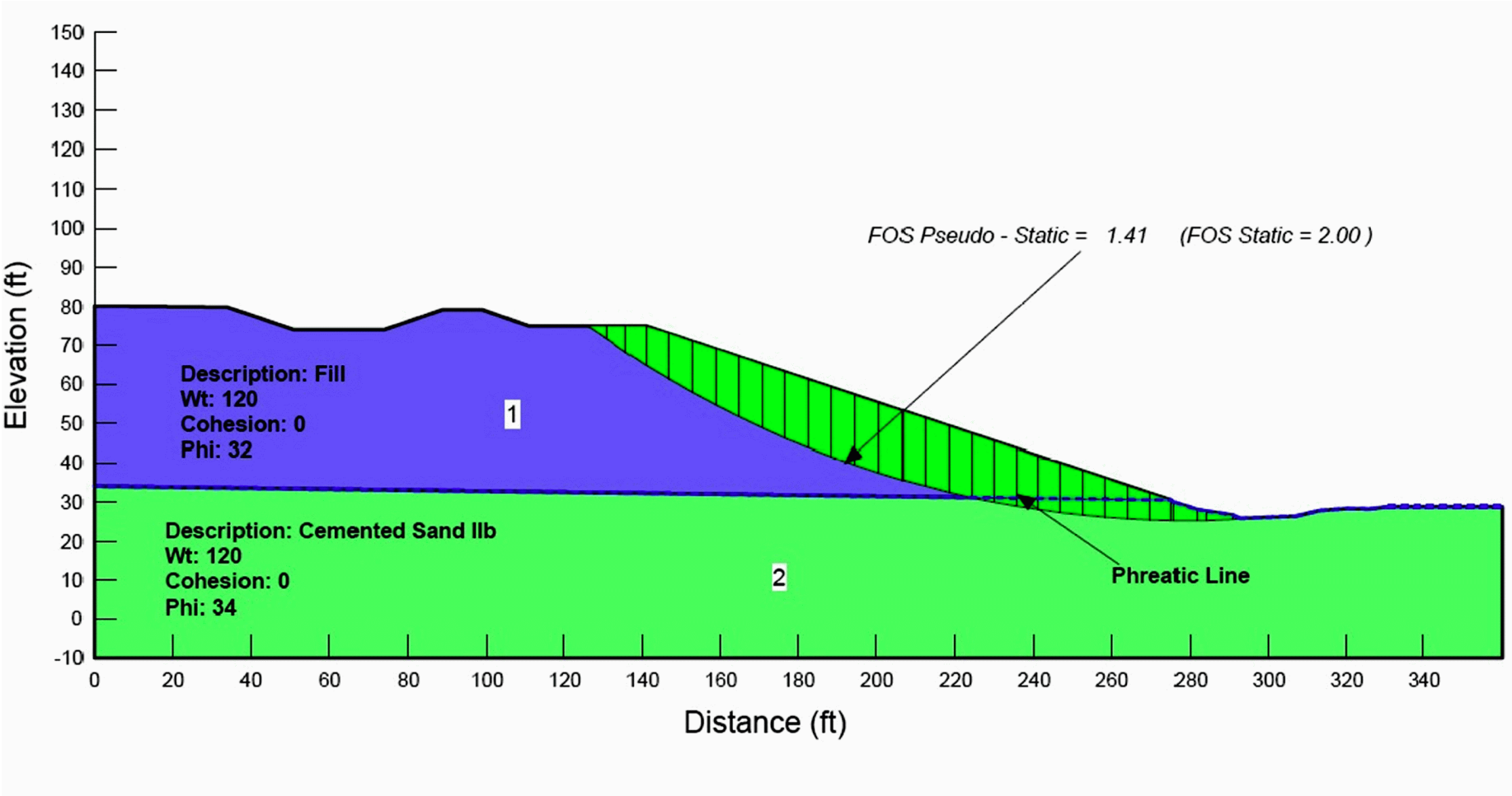


Figure 2.5-173—{Static and Pseudo-Static Stability Analyses of Slope Section C (Forced Deeper Surface)}

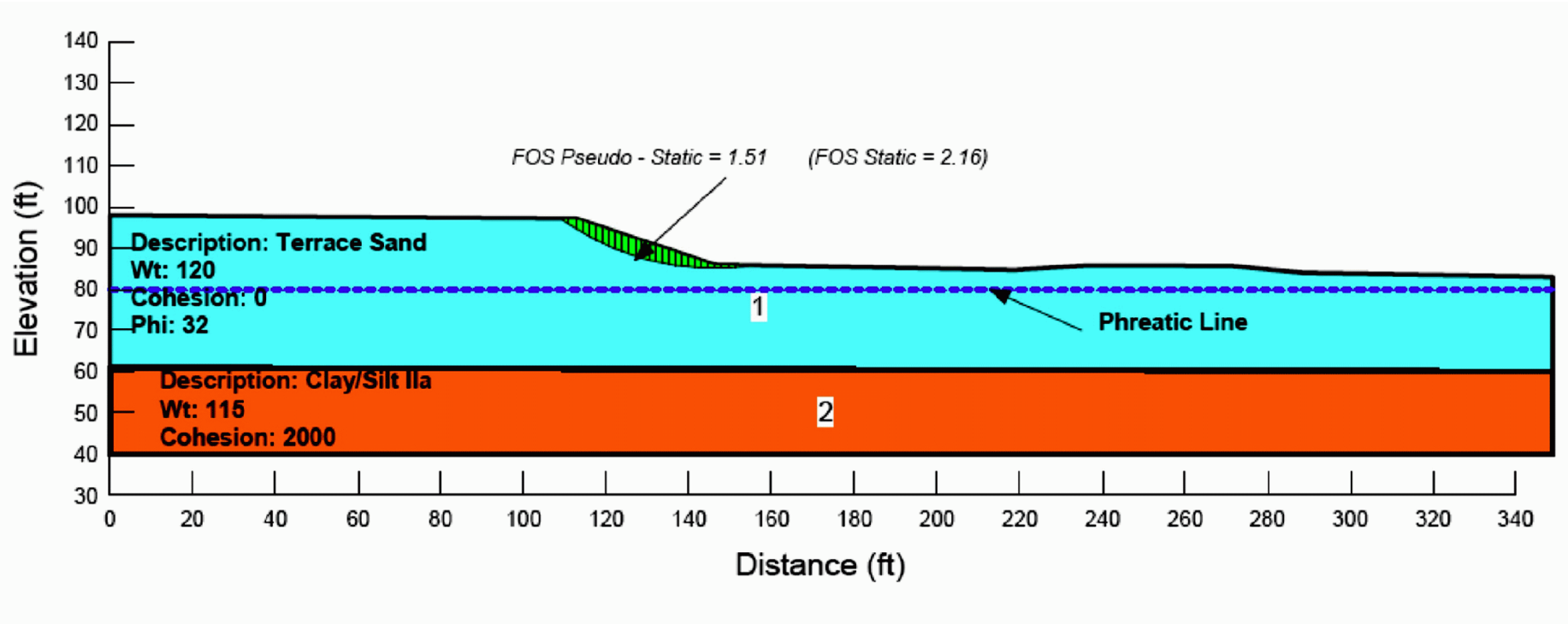


Figure 2.5-174—{Static and Pseudo-Static Stability Analyses of Slope Section D (Forced Deeper Surface)}

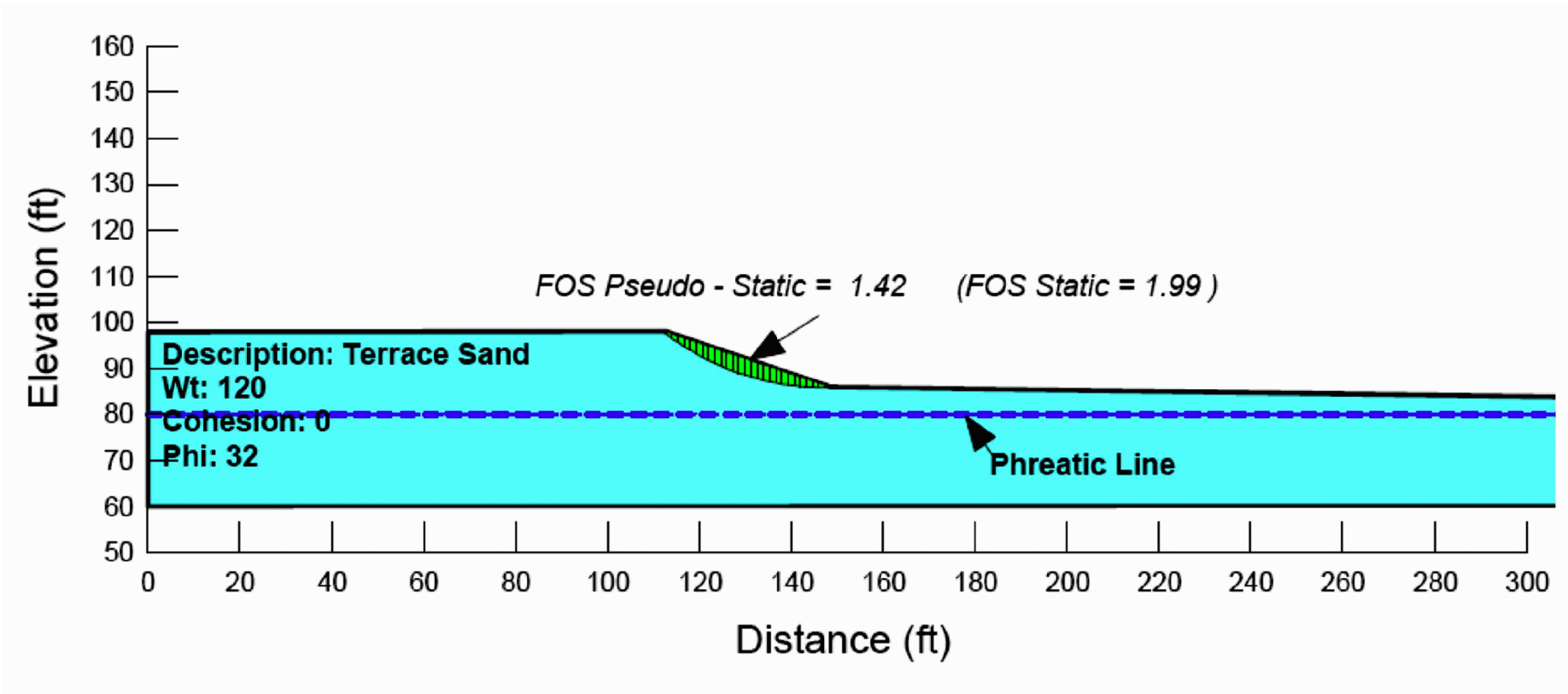


Figure 2.5-175—{Static and Pseudo-Static Stability Analyses of Slope Section E (Forced Deeper Surface)}

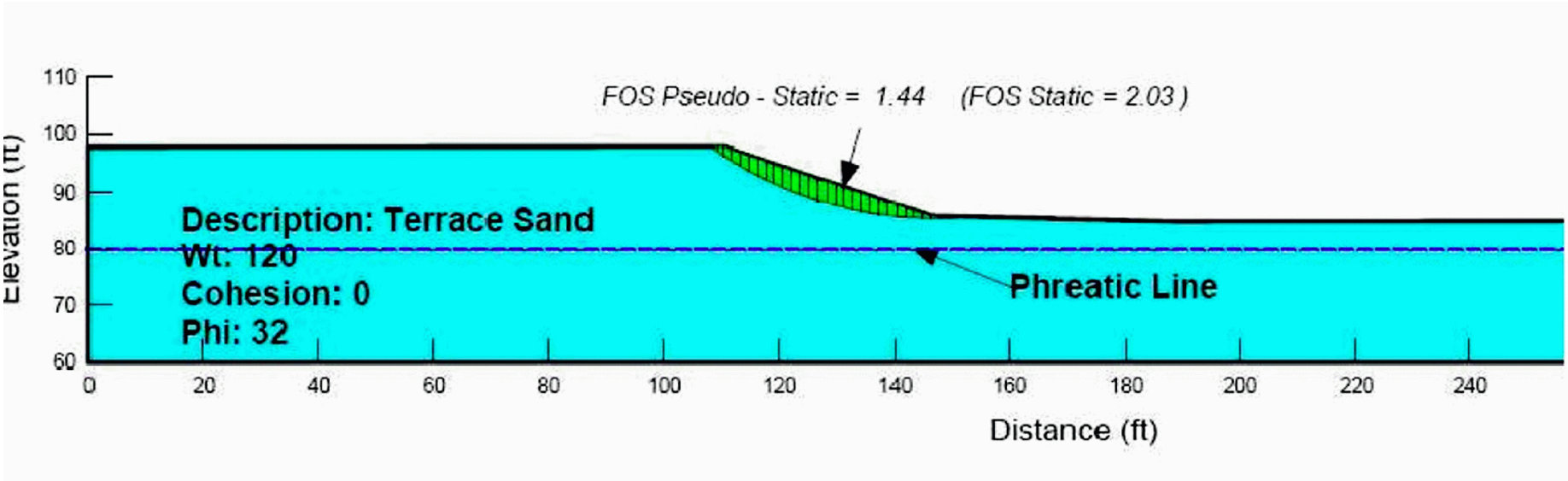


Figure 2.5-176—{Static and Pseudo-Static Stability Analyses of Slope Section F (Forced Deeper Surface)}

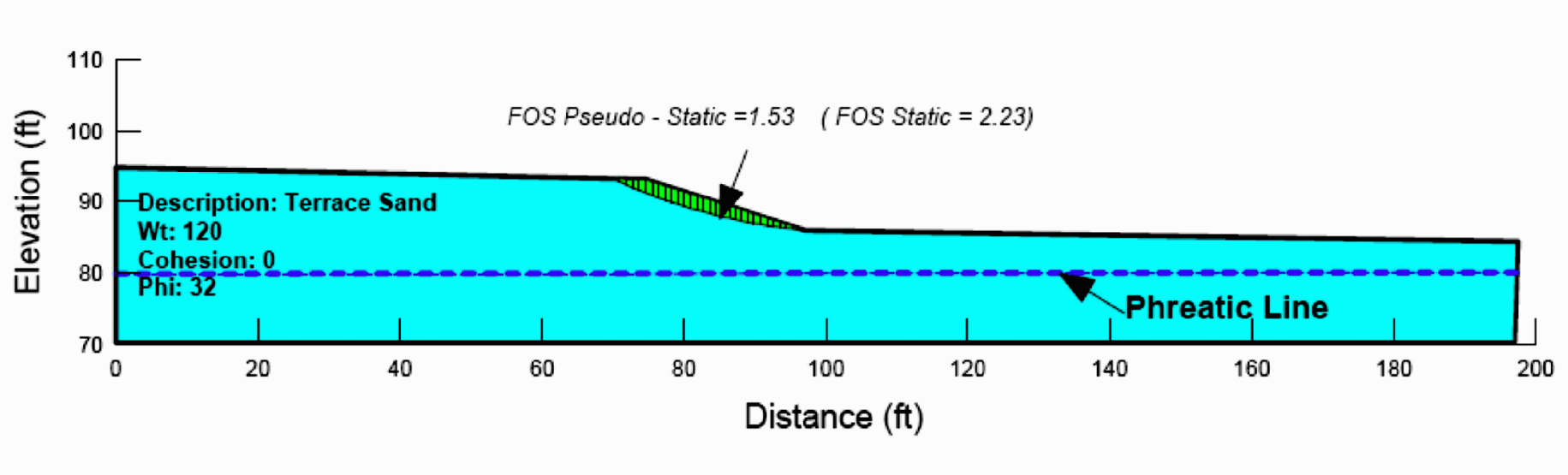


Figure 2.5-177—{Stability Analysis of Slope Section H}