ATTACHMENT 3

Peach Bottom Atomic Power Station Units 2 and 3 Docket Nos. 50-277 and 50-278

License Amendment Request Response to Request for Additional Information

Alternative Source Term (AST)

PBAPS Calculation PS-049, Revision 2, "Seismic Evaluation of Consoles 2(3)0C05A, 2(3)0C04A and 2(3)0C03 for Mod 955"

HENVICE
NUCLEAR ENGINEERING
DEPARTMENT

Mod# 955

6Pm

8. Description:

CALCULATION COVER SHEET

1. Calculation No. PS-049 Page 1 of 78 8			
2. 🗆 LGS 🖾 PBAPS	UNIT(S) 2 & 3		

M-20599 Rev. 2/89 DOCTYPE 061		Attachment 1, Pg1,2 A
3. Initialing Document:	4. Responsible Branch:	5. Total No. of Sheets:

9. System No.: (A) 70 B

Seismic Evaluation of Consoles 2(3)0CO5A, 2(3)0CO4A Ind 2(3)0CO3 for MOD. 955

Computers Branch

Structure: CONTROL RM/RAD WASTE BLDG.

Component: 2(3)0 co5A, 2(3)0 co 4A, 2(3)0 co 3

RECORD OF REVISIONS

10. No.	44	Description of Revision	1	2. Vendor Calc.		13. Other Calcs	14. Signatures		
10.140.				Number		Requiring Revision	Preparer	Reviewer	Approver/Date
0	issue for	use		19597-NM(c) PS-049	<u> A</u>	N/A	C H Chen	S. Wava (Nort /10-15-90 \ 10 123 19
<u>* / </u>	Added recorder	and various misc. cha	inges /	9597-NM(c) PS-049	B	N/A	2 14 Chen	7 Kowal	NBM /10-15-90 VIII. 17-1
		er trend record		N/A	MA	NA	1.2.01 Cal	In withe	Margare d' 12/90
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15. Related	Provides Info to:	N/A	116.10	1 (sel -				16.	
Calc. lumbers	Receives Inlo from:	N/A		4/2/91				1 '	Manual Computer
	Supersedes:	N/A						∞	mputer program and version

	MAN. CALC.	COMP.	ATTRIBUTES *	REV.	REV.	REV.	REV.	REV.	REV.	REV.	REV.
1.	×	×	SOURCES OF DATA & FORMULAE WERE REVIEWED AND VERIFIED TO BE CORRECT AND COMPLETE.	4K	416ml						
2.	×	X.	INPUT DATA FROM SOURCES IN ITEM 1, ABOVE, IS CORRECT AND PROPERLY EMPLOYED IN THE CALC.	JK.	416ml	BM			·		
3	×	x	CALCULATION ASSUMPTIONS WERE REVIEWED AND FOUND TO BE COMPLETE AND VALID.	The state of the s	7°C M	رمهل					
4.	x		THE ANALYTICAL METHOD EMPLOYED IN THE CALC. HAS BEEN CONSIDERED AND IS PROPER FOR THE INTENDED USE OF THE CALCULATION.	EK.	170 ml	Bul					
5.	x		MATHEMATICAL ACCURACY HAS BEEN CHECKED AND IS CORRECT (INDICATE METHOD USED):	YK	fignal						
1			a) COMPLETE CHECK OF EACH COMPUTATION	1 6		ggni					
			b) SPOTCHECK OF SELECTED COMPUTATIONS WHICH ARE INITIALED IN THE CALCULATION		NIA	NA Divid					
			c) PERFORMANCE OF ALTERNATE OR APPROXIMATION CALCULATION PER ERDP 39 CALC. IS ATTACHED.		NIA tre we	المراد المراد					
В	x	x	CALCULATION RESULTS WERE CHECKED AGAINST APPLICABLE, DOCUMENTED DESIGN CRITERIA AND FOUND TO BE IN CONFORMANCE.	K	N/A Figure	NA					
7.	X	X	EXISTING CALCULATION WHICH REQUIRE REVISION BECAUSE OF THIS CALCULATION HAVE BEEN IDENTIFIED.	N/A	-fre na	982					
B.		х	THE ANALYTICAL METHOD DESCRIBED IN THE COMPUTER CALCULATION SUM- MARY IS PROPER FOR THE INTENDED USE OF THE CALCULATION.	N/A	YA a						
9.		х	COMPUTATIONAL ACCURACY HAS BEEN CHECKED AND FOUND CORRECT (INDICATE METHOD USED): a) CHECK SAMPLE CALC. USING DATA OTHER THAN USED IN SAMPLE. b) PERFORMANCE OF ALTERNATE OR APPROXIMATE CALCULATION PER ERDP 39		1/A 10 m	Mr Mal					
	·	:	CALC. IS ATTACHED. SWEC. GA ACCEPTED C) DESCRIBE OTHER METHOD IF 9.A OR 9.B IS NOT USED. COMPUTER PROGRAM	NIA							
0		x	OUTPUT IS REASONABLE CONSIDERING THE INPUT.	1K	fry m	10 J					

THESE ARE THE MINIMUM ATTRIBUTES AND ARE NOT INTENDED TO LIMIT THE INITIATIVE OF THE CHECKER TO REVIEW OTHER ATTRIBUTES. ATTRIBUTES APPLICABLE TO MANUAL AND COMPUTER CALCULATIONS ARE NOTED BY AN (X) IN THE APPROPRIATE COLUMN. FOR CHECKING OF REVISIONS TO MANUAL CALCULATIONS, THE ATTRIBUTES MAY BE LIMITED TO ONLY REVISED PORTIONS OF THE CALCULATION. CHECKER SHALL INITIAL EACH ATTRIBUTE COMPLETED.

STONE & WEBSTER ENGINEERING CORPORATION CALCULATION TITLE PAGE

						·	
CLIENT PHILADEI PROJECT PEACH B	LPHIA ELECTRIC CO. IOTTOM ATOMIC POWER S	STATION - UN	NIT .			PAGE 3 (78 AB
	Evaluation of				· · · · · · · · · · · · · · · · · · ·	OA CATEO	
2(3)00	04A and 2(3)0	C03 +	for MOD	955		REL	ATED
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a-89/05/01

CALCULATION SHEET

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SWEC Tit	-		•	3
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2.0 Ob	jective			10
3.0 Dea	sign Input			11
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		f Modification (MOD) 955		
Sug	ports/Braces			36
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		on of Support for 13 in. RWM (on of Panel Switch Support	JRT	38
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6.2.6	- -	f Calculated Stresses with		-
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·		Unit 2 Console 20C05A	324,32	, 7
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	tracor westr	onics multipens recorder		-, }
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0.0	Devices and Con				
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В	52a 52b		support design for 5A. This change is	RWM CRT for unit 2 required due to spac	e		
He v.	52c	availability	of unit 2 console.				
10/23	(acement of a Gemax 6	"x6"		
1010	52d	computer tre	nd recorder with a 6	"x6" tracor westroni	cs		
	52e	multipens re	corder. This modifi	cation was not			
		originally p	laned for installati	on during the upcomi	ng		
]		outage.		· -	_		
			nges for bolts, nuts	, washers and screws	to		
	16		endor deviation requ				
	25	Replace page 16 with a new page.					
				of existing Q devic	es		
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		& 2 above.					
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	6			4 & 5" to "item 4".			
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11	1,2		d pages 16 and 58 as	Attachment 1 for re	cord		
2		Revision 2 in	ncludes the following	g changes:			
	6,11	1. Removal of t	the existing RWM open	rator interface and	· · · · · · · · · · · · · · · · · · ·		
	52 d	replacement	with a new 6" x 6" o	computer trend			
	52 f	recorder.					
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CALCULATION SHEET

5010.65

 CALCULATION IDENTIFICATION NUMBER

JOB NUMBER DIVISION & GROUP CALCULATION NO. OPTIONAL TASK CODE PAGE

19597 NM (C) PS-049 N/A 6

1.0 INTRODUCTION

PECO Nuclear Engineering Department intends to install additional devices on control room consoles 2(3)0C05A, 2(3)0C04A and 2(3)0C03 (hereafter referred to as 05A, 04A and 03) during the upcoming outage in October, 1990 (ref. 1). This installation is identified as modification (MOD) 955, which involves mounting of the following devices:

05A

- 1. Remove the existing computer trend Gemax recorder and install a new 13 in. Rod Worth Minimizer (RWM) CRT
- 2. Install a panel switch
- 3. Install two digital displays with associated electronics and power supplies.
- 4. Remove old RWM operator interface and install a 6"x6" computer trend recorder.
- 5. Remove Gemax 6"x6" computer trend recorder and replace with 6"x6" Tracor Westronics multipen recorder

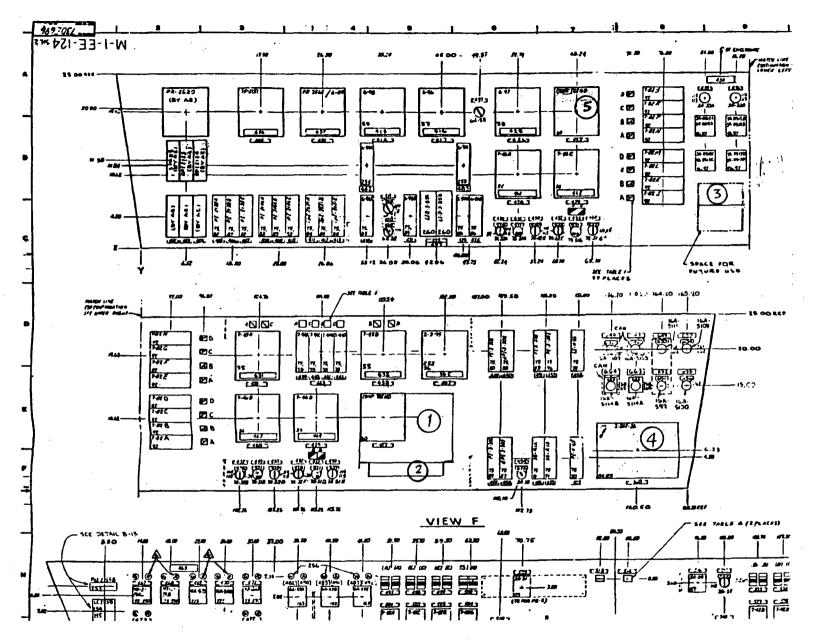
Note: Items 4 and 5 will not be installed during the upcoming outage. Therefore, qualification of supports for these items will not be performed in the calculation.

04A

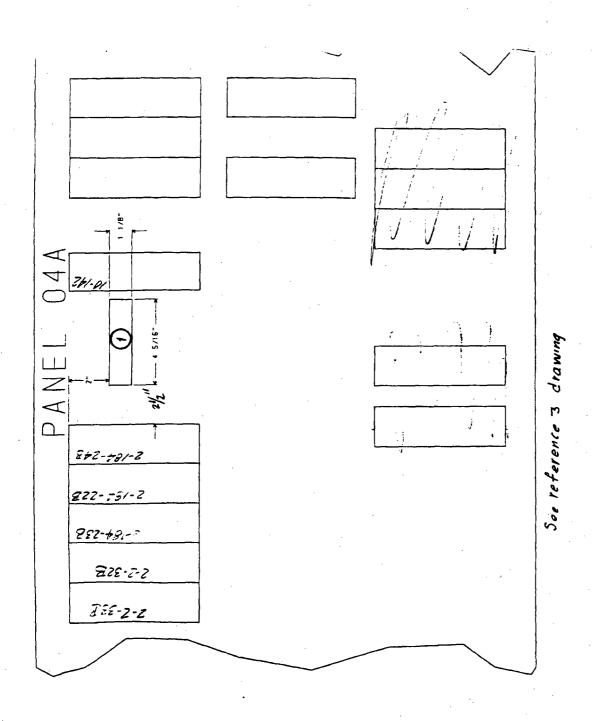
1. Install a digital display with its associated electronics and power supply.

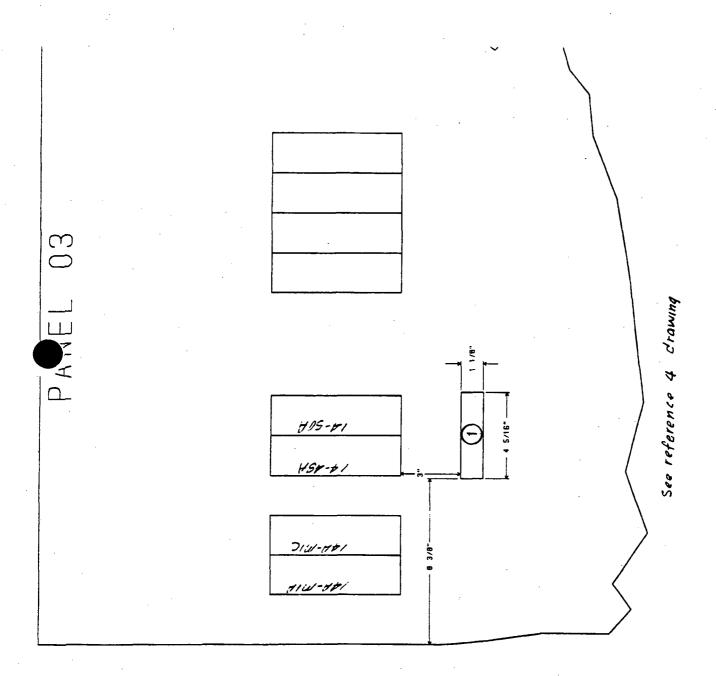
1. Same as 04A

The locations of the above modifications are identified on the attached sketches (pages 7, 8 AND 9).



Console OSA





PANEL 04A

CALCULATION SHEET

5010.65

CALCULATION IDENTIFICATION NUMBER					
JOB NUMBER	-DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	PAGE	
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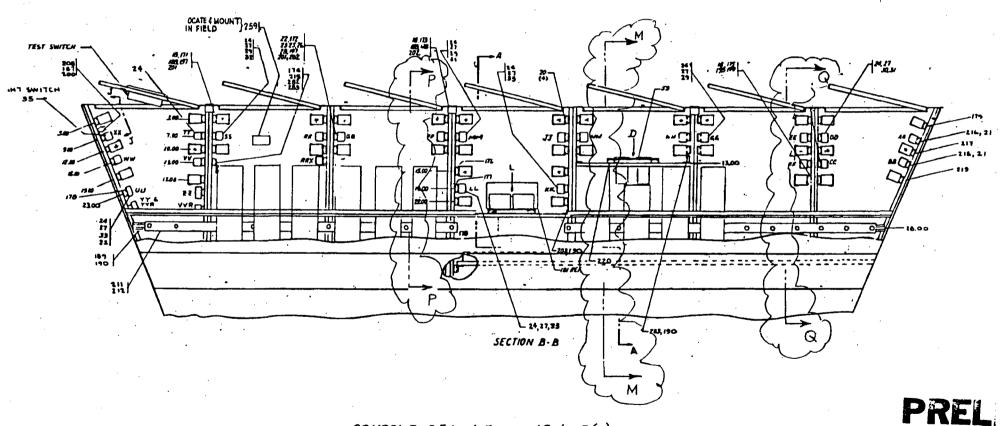
The above modifications involve cutouts of the console panel to provide openings for mounting the devices, and attachment of the devices to console structural members, etc. Therefore, the impacts that may arise from the modifications need to be addressed, which are stated in the "Objective" section.

2.0 OBJECTIVE

The objectives of this calculation are:

- 1. To demonstrate the adequacy of MOD 955 supports/braces.
- 2. To address the impact of the modifications on the existing consoles and devices.
- 3. To assess the impact of non Q devices on the Q devices and consoles and provide resolution as necessary (for console 05A only).
- 4. To provide seismic capability limits of existing Q devices from SWEC/Vendor data bases (for console 05A only).

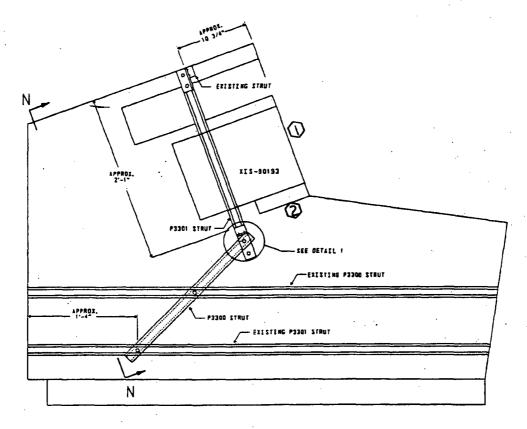
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3.0	Design Input				
		s (provided by Pi	ECO)		
	Rod Worth Min	imizer(RWM) CRT		35 lb	
	Panel switch			. 5	
	RWM cable con	nector		2	
	Digital displ	ay		1	•
		•	٠		
	Electronic bo			2	
	6"x6" Compute	r Trend Records		20 /2	
	Power supply		_	4	-
	Tracor westro	nics multipens re	ecorder	20 🔼	
	Material		Sy	Su Re	ef.
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	A-569 sheet	metal	25	*	
•	Bolt, A-307			60 5	
	Stud, A-108 G	r. 1010 or 1018		40-60 **	:
	Screw, A-193,		105	125 5	
•	Unistrut A-57		33	52 7	
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	* Metal Hand	book, 8th ed. Ame	erican Soc	iety for Met	als
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	and Deform Industry. Allowable Stress A-569 sheet Bolt, A-307 Stud, A-108 G O Screw, A-193, Unistrut A-57	es, ksi Te metal 0.6 S 26 r. 1010 or 1018 0.33 Gr B7 0.33	ensile Sy(15) - 1.8 fv Su(13.2) Su(41)	Shear 0.4 Sy(10) 20 0.17 Su(6.8 0.17 Su(21)	Ref 6 6) 6
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CONSOLE OSA (Pages 12 to 26)

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INTERIM DRAWI



SECTION M-M

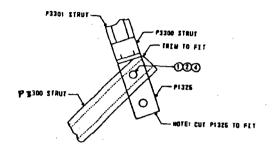
13" RWM CRT
Panel switch

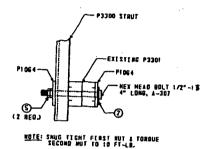
NOTES:

ALL CONNECTIONS USING UNISTRUT BOLTS SHALL BE NOTED:

1/4" BOLT 6 FT-LB.
1/2" BOLT 50 FT-LB.

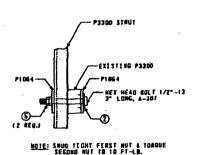
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I TEM NUMBER	PART DISCRIPTION	CAT. NUMBER
0	HEX HEAD BOLT 1/2"X 1 3/16"	HHC2020113EC
2	1/2" WASHER	HFLW050EG
3	FLAT PLATE	P1 06 4
4	1/2" SPRING NUT	P4010
<u>(S)</u>	1/2" NUT	HHXN050EG

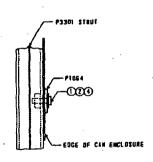




DETAIL 1

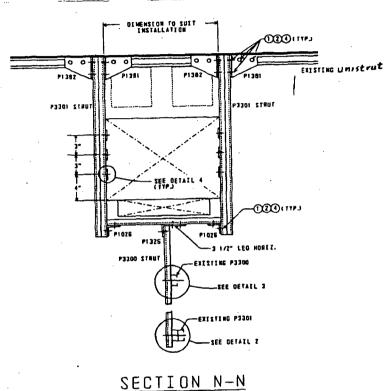
DETAIL 2

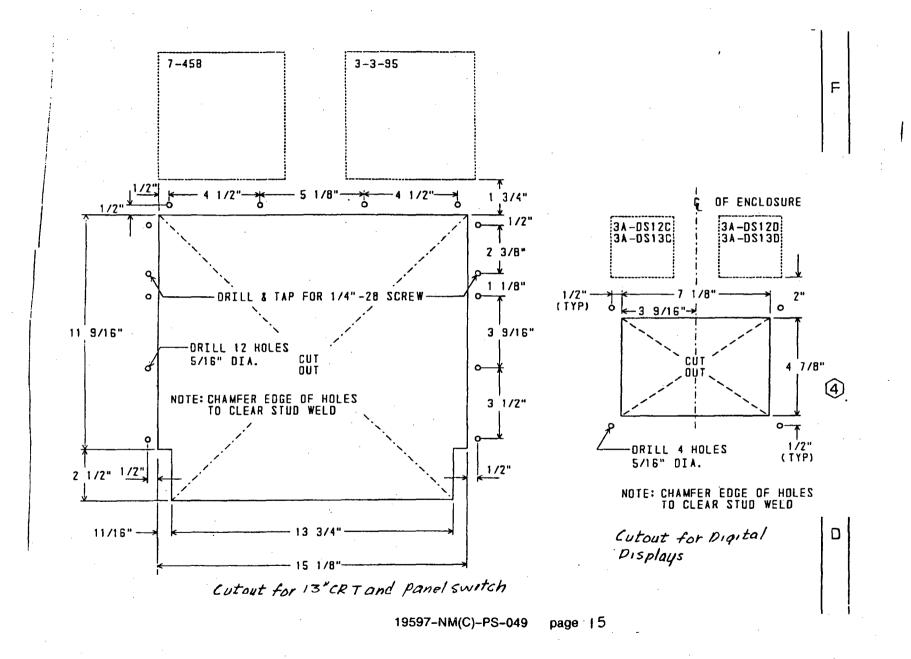


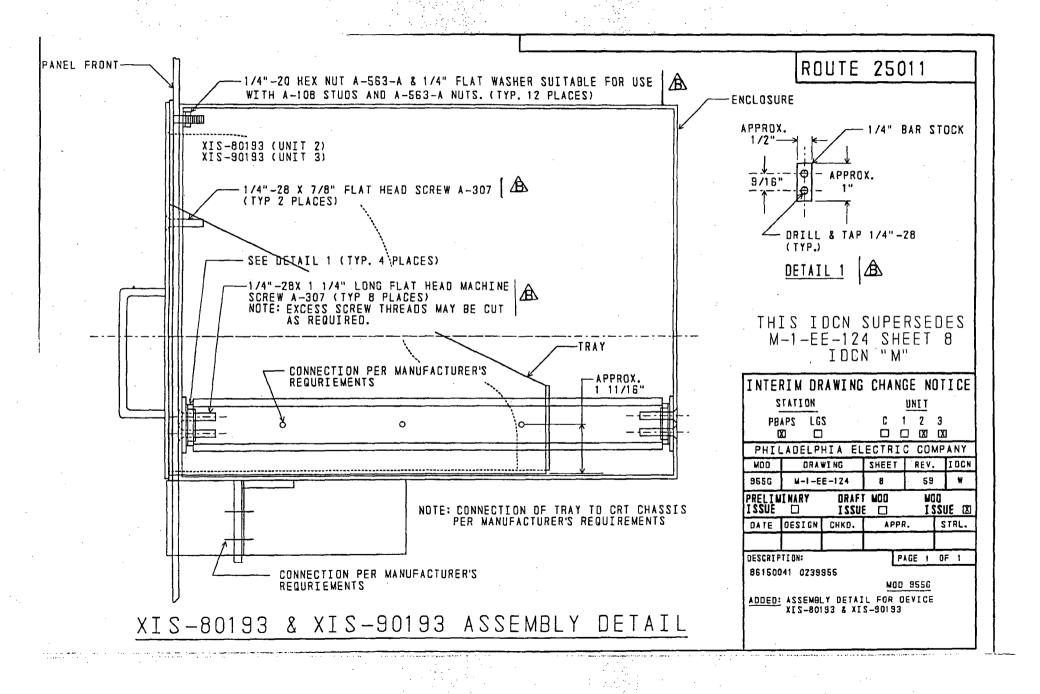


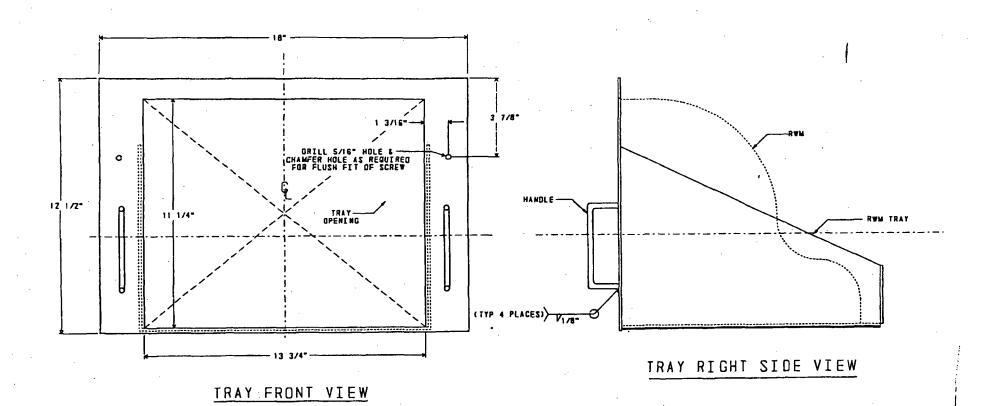
DETAIL 3

DETAIL 4

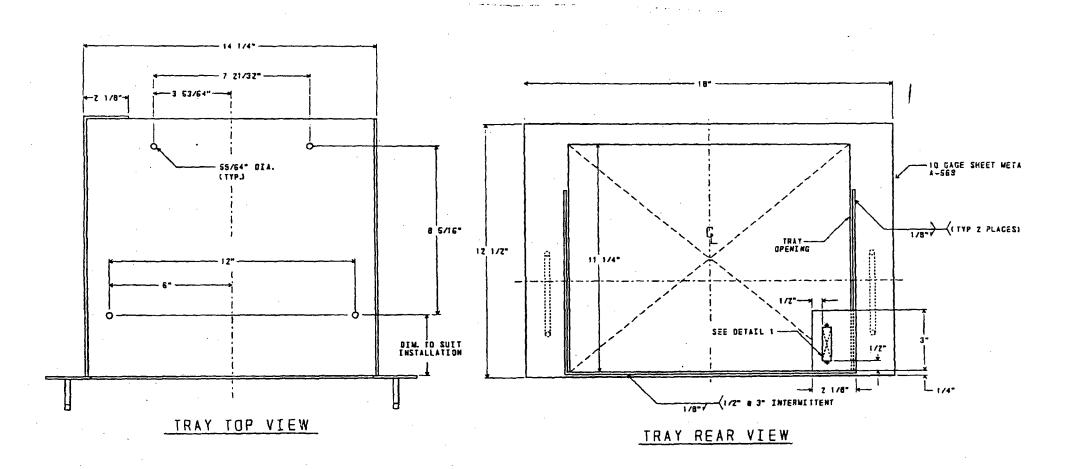




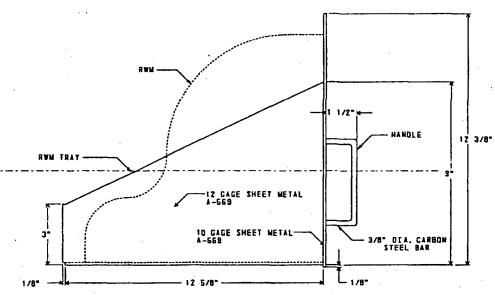




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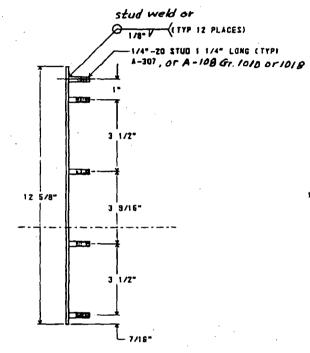


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TRAY LEFT SIDE VIEW

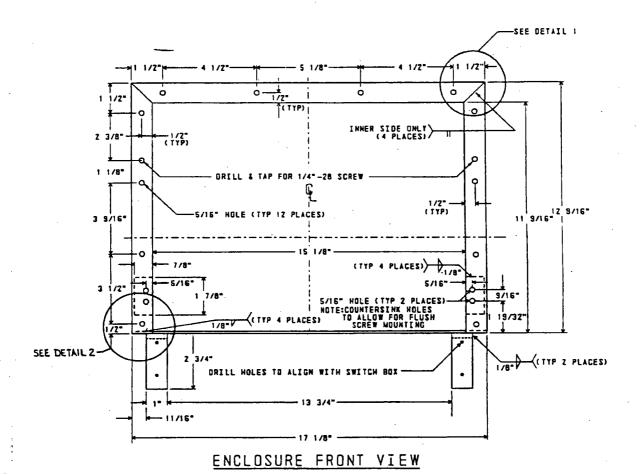
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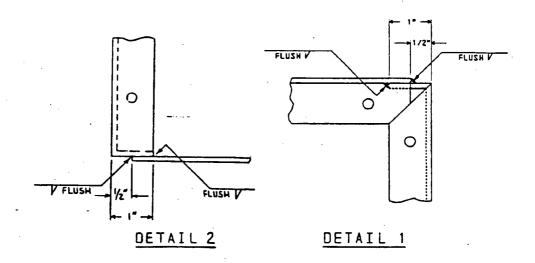


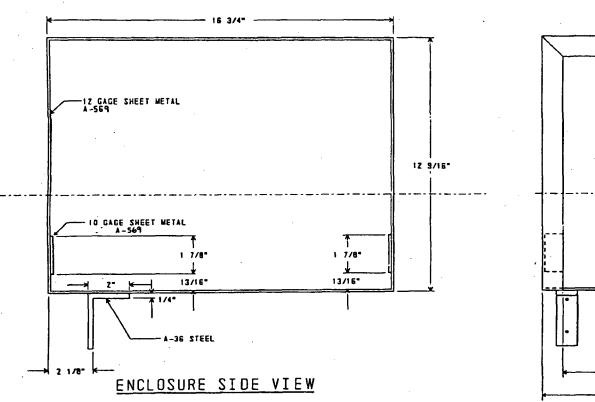
11 15/16" | 1 15/16" | 1 15/16" | 1 15/16" | 1 15/16" | 1 15/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" | 1 17/16" |

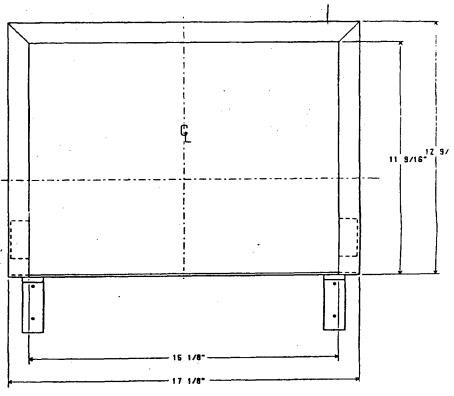
COLLAR SIDE VIEW

COLLAR FRONT VIEW

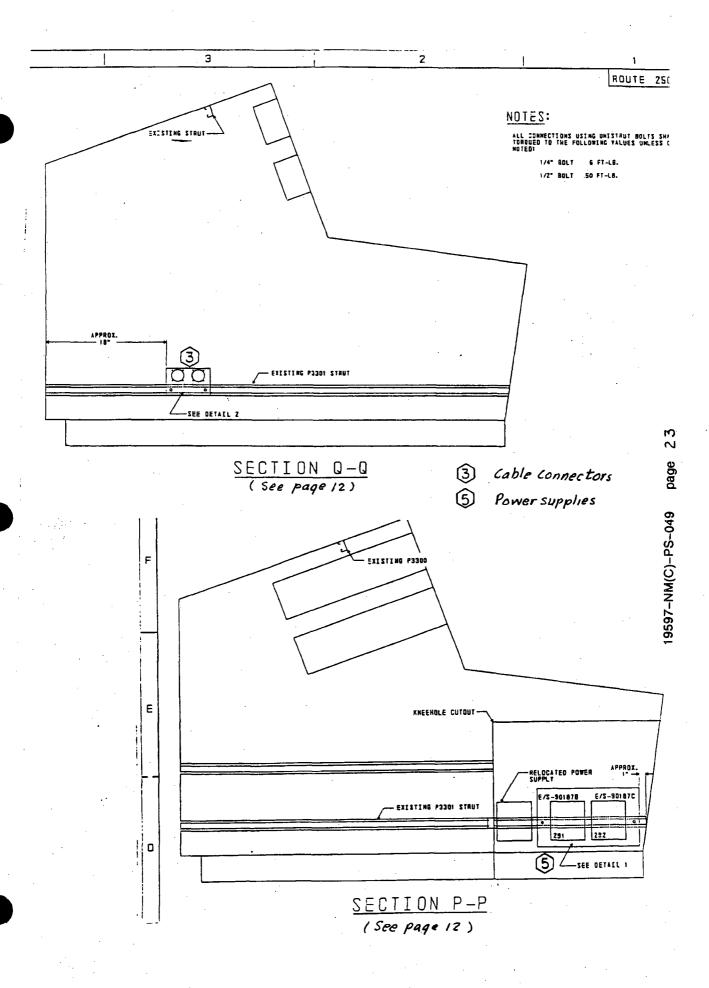


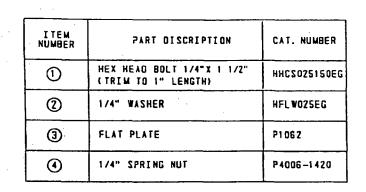


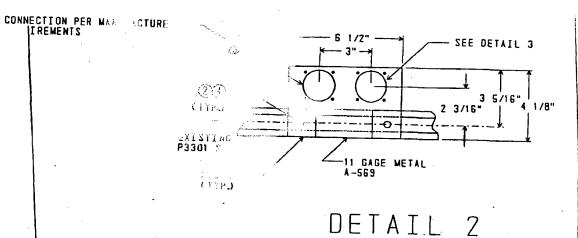


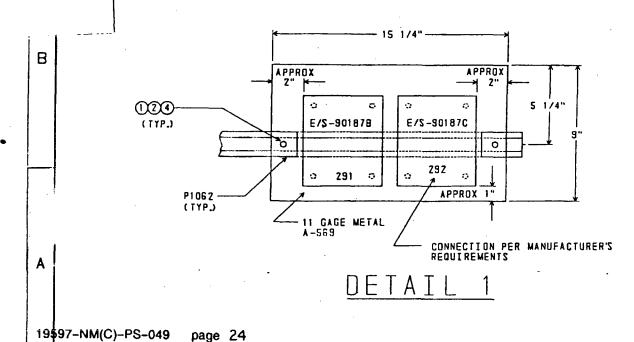


ENCLOSURE REAR VIEW







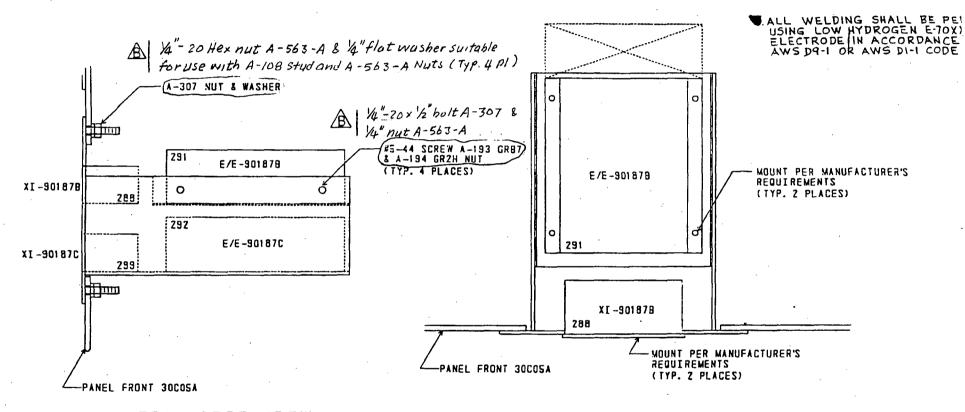


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ROUTE 251

NOTES :

ASSEMBLY DETAILS



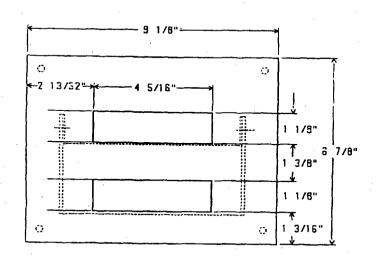
TRAY SIDE VIEW

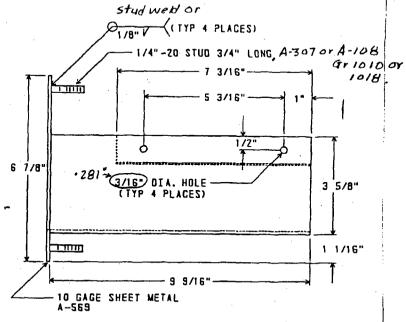
TRAY TOP VIEW

Digital Displays &
 Electronics Boxes

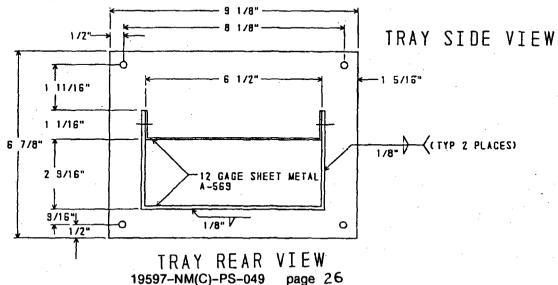
Console 05A

FABRICATION DETAILS

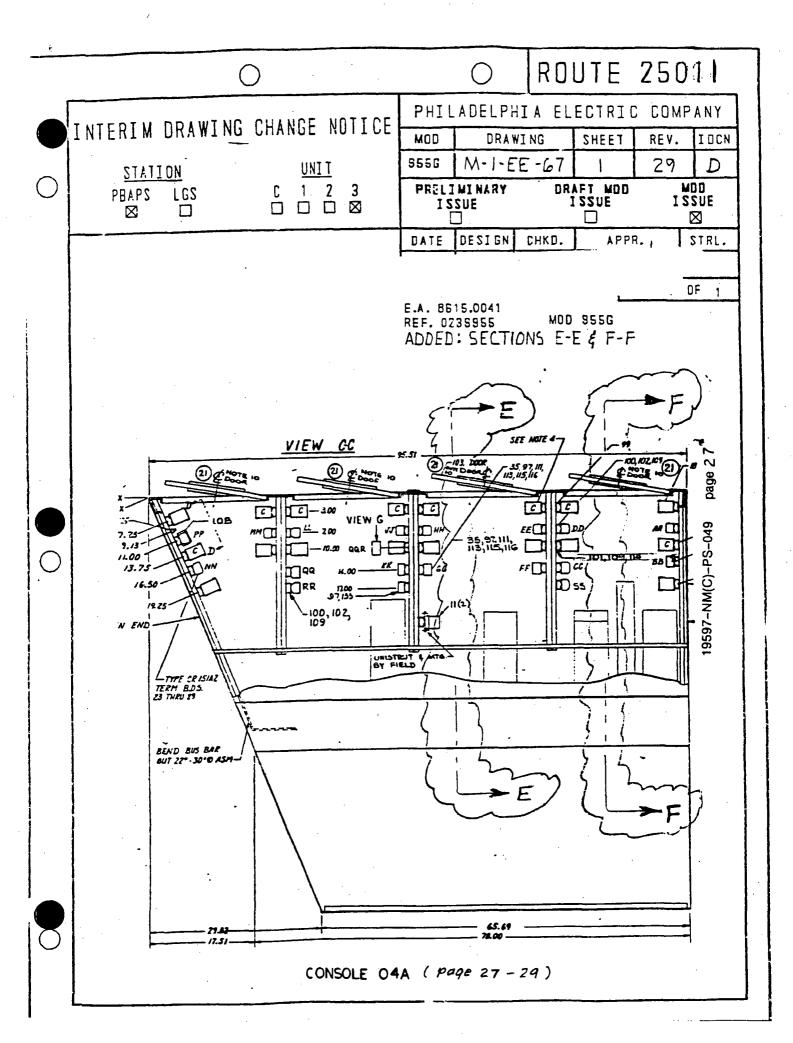


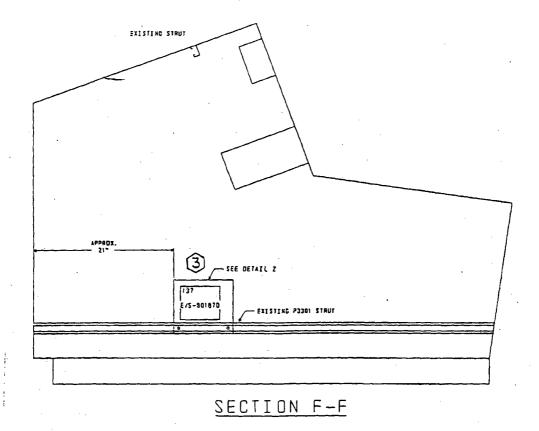


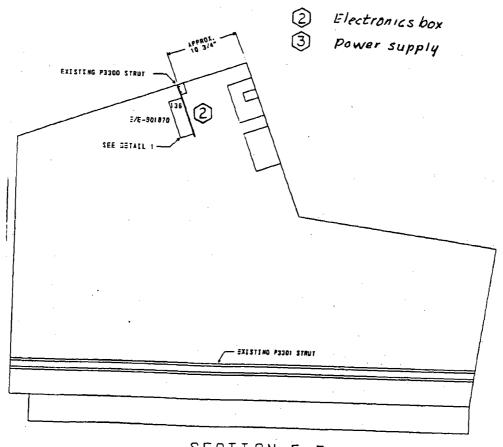
TRAY FRONT VIEW



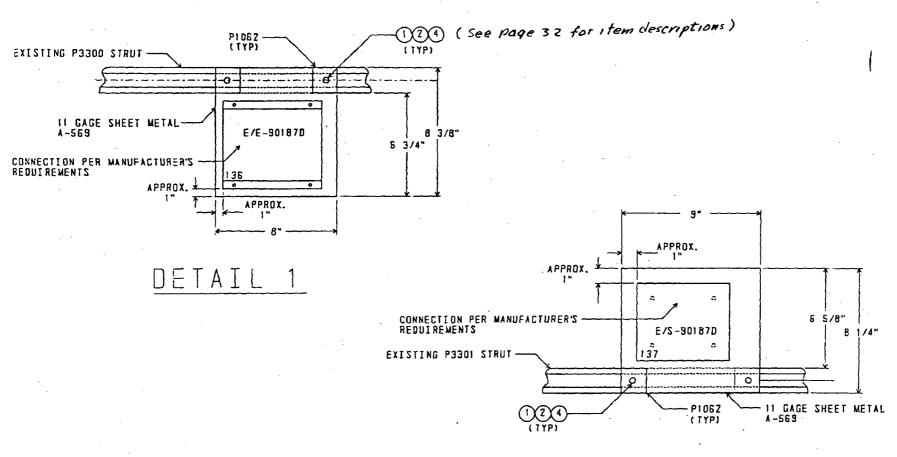
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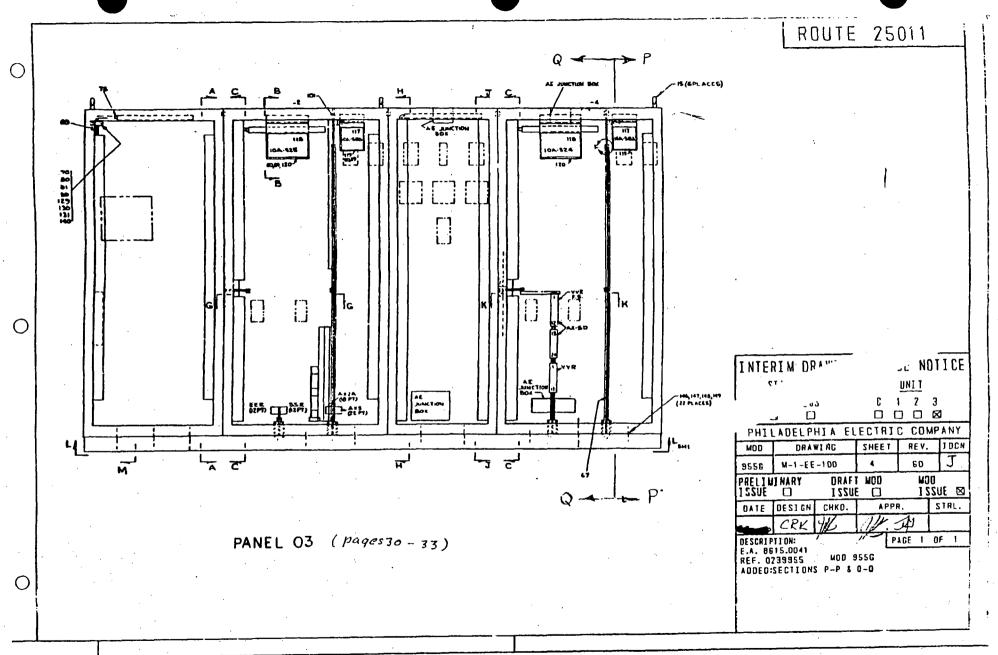




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DETAIL 2



1

ROUTE 25011

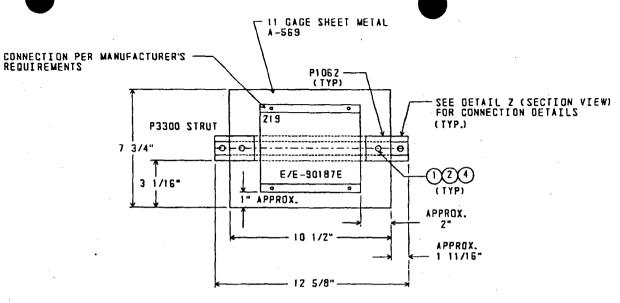
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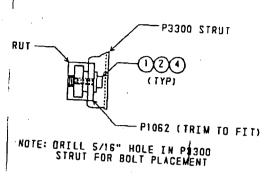
NOTES:

1. ALL CONNECTIONS USING UNISTRUT BOLTS SHALL BE TORQUED TO THE FOLLOWING VALUES UNLESS OTHERWISE NOTED.

1/4" BOLT 6 FT-LB. 1/2" BOLT 50 FT-LB.

I TEM NUMBER	PART DISCRIPTION	CAT. NUMBER
①	HEX HEAD BOLT 1/4"X 1 1/2" (CUT TO 1" LENGTH)	HHCS025150EC
2	1/4" WASHER	HFL WO2SEG
3	FLAT PLATE	P1062
①	1/4" SPRING NUT	P4006-1420
(§)	HEX HEAD BOLT 1/4"X 3/4"	HHC\$025075EG
6	1/4" NUT	HHXN025EG

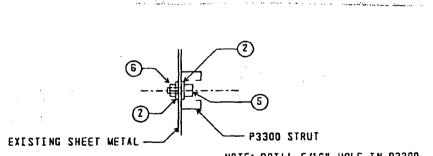




SECTION R-R

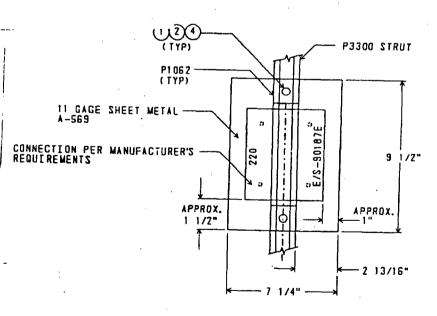
NOTE: DRILL S/16" HOLE IN P3300 STRUT FOR BOLT PLACEMENT





NOTE: DRILL 5/16" HOLE IN P3300 STRUT FOR BOLT PLACEMENT

DETAIL 2



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DETAIL 4

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	19597	NM (C)	PS-049	<u></u>	N/A					
1	4.0	References								
2		man for actions	Total value of Gancal		00053 2(3)0	20.43				
3	1.	TSD for Seismic Evaluation of Consoles 2(3)0C05A, 2(3)0C04A								
4		and 2(3)0C03 for MOD 955, Peach Bottom Atomic Power								
5		Station, Units 2 & 3. TSD no. P-0048.								
6	_	PECO drawing 6280-W-1-FE-124/1\ row 58 console 05%								
7	2.	PECo drawing 6280-M-1-EE-124(1), rev. 58. console 05A								
8		PECo drawing 6280-M-1-EE-67, rev. 29. console 04A								
9	٥.	PECO GLAWING 0200-M-1-EE-0/, 16v. 27. CONSOLE 04A								
10		DECO drawing 6280-W-1-FE-100 console 03								
11	4.	PECo drawing 6280-M-1-EE-100, console 03								
12	5	ASME Boiler and Pressure Vessel, Division 1, Section III,								
14		Appendix I, 1980								
15		Appendix 1/ 1/0								
16	6	ATSC-Manual of	Steel Construction, 8	Sth ed.	. 1980					
17		11200 11411041 02	,							
18	7.	Unistrut Genera	l Engineering Catalog	No. 9	. 1981	•				
19										
20	8.	Final Safety An	alysis Report. Vol. 3	l, Peacl	n Bottom Atom	nic				
21		Power Station,	_							
22	•									
23	9.	Seismic Qualifi	cation of Class 1 Ele	ectrica	l Equipment,					
24		NEDO-10678,72NE	D81, Class 1, Nov. 19	972, GI	Ξ					
25										
26	10	General Project Requirements for Seismic Design and								
27		Analysis of Equipment and Equipment Supports for Peach								
28		Bottom Atomic Power Station, Units 2 & 3, PECo Spec.								
29		11187-G-14.								
30		•								
31	11		ress and Strain, Roam	rk and ?	Young, 5th ed	i.				
32		McGraw-Hill.								
33			· · · · ·	_		_				
34	12		Handbook for Mechani	ical Eng	gineers, 8th	ed.				
35		McGraw-Hill.								
36	13.	Bechtel Corp let	ter to Philadelphia Elect	ric Comp	any, E.A.Patel	to				
37		1.E.Shannon, "Co	ntol Panel Anchorage Modi	lfication	2376, "Septemb	er				
38		10, 1300, BLP 24	646, Document Control No.	. 024749						
39	14.	General Electric	Company Drawing No. 73	NEKOK E	110 6000 4	T				
40		EE-124-(2)-rev-5	9: annotated by PECo in th	Tee verei	ins to highligh	1-				
41		a. All Q-list	items per QLAR #115	1679	ra-m co urkuriku					
42		b. Equipment	types and locations		•					
43		c. Panel know	n modifications							
44	. 15	Comerci 73		_						
45	15.			Reeval	uation Class	1E				
48		Equipment, Rev. 4	1, 4/11/86.							

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5.0 Conclusions

Based on the analysis and evaluation presented in this calculation, the following conclusions can be made:

- 1. The mounting supports for the installation of new devices identified in MOD 955 are atructurally adequate to withstand earthquake loads.
- 2. The modifications to the consoles have been designed and proven to have no impact on the structural integrity of the consoles and the existing device qualifications.
- 3. Console 05A is concluded to have no unacceptable II/I interaction.
- 4. Seismic capability limits of existing Q devices on console 05A are tabulated in Section 9.0.
- 5. The support designs for the installation of new devices identified in MOD 955 are acceptable for units 2 and 3 consoles. However, for unit 2 console modifications, constructibility shall be reviewed to ensure that sufficient spaces are available at the identified locations for the installation of the new devices.

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6.0 Qualification of MOD 955 Supports/Braces

Support designs for MOD 955 proposed by PECo as stated in the previous section are shown on the attached sketches(section 3.0) in the order listed below:

Console 05A

- 1. 13 in. RWM CRT
- 2. Panel switch
- 3. RWM cable connector
- 4. Digital displays and associated electronics
- 5. Power supplies

Console 04A

- 1. Digital display
- 2. Electronics
- 3. Power supply

Console 03

Same as 04A

6.1 Method

In this section, conceptual designs for MOD 955 supports provided by PECo will be analyzed to the requirements of AISC (ref. 6) to ensure that they are structurally adequate to withstand seismic loads. In addition, these supports will also be verified to ensure that they are sufficiently rigid so that local modifications will not affect the nearby existing device qualifications.

To this end, the frequencies of the local frames or mounting plates will be verified to show that they are in the rigid range of seismic response spectra.

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Seismic Loads

Control room consoles are located at floor elevation 165 ft. in the Radwaste Building. For this analysis response spectra at elev. 165 ft. for maximum credible earthquake are used with a 2 % damping value for welded steel assemblies (ref. 8).

The appropriate acceleration values to be used depend on the frequency of the console to which the devices are mounted. The consoles are supplied by GE and are qualified with generic method as stated in reference 9. Based on this reference and the past experience with other operating plants, the consoles are most likely in the rigid range. However, for the evaluation of these modifications peak accelerations are used, since specific information regarding these consoles is not available.

Horizontal A = $1.32 \times 2.4 \times (1.5) = 4.75 g$ (ref. 8,10)

Vertical Av = (2/3) A = 3.2 g

Supports are analyzed for seismic loads in N-S and vertical directions, and E-W and vertical directions. For each case, responses due to the horizontal and the vertical directions are added absolutely.

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6.2 Analysis

6.2.1 Qualification of support for 13 in. RWM CRT(see sketches on pages 12 to 22)

The configuration for CRT support is shown on the attached sketches. The CRT is enclosed in a can (box) which is mounted at the front panel and supported at the back by a support frame. For qualification of this support, the following three parts of analyses will be performed:

Frequency evaluation
Support frame qualification
CRT can and tray mounting qualification

Frequency evaluation

a. Weight of CRT and mounting devices

Tray: 12 gage sheet metal t = .1046 in. (ref. 6)

13 x 14 = 182 sq in.
2((1/2)(10 + 3) x 13) =
$$\frac{169}{351}$$
 x.1046 x.283 = 10.4 lb

Tray flange: 10 gage sheet metal t = .1345 in.(ref. 6)

$$2[1/2(18 - 13.75) \times 12.5] = 53$$

 14×1 = $\frac{14}{67} \times .1345 \times .283 = 2.6 \text{ lb}$

Can: 12 gage

$$2 \times 16.75 \times (12.5 + 17.125) = 992.4$$

 $\times .1046 \times .283 = 29.4 \text{ lb}$

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Collar flange: 10 gage

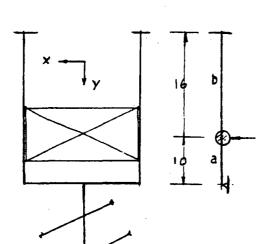
12.5 x 1.435 x2 = 36
16 x 1 =
$$\frac{16}{52}$$
 x.1345 x.283 = 2.0

Total 45.6 lb Weight of CRT 35 lb Miscelleneous 9.4 lb

Total 90 lb

b. Frequency of supporting frame in X- direction

Consider that the total weight is equally distributed to the front panel and the unistrut frame.



$$W = 90/2 = 45 \text{ lb}$$

$$f = 1/(2 \pi) \times sqrt[g/\Delta]$$
 (ref. 11)

$$\Delta = Wa^2b^3 (3L+a)/(12EIL^3)$$
 (ref. 6)
= .000733 in.

where

$$I = .181$$
 for P3301 (ref. 7)

$$W = 45/2 = 22.5 lb$$

$$L = 26 in.$$

$$b = 16 in.$$

$$g = 386.4$$
 in/sq sec.

$$f = 115 Hz.$$

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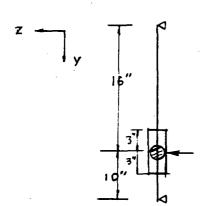
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c. Frequency of supporting frame in Z-direction (front-to-back)

The CRT is supported by the panel at the front and by the unistrut frame at the back. In the front-to-back direction, the front panel is braced with a box made of sheet metal, which is then connected to the frame. Thus, the front panel and the frame support unite together in resisting the motion in the Z-direction. For frequency estimation, however, the rigidity of the front panel will be totally neglected and the total weight will be conservatively attributed to the frame support and applied at a single point (not distributed +/- 3").



$$f = 1/(2\pi) \times sqrt (g/\Delta)$$

 $\Delta = Wa^2 b^2 / (3EIL) = .001732 in$

where

W = 45 lb

a = 10 in.

b = 16 in.

I = .294 in

L = 26 in.

f = 75 Hz.

Noted that the flxibility of the cross piece has been neglected by virtue of the other conservatisms.

d. Frequency of supporting frame in the vertical direction

Based on the support configuration, the support is much stiffer in the vertical direction than in the horizontal directions, since the frame is stiff in the vertical direction, and is connected to an existing unistrut which is welded to the top panel. Thus, the frequency in the vertical direction is higher than that in the horizntal directions.

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Support Frame Qualification

a. Support loads

The vertical bench board and support frame are slanted 20 degrees from the vertical. Therefore, support loads in x, y and z directions will be converted into the directions in line with the supporting directions.

Dead weight W = 90 lb

 $F1 = W \sin 20 = 31 lb$

 $F2 = W \cos 20 = 85 lb$

Seismic loads

y-direction

 $Fy = 90 \times 3.2 = 288 \text{ lb}$

F1 = 99 1b

F2 = 271 lb

z-direction

 $Fz = 90 \times 4.75 = 427$

 $F1 = 427 \times \cos 20 = 401 \text{ lb}$

 $F2 = 427 \times \sin 20 = 146 \text{ lb}$

x-direction

 $Fx = 90 \times 4.75 = 427 \text{ lb}$

F3 = 427 1b

Dead wt.+seismic load(x + y) Dead wt.+seismic load(y + z)

F1 = 31+99 = 130 lb

F2 = 85 + 271 = 356

F3 = 427

F1 = 31+99+401 = 531 lb

F2 = 85+271+146 = 502

F3 = 0

2

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b. Frame analysis due to dead wt. + seismic load(x +y)

The loads are considered equally distributed to the front panel and the support frame and then equally distributed to each of the two vertical members.

$$F1 = 130/4 = 32.5 lb$$

F2 = 356/4 = 89

F3 = 427/4 = 107

Vertical member -- P3301 (ref. 7)

A = .794 sq in.

z1 = .207 cu in.

 $z_3 = .362$ cu in.

 $R1 = F1 \times 16/26 = 20 \text{ lb}$

R2 = F2/2 = 44.5

 $R3 = F3xb^2/(2L^3)x (a + 2L)$

= 48

 $M3 = R1 \times 10 = 200 \text{ in-lb}$

 $M1 = R3 \times 10 = 480$

R2/A = 56 psi

M3/Z3 = 552

M1/Z1 = 2319

Total = 2927 psi

Horizontal member--P3300 (ref. 7)

A = .397 sq in.

z1 = .078 cu in.

 $z_2 = .181$ cu in.

 $M1 = R2 \times 8.6 = 382.7 in-lb$

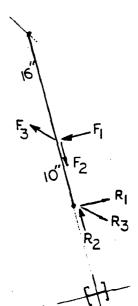
 $M2 = R1 \times 8.6 = 172$

R3/A = 121 psi

M1/Z1 = 4909

M2/Z2 = 950

Total = 5980 psi





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Brace member -- P3300

A = .397 sq in.

z1 = .078 cu in.

 $z_2 = .181$ cu in.

 $P1 = 2(R1 \sin 25 + R2 \cos 25)$

= 98 lb

 $P3 = 2(R1 \cos 25 + R2 \sin 25)$

= 74 lb

 $M2 = P1 \times 6 = 588 \text{ in-lb}$

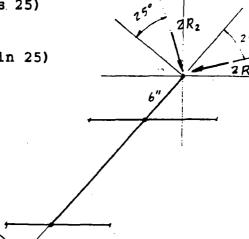
 $M1 = 2R3 \times 6 = 576$

P3/A = 186 psi

M2/Z2 = 3249

M1/Z1 = 7385

Total = 10820 psi



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c. Frame analysis due to dead wt. + seismic load(y +z)

The loads are considered equally distrbuted to the front panel and the support frame and then equally distributed to each of the two vertical members.

$$F1 = 531/4 = 133 lb$$

$$F2 = 502/4 = 126$$

F3 = 0

Vertical member -- P3301 (ref. 7)

$$A = .794 \text{ sq in.}$$

$$z1 = .207$$
 cu in.

$$z_3 = .362 cu in.$$

$$R1 = F1 \times 16/26 = 82 \text{ lb}$$

$$R2 = F2/2 = 63$$

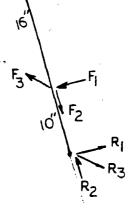
R3 = 0

$$M3 = R1 \times 10 = 820 \text{ in-1b}$$

$$R2/A = 79$$
 psi

$$M3/Z3 = 2265$$

Total = 2344 psi





Horizontal member--P3300 (ref. 7)

$$A = .397 \text{ sq in.}$$

$$z1 = .078$$
 cu in.

 $z_2 = .181$ cu in.

 $M1 = R2 \times 8.6 = 542 in-lb$

$$M1 = R2 \times 8.6 = 542 \text{ in-1}.$$

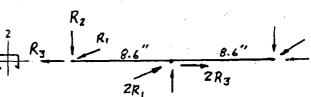
 $M2 = R1 \times 8.6 = 705$

R3/A = 0 psi

$$M1/Z1 = 6949$$

$$M2/Z2 = 3895$$

Total = 10844 psi



2R,

CALCULATION SHEET

5010.65

JOB NUMBER

CALCULATION IDENTIFICATION NUMBER

DIVISION & GROUP | CALCULATION NO. | OPTIONAL TASK CODE | PAGE |
NM (C) | PS-049 | N/A | 45

Brace member--P3300

A = .397 sq in.

 $z_1 = .078$ cu in.

Z2 = .181 cu in.

 $P1 = 2(R1 \sin 25 + R2 \cos 25)$

= 184 lb

 $P3 = 2(R1 \cos 25 + R2 \sin 25)$

= 202 1b

 $M2 = P1 \times 6 = 1104 \text{ in-lb}$

P3/A = 509 psiM2/Z2 = 6099

Total = 6608 psi

d. Unistrut Connections

The above analyses show that unistrut members are subjected to a maximum applied load of less than 150 lbs. The design load data for typical unistrut channel connections as shown in reference 7 are 1000 lbs or larger. Therefore, the connection parts used in the support designs are adequate.

		CALCULATION SH	EET		1
5010.85	CALCULAT	ION IDENTIFICATION NUMBER			<u>'</u>
JOB NUMBER	DIVISION & GROUP	CALCULATION NO.		OPTIONAL TASK CODE	PAGE
19597	NM(C)	PS-049		N/A	46
		·			
•				•	
CRT can	and tray qual	ification			
mbo C	DM is mounted	to the bottom sh	ant motal o	of the train h	
		tray is connected			
		ray. The tracks		·	
		t the front panel			
		etailed configura			4 01
_	hed sketches.	_			
a. CR	T mounting		·		
	•				
We	ight of CRT =	35 lb			
_					
De	ad wt + seism	ic load(y+z)	Dead wt +	seismic load	d (x+y
•	F1 = 206		* F1 = 5	0	
-	F2 = 195		F2 = 1		
	12 - 155	•	F3 = 1		
CR	T mounting sc	rews: 4- 1/4" dia	a. screws		
•				* These loa	ds
	A = .0362 sq	in. (ref. 12)		derived u	sing
				same	
		ss = F2/(4A) = 13	-	methodolo	
	Shear stress	= (F1 + F3)/(4A)	= 1492 psi		the
Ch.	eet metal bea	ring atrace		previous	
511	eet metal bea	ring stress		section.	
	12 gage, t	= 1046 in			
	12 gage, c	.1040 111.			
·	Bearing stre	ss = (F1 + F3)/(4	ltd) = 2065	psi	
	.				
b. Tr	ay mounting			•	
			•		
	-	= 10.4 + 2.6 = 13			
	ight of CRT =		5	•	
To	tal	4	8 lb	•	
D -		da lood/e	Dand		 /
ре	ad wt + seism	ic load(y+z)	nead wt +	seismic loa	л (х+й
	F1 = 283		* F1 = 6	Q	
	F2 = 268		F2 = 1	-	
•	FZ - 200		FZ - 1	. 3 U	

F3 = 228

JOB NUMBER	1	-DIVISION & GROUP	ON IDENTIFICATION NUMBER CALCULATION NO.		OPTIONAL TASK CODE	PAGE
					1	47
19597		NM (C)	PS-049		N/A	7,
	by For bet	the tray fla ce F3 does n ween the tra	isted by the tra- nge which bears ot induce any lo- y and the tracks st the tracks.	against the ad to the co	front panel.	ews
•	Tra	y mounting s	crews: 6-#5 scre	ws, A = .008	2 sq in.(ref.	12)
		shear stress	= F2/(6A) = 544	7 psi	•	
	Tra	ck mounting	screws: 8-1/4"-2	8 screws,	A = .0362 sq	in.
		shear stress	= (F2+F3)/(8A)	= 1443 psi		
	Tra	y flange: 10	gage, t = .1345	in.		
		_	H = 13 3/4" ng H = 15 1/8" a = (15 1/8 - 1	3 3/4)/2 =	.6875 in.	•
-		$Z = 1/6 \text{ (bt}^2$ M/Z = 3243 p	2 = 97 in-lb 2) = 1/6 x [(10) si et weld on two v			.e
c.	Can	mounting				
	•	Dead wt + se	ismic load(y+z)	Dead wt	seismic loa	d(x+
		F1 = 531 F2 = 502	1b	F1 = 3 F2 = 3 F3 = 4		
	Con	nection betw	een can and unis	trut P3301		
		six 1/2" bol	ts: bolt strengt	h is more t	han adequate.	٠.
	ļ	Sheet metal	bearing stress =	[(F1+F2)/2]/(6td) = 164	l4 ps
	Con	nection betw	een can, console	pl. and co	llar flange	•
		Twelve 1/4"	studs: bolt st	rength is π	nore than ade	quat
		Sheet metal	bearing stress =	[(F2+F3)/2]/(12td) = 12	248 t

adequate.

19597	CALCULATION & GROUP NM (C)	ON IDENTIFICATION NUMBER CALCULATION NO.		OPTIONAL TASK CODE	PAGE
			*	OPTIONAL TASK CODE	PAGE
19391		PS-049		N/A	48
	, AA (C)	15 049		<u> </u>	
6.2.2	Qualification o	f Panel Switch Supp	ort		
	sheet metal of	mounted to two bra			ttom)
	sketches. The	switch weighs 5 lb.			
•	Frequency Evalu	ation			
	Moment of in	ertia of the bracke	ts		
	I = 1/12 (2)	$(1/4)^3 = .0026$			
	f = 1.732/(2	π) x sqrt[EIg/(WL	^3)] = 4:	13 Hz (ref.	11)
	where	•			
	L = 2.75/ W = 5 lb	2 = 1.375 in.			
	Panel Switch Mor	unting			
	Dead + seism	ic load(y+z) Dead	l + seisπ	ic load(x+y)	
	F1 = 30 1	b 1	F1 = 7 lb)	
	F2 = 28		F2 = 20	,	
		1	F3 = 24		
	Brackets				
	A = 1 x .	25 = .25 sq in			
		/4)^2 /6 = .0104 cu	in.		
		2.75/2 = 41.3 in-lb			
	F2/(2A) =	56 psi			
	M/(2Z) = 1	1986			
	Total	2042 psi		·	
	Sheet metal				
	Z = (1) (.	1046)^2/6 =.00182			
	W//27\ - *	11346 psi			

CALCULATION SHEET

	CALCULAT	ION IDENTIFICATION NUMBER		
JOB NUMBER	-DIVISION & GROUP	CALCULATION NO.	OPTIONAL TASK CODE	PAGE
19597	NM (C)	PS-049	N/A	49

6.2.3 Qualification of Support for Digital Displays with Associated Electronics (Console 05A)

This design provides support for two digital displays and two electronics boxes. The digital displays are mounted on a cover plate and the electronics are mounted on a tray which is welded to the cover plate as shown in the attached sketches.

Weight of tray

39

 cover plate: 10 gage

6 7/8 x9 1/8 = 63 sq in. $x \cdot 1345 \times 283 = 2.4 \text{ lb}$

sides $2(4 \times 9 \ 3/8) = 75$ bottom $6.75 \times 9 \ 3/8 = 63$ $138 \times .1046 \times .283 = 4.1$

top tray $2(1.5 \times 7) = 21$ 6.75 x 7 = $\frac{47}{68}$ x .1046 x.283 = 2 lb

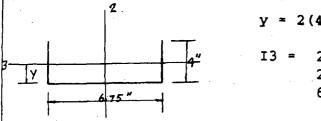
subtotal 9 lb

Digital displays
2 x 1 = 2

Electronics
2 x 2 = 4

Total 15 lb

Frequency Evaluation



$$y = 2(4 \times 2)/(8 + 6.75) = 1.08 in.$$

I3 =
$$2[(4)^3 / 12]$$
 = 10.67
 $2(4)(.92)^2$ = 6.77
 $6.75 \times (1.08)^2$ = 7.87
 $25.31 \times .1046 = 2.65$

CALCULATION SHEET

NM(C) PS-049 N/A			ON IDENTIFICATION NUMBER		1
W = 15 - 2 - 2.4 = 10.6 lb f = 1.732/(2 \pi) x sqrt[EIg/(W = 546 Hz) Tray Mounting Dead wt + seismic load(y+z) Dead wt + seismic load(F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/(2(4 7/8)) = 53 lb V = 84/4 Due to F1 T = 89/4 P = 22 Due to F1 T = 89/4 P = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/(2(7.125)) = 31 V = 71/4 P = 18 Total T = 53 + 22 + 31 P = 106 lb V = 21 + 18 P = 39		1			PAGE
f = 1.732/(2π) x sqrt[EIg/(W = 546 Hz Tray Mounting Dead wt + seismic load(y+z) Dead wt + seismic load(F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4	19597	NM (C)	PS-049	N/A	
f = 1.732/(2π) x sqrt[EIg/(W = 546 Hz Tray Mounting Dead wt + seismic load(y+z) Dead wt + seismic load(F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4	*				
f = 1.732/(2π) x sqrt[EIg/(W = 546 Hz Tray Mounting Dead wt + seismic load(y+z) Dead wt + seismic load(F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4			W = 15	- 2 - 2 4 - 10 6 lb	•
Tray Mounting Dead wt + seismic load(y+z) Dead wt + seismic load(F1 = 89 lb F2 = 84 F2 = 59 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 Due to F1 Due to F1 T = 89/4 T = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 Total T = 53 + 22 + 31 = 39	-			- 2 - 2.4 - 10.6 1B	
Tray Mounting Dead wt + seismic load(y+z) F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] V = 84/4 Due to F1 Due to F1 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] V = 71/4 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total		•	f = 1.7	$732/(2\pi)$ x sart[EIa/	(WI.^
Tray Mounting Dead wt + seismic load(y+z) Dead wt + seismic load(F1 = 89 lb F2 = 84 F3 = 0 F3 = 71 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/(2(4 7/8)) = 53 lb V = 84/4 Due to F1 Due to F1 T = 89/4 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 Total	.	∮F ₂	1		
Tray Mounting Dead wt + seismic load(y+z) Dead wt + seismic load(F1 = 89 lb F2 = 84 F2 = 59 F3 = 0 F3 = 71 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39					
Dead wt + seismic load(y+z) F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] V = 84/4 Due to F1 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] V = 71/4 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total	-	6.2"			
Dead wt + seismic load(y+z) F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] V = 84/4 Due to F1 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] V = 71/4 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total T = 53 + 22 + 31 Total	•	. 1			
F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39	Tra	y Mounting			
F1 = 89 lb F2 = 84 F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39					
F2 = 84 F3 = 0 F3 = 71 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39		Dead wt + se	ismic load(y+z) I	Dead wt + seismic loa	+x) b
F2 = 84 F3 = 0 F3 = 71 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39		m4 00 31	L	m4 00 11	
F3 = 0 Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39			D.	i	
Envelope load F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39					
F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39		F3 - 0		£3 - /T	
F1 = 89 lb F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39		Envelope los	đ	•	
F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 Due to F1 T = 89/4 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 Total		-midlope roa	_		
F2 = 84 F3 = 71 Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 Due to F1 T = 89/4 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 Total		F1 = 89 1	р		
Mounting studs: four 1/4"-20 studs A = .0317 sq in. Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39				·	
Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39		F3 = 71	•		
Due to F2 M3 = 84 x 6.2 = 521 in-lb T = M3/[2(4 7/8)] = 53 lb V = 84/4 = 21 Due to F1 T = 89/4 = 22 Due to F3 M2 = 71 x 6.2 = 440 in-lb T = M2/[2(7.125)] = 31 V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39					
$T = \frac{M3}{2(4 7/8)} = 53 \text{ lb}$ $V = \frac{84}{4} = 21$ Due to F1 $T = \frac{89}{4} = 22$ Due to F3 $\frac{M2}{T} = \frac{71 \times 6.2}{2(7.125)} = \frac{31}{31}$ $V = \frac{71}{4} = 18$ Total $T = \frac{53}{2} + 22 + 31 = \frac{106}{3} \text{ lb}$ $V = 21 + 18 = 39$		Mounting stu	ds: four 1/4"-20 st	uds $A = .0317$ sq in	•
$T = \frac{M3}{2(4 7/8)} = 53 \text{ lb}$ $V = \frac{84}{4} = 21$ Due to F1 $T = \frac{89}{4} = 22$ Due to F3 $\frac{M2}{T} = \frac{71 \times 6.2}{2(7.125)} = \frac{31}{31}$ $V = \frac{71}{4} = 18$ Total $T = \frac{53}{2} + 22 + 31 = \frac{106}{3} \text{ lb}$ $V = 21 + 18 = 39$					
$V = 84/4 = 21$ Due to F1 $T = 89/4 = 22$ Due to F3 $M2 = 71 \times 6.2 = 440 \text{ in-lb}$ $T = M2/[2(7.125)] = 31$ $V = 71/4 = 18$ Total $T = 53 + 22 + 31 = 106 \text{ lb}$ $V = 21 + 18 = 39$		Due to F2			
Due to F1 $T = 89/4 = 22$ Due to F3 $M2 = 71 \times 6.2 = 440 \text{ in-lb}$ $T = M2/[2(7.125)] = 31$ $V = 71/4 = 18$ Total $T = 53 + 22 + 31 = 106 \text{ lb}$ $V = 21 + 18 = 39$					
Due to F3 $M2 = 71 \times 6.2 = 440 \text{ in-lb}$ $T = M2/[2(7.125)] = 31$ $V = 71/4 = 18$ Total $T = 53 + 22 + 31 = 106 \text{ lb}$ $V = 21 + 18 = 39$	·		v = 84/4	= 21	
Due to F3 $M2 = 71 \times 6.2 = 440 \text{ in-lb}$ $T = M2/[2(7.125)] = 31$ $V = 71/4 = 18$ Total $T = 53 + 22 + 31 = 106 \text{ lb}$ $V = 21 + 18 = 39$		Due to E1	m = Ω0/A	* 22	
T = M2/[2(7.125)] = 31 $V = 71/4 = 18$ $T = 53 + 22 + 31 = 106 lb$ $V = 21 + 18 = 39$	•	Due, to FI	1 - 03/1	- 44	
T = M2/[2(7.125)] = 31 $V = 71/4 = 18$ $T = 53 + 22 + 31 = 106 lb$ $V = 21 + 18 = 39$		Due to F3	$M2 = 71 \times 6.2$	2 = 440 in-lb	
V = 71/4 = 18 Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			
Total T = 53 + 22 + 31 = 106 lb V = 21 + 18 = 39					
V = 21 + 18 = 39			·		
	• •	Total		- 31 = 106 lb	
		<i>:</i>	V = 21 + 18	- 39	
────────────────────────────────────					
Tensile stress = T/A = 3344 psi					
Shear stress = V/A = 1230			Shear stress	= V/A = 1230	

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STONE & WEBSTER ENGINEERING CORPORATION

CALCULATION S	;}	1EE	T
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CALCULATION IDENTIFICATION NUMBER

10.65	•	

JOB NUMBER DIVISION & GROUP CALCULATION NO. OPTIONAL TASK CODE PAGE 19597 NM (C) PS-049 N/A

2 3

5 6

7

8

10

11

12 13

14 15 Weld

A = 8

22 = 4x6.75 = 27

 $z_3 = 16/3 = 5.33$

 $f = sqrt[(F1/A+M2/Z2+M3/Z3)^2 + (F2/A)^2]$ $+ (F3/A)^2$ = 126 lb per in

w = f/(.707x18000) = .01 in.1/8" filet weld is adequate.

6.2.4 Support for RWM Cable Connectors

Two cable connectors have the same weight as a power supply. This support is acceptable by comparison of the support configuration with the support for the power supply on console 04A, which is qualified in the next section.

6.2.5 Support for Power Supplies (Console 05A) Supports for Electronics Boxes and Power supplies (Consoles 04A and 03)

Power supplies on console 05A, and electronics boxes and power supplies on consoles 04A & 03 are mounted separately from the associated digital displays. These supports are similar in configuration. Review of the individual device weights and their mounting configurations as shown in the sketches concludes that the support for power supply on console 04A is the governing case which envelops the remaining cases.

Frequency Evaluation

Mounting plate: 12 gage

 $I = (1/12)(9)(.1046)^3 = .000858$

Weight

9x6.63x.1046x.283 = 1.8 1bplate power supply 5.8 lb

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1		

 $f = 1.732/(2 \pi) \times \text{sqrt}[Eig/(WL^3)]$ L = 6.63/2 = 3.32 in.

= 59 Hz

Plate stress

M = (5.8x4.75)x3.32 = 91.5 in-lbMoment due to eccentricity (1") of the power supply = 12.8 $= (4 \times 3.2)(1)$ Total = 104.3

F = 5.8x3.2= 18.6 lbZ = I/(t/2) = .0164A = 9x.1046 = .941 sq inM/Z= 6360 psi F/A 6380 psi

Note: The mounting plate has been revised from 12 gage to 11 gage as shown on the drawing. This change is on the conservative side, and therefore, is acceptable.

6.2.6 Comparison of Calculated Stresses with Allowable Stresses

The calculated stresses shown above for sheet metals, screws and bolts are much lower than the allowable stresses given in section 3.0. Therefore, these device mounting supports are acceptable.

CALCULATION SHEET

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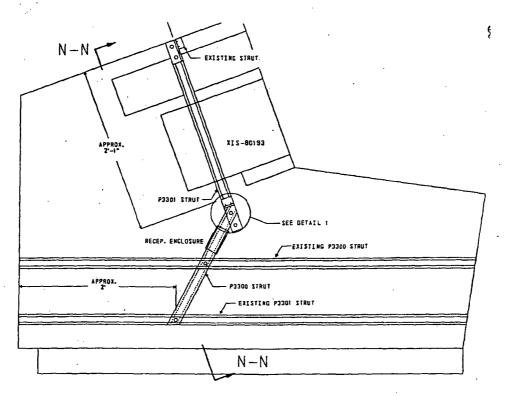
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6.2.7 Deviation of Support Design for 13 in. RWM CRT for unit 2 Console 20C05A

The RWM CRT support for unit 2 console deviates slightly from the same support for unit 3 console. This deviation includes the orientation of the brace member to the support frame (see attached support sketches), ie., (1) the connection of the brace with the existing horizontal P3301 strut is approximately 2 ft. (instead of 1 ft. 4 in. for unit 3) from the back vertical panel, (2) the cross-sectional orientation of the brace is 180 degrees opposite to the brace for unit 3 console. This deviatin is necessary because of the space availability of unit 2 console.

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Review of the CRT support qualification for unit 3 console concludes that a sufficient design margin exists to compensate for the negligible effect caused by the minor deviation described above. Therefore, the CRT support design for unit 2 console is acceptable.

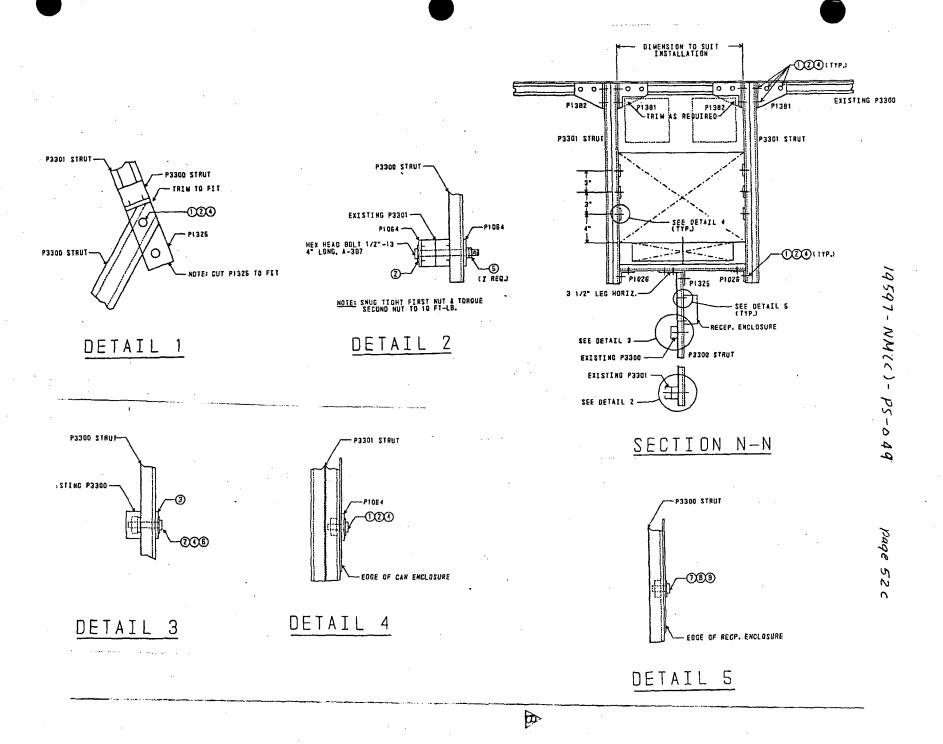


SECTION M-M

I TEM NUMBER	PART DISCRIPTION	CAT. NUMBER
0	HEX HEAD BOLT 1/2"X 1 3/16"	HHC2020119EG
2	1/2" WASHER	HFL WOSDEG
3	FLAT PLATE	P1064
•	1/2" SPRING NUT	P4010
⑤	1/2" NUT	HHXNOSOEG
©	HEX HEAD BOLT 1/2"X 1 3/4"	HHCSQSQ17SEG
0	HEX HEAD BOLT 1/4"X 3/4"	HHCS025075EG
8	1/4" SPRING HUT	P4006-1420
9	1/4" WASHER	HFL WOZSEG

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N/A

6.2.8 Qualification of support for 6"x6" tracor westronics multipens recorder

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This modification involves replacement of the existing 6"x6" computer trend recorder with 6"x6" tracor westronics multipens recorder. existing recorder weighs approximately 40 lbs, while the new recorder weighs only 20 lbs. new recorder is mounted at the front panel and supported at the back by a support frame as shown on the attached sketches. Thus, the new recorder is better supported than the old one before the modification. Therefore, this modification will not affect the adjacent device qualification.

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Judging from the sizes of the structural members and bolts, the support frame is structurally adequate to support the new recorder, which weighs only 20 lbs.

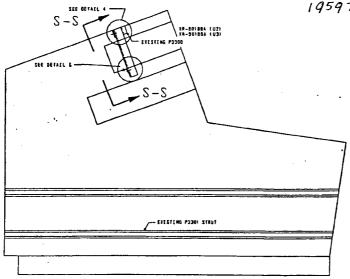
6.2.9 Qualification of support for 6" x 6" computer trend recorder

> This modification involves the replacement of the existing RWM operator interface with a non-Q 6"x6" computer trend recorder. The new recorder weighs 20 lbs. and it is judged that the existing RWM weighs more than 20 lbs. since it is much larger in size. The cut out in the panel skin due to the RWM will be covered by an 11 gauge plate with mounting bolts/holes similar to the RWM installation. smaller cut out in this 11 gauge plate will be made to accommodate the installation of the new trend The new recorder is mounted at the front recorder. of the panel and supported at the back by a support frame as shown on the sketches on pages 52f and 52g. Thus, the new recorder is better supported then the old one prior to this modification. Therefore, the installation of this computer trend recorder will not affect the adjacent device qualification.

Judging from the sizes of the structural members and bolts, the support frame is structurally adequate to support the new trend recorder, which weighs only 20 lbs.

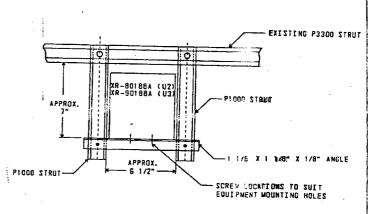


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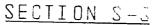


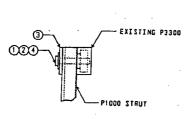
SECTION P-P

ITEM PART DISCRIPTION		CAT. NUMBER
0	HEX HEAD BOLT 1/2"X 2 1/2"	HHCS050250EG
0	1/2" WASHER	HFL WOSGEG
3	FLAT PLATE	P1064
•	1/2" SPRING NUT	P4010
⑤	HEX HEAD BOLT 1/4"X 1 1/2"	HHCZ025150EG
⑤	1/4" WASHER	HFL WOZSEG
0	1/4" SPRING NUT	P4005-1420
8	1/4" HUT	HHXNOZSEG

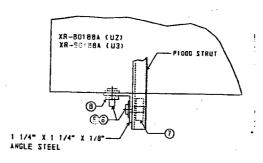


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DETAIL 4



DETAIL 5



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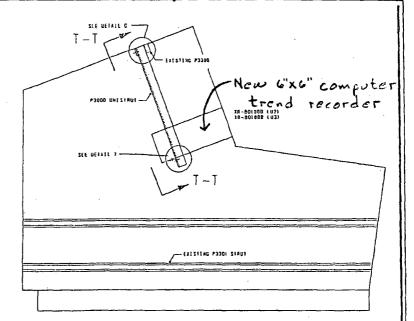
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ORIGINATOR T.M. C. PLANE 12-4-90
REVIEWER APPLICATE DATE 12-4-90

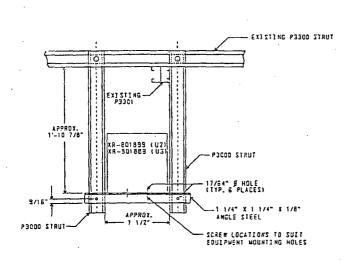
20C05A & 30C05A

I TEM Number	PART DISCREPTION	
0	HEX HEAD BOLT 1/2"X 1 1/2" A-307	*
0	1/2" SPRING HUT	
3	FLAT PLATE PIOS4	1
0	MEX HEAD BOLT 1/4"X 1"	*
©	1/4" SPRING NUT	
⑥ .	1/4" NUT	

^{*} BOLT LENGTH MAY DE TRIMED AS REC'D, PROVIDED FULL NUT ENGAGEMENT IS MAINTAINED.

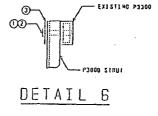


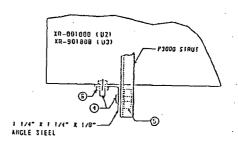
SECTION Q-Q



NOTE: UNLESS OTHERWISE NOTED, ALL OIMENSIONS SHALL HAVE A TOLERANCE OF ± 1/15"

SECTION I-I





DETAIL 7

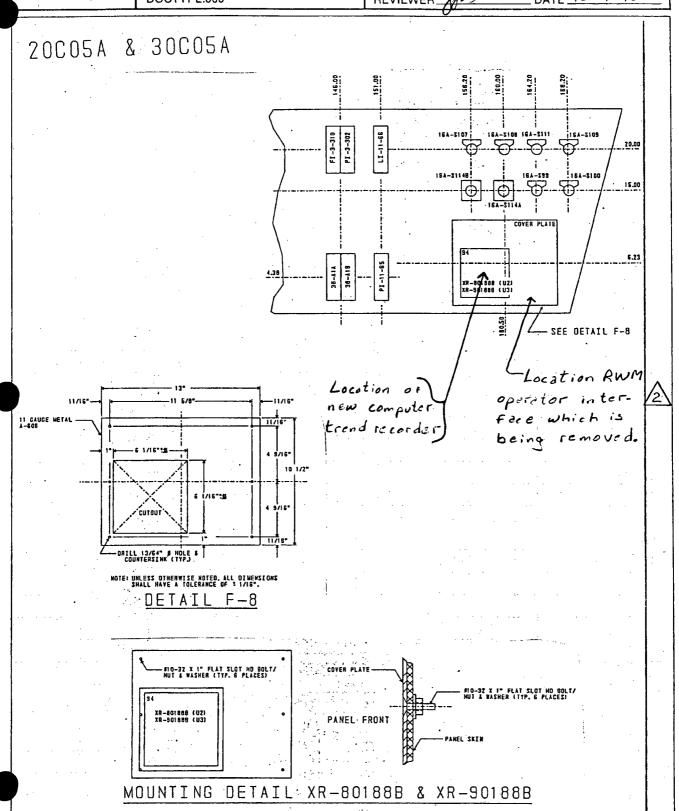


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REVIEWER SAWhater DATE 12-4-90



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7.0 Assessment of the Impact of the Modification on the Existing Q Console and Q Devices

MOD 955 involves replacement of the existing computer trend Gemax recorder with a new 13 inch RWM CRT, and installations of several other small devices such as, a panel switch, cable connectors, digital displays and associated electronics boxes and power supplies. The following sections assess the impact of these modications on the existing console and devices.

7.1 13 inch RWM CRT

The existing computer trend Gemax recorder weighs approximately 40 lbs, and its size is 6"x6 th 22 inches protruded into the back of the part. The replacement with the new CRT weighs 90 lbs(including the mounting tray and can), and it requires a cutout of 14" x 15" on the front panel to provide an operation of the panel, and heavier weight increases the mass to also supporting structure. This, in turn, might alter the local dynamic characteristics of the panel if proper measures are not taken.

To minimize such effect, a support frame is istalled at the back of the panel to share the load and thereby reduce the load at the front panel to approximately the same as was before the modification. The interpolation is further reinforced by the CRT support can the istalled to the support frame at the back. In addition, the support frame is braced to ensure that the frequencies in all three directions are in the rigid range of seismic response spectra, and that the support frame is structurally adequate to withstand seismic loads.

Thus, incorporation of the above design into the modification assures that the support structure is rigid, and that the front panel is properly reinforced and supports the same weight as it did before the modification. Therefore, the response of the new device due to seismic excitations will at least remain the

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same as or smaller than it was before the modification, and thus, the adjacent devices and the console will not be affected by this modification.

The closest Q device to this modification is a single GE CR 2940 switch located approximately 1 ft. from the cutout and near a panel edge which provides a stiff support for the switch. This and other Q devices in the upper panel of the console are all switches and push buttons with very high seismic capacities in any event.

The overall weight being added to the console is negligible and no change can be expected in the overall console response as well.

7.2 Other Small Devices

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Other modifications involve installation of small devices such as digital displays, panel switch, etc., which weigh from 1 lb to 5 lbs.

The mounting supports for these devices are made rigid to ensure that seismic responses are not amplified. In addition, the weights of the devices are negligibly small compared to the total weight of the console, therefore, the responses of these devices will be negligible and will not affect the integrity of the console and the adjacent devices.

7.3 6" x 6" Computer Trend Recorder

This new device is smaller and is judged to weigh less than the device it is replacing, the RWM operator interface. The new recorder also has a new rear support bracket further strengthen its mounting. Therefore, the installation of the computer trend recorder in place of the RWM operator interface will not impact the existing qualification of the Q devices in panel 05A.



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8.0 ASSESSMENT OF THE IMPACT OF THE FAILURE OF NON-Q DEVICES ON THE Q DEVICES AND THE CONSOLES AND RECOMMENDATIONS FOR ANY MODIFICATIONS IF REQUIRED (FOR O5A ONLY)

The installed console was seismically qualified with all devices in their original configuration. All original devices are therefore concluded to be acceptable from a II/I consideration. This includes confirmation of both structural integrity and the absence of unacceptable spatial interactions not involved with structural failure (breaking loose). Console modifications completed after installation have been assessed using information (1) provided by PECo and (2) obtained during a walkdown of the this specific console as well as other seismically qualified consoles and cabinets. Section 6.0 above confirms the integrity of Modification (MOD) 955. Console 05A is concluded to have no unacceptable II/I interactions.

8.1 DISCUSSION

The conclusion stated above is derived from the following:

- 8.1.1 The console and all devices installed at the time of plant installation were seismically qualified as a part of the original seismic qualification of the console. (The Task Scoping document, ref 1, states that "PECo has established that the existing consoles are Q commodities seismically qualified with all the devices attached.")
- 8.1.2 Console modifications performed subsequent to the installation of the seismically qualified configuration have been characterized as minor. This means either:
 - a. Minor changes involving inconsequential structural modifications (small holes being drilled or covered, or
 - Additions (or deletions) of inconsequential weight and/or size to the console, or
 - c. The exchange of like item for like item in which the new item is sufficiently similar to the replaced item (in size, weight, anchorage details, etc.) to be a basis to conclude that equivalent and satisfactory structural integrity exists.

The purpose of the walkdown is to confirm that such modifications are not significant for a II/I interaction assessment.

8.1.3 Reference 13 confirms basic console anchorage. No further assessment is required.

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8.1.4 The process of review of non-Q modifications to console 05A is as follows:

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- a. As a result of the walkdown of this console and its prior seismic qualification, it was judged that spatial interaction (rattling) of any of the non-Q modifications with Q-devices was not credible. Structural failure (and falling) would be the only necessary concern as a precursor to any unacceptable II/I interaction.
- b. With information from the walkdown, the non-Q modifications to the original configuration are confirmed as minor and, therefore, judged to be capable of remaining integral during an earthquake. Subsequent steps are also applied since it was not possible to precisely identify every modification. No II/I concern is concluded.
- c. If the non-Q modification is sufficiently distant from any Q device(s) to be of no concern (even if structural failure and associated falling were to take place), an acceptable condition could be concluded to exist. Because potential failure paths could include safety-related wiring or interior devices not apparent from either the walkdown or available drawings, this option was not used for II/I assessment of this console.
- d. If Q devices are inside the potential failure path of the non-Q modification (or if it is desirable to assure against possible structural failure) the non-Q modification must be assured against structural failure and any unacceptable spatial interaction by one of the following methods:
 - (1) Comparison with other devices qualified in this or other plant consoles combined with a judgement regarding panel location dependent response. Many added devices are identical (or sufficiently similar) to qualified devices. Structural integrity is easily inferred.
 - (2) If necessary and/or appropriate, utilize information from other plants (experience) to confirm structural integrity.
 - (3) Perform a case specific evaluation by either:
 calculation, or
 engineering judgement (insufficient weight, etc.).
 All indicating lights are judged to be of sufficient weight/strength so as to be incapable of structural failure due to earthquake loading.

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- 8.1.5 Modifications to the original installation were concluded to result in an acceptable console configuration. No potential II/I interactions were identified.
- 8.1.6 The modification (MOD) 955 outlined in Section 1.0 and the calculations presented in Section 6.0 confirm the structural integrity of the design and confirm an acceptable console configuration. No potential II/I interactions will result from this MOD.

8.2 DETAILED DISCUSSION

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 The upper (vertical) panel of the console contains the following Q-devices: (identification and location (X, Y) coordinates are according to Ref. 14, view F; locations of device clusters are approximate and provided for reference and information; see Figure 8-1 attached)

- 1. 2ea manual scram push buttons (84, 22)
- 2. 6ea GE CR2940 switches (160, 18)
- 3. 2ea GE CR115 switches (158, 15)
- 4. lea GE CR2940 switch (142.75, 2)

These devices are sufficiently isolated above or beside any identified modification to assure against any potential interaction. Structural failure of any non-Q device on the vertical panel will not occur and therefore will not create any unacceptable interaction with Q devices located below. Non-Q dial indicators (37ea G.E. 180), recorders (15ea 6x6 multipen), and local power indicators (16ea) are concluded to be structurally qualified because they are either (1) original equipment, or (2) similar to Q or non-Q devices installed in another Q commodity (e.g. ECCS panel or console 04A). Remaining non-Q devices (pushbutton switches, indicator lights, etc., were judged structurally qualified because they are either (1) original equipment, or (2) low weight, compact items with sufficient strength.

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The lower (horizontal) panel of the console contains the following Q-devices: (identification and location (X, Y) coordinates are according to Ref. 14, view E; locations of device clusters are approximate and provided for reference and information; see Figure 8-1 attached)

- 5. Sea GE CR2940 switches (159, 6) Unit 3; 4ea on Unit 2
- 6. lea SBLC pump switch (152, 15)
- 7. 2ea GE SBM switches (142, 14.84)
- 8. 3ea Neutron Monitoring System switches (129.75, 10)
- 9. 4ea IRM Power Range switch (110, 10)
- 10. 4ea IRM Power Range switch (58. 10)
- 11. lea GE SBM switch (96, 3.6)
- 12. lea Reactor Mode switch (70.75, 3.8)
- 13. 2ea GE SBM switches (24, 4.84)

These devices are concluded to be sufficiently isolated from any identified modification to assure against any potential interaction. Structural failure of any non-Q device on the horizontal panel will not occur and therefore will not create any unacceptable interaction with Q devices located below. Major non-Q devices include: the Manual/Auto HCS Stations (8ea 7-GEMAC, 1-L&N), the rod select matrix, switches (18ea G.E. SBM; 5 additional SBM switches are Q qualified), and Neutron Monitoring System bypass switches (6ea).

9.0 SEISMIC CAPABILITY LIMITS OF EXISTING Q DEVICES FROM SWEC/VENDOR DATA BASE (FOR 05A ONLY)

Reference 9 addresses the seismic qualification of "Class I" electric equipment on all standard product line reactors. It summarizes qualification data obtained by methods which "did not differ markedly" from IEEE 344-1971. Reference 15 provides data for switches similar to those used for the Peach Bottom consoles and represents test to more current requirements (i.e. multi-axis testing, etc.). Certain data have been extracted and compared below.

DEVICE I	REFERENCE		NCTION HOR-2	LIMIT VERTICAL	DEVICE CLUSTER (FIG. 8-1)
Switch, type CR2940	15 9	20 g 20	209 20	20 § 20	2, 4, 5
Switch SB-1	9	10	10	10	G.E. (typical)
Switch SBM	9	25	25	25	7, 11, 13
Range Switch	9	8.5	8.5	8.5	9, 10

All devices, including the remaining Q device clusters (1, 3, 6, 8, and 12) are seismically qualified to at least the specified plant levels.

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TABLE 8-1

SUMMARY TABLE OF	EQUIPMENT	FOR CONSOLE(S)	2(3)0C05(A)
(extracted	from Ref.	14 in the same	order as annotations)

UPPER	(VERTICAL) PANEL
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LOWER (HORIZONTAL) PANEL

15- 6x6 MULTIPEN RECORDERS	
37- G.E. INDICATORS	
22- G.E. CR2940 SWITCHES	23- G.E.CR2940 SWITCHES
(7- Q)	(5- Q)
	23- G.E.SBM SWITCHES
•	(5- Q)
	8- MANUAL/AUTO HCS STATIONS

16- LOCAL POWER INDICATORS

- 8- IRM POWER RANGE SWITCHES
 (8- Q)
 6- NEUTRON MONITORING SYSTEM
- 6- NEUTRON MONITORING SYSTEM BYPASS SWITCHES (3- Q)
- 2- VALVE POSITION INDICATORS
- RWM OPERATOR INTERFACE (1) (DELETED BY MOD 955)
- 1- REACTOR MODE SWITCH (2) (1- Q)
- 1- SBLC PUMP SWITCH (3) (1- Q)
- 4- RFPT LOCKOUT RELAYS(4)

2- MANUAL SCRAM PB's (5) (2- Q)

- 1- ROD SELECT MATRIX (6) 4- ARI PUSH BUTTONS (7)
- 4- ROD DISPLAY INDICATORS (8) 2- G.E. CR115 SWITCHES (9) (2- Q)

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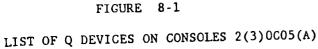
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Client

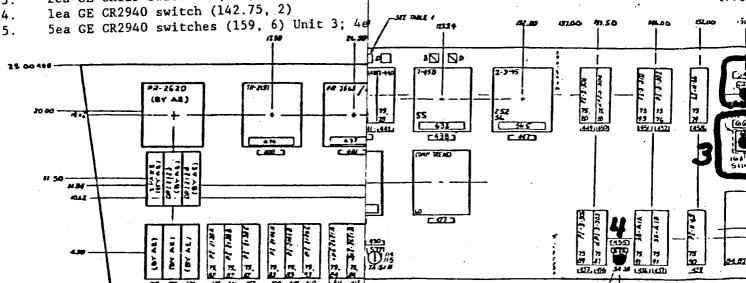
Subject or Apparatus

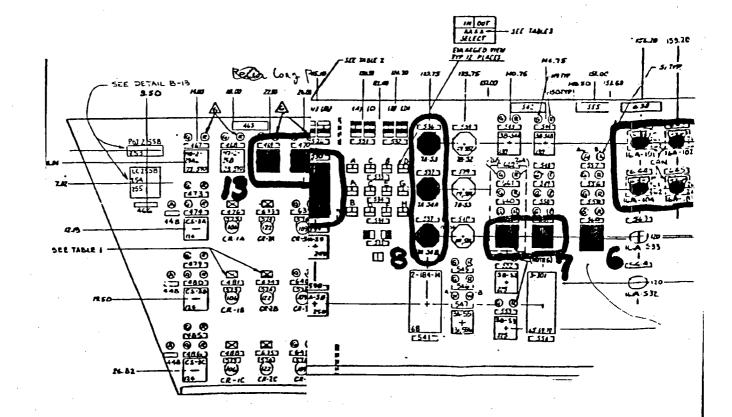
Reactor mode

VIEW F



- 2ea manual scram push buttons (84, 22) 1. 6ea GE CR2940 switches (160, 18)
- 2. 2ea GE CR115 switches (158, 15)
- 3.





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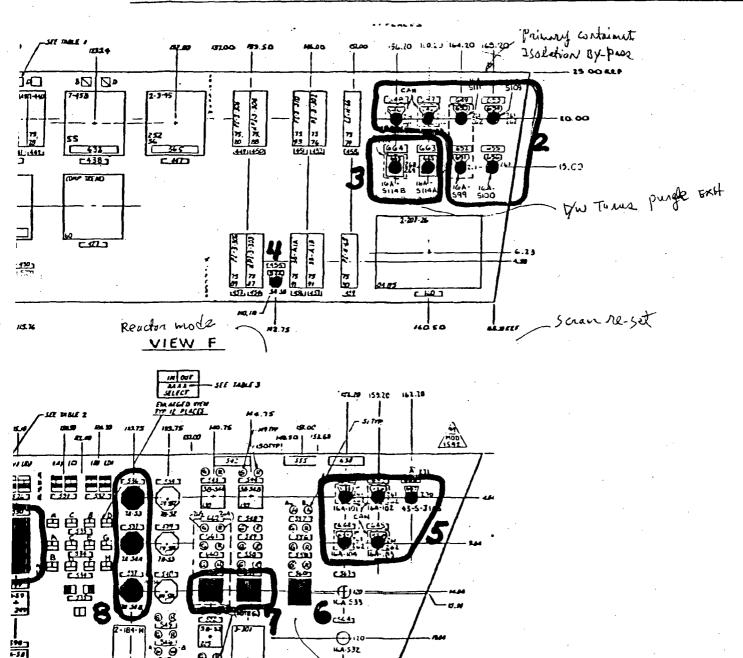
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Client	Location	Est. No.	J.O. No.
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Scram Visa



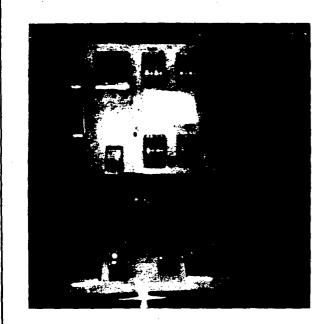
CALCULATION SHEET

Appendix 1

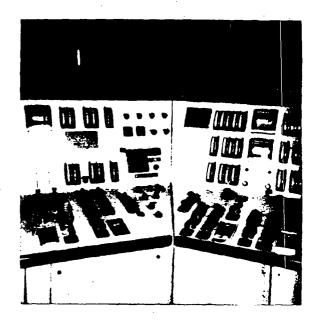
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(REACTOR) (RECIRC/RWCU) 30C05A → 30C004A



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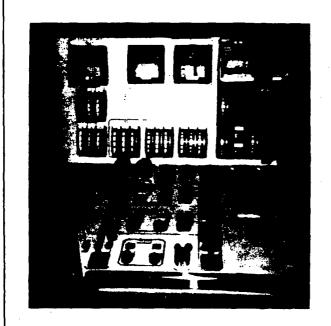
Appendix 1

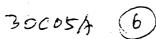
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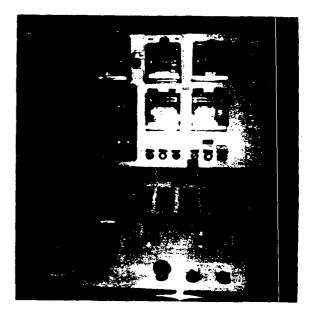
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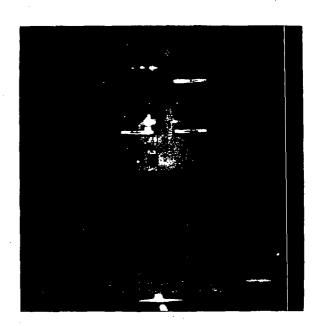




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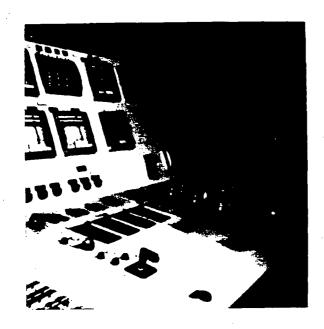
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Appendix 1

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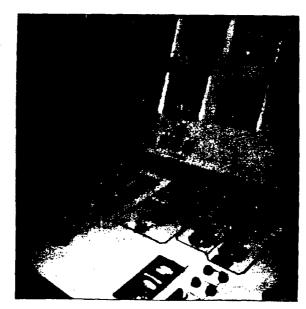
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30COSA Q-devices



30COSA Q-Devices



30 COSA Q-devices

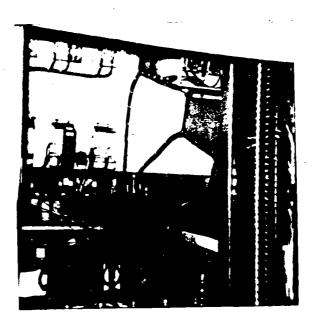
CALCULATION SHEET

Appendix 1

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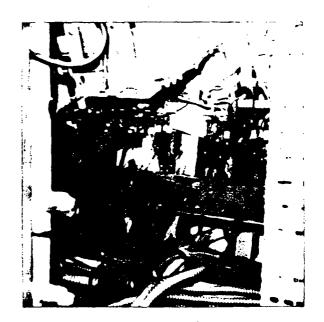
J.O. OR W.O. NO. DIVISION & GROUP CALCULATION NO. OPTIONAL TASK CODE PAGE 4

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30005A Framing





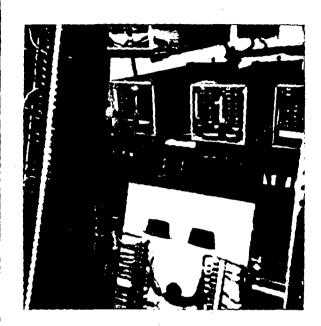
STONE & WEBSTER ENGINEERING CORPORATION CALCULATION SHEET

AFFENDIX |

CALCULATION IDENTIFICATION NUMBER

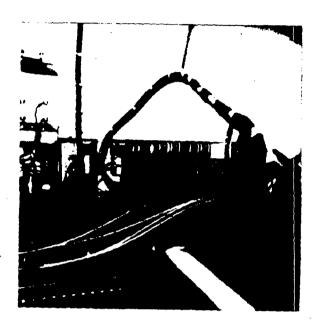
J.O. OR W.O. NO. DIVISION & GROUP | CALCULATION NO. | OPTIONAL TASK CODE | PAGE | 5 |

19597 | NM(C) | PS-049

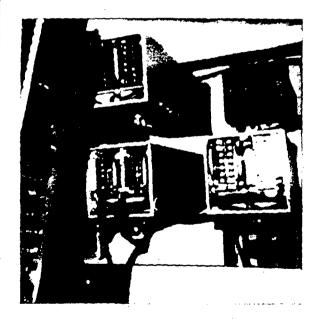


3 I

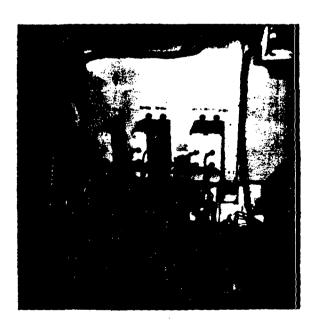
30005A (DEVICE detail_)



30005A (device detail) = Rom Inside



30005A



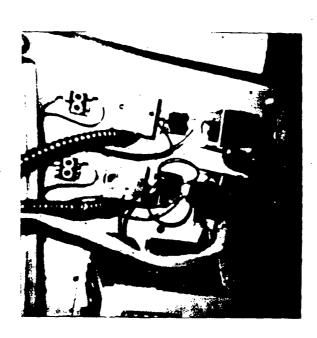
30 CO5 A (NON-Q device)

CALCULATION SHEET

A 5010.65

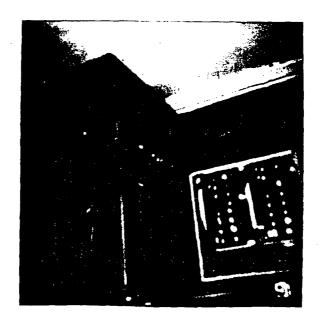
CALCULATION IDENTIFICATION NUMBER

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30005A (From back, Switch detail)

30COSA 22



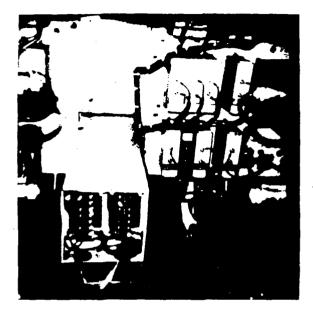
CALCULATION SHEET

Appendix 1

▲ 5010.65

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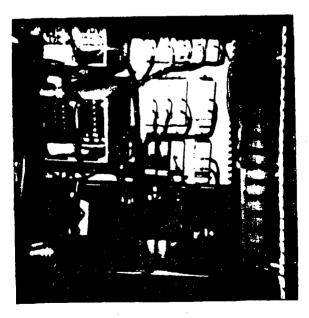
19597 NM 102 PS-049



30 COO4A (a-device defail)

30 CocaA (detail)





300004A (R-device)

CALCULATION SHEET

Appendix 1

5010.65				
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30CO7A (Q-concole)

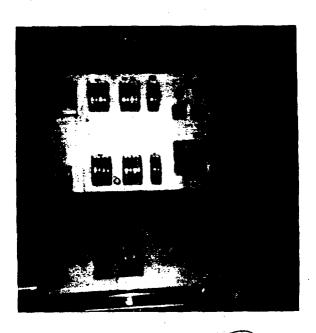
CALCULATION SHEET

Appendix 1

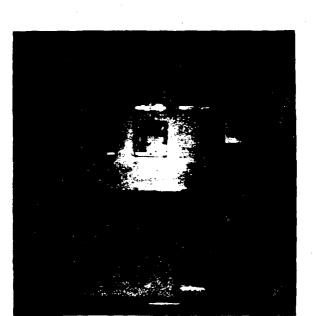
CALCULATION IDENTIFICATION NUMBER

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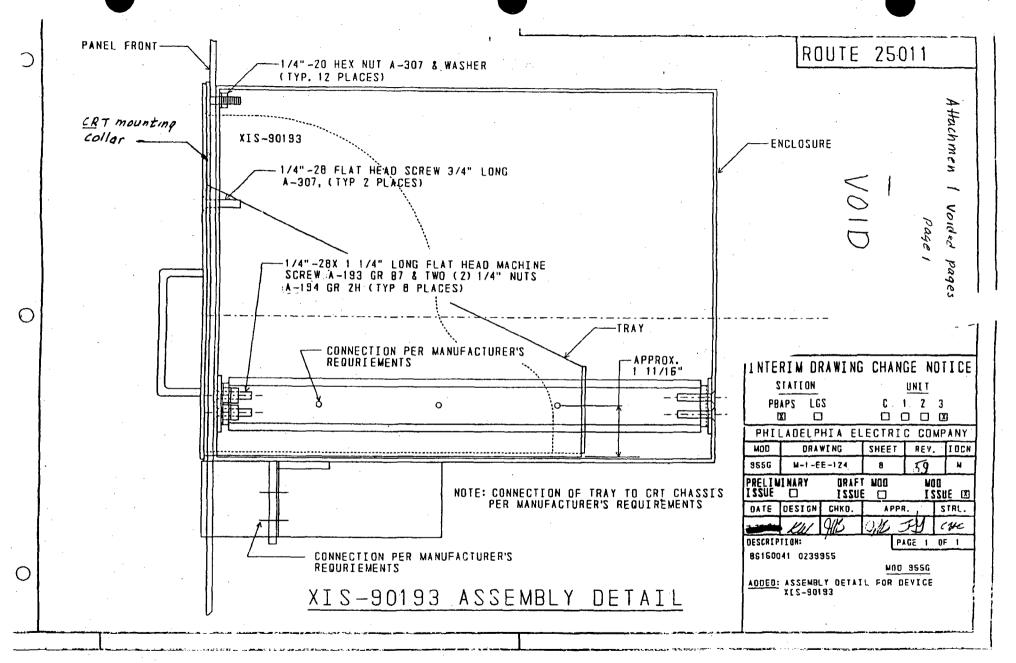


ZOCOSA



20005/





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CALCULATION SHEET

CALCULATION IDENTIFICATION NUMBER

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Attachment 1 Voided Pages page 2

The lower (horizontal) panel of the console contains the following Q-devices: (identification and location (X, Y) coordinates are according to Ref. 14, view E; locations of device clusters are approximate and provided for reference and information; see Figure 8-1 attached)

- 5. Sea GE CR2940 switches (159, 6) Unit 3; 4ea on Unit 2
- 6. lea SBLC pump switch (152, 15)

H

1.5

2 1

- 7. 2ea GE SBM switches (142, 14.84)
- 8. 3ea Neutron Monitoring System switches (129.75, 10)
- 9. 4ea IRM Power Range switch (110, 10)
- 10. 4ea IRM Power Range switch (58. 10)
- 11. lea GE SBM switch (96, 3.6)
- 12. lea Reactor Mode switch (70.75, 3.8)
- 13. 2ea GE SBM switches (24, 4.84)

These devices are concluded to be sufficiently isolated from any identified modification to assure against any potential interaction. Structural failure of any non-Q device on the horizontal panel will not occur and therefore will not create any unacceptable interaction with Q devices located below. Major non-Q devices include: the Manual/Auto HCS Stations (8ea 7-GEMAC, 1-L6N), the rod select matrix, switches (18ea G.E. SBM; 5 additional SBM switches are Q qualified), and Neutron Monitoring System bypass switches (6ea).

9.0 SEISMIC CAPABILITY LIMITS OF EXISTING Q DEVICES FROM SWEC/VENDOR DATA BASE (FOR 05A ONLY)

Reference 9 addresses the seismic qualification of "Class I" electric equipment on all standard product line reactors. It summarizes qualification data obtained by methods which "did not differ markedly" from IEEE 344-1971. Reference 15 provides data for devices similar to those used for the Peach Bottom consoles and represents test to more current requirements (i.e. multi-axis testing, etc.). Certain data have been extracted and compared below.

DEVICE		REFERENCE		MALFUNCTION LIMIT		
				HOR-1	HOR - 2	VERTICAL
	Switch type	CR2940	9	20 g	20 9	20 9
			15	20	20	20
	Switch SB-1		15	10	10	10
	Switch SBM		15	25	25	25
	Range Switch		15	8.5	8.5	8.5