

# EN DES CALCULATIONS

TITLE EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS				UNID SYSTEM(S)	PLANT/UNIT <i>111 ENTS</i> BAR SECTION(S)
PREPARING ORGANIZATION CEB		REV	(FOR MEDS USE)	MEDS ACCESSION NUMBER	
APPLICABLE DESIGN DOCUMENTS DS-C1.7.1	BRANCH/PROJECT IDENTIFIERS CSG-85-001	R0	<i>840229A0029</i>	<i>CEB '84 0227 001</i>	
		R1	<i>840613C0022</i>	<i>CEB '84 0522 011</i>	
		R2		<i>B41 '85 0731 002</i>	
KEY NOUNS Baseplates Expansion Anchors		R3			
REV	R0	R1	R2	STATEMENT OF PROBLEM	
DATE	<i>2/24/84</i>	<i>5/22/84</i>	<i>JUL 31 1985</i>	Civil Design Standard DS-C1.7.1 for General Anchorage to Concrete - Determination of Tensile Loads for Expansion Anchors - Evaluation of Plate Rigidity Requirement for Use of Rigid Baseplate Analysis	
PREPARED	<i>A.B. Ramsey</i>				
CHECKED	<i>A.B. Ramsey</i>				
SUBMITTED	<i>J. Kruttel</i>				
APPROVED	<i>N.H. Perry</i>				
ATTACHMENTS MICROFILMED:					
LIST ALL PAGES * ADDED BY THIS REV:			<i>PAGES 10A thru 40L</i>		
LIST ALL PAGES * DELETED BY THIS REV:					
LIST ALL PAGES * CHANGED BY THIS REV:			<i>PAGES 1-13</i>		

**ABSTRACT**

To evaluate the requirements for use of rigid plate analysis given in DS-C1.7.1, four sizes of plates were analyzed. The plates were sized to produce edge distances from the face of the attachment of 4- and 6-plate thicknesses. A pattern of eight expansion anchors was used on all of the plates. Two load conditions, axial load only and bi-axial moments only, were applied to the plates through a centered attachment. Analysis was performed using the CASD BAP program (a rigid analysis program) and Baseplate II (a finite element flexible analysis program). The results were compared to show the requirements for rigid baseplate analysis given in design Design Standard DS-C1.7.1 are acceptable.

*Computer output on microfilm roll # 50100*

*R1 Clarified statement on page 2*

*R2 ADDITIONAL CALCULATIONS WERE ADDED FOR THE EVALUATION OF THE EFFECTS OF COMBINED AXIAL (TENSION) AND BI-AXIAL BENDING ON THE REQUIREMENTS OF RIGID PLATE ANALYSIS.*

*Please return originals to NHL Perry (W9B108)*

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|R2

SHEET 1 OF 40  
EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS  
 COMPUTED TBR DATE 7/30/85  
 CHECKED LS DATE 7-30-85

PURPOSE

To evaluate the baseplate rigidity requirements given in Design Standard DS-C1.7.1, and to show that the requirements for baseplate analysis give acceptable anchor loads.

ASSUMPTIONS

1. An average anchor stiffness of 100 k/in was assumed for a 3/4-inch-diameter expansion anchor. A stiffness of 300 k/in was also used in the evaluation.
2. 3/4-inch-diameter self-drilling expansion anchors were used for the rigid evaluation.

DISCUSSION

The plates were sized to give the required distance between the face of the attachment and the edge of the plate. An 8-bolt pattern was selected to obtain varied spacings between the attachment and the anchors. For the smaller plates, 10-inch and 13-inch, the closer anchor spacings would be unacceptable with respect to concrete capacity but are conservative with respect to anchor load distribution. The configurations of plate size, anchor spacings, attachment size, and applied loads were used only for this evaluation and do not represent actual designs. For some of the configurations, the stress in the plate and tube section exceeds normal allowables. However, for this comparative analysis plate, stresses do not affect the results.

Table 1 summarizes the results from the evaluation. The rigid method for plates loaded with moment gives conservative anchor loads for the 4t edge distance and a  $K_t=100$  k/in. A 4t extension with an anchor stiffness of 300 k/in only under estimates the anchor load by 4 percent.

For the axial load condition, the flexible analysis will always give higher anchor loads than a rigid plate analysis unless all anchors are equidistant from the attachment. Even a smaller edge distance of 2t as given in the NCR 79-02 Bulletin will give an underestimation of the anchor loads by a few percent. For an anchor stiffness of 100 k/in, a rigid plate analysis for a plate with a 4t extension will result in an underestimation of the anchor load by only 5 percent.

CONCLUSION

Based on the above discussion, the 4t limitation for use of rigid plate analysis given in DS-C1.7.1 is acceptable for expansion anchors.

SHEET 2 OF 40  
EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS  
 COMPUTED TBR DATE 5/22/84  
 CHECKED ESK DATE 5/22/84

TABLE 1

Plate Size	Edge Dist.	CASDBAP	Maximum Anchor Loadings, kips			
			BASEPLATE II K = 100 k/in	* Z	BASEPLATE II K = 300 k/in	* Z
<b>Bi-Axial Moment Loading</b>						
10x10x3/4	4t	4.96	4.30	+15	4.78	+ 4
13x13x3/4	6t	4.46	4.44	-	4.51	- 1
16x16x1-1/2	4t	3.90	3.62	+ 8	4.06	- 4
22x22x1-1/2	6t	3.44	3.66	- 6	3.68	- 7
<b>Axial Loading</b>						
10x10x3/4	4t	2.50	2.62	- 5	2.82	-11
13x13x3/4	6t	2.88	3.35	-14	4.03	-29
16x16x1-1/2	4t	3.25	3.43	- 5	3.74	-13
22x22x1-1/2	6t	3.75	4.26	-12	5.03	-25
<b>Combined Axial and Bi-axial Moment Loading</b>						
10x10x3/4	4t	5.11	4.44	+15	4.91	+ 4
13x13x3/4	6t	3.58	3.53	+ 1	3.35	+ 7
16x16x1-1/2	4t	2.75	2.55	+ 8	2.83	- 3
22x22x1-1/2	6t	1.88	1.95	- 4	1.93	- 3

\* Z = ((Rigid-Flexible)/Flexible) x 100%

SHEET 3 OF 40  
EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS  
 COMPUTED TBR DATE 7/30/85  
 CHECKED LS DATE 7-30-85

REFERENCES

1. TVA Civil Design Standard DS-CI.7.1, Revision 1 Concrete Anchorages.
2. TVA Computer Program "CASDBAP" rigid baseplate analysis. TVA program identification number 262256.
3. Cybernet Services computer program "BASEPLATE II" finite element flexible plate analysis. BASEPLATE II User Manual Publication number 84002770.

SHEET 4 OF 40  
EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS  
COMPUTED TBL DATE 2/10/84  
CHECKED JSU DATE 2/13/84

EVALUATION OF BASEPLATE RIGIDITY  
REQUIREMENTS

5 of 40

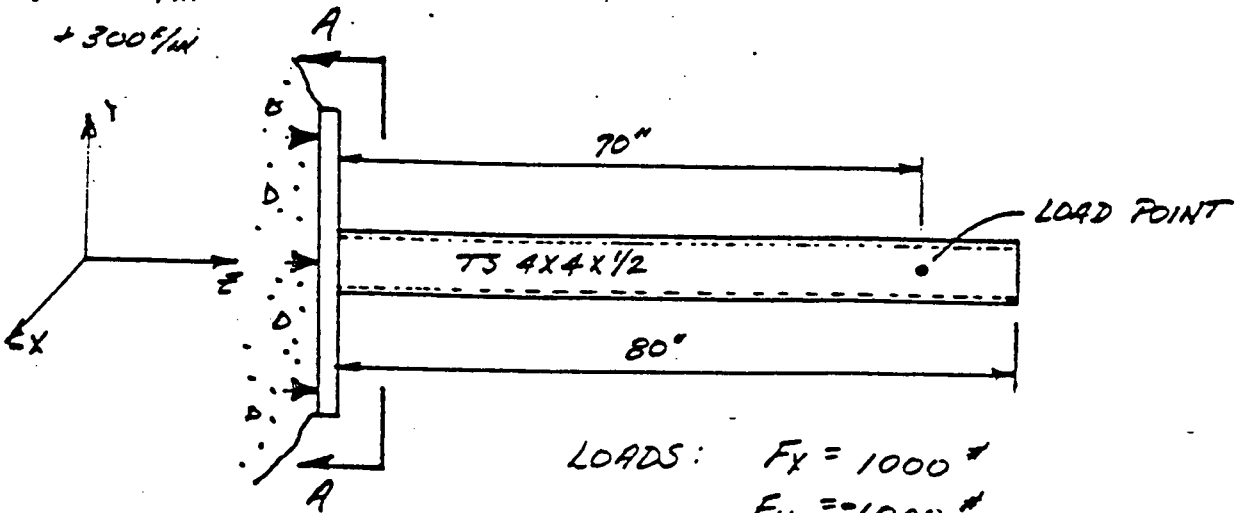
DS-CI.7.1

COMPUTED: TCR DATE 1/2/84

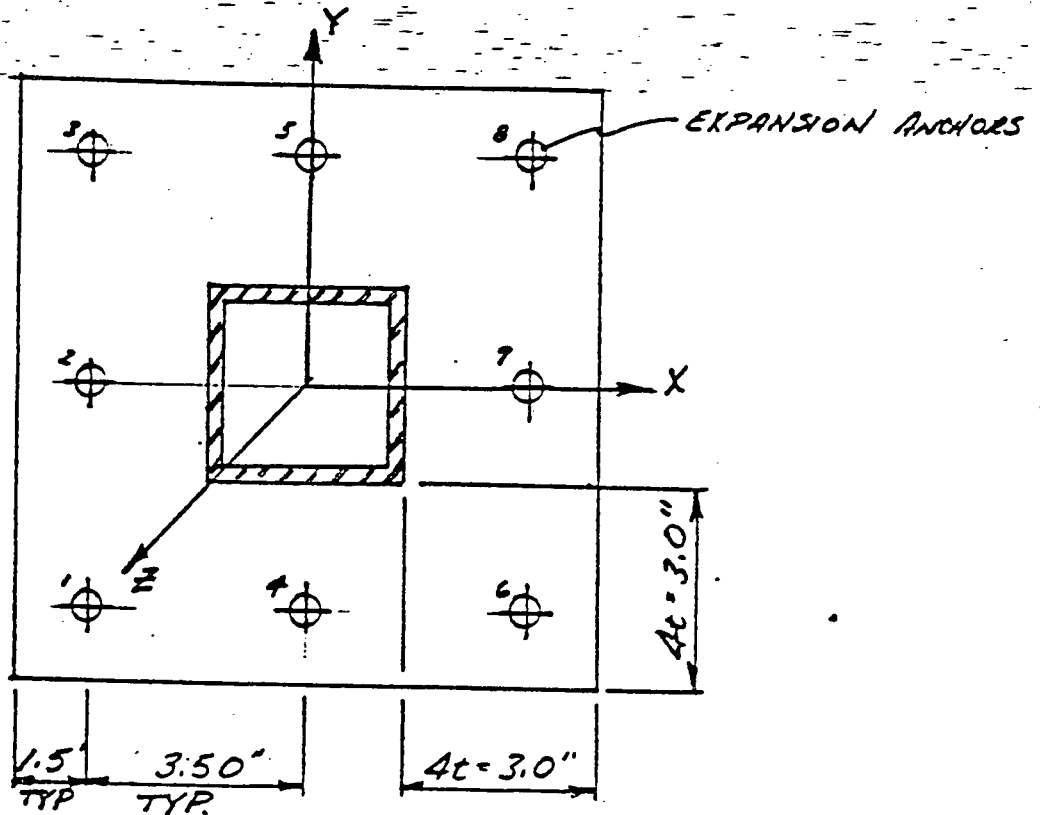
CHECKED: EJE DATE 1/31/84

PLATE SIZE - 10" x 10" x 3/4"

$K_x = 100 \text{ } \frac{\text{lb}}{\text{in}}$   
 $+ 300 \text{ } \frac{\text{lb}}{\text{in}}$



LOADS:  $F_x = 1000 \text{ } \#$   
 $F_y = -1000 \text{ } \#$   
 $F_z = 0$



SECTION A-A

EVALUATION OF BASEPLATE RIGIDITY  
REQUIREMENTS

SHEET 6 OF 40

DS-C1.7.1

COMPUTED TBR DATE 1/31/84

CHECKED PHL DATE 1/31/84

SUMMARY OF RESULTS  
FOR PLATE SIZE 10'x10'x3/4"

ANCHOR LOADS

ANCHOR NUMBER	RIGID KIPS	FLEXIBLE, KIPS	
		$K_f = 100 \text{ lb/in}$	$K_f = 300 \text{ lb/in}$
1	1.22	1.55	1.36
2	3.09	3.06	3.53
3	4.96	4.30	4.78
4	0	.34	0
5	3.09	3.06	3.53
6	0	0	0
7	0	.34	0
8	1.22	1.55	1.36

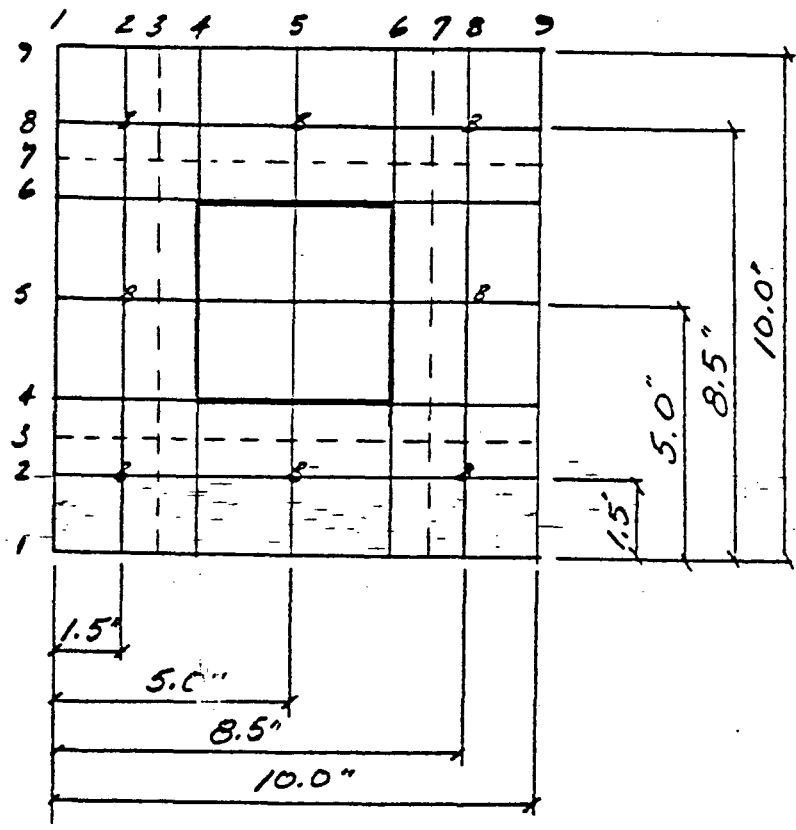
# EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

COMPUTED TBR DATE 1/29/84

CHECKED 9sh DATE 1/31/84

## BASEPLATE II (FINITE ELEMENT) MODEL

2 10x10x3/4"  
K<sub>t</sub> = 100 K/IN, 300 K/IN





IST

84/01/21. 07.27.59.  
PROGRAM BAPI0

00100 1.4.UM.0  
00110 3.9.NC.1.5  
00120 10.10.1.1  
00130 8  
00140 SD,.75,1.5,1.5  
00150 SD,.75,1.5,5.0  
00160 SD,.75,1.5,8.5  
00170 SD,.75,5.0,1.5  
00180 SD,.75,5.0,8.5  
00190 SD,.75,8.5,1.5  
00200 SD,.75,8.5,5.0  
00210 SD,.75,8.5,8.5  
00220 4.4,5.0,5.0  
00230 0.0,0.5,0.5,0.5,0.5  
READY.

SHEET 8 OF 40  
EVALUATION OF BASELINE  
READY REQUIREMENTS  
COMPUTED TBE DATE 1/21/84  
CHECKED gjk DATE 1/31/84

READY.

84/01/31. 07.29.30.  
PROGRAM CASDOOT

WIDTH OF PLATE = 10 INCHES  
LENGTH OF PLATE = 10 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	5
3	.334	1.5	8.5
4	.334	5	1.5
5	.334	5	8.5
6	.334	8.5	1.5
7	.334	8.5	5
8	.334	8.5	8.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1  
VERT LOAD = 0 KIPS LOCATION X = 5 INCHES Y = 5 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 5.83 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 5.83 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 0 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 5.83 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 5.83 KIP FEET

PARTIAL PRESSURE CASE

CONCRETE PRESSURE (KSI)  
C2 = 1.37

CONCRETE PRESSURE FORCE, CF = 13.6 KIPS  
LOCATION IN X-DIRECTION = 5.87 INCHES  
LOCATION IN Y-DIRECTION = 1.93 INCHES

FRICTION SHEAR CAPACITY, SF = 6.8 KIPS

PRESSURE BULB GEOMETRY  
Z3 = 7.71  
Z6 = 7.71

ANCHOR NUMBER	STRESS GROSS AREA (KSI)	LOAD KIPS
1	3.86	1.22
2	9.26	3.09
3	14.86	4.96
5	9.26	3.09
8	3.86	1.22

SHEET 9 OF 40

*EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS*

COMPUTED TBE DATE 1/21/84  
 CHECKED SKL DATE 1/31/84

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

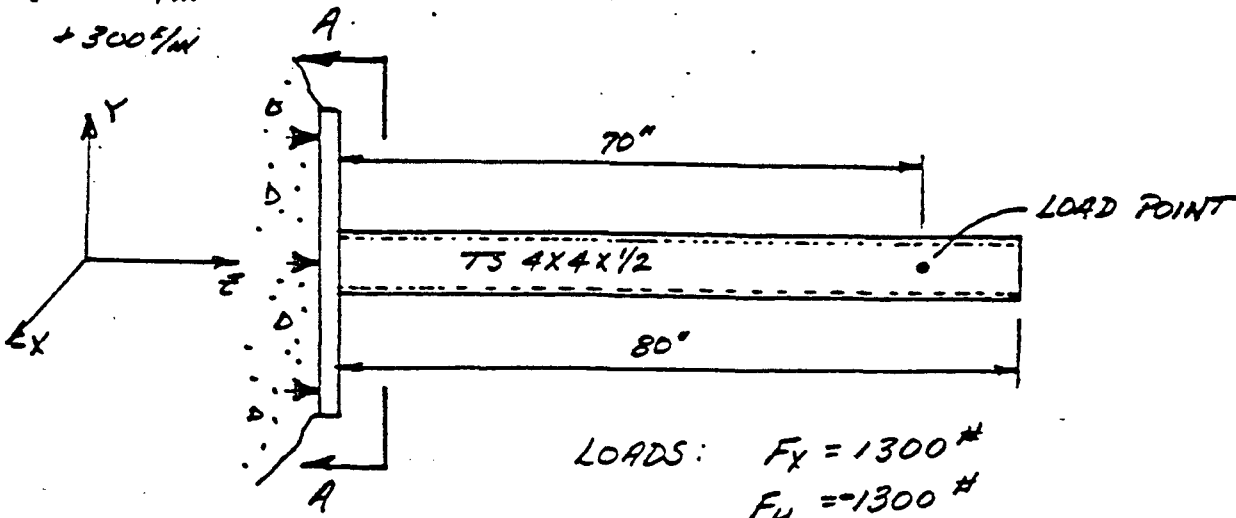
DS-C1.7.1

COMPUTED: TBR DATE 1/31/84

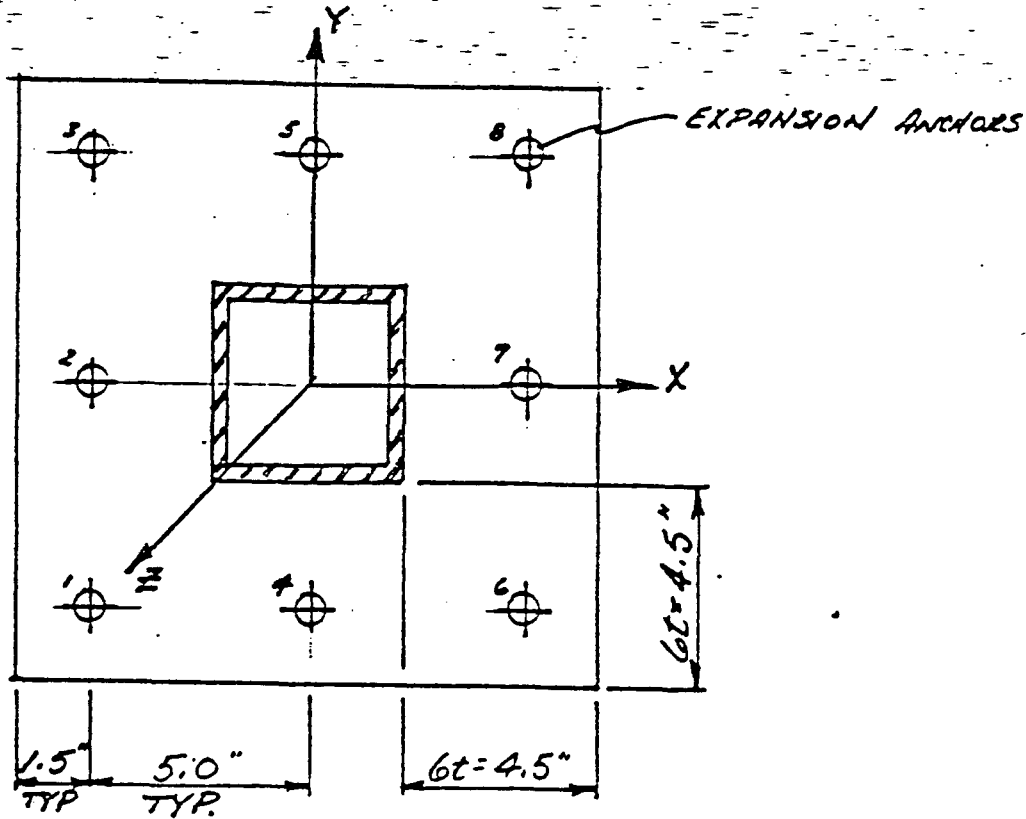
CHECKED: 95ll DATE 1/31/84

PLATE SIZE - 13" x 13" x 3/4"

$K_L = 100 \text{ } \#/\text{in}$   
 $+ 300 \text{ } \#/\text{in}$



LOADS:  $F_x = 1300 \text{ } \#$   
 $F_y = -1300 \text{ } \#$   
 $F_z = 0 \text{ } \#$



SECTION A-A

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

DS-C1.7.1

COMPUTED TBR DATE 1/31/8

CHECKED FAL DATE 1/31/8

SUMMARY OF RESULTS  
FOR PLATE SIZE 13"X13"X3/4"

ANCHOR LOADS

ANCHOR NUMBER	RIGID KIPS	FLEXIBLE, KIPS	
		K <sub>r</sub> = 100 <sup>psi</sup> /in	K <sub>r</sub> = 300 <sup>psi</sup> /in
1	1.28	1.27	.98
2	2.87	3.57	4.51
3	4.46	4.44	4.23
4	0	.21	.04
5	2.87	3.57	4.51
6	0	0	0
7	0	.21	.04
8	1.28	1.27	.98

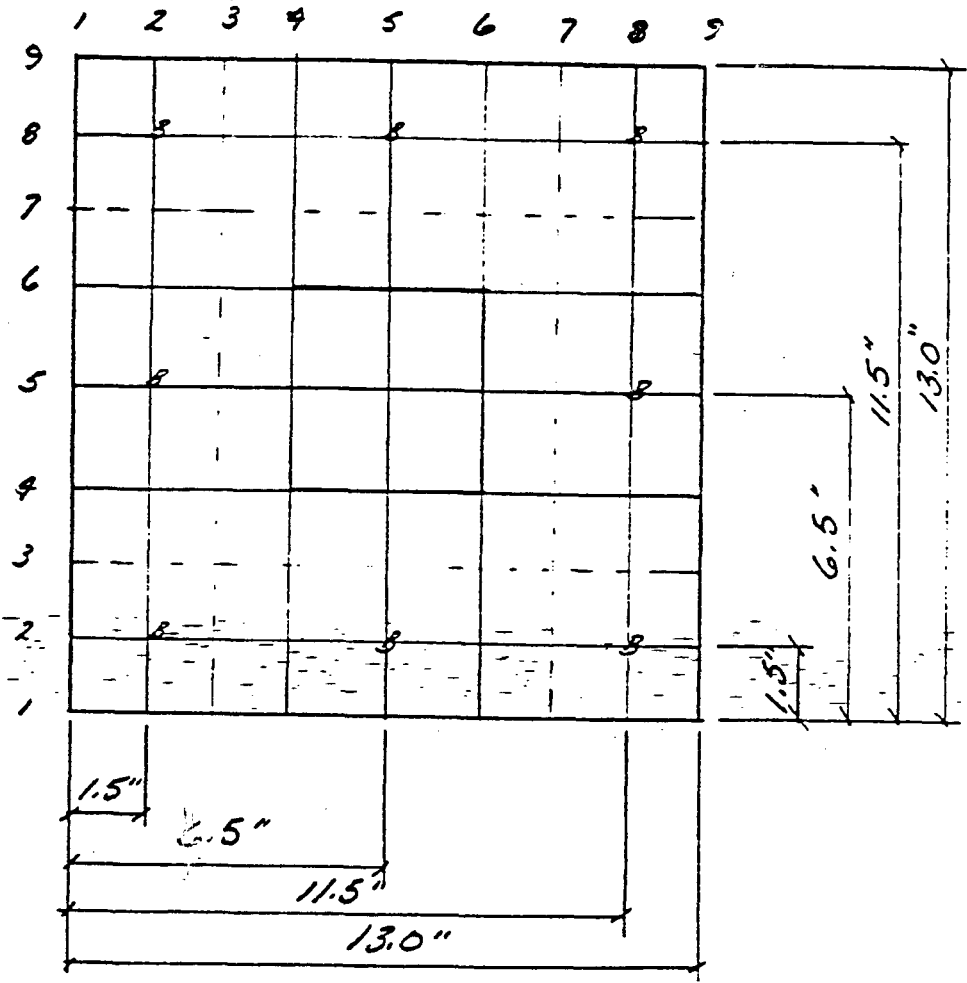
EVALUATION OF BASEPLATE RIGIDITY  
REQUIREMENTS

DS-C1.7.1

COMPUTED TBR DATE 1/20/84  
CHECKED Psh DATE 1/31/84

BASEPLATE II (FINITE ELEMENTS) MODEL

TR 13" X 13" X 3/4"  
K<sub>t</sub> = 100 K/IN, 300 K/IN



LIS

84-01/21. 07.37.17.  
PROGRAM BAPI3

00100 1.4.UH.0  
00110 3.9.HC.1.5  
00120 13.13.1.1  
00130 8  
00140 SD..75.1.5.1.5  
00150 SD..75.1.5.6.5  
00160 SD..75.1.5.11.5  
00170 SD..75.6.5.1.5  
00180 SD..75.6.5.11.5  
00190 SD..75.11.5.1.5  
00200 SD..75.11.5.6.5  
00210 SD..75.11.5.11.5  
00220 4.4.6.6.6.5  
00230 0.0.0.7.58.7.58.0  
READY.

SHEET 13 OF 40

EVALUATION OF BUREAU

ROADWAY REQUIREMENTS

COMPUTED TBE DATE 12/1/84

CHECKED PAK DATE 1/31/84

01/21. 07.38.18.  
PROGRAM CASTOUT

WIDTH OF PLATE = 13 INCHES  
LENGTH OF PLATE = 13 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	6.5
3	.334	1.5	11.5
4	.334	6.5	1.5
5	.334	6.5	6.5
6	.334	6.5	11.5
7	.334	11.5	1.5
8	.334	11.5	6.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1

VERT LOAD = 0 KIPS LOCATION X = 6.5 INCHES Y = 6.5 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 7.58 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 7.58 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 0 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 7.58 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 7.58 KIP FEET

PARTIAL PRESSURE CASE

CONCRETE PRESSURE (KSI)  
C2 = .95

CONCRETE PRESSURE FORCE, CF = -12.76 KIPS  
LOCATION IN X-DIRECTION = 10.76 INCHES  
LOCATION IN Y-DIRECTION = 2.24 INCHES

FRICTION SHEAR CAPACITY, SF = 6.38 KIPS

PRESSURE BULB GEOMETRY  
Z1 = 8.98  
Z2 = 8.98

ANCHOR NUMBER	STRESS CROSS AREA (KSI)	LOAD KIPS
1	3.87	1.28
2	8.59	2.87
3	13.38	4.46
5	8.59	2.87
8	3.87	1.28

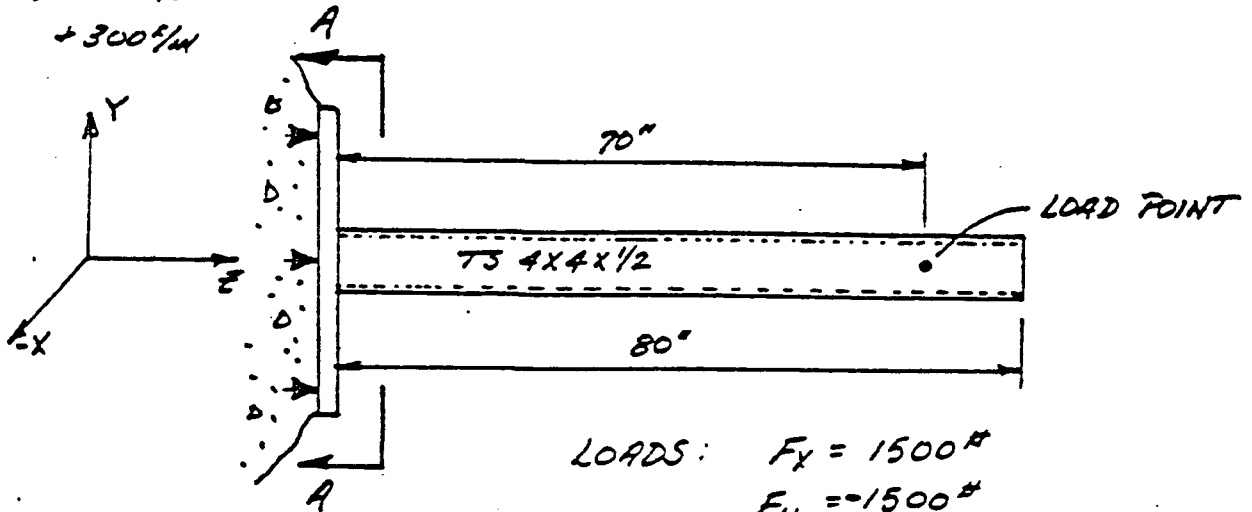
SHEET 14 OF 40  
EVALUATION OF SHEAR  
REINFORCEMENT REQUIREMENTS  
COMPUTED TRE DATE 1/21/84  
CHECKED SL DATE 1/21/84

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

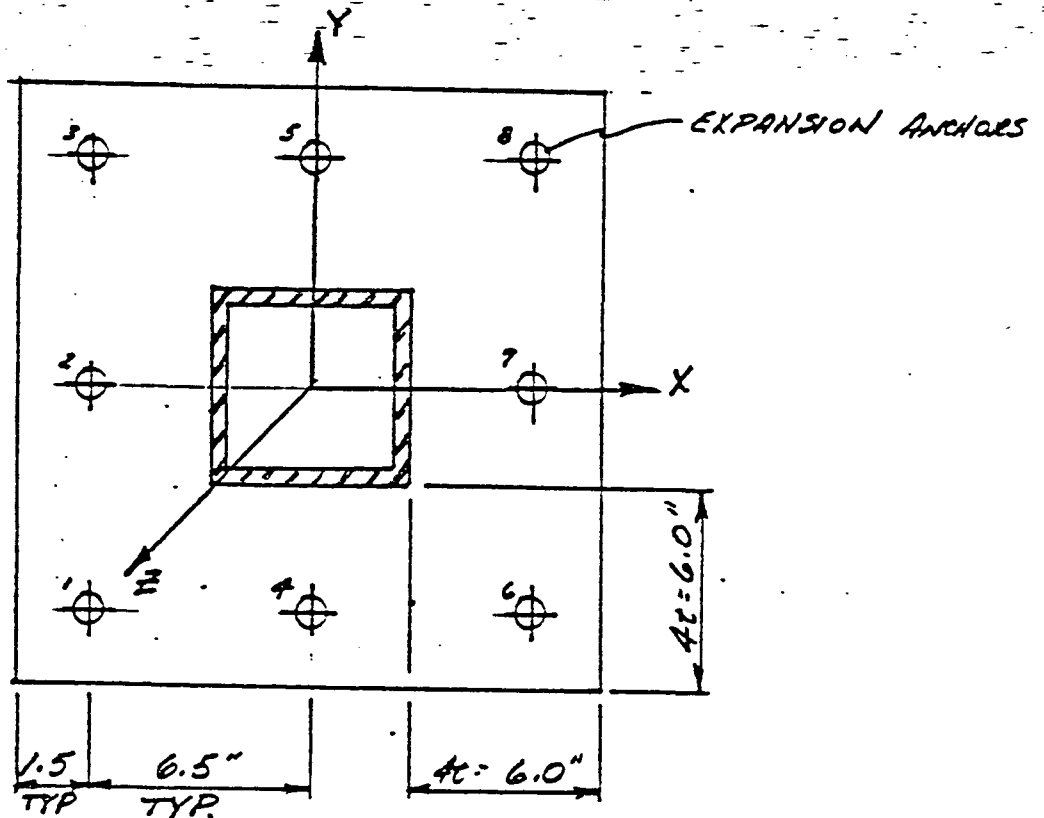
PLATE SIZE - 16" x 16" x 1 1/2"

$K_x = 100 \text{ } \frac{\text{lb}}{\text{in}}$

$+ 300 \text{ } \frac{\text{lb}}{\text{in}}$



LOADS:  $F_x = 1500 \text{ } \#$   
 $F_y = -1500 \text{ } \#$   
 $F_z = 0 \text{ } \#$



SECTION A-A



EVALUATION OF BASEPLATE RIGIDITY  
REQUIREMENTS

DS-C1.7.1

COMPUTED TBR DATE 1/31/84  
CHECKED [Signature] DATE 1/31/84

SUMMARY OF RESULTS  
FOR PLATE SIZE 16X16X1 1/2"

ANCHOR LOADS

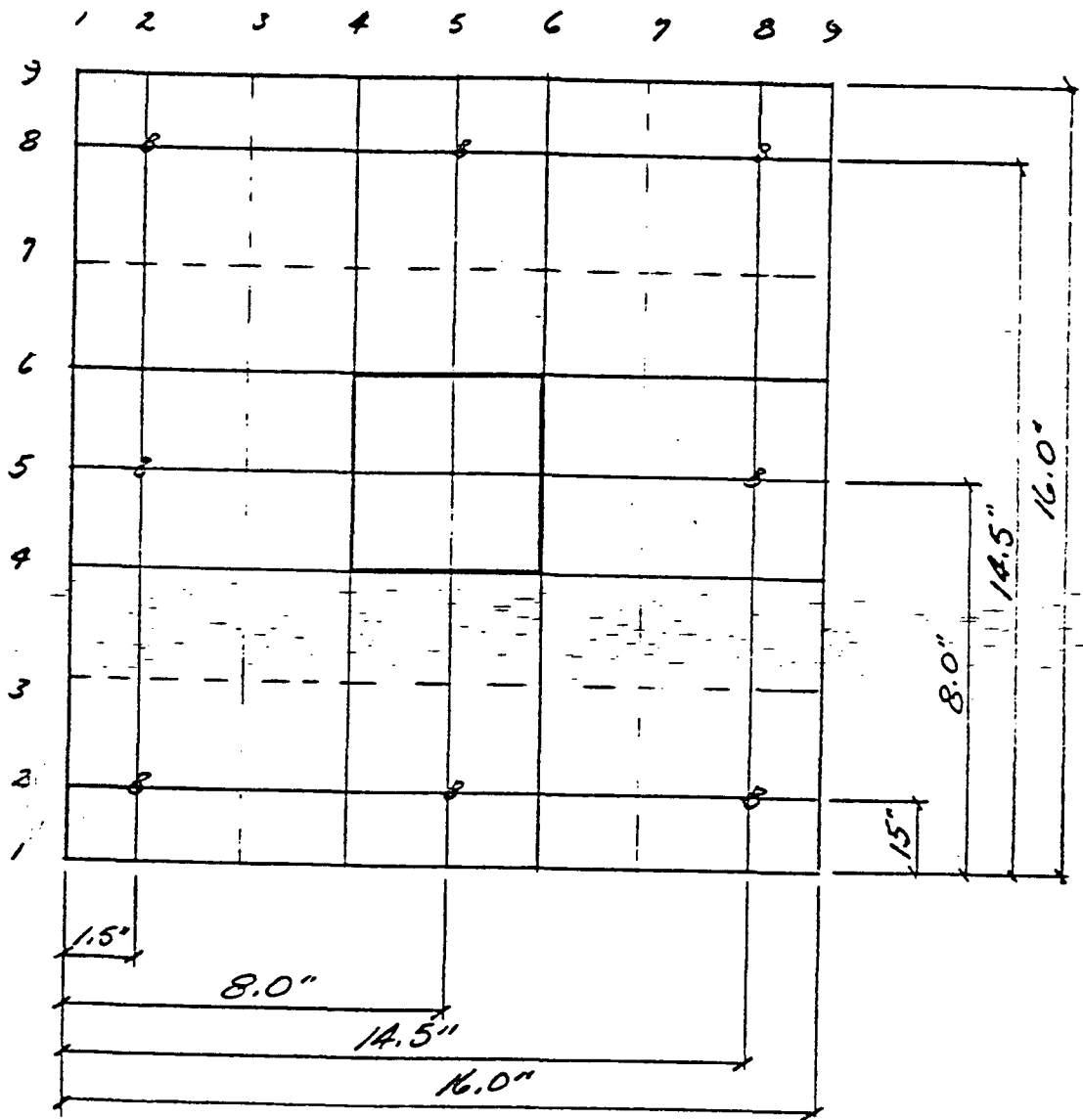
ANCHOR NUMBER	RIGID KIPS	FLEXIBLE, KIPS	
		$K_f = 100 \text{ k/in}$	$K_f = 300 \text{ k/in}$
1	1.23	1.16	1.02
2	2.56	2.56	3.09
3	3.90	3.62	4.06
4	0	.19	0
5	2.56	2.56	3.09
6	0	0	0
7	0	.19	0
8	1.23	1.16	1.02

EVALUATION OF BASEPLATE RIGIDITY  
REQUIREMENTS

DS-CI.7.1

COMPUTED TBR DATE 1/20/84  
CHECKED PJK DATE 1/31/84

BASEPLATE II (FINITE ELEMENTS) MODEL  
R<sub>2</sub> 16x16x1 1/2"  
K<sub>t</sub> = 100 K/IN, 300 K/IN



15

84/01/21. 07.39.03.  
PROGRAM DAP16

00100 1,4,UN,0  
00110 3,9,NC,1.5  
00120 16,16,1,1  
00130 8  
00140 SD,.75,1.5,1.5  
00150 SD,.75,1.5,8.0  
00160 SD,.75,1.5,14.5  
00170 SD,.75,8.0,1.5  
00180 SD,.75,8.0,14.5  
00190 SD,.75,14.5,1.5  
00200 SD,.75,14.5,8.0  
00210 SD,.75,14.5,14.5  
00220 4,4,8.0,8.0  
00230 0.0,0.8,75,8.75,0  
READY.

PRINT 18 OF 40  
*EMILY POY OF BOSTON*  
*ALDERTY BOURGEMONT*  
COMPUTED TR DATE 1/21/84  
CHECKED PAL DATE 1/31/84

84-01/21. 07.40.04.  
PROGRAM CASDOUT

WIDTH OF PLATE = 16 INCHES  
LENGTH OF PLATE = 16 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	8
3	.334	1.5	14.5
4	.334	8	1.5
5	.334	8	14.5
6	.334	14.5	1.5
7	.334	14.5	8
8	.334	14.5	14.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1  
VERT LOAD = 0 KIPS LOCATION X = 8 INCHES Y = 8 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 8.75 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 8.75 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 0 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 8.75 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 8.75 KIP FEET

PARTIAL PRESSURE CASE

CONCRETE PRESSURE (K/IN)  
C2 = .68

CONCRETE PRESSURE FACTOR, CF = 11.47 KIPS  
LOCATION IN X-DIRECTION = 13.49 INCHES  
LOCATION IN Y-DIRECTION = 2.51 INCHES

FRICTION SHEAR CAPACITY, SF = 5.74 KIPS

PRESSURE WILD GEOMETRY  
Z3 = 10.03  
Z8 = 10.03

ANCHOR NUMBER	STRESS GROSS AREA (KSI)	LOAD KIPS
1	3.67	1.23
2	7.67	2.56
3	11.67	3.8
5	7.67	2.56
8	3.67	1.23

SHEET 19 OF 10

*EMULATION OF CASDOUT*  
*PRIORITY REVIEW COMMENTS*

COMPUTED TBR DATE 1/21/84  
CHECKED SLH DATE 1/31/84

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

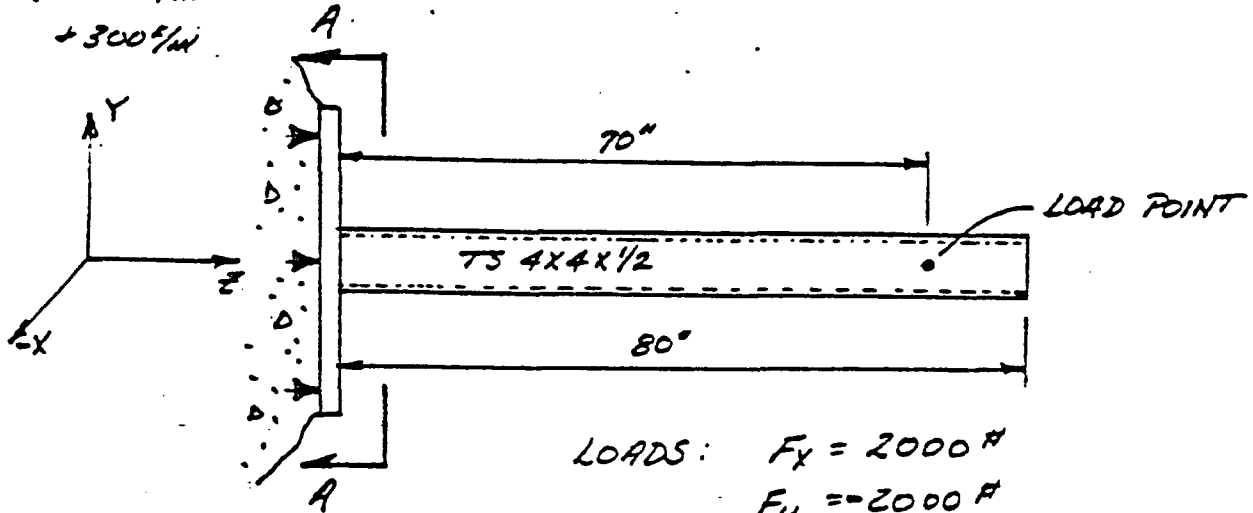
DS-C1.7.1

COMPLETED TBR DATE 1/31/84

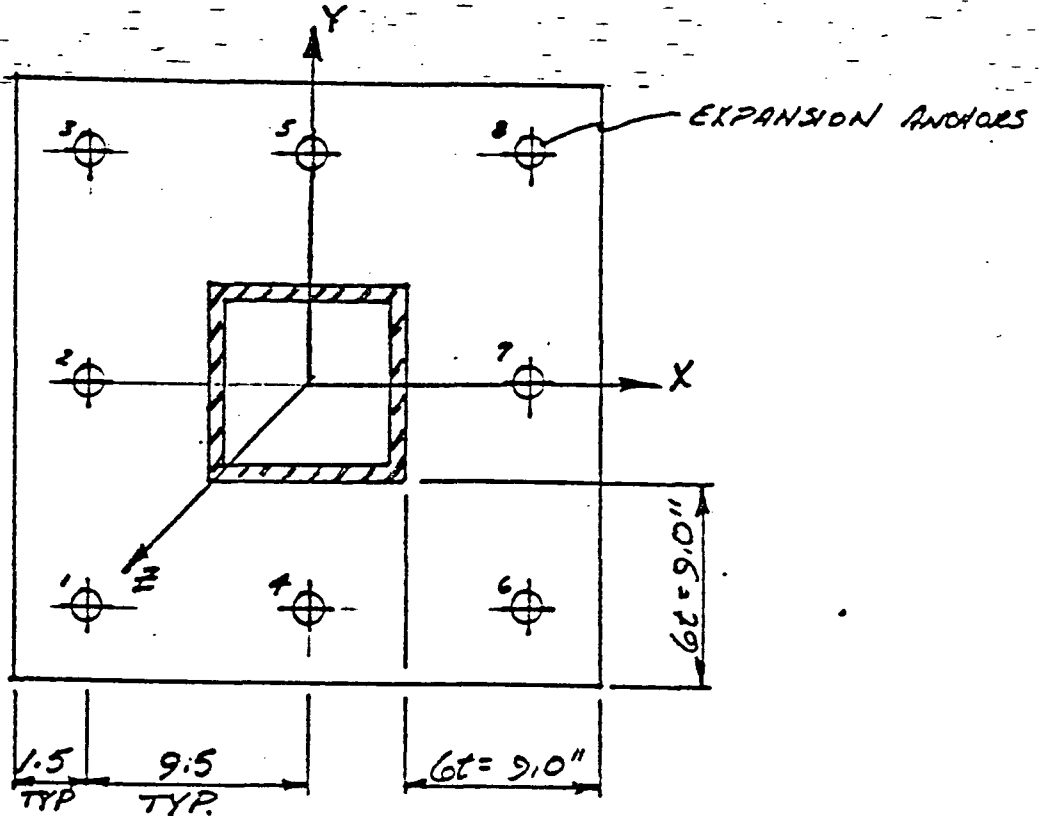
CHECKED [Signature] DATE 1/31/84

PLATE SIZE - 22" x 22" x 1 1/2"

$K_x = 100 \text{ kN}$   
 $+ 300 \text{ kN}$



LOADS:  $F_x = 2000 \text{ lb}$   
 $F_y = -2000 \text{ lb}$   
 $F_z = 0 \text{ lb}$



SECTION A-A

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

DS-C.1.7.1

COMPUTED TBR DATE 1/31/84

CHECKED 9/24 DATE 1/31/84

SUMMARY OF RESULTS  
FOR PLATE SIZE 22x22x1 1/2

ANCHOR LOADS

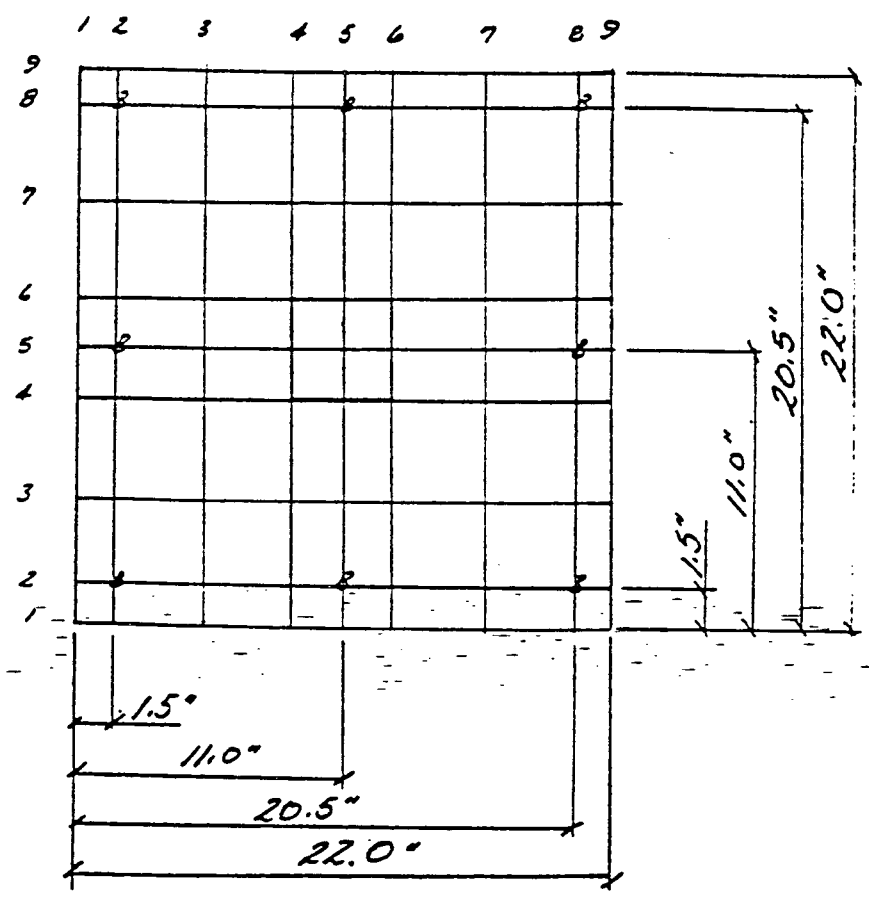
ANCHOR NUMBER	RIGID KIPS	FLEXIBLE, KIPS	
		K <sub>r</sub> = 100% <sub>W</sub>	K <sub>r</sub> = 300% <sub>W</sub>
1	1.20	.94	.75
2	2.32	2.77	3.56
3	3.44	3.66	3.68
4	.09	.07	0
5	2.32	2.77	3.56
6	0	0	0
7	.09	.07	0
8	1.20	.94	.75

EVALUATION OF BASEPLATE RIGIDITY  
REQUIREMENTS

DS-C.1.7.1

COMPUTED TBC DATE 1/20/84  
CHECKED P. J. DATE 1/31/84

BASEPLATE II (FINITE ELEMENTS) MODEL  
R 22'x22'x 1 1/2"  
Kt = 100#/IN, 300#/IN



LIS

34/01/21. 07.40.45.  
PROGRAM BAP22

00100 1,4,UN,0  
00110 3,9,NC,1.5  
00120 22,22,1,1  
00130 8  
00140 SD,.75,1.5,1.5  
00150 SD,.75,1.5,11.0  
00160 SD,.75,1.5,20.5  
00170 SD,.75,11.0,1.5  
00180 SD,.75,11.0,20.5  
00100 SD,.75,20.5,1.5  
00200 SD,.75,20.5,11.0  
00210 SD,.75,20.5,20.5  
00220 4,4,11.0,11.0  
00230 0,0,0,11.66,11.66,0  
READY.

SHEET 23 OF 40

EVALUATION OF BISEPUMI

Biology Requirements

COMPUTED TBE DATE 1/21/84

CHECKED PK DATE 1/31/84



.42.48.  
PROGRAM CASPOUT

WIDTH OF PLATE = 22 INCHES  
LENGTH OF PLATE = 22 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X COORDINATE IN	Y COORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	11
3	.334	1.5	20.5
4	.334	11	1.5
5	.334	11	20.5
6	.334	20.5	1.5
7	.334	20.5	11
8	.334	20.5	20.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1  
VERT LOAD = 0 KIPS LOCATION X = 11 INCHES Y = 11 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 11.68 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 11.68 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 0 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 11.68 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 11.68 KIP FEET

PARTIAL PRESSURE CASE

CONCRETE PRESSURE (KSI)  
C2 = .45

CONCRETE PRESSURE FORCE, CF = 10.87 KIPS  
LOCATION IN X-DIRECTION = 19.06 INCHES  
LOCATION IN Y-DIRECTION = 2.94 INCHES

FRICTION SHEAR CAPACITY, CF = 5.53 KIPS

PRESSURE BUILD GEOMETRY  
Z1 = 11.78  
Z6 = 11.78

ANCHOR NUMBER	STRESS GROSS AREA (KSI)	LOAD KIPS
1	3.61	1.2
2	6.96	2.32
3	10.31	3.44
4	.26	.09
5	6.96	2.32
7	.26	.09
8	3.61	1.2

SHEET 24 OF 40  
EVALUATION OF BASEMENT  
STABILITY REQUIREMENTS  
COMPUTED TBR DATE 1/21/84  
CHECKED SA DATE 1/31/84

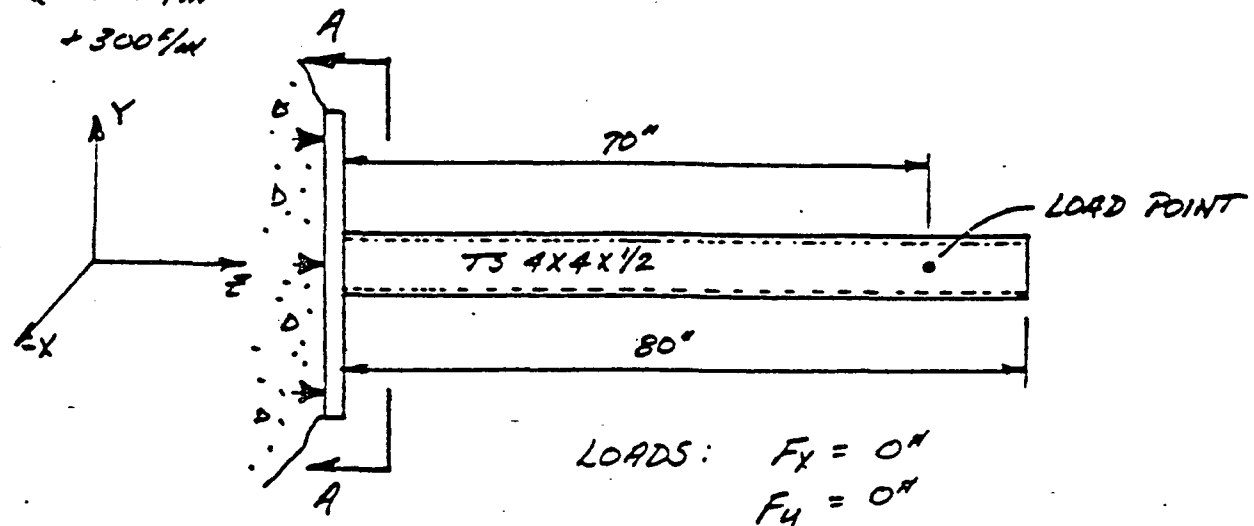
# EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

DS-C1.7.1

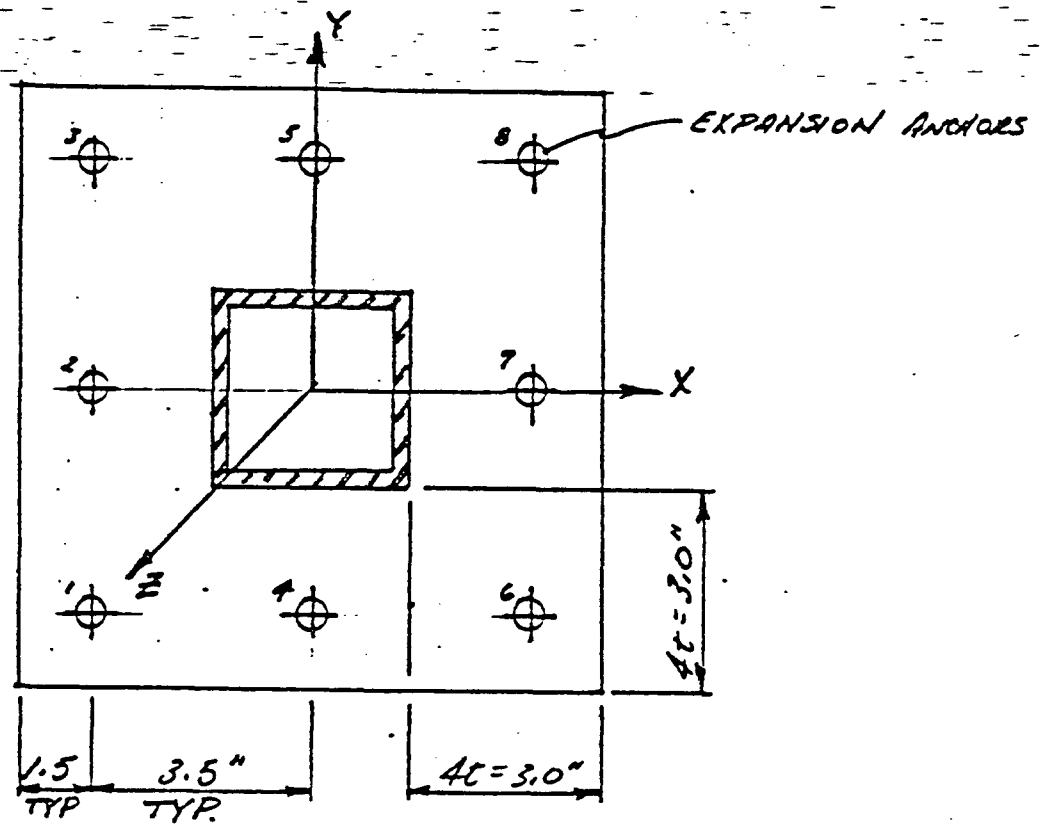
COMPUTED TBR DATE 1/31/84  
CHECKED 9AM DATE 1/31/84

PLATE SIZE - 10" x 10" x 3/4"

$K_x = 100 \text{ k/in}$   
 $+ 300 \text{ k/in}$



LOADS:  $F_x = 0 \text{ k}$   
 $F_y = 0 \text{ k}$   
 $F_z = 20000 \text{ k}$



SECTION A-A

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

SHEET 26 OF 40

DS-C1.7.1

COMPUTED TBR DATE 1/31/84

CHECKED PSL DATE 1/31/84

SUMMARY OF RESULTS  
FOR PLATE SIZE 10X10X3/4

ANCHOR LOADS

ANCHOR NUMBER	RIGID KIPS	FLEXIBLE, KIPS	
		$K_r = 100^{\text{th}}$	$K_r = 300^{\text{th}}$
1	2.5	2.39	2.19
2	2.5	2.61	2.81
3	2.5	2.39	2.19
4	2.5	2.62	2.82
5	2.5	2.62	2.82
6	2.5	2.39	2.18
7	2.5	2.61	2.81
8	2.5	2.39	2.18

READY.  
CIS

84/01/28. 10.11.20.  
PROGRAM BAP10

00100 1,4,UN,0  
00110 3,9-NO,1.5  
00120 10,10,1,1  
00130 8  
00140 8D,.75,1.5,1.5  
00150 8D,.75,1.5,5.0  
00160 8D,.75,1.5,9.5  
00170 8D,.75,5.0,1.5  
00180 8D,.75,5.0,8.5  
00190 8D,.75,8.5,1.5  
00200 8D,.75,8.5,5.0  
00210 8D,.75,8.5,8.5  
00220 4,4,5.0,5.0  
00230 0,0,20,0,0,0  
READY.

SHEET 27 OF 40  
EVALUATION OF BASEPLATE

RIGIDITY REQUIREMENTS

COMPUTED TBR DATE 1/23/84

CHECKED Pdk DATE 1/31/84

DISCREPANCY  
LIG.F=CASIDOUT  
ILLEGAL COMMAND  
LIG.F=CASIDOUT

EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS

BA701728. 10.11.77.  
PROGRAM CASIDOUT

COMPUTED TBR DATE 1/23/84

CHECKED CHK DATE 1/31/84

WIDTH OF PLATE = 10 INCHES  
LENGTH OF PLATE = 10 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X ORDNATE IN	Y ORDNATE IN
1	.334	1.5	1.5
2	.334	1.5	5
3	.334	1.5	8.5
4	.334	5	1.5
5	.334	5	5
6	.334	5	8.5
7	.334	8.5	1.5
8	.334	8.5	5
		8.5	8.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1

VERT LOAD = 20 KIPS LOCATION X = 5 INCHES Y = 5 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 0 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 0 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT (S) = 20 KIP (S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 0 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 0 KIP FEET

UPLIFT CONDITION ON ANCHORS

ANCHOR NUMBER	STRESS KSI	TOTAL FORCE KIPS
1	7.49	2.5
2	7.49	2.5
3	7.49	2.5
4	7.49	2.5
5	7.49	2.5
6	7.49	2.5
7	7.49	2.5
8	7.49	2.5

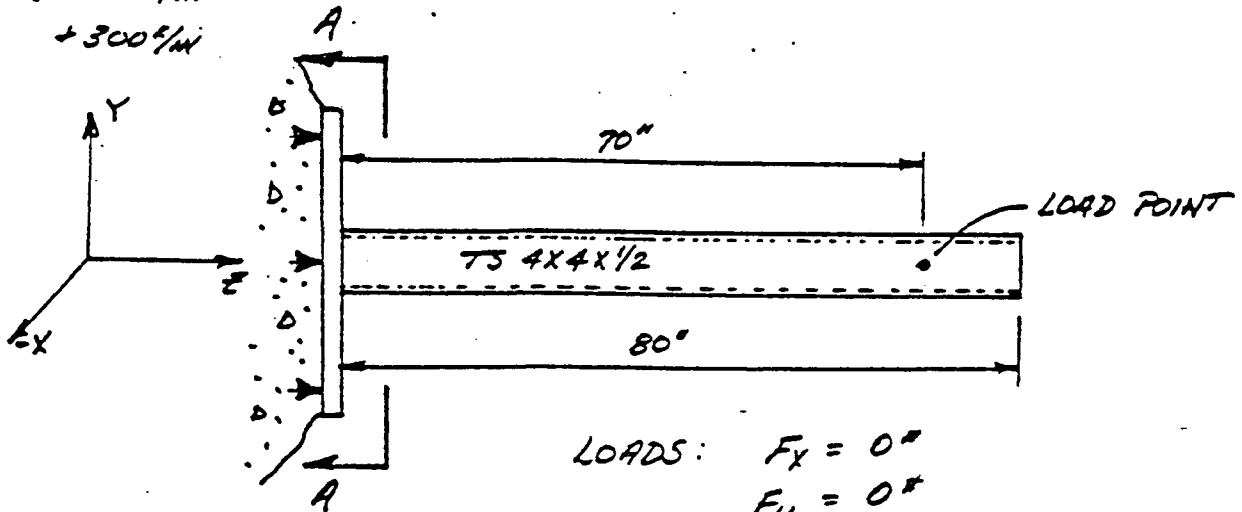
READY.

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

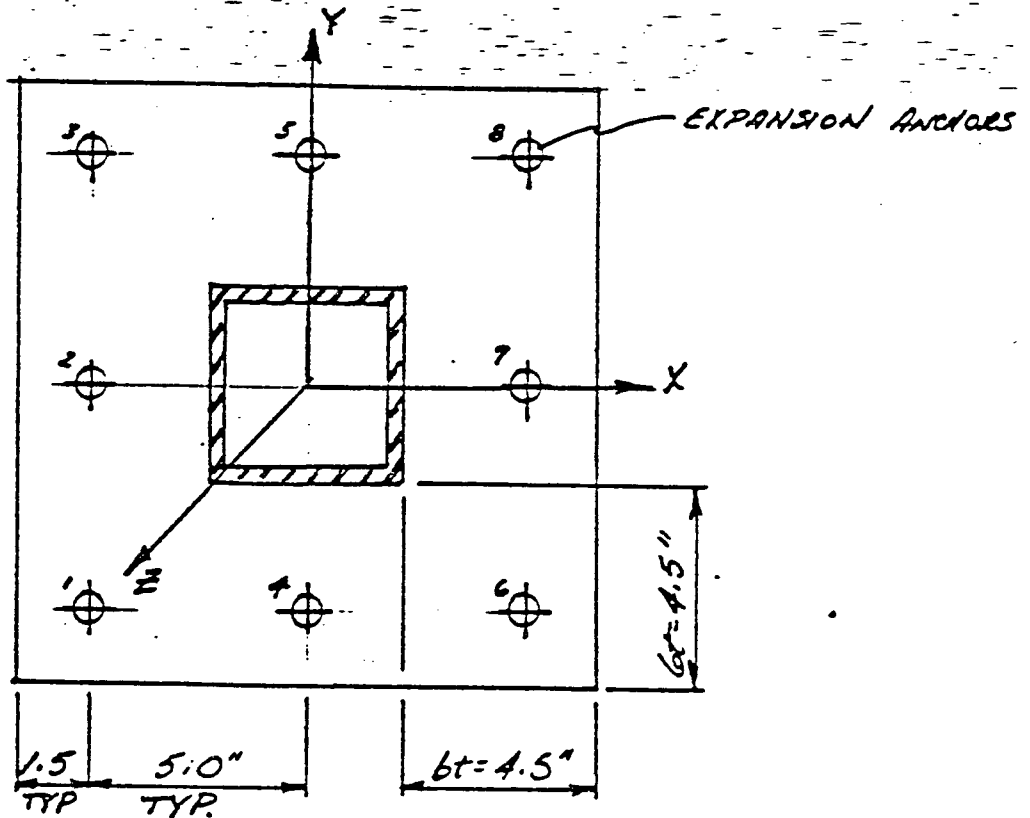
COMPUTED TBR DATE 1/31/84  
CHECKED C. M. DATE 1/31/84

PLATE SIZE - 13" x 13" x 3/4"

$K_E = 100 \text{ lb/in}$   
 $+ 300 \text{ lb/in}$



LOADS:  $F_x = 0 \text{ lb}$   
 $F_y = 0 \text{ lb}$   
 $F_z = 23000 \text{ lb}$



SECTION A-A

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

DS-C.1.7.1

COMPUTED TBR DATE 1/31/8

CHECKED Jsk DATE 1/31/8

SUMMARY OF RESULTS  
FOR PLATE SIZE 13"X13"X3/4"

ANCHOR LOADS

ANCHOR NUMBER	RIGID KIPS	FLEXIBLE, KIPS	
		K <sub>r</sub> = 100%W	K <sub>r</sub> = 300%W
1	2.88	2.40	1.91
2	2.88	3.35	4.02
3	2.88	2.40	1.91
4	2.88	3.35	4.03
5	2.88	3.35	4.03
6	2.88	2.40	1.91
7	2.88	3.35	4.02
8	2.88	2.40	1.91

UNRECEIVED  
READY.  
L108  
ILLEGAL COMMAND.  
L18

SHEET 31 OF 40

EVALUATION OF BASERATE  
RIGIDITY REQUIREMENTS

COMPUTED TBR DATE 1/23/84

CHECKED PH DATE 1/31/84

84701/28. 10.15.17.  
PROGRAM: RAPID

00100 1.4.0N.0  
00110 3.9.NC.1.5  
00120 18.13.1.1  
00130 8  
00140 8D.75.1.5-1.5  
00150 8D.75.1.5-6.5  
00160 8D.75.1.5.11.5  
00170 8D.75-6.5.1.5  
00180 8D.75.1.5.11.0  
00190 8D.75.11.5.1.5  
00200 8D.75-11.5.6.5  
00210 8D.75.11.5.11.5  
00220 4.4.6.5.6.5  
00230 0.0.28.0.0.0  
READY.



EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS

WIDTH OF PLATE = 18 INCHES  
LENGTH OF PLATE = 18 INCHES  
MODULAR RATIO = 4

COMPUTED TBR DATE 1/23/84  
CHECKED PH DATE 1/31/84

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	6.5
3	.334	1.5	11.5
4	.334	6.5	1.5
5	.334	6.5	11.5
6	.334	11.5	1.5
7	.334	11.5	6.5
8	.334	11.5	11.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1

VERT LOAD = 28 KIPS LOCATION X = 6.5 INCHES Y = 6.5 INCHES

APPLIED MOMENT ABOUT Y AXIS = 0 KIP FEET

APPLIED MOMENT ABOUT X AXIS = 0 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 28 KIP(S)

TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 0 KIP FEET

TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 0 KIP FEET

UPLIFT CONDITION ON ANCHORS

ANCHOR NUMBER	STRESS KSI	TOTAL FORCE KIPS
1	8.61	2.88
2	8.61	2.88
3	8.61	2.88
4	8.61	2.88
5	8.61	2.87
6	8.61	2.88
7	8.61	2.87
8	8.61	2.87

READY.

# EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

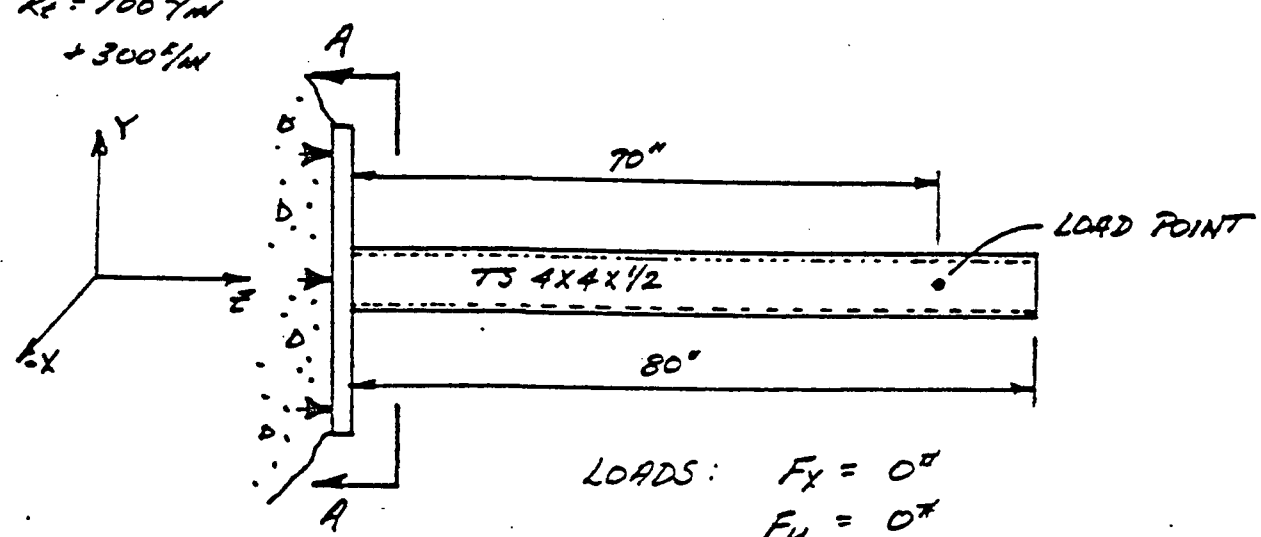
DS-C1.7.1

COMP. TEC. TBR DATE 1/31/84

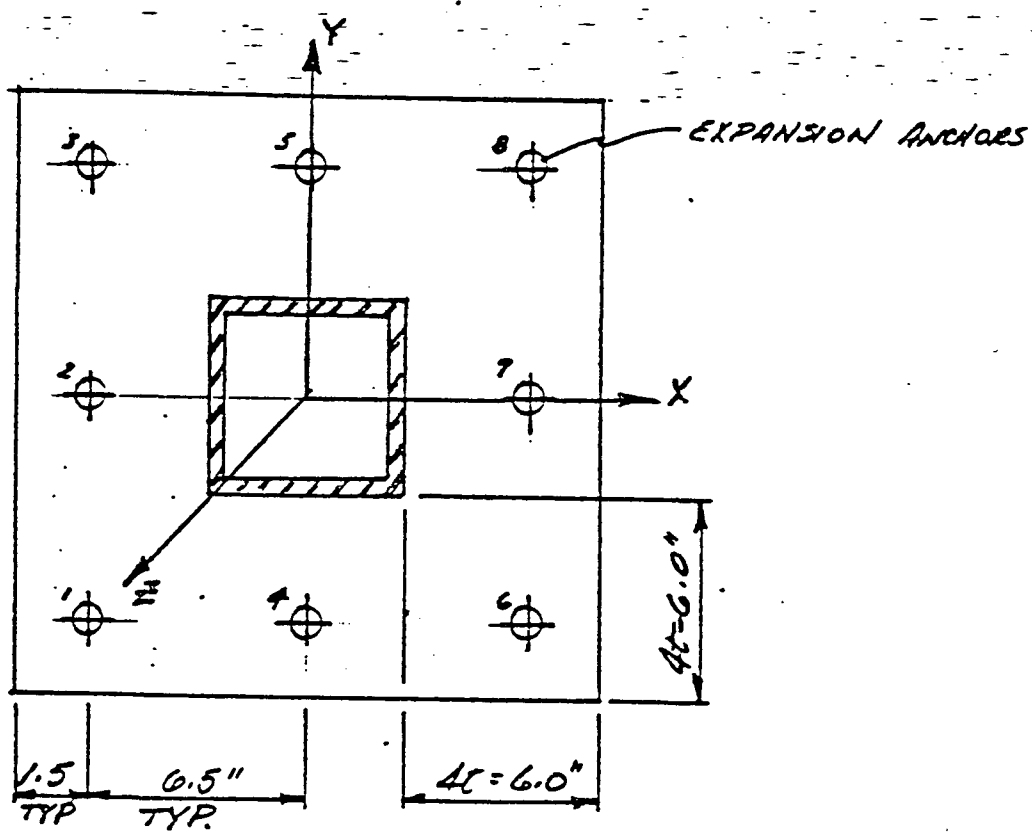
CHECKED SAK DATE 1/31/84

PLATE SIZE - 16" x 16" x 1 1/2"

$K_x = 100 \text{ } \frac{\text{lb}}{\text{in}}$   
 $+ 300 \text{ } \frac{\text{lb}}{\text{in}}$



LOADS:  $F_x = 0 \text{ } \text{lb}$   
 $F_y = 0 \text{ } \text{lb}$   
 $F_z = 26,000 \text{ } \text{lb}$



SECTION A-A

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

DS-C1.7.1

COMPLETED TBR DATE 1/31/84

CHECKED CJK DATE 1/31/84

SUMMARY OF RESULTS  
FOR PLATE SIZE 16x16x1 1/2

ANCHOR LOADS

ANCHOR NUMBER	RIGID KIPS	FLEXIBLE, KIPS	
		$K_f = 100\%/W$	$K_f = 300\%/W$
1	3.25	3.08	2.77
2	3.25	3.42	3.73
3	3.25	3.08	2.77
4	3.25	3.43	3.74
5	3.25	3.43	3.74
6	3.25	3.08	2.76
7	3.25	3.42	3.73
8	3.25	3.08	2.76

READY.  
LIB

EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS

84/01/28. 10.17.41.  
PROGRAM PAPIA

COMPUTED TBR DATE 1/23/84  
CHECKED PA DATE 1/31/84

00100 1.4,UN,0  
00110 8.9,NO,1.5  
00120 16.16,1.1  
00130 8  
00140 SD,.75,1.5,1.5  
00150 SD,.75,1.5,8.0  
00160 SD,.75,1.5,14.5  
00170 SD,.75,8.0,1.5  
00180 SD,.75,8.0,14.5  
00190 SD,.75,14.5,1.5  
00200 SD,.75,14.5,8.0  
00210 SD,.75,14.5,14.5  
00220 4.4,8.0,8.0  
00230 0.0,26.0,0.0  
READY.

EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS

WIDTH OF PLATE = 16 INCHES  
LENGTH OF PLATE = 16 INCHES  
MODULAR RATIO = 3

COMPUTED TBR DATE 1/23/84

CHECKED SH DATE 1/31/84

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	8
3	.334	1.5	14.5
4	.334	8	1.5
5	.334	8	14.5
6	.334	14.5	1.5
7	.334	14.5	8
8	.334	14.5	14.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1

VERT LOAD = 26 KIPS LOCATION X = 8 INCHES Y = 8 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 0 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 0 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 26 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 0 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 0 KIP FEET

UPLIFT CONDITION ON ANCHORS

ANCHOR NUMBER	STRESS KSI	TOTAL FORCE KIPS
1	9.73	3.25
2	9.73	3.25
3	9.73	3.25
4	9.73	3.25
5	9.73	3.25
6	9.73	3.25
7	9.73	3.25
8	9.73	3.25

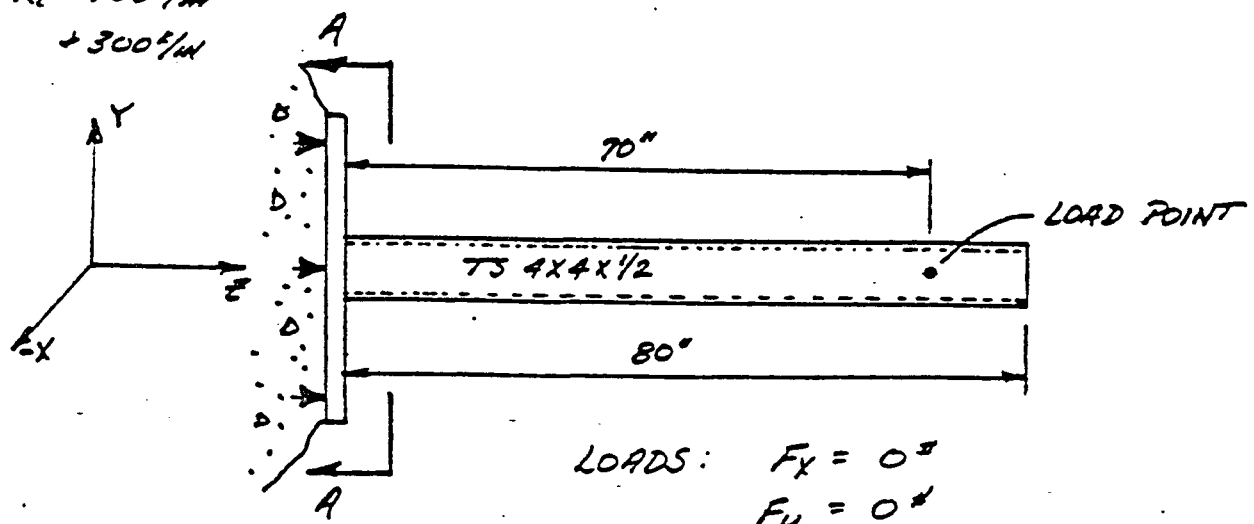
READY.

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

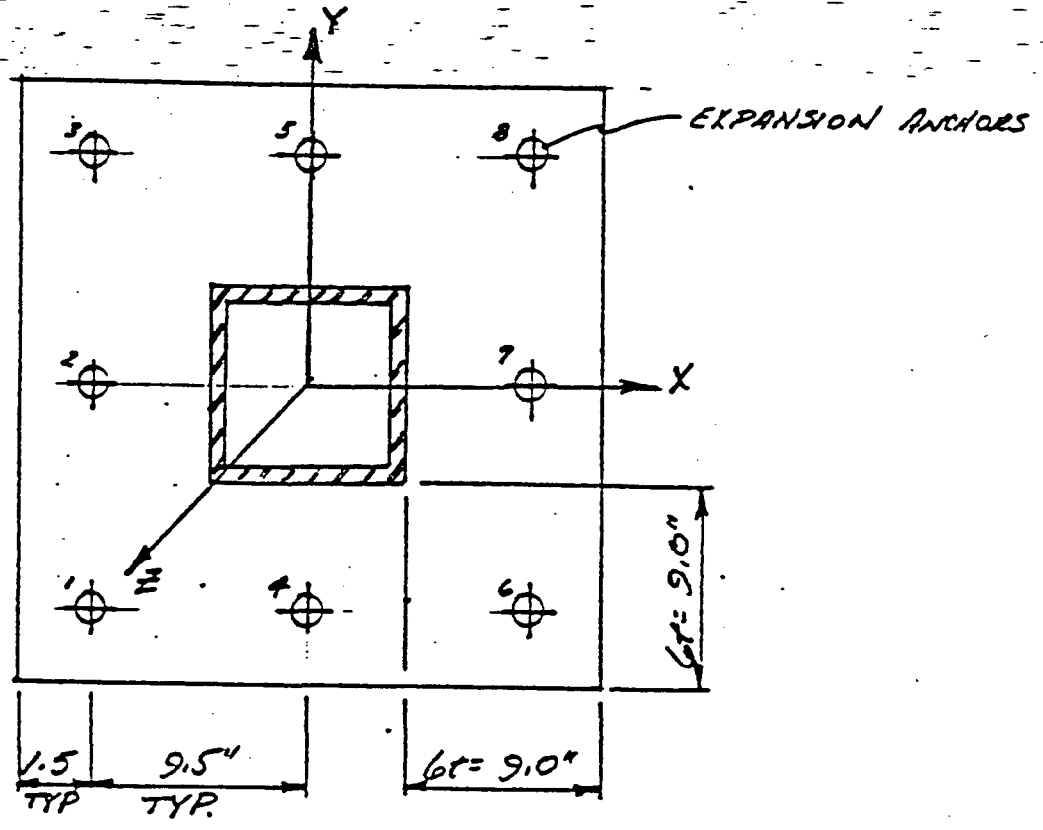
COMPUTED TBR DATE 1/31/8  
 CHECKED PSL DATE 1/31/8

PLATE SIZE - 22" x 22" x 1 1/2"

$K_2 = 100 \text{ k/in}$   
 $+ 300 \text{ k/in}$



LOADS:  $F_x = 0 \text{ k}$   
 $F_y = 0 \text{ k}$   
 $F_z = 30000 \text{ k}$



SECTION A-A

TVA 11030 (WM-7-75)

EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

DS-C1.7.1

COMPUTED TBL DATE 1/31/8  
CHECKED PSL DATE 1/31/8

SUMMARY OF RESULTS  
FOR PLATE SIZE 22x22x1/2

ANCHOR LOADS

ANCHOR NUMBER	RIGID KIPS	FLEXIBLE, KIPS	
		K <sub>r</sub> = 100% <sub>W</sub>	K <sub>r</sub> = 300% <sub>W</sub>
1	3.75	3.25	2.47
2	3.75	4.25	5.02
3	3.75	3.25	2.47
4	3.75	4.26	5.03
5	3.75	4.26	5.03
6	3.75	3.25	2.47
7	3.75	4.25	5.03
8	3.75	3.25	2.47

OLD: SAP22  
READY.  
LIB

84/01/23. 10.21.84.  
PROGRAM SAP22

EVALUATION OF BASEPLATE  
RIGIDITY REQUIREMENTS

00100 1.4, UN, 0  
00110 3.9, ND, 1.5  
00120 22, 22, 1, 1  
00130 3  
00140 SD, .75, 1.5, 1.5  
00150 SD, .75, 1.5, 11.0  
00160 SD, .75, 1.5, 20.5  
00170 SD, .75, 11.0, 1.5  
00180 SD, .75, 11.0, 20.5  
00190 SD, .75, 20.5, 1.5  
00200 SD, .75, 20.5, 11.0  
00210 SD, .75, 20.5, 20.5  
00220 4.4, 11.0, 11.0  
00230 0.0, 30.0, 0.0  
READY.

COMPUTED TOP DATE 1/23/84  
CHECKED SAH DATE 1/31/84



WIDTH OF PLATE = 22 INCHES  
LENGTH OF PLATE = 22 INCHES  
MODULAR RATIO = 9

COMPUTED TOL DATE 1/23/84  
CHECKED JW DATE 1/31/84

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	11
3	.334	1.5	20.5
4	.334	11	1.5
5	.334	11	20.5
6	.334	20.5	1.5
7	.334	20.5	11
8	.334	20.5	20.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1

VERT LOAD = 30 KIPS LOCATION X = 11 INCHES Y = 11 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 0 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 0 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 30 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 0 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 0 KIP FEET

UPLIFT CONDITION ON ANCHORS

ANCHOR NUMBER	STRESS KSI	TOTAL FORCE KIPS
1	11.23	3.75
2	11.23	3.75
3	11.23	3.75
4	11.23	3.75
5	11.23	3.75
6	11.23	3.75
7	11.23	3.75
8	11.23	3.75

READY.

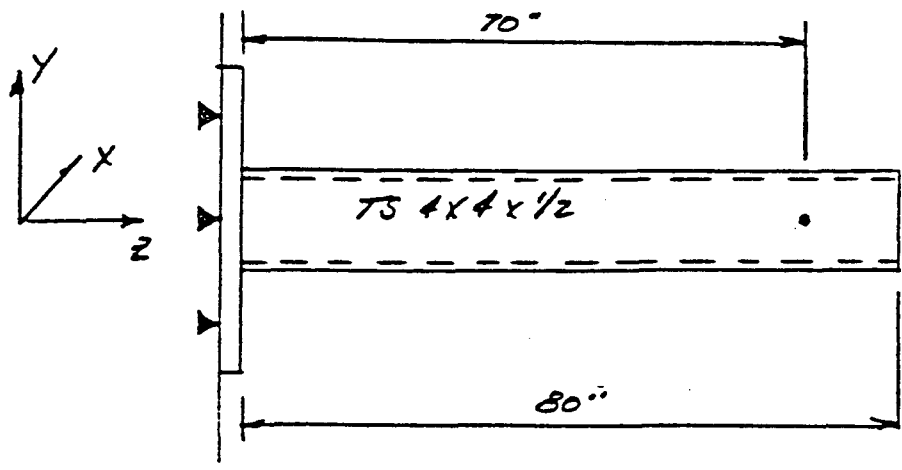
# EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

DS-C1.7.1

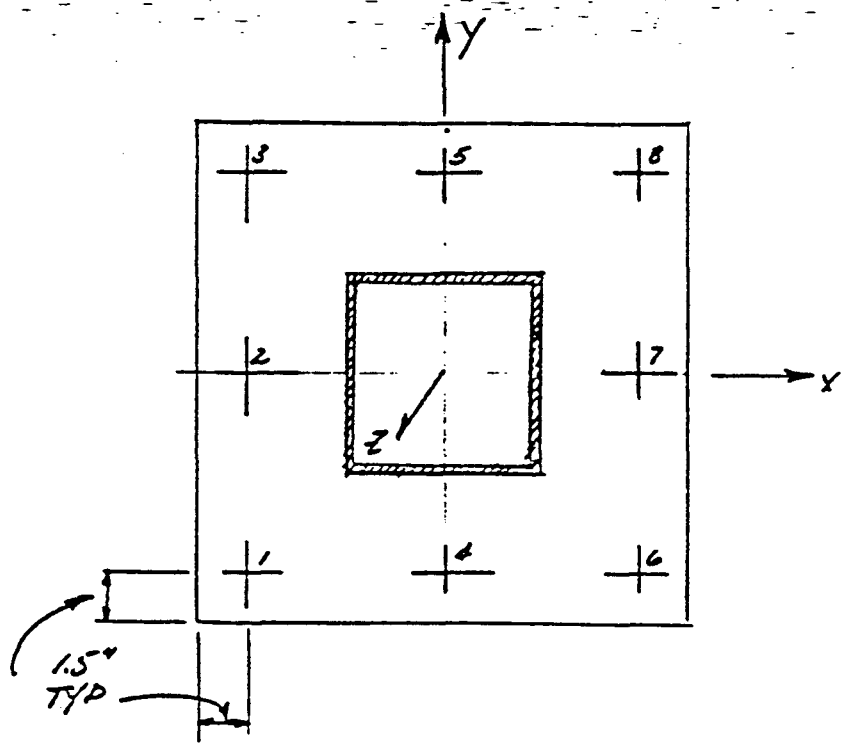
COMPUTED EBL DATE 7/19/83

CHECKED LS DATE 7-30-83

PLATE SIZE 10X10X3/4 W/8-3/4"  $\phi$  SSD ANCHORS LS  
 $K_1 = 100^4/\text{IN} + 300^4/\text{IN}$



LOADS  
 $F_x = 1000 \text{ lb}$   
 $F_y = -1000 \text{ lb}$   
 $F_z = 1000 \text{ lb}$



85/07/19. 10.15.35.  
PROGRAM CASIDOUT

PROGRAM CASIDBP ID 262256 REV. 1 04/84

DATE = 85/07/19.  
FILE = BAP10

TIME = 10.15.07.

SHEET 108 OF 10  
STRUCTURE ENGINEER  
PREPARED TGA DATE 7/22/85  
CHECKED LB DATE 7-30-85

ANCHORS TAKE NO COMPRESSION  
WIDTH OF PLATE = 10 INCHES  
LENGTH OF PLATE = 10 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	5
3	.334	1.5	8.5
4	.334	5	1.5
5	.334	5	8.5
6	.334	8.5	1.5
7	.334	8.5	5
8	.334	8.5	8.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1

VERT LOAD = 1 KIPS LOCATION X = 5 INCHES Y = 5 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 5.83 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 5.83 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 1 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 5.83 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 5.83 KIP FEET

PARTIAL PRESSURE CASE

CONCRETE PRESSURE (KSI)  
C2 = -1.3711

CONCRETE PRESSURE FORCE, CF = -13.16 KIPS  
LOCATION IN X-DIRECTION = 8.1 INCHES  
LOCATION IN Y-DIRECTION = 1.9 INCHES

FRICTION SHEAR CAPACITY, SF = 6.58 KIPS

PRESSURE BULB GEOMETRY

Z3 = 7.5876  
Z6 = 7.5876

ANCHOR NUMBER	STRESS GROSS AREA (KSI)	LOAD KIPS
1	3.9234	1.3104
2	9.6156	3.2116
3	15.3077	5.1128
5	9.6156	3.2116
8	3.9234	1.3104

DS-C1.7.1

# EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

COMPUTED TBL DATE 7/19/85

CHECKED LS DATE 7-30-85

## SUMMARY OF RESULTS FROM PLATE SIZE 10X10X 3/4

ANCHOR NUMBER	RIGID ANAL. KIPS	ANCHOR LOADS FLEXIBLE ANALYSIS, KIPS	
		K <sub>f</sub> =100 <sup>4</sup> /IN	K <sub>f</sub> =300 <sup>4</sup> /IN
1	1.31	1.62	1.42
2	3.21	3.18	3.66
3	5.11	4.44	4.91
4	0	.38	0
5	3.21	3.18	3.66
6	0	0	0
7	0	.38	0
8	1.31	1.62	1.42

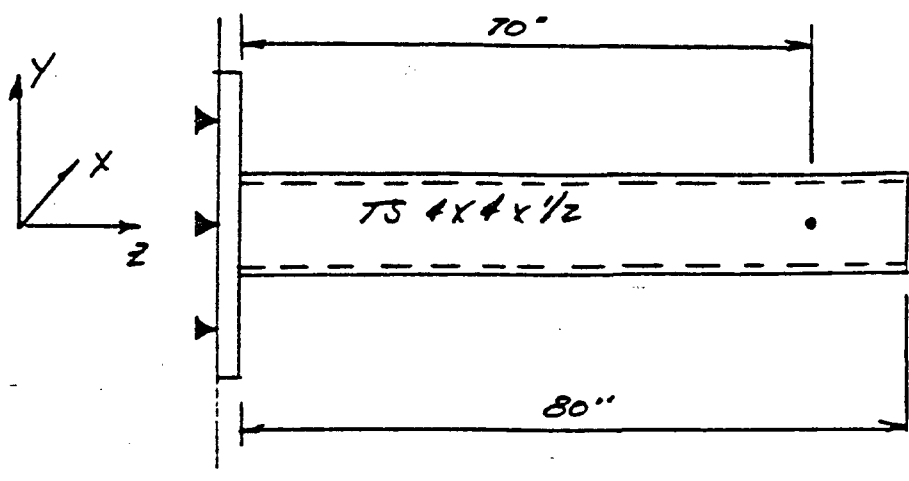
X fac FLEXIBLE ANALYSIS PLATE MODEL SEE CH. 7  
26.

DS-C1.7.1

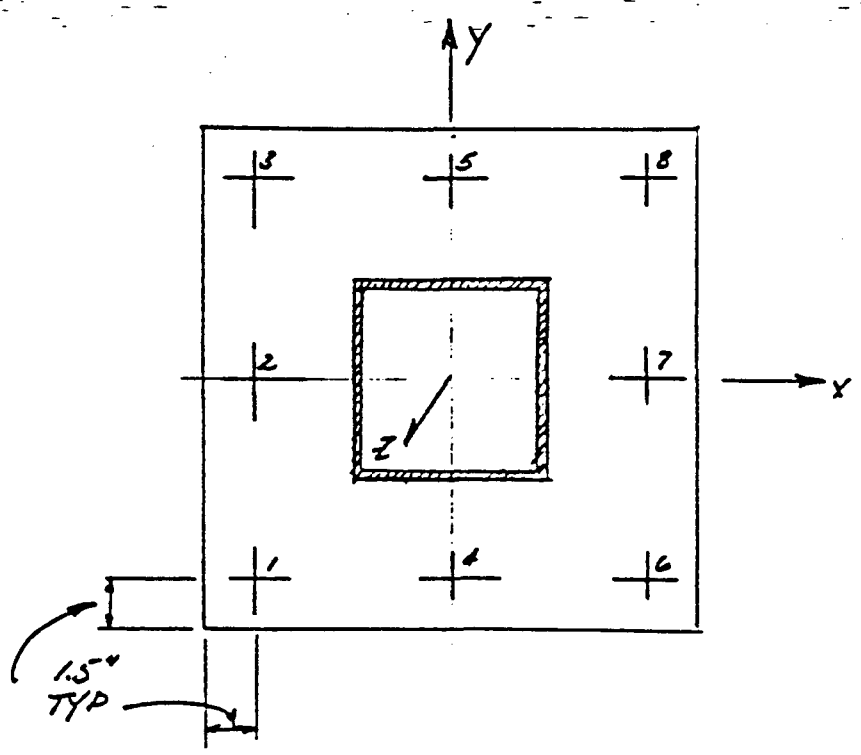
# EVALUATION OF BASE PLATE RIGIDITY REQUIREMENTS

COMPUTED TBL DATE 7/7/85  
CHECKED LS DATE 7-30-85

PLATE SIZE 13 X 13 X 3/4 W/ 8 - 3/4"  $\phi$  SSD ANCHORS  
 $K_L = 100^4 \text{ IN} + 300^4 \text{ / IN}$



LOADS  
 $F_x = 1000 \text{ lb}$   
 $F_y = -1000 \text{ lb}$   
 $F_z = 1000 \text{ lb}$



LIS.F=CASDOUT

85/07/19. 10.18.27.  
PROGRAM CASDOUT

PROGRAM CASDBAP ID 262256 REV. 1 04/84

DATE = 85/07/19.  
FILE = BAP13

TIME = 10.18.00.

SHEET 406 OF 40  
BASE PLATE RIGIDITY  
PREPARED ZAL DATE 2/12/85  
CHECKED LS DATE 2-30-85

ANCHORS TAKE NO COMPRESSION  
WIDTH OF PLATE = 13 INCHES  
LENGTH OF PLATE = 13 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	6.5
3	.334	1.5	11.5
4	.334	6.5	1.5
5	.334	6.5	6.5
6	.334	6.5	11.5
7	.334	11.5	1.5
8	.334	11.5	6.5
		11.5	11.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1

APPLIED LOAD = 1 KIPS LOCATION X = 6.5 INCHES Y = 6.5 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 5.83 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 5.83 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 1 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 5.83 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 5.83 KIP FEET

PARTIAL PRESSURE CASE

CONCRETE PRESSURE (KSI)  
C2 = -.7333

CONCRETE PRESSURE FORCE, CF = -9.37 KIPS  
LOCATION IN X-DIRECTION = 10.81 INCHES  
LOCATION IN Y-DIRECTION = 2.19 INCHES

FRICTION SHEAR CAPACITY, SF = 4.69 KIPS

PRESSURE BULB GEOMETRY

Z3 = 8.7574  
Z6 = 8.7574

ANCHOR NUMBER	STRESS GROSS AREA (KSI)	LOAD KIPS
	3.1973	1.0679
3	6.9653	2.3264
5	10.7333	3.5849
	6.9653	2.3264
8	3.1973	1.0679

DS-C1.7.1

EVALUATION OF BASEPLATE RIGIDITY  
REQUIREMENTS

COMPUTED TBL DATE 7/19/85

CHECKED LS DATE 7-30-85

SUMMARY OF RESULTS  
FROM PLATE SIZE 15X13 X 3/4

ANCHOR NUMBER	RIGID ANAL. KIPS	ANCHOR LOADS *FLEXIBLE ANALYSIS, KIPS	
		$K_f=100^{\circ}/in$	$K_f=300^{\circ}/in$
1	1.07	1.02	.80
2	2.33	2.85	3.61
3	3.58	3.53	3.35
4	0	.18	.07
5	2.33	2.85	3.61
6	0	0	0
7	0	.18	.07
8	1.07	1.02	.80

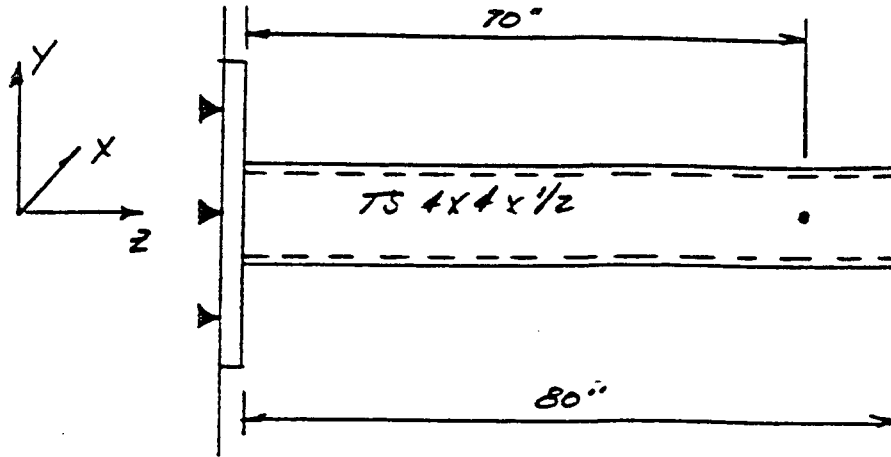
\* FOR FLEXIBLE ANALYSIS (BASED ON) PLATE MODEL  
SEE SHEET 12. LD.

# EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

DS-C.1.7.1

COMPUTED TBE DATE 7/12/85  
 CHECKED LS DATE 7-30-85

PLATE SIZE 16x16x1/2 W/ 8 - 3/4"  $\phi$  550 ANCHORS.  
 $K_1 = 100^4 \text{ IN} + 300^2 \text{ IN}$

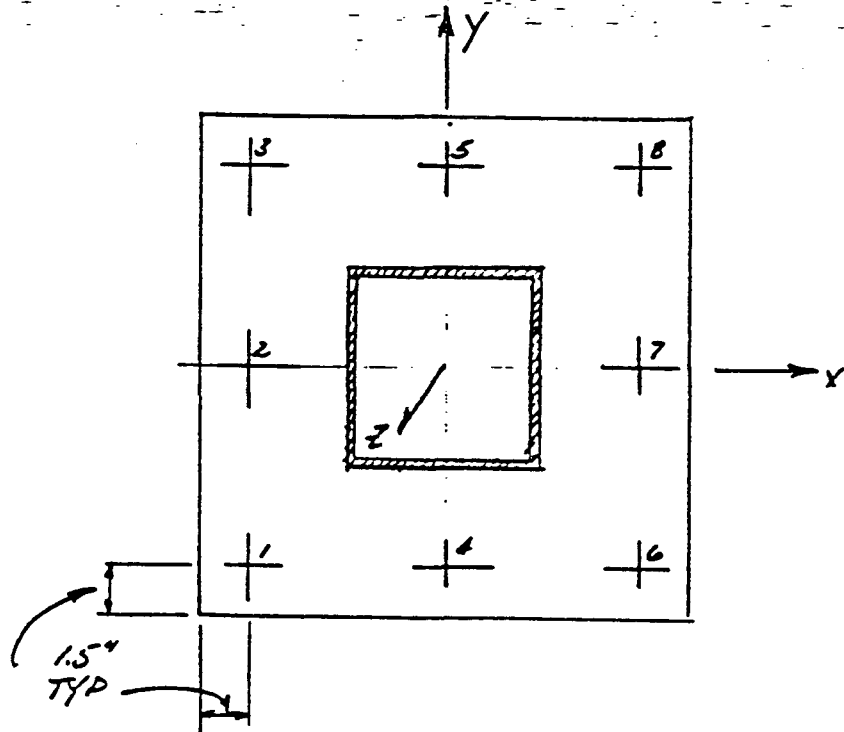


### LOADS

$F_x = 1000 \text{ lb}$

$F_y = -1000 \text{ lb}$

$F_z = 1000 \text{ lb}$





85/07/19. 10.20.50.  
PROGRAM CASDOUT

PROGRAM CASDBAP ID 262256 REV. 1 04/84

DATE = 85/07/19. TIME = 10.20.22.  
FILE = BAP16

SHEET 40H OF 40  
BASELINE PRIORITY  
PREPARED TBC DATE 9/12/85  
CHECKED LS DATE 9-30-85

ANCHORS TAKE NO COMPRESSION  
WIDTH OF PLATE = 16 INCHES  
LENGTH OF PLATE = 16 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	8
3	.334	1.5	14.5
4	.334	8	1.5
5	.334	8	14.5
6	.334	14.5	1.5
7	.334	14.5	8
8	.334	14.5	14.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1  
VERT LOAD = 1 KIPS LOCATION X = 8 INCHES Y = 8 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 5.83 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 5.83 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 1 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 5.83 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 5.83 KIP FEET

PARTIAL PRESSURE CASE

CONCRETE PRESSURE (KSI)  
C2 = -.4601

CONCRETE PRESSURE FORCE, CF = -7.21 KIPS  
LOCATION IN X-DIRECTION = 13.58 INCHES  
LOCATION IN Y-DIRECTION = 2.42 INCHES

FRICTION SHEAR CAPACITY, SF = 3.6 KIPS

PRESSURE BULB GEOMETRY  
Z3 = 9.6943  
Z6 = 9.6943

ANCHOR NUMBER	STRESS GROSS AREA (KSI)	LOAD KIPS
1	2.6932	.8995
2	5.4695	1.8268
3	8.2457	2.7541
5	5.4695	1.8268
8	2.6932	.8995

DS-C1.7.1

# EVALUATION OF BASEPLATE RIGIDITY REQUIREMENTS

COMPUTED TBL DATE 7/19/85

CHECKED LS DATE 7-30-85

## SUMMARY OF RESULTS FROM PLATE SIZE 16 X 16 X 1 1/2

ANCHOR NUMBER	RIGID ANAL. KIPS	ANCHOR LOADS *FLEXIBLE ANALYSIS, KIPS	
		K <sub>f</sub> =100 <sup>5</sup> /IN	K <sub>f</sub> =300 <sup>5</sup> /IN
1	.90	.84	.73
2	1.83	1.82	2.17
3	2.75	2.55	2.83
4	0	.16	0
5	1.83	1.82	2.17
6	0	0	0
7	0	.16	0
8	.90	.84	.73

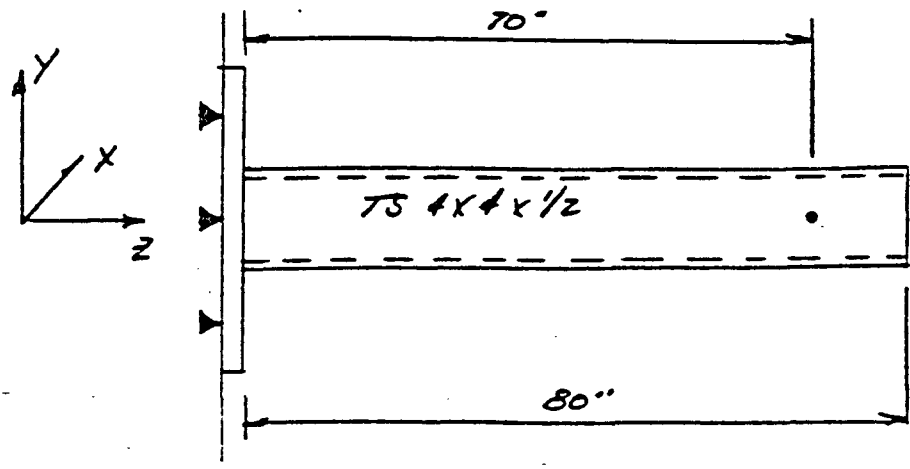
\* FOR FLEXIBLE ANALYSIS (BASEPLATE II) PLATE MODEL  
SEE SHEET 17.  
L.S.

# EVALUATION OF BASE PLATE RIGIDITY REQUIREMENTS

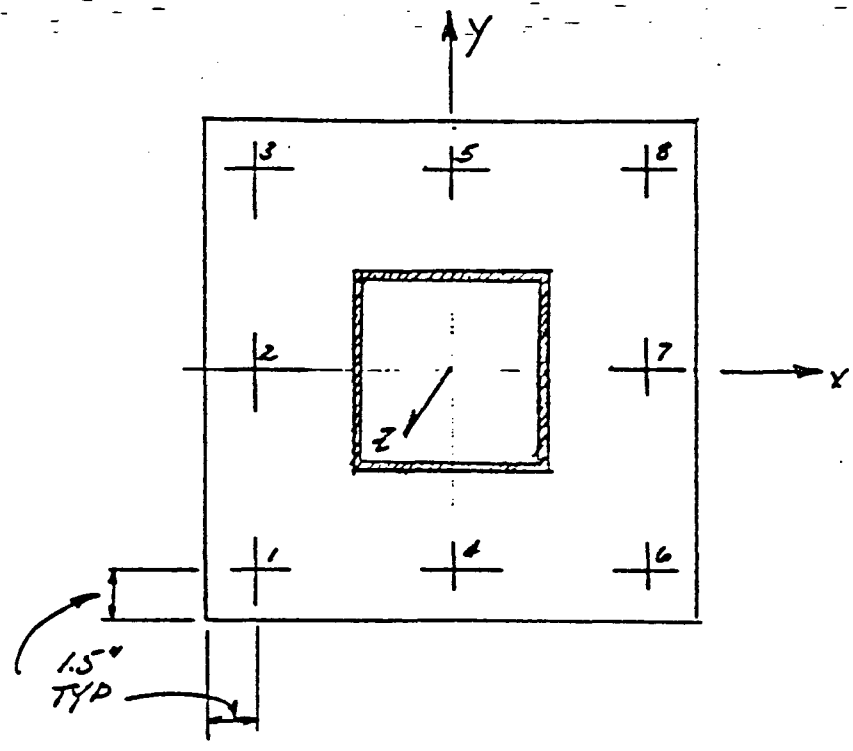
DS-C1.7.1

COMPUTED TBL DATE 7/19/85  
 CHECKED LS DATE 7-30-85

PLATE SIZE 22X22X1 1/2 W/8-3/4"  $\phi$  350 ANCHORS.  
 $K_1 = 100^4/\text{IN} + 300^4/\text{IN}$



LOADS  
 $F_x = 1000 \text{ lb}$   
 $F_y = -1000 \text{ lb}$   
 $F_z = 1000 \text{ lb}$



DATE = 85/07/19.  
FILE = BAP22

TIME = 10.22.37.

SHEET 40K OF 40  
SHEARTE RCI021  
PREPARED TSC DATE 2/12/85  
CHECKED LS DATE 7-30-85

ANCHORS TAKE NO COMPRESSION  
WIDTH OF PLATE = 22 INCHES  
LENGTH OF PLATE = 22 INCHES  
MODULAR RATIO = 9

ANCHOR NUMBER	AREA IN	X ORDINATE IN	Y ORDINATE IN
1	.334	1.5	1.5
2	.334	1.5	11
3	.334	1.5	20.5
4	.334	11	1.5
5	.334	11	20.5
6	.334	20.5	1.5
7	.334	20.5	11
8	.334	20.5	20.5

LOAD CONDITION NUMBER = 1

ATTACHMENT 1

VERT LOAD = 1 KIPS LOCATION X = 11 INCHES Y = 11 INCHES  
APPLIED MOMENT ABOUT Y AXIS = 5.83 KIP FEET  
APPLIED MOMENT ABOUT X AXIS = 5.83 KIP FEET

SUMMATION OF APPLIED LOADS FOR 1 ATTACHMENT(S) = 1 KIP(S)  
TRANSLATED MOMENT CENTERLINE ABOUT Y AXIS = 5.83 KIP FEET  
TRANSLATED MOMENT CENTERLINE ABOUT X AXIS = 5.83 KIP FEET

PARTIAL PRESSURE CASE

CONCRETE PRESSURE (KSI)  
C2 = -.2362

CONCRETE PRESSURE FORCE, CF = -4.97 KIPS  
LOCATION IN X-DIRECTION = 19.19 INCHES  
LOCATION IN Y-DIRECTION = 2.81 INCHES

FRICTION SHEAR CAPACITY, SF = 2.48 KIPS

PRESSURE BULB GEOMETRY

Z3 = 11.2316  
Z6 = 11.2316

ANCHOR NUMBER	STRESS GROSS AREA (KSI)	LOAD KIPS
1	2.0378	.6806
2	3.8356	1.2811
3	5.6334	1.8816
4	.24	.0802
5	3.8356	1.2811
7	.24	.0802
8	2.0378	.6806

DS-C1.7.1

EVALUATION OF BASEPLATE RIGIDITY  
REQUIREMENTS

COMPUTED TBR DATE 7/13/85  
CHECKED LB DATE 7-30-85

SUMMARY OF RESULTS  
FROM PLATE SIZE 22X22X1 1/2

ANCHOR NUMBER	RIGID ANGL. KIPS	ANCHOR LOADS	
		*FLEXIBLE ANALYSIS, KIPS	
		K <sub>f</sub> =100 <sup>2</sup> /W	K <sub>f</sub> =300 <sup>2</sup> /W
1	.68	.52	.40
2	1.28	1.50	1.90
3	1.88	1.95	1.93
4	.08	.06	0
5	1.28	1.50	1.90
6	0	0	0
7	.08	.06	0
8	.68	.52	.40

\* For FLEXIBLE ANALYSIS (BASE PLATE II) PLATE MODEL.  
SEE SHEET 22. LB.