



U.S. NUCLEAR REGULATORY COMMISSION

STANDARD REVIEW PLAN

3.8.3 CONCRETE AND STEEL INTERNAL STRUCTURES OF STEEL OR CONCRETE CONTAINMENTS

REVIEW RESPONSIBILITIES

Primary - Organization responsible for the review of concrete and steel internal structures of steel or concrete containments

Secondary - None

I. AREAS OF REVIEW

The specific areas of review are as follows:

1. Description of the Internal Structures

The descriptive information, including plans and sections of the various internal structures, is reviewed to establish that sufficient information is provided to define the primary structural aspects and elements relied upon to perform the safety-related functions of these structures. To perform safety-related functions, these structures must be capable of resisting loads and load combinations to which they may be subjected and should not become the initiator of a loss-of-coolant accident (LOCA). If such an accident were to occur, the structures should be able to mitigate its consequences by protecting the containment and other engineered safety features from the accident's effects such as jet forces and whipping pipes.

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USNRC STANDARD REVIEW PLAN

This Standard Review Plan, NUREG-0800, has been prepared to establish criteria that the U.S. Nuclear Regulatory Commission staff responsible for the review of applications to construct and operate nuclear power plants intends to use in evaluating whether an applicant/licensee meets the NRC's regulations. The Standard Review Plan is not a substitute for the NRC's regulations, and compliance with it is not required. However, an applicant is required to identify differences between the design features, analytical techniques, and procedural measures proposed for its facility and the SRP acceptance criteria and evaluate how the proposed alternatives to the SRP acceptance criteria provide an acceptable method of complying with the NRC regulations.

The standard review plan sections are numbered in accordance with corresponding sections in the Regulatory Guide 1.70, "Standard Format and Content of Safety Analysis Reports for Nuclear Power Plants (LWR Edition)." Not all sections of the standard format have a corresponding review plan section. The SRP sections applicable to a combined license application for a new light-water reactor (LWR) will be based on Regulatory Guide 1.206, "Combined License Applications for Nuclear Power Plants (LWR Edition)," until the SRP itself is updated.

These documents are made available to the public as part of the NRC's policy to inform the nuclear industry and the general public of regulatory procedures and policies. Individual sections of NUREG-0800 will be revised periodically, as appropriate, to accommodate comments and to reflect new information and experience. Comments may be submitted electronically by email to NRR_SRP@nrc.gov.

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The major containment internal structures that are reviewed, together with the primary structural function of each structure and the extent of descriptive information required for each structure, are indicated below. This SRP section reviews the intervening structural elements between (distribution systems including their supports, e.g., cable trays; conduit; heating, ventilation, and air conditioning; and piping and equipment supports) building structural steel/concrete (e.g., steel platforms, building frame members, embedment plates, and building steel members beyond the jurisdictional boundary of supports to mechanical components).

A. PWR Dry Containment Internal Structures

i. Concrete Supports for Reactor

The pressurized-water reactor (PWR) vessel should be supported and restrained to resist normal operating loads, seismic loads, and loads induced by postulated pipe rupture, including LOCAs. The support and restraint system should restrain the movement of the vessel to within allowable limits under the applicable loading combinations. However, the support system should minimize resistance to the thermal movements expected during operation.

With these functional requirements in mind, the review evaluates the general arrangement and principal features of the reactor vessel supports, with an emphasis on the methods of transferring loads from the vessel to the support and ultimately to the structure and its foundations.

ii. Concrete Supports for Steam Generator

Steam generators should be supported and restrained to resist normal operating loads, seismic loads, and loads induced by pipe rupture. The support system should prevent the rupture of the primary coolant pipes from a postulated rupture in steam or feedwater pipes and vice versa. However, the system should minimize resistance to the thermal movements expected during operation.

With these functional requirements in mind, the review evaluates the general arrangement and principal features of the steam generator supports, with an emphasis on the methods of transferring loads from the vessel to the support and eventually to the structure and its foundations.

iii. Primary Shield Wall and Reactor Cavity

The primary shield wall forms the reactor cavity and usually supports and restrains the reactor vessel. It is often a thick wall that surrounds the reactor vessel and may be anchored through the liner plate to the containment base slab.

The review evaluates the general arrangement and principal features of the wall and cavity, including the main reinforcement and anchorage system.

iv. Secondary Shield Walls

The secondary shield walls surrounding the primary loops form the steam generator compartments and protect the containment from the effects of pipe rupture accidents inside the compartment. These walls may also support intermediate floors and the operating floor. The review evaluates the general arrangement and principal features of these walls, with an emphasis on the method of structural framing and expected behavior under compartment pressure loads and jet forces, particularly those associated with a LOCA.

v. Other Interior Structures

The review also evaluates other major interior structures of PWR dry containments in a similar manner, including the concrete refueling pool walls, **refueling water storage tank (if applicable)**, the operating floor, other intermediate floors **and platforms**, and the polar crane supporting elements.

B. PWR Ice-Condenser Containment Internal Structures

The following elements, in addition to the applicable structures reviewed in dry PWR containments, are reviewed for PWR plants using an ice-condenser containment system:

i. The Divider Barrier

In the PWR ice-condenser containment system, which uses the pressure-suppression concept, the divider barrier surrounds the reactor coolant system. The upper portion of the divider barrier is nearly surrounded by the ice-condenser, which is bounded by the containment shell on the outside and by the divider barrier wall on the inside. Several venting doors connect the space inside the divider barrier to the ice-condenser.

In the event of a LOCA, the divider barrier will contain the steam released from the reactor coolant system and, while temporarily acting as a pressure-retaining envelope, will channel the steam through the venting doors and into the ice-condenser. The ice will condense the steam, thus minimizing the energy released to the containment.

Following such a LOCA and before blowdown is complete, the divider barrier will be subjected to differential pressure and possibly jet forces. Any structural failure in its boundary may result in steam bypassing the ice-condenser and flowing directly into the containment, possibly generating a containment pressure higher than that for which it has been designed.

With this functional requirement in mind, the review evaluates the general arrangement and principal features of the divider barrier with an

emphasis on structural framing and expected behavior when subjected to the design loads.

ii. Ice-Condenser

A major feature of the ice-condenser containment are the baskets of ice forming the heat sink essential for pressure suppression. The structurally significant components of the ice-condenser reviewed are the vent doors, ice baskets, brackets, couplings and lattice framings, lower and upper supports, and insulating and cooling panels.

The review evaluates the general arrangement and principal features of these major components, with an emphasis on the structural framing, supports, and expected behavior when subjected to design loads.

C. BWR Containment Internal Structures

This Standard Review Plan (SRP) section is oriented toward the boiling-water-reactor (BWR) Mark III containment concept. Other BWR containment types are reviewed in a similar manner.

The following major BWR containment internal structures are reviewed, together with the primary structural function of each structure and the extent of descriptive information required for each structure:

i. Drywell

In the BWR containment system, which uses the pressure-suppression concept, the drywell surrounds the reactor coolant system. The lower portion of the drywell is surrounded by the suppression pool which is bounded by the containment shell on the outside and by a weir wall located just inside the drywell wall. A series of vent holes connects the drywell to the suppression pool. In the event of a LOCA, the drywell will contain the steam released from the reactor coolant system and, while temporarily acting as a pressure-retaining envelope, will channel the steam through the vent holes and into the suppression pool. The pool water will condense the steam, thus minimizing the energy released to the containment.

Following such a LOCA and before blowdown is complete, the drywell will be subjected to a differential pressure and possibly to jet forces. Any structural failure in its boundary would result in steam bypassing the suppression pool and flowing directly into the containment, possibly generating a containment pressure higher than that for which it was designed.

With this functional requirement in mind, the review evaluates the general arrangement and principal features of the drywell, with an emphasis on structural framing and expected behavior under loads. Because the drywell geometrically resembles, to a certain degree, a containment, the descriptive information reviewed is similar to that reviewed for

containments in Subsection I.1 of SRP Section 3.8.1. The major components of the drywell reviewed, other than the main body of the drywell, include the bottom vent region, the roof and drywell head, and major penetrations.

ii. Weir Wall

The weir wall forms the inner boundary of the suppression pool and is located inside the drywell. It completely surrounds the lower portion of the reactor coolant system. The review evaluates the general arrangement and principal features of the weir wall, with an emphasis on structural framing and behavior under loads.

iii. Refueling Pool and Operating Floor

The refueling pool walls are located on top of the drywell. The outer walls form a rectangular pool that is usually subdivided by two interior crosswalls. The base slab of the pool is common to the drywell roof slab. The pool may be filled continuously with water for shielding purposes during operation.

The review evaluates the general arrangement and principal features of the refueling pool, with an emphasis on structural framing and behavior under loads.

The operating floor is intended to provide laydown space for refueling operations and is usually a combination of reinforced concrete and structural steel framing. The containment walls and the refueling pool walls may support the floor.

The review evaluates the general arrangement and principal features of the operating floor.

iv. Concrete Supports for Reactor and Recirculation Pump

The support systems of the BWR vessel and recirculation pumps have the same functions as the support systems for PWR vessels and pumps and thus are similarly reviewed.

v. Reactor Pedestal

The reactor pedestal is usually a cylindrical structure located below and supporting the reactor vessel, which is anchored to the top of the pedestal. The review evaluates the general arrangement and principal features of the reactor pedestal, with an emphasis on structural framing, main reinforcement, and the manner in which the pedestal is anchored to the containment base slab.

vi. Reactor Shield Wall

The reactor shield wall is usually a cylindrical wall surrounding the reactor vessel for radiation shielding purposes. It is supported on the reactor pedestal. The wall may be lined on both surfaces with steel plates which also may act as the main structural components of the wall. In addition, the wall may be used as an anchor for pipe restraints.

The review evaluates the general arrangement and principal features of the reactor shield wall, with particular emphasis on structure framing and behavior under loads.

vii. Other Interior Structures

The review evaluates the other major interior structures constructed of reinforced concrete or structural steel, or combinations thereof, including the floors located inside the drywell and in the annulus between the drywell and the containment and the polar crane supporting elements. The general arrangement and principal features of these structures are reviewed.

Some recent applications (e.g., AP600 and AP1000) also use modular construction methods for the major containment internal structures. Wall modules are typically constructed from large, prefabricated sections of steel plates spaced apart with intermittent steel members, joined with other modules at the site, and then filled with concrete. The concrete fill used in wall modules could be structural concrete with reinforcement (composite construction) or fill concrete of low strength without reinforcement, or heavy concrete for radiation shielding. Floor modules consist of prefabricated steel members and plates and are combined with poured concrete to create a composite section. In view of the new application of modules to nuclear power plants, the structural module design, fabrication, configuration, layout, and connections will be reviewed on a case-by-case basis.

2. Applicable Codes, Standards, and Specifications

The review evaluates the information pertaining to design codes, standards, specifications, and regulatory guides, as well as industry standards, that is applied in the design, fabrication, construction, testing, and surveillance of the containment structures. The specific editions, dates, or addenda identified for each document are also reviewed.

3. Loads and Loading Combinations

The review evaluates the information pertaining to the applicable design loads and associated load combinations. The loads normally applicable to containment internal structures include the following:

- A. Loads encountered during construction of containment internal structures, including dead loads, live loads, prestress loads, temperature, wind, earth pressure, snow, rain, and ice, and construction loads that may be applicable, such as material loads, personnel and equipment loads, horizontal construction loads, erection and fitting forces, equipment reactions, and form pressure.

- B. Loads encountered during normal plant startup, operation, and shutdown, including dead loads, live loads, thermal loads from operating temperature, and hydrostatic loads such as those in refueling and pressure suppression pools. In addition, hydrodynamic loads resulting from actuation of safety relief valves (SRVs) and manifested as drag load, jet impingement, and/or pressure loads should be considered. The appendix to SRP Section 3.8.1 includes a further description of the loads associated with SRVs.
- C. Loads to be sustained during severe environmental conditions, including those induced by the operating-basis earthquake (OBE) specified for the plant site. Subsection II.3.A of this SRP defines the condition for which the OBE load is required for design of containment internal structures.
- D. Loads to be sustained during extreme environmental conditions, including those induced by the safe-shutdown earthquake (SSE) specified for the plant site.
- E. Loads to be sustained during abnormal plant conditions. The design-basis LOCA is the most critical abnormal plant condition during which most of the containment internal structures have to perform their primary function. Ruptures of other high-energy pipes should also be considered. Time-dependent and dynamic loads induced by such accidents include elevated temperatures and differential pressures across compartments, jet impingement, impact forces associated with the postulated ruptures of piping, and loads applicable to some structures, such as drag forces in the PWR ice-condenser containment. In addition, for structures or structural components located in or above the suppression pools of BWR containments, the review should consider the applicable LOCA-related and SRV-related hydrodynamic loads manifested as jet loads and/or pressure loads. The appendix to SRP Section 3.8.1 further describes the loads associated with LOCAs.

The various combinations of the above loads that are normally postulated and reviewed include construction loads, normal operating loads, normal operating loads with severe environmental loads, normal operating loads with extreme environmental loads, normal operating loads with abnormal loads, normal operating loads with severe environmental and abnormal loads, and normal operating loads with extreme environmental and abnormal loads.

4. Design and Analysis Procedures

The review evaluates the design and analysis procedures used for the containment internal structures, with an emphasis on the extent of compliance with the applicable codes as indicated in Subsection II.2 of this SRP. The review includes the design and analysis procedures applicable to the following areas:

- A. PWR Dry Containment Internal Structures
 - i. Concrete Supports for Reactor Coolant System

The support system for the reactor vessel and steam generators, as described in Subsection I of this SRP section, should be designed to

resist various combinations of loadings, including normal operating loads, seismic loads, and loss-of-coolant and other pipe rupture accident loads.

SRP Section 3.7.2 describes the analytical procedures for determining seismic loads.

After the procedures for determining individual loads and combinations thereof are reviewed, the design and analysis methods used for the supports are considered, including the type of analysis, the methods of load transfer, and the assumptions of boundary conditions.

ii. Primary Shield Wall and Reactor Cavity

The primary shield wall should withstand all applicable loads, including those transmitted through the reactor supports. The wall is subjected to most of the loads described in Subsection I.3 of this SRP section and should be designed and analyzed for all applicable load combinations. During normal plant operation, the attenuation heat of gamma and neutron radiation originating from the reactor core generates a thermal gradient across the wall. Insulation and cooling systems may be provided to reduce the severity of this gradient by limiting the rise in temperature to an acceptable level.

Procedures for determining seismic loads on the primary shield wall are reviewed in accordance with SRP Section 3.7.2.

LOCA loads applicable to the primary shield wall include a different pressure created across the reactor cavity by a pipe break in the vicinity of the reactor nozzles. Such a transient pressure may act on the entire cavity or on portions of the cavity. Procedures for determining such pressures are reviewed in accordance with guidance provided in SRP Section 6.2.1.

Other LOCA loads that apply are those transmitted to the wall through the reactor supports, including pipe rupture reaction forces which may induce simultaneous shear forces, torsional moments, and bending moments at the base of the wall. The elevated temperature within and around the primary shield created by the accident may also produce transient thermal gradients across the thick wall. The review evaluates these design and analysis procedures accordingly.

iii. Secondary Shield Walls

The secondary shield walls surrounding the primary loops and supporting the operating floor should be designed for loads similar to those applicable to the primary shield wall, including loads of fluid jets from a postulated break of a primary pipe which can impinge on them. The analytical techniques used for these walls are reviewed, including their structural framing and behavior under loads. When elasto-plastic behavior is assumed and the ductility of the walls is relied upon to absorb the energy associated with jet loads, the review evaluates the procedures

and assumptions, with particular emphasis on such areas as modeling techniques, boundary conditions, force-time functions, and assumed ductility. For the time-dependent differential pressure, however, elastic behavior is required, and the review considers the methods of determining an equivalent static load accordingly.

iv. Other Interior Structures

Many of the other interior structures reviewed are combinations of slabs, walls, beam, and columns classified as Category I structures. These structures are subjected to most of the loads and load combinations described in Subsection I.3 of this SRP section. The review evaluates the analytical techniques for these structures on the same basis as the review of the structures described above.

B. PWR Ice-Condenser Containment Internal Structures

i. Divider Barrier

Because the divider barrier has to maintain a certain degree of leaktightness during a LOCA and is thus a critical structure with respect to the proper functioning of the containment, it is treated on the same basis as the containment.

The loads that usually govern the design of the divider barrier are those induced by the LOCA, including the time-dependent differential pressure across the barrier and any concurrent concentrated jet impingement loads. Because the divider barrier is typically a combination of walls and slabs framed together, the design and analysis procedures are conventional. The review evaluates them accordingly, with an emphasis on the assumed boundary conditions and behavior under loads. Since the differential pressure and jet impingement loadings are dynamic impulsive loads that vary with time, the review considers the techniques used to determine their equivalent static loads.

ii. Ice-Condenser

The design of the ice-condenser and its various components may be based on a combination of analysis and testing. The review includes the analytical and testing procedures for the ice baskets and brackets (couplings), the lattice frames and columns, including attachments; the supporting structures comprising the lower supports; the wall panels; and cooling duct and supports of various auxiliary components.

The ice-condenser and its components should be analyzed or tested for various loads and combinations thereof, including dead and live loads, thermal loads induced by differential thermal expansion within the various elements, seismic loads, and loads induced by a LOCA. Accident loads include pressure differential drag loads and loads induced by the change of momentum of the flowing steam.

Elastic analysis is usually used for the ice-condenser and its components. However, plastic analysis may also be used as an alternate approach. Accordingly, the review evaluates the load factors that are applied to each of the applicable loads and their basis and justification.

When experimental verification of the design using simulated load conditions is employed, the review evaluates the procedures used to account for similitude relationships which exist between the actual component and the test model to ensure that the results obtained from the test are a conservative representation of the load-carrying capability of the actual component under the postulated loading.

C. BWR Containment Internal Structures

This SRP section is oriented toward the BWR Mark III containment concept. Other BWR containment types are reviewed in a similar manner.

i. Drywell

The drywell, which has to maintain a certain degree of leaktightness during a LOCA, is critical to the proper functioning of the containment. Because it geometrically resembles a containment, the design and analysis procedures used for the drywell are reviewed on a basis similar to that used to review containments as described in Subsection I.4 of SRP Sections 3.8.1 and 3.8.2 for concrete and steel portions, respectively.

ii. Weir Wall

One of the major loads to which the weir wall may be subjected is a jet impingement load induced by a pipe rupture in a nearby recirculation loop. Under such a concentrated load, the weir wall should not deform to an extent that it might impair or degrade the pressure-suppression performance. Accordingly, the review evaluates the procedures used to analyze the wall for such dynamic time-dependent loads with particular emphasis on modeling techniques, assumptions on boundary conditions, and behavior under loads.

iii. Refueling Pool and Operating Floor

The refueling pool is assumed to be continuously filled with water to provide biological shielding above the reactor. The operating floor, which may be supported on the walls of the refueling pool on one side and on the containment shell on the other side, is a combination of reinforced concrete and structural steel. The design and analysis procedures for the refueling pool and the operating floor are conventional and are reviewed accordingly, with particular emphasis on the structural framing and behavior under loads. In cases in which the floor beams are supported vertically on the containment shell, they should be laterally isolated to minimize interaction between the containment and its interior.

iv. Concrete Supports for Reactor and Recirculation Pump

The design and analysis procedures used for the reactor and recirculation pump supports are reviewed in a manner similar to that used for the PWR reactor and reactor coolant system supports, as already described in this SRP section.

v. Reactor Pedestal

The reactor pedestal supports the reactor and must withstand the loads transmitted through the reactor supports. It is thus subjected to most of the loads described in Subsection I.3 of this SRP section and is designed and analyzed for all applicable load combinations.

Because of the similarity in geometry and function of the BWR reactor pedestal to the PWR primary shield wall, their design and analysis procedures are similar. Hence, the review evaluates the reactor pedestal using the method previously discussed in this SRP section.

vi. Reactor Shield Wall

This cylindrical wall, which surrounds the reactor and provides biological shielding, is also subjected to most of the loads described in Subsection I.3 of this SRP section. In most cases, the wall is used to anchor pipe restraints in the vicinity of the reactor nozzles. A pipe rupture in this area may pressurize the space within the wall. The wall is usually lined on both faces with steel plates which may constitute the major structural elements relied upon to resist the design loads.

The review evaluates the analytical and design techniques used to determine the effect of the design loads on the wall with particular emphasis on the assumed boundary conditions and the behavior of the wall under loads.

vii. Other Interior Structures

Several platforms exist within the BWR containment, some of which are inside the drywell while others are outside the annulus between the drywell and the containment. Platforms inside the drywell are usually constructed of structural steel, and their main structural function is to provide foundations for the pipe restraints inside the drywell. Platforms outside the drywell are usually constructed of a combination of steel and concrete and must be designed to resist the various applicable loads, particularly the effects of pool swell during a LOCA and/or SRV actuation. The analytical procedures for determining pool swell loads are reviewed in accordance with guidance provided in SRP Section 6.2.1. The review evaluates the design and analysis procedures for these platforms, with particular emphasis on the framing and structural behavior under loads.

D. Design Reports

The applicant's design report, as described in Appendix C to SRP Section 3.8.4, is reviewed.

E. Structural Audit

A structural audit, as described in SRP Section 3.8.4, Appendix B, is conducted.

5. Structural Acceptance Criteria

The review evaluates the design limits imposed on the various parameters that quantify the structural behavior of the various interior structures of the containment, particularly with respect to stresses, strains, deformations, and factors of safety against structural failure, with emphasis on the extent of compliance with the applicable codes indicated in Subsection II.5 of this SRP.

6. Materials, Quality Control, and Special Construction Techniques

The review evaluates the information provided on the materials that are used in the construction of the containment internal structures. Concrete ingredients, reinforcing bars and splices, structural steel, and various supports and anchors are among the major materials of construction reviewed.

The review evaluates the quality control program proposed for the fabrication and construction of the containment internal structures, including nondestructive examination of the materials to determine physical properties, placement of concrete, and erection tolerances.

Special, new, or unique construction techniques, such as the use of modular construction methods, if proposed, are reviewed on a case-by-case basis to determine their effects on the structural integrity of the completed containment internal structures.

In addition, the following information should be provided:

A. The extent to which the materials and quality control programs comply with American Concrete Institute (ACI) 349, as supplemented by additional guidance provided by Regulatory Guides (RG) 1.142 and 1.199 for concrete and anchors (steel embedments), respectively, and American National Standards Institute/American Institute of Steel Construction (ANSI/AISC) N690-1994 including Supplement 2 (2004), for steel, as applicable.

B. If welding of reinforcing bars is proposed, describe the extent to which the applicant complies with the applicable sections of the American Society of Mechanical Engineers (ASME), Boiler and Pressure Vessel Code (hereafter referred to as Code) Section III, Division 2, Subsection CC, as supplemented with additional guidance provided by RG 1.136. Provide justification for any exceptions.

7. Testing and Inservice Surveillance Programs

For Category I structures inside containment, the review evaluates information on structures monitoring and maintenance requirements.

For containment internal structures, it is important to accommodate inservice inspection of critical areas. The review includes any special design provisions (e.g., sufficient physical access, alternative means for identification of conditions in inaccessible areas that can lead to degradation, remote visual monitoring of high radiation areas) to accommodate inservice inspection of containment internal structures.

Postconstruction testing and inservice surveillance programs for containment internal structures, such as pressure testing of the drywell/wetwell in a BWR containment and periodic examination of inaccessible areas, are reviewed on a case-by-case basis.

The structural integrity test for the drywell of the BWR containment is reviewed in a similar manner to that used to review the containment.

8. Inspection, Test, Analysis, and Acceptance Criteria (ITAAC).

For design certification (DC) and combined license (COL) reviews, the applicant's proposed information on the ITAAC associated with the structures, systems, and components (SSCs) related to this SRP section is reviewed in accordance with SRP Section 14.3, "Inspections, Tests, Analyses, and Acceptance Criteria - Design Certification." The staff recognizes that the review of ITAAC is performed after review of the rest of this portion of the application against acceptance criteria contained in this SRP section. Furthermore, the ITAAC are reviewed to assure that all SSCs in this area of review are identified and addressed as appropriate in accordance with SRP Section 14.3.

9. COL Action Items and Certification Requirements and Restrictions.

COL action items may be identified in the NRC staff's final safety evaluation report (FSER) for each certified design to identify information that COL applicants must address in the application. Additionally, DCs contain requirements and restrictions (e.g., interface requirements) that COL applicants must address in the application. For COL applications referencing a DC, the review performed under this SRP section includes information provided in response to COL action items and certification requirements and restrictions pertaining to this SRP section, as identified in the FSER for the referenced certified design.

Review Interfaces

The listed SRP sections interface with this section as follows:

1. Determination of structures that are subject to quality assurance programs in accordance with the requirements of 10 CFR 50, Appendix B, is performed in accordance with SRP Sections 3.2.1 and 3.2.2. The review of safety-related structures is performed on that basis.

2. Determination of pressure loads from high-energy lines located in safety-related structures is performed in accordance with SRP Section 3.6.1. The loads thus generated are included in the load combination equations of this SRP section.
3. The exclusion of postulated pipe ruptures from the design basis is generally referred to as the “leak before break.” The review of those applications that propose to eliminate consideration of design loads associated with the dynamic effects of pipe rupture is performed in accordance with SRP Section 3.6.3.
4. Determination of loads generated from pressure under accident conditions is performed in accordance with SRP Section 6.2.1. The loads thus generated are included in the load combinations in this SRP section.
5. Distribution systems including their supports (e.g., cable trays; conduit; heating, ventilation, and air conditioning) and equipment supports are reviewed in accordance with SRP Sections 3.9.2 and 3.9.3.
6. The review for quality assurance is coordinated and performed in accordance with SRP Chapter 17.

The acceptance criteria and review procedures are contained in the referenced SRP sections.

II. ACCEPTANCE CRITERIA

Requirements

Acceptance criteria are based on meeting the relevant requirements of the following Commission regulations:

1. 10 CFR 50.55a and 10 CFR 50, Appendix A, General Design Criterion (GDC) 1, as they relate to the design, fabrication, erection, and testing of containment **internal** structures in accordance with quality standards commensurate with the importance of the safety function to be performed.
2. GDC 2, as it relates to the ability of the containment internal structures without loss of capability to perform their safety function, to withstand the effects of natural phenomena, such as earthquakes, tornadoes, floods, and the appropriate combination of all loads.
3. GDC 4, as it relates to the protection of containment internal structures **against dynamic effects, including the effects of missiles, pipe whipping, and discharging fluids, that may result from equipment failures and from events and conditions outside the nuclear power unit.**
4. **GDC 5, as it relates to safety-related structures not being shared among nuclear power units, unless it can be shown that such sharing will not significantly impair their ability to perform their safety functions.**
5. GDC 50, as it relates to the design of containment internal structures with sufficient margin of safety to accommodate appropriate design loads.

6. 10 CFR 50, Appendix B, as it relates to the quality assurance criteria for nuclear power plants.
7. 10 CFR 52.47(b)(1), as it relates to ITAAC (for DC) sufficient to assure that the SSCs in this area of review will operate in accordance with the certification
8. 10 CFR 52.80(a), as it relates to ITAAC (for COL) sufficient to assure that the SSCs in this area of review have been constructed and will be operated in conformity with the license, the provisions of the Atomic Energy Act and the Commission's rules and regulations

SRP Acceptance Criteria

Specific SRP acceptance criteria acceptable to meet the relevant requirements of the NRC's regulations identified above are as follows for review described in Subsection I of this SRP section. The SRP is not a substitute for the NRC's regulations, and compliance with it is not required. However, an applicant is required to identify differences between the design features, analytical techniques, and procedural measures proposed for its facility and the SRP acceptance criteria and evaluate how the proposed alternatives to the SRP acceptance criteria provide acceptable methods of compliance with the NRC regulations.

1. Description of the Internal Structures

The descriptive information in the safety analysis report (SAR) is considered acceptable if it meets the minimum requirements set forth in Section 3.8.3.1 of RG 1.70.

During the application acceptance review, the reviewer identifies deficient areas of descriptive information and initiates a request for additional information. New or unique design features that are not specifically covered in Regulatory Guide 1.70 may require a more detailed review. The reviewer determines whether additional information is required to accomplish a meaningful review of the structural aspects of such new or unique features.

The NRC Regulatory Guide 1.206 provides the basis for evaluating the description of seismic Category I structures included in a COL application.

2. Applicable Codes, Standards, and Specifications

The design, materials, fabrication, erection, inspection, testing, and inservice surveillance, if any, of **containment internal** structures are covered by the following codes, standards, and guides that are applicable either in their entirety or in portions thereof:

Code, Standard, or
Specification

Title

ACI 349

Code Requirements for Nuclear Safety-Related
Concrete Structures (supplemented with additional
guidance by RG 1.142 and 1.199)

ASME Code	Section III, Division 2, Subsection CC, "Code for Concrete Reactor Vessels and Containments"
ASME Code	Section III, Division 1, Subsection NE, "Class MC Components"
ANSI/AISC N690-1994 including Supplement 2 (2004)	Specification for the Design, Fabrication and Erection of Steel Safety-Related Structures for Nuclear Facilities
<u>RG</u>	
1.57	Design Limits and Loading Combinations for Metal Primary Reactor Containment
1.69	Concrete Radiation Shields for Nuclear Power Plants
1.136	Materials, Construction, and Testing of Concrete Containments
1.142	Safety-Related Concrete Structures for Nuclear Power Plants (Other Than Reactor Vessels and Containments)
1.143	Design Guidance for Radioactive Waste Management Systems, Structures, and Components Installed in LWR Plants
1.160	Monitoring the Effectiveness of Maintenance at Nuclear Power Plants
1.199	Anchoring Components and Structural Supports in Concrete

3. Loads and Load Combinations

The loads and load combinations for containment **internal** structures described in Subsection I.1 of this SRP are acceptable if they are consistent with the guidance given below. The loads and load combinations for the divider-barrier and ice-condenser elements of the ice-condenser PWR containment and the drywell of the BWR containment are presented following the general criteria given for concrete and steel structures.

A. **Concrete Structures**

All loads and load combinations are to be in accordance with ACI 349 and RG 1.142. Supplemental criteria on the use of loads and load combinations are presented below.

Dead loads include hydrostatic loads, and, for equipment supports they include static and dynamic head and fluid flow effects.

Live loads include any movable equipment loads and other loads that vary with intensity and occurrence. For equipment supports, they also include loads caused by vibration and any support movement effects. Alternate load cases in which the magnitudes and locations of the live loads are arranged so that worst-case conditions are included in the design should be investigated, as appropriate.

As per 10 CFR 50, Appendix S, the OBE is only associated with plant shutdown and inspection unless the applicant specifically selects it as a design input. If the OBE is set at one-third or less of the SSE ground motion, an explicit response or design analysis is not required. If the OBE is set at a value greater than one-third of the SSE, an analysis and design must be performed to demonstrate that the containment internal structures remain functional and are within applicable stress, strain, and deformation limits. SRP Section 3.7.1 and 3.7.2 provides further guidance on the use of OBE.

For structures or structural components subjected to hydrodynamic loads resulting from LOCA and/or SRV actuation, such loads should be considered as indicated in the appendix to SRP Section 3.8.1. Fluid structure interaction associated with these hydrodynamic loads and those from earthquakes should be taken into account.

The design of concrete structures must consider the loads and load combinations that may occur during their construction. These loads consist of dead loads, live loads, temperature, wind, snow, rain, and ice. Applicable construction loads include material loads, personnel and equipment loads, horizontal construction loads, erection and fitting forces, equipment reactions, and form pressure. Structural Engineering Institute (SEI)/ASCE Standard 37 provides additional guidance on construction loads. This standard may be used for supplemental guidance. When the standard and the Code/SRP provide conflicting criteria, the criteria provided in Code/SRP governs.

B. Steel Structures

All loads and load combinations are to be in accordance with ANSI/AISC N690-1994 including Supplement 2 (2004). This specification uses the allowable stress design (ASD) method. Use of the load and resistance factor design (LRFD) version of the specification (N690L) is reviewed on a case-by-case basis. The supplemental criteria on the use of loads and load combinations presented above for concrete structures also apply to steel structures.

C. Divider Barrier and Ice-Condenser of the PWR Ice-Condenser Containment

Specific load and load combination criteria applicable to the divider barrier and ice-condenser elements are given below. Supplemental criteria presented in Subsection II.3.A of this SRP section are also applicable.

i. Divider Barrier

Because the structural integrity of the divider barrier and, to a certain extent, its leaktight integrity are important to the proper functioning of the ice-condenser containment system, it is treated, for design purposes, in a manner similar to the containment itself. Accordingly, for concrete pressure-resisting portions of the divider barrier, the loads and load combinations of Article CC-3000 of the ASME Code, Section III, Division 2, with additional guidance provided by applicable portions of SRP Section 3.8.1 and RG 1.136.

For other concrete portions of the divider barrier, the loads and load combinations as defined in Subsection II.3.A apply.

Steel portions of the divider barrier that resist the design differential pressure and are not backed by concrete, such as penetrations, hatches, locks, and guard pipes, should be designed in accordance with the appropriate sections of Subsection NE of the ASME Code, Section III, Division 1, with additional criteria provided by applicable portions of SRP Section 3.8.2 and RG 1.57 apply.

For other steel portions of the divider barrier, the loads and load combinations as defined in Subsection II.3.B apply.

ii. Ice-Condenser Elements

The structural integrity of the ice baskets, ice-bed framing, and their supports is important to the functional integrity of the ice-condenser containment system. Loads and load combinations for the ice-condenser elements are acceptable if found to be in accordance with ANSI/AISC N690-1994 including Supplement 2 (2004). For the ice-condenser, the load P_a is the LOCA pressure load induced by drag and change in the momentum of flowing air and steam.

D. BWR Containment Drywell

This SRP section is oriented toward the BWR Mark III containment concept. Other BWR containment types are reviewed in a similar manner.

Because the structural integrity of the drywell and, to a certain extent, its leaktight integrity are critically important to the proper functioning of the pressure-suppression system, the drywell is treated, for design and testing purposes only, in a manner similar to the containment itself. Accordingly, for the concrete pressure-resisting portions of the drywell, the loads and loading combinations of Article CC-3000 of ASME Code, Section III, Division 2, will apply, with additional criteria provided by applicable portions of SRP Section 3.8.1 and RG 1.136.

For steel components of the drywell that resist pressure and are not backed by concrete, the appropriate sections of Subsection NE of ASME Code, Section III, Division 1, should be used with additional guidance provided by applicable

portions of SRP Section 3.8.2 and RG 1.57. Specifically, the loads and load combinations of Subsection II.3 of SRP Section 3.8.2 apply.

Additional criteria presented in Subsection II.3.A of this SRP section are also applicable to the BWR containment drywell.

For the lower vent portion of the drywell, the following conditions apply:

- i. If the main reinforcement of the drywell is carried down between the vent holes, and the reinforced concrete section is relied upon for structural purposes, the criteria that apply to concrete portions of the drywell as described above will apply.
- ii. If the main reinforcement of the drywell is terminated above the vent holes, and two steel plates lining both faces of the drywell are used for structural purposes, the criteria that apply to steel portions of the drywell as described above will apply.
- iii. If other structural systems are used in the vent region, the loads and load combinations are reviewed and judged on a case-by-case basis.

4. Design and Analysis Procedures

The design and analysis procedures used for the containment internal structures are acceptable if found to be in accordance with the following:

A. PWR Dry Containment Internal Structures

i. Primary Shield Wall and Reactor Cavity

The design and analysis procedures used for the shield wall are acceptable if found to be in accordance with ACI 349 with additional guidance provided by RG 1.142. This code is based on the strength design method. The design and analysis of anchors (steel embedments) used for component and structural supports on concrete structures are acceptable if found to be in accordance with ACI 349, Appendix B, with additional guidance provided by RG 1.199.

Analyses for LOCA loads applicable to the primary shield wall, such as the cavity differential pressure combined with pipe rupture reaction forces, are acceptable if these loads are treated as dynamic time-dependent loads. This requires that either a detailed time-history analysis be performed or a static analysis using the peak of the forcing function amplified by an appropriate chosen dynamic factor be employed. Elastic behavior of the wall should be maintained under the differential pressure. However, for the concentrated accident loads, such as Y_r , Y_j , or Y_m , elasto-plastic behavior may be assumed if the deflections are limited to maintain functional requirements. Simplified methods for determining effective dynamic load factors for elastic behavior are acceptable if found to be in accordance with recognized dynamic analysis methods.

ii. Secondary Shield Walls

Design and analysis procedures used for the secondary shield walls are acceptable if found to be in accordance with conventional beam/slab design and analysis procedures described in ACI 349, with additional guidance provided RG 1.142. The design and analysis of anchors (steel embedments) used for component and structural supports on concrete structures are acceptable if found to be in accordance with ACI 349, Appendix B, with additional guidance provided by RG 1.199.

Similar to the primary shield wall, the secondary shield walls are also subject to dynamic LOCA loads and the methods described in Subsection II.4.A.i are, therefore, applicable and acceptable.

iii. Other Interior Structures

Most of the other interior structures that are reviewed are combinations of reinforced concrete slabs, walls, beams, and columns, and steel beams and columns, which are classified as Category I structures subject to the loads and load combinations described in Subsection II.3 of this SRP section.

Analytical techniques for these structures are acceptable if found to be in accordance with those described in ACI 349, as with additional guidance provided by RG 1.142 and 1.199 for concrete and anchors (steel embedments), respectively, and with ANSI/AISC N690-1994 including Supplement 2 (2004) for steel.

B. PWR Ice-Condenser Containment Internal Structures

i. Divider Barrier

The most important loads that usually govern the design of the divider barrier are those induced by a LOCA, including the differential pressure across the barrier and any concentrated jet impingement loads. Because the structural integrity of the divider barrier and, to a certain extent, its leaktight integrity are important to the proper functioning of the ice-condenser containment system, it is treated, for design purposes, in a manner similar to the containment itself. Accordingly, for concrete pressure-resisting portions of the divider barrier, the design and analysis procedures of Article CC-3000 of the ASME Code, Section III, Division 2, apply with additional guidance provided by applicable portions of SRP Section 3.8.1 and RG 1.136. For the other concrete portions of the divider barrier, the design and analysis procedures are acceptable if found to be in accordance with ACI 349, with additional guidance provided by RG 1.142 and 1.199.

These methods are based on linear elastic design methods unless the structure is subjected to concentrated accident loads, as discussed in Subsection II.4.A.i, in which elasto-plastic behavior may be assumed.

For steel portions of the divider barrier that resist pressure but are not backed by structural concrete, the design and analysis procedures are acceptable if found to be in accordance with the applicable provisions of Subsection NE of the ASME Code, Section III, Division 1 apply, with additional guidance provided by applicable portions of SRP Section 3.8.2 and RG 1.57.

ii. Ice-Condenser Elements

The design and analysis procedures for the ice-condenser and its various components are acceptable if found to be in accordance with either the elastic/linear design method of Part 1 of ANSI/AISC N690-1994 including Supplement 2 (2004), or the plastic design method of Part 2 of the same specifications. For components using experimental testing to verify the design, the testing procedures are acceptable if found to be in accordance with recognized prototype or model testing procedures that consider the effect of scaling and similitude.

C. BWR Containment Internal Structures

This SRP section is oriented toward the BWR Mark III containment concept. Other BWR containment types are reviewed in a similar manner.

i. Drywell

The design and analysis procedures used for concrete portions of the drywell are acceptable if found to be in accordance with Subsection II.4 of SRP Section 3.8.1. For steel portions of the drywell that resist pressure but are not backed by structural concrete, the design and analysis procedures are acceptable if found to be in accordance with the applicable provisions of SRP Section 3.8.2, Subsection II.4.

ii. Weir Wall

One of the major loads to which the weir wall may be subjected is a jet impingement load induced by a pipe rupture in a nearby recirculation loop. The deflection of the wall under such a load must be limited so as not to impair the pressure-suppression performance. The procedures used to analyze the wall for such a dynamic time-dependent load are acceptable if a detailed time-history dynamic analysis is performed or if an equivalent static analysis is performed using the peak of the jet load amplified by an appropriately chosen dynamic load factor. The design and analysis procedures for concrete weir walls are acceptable if found to be in accordance with conventional methods described in ACI 349, with additional guidance provided by RG 1.142 and 1.199, for concrete and anchors (steel embedments), respectively.

iii. Refueling Pool and Operating Floor

The refueling pool and the operating floor, which may be supported on the walls of the refueling pool on one side and on the containment shell

on the other side, are constructed of a combination of reinforced concrete and structural steel. The design and analysis procedures are acceptable if found to be in accordance with conventional methods described in ACI 349, with additional guidance provided by RG 1.142 and 1.199, for concrete and anchors (steel embedments), respectively, and in ANSI/AISC N690-1994 including Supplement 2 (2004) for structural steel.

iv. Supports for Reactor

The support system for the reactor vessel, described in Subsection I of this SRP section, should be designed to resist various combinations of loadings as indicated in Subsection II.3 of this SRP section. Among the major loads that should be considered are normal operating loads, seismic loads, and LOCA loads.

The design and analysis procedures used for the reactor supports (beyond the jurisdictional boundary of the ASME-designed supports) are acceptable if found to be in accordance with the same criteria for concrete and steel that apply to the refueling pool and operating floor.

v. Reactor Pedestal

The reactor pedestal, which supports the reactor and must withstand the loads transmitted through the reactor supports, should be subjected to most of the loads described in Subsection II.3 of this SRP section and should be designed for all applicable load combinations.

The design and analysis procedures used for the reactor pedestal are acceptable if found to be in accordance with the same criteria for concrete applicable to the refueling pool and operating floor.

vi. Reactor Shield Wall

This cylindrical wall, which surrounds the reactor and provides biological shielding, should be subjected to most of the loads described in Subsection II.3 of this SRP section. In many cases, the wall is used to anchor most of the pipe restraints placed around the reactor coolant system piping. A pipe rupture in the vicinity of the reactor nozzles may pressurize the space within the wall. The wall may be lined on both faces with steel plates which may constitute the major structural elements relied upon to resist the design loads. Like the reactor pedestal, the biological shield wall is also subjected to dynamic LOCA loads and the same methods are, therefore, applicable and acceptable.

The design and analysis procedures used for the reactor shield wall are acceptable if found to be in accordance with the same criteria for concrete that apply to the refueling pool and operating floor. If the shield wall is constructed from steel plates filled with unreinforced concrete, then the design and analysis procedures are reviewed on a case-by-case basis.

vii. Miscellaneous Platforms

Platforms inside the drywell are usually constructed of structural steel and their main structural function is to provide foundations for the pipe restraints inside the drywell. Platforms outside the drywell are usually combinations of steel and concrete.

The design and analysis procedures used for miscellaneous platforms are acceptable if found to be in accordance with the same criteria for concrete and steel that apply to the refueling pool and operating floor. Of particular interest are the dynamic loads induced on these floors by pool swell during a LOCA.

- D. For all containment internal structures, the design and analysis methods described in Subsections II.4 of SRP Sections 3.8.1 and 3.8.2, which are applicable to the containment internal concrete and steel structures, respectively, also need to be considered. These items include assumptions on boundary conditions, axisymmetric and nonaxisymmetric loads, transient and localized loads, shrinkage and cracking of concrete, computer programs, and evaluation of liner plates and anchors.
- E. Design of structures that use modular construction methods are reviewed on a case-by-case basis. NUREG/CR-6486 provides guidance related to the use of modular construction methods. Appendix B to NUREG/CR-6486 includes proposed modular construction review criteria.
- F. A structural design audit is conducted as described in Appendix B to SRP Section 3.8.4.
- G. The applicant's design report is considered acceptable if it satisfies the guidelines of Appendix C to SRP Section 3.8.4.

5. Structural Acceptance Criteria

The structural acceptance criteria for containment internal structures described in Subsection I.1 of this SRP section are acceptable if found to be in accordance with the guidance given below. The acceptance criteria for the divider-barrier and ice-condenser elements of the ice-condenser PWR containment and the drywell of the BWR containment are presented following the criteria given for concrete and steel structures. The structural acceptance criteria for structures that use modular construction methods are reviewed on a case-by-case basis. See Section II.4.E of this SRP section for criteria relating to modular construction.

A. Concrete Structures

ACI 349 and RG 1.142 define the structural acceptance criteria for concrete structures. The structural acceptance criteria for anchors (steel embedments) used for support of systems and components to concrete structures are acceptable if found to be in accordance with Appendix B to ACI 349, with additional guidance provided by RG 1.199.

B. Steel Structures

ANSI/AISC N690-1994 including Supplement 2 (2004) defines the structural acceptance criteria for steel structures. This specification uses the ASD method. Use of the LRFD version of the specification (N690L) is reviewed on a case-by-case basis.

C. Divider Barrier and Ice-Condenser of PWR Ice-Condenser Containment

i. Divider Barrier

For concrete pressure-resisting portions of the divider barrier, the specified limits for stresses and strains are acceptable if found to be in accordance with Subsection CC-3400 of ASME Code Section III, Division 2, with additional guidance provided by applicable portions of SRP Section 3.8.1 and RG 1.136.

For steel portions of the divider barrier that resist pressure but are not backed by structural concrete, the design should be similar to that of steel containments. Accordingly the stress limits are acceptable if found to be in accordance with Subsection NE of the ASME Code, Section III, Division 1, with additional guidance provided by applicable portions of SRP Section 3.8.2 and RG 1.57.

For the other concrete and steel portions of the divider barrier, the specified limits for stresses and strains are acceptable if found to be in accordance with those provided in Subsections II.5.A and B for concrete and steel, respectively.

ii. Ice-Condenser Elements

For load combination delineated in Subsection II.3 of this SRP section, the specified limits for stresses and strain are acceptable if found to be in accordance with those given in ANSI/AISC N690-1994 including Supplement 2 (2004).

D. BWR Containment Drywell

This SRP section is oriented toward the BWR Mark III containment concept. Other BWR containment types are reviewed in a similar manner.

For concrete and steel portions of the drywell, the specified limits for stresses and strain are acceptable if found to be in accordance with the acceptance criteria of item II.5.C.i as described for the divider barrier.

For the lower vent portion of the drywell, the following conditions apply:

- i. If the main reinforcement of the drywell is carried down between the vent holes, and the reinforced concrete section is relied upon for structural purposes, the structural acceptance criteria are the same as for item II.5.C.i above for concrete.

- ii. If the main reinforcement of the drywell is terminated above the vent holes, and two steel plates lining both faces of the wall are used for structural purposes, the acceptance criteria are reviewed on a case-by-case basis.
- iii. If other structural systems are used in the vent region, the acceptance criteria are also reviewed on a case-by-case basis.

6. Materials, Quality Control, and Special Construction Techniques

The specified materials of construction and quality control programs are acceptable if found to be in accordance with the public code or standard as indicated in Subsection I.6 of this SRP section.

Special construction techniques, if any, are treated on a case-by-case basis. For modular construction, the materials, quality control, and special construction techniques are also reviewed on a case-by-case basis. See Section II.4.E of this SRP section for further information.

7. Testing and Inservice Surveillance Requirements

BWR containment drywells, such as those used for the Mark III containment, should be subjected to a structural proof test. Such a test is acceptable if found to be in accordance with the following:

- A. The drywell should be subjected to an acceptance test that increases the drywell internal pressure in three or more approximately equal pressure increments ranging from atmospheric pressure to at least the design pressure. The drywell should be depressurized in the same number of increments. Measurements should be recorded at atmospheric pressure and at each pressure level of the pressurization and depressurization cycles. At each level, the pressure should be held constant for at least 1 hour before the deflections and strains are recorded.
- B. So that the overall deflection pattern can be determined in prototype drywells, radial deflections should be measured at a minimum of three points along each of at least three meridians equally spaced around the drywell, including locations with varying stiffness characteristics. Radial deflections should be measured at the lower vent region, about mid-height, and near the top of the cylindrical design. Measurement points may be relocated, depending on the distribution of stresses and deformations anticipated in each particular design.
- C. In prototype drywells only, strain measurements sufficient to permit an evaluation of strain distribution should be recorded for at least two opposing meridians at the following locations on the wall:
 - i. At the bottom of the wall
 - ii. At mid-height of the wall

These strain measurements should be made at a minimum of three positions within the wall section—one at the center and one each near the inner and outer surfaces.

- D. In nonprototype drywells, deflection and strain measurements need not be made if strain levels have been correlated with deflection measurements during the acceptance test of a prototype drywell when measured strains and deflections are within the predefined tolerance of their predicted responses.
- E. Any reliable system of displacement meters, optical devices, strain gauges, or other suitable apparatus may be used for the measurements.
- F. If the test pressure drops as a result of unexpected conditions to or below the next lower pressure level, the entire test sequence should be repeated. Significant deviations from the previous test should be recorded and evaluated.
- G. If any significant modifications or repairs are made to the drywell following, and because of, the initial test, the test should be repeated.
- H. A description of the proposed acceptance test and instrumentation requirements should be included in the preliminary SAR.
- I. The following information should be submitted before the performance of the test:
 - i. The numerical values of the predicted responses of the structure which will be measured
 - ii. The tolerances to be permitted on the predicted responses
 - iii. The bases on which the predicted responses and the tolerances were established
- J. The following information should be included in the final test report:
 - i. A description of the actual test and instrumentation
 - ii. A comparison of the test measurements with the allowable limits (predicted response plus tolerance) for deflections and strains
 - iii. An evaluation of the accuracy of the measurements
 - iv. An evaluation of any deviations (i.e., test results that exceed the allowable limits), the disposition of the deviations, and the need for corrective measures
 - v. A discussion of the calculated safety margin provided by the structure as deduced from the test results

For Category I structures inside containment, structures monitoring and maintenance requirements are acceptable if found to be in accordance with 10 CFR 50.65 and RG 1.160.

It is important that Category I structures inside containment accommodate inservice inspection of critical areas. The staff considers that monitoring and maintaining the condition of the containment internal structures is essential for plant safety. Any special design provisions (e.g., providing sufficient physical access, providing alternative means for identifying conditions in inaccessible areas that can lead to degradation, remote visual monitoring of high radiation areas) to accommodate inservice inspection of containment internal structures are reviewed on a case-by-case basis.

Technical Rationale

The technical rationale for application of these requirements and/or acceptance criteria to the areas of review addressed by this SRP section is discussed in the following paragraphs:

1. Compliance with 10 CFR 50.55a requires that SSCs be designed, fabricated, erected, constructed, tested, and inspected to quality standards commensurate with the importance of the safety function to be performed.

SRP Section 3.8.3 cites RG 1.57, 1.69, 1.136, 1.142, 1.143, 1.160, and 1.199 for guidance regarding construction, quality control, tests, and inspections that are acceptable. ACI 349, with additional guidance provided RG 1.142 and 1.199; ASME Code, Section III, Division 1, Subsection NE, and ASME Code, Section III, Division 2, Subsection CC, with additional guidance provided by RG 1.57 and 1.136, respectively; and ANSI/AISC N690-1994 including Supplement 2 (2004) contain criteria for concrete and steel structures.

Meeting these requirements provides assurance that the SSCs described herein will perform their safety function and limit the release of radioactive materials.

2. Compliance with GDC 1 requires that (1) SSCs important to safety be designed, fabricated, erected, and tested in accordance with quality standards commensurate with the importance of their safety function, (2) a quality assurance program be established and implemented, and (3) sufficient and appropriate records be maintained. When generally recognized codes and standards are used, they shall be identified and evaluated to determine their applicability, adequacy, and sufficiency and shall be supplemented or modified as necessary to assure a quality product in keeping with the required safety function.

SRP Section 3.8.3 provides guidance related to static and dynamic loadings and evaluation criteria for containment internal structures. It also describes acceptable materials, design methodology, quality control procedures, construction methods, and inservice inspections, as well as documentation criteria for design and construction controls.

SRP Section 3.8.3 cites ACI 349; ASME Code Section III, Division 1, Subsection NE, and ASME Code, Section III, Division 2, Subsection CC, with additional guidance provided by RG 1.57 and 1.136, respectively; ANSI/AISC N690-1994 including Supplement 2 (2004); and regulatory guidance describing design methodology,

materials testing, and construction techniques that are commensurate with the importance of the safety function to be performed. Conformance with these requirements imposes specific restrictions to ensure that containment internal structures will perform acceptably, commensurate with their intended safety function, when designed in accordance with the above standards.

Meeting these requirements provides assurance that the SSCs described herein will perform their intended safety function.

3. Compliance with GDC 2 requires that SSCs important to safety be designed to withstand the effects of expected natural phenomena, such as earthquakes, tornadoes, hurricanes, floods, tsunami, and seiches, without a loss of capability to perform their safety functions. The design bases for these SSCs shall reflect appropriate combinations of the effects of normal and accident conditions with the effects of the natural phenomena.

To ensure that structures of a nuclear power plant are designed to withstand natural phenomena, it is necessary to consider the most severe natural phenomena that have been historically reported with sufficient margin for the limited accuracy, quantity, and period of time in which the historical data have been accumulated. These data shall be used to specify the design requirements of nuclear power plant components to be evaluated as part of construction permit, operating license (OL), COL, and ESP reviews, or for site parameter envelopes in the case of DCs, thereby ensuring that components important to safety will function in a manner that will maintain the plant in a safe condition.

SRP Section 3.8.3 provides detailed acceptance criteria and cites appropriate regulatory guidance for design methodology, materials testing, and construction techniques that are acceptable to the staff. GDC 2 requires that containment internal structures be designed to withstand the effects of natural phenomena, combined with those of normal and accident conditions, without a loss of capability to perform their safety function. Load combinations and specifications cited in this SRP section provide acceptable engineering criteria to accomplish that function.

Meeting these requirements provides assurance that safety-related structures inside containment will be designed to withstand the effects of natural phenomena and will perform their intended safety function.

4. Compliance with GDC 4 requires that nuclear power plant SSCs important to safety be designed to accommodate the effects of, and be compatible with, environmental conditions associated with normal operation, maintenance, testing, and postulated accidents, including LOCAs. In addition, these SSCs must be appropriately protected against dynamic effects, including the effects of missiles, pipe whipping, and discharging fluids, that may result from equipment failures and from events and conditions outside the nuclear power unit.

SRP Section 3.8.3 provides methods acceptable to the staff, including load combinations, acceptance criteria, standards, and codes to assure compliance with GDC 4. Meeting this requirement provides assurance that the containment internal structures will withstand loads from internal events, such as those described above, and

from external sources such as earthquakes, thus decreasing the probability that these events will damage containment internal structures.

Meeting these requirements provides assurance that the internal structures of the containment will function as designed, be capable of maintaining their structural integrity, and perform their intended safety function.

5. Compliance with GDC 5 prohibits the sharing of structures important to safety among nuclear power units unless it can be shown that such sharing will not significantly impair their ability to perform their safety functions, including, in the event of an accident in one unit, an orderly shutdown and cooldown of the remaining units.

The requirements of GDC 5 are imposed to ensure that the use of common structures in multiunit plants will not significantly affect the orderly and safe shutdown and cooldown in one plant in the event of an accident in another. The load combination equations combine loads from normal operation and design-basis accidents so that the resulting structural designs provide for mutual independence of shared structures.

Meeting this requirement provides assurance that containment internal structures and their associated components are capable of performing their required safety functions, even if they are shared by multiple nuclear power units.

6. Compliance with GDC 50 requires that the reactor containment structure, including access openings, penetrations, and the containment heat removal system, be designed so that the containment structure and its internal compartments can accommodate, without exceeding the design leakage rate and with sufficient margin, the calculated pressure and temperature conditions resulting from any LOCA.

SRP Section 3.8.3 provides detailed acceptance criteria and cites appropriate regulatory guidance for design methodology, material testing, and construction techniques that are acceptable to the staff. GDC 50 requires that the internal structures of the containment be designed to withstand the effects of natural phenomena, combined with those of normal and accident conditions, including LOCA loads, without a loss of capability to perform their safety function. The load combinations and specifications cited in this SRP section provide acceptable engineering criteria to accomplish that function.

Meeting these requirements provides assurance that the internal structures of the containment will perform their intended safety function with sufficient margin when subjected to LOCA loads in combination with other applicable loads.

7. Compliance with 10 CFR 50, appendix B, requires that applicants establish and maintain a quality assurance program for the design, construction, and operation of SSCs.

SRP Section 3.8.3 provides guidance specifically related to design, construction, testing and inservice surveillance of structural concrete and steel of containment internal structures. Subsection II.2 of this SRP section cites ACI 349, with additional guidance provided by RG 1.142 and 1.199; ASME Code, Section III, Division 1, Subsection NE, and ASME Code, Section III, Division 2, Subsection CC, with additional guidance provided by RG 1.57 and 1.136, respectively; ANSI/AISC N690-1994 including Supplement 2 (2004); and other regulatory guides to satisfy the requirements of 10 CFR 50, Appendix B.

Meeting these requirements provides assurance that structures covered in this SRP section will meet the requirements of 10 CFR 50, Appendix B and thus perform their intended safety function.

III. REVIEW PROCEDURES

The reviewer will select and emphasize material from the review procedures described below, as may be appropriate for a particular case.

For each area of review specified in Subsection I of this SRP section, the review procedure is identified below. These review procedures are based on the identified SRP acceptance criteria. For deviations from these specific acceptance criteria, the staff should review the applicant's evaluation of how the proposed alternatives to the SRP criteria provide an acceptable method of complying with the relevant NRC requirements identified in Subsection II.

1. Description of the Internal Structures

After the type of structure and its functional characteristics are identified, information on similar and previously licensed plants is obtained for reference. Such information, which is available in SARs and amendments of previous license applications, can identify differences in the case under review. These differences require additional justification and evaluation for meaningful review. New and unique features that have not been used in the past are examined in greater detail. The information furnished in the SAR is reviewed for completeness in accordance with RG 1.70.

2. Applicable Codes, Standards, and Specifications

The list of codes, standards, guides, and specifications is checked against the list in Subsection II.2 of this SRP section. The reviewer verifies the use of the appropriate code or guide and the acceptability of the applicable edition and stated effective addenda.

3. Loads and Loading Combinations

The reviewer verifies that the loads and load combinations are consistent with those specified in Subsection II.3 of this SRP section. Any deviations from the acceptance criteria for loads and load combinations are reviewed for adequate justification.

4. Design and Analysis Procedures

The reviewer becomes familiar with the design and analysis procedures that are generally used for the type of structures being reviewed. Because the assumptions regarding the expected behavior of the structure and its various elements under loads may be significant, the reviewer evaluates their acceptability based on the acceptance criteria provided in Section II. The design and analysis procedures, including the behavior of the structures under various loads and the manner in which these loads are treated in conjunction with other coexistent loads are reviewed to establish compliance with procedures delineated in Subsection II.4 of this SRP section. These include the criteria for computer programs, consideration of concrete cracking, design reports, and the structural audit.

As discussed in Subsection II.4.E of this SRP section, the use of modular construction methods is reviewed on a case-by-case basis using guidance provided in NUREG/CR-6486.

5. Structural Acceptance Criteria

The limits on allowable stresses and strains in the structural elements, including concrete, reinforcement, structural steel, and anchors, are compared with those specified in Subsection II.5 of this SRP section. The reviewer evaluates the justification provided to demonstrate that the functional integrity of the structure will not be affected, when the applicant proposes to exceed some of these limits for certain load combinations and at certain localized points on the structure.

6. Materials, Quality Control, and Special Construction Techniques

The information provided on materials, quality control programs, and special construction techniques, if any, is reviewed and compared with that specified in Subsection II.6 of this SRP section. For a new material that has not been used in prior license applications, the applicant is requested to provide sufficient test and user data to establish the acceptability of such a material. Similarly, the reviewer evaluates any new quality control programs or construction techniques to ensure that no degradation of structural quality that might affect the structural integrity of the structure will occur.

7. Testing and Inservice Surveillance Requirements

Procedures for the structural test of the BWR containment drywell are reviewed and compared with the procedures described in Subsection II.7 of this SRP section. Any other proposed testing and inservice surveillance programs are reviewed on a case-by-case basis.

For containment internal structures, the reviewer verifies that structure monitoring and maintenance requirements are in accordance with 10 CFR 50.65 and RG 1.160.

Any special design provisions (e.g., providing sufficient physical access, providing alternative means for identification of conditions in inaccessible areas that can lead to degradation, remote visual monitoring of high radiation areas) to accommodate inservice inspection of containment internal structures are reviewed on a case-by-case basis.

Any other proposed testing and inservice surveillance programs are reviewed on a case-by-case basis.

8. Design Certification/Combined License Application Reviews

For reviews of DC and COL applications under 10 CFR Part 52, the reviewer should follow the above procedures to verify that the design set forth in the safety analysis report, and if applicable, site interface requirements meet the acceptance criteria. For DC applications, the reviewer should identify necessary COL action items. With respect to COL applications, the scope of the review is dependent on whether the COL applicant references a DC, an ESP or other NRC-approved material, applications, and/or reports.

After this review, SRP Section 14.3 should be followed for the review of Tier I information for the design, including the postulated site parameters, interface criteria, and ITAAC.

IV. EVALUATION FINDINGS

The reviewer verifies that the applicant has provided sufficient information and that the review and calculations (if applicable) support conclusions of the following type to be included in the staff's safety evaluation report. The reviewer also states the bases for those conclusions.

The staff concludes that the design of the containment internal structures is acceptable and meets the relevant requirements of 10 CFR 50.55a, 10 CFR 50, Appendix B, and GDC 1, 2, 4, 5, and 50. This conclusion is based on the following:

1. The applicant has met the requirements of Section 50.55a and GDC 1 with respect to ensuring that the containment internal structures are designed, fabricated, erected, constructed, tested, and inspected to quality standards commensurate with the safety function to be performed.
2. The applicant has met the requirements of GDC 2 by designing the containment internal structures to withstand the most severe earthquake that has been established for the site with sufficient margin, as well as the combinations of the effects of normal and accident conditions with the effects of environmental loadings, such as earthquakes and other natural phenomena.
3. The applicant has met the requirements of GDC 4 by ensuring that the design of the containment internal structures is capable of withstanding the dynamic effects associated with missiles, pipe whipping, and discharging fluids.
4. The applicant has met the requirements of GDC 5 by demonstrating that SSCs are not shared between units or that sharing will not impair their ability to perform their intended safety functions.
5. The applicant has met the requirements of GDC 50 by designing the containment internal structures to accommodate, with sufficient margin, the design leakage rate, calculated pressure, and temperature conditions resulting from accident conditions and by ensuring that the design conditions are not exceeded during the full course of the accident condition. In meeting these design requirements, the applicant has used the recommendations of the regulatory guides and industry standards indicated below. The applicant has also performed appropriate analysis to demonstrate that the ultimate capacity of the structures will not be exceeded and to establish the minimum margin of safety for the design.
6. The applicant has met the requirements of 10 CFR 50, Appendix B by providing a quality assurance program that includes adequate measures for implementing guidelines relating to structural design.

The criteria used in the design, analysis, and construction of the containment internal structures to account for anticipated loadings and postulated conditions that may be imposed upon each structure during its service lifetime conform to established criteria, codes, standards, and specifications acceptable to the NRC staff. This includes meeting the positions of RG 1.57,

1.69, 1.136, 1.142, 1.143, 1.160, and 1.199, as well as industry codes and standards ACI-349; ANSI/AISC N690-1994 including Supplement 2 (2004); and, depending on the structure, ASME Code, Section III, Division 1, Subsection NE, or ASME Code, Section III, Division 2, Subsection CC, with additional guidance provided by RG 1.57 and 1.136, respectively.

The use of these criteria, as defined by the applicable codes, standards, and specifications; loads and loading combinations; design and analysis procedures; structural acceptance criteria; materials, quality control programs, and special construction techniques; and testing and inservice surveillance requirements, provides reasonable assurance that, in the event of earthquakes and various postulated accidents occurring within the containment, the **containment internal** structures will withstand the specified design conditions without impairment of structural integrity or the performance of required safety functions.

For DC and COL reviews, the findings will also summarize (to the extent that the review is not discussed in other SER sections) the staff's evaluation of the ITAAC, including design acceptance criteria, as applicable, and interface requirements and combined license action items relevant to this SRP section.

V. IMPLEMENTATION

The staff will use this SRP section in performing safety evaluations of DC applications and license applications submitted by applicants pursuant to 10 CFR Part 50 or 10 CFR Part 52. Except when the applicant proposes an acceptable alternative method for complying with specified portions of the Commission's regulations, the staff will use the method described herein to evaluate conformance with Commission regulations.

The provisions of this SRP section apply to reviews of applications docketed six months or more after the date of issuance of this SRP section, unless superseded by a later revision.

Implementation schedules for conformance to parts of the method discussed herein are contained in the referenced regulatory guides.

VI. REFERENCES

1. 10 CFR 50.55a, "Codes and Standards."
2. 10 CFR 50.65, "Monitoring the Effectiveness of Maintenance at Nuclear Power Plants."
3. 10 CFR Part 50, Appendix A, General Design Criterion 1, "Quality Standards and Records."
4. 10 CFR Part 50, Appendix A, General Design Criterion 2, "Design Bases for Protection Against Natural Phenomena."
5. 10 CFR Part 50, Appendix A, General Design Criterion 4, "Environmental and Dynamic Effects Design Bases."
6. 10 CFR Part 50, Appendix A, General Design Criterion 5, "Sharing of Structures, Systems, and Components."

7. 10 CFR Part 50, Appendix A, General Design Criterion 50, "Containment Design Bases."
8. 10 CFR Part 50, Appendix B, "Quality Assurance Criteria for Nuclear Power Plants and Fuel Reprocessing Plants."
9. 10 CFR Part 50, Appendix S, "Earthquake Engineering Criteria for Nuclear Power Plants."
10. 10 CFR Part 52, "Early Site Permits; Standard Design Certifications; and Combined Licenses for Nuclear Power Plants."
11. ACI 349, "Code Requirements for Nuclear Safety Related Concrete Structures," American Concrete Institute.
12. ANSI/AISC N690-1994 including Supplement 2 (2004), "Specification for Design, Fabrication and Erection of Steel Safety-Related Structures for Nuclear Facilities."
13. ASME Boiler and Pressure Vessel Code, Section III, Division 2, Subsection CC, "Code for Concrete Reactor Vessels and Containments," American Society of Mechanical Engineers.
14. ASME Boiler and Pressure Vessel Code, Section III, Division 1, Subsection NE, "Class MC Components," American Society of Mechanical Engineers.
15. NUREG/CR-6486, "Assessment of Modular Construction for Safety-Related Structures at Advanced Nuclear Power Plants," March 1997.
16. Regulatory Guide 1.57, "Design Limits and Loading Combinations for Metal Primary Reactor Containment System Components."
17. Regulatory Guide 1.69, "Concrete Radiation Shields for Nuclear Power Plants."
18. Regulatory Guide 1.70, "Standard Format and Content of Safety Analysis Reports for Nuclear Power Plants."
19. Regulatory Guide 1.136, "Materials, Construction, and Testing of Concrete Containments."
20. Regulatory Guide 1.142, "Safety-Related Concrete Structures for Nuclear Power Plants."
21. Regulatory Guide 1.143, "Design Guidance for Radioactive Waste Management Systems, Structures, and Components Installed in LWR Plants."
22. Regulatory Guide 1.160, "Monitoring the Effectiveness of Maintenance at Nuclear Power Plants."
23. Regulatory Guide 1.199, "Anchoring Components and Structural Supports in Concrete."

24. Regulatory Guide 1.206, "Combined License Applications for Nuclear Power Plants (LWR Edition)."
25. SEI/ASCE 37, "Design Loads on Structures During Construction," American Society of Civil Engineers, 2002.

PAPERWORK REDUCTION ACT STATEMENT

The information collections contained in the draft Standard Review Plan are covered by the requirements of 10 CFR Part 50 and 10 CFR Part 52, and were approved by the Office of Management and Budget, approval number 3150-0011 and 3150-0151.

PUBLIC PROTECTION NOTIFICATION

The NRC may not conduct or sponsor, and a person is not required to respond to, a request for information or an information collection requirement unless the requesting document displays a currently valid OMB control number.

SRP Section 3.8.3

Description of Changes

This SRP section affirms the technical accuracy and adequacy of the guidance previously provided in (Draft) Revision 2, dated April 1996 of this SRP, with the following exceptions, as applicable. See ADAMS accession number ML052070326.

1. Revised section on review interfaces to remove reference to specific branch names for review responsibility and to replace them with a generic reference to the responsible organization. This change is reflected throughout the SRP.
2. Revised and expanded the criteria given for the elimination of the OBE, if applicable. This change is reflected throughout the SRP.
3. Revised technical rationale to better explain the basis for meeting each of the acceptance criteria; added technical rationale for Appendix B to 10 CFR Part 50.
4. Revised the criteria given for standard DC and OL application reviews under 10 CFR Part 52 and ITAAC. This change is reflected throughout the SRP.
5. Deleted reference to ACI 318-1977 in several subsections of this SRP section since the NRC no longer refers to it.

In addition this SRP section was administratively updated in accordance with NRR Office Instruction, LIC-200, Revision 1, "Standard Review Plan (SRP) Process." The revision also adds standard paragraphs to extend application of the updated SRP section to prospective submittals by applicants pursuant to 10 CFR Part 52.

The technical changes are incorporated in Revision 2, dated 200X:

Review Responsibilities - Reflects changes in review branches resulting from reorganization and branch consolidation. Change is reflected throughout the SRP.

I. AREAS OF REVIEW

1. Added guidance on the review of containment internal structures.
2. Added guidance on the review of applications that use modular construction methods. This change is reflected throughout the SRP.
3. Added discussion on loads encountered during construction. This change is reflected throughout the SRP.
4. Clarified use of the OBE.
5. Added a reference to SRP Section 3.8.1 for further description of the loads associated with SRVs and LOCAs.
6. Deleted existing guidance for loading criteria to comply with ACI 349 and ASME Code, Section III, Division 1, Subsection NE, and ASME Code, Section III, Division 2, Subsection CC, as applicable for containment internal structures. These items are

considered to be acceptance criteria and are already addressed in Subsection II.3 of this SRP section. This also makes the location of such criteria consistent with other SRP sections.

7. Corrected reference to other subsections in Sections 3.7 and 3.8 of the SRP related to design and analysis procedures.
8. Added guidance in other subsections of this SRP section to indicate that the section is oriented toward the BWR Mark III-type containment concept and that other BWR containment types are reviewed in a similar manner.
9. Updated references to ACI 349 and ANSI/AISC N690.
10. Added guidance on testing and inservice surveillance programs.

II. ACCEPTANCE CRITERIA

1. Revised discussion of Commission regulations and added references to Appendix B to 10 CFR Part 50, 10 CFR 52.47(a)(1)(vi), and 10 CFR 52.97(b)(1). This change is reflected throughout the SRP.
2. Added footnote stating that the SRP is not a substitute for the NRC's regulations and compliance with it is not required.
3. Added reference to DG-1145 and Regulatory Guide 1.206.
4. Updated reference to ACI 349, as augmented by Regulatory Guides 1.142 and 1.199, and ANSI/AISC-1994 including Supplement 2 (2004).
5. Deleted reference to Regulatory Guide 1.94 and ANSI N45.2.5 because the staff has recommended its withdrawal. This guide originally endorsed ANSI N45.2.5 with exceptions. The guidance on quality assurance and quality control matters are incorporated in SRP Section 3.8.3 via reference to ACI 349, as augmented by Regulatory Guides 1.142 and ANSI/AISC N690-1994 including Supplement 2 (2004).
6. Added reference to Regulatory Guides 1.69, 1.136, 1.143, 1.160, and 1.199.
7. Deleted existing load and load combination criteria for containment internal structures that were explicitly provided in the SRP subsection. In their place, reference was made to the load and load combination criteria given in ACI 349, as augmented in Regulatory Guides 1.142 and 1.199 for concrete structures and anchors, respectively, and ANSI/AISC N690-1994 including Supplement 2 (2004) for steel structures. In addition, for the divider barrier and ice-condenser of a PWR ice-condenser containment and the drywell of a BWR containment (which resist pressure), for design purposes, reference was made to ASME Section III, Division 1, Subsection NE, and Regulatory Guide 1.57 for steel structures, and ASME Section III, Division 2, Subsection CC, and Regulatory Guide 1.136 for concrete structures.
8. Provided supplemental criteria for certain loads, including dead loads, live loads, OBE, hydrodynamic loads, and construction loads.

9. Added discussion on fluid structure interaction associated with hydrodynamic and earthquake loads.
10. Revised the design and analysis procedures for containment internal structures to reflect the criteria given in ACI 349, as augmented by Regulatory Guides 1.142 and 1.199 for concrete structures and anchors, respectively, and ANSI/AISC N690-1994 including Supplement 2 (2004) for steel structures. In addition, for the divider barrier and ice-condenser of a PWR ice-condenser containment and the drywell of a BWR containment (which resist pressure), clarified the use of ASME Code, Section III, Division 1, Subsection NE, and Regulatory Guide 1.57 for steel structures, and ASME Code, Section III, Division 2, Subsection CC, and Regulatory Guide 1.136 for concrete structures.
11. Added design and analysis criteria for the weir wall which refer to ACI 349, as augmented by Regulatory Guides 1.142 and 1.199, for concrete and anchors (steel embedments), respectively.
12. Added additional criteria for design and analysis procedures similar to those in SRP Section 3.8.1. These include assumptions regarding boundary conditions, axisymmetric and nonaxisymmetric loads, transient and localized loads, shrinkage and cracking of concrete, computer programs, and evaluation of liner plates and anchors.
13. Deleted existing structural acceptance criteria for containment internal structures which were explicitly referenced in the SRP subsection corresponding to each load combination. In their place, referenced the structural acceptance criteria given in ACI 349, as augmented in Regulatory Guides 1.142 and 1.199, and ANSI/AISC N690-1994 including Supplement 2 (2004) for concrete and steel structures, respectively. In addition, for the divider barrier and ice-condenser of a PWR ice-condenser containment and the drywell of a BWR containment (which resist pressure), for design purposes, referenced ASME Code Section III, Division 1, Subsection NE, and ASME Code Section III, Division 2, Subsection CC (and associated Regulatory Guides 1.57 and 1.136), for the steel and concrete structures, respectively.
14. Deleted the exception in SRP Section 3.8.3 which allows a 33-1/3-percent increase in allowable stress for temperature loads, but not for OBE seismic or wind loads. This exception is no longer required because the more recent editions of the ASME Code (Table CC-3431-1 in Section III, Division 2, Subsection CC) does not allow the 33-1/3-percent increase for load combinations that include the OBE and wind loads.
15. Added guidance on inservice surveillance requirements with reference to 10 CFR 50.65 and Regulatory Guide 1.160. Added discussion on design provisions to accommodate inservice inspection.

III. REVIEW PROCEDURES

1. Revised to be consistent with changes discussed in the above Acceptance Criteria section.
2. Added section on DC/COL reviews with reference to review of ITAAC under SRP Section 14.3.

IV. EVALUATION FINDINGS

1. Revised to be consistent with changes discussed in the above Acceptance Criteria section.

V. IMPLEMENTATION

1. Incorporated the standard staff explanation regarding the use of this SRP section for DC and license applications

VI. REFERENCES

1. Added reference listings for 10 CFR 50.65, Appendices B and S to 10 CFR Part 50, and 10 CFR Part 52.
2. Updated reference listing for ANSI/AISC N690.
3. Deleted reference listing for Regulatory Guide 1.94 and ANSI N45.2 because the staff has recommended its withdrawal. Regulatory Guide 1.94 originally endorsed ANSI N45.2.5, with exceptions. The guidance on quality assurance and quality control matters are incorporated in SRP Section 3.8.1 via reference to ACI 349, as augmented by Regulatory Guide 1.142; ANSI/AISC N690-1994 including Supplement 2 (2004); and ASME Code, Section III, Division 1, Subsection NE and ASME Code, Section III, Division 2, Subsection CC.
4. Deleted reference listing for ASME Code, Section III, Division 1, Subsection NF because these components are reviewed under SRP Sections 3.9.2 and 3.9.3.
5. Added reference listings for Regulatory Guides 1.69, 1.136, 1.143, 1.160, 1.199, and 1.206 and DG-1145.
6. Added reference listing for NUREG/CR-6486.
7. Added reference listing for SEI/ASCE 37.