Battelle

The Business of Innovation

505 King Avenue Columbus, Ohio 43201-2693 (614) 424-6424 Fax (614) 424-5263

February 14, 2005

Mr. Mike McCann U.S. Nuclear Regulatory Commission Region III 2443 Warrenville Road Suite 210 Lisle, IL 60532-4352

Dear Mr. McCann:

Subject: Submittal of Battelle's technical basis documentation for groundwater monitoring well locations at the West Jefferson North site under the SNM-7 License.

Mike, per your request I am sending two copies each of the following documents, which in summary represent Battelle's technical basis documentation for selection of the groundwater monitoring well locations at the Battelle West Jefferson North site. These groundwater monitoring wells are also identified in our current site Environmental Monitoring Plan of which I am also sending two copies of to your attention.

- 1) Letter from Fred Klaer, Jr. and Associates to Battelle dated April 2, 1963
- 2) Quality Assurance Plan for Battelle Groundwater Monitoring dated June 18, 1990
- 3) Procedure for Well Installation and Well/Borehole Abandonment dated September 5, 1989
- 4) Site Characterization West Jefferson North Site Groundwater Monitoring Well Hydraulic Conductivity Testing and Analysis dated January 31, 1990
- 5) Geology and Hydrogeology of West Jefferson North Site dated September 14, 1990
- 6) Environmental Monitoring Plan- Columbus Closure Project dated June 2004

If you have any questions or comments please do not hesitate to call me at 614-424-4098.

Sincerely,

Joe Jacobsen BCLDP Radiation Safety Officer

SC-SP-004.1 Revision 1

# QUALITY ASSURANCE DOCUMENT

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# PROCEDURE FOR WELL INSTALLATION AND WELL/BOREHOLE ABANDONMENT

BATTELLE 505 King Avenue Columbus, Ohio 43201

- Ar S. Nelson

September 5, 1989 Date

APPROVED BY APPROVED BY Michael J. Sterhome <u>\_\_\_\_</u> Date 922 Date

APPROVED BY . APPROVED BY pM1~ 9/22/89 1 zin 9)-7/29 Date Date

SC-SP-004.1 Revision 1 September 29, 1989 Page i of i

# DOCUMENT REVISION RECORD

Title	Procedure for Well Installation and	No.	SC-SP	-004	.1
	Well/Borehole Abandonment	Page	i	of	i

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INDEX OF PAGE REVISIONS

Page No.	i	ii	iii				
Rev. No.	1						

Page No.	1	2	3	4	5	6	7	8	9	10
Rev. No.	1	1	1	1	1	1	1	1		

Page No.					
Rev. No.					

Page No.					
Rev. No.					

Page No.					
Rev. No.					

# **REVISION RECORD**

Rev. No.	00	01	02	03	04	05	06	07	08	09
Date		9/29/2								
Approval		Part .								

SC-SP-004.1 Revision 1 September 29, 1989 Page 1 of 8

# PROCEDURE FOR WELL INSTALLATION AND WELL/BOREHOLE ABANDONMENT

## 1.0 <u>Scope</u>

This document describes the procedure for installing wells in boreholes and for abandoning wells or boreholes.

# 2.0 Purpose

The purpose of this procedure is to provide a method for performing well installations in support of drilling/coring operations. The procedure is primarily in support of soil characterization work in the Nuclear Sciences Area of Battelle's West Jefferson Site; however, it may be applied to other locations where soil characterization work is being performed. The major reason for installing wells is to obtain information from subsequent measurements relating to water levels and hydraulic conductivity. A method for abandoning wells/boreholes is also provided for restoration of the land surface and safety purposes.

### 3.0 <u>References</u>

3.1 Ohio Administrative Code 3745-9-10, Water Well Standards and Waivers.

#### 4.0 General

## 4.1 Materials

- 4.1.1 Pipe casing, 2-inch Schedule 40 PVC, with suitable flushthreaded fittings. All connections will be flush-joint threaded.
- 4.1.2 Screen, 2-inch diameter PVC having 0.010-inch slots. The screen will be capped at the bottom.
- 4.1.3 Rounded sand or gravel, washed and bagged, with a grain-size distribution (U.S. Sieve Size) compatible with the screen and formation.

SC-SP-004.1 Revision 1 September 29, 1989 Page 2 of 8

- 4.1.4 Bentonite, granulated or pelletized.
- 4.1.5 Cement grout; nominally 74 percent Portland Class A cement, 24 percent Pozzolan cement, and 2 percent bentonite.

# 5.0 <u>Responsibilities</u>

- 5.1 The hydrogeologist shall be responsible for the placement of wells, i.e. for determining which boreholes are to have a well installed.
- 5.2 The drilling subcontractor shall be suitably qualified in the installation of wells, as determined previously (during bid selection) by the hydrogeologist responsible for bid selection.
- 5.3 The Technical Project Manager shall determine which method to be used for each borehole.

## 6.0 Procedure

- 6.1 Well Casing Initial Installation
  - 6.1.1 Place the screen and casing into the borehole.
    - NOTE: If borehole walls are found to be prone to slumping during well drilling, the hollow stem auger can be used as a temporary casing through which screens and casing can be run into the borehole.
  - 6.1.2 Place the sand/gravel pack (Step 4.1.3) into the casing to fill the well from the bottom of the borehole to 1 foot above the top of the screen. NOTE: If the water table is close to the land surface, the field hydrogeologist will reduce this quantity of sand/gravel pack above the screen so that no surface runoff will seep into the wells.
  - 6.1.3 Tremie bentonite (Step 4.1.4) above the sand/gravel pack, to a minimum thickness of 3 feet.
  - 6.1.4 Tremie-grout cement grout (Step 4.1.5) from above the bentonite seal to the land surface.

SC-SP-004.1 Revision 1 September 29, 1989 Page 3 of 8

6.2 Completion of Well

## 6.2.1 METHOD 1

- Casing Flush or Below the Land Surface (See Figure 1):
- 6.2.1.1 Set the casing 2 to 3 inches below land surface, using cement.
- 6.2.1.2 Complete the assembly with a protective steel casing, equipped with a locking lid.
- 6.2.1.3 Install protective housing consisting of a castiron valve box assembly centered in a 3-footdiameter concrete pad sloped away from the valve box.
- 6.2.1.4 Maintain free drainage away from the well within the valve box.
- 6.2.1.5 Install a screw-type stainless steel cap with Teflon or Viton O-ring to prevent infiltration of surface water.
- 6.2.1.6 Maintain a minimum of 1 foot of clearance between the casing top and the bottom of the valve box lid.

#### 6.2.2 METHOD 2

Above-Ground Surface Completion (See Figure 2):

- 6.2.2.1 Extend the well pipe approximately 2 feet above land surface.
- 6.2.2.2 If the well is located near a depression, lake, or creek with a history of flooding, install this extension (riser) higher than the flood stage.
- 6.2.2.3 Provide an aboveground stainless steel endplug or casing cap.
- 6.2.2.4 Shield the above-ground pipe with a steel casing placed over the PVC pipe.

SC-SP-004.1 Revision 1 September 29, 1989 Page 4 of 8

- 6.2.2.5 Seat all wells of this type in a 2-foot diameter by 4-inch thick concrete surface pad.
- 6.2.2.6 Slope the pad away from the well casing.
- 6.2.2.7 Install a lockable cap or lid on the steel casing.
- 6.2.2.8 If necessary (as determined by the Technical Project Manager), install 3-inch diameter steel guardposts for additional protection.
- 6.2.2.9 Install these guard posts about 5 feet high, radially from each wellhead, and recessed approximately 2 feet into the ground.
- 6.2.2.10 Paint the protective steel guard posts and clearly number the well on the lid exterior.
- 6.2.3 Provide locks for both flush and above ground well assemblies. Turn over lock keys to the Technical Project Manager following completion of the field sampling.
- 6.2.4 Develop all groundwater monitoring wells after installation.Prior to development, monitor water levels (to the nearest 0.01 inch) with respect to an established survey point at the top of the well casing.
- 6.2.5 Details of the well installation. including exact measurements, will be filled out on the Well Construction/Completion Report Sheet (DDO-125).

## 6.3 Well/Borehole Abandonment

6.3.1 Seal wells/boreholes according to the recommended procedure (Reference 4.3), using material impervious to migration of water in the hole or within the hole (i.e. grout).

SC-SP-004.1 Revision 1 September 29, 1989 Page 5 of 8

# 7.0 <u>Records</u>

7.1 The QA records generated by the implementation of this procedure are completed and approved form DDO-125 and copies of quality affecting and relevant information entered in Laboratory Record Books.

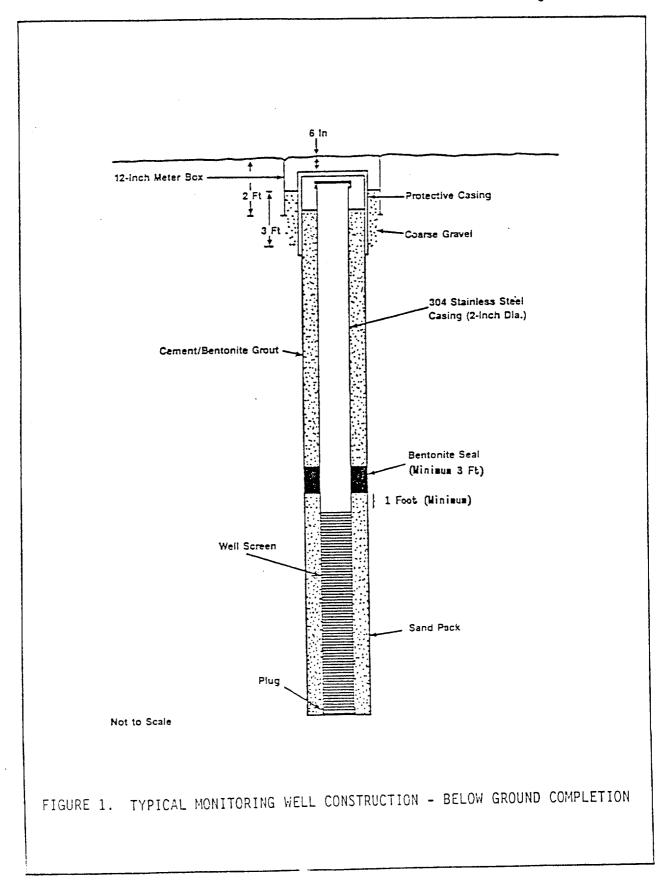
#### 8.0 Figures and Forms Referenced in This Procedure

- 8.1 Figures
  - 8.1.1 Figure 1, Typical Monitoring Wall Construction Below Ground Completion
  - 8.1.2 Figure 2, Typical Wall Construction Above Ground Completion

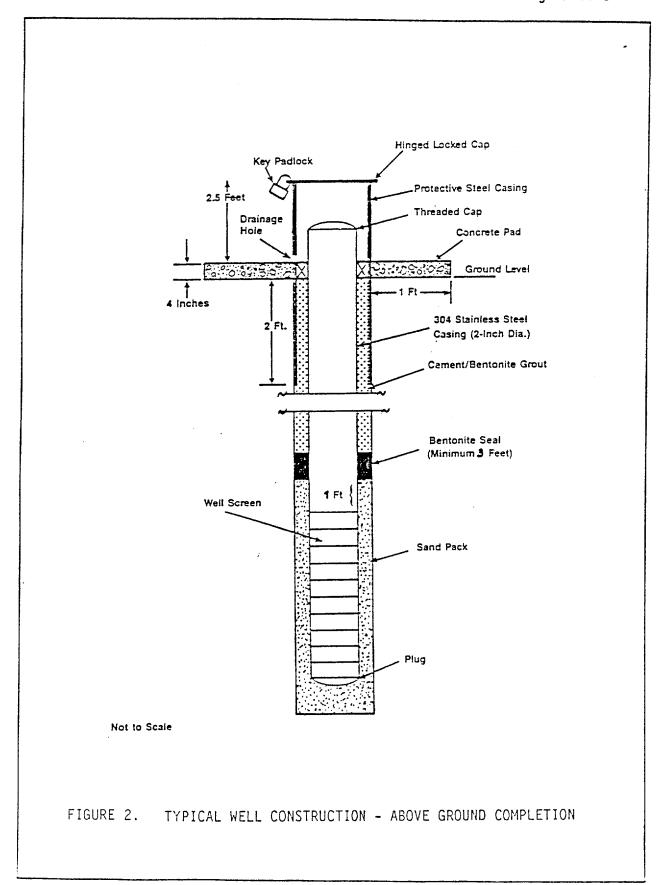
# 8.2 Forms

8.2.1 DDO-125, Well Construction/Completion Report Sheet

SC-SP-004.1 Revision 1 September 29, 1989 Page 6 of 8



SC-SP-004.1 Revision 1 September 29, 1989 Page 7 of 8



SC-SP-004.1 Revision 1 September 29, 1989 Page 8 of 8

Name of Site:	Summary of Well Construct
Well ID Number:	Well Cap Elevation:
Drilling Location: (Use Coordinates if Available) 777	Ground Surface Elevation:ft.
Total Depth:ft.	Conductor Hole Dia.:toft.
Drilling Contractor:	
Type of Rig:	Surface Conductor
Date Installation Completed:	Type:
(Y Y H H D D)	Borehole Dia.:in.
Observations of Monitoring Zone and Further Information:	[] [] toft
	Filler Haterial:
	Haterial:ft
	Casing (with screen) Haterial:
	Hateriai: ID:in. 0D:in toft
-	
	Screen Material:
	ID:in. OD:in toft
	Slot Size:
	2Pack
	Type: Size: toft
Fotorod by:	
Entered by:, (Date)	Approved by:,, (Cate)

J. Tholen

EM-QAP-2.0 Revision 0

## QUALITY ASSURANCE PLAN

for

#### BATTELLE GROUNDWATER MONITORING

Battelle 505 King Avenue Columbus, Ohio 43201

Prepared by

E.R. Swindall

<u>June 18, 1990</u> Date

APPROVED BY

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EM-QAP-2.0 Revision 0 August 17, 1990 Page i of i

# DOCUMENT REVISION RECORD

Title	Quality Assurance Plan for	No.	EM-Q	AP-2.0		
	Battelle Groundwater Monitoring	Page	i	of	i	

INDEX OF PAGE REVISIONS

Page No.	i	ii	iii							
Rev. No.	0									
Page No.	1	2	3	4	5	6	7	8	9	10
Rev. No.	0	0	0	0	0	0	0	0	0	0
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Page No.	11	12	13	14	15	16	17	18		
Rev. No.	0	0	0	0	0	0	0	0		
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Page No.										
Rev. No.			1							
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Page No.										
Rev. No.				1						

#### **REVISION RECORD**

Rev. No.	00	01	02	03	04	05	06	07	08	09
Date	8-17-90									
Approval	KSR									

DDO-009

EM-QAP-2.0 Revision 0 August 17, 1990 Page 1 of 19

#### BATTELLE GROUNDWATER MONITORING

#### INTRODUCTION

This plan specifically describes the quality assurance (QA) program for conducting environmental groundwater monitoring at the Battelle West Jefferson site (Figures 1 & 2) during the two phases of the Battelle Columbus Laboratories Decommissioning Project (BCLDP): surveillance and maintenance (S&M), and decontamination and decommissioning (D&D). It will be adopted as a subtier document under the Battelle Nuclear Quality Assurance (NQA) manual for the Decommissioning and Decontamination (DDO) Group. QA procedures and documents have been and will be developed and revised to provide the necessary planning, control, documentation, and safety for all activities associated with this effort.

The groundwater monitoring program is designed and will be implemented in accordance with monitoring requirements of 40 CFR Part 264, Subpart F, and 40 CFR Part 265, Subpart F. Monitoring for radionuclides shall be in accordance with DOE Orders in the 5400 series. The Task Leader of the Environmental Monitoring Group will coordinate this effort. This plan shall be reviewed annually and updated every three years until the contract expires. This plan was developed to be responsive to the requirements of DOE Order 5400.1, Chapter III, "General Environmental Protection Program", paragraph 4.a "<u>Special Program Planning Requirements</u>, Groundwater Protection Management Program, and the requirements of the groundwater monitoring plan pursuant to Chapter IV, paragraph 9 of DOE Order 5400.1, and all applicable DOE Orders of the 5400 series in addition to ANSI/ASME NQA-1 as listed below. Each applicable criteria is discussed in the sections that follow.

Organization (1)(2) Quality Assurance Program Procurement Document Control (4) Instructions, Procedures, and Drawings (5) Document Control (6) (7) Control of Purchased Items and Services Identification and Control of Items (8) (10)Inspection (11)Test Control (12)Control of Measuring and Test Equipment (13) Handling, Storage, and Shipping Inspection, Test and Operating Status (14) (15) Control of Nonconforming Items Corrective Action (16)(17)Records (18)Audits

EM-QAP-2.0 Revision 0 August 17, 1990 Page 2 of 19

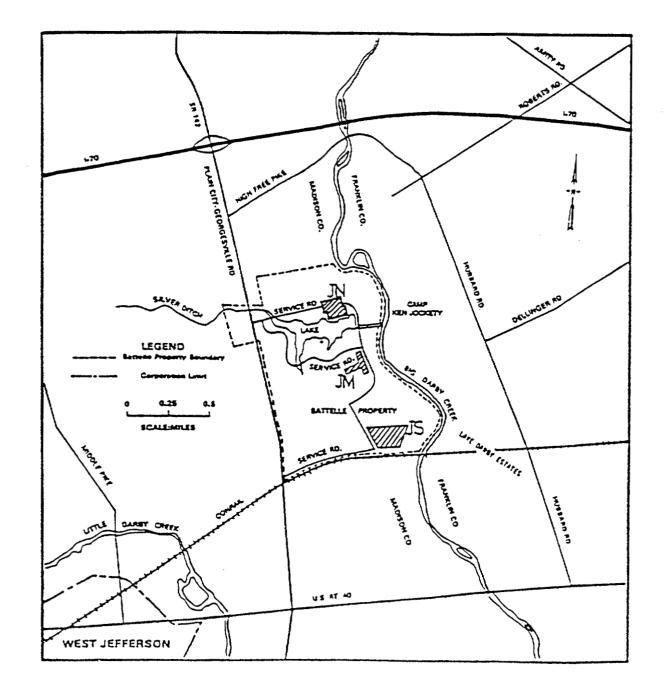
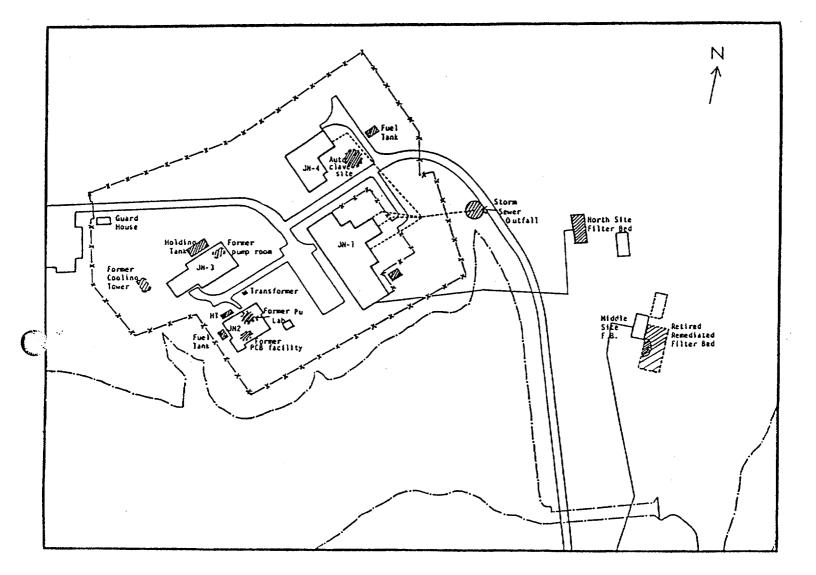


FIGURE 1. LOCAL VICINITY MAP OF WEST JEFFERSON SITE

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EM-QAP-2.0 Revision O August 17, 1990 Page 3 of 19



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FIGURE 2. NUCLEAR SCIENCES AREA

EM-QAP-2.0 Revision 0 August 17, 1990 Page 4 of 19

Since no specific designs are being developed, and no special processes will be conducted during the sampling program, NQA-1 requirements (3) Design Control, and (9) Control of Processes and are not applicable to this project and are not addressed in this plan.

#### 1.0 Organization

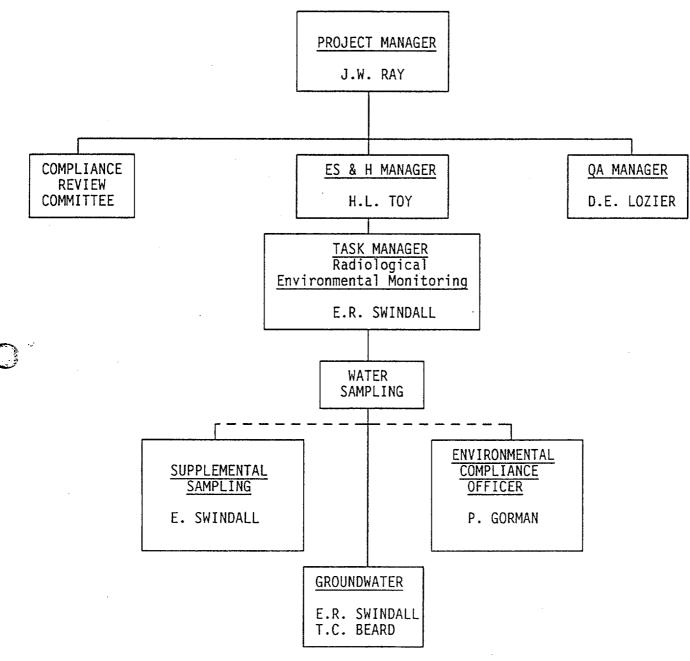
The organizational structure for Groundwater Monitoring is shown in Figure 3. The Environment, Health and Safety (ES&H) Manager, the QA Manager, and the Compliance Review Committee report directly to the BCLDP Project Manager. The Task Manager of the Radiological Environmental Monitoring Group reports directly to the ES&H Manager and is responsible for the day to day monitoring activities, equipment calibration, and review and evaluation of data generated. The Environmental Compliance Officer will assist in interpretation of data for compliance purposes.

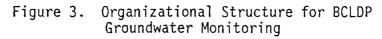
# 2.0 Program

Battelle has done limited groundwater monitoring for radionuclides since the early 1970s. The area of concern has been an underground aquifer running in the vicinity of the nuclear fuel storage pool for the West Jefferson, JN-1 facility. Two monitoring wells were installed at the time the pool was put in. One well is a sump that collects condensate from the pool liner, while the other collects water from the aquifer on the down gradient side of the pool. Samples have been collected on a monthly basis. The samples have been analyzed for gross alpha and gross beta emitters, fission products and activation products. There has been no indication that the aquifer has become contaminated from the pool. This sampling schedule is expected to continue for at least three to six months after the pool is emptied of all water to assure that there are no leaks from the groundwater to the inside sump which would imply a leak in the pool liner that could yet allow transfer of radioactivity to the groundwater system.

Additional radiological groundwater monitoring is done at a former supply well (JN) for the West Jefferson Nuclear Sciences Area and from

EM-QAP-2.0 Revision 0 August 17, 1990 Page 5 of 19





EM-QAP-2.0 Revision 0 August 17, 1990 Page 6 of 19

existing supply wells before the water is treated in any form. Samples that have been collected annually over the last five years have undergone the same analysis for gross alpha and gross beta emitters, fission products, and activation products. These wells continue to be sampled on a routine basis with additional chemical analysis of the JN well sample planned for volatile organic compounds. The Summary from the Interim Report on Site Characterization, West Jefferson North Site, Stage 1 Sampling and Analysis, Chemical Sampling Summary Report, December 22, 1989, has been modified to represent the current status of results and is presented here as the basis for the additional analysis.

#### Summary

A sampling and analysis program for chemical contaminants was performed in November 1989, at Battelle's Nuclear Sciences Area, West Jefferson Site, Ohio. A total of 32 sampling locations provided 29 soil and 3 groundwater samples for chemical analysis. During drilling operations, and the subsequent collection of soil cores, some hydrocarbon contamination of soil, assumed to be fuel oil, was observed around the three fuel storage tanks on site. Subsequent analysis of soil samples collected in these locations confirmed the presence of oil at levels of about 1300 ppm (JN-1), 1200-1500 ppm (JN-2), and 25-50 ppm (JN-4). Further characterization is underway. The EP Toxicity test for metals showed no concentrations above the RCRA limit of 1 ppm for most soil samples analyzed. PCBs were found in only one soil sample, taken close to the on-site transformer beside building JN-2, but at a ppb concentration, well below the action limit of 50 ppm. The only other contaminants, found at ppb concentrations in a few soil samples, were several volatile organic compounds, with acetone predominant. While the concentrations of these compounds are lowlevel, some additional sampling of soil in the storm-sewer outfall area is recommended in conjunction with the additional (Stage 2) sampling proposed for radiological purposes. No contamination was found in the groundwater samples collected.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 7 of 19

In addition to the three chemical sampling wells, twelve shallow wells were installed around the West Jefferson North site as part of the site characterization and will be used for monitoring radionuclides. (See Figure 4.) A full hydrogeological study is expected to be completed in June, 1990, that will allow for the reduction of the number of radiological monitoring wells to six. The need for additional chemical monitoring wells in the vicinity of the fuel storage tanks is being evaluated and appropriate monitoring will be instituted.

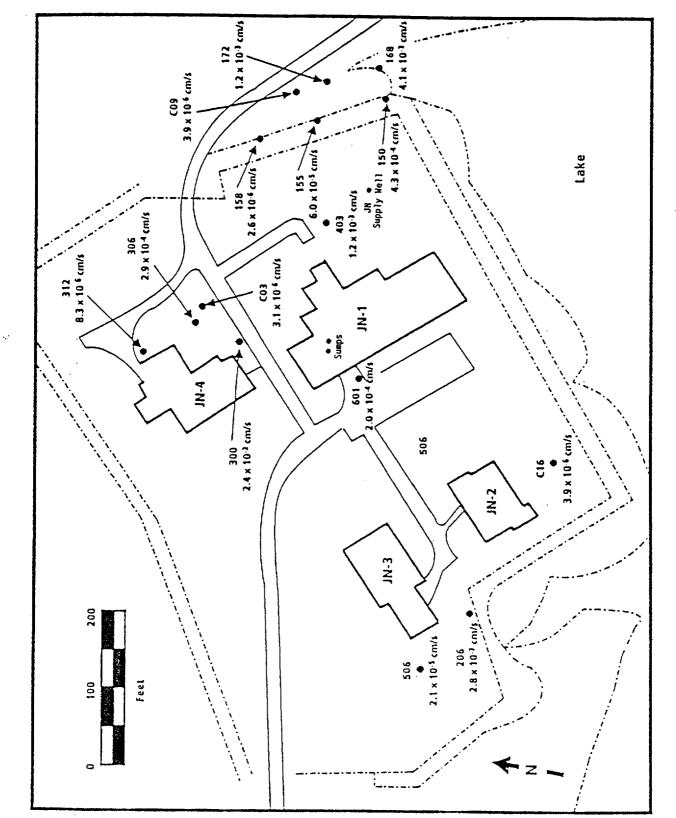
Three of the shallow radiological monitoring wells, also installed as part of the site characterization, are located in and near two former filter beds that still shows traces of radioactive materials. The Summary mentioned above suggests the need for installing chemical wells in this area to identify any pockets of chemicals that may exist from a build up of chemicals that may have gotten into the drain over the life of the filter bed. Radiological monitoring will be performed on an annual basis using existing wells with routine chemical monitoring being added when the wells are completed.

Liquid effluents at Battelle's King Avenue site are discharged into the city sanitary sewer system. The discharge points are currently monitored under the Environmental Monitoring program. All electrical transformers are housed within the facility with secondary containment dikes. There has been no known waste disposal or treatment on site. As a result of these conditions, no groundwater sampling is planned for the King Avenue site.

The budgetary resources for this program are from the BCLDP operating funds which will have to be increased or another source found for installing additional wells and covering the cost of routine chemical sampling.

Supplemental environmental monitoring, done in support of groundwater monitoring, includes the routine sampling or monitoring of effluents from, and the collection of routine samples of surface water, soil, and biota in the environs of a facility. The data and information collected are assessed to determine the impact of the operations in the facility on the environs and persons present in the environs, and to provide guidance for adjusting the operations if the impact is inappropriate.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 8 of 19



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FIGURE 4. LOCATIONS OF GROUNDWATER MONITORING WELLS

EM-QAP-2.0 Revision 0 August 17, 1990 Page 9 of 19

Objective: 1. Ensure that appropriate media samples are analyzed and data correctly interpreted to determine the impact of elements, compounds and radionuclides to groundwater. The annual objective is the reporting and assessment of all data culminating in a formal report to DOE.

> Maintain appropriate instrumentation and equipment in good repair and calibration to effectively collect and assess all sampled media.

Work Statement: Maintain the schedule of routine sampling, monitoring and analytical activities as provided in Appendix A and evaluate and perform additional sampling as determined necessary to adequately characterize the impact of operations on the environment.

## 4.0 Procurement

Procurement of items affecting quality of sampling shall be controlled through documents QA-AP-4.1. Procurements will be usually limited to replacement of worn or defective equipment from approved vendors.

## 5.0 Instructions, Procedures, and Drawings

Besides the application of ANSI/ASME NQA-1, the following groundwater monitoring procedure shall be used for routine monitoring, along with other procedures listed that may be used for supplemental sampling to identify potential hazards. Procedures and dates in parentheses indicate replacement procedures and the anticipated dates for completion of the replacement. Nonroutine sampling shall be handled through work instruction as outlined in QA-AP-5.2.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 10 of 19

Procedure	Revision	Date	Title
√ SC-SP-012	Rev. 0.1	05-30-90	Collection of Groundwater Samples in Support of Site Groundwater Charac- terization
NS-NS-10.1 (EM-SP-009)	Rev. O	07-07-81 (08-31-90)	Procedure for the Collection of Environmen- tal Hazardous Chemical Samples
√ EM-SP-002	Rev. 0.1	06-11-90	Procedure for the Collec- tion of Environmental Radiological Water Sam- ples
NS-NS-11.1 (EM-SP-010)	Rev. O	07-07-81 (08-31-90)	Procedure for the Collection of Environmen- tal Hazardous Chemical Water Samples
√ NS-NS-12 (EM-SP-003)	Rev. 1	07-31-90 (08-31-90)	Procedure for the Collection of Environmen- tal Radiological Soil Samples
√ NS-NS-13 (EM-SP-004)	Rev. 1	05-07-84 (08-31-90)	Procedure for the Collection of Environmen- tal Vegetation Samples - Annual Grass
SC-SP-006	Rev. O	01-29-90	Sampling of Sediment and Sludge in Ponds, Streams, Sumps and Closures

# 6.0 Document Control

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The following Project and QA program personnel or their designated alternates shall have the authority to approve quality documents:

EM-QAP-2.0 Revision 0 August 17, 1990 Page 11 of 19

ES&H Manager

Task Manager

• Health Physics Supervisor

QA Manager

• Radiochem. Lab. Manager

All documents shall be controlled by the review, approval and issue of process document QA-AP-6.1. Document revision and operation under temporary procedure changes shall be accomplished as specified in QA-AP-6.1. Documents shall be controlled by distribution of a document index containing the current document revision, and by staff responsibility for possessing current documents.

#### 7.0 Control of Purchased Items and Services

All project purchases other than routine supplies shall be reviewed by the QA Manager to determine if the purchase is a quality item based on the evaluation process described in QA-AP-4.1 and QA-AP-2.1. The QA organization personnel shall evaluate objective evidence of quality furnished by subcontractors to determine their suitability for placement on the approved suppliers list. Selection of quality-affecting subcontractors shall be made from records of past performance, incorporation on an approved suppliers list, and/or site visit evaluations, if necessary, as controlled by document QA-AP-7.1. The Project and QA organization may perform on-site surveys of the proposed subcontractor for acceptance on the approved suppliers list.

Procurements from time of order placement to receipt at Battelle shall be controlled by methods in QA-AP-7.2. The QA organization shall participate in the examination of all purchased quality items and services to determine their compliance to specifications of the purchase order. Approved inspection plans for items shall be drawn up in advance as specified in QA-AP-7.2 and QA-QP-10.1.

#### 8.0 Identification and Control of Items

All specimens, samples or any items quality-related to the program shall be identified by an affixed identification designation and/or in documents

EM-QAP-2.0 Revision 0 August 17, 1990 Page 12 of 19

traceable to the items. Routine samples shall use the appropriate sample identification found in appendix A, along with the date and time of sample collection. These samples are further identified in traceable documents and procedures. Non-routine sample identification shall be spell out in work instructions and identified in traceable documentation.

## 10.0 Inspections

Inspections for items shall be conducted in accordance with the requirements of QA-AP-10.1. The acceptance of items shall be documented and approved by the Task Manager or by higher management, as necessary and appropriate.

Spot surveillance of activities by observation by the QA Manager or his designee to assess their conformance with requirements and approved procedures. Any discrepancies noted shall be resolved with the Task Manager. Any noncompliance reports (NCRs), deficiency notices (DNs), and corrective action reports (CARs) shall be prepared, processed, and resolved in accordance with Sections 15 and 16 of this plan.

#### 11.0 Test Control

Analytical testing activities will be performed to collect data from the groundwater samples. Radioanalytical activities performed at Battelle will be controlled, documented and evaluated under procedure EL-AP-1.0 and its associated testing procedures.

#### 12.0 Control of Measuring and Test Equipment

- 12.1 The following items of systems are quality-affecting but not directly data generating, and requiring calibration.
  - 12.1.1 Teflon 1-1/2 inch bailer
  - 12.1.2 Composite Water Sampling System
  - 12.1.3 Radioanalytical Lab Counting Equipment
  - 12.1.4 Chemical Analytical Lab

from approved vendors list

EM-QAP-2.0 Revision 0 August 17, 1990 Page 13 of 19

- 12.2 The equipment indicated in Section 12.1.3 shall be calibrated to a standard traceable to the National Institute of Standards and Technology (NIST), formerly NBS.
- 12.3 The re-calibration time sequence for the calibrated equipment should not exceed one year unless justification is documented.

#### 13.0 Handling, Storage, and Shipping

The handling of all specimens, samples, and quality-related items shall be performed under controlled conditions predetermined to prevent damage, loss, minimize deterioration and assure safety. The storage of all specimens, samples, and quality-related items shall be implemented under controlled conditions predetermined to prevent damage, loss, minimize deterioration, and assure safety. The cleaning of all specimens, samples, and quality-related items shall be implemented under controlled conditions predetermined to prevent damage, loss, minimize deterioration, and assure safety. The packaging and shipping of all sample materials shall be implemented under controlled conditions predetermined to minimize loss, damage, and minimize deterioration.

#### 14.0 Inspection, Test, and Operating Status

The status of inspection and test activities shall be documented in records traceable to the items and the items tagged or identified where possible to assure the tests or inspections are performed. All items not meeting the inspection or test specifications or allowable limits shall be marked and/or separated from the approved items to prevent their inadvertent use, transport or disposal. All samples containing contaminant radioactivity will be identified with appropriate radioactivity identification tag and segregated from uncontaminated material. All samples containing contaminant levels of chemicals will be identified with a chemical contamination tag and segregated from uncontaminated material.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 14 of 19

### 15.0 Control of Nonconforming Items

All items or operations not meeting specifications or performed in accordance with approved procedures shall be documented on a nonconformance report. The nonconformance report shall be processed in accordance with procedure QA-AP-15.1.

### 16.0 Corrective Actions

All proposed corrective actions generated to resolve conditions adverse to quality shall be submitted to the NQA Manager for approval of the adequacy and time schedule of the action. Corrective actions shall conform to the requirements of procedure QA-AP-16.1. The cause of the adverse condition shall be determined, if possible, and corrective actions taken to preclude its recurrence. Follow-up action shall be taken by the program Technical Manager and Q.A. Manager to verify implementation and effectiveness of the corrective action.

#### 17.0 Quality Assurance Records

Records which furnish documentary evidence of quality shall be specified, prepared, and maintained.

Specified records include, but are not limited to the following:

- a. Maps identifying sampling locations
- b. Sampler Record Book
- c. Sample inventory
- d. Technical procedures and data sheets
- e. Calculation and analyses records
- f. Reports
- g. Q.A. Surveillance and Audit Records
- h. Program correspondence

Records shall be made part of the BCLDP record management system and subject to all the requirements and restrictions of the system.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 15 of 19

18.0 Audits

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Audits shall be planned and implemented in accordance with procedure QA-AP-18.1. A pre-program audit is not required as this is a continuation of an existing program. Periodic audits will be conducted over the life of the program.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 16 of 19 ¥.

APPENDIX A

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EM-QAP-2.0 Revision 0 August 17, 1990 Page 17 of 19

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# ROUTINE ENVIRONMENTAL GROUNDWATER MONITORING SCHEDULE

This monitoring schedule describes the well identification, general location, sampling frequency, and the measurements to be made. Radiological measurements are to consist of gross alpha and gross beta-gamma measurements with isotopic analysis to be performed on samples that are five times the counting background.

Chemical sampling is to consist of any combination of the following parameters and analytical methods or other approved methods that will produce the same sensitivity or better.

#### Analytical Methods

Volatile organic compounds	 SW-846 Method 8240 (GCMS)
Semi-volatile compounds	 SW-846 Method 8270 (GCMS)
Oil and grease	 SW-846 Method 423.1 (gravimetric)
Metals - EP Toxicity	 SW-846 Several Methods
PCBs	 SW-846 Method 8080 (GCMS)
рН	 SW-846 Method 150.1 (electrometric)

Sample collection frequency is to be annually (A) unless otherwise specified.

A - Annually M - Monthly

EM-QAP-2.0 Revision 0 August 17, 1990 Page 18 of 19

# A-2

# GROUNDWATER SAMPLING -WEST JEFFERSON LOCATION

North Area West Jefferson Site				
WELL #	Location	Frequency	Measurements	
150	Storm Sewer Outfall	Α .	$\alpha$ , $\beta$ (Diss/susp)	
155	Storm Sewer Outfall	А	$\alpha$ , $\beta$ (Diss/susp)	
158	Storm Sewer Outfall	А	$\alpha$ , $\beta$ (Diss/susp)	
168	Storm Sewer Outfall	А	$\alpha$ , $\beta$ (Diss/susp)	
172	Storm Sewer Outfall	A	$\alpha$ , $\beta$ (Diss/susp)	
206	South of JN-3	A	$\alpha$ , $\beta$ (Diss/susp)	
300	Southeast of JN-4	А	$\alpha$ , $\beta$ (Diss/susp)	
306	East of JN-4	A	$\alpha$ , $\beta$ (Diss/susp)	
312	Northeast of JN-4	А	$\alpha$ , $\beta$ (Diss/susp)	
403	East of JN-1	А	$\alpha$ , $\beta$ (Diss/susp)	
506	West of JN-3	А	$\alpha$ , $\beta$ (Diss/susp)	
601	West of JN-1	A	$\alpha$ , $\beta$ (Diss/susp)	
C03	East of JN-4	A	$\alpha$ , $\beta$ (Diss/susp)	
C09	Storm Sewer Outfall	A	$\alpha$ , $\beta$ (Diss/susp)	
C16	Southeast of JN-2	A	$\alpha$ , $\beta$ (Diss/susp)	
	Remediated Filter		Mazzunamenta	
<u>Well #</u>		Frequency	<u>Measurements</u>	
101	East Side of Filter Bed	А	$lpha,\ eta$ (Diss/susp), and Chemical Sampling	
104	Southeast of Filter Bed	A	$lpha,\ oldsymbol{eta}$ (Diss/susp), and Chemical Sampling	

West Side of Filter

Bed

А

110

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 $lpha,\ eta$  (Diss/susp), and Chemical Sampling

EM-QAP-2.0 Revision O August 17, 1990 Page 19 of 19

Supply Wells				
Well ID	Location	Frequency	Measurements	
JN	Nuclear Science Area (Supply Well-Inactive)	A M	$\alpha$ , $\beta$ (Diss/susp), and Chemical Sampling	
JM	West Jefferson Middle Area (Supply Well- Active)	A	$\alpha$ , $\beta$ (Diss/susp)	
JM-1	West Jefferson Middle Area (Facility Well)	A	$\alpha$ , $\beta$ (Diss/susp)	

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JAMI WEIIS The Files Well WH-1

# FRED H. KLAER, JR. & ASSOCIATES

CONSULTING GROUND-WATER GEOLOGISTS AND HYDROLOGISTS Lode -

16 LELAND AVENUE COLUMBUS 14, OHIO

PHONE AMHERST 8-3316 April 2, 1963

Mr. C. T. Greenidge Battelle Memorial Institute 505 King Avenue Columbus 1, Ohio

Dear Mr. Greenidge:

#### West Jefferson, Ohio.

In accordance with our discussion on March 14, Mr. Don Kyser of this office met Mr. Glen Williams at the West Jefferson plant of Battelle Memorial Institute on March 15.

The non-pumping or static level in the south well (Layne No.1) was measured as 42.1' below the pump hase. At the time the well was drilled in September 1954, the static level was reported by Layne as 41 feet. The apparent difference in static level is insignificant and suggests that there has been no serious regional decline in water levels during the past 9 years. It was not possible to measure the pumping level, because of the danger of flooding the basement. I believe this should be done, however, by making arrangements to waste water through several outlets within the building, if this can be worked out. (#2)

The static water level in the north well was measured as 18.17 feet below the pump base. The static water level in April 1955, as reported by Layne was 18'5". This also shows that there has been no significant change in water levels in this area. The pumping level was measured as 39.22 feet below the pump base after 3 to 4 minutes of pumping. This is not particularly significant because of the short period of pumping.

At the present time both wells operate automatically for such short periods of time that it is impossible to get any true value for pumping level. It may be possible to make arrangements to waste water in some way, so that the pumps could operate for one-half to one hour without shutting off. This I believe should be investigated. It is obvious, however, that you are not fully utilizing the capacities of these wells.

We have plotted up the data on the south well provided to you in a letter report from Burgess and Niple, dated September 16, 1954. This indicates that the transmissibility of the limestone aquifer is about 16,500 gallons per day per foot. Assuming a coefficient of storage of 0.0001, which is reasonable for limestone aquifers, we estimate that this well could be pumped continuously at 250 gpm. for a period of a year without recharge, without lowering the pumping level below the pump intake.

Using the values assumed above, we have computed the cone of influence of the south well pumping continuously at 250 gallons per minute without recharge, a copy of which is attached. This indicates that pumping the south well continuously will lower the water level in the north well, about 3700 feet away, about 2.5 feet in 24 hrs., 6.4 feet in 10 days, 11.4 feet in 180 days and about 12.6 feet in one year. At the present time your pumping schedule is so infrequent that one well probably does not affect the other.

It is believed that the wells of West Jefferson are about 10,000 feet from the south well. The computed cone of influence graph shows that unless the pumping from the West Jefferson wells now reported as 200 - 300 gpm. increases considerably, it is unlikely that your wells will be seriously affected.

This diagram can be used to determine proper spacing for additional wells, depending on the rates and expected duration of pumping.

We will be glad to discuss this with you at your convenience.

Sincerely yours,

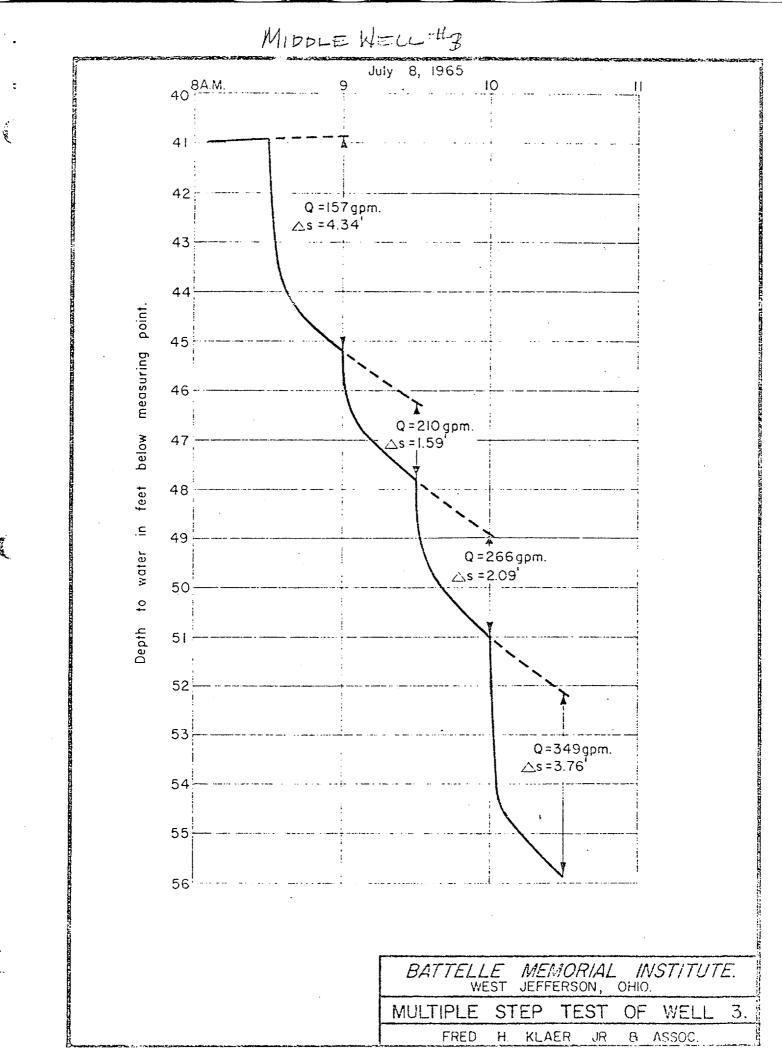
FRED H. KLAER, JR. & ASSOCIATES

Fred H. Klaerfr.

Fred H. Klaer, Jr. Consulting Ground-Water Geologist and Hydrologist.

FHKJr:eh

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A. S. S.

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(FORMERLY ROBERT C. SMITH & ASSOCIATES)

N. N.

MIDDLE MELC +- 8

FRED H. KLAER, JR. & ASSOCIATES

CONSULTING GROUND WATER GEOLOGISTS AND HYDROLOGISTS

16 LELAND AVENUE COLUMBUS, OHIO 43214 P.O. BOX 3496 PHONE AMAGENET 8-3316

July 13, 1965.

Eattelle Memorial Institute 505 King Avenue Colubus, Ohio 43201

Attention: Mr. Daun Peterseim

## Well 3, West Jefferson, Ohio.

Gentlemen,

In accordance with your instructions, we have followed closely the drilling of Well 3 near your proposed greenhouse site at your West Jefferson, Ohio, plant. The well was completed and tested during the week of July 6, 1965.

The driller's log of the well is reported as follows:

0 - 3	topsoil
3 - 5	light brown clay
5 - 48	brown clay and gravel
48 - 54	gravel
54 115	light brown clay and gravel
115 - 119	gravel
119 - 149	brown lime rock
149 - 152	red clay
152 - 161'9"	brown lime rock
118:6"	pipe in hole

Driller reports crevice in limestone at 141 feet. Static water level, June 8, 1965, 40.92 feet below top of casing.

On June 8, 1965, a multiple step drawdown test was run to determine at what depths water was entering the well and to determine the well losses within the pumping well. The well was pumped at rates of 157, 210, 266, and 349 gpm. for periods of 30 minutes at each rate. Prior to the step test, the static water level was at a depth of 40.92 feet below the top of the casing. At the end of the test, the pumping level was at a depth of 55.87 feet. A hydrograph of the multiple step test is shown in an attached figure.

## MIDDLE KIELL #3

#### FRED H. KLAER, JR. & ASSOCIATES CONSULTING GROUND WATER GEOLOGISTS AND HYDROLOGISTS

During the step test, the pumping level at a rate of 349 gpm. was at a depth of 55.87 feet below the top of the casing. Since the casing extended to a depth of 118 feet, 6 inches, it was not possible to lower the water level below the casing to show where the water enters the well. Since the driller reported a crevice at a depth of 141 feet, we must assume that the major part of the water enters the well below a depth of 118 feet and probably at about 141 feet.

The well loss factor was computed to be 8.92 and the actual well loss at a pumping rate of 349 gpm. was 5.36 feet. This is not unreasonable for a limestone well and we believe the well is acceptable.

Some difficulty was encountered originally in developing the well to pump clear water. It was necessary to spend several days in developing. It was planned to use acid treatment to clear up the well. However, no acid was used in the well and the capacity was increased by surging and pumping. At a rate of 349 gpm. the water pumped was completely clear.

On June 9, 1965, a constant rate test was run. The well was purped at a constant rate of 349 gpm. from 8:15 a.m. until 2:45 p.m., a total of 6-1/2 hours. The static water level at the start of the test was at a depth of 41.22 feet below the top of the casing and the pumping level at the end of the test was 62.75 feet, giving a total drawdown of 21.53 feet. The apparent specific capacity was about 16.2 gpm. per foot of drawdown.

The transmissibility of the limestone aquifer was determined as 9050 gallons per day per foot. This is somewhat lower than that computed from a pumping test on the south well of about 16,500 gpd. per ft., reported to Mr. Greenidge in our letter of April 2, 1963, based on a pumping test run by Burgess and Niple in September 1954.

Using a value of transmissibility of 9050 gpd. per ft. and a storage coefficient of 0.0001 (which is reasonable for the limestone aquifer in this area), the computed drawdown of a well pumping continuously at a rate of 500 gpm. for one year from ground water storage will be about 69.1 feet, plus 11 feet of well loss or a total of 80.1 feet. The total available drawdown will be the difference between the static water level of about 45 feet and the point where the water enters the well, say at 140 feet, giving a total drawdown of 95 feet. It is our opinion, therefore, that the safe capacity of the well is about 500 gpm. If it is necessary to pump the well at this rate the bottom of the pump intake should be set at a depth of 140 feet.

2

## MIDDLE KELL#1

FRED H. KLAER, JR. & ASSOCIATES CONSULTING GROUND-WATER GEOLOGISTS AND HYDROLOGISTS

We will be glad to discuss the results of these tests you in more detail at your convenience.

Very truly yours,

FRED H. KLAER, JR. AND ASSOCIATES

Fed H. Klassy.

Fred H. Klaer, Jr. Consulting Ground-Water Geologist and Hydrologist.

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Jate Started.	•••••		••••••				
					FORMA'	TIONS	
DEPTH TO	DEPTH TO	THICK-	STATIC	1-HOUR B	AILING TEST	HOW FAR DID	
TOP OF STRATUM	BOTTOM OF STRATUM	NESS OF STRATUM	WATER LEVEL	AVERAGE G. P. M.	DRAWDOWN FEET	FORMATION HEAVE	FORMATION FOUND REMARK
01	12*	121					Top soll
12	33	21			-		Blue clay
-33	38	5	24.7"		-		Dirty gravel; e little water
38	60	22	-				blue cley
60	68	8	24.7"				Dirty gravel
68	88	20	-	······································			Red clay
38	99	11	-				Cray clay
99	103	 	41*	103' #1	12" pipe in	hole	Gravel
103	106	3	41.	hambed 5	50 g.p.m.	AT 68'6"	THTTOM TTRUE POULS
106	111	5	-		s level art	er 12 hrs.	Gray
111	115	žą. T	-	-			Linestone; layers of clay Gray linestone
<u>    115</u> 120 <u>    130    </u>	120 130 138	10	41*		· ·		Ligestone and clay Gray limestone
	Toledo Ro	gers					

S-G P-897

DRILLER

(SKETCH OF LOCATION ON BACK OF THIS LOG SHEET)

COLUMBUS, OHIO 12" LOG OF TEST WELL No. 2

Battelle Memorial Institute City West Jefferson State Ohio For.

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Location

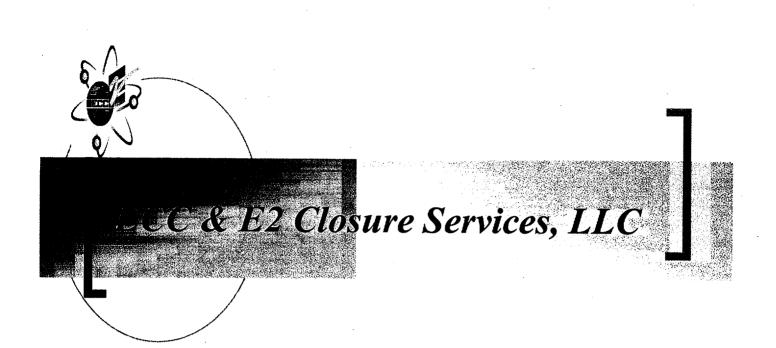
Date Started 3-16-55

## FORMATIONS

	DEPTH TO	<b>ДЕРТН ТО</b>	THICK-	STATIC	1-HOUR B	AILING TEST	HOW FAR DID	
OP OF RATUM	BOTTOM OF STRATUM	NESS OF STRATUM	WATER LEVEL	AVERAGE G. P. M.	DRAWDOWN FEET	FORMATION HEAVE	FORMATION FOUND REMARKS	
01	61	61					Yellow clay	
6	7	11			· · · · · · · · · · · · · · · · · · ·		Sand and gravel	
7	32	25					Blue clay	
32	34	2					Sloppy sand	
34	80	46					Clay and gravel	
80	98	18					Brown clay	
98	102	<u> </u>	2015"				Red clay and soft rock	
.02	115	13					Brown limestone	
15 18	118 119	3					Brown lime and clay Break	
19 .22	122 123	3	18151				Brown limestone Break	
.2 <u>3</u> .28	128 130	5					Brown lime Blue shale	

Andy Ego DRILLER.

MATERIAL: 107' of 12" pipe (extending 1'6" above ground) PERMANENT LOG SHOWING WELL FORMATIONS AND CONSTRUCTION AND PUMPING (SKETCH OF LOCATION ON BACK OF THIS LOG SHEET) EQUIPMENT WILL BE PREPARED AS SOON AS PUMPING EQUIPMENT IS INSTALLED.



2

# **Environmental Monitoring Plan**

**EM-AP-001** 

Columbus Closure Project June, 2004

Contract Number: DE-AC24-04OH20171

"A Path to Closure"



## **ECC & E2 Closure Services**

#### ENVIRONMENTAL MONITORING PLAN

EM-AP-001 **Revision** 0

## **Columbus Closure Project** Columbus, OH

June, 2004

**Reviewed by Technical Manager** 

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Approved by Sit

.01/0 a

Approved by QA Manager

✓ Keith D. Anderson

Approved by Independent Oversight Manager

<u>6-24-04</u> Date <u>6/24/04</u> Date

06./24/01 Date

<u>6/24/04</u> Date

<u>06/24/04</u> Date

Controlled Copy No:

Contract Number: DE-AC24-04OH20171

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## TABLE OF CONTENTS

1.0	Purpose and Scope
1.1	An Environmental Monitoring Plan 7
1.2	Effluent Monitoring
1.3	Environmental Surveillance8
1.4 Gui	Radiation Protection and Environmental Standards, Recommendations, and dance9
2.0	Environmental Monitoring Guidance11
R 2	Nuclear Regulatory Commission Rules and Guidance
Star 2 F 2	American Society of Mechanical Engineers (ASME) and American National         ndard Institute (ANSI
A 2	.3.1EPA Standards in 40 CFR Part 61, National Emission Standards for HazardousAir Pollutants.12.3.240 CFR Part 141 EPA Standards for Community Drinking Water Systems.12.3.3Ohio Environmental Protection Agency.12
	Legislative Acts       12         .4.1       Endangered Species Act       12         .4.2       Federal Wild and Scenic Rivers Act       13
2.5	DOE
2.6	EPA
2.7	Non-Radiological Monitoring
3.0 Moni <i>3.1</i>	The Technical Basis for the Columbus Closure Project Environmental         toring Plan       14         Present Decommissioning Activities       15
	1.1.1 Decontamination and Decommissioning Activities
3.2 3	Potential Source Terms       16         9.2.1       West Jefferson Site – Airborne Releases       16



3.2.1	2 West Jefferson Site – Liquid Effluent Discharges	16
3.2.		
3.2.	4 West Jefferson Site – Ground Water	19
3.3	Estimated Radiation Doses to the Public	
3.4	Atmospheric Modeling	24
3.5	Non-Radiological Contaminant Inventory	
3.6	Implications for Effluent Monitoring and Environmental Surveillance	25
3.6.		
3.6.	1 0	
3.6.	3 General Environmental Surveillance	
4.0 L	ocation of Monitoring Stations, Type/Frequency of Sampling and Anal	yses 26
4.1	Effluent Monitoring Summary	
4.2	Airborne Effluent Monitoring	
4.2.		
4.2.		
4.2.	3 Building JN-3	
4.3	Liquid Effluent Monitoring	31
4.4	Environmental Surveillance	31
	Environmental bar ventance	
4.5		
4.5	Integration of Columbus Closure Project Emergency Management Plan ir onmental Monitoring Program	nto the
4.5 Enviro 5.0 S	Integration of Columbus Closure Project Emergency Management Plan ir conmental Monitoring Program	nto the 
4.5 Enviro 5.0 S Consequ	Integration of Columbus Closure Project Emergency Management Plan in conmental Monitoring Program	nto the 
4.5 Enviro 5.0 S Consequ 5.1	Integration of Columbus Closure Project Emergency Management Plan in conmental Monitoring Program campling and Analyses Methodology and Determination of Off-Site Implence Assessments Sampling and Monitoring.	nto the 
4.5 Enviro 5.0 S Consequ	Integration of Columbus Closure Project Emergency Management Plan in conmental Monitoring Program ampling and Analyses Methodology and Determination of Off-Site Implence Assessments Sampling and Monitoring Analyses	nto the 
4.5 Enviro 5.0 S Consequ 5.1 5.2	Integration of Columbus Closure Project Emergency Management Plan in conmental Monitoring Program ampling and Analyses Methodology and Determination of Off-Site Implence Assessments Sampling and Monitoring Analyses Chain-of-Custody Procedures for handling Environmental Samples	ato the 
4.5 Enviro 5.0 S Consequ 5.1 5.2 5.3	Integration of Columbus Closure Project Emergency Management Plan in conmental Monitoring Program ampling and Analyses Methodology and Determination of Off-Site Implence Assessments Sampling and Monitoring Analyses	nto the 36 <b>pact and</b> 36 37 37 37 37
4.5 Enviro 5.0 S Consequ 5.1 5.2 5.3 5.4 5.5	Integration of Columbus Closure Project Emergency Management Plan in onmental Monitoring Program ampling and Analyses Methodology and Determination of Off-Site Imp nence Assessments Sampling and Monitoring Analyses Chain-of-Custody Procedures for handling Environmental Samples Data Analysis and Statistical Treatment of Data	nto the 
4.5 Enviro 5.0 S Consequ 5.1 5.2 5.3 5.4 5.5 6.0 T	Integration of Columbus Closure Project Emergency Management Plan in onmental Monitoring Program ampling and Analyses Methodology and Determination of Off-Site Implence Assessments Sampling and Monitoring Analyses Chain-of-Custody Procedures for handling Environmental Samples Data Analysis and Statistical Treatment of Data Determination of Off-Site Impact and Consequence Assessments The Environmental Monitoring Plan's Quality Assurance Program	nto the 
4.5 Enviro 5.0 S Consequ 5.1 5.2 5.3 5.4 5.5	Integration of Columbus Closure Project Emergency Management Plan ir onmental Monitoring Program	nto the 36 0 act and 36 
4.5 Enviro 5.0 S Consequ 5.1 5.2 5.3 5.4 5.5 6.0 T 6.1	Integration of Columbus Closure Project Emergency Management Plan in onmental Monitoring Program	ato the 36 <b>Dact and 36</b> 
4.5 Enviro 5.0 S Consequ 5.1 5.2 5.3 5.4 5.5 6.0 T 6.1 6.1	Integration of Columbus Closure Project Emergency Management Plan in onmental Monitoring Program	ato the 36 <b>Dact and 36</b> 
4.5 Enviro 5.0 S Consequ 5.1 5.2 5.3 5.4 5.5 6.0 T 6.1 6.1 6.1	Integration of Columbus Closure Project Emergency Management Plan in onmental Monitoring Program	ato the       36         pact and       36
4.5 Enviro 5.0 S Consequ 5.1 5.2 5.3 5.4 5.5 6.0 T 6.1 6.1 6.1 6.1	Integration of Columbus Closure Project Emergency Management Plan in onmental Monitoring Program	ato the 

••



6.5	Instrumental Analyses	
6.6	Data Reduction, Storage, and Reporting	42
6.7	Quality Assurance Records	
7.0	Implementation of the Environmental Monitoring Plan	
7.1	Environmental Monitoring Plan Implementation	
<i>7.2</i>	Environmental Monitoring Activities	
7.3	Ground Water Protection Monitoring	44
8.0	References	44



## List of Figures

Figure 1. Local Vicinity Map of North Research Area – West Jefferson Site	15
Figure 2. North Research Area – West Jefferson North	17
Figure 3. Map of Site Air, Water, Sediment, and Sampling Locations	18
Figure 4. North Site Ground Water Monitoring Wells	20
Figure 5. Remediated Filter Bed Area	20
Figure 6. Map of TLD Locations within Three-Fourth –Mile Radius of the Columbus	
Closure Project	34
Figure 7. Map of Grass, Food Crop, and Soil Sampling Locations	35
Figure 8. Wind Rose Pattern (Wind from) for West Jefferson Site	40
Figure 9. Closure Services Organizational Chart	43



## List of Tables

Table 1. Federal and State Environmental Statutes & Regulations Guidance	. 10
Table 2. Radiological Significance of West Jefferson North Buildings	. 16
Table 3. Radionuclides in Media of Filter Beds (Historical Data - 1988)	. 19
Table 5. Population Within 50 Miles – West Jefferson Facility	. 24
Distance in Miles	. 24
Table 6. Effluent Monitoring Program for Columbus Closure Project	. 27
Table 7. Environmental Surveillance program for Columbus Closure Project	. 33
Table 8. Average Percent Frequency of Wind Direction (Wind From) and Average Wind	
Speed (1990)	. 39



## 1.0 **Purpose and Scope**

## 1.1 An Environmental Monitoring Plan

The ECC & E2 Closure Services (Closure Services) Environmental Monitoring Plan (EMP) is developed to meet the requirements of Section C(4)(2)(B) of Contract DE-AC24-04OH20171 (the Contract) between Closure Services and the U.S. Department of Energy (DOE). Section C(4)(2)(B) requires Closure Services to provide an environmental monitoring program and annual report to demonstrate that all discharges and releases are in compliance with regulatory requirements. The environmental monitoring program includes collection of required samples from on-site and off-site locations to ensure that cross-contamination as a result of work activities has not occurred.

Closure Services recognizes and accepts within the boundaries of the Contract, the BMI background data, information and technical justification that formulates and describes environmental monitoring and surveillance during decontamination and decommissioning (D&D) activities at the West Jefferson North (WJN) facility in West Jefferson, Ohio. In turn, Closure Services has developed this plan to:

- Evaluate control measures for prevention of releases of radioactive materials to the environment;
- Characterize the nature and amount of any release;
- Assess the transport and fate of materials released in the environment;
- Estimate potential doses to the most probable receptors of the general public; and
- Ensure that regulatory, permit, and license conditions for the protection of the public and the environment.

BMI will periodically monitor non-radiological parameters according to the National Pollution Discharge Elimination System (NPDES), as reflected in the approval of Application No. OH0005461.

Closure Services will ensure that the number and frequency of selected sampling locations for the environmental monitoring activities effectively demonstrates compliance to the above listed parameters.

The EMP describes the direct measurement, collection, and analysis of various media by effluent monitoring and environmental surveillance. Closure Services staff perform effluent monitoring at potential points of release of radioactive materials or other regulated pollutants to the environment. Effluent monitoring includes routine monitoring of specific emission sources, general environmental surveillance of liquid and atmospheric media, and emergency response at the WJN. Section 1.2 of this plan details effluent monitoring activities.

Closure Services staff perform environmental surveillance by sampling and analyzing various media on and off the WJN facility. These include air, surface water, groundwater,



grass, fish, food crop[s], sediment, and soil periodically collected and analyzed for radionuclides and chemicals of concern. Section 1.3 details environmental surveillance activities. Closure Services will report the results of the monitoring and the surveillance activities to the public in an annual Site Environmental Monitoring Report (SEMR).

The Closure Services Environmental Monitoring (EM) and Radioanalytical laboratory (RAL) administrative and operating procedures contain specific details for sampling and analytical activities.

## 1.2 Effluent Monitoring

Effluent monitoring is the collection and analysis of airborne and liquid effluents for the purpose of characterizing and quantifying contaminants, assessing the exposure of members of the public to radiation and chemical sources, providing a means to control effluents at or near the point of discharge, and demonstrating compliance with applicable government standards.

Potential liquid source terms include sprays and waste water from decontamination activities, out-flow from excavation areas, and laboratory drains. Decontamination operations will be conducted within areas constructed to control the release of decontamination liquids. Engineered controls include spray barriers and collection basins. Out-flow from excavations will be prevented by constructing barriers and pumping liquids into holding tanks. Collected liquids will be filtered, sampled, and verified as meeting the applicable NPDES limits as stated in the approved Application No. OH0005461, tested against the acceptance criteria of and transferred to a local publicly owned treatment works (POTW). Closure Services will ensure that the transfer is coordinated and communicated with the DOE and BMI.

Discharges from the RAL will be collected and transferred to the permitted BMI treatment works.

Stack air samplers will continuously monitor the exhaust stack emissions from the potential source contributors (i.e. JN-1, JN-2) to assess the effectiveness of the systems controlling airborne emissions. Once the source term is removed from the buildings, the stack monitors will be disabled and this requirement will be closed out. Dust suppression will be implemented and supplemental air monitoring will be conducted during demolition of structures.

## 1.3 Environmental Surveillance

Environmental surveillance is the collection and analysis of samples, or direct measurement, of air, water, soil, foodstuffs, biota, and other media from the sites and their environs to determine compliance with applicable standards and permit requirements and to assess the radiation and chemical exposure of members of the public and its effect, if any, on the local environment.



Closure Services will conduct continuous air monitoring to evaluate the impact of releases of radioactive and hazardous substances to the general public and the environment. A comprehensive air monitoring program is important as air is the primary exposure pathway to the general public during the D&D activities. Releases to the air may also lead to the deposition of particulates in the environment. As such, the environmental surveillance activities include other pathways that may result in contributing dose due to transfer or uptake of particulate contamination.

# 1.4 Radiation Protection and Environmental Standards, Recommendations, and Guidance

Normally, "radiation protection standards, recommendations, and guidance<sup>1</sup>" are defined separately from other "environmental standards, recommendations, and guidances." Radiation protection standards specify limits on exposure that are regarded as necessary for protection of public health and should be met, except in the case of accidents or emergencies, regardless of cost. As used in this document, the terms "radiation protection standards, recommendations, or guidance" refer to standards, recommendations, or guidance that are generally applicable to all sources of exposure, exclusive of natural background radiation and deliberate medical practices.

The term "environmental radiation standards or guidance" specifies limits on exposure for particular practices or sources. Most "environmental radiation standards and guidances" are judgmental and are based on the "as low as reasonably achievable" (ALARA) principle. The one exception is the U.S. Environmental Protection Agency (EPA) standard for airborne radionuclide emissions in 40 CFR Part 61, which is based on limits on lifetime risk for maximally exposed individuals and average individuals in large population groups.

Environmental monitoring standards are defined primarily by the EPA, the Occupational Safety and Health Administration, and associated state agencies. These standards are detailed in applicable sections of 40 CFR and 29 CFR pursuant to regulations promulgated under the Clean Water Act, Clean Air Act, Safe Drinking Water Act, Resource Conservation and Recovery Act, and the Occupational Safety and Health Act (see Table 1). Table 1 lists the federal and state environmental statutes and regulations that provide guidance for conducting environmental monitoring. The listed guidance does not necessarily establish the compliance basis for conducting the D&D activities.



Regulator	Regulation	Description	Compliance Status	
Council for National Env. Environmental Quality/DOE Policy Act		Federal agencies must follow prescribed process to evaluate impacts on the environmental of major federal actions and alternatives.	Activities consistent with the existing BCLDP Environmental Assessment and Finding of No Significant Impact.	
EPA Resource Conservation & Recovery Act		Governs the generation, storage, handling and disposal of hazardous waste.	RCRA compliance is the responsibility of Closure Services Waste Management Group, large generator operating under 90-day storage limitations.	
EPA	Clean Air Act	Regulates the release of air pollutants through the use of permits and air quality limits	Administered in Ohio by the OEPA	
EPA Clean Water Act		Seeks to improve the quality of surface waters by implementing a permitting program and establishing water quality standards.	Administered in Ohio by the OEPA	
ЕРА	Safe Drinking Water Act	Establishes minimum drinking water standards and monitoring requirements	Administered in Ohio by the OEPA	
EPA	Toxic Substance Control Act	Regulates the manufacture, use and distribution of all chemicals	Administered by the USEPA	
US Fish & Wildlife Service	Endangered Species Act	Establishes threatened and endangered categories of wildlife and provides protection for critical habitats.	The State of Ohio lists 5 species of fish (including 1 federal endangered) and 8 species of mollusks (including 2 federal endangered) identified along Big Darby Creek.	
NPS	Federal Wild & Scenic Rivers Act	Provides preservation of wild and scenic free-flow rivers in their natural condition.	The Big Darby Creek has been designated as a component of the National Wild & Scenic Rivers system. CCP activities are not subject as there is no affect the free flowing nature of the Big Darby Creek.	
Advisory Council on Historic Preservation	National Historic Preservation Act	Identifies, evaluates, and protects historic properties eligible for listing in the National Register of Historic Places.	WJN facilities not eligible for inclusion into the national Register of Historic Places.	
EPA	Federal Insecticide, Fungicide, & Rodenticide Act	Governs the manufacture, use, storage, and disposal of pesticides and herbicides, as well as pesticide containers and residues	This act is not applicable to CCP. Pesticides used in CCP areas are USEPA registered and purchased from a registered establishment.	
EPA	Superfund Amendments and Reauthorization Act, Title III	Requires reporting of emergency planning information, hazardous chemical inventories, and environmental releases to federal, state and local authorities.	Closure Services reports under EPCRA 311-312: Material Safety Data Sheet/Chemical Inventory.	
DOE	Executive Order 11990, Protection of Wetlands	Established to mitigate adverse effects to wetlands and to avoid construction in wetlands.	CCP operations should not impact any wetland areas.	

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Table 1. Federal and State Environmental	Statutes & Regulations Guidance
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## 2.0 Environmental Monitoring Guidance

Numerous government orders, standards, guidance, and criteria influence an EMP. They determine the content and form of an EMP, and they also influence its intent. In addition, documents published by the American National Standards Institute (NSI) and the American Society of Mechanical Engineers (ASME) describe specific radiation instrument standards and support certain aspects of the EMP, specifically quality assurance.

## 2.1 Nuclear Regulatory Commission Rules and Guidance

# 2.1.1 NUREG/CR-5212, Emergency Environmental Sampling and Analysis for Radioactive Material Facilities.

NUREG/CR- $5212^2$  provides information that could be used in an environmental sampling and analysis program for emergency or non-routine events. Sample collection and measurement locations, sample collection procedures, and quality assurance programs are applicable to this project.

# 2.1.2 10 CFR Part 20, Subpart D, Radiation Dose Limits for Individual Members of the Public.

10 CFR Part 20, Subpart  $D^{3,a}$  establishes radiation dose limits for the public.

## 2.2 American Society of Mechanical Engineers (ASME) and American National Standard Institute (ANSI

## 2.2.1 ASME NQA-1a, Quality Assurance Program Requirements for Nuclear Facilities.

The Closure Services Quality Assurance Plan is based, in part, the requirements of ASME NQA-1a<sup>4</sup>. This EMP has been prepared in accordance with the Closure Services Quality Assurance Plan and implementing procedures.

## 2.2.2 ANSI N42.18-1974, Specifications and Performance of Onsite Instrumentation for Continuously Monitoring Radioactivity in Effluents.

ANSI N42.18-1974<sup>5</sup> applies to continuous monitors that measure normal releases, detect inadvertent releases, show general trends, and annunciate radiation levels that have exceeded predetermined values.

## 2.3 Environmental Protection Agency

# 2.3.1 EPA Standards in 40 CFR Part 61, National Emission Standards for Hazardous Air Pollutants.

40 CFR Part 61<sup>6</sup> establishes limited on the annual effective dose equivalent for DOE facilities emitting any radionuclide, other than radon, and other pollutants, except disposal facilities.

## 2.3.2 40 CFR Part 141 EPA Standards for Community Drinking Water Systems

40 CFR Part 141<sup>1</sup> applies 1) to public or private water systems with at least 15 service connections or serving at least 25 persons and 2) at the tap rather than at the source.

## 2.3.3 Ohio Environmental Protection Agency

The Ohio EPA has established discharge limitations as part of its National Pollutant Discharge Elimination System (NPDES) permit for the West Jefferson facility. The discharge limitations are based on daily and monthly concentration and loading factors.

In addition, the Ohio EPA has established maximum contaminant levels for inorganic chemicals and microbiological contaminants which must be met at the West Jefferson facility. The facility operates as a non-transient, non-community water supply.

## 2.4 Legislative Acts

## 2.4.1 Endangered Species Act

Closure Services recognizes the importance of wildlife. Sampling procedures specify that care should be taken to avoid the collection of any endangered species. Any wildlife species that are inadvertently captured during sample collection along Big Darby Creek are to be released.

At the WJN Site, the following endangered species have been identified:

## Endangered Fish

- Goldeye (Hiodan alosoides)
- Northern Brook Lamprey (Ichthyomyzon fossor)
- Northern Madtom (Noturus stigmosus)
- Scioto Madtom (Noturus trautmani) (also federal endangered)
- Spotted Darter (Etheostorna maculatum)

Endangered Mollusks

- Clubshell (Pleurobema clava) (also federal endangered)
- Elephant-ear (Elliptio crassiens)
- Northern riffleshell (Epioblasma torulosa rangiana) also federal endangered)
- Pocketbook (Lapsilis ovata)
- Rabbitsfoot (Quadrula cyclindrical)
- Rayed Bean (Villosa fabalis)
- Snuffbox (Epioblasma trquertra)
- Washboard (Megalonaias nervosa)

## 2.4.2 Federal Wild and Scenic Rivers Act

The Big Darby Creek was designated as a component of the National Wild and Scenic River System in 1994. At the present time, Closure Services activities are not subject to the requirements under this act, because they do not affect the free-flowing nature of the big Darby Creek. Additional state or local requirements may be implemented in the future.

## 2.5 DOE

Closure Services has considered the guidance as provided in DOE/EH-0173T, Environmental Regulatory Guide for Radiological Effluent Monitoring and Environmental Surveillance, in developing the EMP.

## 2.6 EPA

The EMP will use airborne effluent monitoring and environmental surveillance sampling and analyses techniques to meet the requirements of EPA's National Emission Standards for Hazardous Air Pollutants<sup>6</sup>, as well as NRC's 10 CFR Part 20, Appendix B.<sup>7</sup>

## 2.7 Non-Radiological Monitoring

The criteria for the non-radiological monitoring are based on U.S. EPA, Ohio EPA, and DOE protocol and programs established by and conducted by BMI.



## 3.0 The Technical Basis for the Columbus Closure Project Environmental Monitoring Plan

Closure Services has reviewed the technical basis established by BMI for the environmental monitoring program. Closure Services does not intend to modify monitoring well location and frequency as each have been accepted by the regulators of concern. Perimeter air sampling location and frequency will remain as established by previous BMI activities. Closure Services will modify the external exposure monitoring locations to reflect currently planned site activities and the significant reduction in the source term at WJN. Closure Services intends to further modify locations and frequency of environmental monitoring and surveillance locations as D&D activities progress and the source term is removed from the site.

Figure 1 shows the WJN facility in relation to the surrounding vicinity. Closure Services monitoring protocol to best detect planned and unplanned releases of radionuclides and chemicals of concern, consistent with the potential for offsite impact due to planned activities and remaining source terms. The following sections detail planned activities and probable impacts to the health and safety of the general public and the environment.

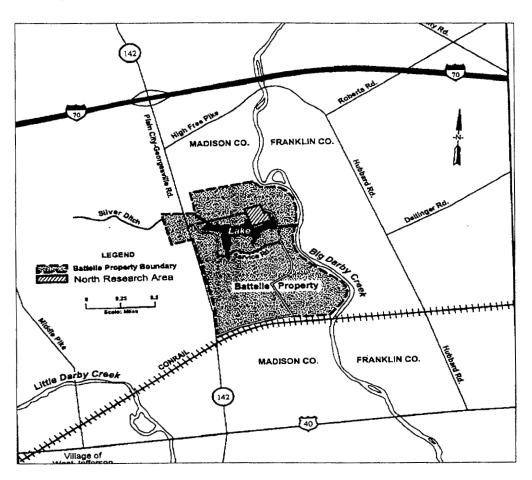


Figure 1. Local Vicinity Map of North Research Area - West Jefferson Site

## 3.1 Present Decommissioning Activities

Closure Services is completing the remaining D&D activities at the WJN facility. Radioactive contamination remaining within Building JN-1, JN-2, and JN-3 consists of mixed fission products, activation products, uranium, thorium and suspect transuranics.<sup>8</sup> Special nuclear materials have been removed from JN-1, packaged for off-site transport, and placed onto the holding pad west of JN-4. Residual radioactive contamination in the form of dust/fragment deposits remain on the surfaces of the hot cells, hot cell equipment, and on materials stored in barrels (see Table 2). All operational materials, special nuclear materials, and stored operational wastes were removed during the phase-out of operations in the buildings.

#### Table 2. Radiological Significance of West Jefferson North Buildings

Building	Major Survey Results				
JN-1	Interior of hot cells and storage rooms highly contaminated with fission products; fixed contamination along exterior surfaces of the mezzanine, top of HEC, other rooms, and sumps. Metal boxes and barrels of highly contaminated waste in an attached Waste Storage Building.				
JN-2	No significant surface contamination; a few spots of fixed contamination in the high bay and in the Radioanalytical Laboratory				
JN-3	No significant surface contamination; fixed contamination throughout. Currently, the only licensed activity conducted in JN-3 is for the storage of waste awaiting shipment for burial.				

## 3.1.1 Decontamination and Decommissioning Activities

The approach for decommissioning these facilities is to decontaminate and remove radioactive or contaminated (PCB or asbestos) facilities, equipment, materials, fluids and/or soil from the site to permit reuse of the property. For the facilities in question, this will generally involve dismantling and/or removing equipment, decontaminating building structures, demolishing the buildings, and removing and disposing of contaminated soil as a low-level radioactive waste.

## 3.2 Potential Source Terms

#### 3.2.1 West Jefferson Site – Airborne Releases

The primary potential source of airborne releases for the West Jefferson site is residual radioactive contamination from destructive and on-destructive testing conducted in several cells throughout the JN-1 Hot Cell Laboratory.<sup>8</sup>

#### 3.2.2 West Jefferson Site – Liquid Effluent Discharges

Following treatment, all sanitary systems for the West Jefferson North and Middle sites have a common discharge point (EW-1) to Big Darby Creek. Potential liquid source terms include sprays and waste water from decontamination activities, outflow from excavation areas, and laboratory drains. See Figure 2 for the location.

Decontamination operations will be conducted within areas constructed to control the release of decontamination liquids. Engineered controls include spray barriers and collection basins. Out-flow from excavations will be prevented by constructing barriers and pumping liquids into holding tanks. Collected liquids will be filtered, sampled, and verified as meeting the applicable NPDES limits as stated in the approved Application No. OH0005461. Discharges from the RAL will be collected and transferred to the permitted BMI treatment works.



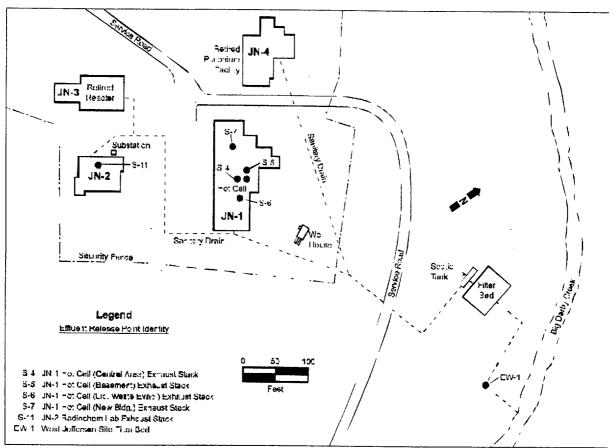
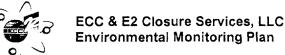


Figure 2. North Research Area - West Jefferson North

## 3.2.3 West Jefferson Site – Soil Contamination

The WJN has several general areas of soil with elevated levels of residual radioactive contamination. One area is a storm sewer outfall (SS-JN-1-4) that collects storm water runoff from the roofs of buildings JN-1 and JN-4 and surface drains at the West Jefferson North Research Area. Outfall SS-JN-1-4 was remediated in 1994 and is routinely sampled as part of the ongoing site environmental monitoring program (see sediment sampling station ED-1 in Figure 3). This outfall remains active and will not be submitted for free release until after the demolition of JN-1.



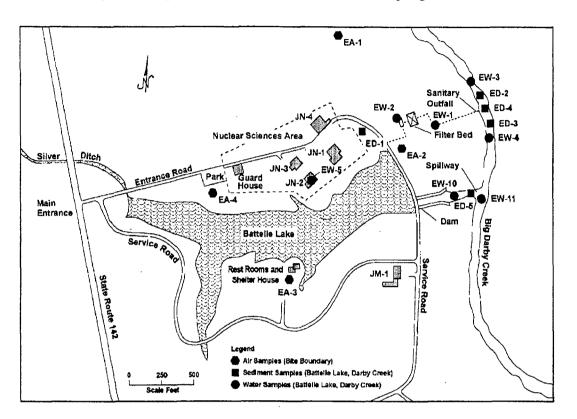


Figure 3. Map of Site Air, Water, Sediment, and Sampling Locations

Table 3 details the concentrations of Cs-137, Co-60, and Am-241 contained in two filter beds, constructed as a secondary control to filter particulates from the wastewater effluent at the WJN facility. The 10-foot deep beds are located between the dam service road and Big Darby Creek. The total volume of soil in the filter beds is less than 2,300 m<sup>3</sup> (81,000 ft<sup>3</sup>). Data from samples collected during 2000 and 2002 indicate concentrations of Cs-137 in the large bed (105 by 60 feet) range from 0.3 to 205 pCi/g, while those in the small bed (75 by 35 feet) range from 0.2 to 25 pCi/g. The maximum concentrations were measured near the surface of the filter beds. Subsequent sampling conducting after 2002 support the contamination levels at approximately the same order of magnitude as previous sampling events. The filter beds are located inside the flood plain of the Big Darby creek. Date presented in Table 3 shows that the contamination is presently immobile. Groundwater monitoring in the vicinity of the filter beds showed no release of radioactivity outside the filter beds.

An additional area of concern is subsurface contamination, located within the confines of the north site perimeter fence, about 75 feet east of JN-4.

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Radionuclides <sup>a</sup>	Large Filter Bed <sup>b</sup> (pCi/g) Maximum Concentratjon	Small Filter Bed <sup>c</sup> (pCi/g) Maximum Concentration	Activity of Both Beds (Ci x 10 <sup>-4</sup> )	CERCLA Reportable Quantity (Ci)
Cs-137	223	32	5.5	1
Co-60	1.3	0.6	10.1	10
Am-241	7.6	0.5	10.6	0.01

#### Table 3. Radionuclides in Media of Filter Beds (Historical Data – 1988)

Filter beds, located between the service road to JN-4 and Big Darby Creek were constructed as a secondary control to filter particulates from sanitary sewer effluent water for 20 years. In 1980, portions of bed media were removed, and clean sand was backfilled and blended with remaining filter media in 1982 and covered with soil.

- <sup>b</sup> Approximately 105 by 60 by 10 feet deep.
  - Approximately 75 by 35 by 10 feet deep.

#### 3.2.4 West Jefferson Site - Ground Water

#### Ground Water Monitoring Requirements

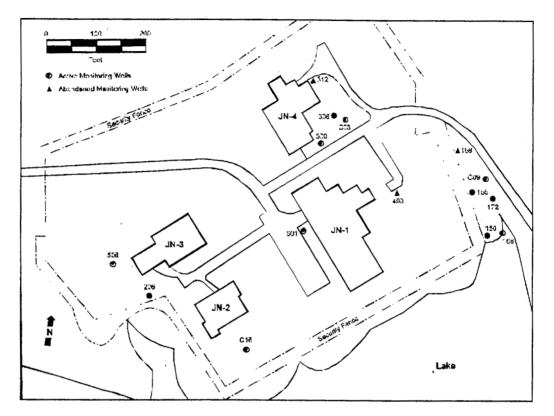
The ground water sampling program is designed generally in accordance with Ohio Administrative Code (OAC) 3745-54-92, *Ground Water Protection Standard*. Although the north area is not regulated by this standard at the present time, the rationale for ground water monitoring will be applied.

#### Radiological Ground Water Monitoring

Routine collection of ground water samples for radiological and chemical analysis has been conducted at the WJN facility at 18 shallow monitoring wells (generally 9 to 35 feet deep) and at three drinking water supply wells (only one of which is in the North Area) at least annually since 1989. See Figures 3 and 4 for well locations, and Table 4 for monitoring data for the North Area. The focus of the Closure Services EMP concerns groundwater monitoring within the project boundary of the WJN facility.

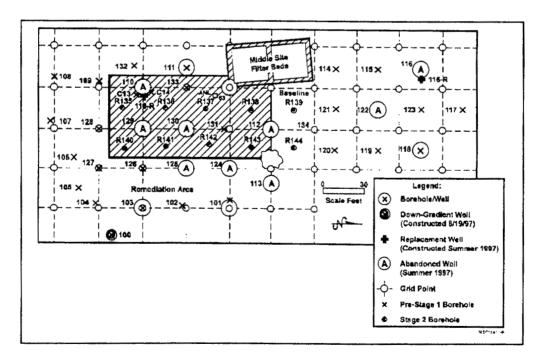
Of the 18 monitoring wells, the highest activities are shown in wells 101, 103, 110, and 118 where radioactivity remains in a former filter bed. The highest combined alpha and beta activity is in well 110. This area has been recommended for further remediation in the Final Assessment of the Radiological Status of Battelle's Nuclear Sciences Area, dated January 1991.<sup>9</sup> Concentrations of radionuclides in the filter beds are summarized in Table 3. Wells C09, 168, and 172 are located to the east of the Nuclear Sciences are near the sewer outfall, where Cs-137, Co-60, Am-241, and Pu-239/240 have been measured. Wells 206 and 506 are located to the south and west of JN-3.

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## Figure 4. North Site Ground Water Monitoring Wells

Figure 5. Filter Bed Area





Well Identification <sup>a</sup>	Location	PCi/L		
		Gross alpha ± sigma <sup>b</sup>	Gross beta ± sigma <sup>t</sup>	
JN-Active Supply Well	Nuclear Sciences Area: East of JN-1	8.93 ± 3.17	2.87 ± 1.35	
JM-Active Supply Well	West Jefferson Middle Area	$7.99 \pm 3.42$	$5.00 \pm 1.51$	
JS-Active Supply Well	West Jefferson Middle South Area	$6.35 \pm 3.06$	4.93 ± 1.49	
CO3	East of JN-4	8.39 ± 4.95	8.43 ± 2.38	
CO9	Storm Sewer Outfall	8.77 ± 4.76	$30.00 \pm 3.42$	
C16	SE of JN-2	$7.97 \pm 4.74$	$3.80 \pm 2.00$	
100	SE of Filter Bed	$13.30 \pm 5.42$	$5.98 \pm 2.13$	
101	E of Filter Bed	$18.80 \pm 5.12$	$14.30 \pm 2.40$	
103	SE of Filter Bed	$11.10 \pm 5.99$	$13.30 \pm 2.76$	
110	W of Filter Bed	$20.90 \pm 6.96$	$15.30 \pm 2.73$	
116	N of Filter Bed	$28.30 \pm 7.67$	$16.50 \pm 2.78$	
118	E of Filter Bed	$26.00 \pm 7.26$	$22.40 \pm 3.06$	
150	Storm Sewer Outfall	$7.73 \pm 4.47$	$6.25 \pm 2.18$	
155	Storm Sewer Outfall	$6.36 \pm 4.59$	$6.53 \pm 2.23$	
168	Storm Sewer Outfall	9.38 ± 5.89	$19.90 \pm 3.25$	
172	Storm Sewer Outfall	$14.70 \pm 6.52$	27.70 ± 3.54	
206	S of JN-3	8.22 ± 5.15	$7.60 \pm 2.33$	
300	SE of JN-4	$7.94 \pm 23.10$	$11.70 \pm 2.63$	
306	E of JN-4	$9.71 \pm 4.67$	8.76 ± 2.31	
506	W of JN-3	$13.10 \pm 6.17$	8.95 ± 2.45	
601	W of JN-1	$19.10 \pm 5.61$	$10.40 \pm 2.29$	

#### Table 4. Radiological Analyses of Ground Water at the West Jefferson Site in 2000

 Adapted from Battelle BCLDP, "BCLDP Site Environmental Report for Calendar Year 2000 on Radiological and Non-Radiological Parameters."

Minimum Detection Limit for gross alpha is 1.0 pCi/L; for gross beta is 2.9 pCi/L.

- During the last half of CY 1995, an environmental geophysics study was conducted at the remediated filter bed area at the West Jefferson site. The study was conducted to define the hydrogeologic framework, characterize potential contamination pathways, and identify possible leakage points in buried pipelines and drainage tile. A total of six shallow piezometers were installed near the retired filter bed area during September of CY 1995. The six new well points were sampled on October 10, 1995, and received gamma spectroscopy analyses. Results from the analyses showed there were no radionuclides present. An additional sampling event of the same six well points conducted on June 5, 1996, yielded identical results. <sup>10</sup>
- The ground water located adjacent to the underground storage tanks located near JN-1 will be sampled annually. The samples will be tested for radiological parameters and PCBs.

Closure Services will conduct sampling and radioanalytical analysis of monitoring wells #118, #206, #306, #506, and #601 on a semi-annual basis as indicated in Table 7. Additionally, monitoring wells #118 and #306 will be sampled on an annual basis for the radioanalytical components as listed in Table 7.

#### Non Radiological Liquid Effluent Monitoring

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- The drinking water system at the West Jefferson site is monitored under Ohio EPA regulations, which regulate all public water supplies. Because this is a non-transient, non-community water supply, BMI will perform various tests.
   Monitoring parameters include total coliform for microbiological contamination on a quarterly basis, and VOCs, SOCs, asbestos, nitrates, MCL inorganics, copper, and lead on a schedule ranging from annual to every three years. The drinking water system has consistently met water quality monitoring requirements established by the Ohio EPA.
  - The three existing supply wells (one located in each of the North, Middle, and South areas) have depths ranging from 130 to 160 feet and have been monitored annually and semi-annually for radiological and drinking water parameters since 1970. The three existing supply wells (JN-W, JM-W, and JS-W) are sampled before the water is treated and have undergone analysis for gross alpha and gross beta emitters, fission products, in addition to Ohio EPA parameters for drinking water supply evaluation. The three supply wells have consistently met water quality monitoring requirements established by the Ohio EPA. Closure Services will only sample and monitor the JN-W supply well. BMI will continue to conduct all other non-radiological effluent monitoring activities.
  - BMI performed detailed chemical monitoring with the results reported in the Interim Report on Site Characterization, West Jefferson North Site, Stage 1 Sampling and Analysis: Chemical Sampling Summary Report, <sup>11</sup> dated December 22, 1989. The results showed the groundwater samples to be free from chemical contamination.
  - Chemical sampling has been performed in three monitoring wells (C03, C09, and C16) since their installation through 2001 on an annual basis. The samples have been analyzed for eight heavy metals, 26 pesticide and PCB compounds, 36 volatile organic compounds (VOCs), 65 semi-volatile organic compounds (SOCs), oil and grease, and pH. These monitoring wells have depths ranging from 8.5 to 15 feet and have been monitored since 1989. No ground water contamination was detected in any of the wells when they were initially sampled.
  - Detailed chemical analyses have been performed annually since 1991 on ground water samples from three chemical monitoring wells (C03, C09, and C16). Samples from all three wells have been analyzed for eight heavy metals, 26 pesticides and PCB compounds, 36 VOCs, 65 SVOCs, oil and grease, and pH. The shallow wells were constructed solely for monitoring purposes. Although ground water from these shallow monitoring wells does not represent site drinking water, the results are compared to U.S. EPA



Primary Drinking Water Standards to put any observed concentrations in perspective.

- Well C03 showed traces of phenol at 17 parts per billion (ppb) (ug/L) during sampling for CY 1991. No traces of any chemical contaminant have been found in this well during sampling since 1991.
- Wells C09 and C16 have shown traces of bis (2-ethylexyl)-phthalate and 1,1,1-trichoroethane in an on-again, off-again pattern during the 1992-2000 sampling time frame. Various factors not related to the site condition, such as laboratory or shipping container contaminants, may account for the presence of these compounds at low concentrations ranging from 5 to 41 ppb (ug/L).

Closure Services will conduct sampling and chemical analysis of monitoring wells #C03, #C09, and #C16 on the frequency as listed in Table 7.

## 3.3 Estimated Radiation Doses to the Public

Estimates of doses to the public and workers are contained in "Finding of No Significant Impact (FONSI) and Environmental Assessment (EA)" June 1990. In 2001, the EA was supplemented by the addition of current conditions and information. The DOE has maintained that the FONSI is still valid for the CCP, Workers' doses are not employed in the environmental surveillance and monitoring criteria and are not evaluated in estimated radiation doses to the public.

WJN falls below an estimated effective dose equivalent of 0.1 mRem. Hence, effluent monitoring requires only periodic confirmatory measurements; calculation of dose for normal operations, assuming that the emission controls are non-operative; and a confirmatory environmental survey at least every five years. Effluent air monitoring requires total beta and total alpha as an indicator and gamma spectrometry on an annual basis.

The filter bed area estimated radiation dose is such that an annual environmental surveillance and analysis is suggested. The estimated radiation dose to a farm family living at the outfall from consumption of crops is such that routine surveillance of all pathways is recommended. Closure Services will sample farm and garden produce in the general area of the outfall to insure that an annual effective dose equivalent of 5 mRem is not exceeded.

Table 5 represents the population distributions around the West Jefferson site. The estimated annual person-rem collective EDE within 80 km (approximately 50 miles) West Jefferson site is about 7.2 person-mRem  $(3.13 \times 10^{-6} \times 2.3 \times 10^{6})$ .

Based on the above, the West Jefferson site requires periodic confirmation (an estimated annual collective effective dose equivalent of less than 25 person-mRem).



#### Table 5. Population Within 50 Miles - West Jefferson Facility

	0 to 1	1 to 2	2 to 3	3 to 4	4 to 5	5 to 10	10 to 20	20 to 30	30 to 40	40 to 50	Total *
North	13	17	98	118	172	3,565	5,504	5,365	9,785	47,453	72,090
NNE	8	34	147	116	192	2,648	33,177	32,783	7,950	19,933	96,988
NE	13	46	76	136	581	30,040	121,109	24,423	11,754	14,779	202,957
ENE	14	257	96	200	2,386	36,041	244,383	71,416	12,862	16,441	384,096
East	309	441	130	203	4,059	41,891	247,925	133,142	25,911	76,727	530,738
ESE	769	589	219	114	332	54,788	73,058	37,024	50,406	20,607	237,906
SE	297	1,046	43	65	419	5,986	18,009	15,228	13,430	9,535	64,058
SSE	45	256	47	54	77	3,349	11,226	5,499	14,437	43,088	78,078
South	26	85	82	116	115	860	4,671	3,116	5,026	12,733	26,830
SSW	7	489	260	135	96	584	1,220	3,925	21,493	7,859	36,068
SW	2	324	2,971	514	42	806	1,019	3,647	8,812	20,209	38,346
wsw	3	24	307	126	14	1,269	9,620	5,916	19,660	173,870	210,809
West	5	23	121	173	163	694	9,880	63,943	53,643	81,331	209,976
WNW	8	14	20	36	94	1,061	3,625	17,383	7,528	7,923	37,692
NW	14	15	29	88	83	439	1,369	5,833	22,469	13,509	43,848
NNW	17	4	45	495	87	676	14,262	10,010	4,361	7,133	37,090
Total	1,550	3,664	4,691	2,689	8,912	184,697	800,057	438,653	289,527	573,130	2,307,570

#### **Distance** in Miles

Block-level data was used in the population calculations. In cases where sector lines plit blocks, the population for the block was allocated based on the proportion of the block area in that sector. Source: 2000 Public Law 94-171, U.S. Bureau of the Census. Prepared by: Office of Strategic Research, Ohio Department of Development. (June 2001)

## 3.4 Atmospheric Modeling

Present locations of air samplers at the West Jefferson site were determined through the use of meteorological modeling, which determined maximum ground level air concentrations.<sup>12</sup> This study was done for West Jefferson nuclear research operations and may not be appropriate for D&D activities. Closure Services will maintain the same air monitoring locations, enhanced monitoring during demolition activities, and close sample locations as source term is eliminated.

## 3.5 Non-Radiological Contaminant Inventory

A sampling and analysis program for chemical contaminants was performed in the past by BMI, some hydrocarbons were identified and remediated, PCBs were identified near a transformer at levels well below action levels, and very low levels of volatile and semivolatile organics were identified in one sludge sample near the storm sewer and a few soil samples. No contamination has been found in the groundwater.



## 3.6 Implications for Effluent Monitoring and Environmental Surveillance

The number and location of effluent monitoring stations and environmental surveillance stations, the frequency of sampling, and the type and frequency of analyses are based on technical assessments that consider the following factors:

- The inventory of radioactive isotopes in each building to be decommissioned.
- The potential for release of radiation and radioactive materials from the facilities into the environment.
- The standard radiation protection measures to be undertaken during D&D operations.
- Applicable laws, regulations, criteria, and standards.
- The capabilities and reliability of available monitoring instruments.

## 3.6.1 Airborne Effluent Monitoring

Environmental monitoring data collected over several years indicate no significant releases of radionuclides. BMI conducted air sampling of aerosol concentrations and exhaust air streams of areas during decontamination tasks. Results of the sampling indicate that facility engineered systems and work control requirement are sufficient to control and quantify emissions.

To establish the basis for continued airborne effluent monitoring during reamining D&D tasks, a comparisons was made between estimated CCP emissions and applicable regulatory requirements.

Based on the results of this comparison:

- The derived dose rate to the public that may result from decontamination activities in the West Jefferson Building JN-1 is large enough to call for continuous monitoring. The details of the airborne emission monitoring plan for JN-1 are given in Section 4.2.1.
- The derived rate to the public that may result from decontamination activities in Building JN-2 is not large enough to call for continuous monitoring. The Radioanalytical Laboratory currently located in JN-2 will be transferred to the temporary facility. Because of the potential for radiological emissions from laboratory operations, airborne emissions will be monitored. The details of the airborne emission monitoring plan for JN-2 and the new portable laboratory are given in Section 4.2.2.

## 3.6.2 Liquid Effluent Monitoring

BMI will continue to perform continuous liquid effluent monitoring at the West Jefferson site at the NPDES permitted outfall into Darby Creek. Several years of



environmental surveillance of liquid effluents at the West Jefferson site have detected no releases or exposures that approach regulatory limits.

## 3.6.3 General Environmental Surveillance

Closure Services will collect environmental surveillance data from numerous locations within the project site boundaries, at the WJN site boundaries, and off site. The current environmental surveillance program at West Jefferson is adequate to accomplish these objectives.

## 4.0 Location of Monitoring Stations, Type/Frequency of Sampling and Analyses

## 4.1 Effluent Monitoring Summary

The details of effluent Monitoring are listed in Table 6 and discussed in detail in Sections 4.2 and 4.3. Procedures for environmental sampling and analysis are listed by title and document number in Section 5.

## 4.2 Airborne Effluent Monitoring

Routine airborne effluent monitoring of D&D operations within the project boundaries will be carried out in accordance with Sections 4.2.1 through 4.2.3. The extent of airborne effluent monitoring for D&D operations will be based on a supplemental estimate of the potential airborne EDE that takes into account the local source term. For example, in a building's large open areas, where filtering room exhaust cannot be ensured, a short-term EDE would be calculated using the open area's radionuclide inventory, a room exhaust filtration factor of 1.00 (no filtration), and the duration of D&D in that area.

This plan will be reviewed and modified as necessary to comply with the goals of the EMP in the event of any change to the number or status of building air discharge points due to completion of D&D plans for the site.

The Environmental Technicians will change out filters at continuous monitoring locations, conduct performance tests, and calibrate and maintain the continuous air and stack monitors at the WJN Site.

Supplemental monitoring will be performed during demolition activities to verify that contamination control measures minimize the potential for releases to the general public and the environment. Monitoring will include upwind and downwind continuous air sampling and real time dust monitoring. Demolition crews will utilize dust suppression methods to reduce dust emissions. Dust suppression methods may be applied using fire hoses spraying building structures and debris during demolition and container loading. Temporary berms will be constructed or placed to collect runoff water resulting from spraying the demolition debris.



Type of Sample	Sampling Site(s)	Sampling and Collection Frequency	Analysis Type and Frequency
Airborne Effluent at Point Sources, ab	One-in-line volumetric sampler at each active fan driven exhaust vent at Buildings JN-1 and JN-2	Continuous Sampling Weekly Collection	Total Beta and Total Alpha Weekly. <sup>c</sup> Gamma Spectrometry Monthly Composite. <sup>d</sup> U Pu Isotopic and Sr-90 Analysis on Quarterly Composite. <sup>c</sup>
Liquid Effluent to NPDES Permitted Outfall <sup>f</sup>	Manhole immediately following chlorinators	Daily <sup>g</sup>	Flow Rate (24-hour total) Color Severity (observation) Odor Severity (observation) Turbidity Severity (observation)
	-	Bi-Weekly Collection	Dissolved Oxygen (grab) Total Suspended Solids (grab) Nitrogen Ammonia (composite) Chlorine, Total Residual (grab) <sup>h</sup> Biochemical Oxygen Demand (composite)
		Monthly	pH (grab)
		Collection	Fecal Coliform (grab) <sup>h</sup>
			Chloroform (grab)
			Alpha Total Activity (composite) Alpha Dissolved Activity (composite) Alpha, Suspended Activity (composition)
			Beta, Total Activity (composite) Beta, Dissolved Activity (composite) Beta Suspended Activity (composite)

Table 6. Effluent Monitoring Program f	for Columbus Closure Project
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- <sup>a</sup> Inactive point sources will be tagged and sealed or otherwise isolated by approved procedures prior to the start of D&D activities.
- <sup>b</sup> The procedure for air sampling is presented in EM-SP-001.
- <sup>c</sup> See RL-TP-005 for details of alpha and beta analysis. For air samples, if half life is greater than 30 minutes for beta and/or greater than 2 hours for alpha, send sample for gamma spectrometric analysis immediately.
- <sup>d</sup> RL-TP-030 describes gamma spectrometric analysis.
- <sup>e</sup> Specific Isotopic analyses: Sr-90 RL-TP-035; Ra-226 RL-TP-025; Ra-228 RL-TP-056; Plutonium RL-TP-054; Isotopic Uranium, Am-241, and Thorium RL-TP-054; H-3 RL-TP-026; and I-129 and C-14 are analyzed off-site.
- <sup>f</sup> NPDES-permitted outfall is for West Jefferson wastewater discharge to big Darby Creek. BMI will conduct all NPDES-outfall monitoring activities.
- <sup>g</sup> Except days when the facility is not normally staffed. BMI will conduct all NPDES-outfall monitoring activities.
- <sup>h</sup> summer only (May 1 through October 31).

#### 4.2.1 Building JN-1

The following monitoring plan reflects the current review and status of air discharge points at Building JN-1. It is based on a comprehensive survey of the building areas conducted in 1992.



<u>Routine</u>: Routine airborne emission monitoring during D&D operations at Building JN-1 will include:

1) Continuous monitoring of the air discharge points (stacks), with weekly sample collection and analyses as listed in Table 6:

(S-7)002 – New Building (HEC) (S-4)003 – Control Area (CAA) (S-6)004 – Liquid Waste Evaporator (S-5)013 – Basement (A/G)

The air monitoring devices will have adjustable set points and have the capability to alarm and shutdown blowers, if the set point is reached.

The locations of these discharge points are shown in Figure 2. Monitoring these air discharge points will be continued until D&D of the specific area(s) serviced by a given stack are completed and the discharge point is sealed. Inactive point sources will be sealed, locked out, disabled, or otherwise isolated to ensure that inadvertent radiological releases via unmonitored pathways during D&D are prevented.

- 2) Prior to the commencement of D&D activities, continuous monitoring will be required for the air discharge points that are currently not being monitored and have not been sealed or otherwise isolated. Any potential release points that remain active during D&D operations will need to be monitored continuously, sampled weekly, and analyzed as indicated in Table 6. Known potential release points in Building JN-1 include:
  - Pump Room
  - Boiler Room Exhaust (3)
  - Restroom
  - Old Stack Blowers
  - Miscellaneous Air Intakes without Backdraft Control.

Supplemental monitoring will be performed during demolition activities to verify that contamination control measures minimize the potential for releases to the general public and the environment. Monitoring will include upwind and downwind continuous air sampling and real time dust monitoring. Demolition crews will utilize dust suppression methods to reduce dust emissions. Dust suppression methods may be applied using fire hoses spraying building structures and debris during demolition and container loading. Temporary berms will be constructed or placed to collect runoff water resulting from spraying the demolition debris.



<u>Contingent:</u> In the even that air monitors or on-site environmental surveillance equipment detects levels of airborne contamination in excess of the action levels in EM-AP-2.0, procedures will be implemented immediately to correct the situation and intensify the monitoring of the building's relevant point source(s) to the extent necessary to accurately assess the amount of contamination released. Intensified monitoring will continue until measurements show that airborne effluents are within action levels.

## 4.2.2 Building JN-2

The following monitoring plan reflects the current review and status of air discharge points at Building JN-2. It is based on a comprehensive survey of the building areas conducted in 1992 and subsequent characterization and decontamination surveys.

<u>Routine</u>: Routine airborne emission monitoring during D&D and RAL operations at Building JN-2 will include:

 Continuous monitoring of the air discharge point (stack) that is currently being monitored, with weekly sample collection and analyses as listed in Table 6:

(S-11)012 – Radioanalytical Laboratory (RAL)

The location of this discharge point is shown in Figure 6. Inactive point sources will be sealed, locked out, disabled, or otherwise isolated to ensure that inadvertent radiological releases via unmonitored pathways during D&D are prevented.

- 2) Prior to the commencement of D&D activities, localized air monitoring will be required for potential release points that remain active during D&D operations. Monitoring will be continuous during D&D activities within the area, sampled weekly or at the end of work activities, and analyzed as indicated in Table 6. Known potential release points in Building JN-2 include:
  - Hood in Room 2106
  - Hood in Room 2108
  - Drying Oven Fume Hood
  - Boiler Room Exhaust (2)
  - Restroom
  - Miscellaneous Air Intakes without Backdraft Control Vaul
  - High Bay

Supplemental monitoring will be performed during demolition activities to verify that contamination control measures minimize the potential for releases to the general public and



the environment. Monitoring will include upwind and downwind continuous air sampling and real time dust monitoring. Demolition crews will utilize dust suppression methods to reduce dust missions. Dust suppression may be applied using fire hoses spraying building structures and debris during demolition and container loading. Temporary berms will be constructed or placed to collect runoff water resulting from spraying the demolition debris.

<u>Contingent:</u> In the event that air monitors or on-site environmental surveillance equipment detects levels of airborne contamination in excess of the action levels in EM-AP-2.0, procedures will be implemented immediately to correct the situation and intensify the monitoring of the building's relevant point source(s) to the extent necessary to accurately assess the amount of contamination released. Intensified monitoring will continue until measurements show that airborne effluents are within action levels.

## 4.2.3 Building JN-3

There are presently no operational air emission monitors at Building JN-3.

<u>Routine:</u> Continuous airborne effluent monitoring of the point sources (vents, stacks, blowers, etc) on the exterior of Building JN-3 will not be performed. The technical basis for this determination rests on results of calculations, shown in Section 4.7.1, that show the small radionuclide inventory in the buildings, coupled with standard D&D radiation protection procedures leading to substantially less than 0.1 mrem/year EDE (the DOE criterion for continuous monitoring).

Supplemental monitoring will be performed during demolition activities to verify that contamination control measures minimize the potential for releases to the general public and the environment. Monitoring will include upwind and downwind continuous air sampling and real time dust monitoring. Demolition crews will utilize dust suppression methods to reduce dust emissions. Dust suppression methods may be applied using fire hoses spraying building structures and debris during demolition and container loading. Temporary berms will be constructed or placed to collect runoff water resulting from spraying the demolition debris.

<u>Contingent:</u> In the event that either recalculation of the EDE for non-standard D&D procedures exceeds 0.1 mrem/year, or in-building air monitors or on-site environmental surveillance equipment detects above-standard levels of airborne contamination, procedures will be implemented immediately to intensify monitoring of the building's relevant point source(s). Intensified monitoring will continue until measurements show that airborne effluents are below applicable regulatory standards.



## 4.3 Liquid Effluent Monitoring

BMI will conduct all liquid effluent monitoring as required under the NPDES permit conditions.

Presently, a waste water treatment system, operated under an NPDES permit in accordance with State of Ohio regulations under 41N00004\*GD, handles all wastewater generated on the WJN site. Sampling of all waste water liquid effluents from the North Research Area to Big Darby Creek is performed using a continuous water sampling system located after the discharge from the UV disinfection tank. Table 5 lists the various parameters measured on daily, weekly, bi-weekly, or monthly schedules. The station, shown as EW-1 in Figure 6, will continue to be monitored during D&D operations. However, based on long-term measurements of liquid effluents from the West Jefferson site and assessments of potential liquid releases from D&D activities, no additional liquid effluent monitoring will be conducted.

Should action levels be detected in liquid samples, an immediate investigation regarding the reasons for the source terms causing the release shall be performed by BMI with the cooperation of Closure Services; and suspect operations will be suspended until corrective actions have been performed.

## 4.4 Environmental Surveillance

Table 7 lists the environmental surveillance tasks for the Columbus Closure Project. Surveillance tasks are designed to meet and/or exceed the licensing conditions for environmental surveillance.

Table 7 lists the sample collection frequency and the associated type and frequency of analyses for each sample. The table also provides reference to specific procedure documents.

Figures 3, 6, and 7, detail monitoring and sampling locations to be implemented under the EMP. Figure 3 details the current and planned sampling locations for on-site air, water, and sediment sampling, except TLDs. Figure 6 shows the locations of the 10 TLDs distributed on and around the West Jefferson site for the project area. Figure 7 shows grass, food crop, and soil sampling locations, and three off site TLD locations.

Closure Services has selected the TLD locations for the following reasons:

Locations along the project boundary line are evenly spaced and represent the most probable receptor from the general public (i.e., BMI personnel working in JN-4). While personnel in JN-4 may be considered occupational in some regard, these individuals do not conduct activities under the NRC SNM-7 License and thus meet the standard as a member of the general public.



- Significant reduction in the source term warrants downgrading environmental TLD locations to those at the site boundary.
- Closure Services has reviewed the environmental TLD records for 2002 and 2003. The annual external dose as monitored at the current 54 locations identified in the BMI EMP all fall below background levels for external sources of radiation. Additionally, the number of dosimeters are all below background. Based upon a review of the historical environmental TLDs there is a clear indication that locations at the recreational area and at the site boundary are less than background. Thus, reducing the number of dosimeters to those at the project fence line will not adversely impact the current practice of documenting negative dose to the general public.

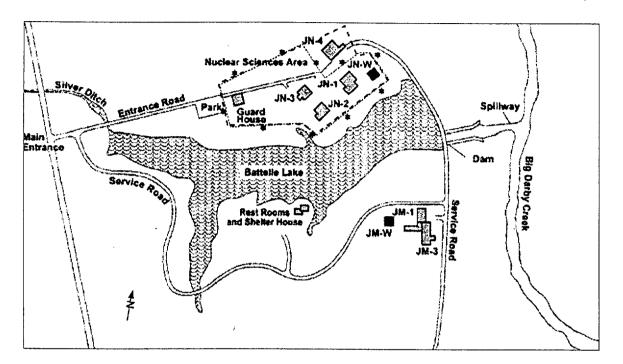


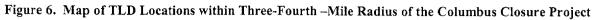
Sampling Type	Sampling Site(s)	Collection Frequency	Analysis Type and Frequency
Airborne Particulates	Locations as described in EM-OP-002	Continuous Sampling Weekly Collection	Total beta and alpha on weekly. <sup>a</sup> Gamma spectrometric analysis on monthly composition. <sup>b</sup> Isotope U, Pu, and Sr-90 on quarterly composite. <sup>c</sup>
Airborne Particulates	Sites as described in EM-SP- 001	Continuous Sampling	Total beta and alpha on weekly. <sup>a</sup> Gamma spectrometric analysis on quarterly composite. <sup>b</sup> Isotopic U, Pu, and Sr-90 on quarterly composite. <sup>c</sup>
Liquid Samples	Sits are described in EM-SP- 002	Weekly Sample Collection	Total beta and alpha on weekly.a Gamma spectrometric analysis on monthly composite.b U, Pu, and Sr-90 on quarterly composite.c C-14 and H-3 when appropriate.
Drinking Water	Onsite in building JN-2 or JN-3	Weekly Sample Collection	Total beta and alpha on monthly composite. <sup>a</sup> Gamma spectrometry on quarterly composite. <sup>b</sup> U, Pu, Sr-90 Ra- 226, Ra-228, I-129 on annual composite. <sup>c</sup> C-14, H-3 when appropriate. (Data used to provide site background values.)
Ground Water	See ground water sampling (discussed in Section 3.2)	Semi-Annual Sample Collection	Total beta and alpha, <sup>a</sup> gamma spectrometry <sup>b</sup> and U, Pu, and Sr-90 and Semi-Annual Sample. <sup>c</sup> C-14, H-3 when appropriate. Total metals: Ag, As, BA, Cd, Cr, Hg, Pb, Se, volatile organic compounds, semi volatile compounds, pesticides and PCBs, oil and grease, and pH for selected wells on an annual basis.
Ground Water	See ground water sampling (discussed in Section 3.2)	Annual	Total beta and alpha, gamma spectrometric U, Pu, and Sr-90. PCBs.
Soil	As described in EM-SP-003	Annual Sample Collection	Gamma spectrometry. <sup>b</sup> U, Pu, and Sr-90 on annual sample. <sup>c</sup>
Vegetation	As described in EM-SP-004	Annual Sample Collection	Gamma spectrometry. <sup>b</sup> U, Pu, and Sr- 90, on annual sample. <sup>c</sup>
Sediment	As described in EM-SP-011	Semi-annual Sample Collection	Total beta and total alpha, <sup>a</sup> gamma spectrometry, <sup>b</sup> U, Pu, and Sr-90 on semi-annual sample
Fish or Mollusks	See EM-SP-007	Annual Sample Collection	Total beta and total alpha, <sup>a</sup> gamma spectrometry, <sup>b</sup> U, Pu, and Sr-90 on annual sample. <sup>c</sup>
Field Corn and/or Soybeans	As Described EM-SP-005	Annual Sample Collection	Gamma spectrometry. <sup>b</sup> U, Pu, and Sr-90 on annual sample <sup>c</sup> .
Garden Crops	See EM-SP-006	Annual Sample at End of Growing Season	Gamma spectrometry. <sup>b</sup> U, Pu, and Sr-90 on annual sample. <sup>c</sup>
Beta-Gamma External (TLD)	See EM-SP-008	Quarterly Collection	Read quarterly

# Table 7. Environmental Surveillance program for Columbus Closure Project

<sup>a</sup> See RL-TP-005 for alpha and beta analyses. For air samples, if half life is greater than 30 minutes for beta and/or greater than 2 hours for alpha, send sample for gamma spectrometric analysis immediately.
 <sup>b</sup> RL-TP-030 describes gamma spectrometric analysis.

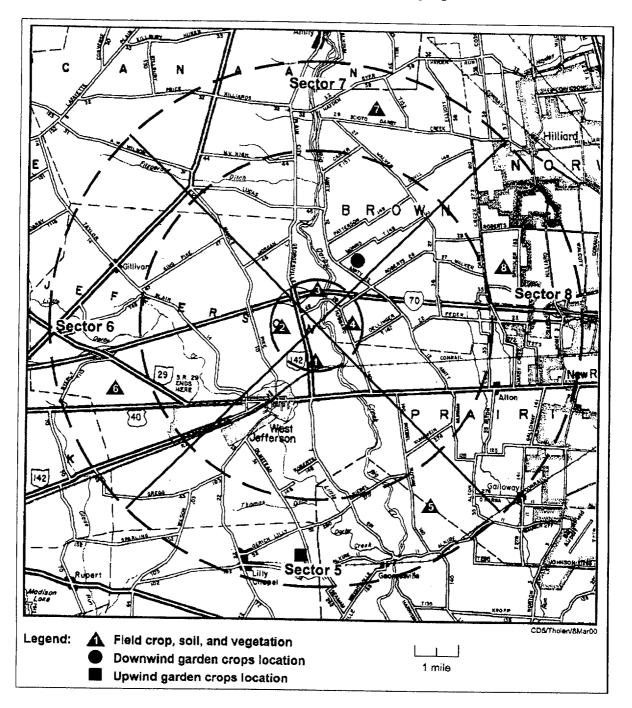
 <sup>c</sup> Specific Isotopic Analyses: Sr-90: RL-TP-035; Ra-226: RL-TP-025; Ra-228: RL-TP-056; I-129: Analyzed off-site; Plutonium, Isotopic Uranium, Am-241, and Thorium: RL-TP-054; C-14 Analyzed off-site; H-3: RL-TP-026





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# 4.5 Integration of Columbus Closure Project Emergency Management Plan into the Environmental Monitoring Program

The EMP requires flexibility to respond in the evening of a radiological accident involving the escape of radioactive materials. Conversely, the CCP emergency plan can provide backup to the EMP, particularly in terms of emergency effluent monitoring. The NRC specifically describes environmental sampling and analysis in a radiological emergency in "Emergency Environmental Sampling and Analysis for Radioactive Material Facilities."<sup>2</sup>

# 5.0 Sampling and Analyses Methodology and Determination of Off-Site Impact and Consequence Assessments

# 5.1 Sampling and Monitoring

The following procedures will describe sampling and monitoring activities.

<u>Document No.</u>
EM-OP-001
EM-OP-002
EM-SP-001
EM-SP-002
EM-SP-003
EM-SP-004
EM-SP-005
EM-SP-006
EM-SP-007
EM-SP-008
EM-SP-009
EM-SP-011
SC-SP-006



# 5.2 Analyses

The following procedures will describe the analyses of samples.

Title	Document No.
Preparation of Environmental Water and Air Samples and Routine Smears for Gross Alpha and Beta Counting	e RL-TP-005
Gross Alpha and Gross Beta Counting of Soil/Sediment/Sludge Samples Using the Tennelec LB5100	RL-TP-007
Gross Alpha and Beta Analysis Using the Tennelec LB5100 Low Background System	RL-TP-020
Analysis of Radium-226 in Environmental Water and Soil Samples	RL-TP-025
Analysis of Tritiated Water and Screening for Low Beta Energy Emitters by Liquid Scintillation Counting	RL-TP-026
Gamma Spectrometric Analysis of Laboratory Samples Using Canberra Procount <sup>™</sup> Software	RL-TP-030
Strontium-90 Analysis by Extraction Chromatogrphay	RL-TP-035
Determination of Actinides in all Sample Matrices	RL-TP-054
Analysis of Radium-228 in Water Using U.s. EPA Method 9320/SW-846	RL-TP-056
Gross Alpha and Gross Beta Analysis of Water Using U.S. EPA Method 9310/SW-846	RL-TP-057

# 5.3 Chain-of-Custody Procedures for handling Environmental Samples

To ensure proper handling, transfer, and accountability for all samples submitted for analysis under the EMP, the chain-of-custody procedures listed in RL-AP-1.0, Administrative Operating Procedure for the Radioanalytical Laboratory, will be followed.

# 5.4 Data Analysis and Statistical Treatment of Data

Effluent monitoring and environmental measurements obtained from sampling and analysis shall be analyzed to compare them to the appropriate environmental standards (Section 2), discern spatial and temporal trends, and eliminate outliers from further statistical analysis. All environmental data obtained through monitoring shall be noted. Data values will be reported as minimum detectable activity (MDA), when activity is at or below MDA.

Comparisons of effluent monitoring and environmental surveillance data shall be made each month to indicate trends in radioactive levels. This includes analysis of all information that is capable of indicating such trends. This requirement does not negate the need of daily vigilance and inspection to determine the efficacy of effluent controls.



Determination of the less-than detectable values is the subject of numerous statistical methodologies. Given that natural background radiation is ubiquitous, and that sources other than the site may contribute to the resultant radioactivity, the criteria employed in this EMP is that detectable levels attributed to D&D activities shall be that amount equivalent to background levels of radioactivity in the environment from other sources. Sampling and analysis techniques conform to these criteria.

Most environmental data follow a normal distribution. Hence, the central tendency of the data shall be expressed as the median value and the variance as the geometric standard deviation. The range of values shall be characterized as falling between the 5<sup>th</sup> and 95<sup>th</sup> percentile.

A test of normality shall be performed on groups of ten or more data points. Plotting data on normal or log-normal probability paper is the simplest method of determining normality.

If the data is normal rather than log-normal, the mean shall be the measure of the central tendency and the standard deviation a measure of variance.

If data is sparse, different assumptions about the distribution of the data may be made and an appropriate statistical analysis employed to determine the range and uncertainty of the data.

To determine spatial and/or temporal trends, comparison of data points or groups of data is required. This type of comparison is also required in comparing monitoring results to environmental standards. Plotting the data on graph paper is often sufficient to elucidate trends over time or differences between sampling locations. Use of parametric and nonparametric statistical techniques shall be employed for groups of data to determine if significant differences exist between them.

Outliers for the purposes of this EMP shall be values more than three standard deviations from the mean (or three geometric standard deviations from the median).

The Annual SER shall summarize the results if statistical analyses become necessary for values greater than MDA.

### 5.5 Determination of Off-Site Impact and Consequence Assessments

10- CFR 20.1101, *Radiation Protection Programs*, <sup>13</sup> puts forth a constraint on airborne emissions of radioactive material to the environment, excluding Ra-222 and its daughters, such that an individual member of the public likely to receive a total effective dose equivalent (TEDE) in excess of 10 mRem per year from these emissions.

A constraint is a dose value above which licensees are required to report to the NRC and to take corrective actions to lower the dose below the constraint value. Enforcement action would only occur if a licensee fails to report the constraint is exceeded or fails to take appropriate and timely corrective actions.



Reg Guide 4.20, Constraint on Releases of Airborne Radioactive Materials to the Environment for Licensees other than Power Reactors, Section c.2.4<sup>14</sup> states that the computer code COMPLY is acceptable to the NRC staff for determining the dose to members of the public from exposure to airborne radioactive materials that have been released to the environment by NRC licensees other than power reactors.

Closure Services demonstrates compliance with 10 CFR  $20.1101(d)^{13}$  by using the COMPLY computer code, version 1.5d.

### Meteorological Monitoring Program

At the present time, Port Columbus International Airport meteorological data are used for the CCP.

As shown in Table 8, the predominant wind direction at the project is southwest, and the next two most frequent directions are west-southwest. Figure 10 shows the wind rose pattern for the project.

Direction	Percent	Average Speed (m/s)				
N	4.5	4.7				
NNE	4.1	4.2				
NE	4.8	4.0				
ENE	5.0	4.1				
E	5.8	4.4				
ESE	4.7	3.8				
SE	5.0	4.3				
SSE	4.3	3.8				
S	5.5	4.5				
SSW	8.1	4.9				
SW	11.5	5.5				
WSW	8.3	5.3				
w	7.8	5.1				
WNW	6.5	4.9				
NW	6.1	4.6				
NNW	4.2	4.2				
CALM	3.8					
TOTAL	100.0	4.5				

Table 8. Average Percent Frequency of Wind Direction	(Wind From) and Average Wind Speed (1990)
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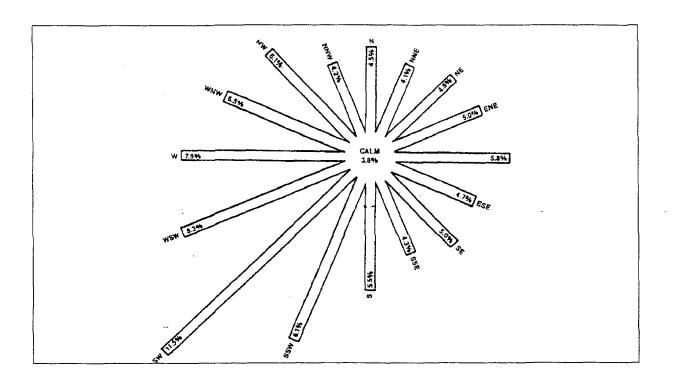


Figure 8. Wind Rose Pattern (Wind from) for West Jefferson Site

# 6.0 The Environmental Monitoring Plan's Quality Assurance Program

### 6.1 Overview

The current SER notes that a DOE quality assessment program is being administered by the DOE's Environmental Measurements Laboratory (EML).<sup>15</sup> The Closure Services in-house Radioanalytical Laboratory is a participant in the EML's quality assessment program. Therefore, it is appropriate that the quality assessment program (QAP) employed by the EML be a pertinent part of this Environmental Monitoring Plan.

The EMP shall be reviewed annually. Specific sampling and analytical procedures shall be reviewed and revised if necessary every two years.

The QAP, under which the RAL and EM operate, includes laboratory certification, a DOE QAP for radioactive materials, and independent data verification. This quality program does not include definitive procedures for quality assurance for non-radiological monitoring.

# 6.1.1 Laboratory Certification

Only certified laboratories will be contracted for analysis work. Sample analysis will be performed by applicable standard methods and covered under the Closure Services QAP. Before hiring a contractor to do environmental sample analysis, the Technical

Manager and the Quality Manager will ascertain that the contractor is properly accredited by such bodies as the EPA or State of Ohio.

Currently the EM Group uses both contractor laboratories and in-house facilities for both organic and inorganic chemistry and radiological work. These in-house labs are approved for analysis through adherence to accepted procedures. In-house radiological analysis is performed in the RAL. The RAL participates in the DOE EML quality assessment program and adheres to the Closure Services QAP. Outside laboratories must be approved for EPA analysis prior to being selected for chemical analysis. The RAL has performed analyses on the EML samples in the past as proof of its qualification. A contract will be negotiated with another off-site laboratory to provide back-up capabilities for both radiochemical and non-radiochemical parameters during the period of D&D activities.

# 6.1.2 DOE Laboratory quality assessment program for Radioactive Materials

Closure Services and the vendor(s) responsible for the analyses of CCP samples in support of the environmental radiological programs will participate in the DOE interlaboratory quality assessment program (coordinated by the DOE Environmental Measurements Laboratory in New York).

# 6.1.3 Independent Data Verification

Closure Services is committed to the maintenance of an effective quality assurance program. The national consensus standard of the ASME is adopted as the preferred standard for quality assurance in the nuclear industry

The Closure Services QAP discusses the written documentation of quality assurance and quality control procedures. This documentation is described in this section.

### 6.2 Definition

Three definitions related to quality assurance practices are given below.

Definitions of quality assurance, quality control and assessment/appraisal are given by DOE Order DOE 414.1A.

**Quality assurance** involves all those planned and systematic actions necessary to provide adequate confidence that a facility, structure, system, or component will perform satisfactorily and safely in service.

**Quality control**, which is included within quality assurance, comprises all those actions necessary to control and verify the features and characteristics of a material, process, product, or service to specified requirements.



Assessment/appraisal is a planned and documented activity performed in accordance with procedures to determine, by examination and evaluation of objective evidence, the adequacy of, and extent to which, applicable elements of the quality assurance program have been developed, documented, and effectively implemented in accordance with specified requirements.

# 6.3 Field Measurements and Sampling

The sampling procedures for field measures and sampling will incorporate quality control standards and techniques. The activities included are field sampling; preparation and storage of samples; coding and record keeping; handling, storage, and shipping; and sample archiving.

# 6.4 Radiochemical Analyses

Quality control standards and techniques for radiochemical analyses are found in RL-QAP-01.0, Radioanalytical Laboratory Quality Assurance Program Plan, and the Radioanalytical Laboratory procedures.

# 6.5 Instrumental Analyses

Instruments used by the RAL are maintained, calibrated, and stabilized by the RAL using their calibration and test procedures.

# 6.6 Data Reduction, Storage, and Reporting

The reduction, storage, and recording of analytical data from the RAL is performed through RL-AP-01.0, Administrative Operating Procedures for the Radioanalytical Laboratory and the RAL testing procedures.

### 6.7 Quality Assurance Records

Records that furnish documentary evidence of quality shall be specified, prepared, and maintained. Specified records include, but are not limited to, the following:

- Maps identifying sampling locations
- Sampler record book
- Sample inventory
- Technical procedures and data sheets
- Calculation and analyses records
- Reports
- QA surveillance and audit records



- Program correspondence

Records shall be made part of the Closure Services project record system.

# 7.0 Implementation of the Environmental Monitoring Plan

### 7.1 Environmental Monitoring Plan Implementation

The Closure Services EMP is primarily for the monitoring and surveillance of the D&D activities to be completed at the WJN facility. The plan will be reviewed annually and updated every two years until the project is completed. The annual review and biennial updates to the plan will be prepared by the Closure Services Environmental Monitoring Group and reviewed by the Quality Manager and the Radiological Technical Support Manager prior to submittal to Ohio Field Office for review.

The environmental monitoring activities have two major phases. The first phase consists of characterizing sources of pollution, including radiological and non-radiological measurements and sampling near the sources. The second phase of the plan includes analysis of pathways to the site boundary and off-site environmental sampling to substantiate the effectiveness of the control of releases. This plan includes measurements and samples taken near the surface (including air monitoring), surface water, and ground water monitoring consisting of measurements taken from a network of wells.

Meteorological and hydrological data are acquired as necessary to support this plan.

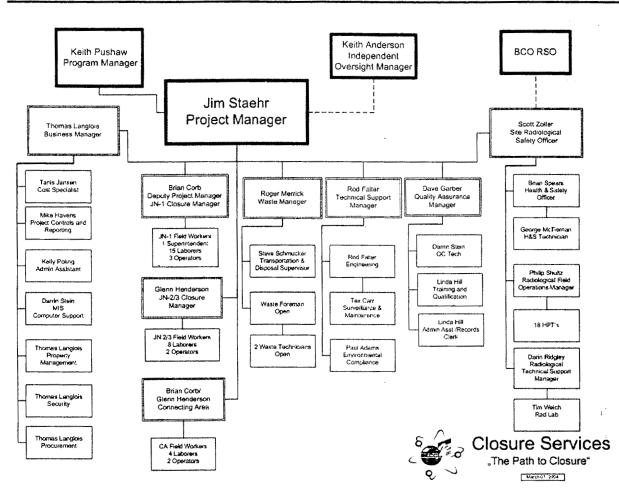
### 7.2 Environmental Monitoring Activities

Monitoring data are collected and analyzed to determine compliance with applicable regulations, are maintained by the Technical Support Manager and submitted appropriate to the Ohio Field Office. The Environmental Monitoring Group is responsible for implementing the EMP, doing the field work, and directing the laboratory analysis. Environmental monitoring and environmental occurrence reporting requirements (including reports for radioactive effluent, on-site discharge, and unplanned releases) are coordinated through the Technical Support Manager. This group reports the monitoring data in accordance with federal, state and local regulatory requirements. The actual reporting is done through the Technical Support Manager, with copies going to the Ohio Field Office.

The organizational structure for various tasks described in the EMP is shown in Figure 11. Each manager reports directly to the Closure Services Project Manager and is responsible for the day-to-day monitoring activities, equipment calibration, and review and evaluation of data generated.

#### Figure 9. Closure Services Organizational Chart





### 7.3 Ground Water Protection Monitoring

The Technical Manager coordinates the monitoring for radionuclides in wells around the CCP. Many of the wells used in the ground water monitoring program were designed in accordance with 40 CFR Part 264, Subpart F, or 40 CFR Part 265, Subpart F.

Ground water monitoring is done at a supply well for the West Jefferson site and from two other existing supply wells before the water is treated in any form. Eighteen (18) wells have been installed for sampling around the West Jefferson site and are randomly being selected for monitoring radionuclides; three of these wells are also monitored for chemical contamination.

### 8.0 References

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# DRAFT REPORT

on

# GEOLOGY AND HYDROGEOLOGY OF WEST JEFFERSON NORTH SITE

to

# U.S. DEPARTMENT OF ENERGY

by

Thomas C. Beard and Neeraj Gupta

September 14, 1990

BATTELLE 505 King Avenue Columbus, Ohio 43201

# TABLE OF CONTENTS

		Page
1.0	INTR	ODUCTION
	1.1 1.2 1.3 1.4	PURPOSE1SCOPE OF WORK4PREVIOUS STUDIES AND SOURCES OF DATA4DESCRIPTION OF AREA5
		1.4.1Geographic and Geologic Setting51.4.2Land Use51.4.3Climate5
2.0	GEOL	OGY
	2.1 2.2	INTRODUCTION
		2.2.1       Soil       9         2.2.2       Alluvium       12         2.2.3       Fill       13         2.2.4       Till       14         2.2.5       Bedrock       15
	2.3 2.4	DRILLING/CORING OPERATIONS FOR SITE CHARACTERIZATION 16 SPECIFIC SAMPLING AREA GEOLOGY
		2.4.1       Area 1 (Storm Sewer Outfall)       25         2.4.2       Area 2 (Between JN-2 and JN-3)       25         2.4.3       Area 3 (Rear of Building JN-4)       27         2.4.4       Area 4 (Rear of JN-1)       27         2.4.5       Area 5 (Loading Area of JN-3)       27         2.4.6       Area 6 (Loading Area of JN-1)       28
	2.5	GEOLOGY OF ABANDONED FILTER BED AREA
3.0	HYDR	OLOGY
	3.1	SURFACE HYDROLOGY
		3.1.1Surface Water and Drainage293.1.2Precipitation, Evaporation, Infiltration30
	3.2	GROUNDWATER HYDROLOGY
		3.2.1 General Description

ت ا

# TABLE OF CONTENTS (Continued)

		3.2.1.1 3.2.1.2 3.2.1.3 3.2.1.4 3.2.1.5	Alluv Fill Till	ium	•	•••	•	• • • •	•	•	•	•	•	•	•	•	•	•	•	31 31 32
	3.2.2	Specific	Sampli	ing Area	is H	ydro	oge	010	gу	•	•	•	•	•	•	٠	•	•	•	33
		3.2.2.1	Water	Levels	•	••	•	••	•	•	•	•	•	•	•	•	•	•	•	33
	3.2.3	Hydrauli	c Condu	uctivity	Te	stiı	ng	•••	•	•	•	•	•	•	•	•	•	•	•	39
		3.2.3.1 3.2.3.2	Data / Result	Analysis ts	•	•••	•	•••	•	•	•	•	•	•	•	•	•	•	•	41 41
4.0	CONCLUSIONS	• • • •	•••		٠	•••	•	••	•	•	•	•	•	•	•	•	•	•	•	45
5.0	REFERENCES				•	••	•		•	•	•	•	•	•	•	•		•	•	46

#### LIST OF APPENDICES

- APPENDIX 1: BORING LOGS FOR ALL SAMPLING AREAS
- APPENDIX 2: MONITORING WELLS CONSTRUCTION DIAGRAMS
- APPENDIX 3: SLUG TEST DATA
- APPENDIX 4: PRECIPITATION RECORD

### LIST OF TABLES

TABLE 1.	SUMMARY OF BOREHOLES DRILLED FOR         SITE CHARACTERIZATION         SITE CHARACTERIZATION	7
TABLE 2.	MONITORING WELL DATA	4
TABLE 3.	WATER LEVEL DATA	5
TABLE 4.	HYDRAULIC CONDUCTIVITY RESULTS	2

Page

# TABLE OF CONTENTS (Continued)

# LIST OF FIGURES

FIGURE	1.	MAP OF WEST JEFFERSON	
		SITE AND VICINITY	
FIGURE	2.	SAMPLING AREA MAP	
FIGURE	3.	SITE TOPOGRAPHIC MAP	
FIGURE	4.	AERIAL PHOTO OF WEST JEFFERSON SITE AND VICINITY	
FIGURE	5.	PREEXISTING BOREHOLE LOCATIONS	
FIGURE	6.	SOIL DISTRIBUTION AROUND JN FACILITIES	
FIGURE	7.	BOREHOLE LOCATIONS IN AREA 1	
FIGURE	8.	BOREHOLE LOCATIONS IN AREAS 2 AND 5	
FIGURE	9.	BOREHOLE LOCATIONS IN AREA 3 FOR STAGES 1 AND 2	
FIGURE	10.	BOREHOLE LOCATIONS IN AREA 4	
FIGURE	<b>1</b> 1.	BOREHOLE LOCATIONS IN AREA 6	
FIGURE	12.	BOREHOLE LOCATIONS IN FILTER BEDS AREA	
FIGURE	13.	CHEMICAL SAMPLING BOREHOLE LOCATIONS	
FIGURE	14.	FENCE DIAGRAM FOR JN AREA	
FIGURE	15.	WELL HYDROGRAPHS FOR AREAS 1, AND 3	
FIGURE	16.	WELL HYDROGRAPHS FOR AREAS 2, 4, 5, AND 6 AND WELL C16	
FIGURE	17.	WATER TABLE SURFACE MAP FOR JN FACILITIES	
FIGURE	18.	HYDRAULIC CONDUCTIVITY DISTRIBUTION	

۷

Page

#### EXECUTIVE SUMMARY

As part of the Battelle Columbus Laboratory Decommissioning Project (BCLDP) a site characterization plan was undertaken for Battelle's Nuclear Sciences Area (known as the JN site) at West Jefferson, Ohio. The site characterization program included six specific sampling areas at the JN site and the retired and remediated filterbeds located on the Big Darby Creek flood plains. This report presents the extensive geological and hydrogeological information obtained through drilling/coring operations and through a review of existing geological data. The study focuses on field evaluation of the shallow geology and groundwater flow systems within the artificial fill and shallow glacial till sediments. An attempt is made to define the location and extent of higher permeability sand and gravel lenses within the glacial sediments.

A total of 168 boreholes were drilled as part of site characterization. 116 of these were drilled during Stage 1 sampling in the summer of 1989, 20 during Stage 2 in the summer of 1990, and 32 boreholes were drilled for chemical sampling in November of 1989. Water level monitoring wells with PVC casing were installed in 25 boreholes, and stainless steel wells were installed in 3 boreholes for groundwater sampling. After drilling, boring logs for each sample point were generated and a generalized geologic fence diagram for the JN area was constructed.

In general, the shallow geology of these areas is the same as regional geology of central Ohio. That is, it is dominated by glacial till deposits. The color of till changes from brown to gray with depth. Most areas contain some artificial fill deposits. But artificial fill is most significant in area 3 and in the retired filter beds area. Sand/gravel lenses are observed in many areas but they are of limited extent. The only exception is the fine sand, silt outwash layer in area 1 which may also be connected with sand stringer found in borehole 403. Alluvial deposits are found in the retired filter beds area.

Dominant surface hydrologic controls are the lake and Big Darby Creek. A review of the water level data collected indicates that much of the shallow ground water flow from JN site discharges into the lake. Some groundwater discharge from JN site and the groundwater discharge from the

i

filterbeds is into the Big Darby Creek. The results of slug tests show that hydraulic conductivity falls within the range of values normally associated with the geologic materials present at each site. These materials can be categorized as the near surface brown till/fill, the confined and unconfined sandy zones, and the dense gray till. The till deposits have very low hydraulic conductivity. The sand/gravel lenses have higher permeability but appear to be discontinuous and isolated for the most part.

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#### **1.0 INTRODUCTION**

As part of the Battelle Columbus Laboratory Decommissioning Project (BCLDP), a Site Characterization Plan (SCP) was prepared for Battelle's Nuclear Sciences Area (JN). This area is situated at the north end of Battelle's West Jefferson, Ohio, site, 17 miles west of Battelle's Columbus Main Operations (Figure 1). The SCP contains a site sampling and analysis program designed to identify areas of soil which may require remedial action in order to release the site for unrestricted use.

Six areas were identified within the site for detailed sampling and analysis on the basis of existing radiological information and a knowledge of previous site activities. Those site areas have been shaded in Figure 2. An additional sampling area to be investigated was a retired and remediated filter bed (Figure 2).

#### 1.1 PURPOSE

The purpose of this report is to present the extensive geological and hydrological information obtained through a review of existing geological data, in addition to extensive drilling/coring information collected as a result of this investigation. This characterization of the nature and extent

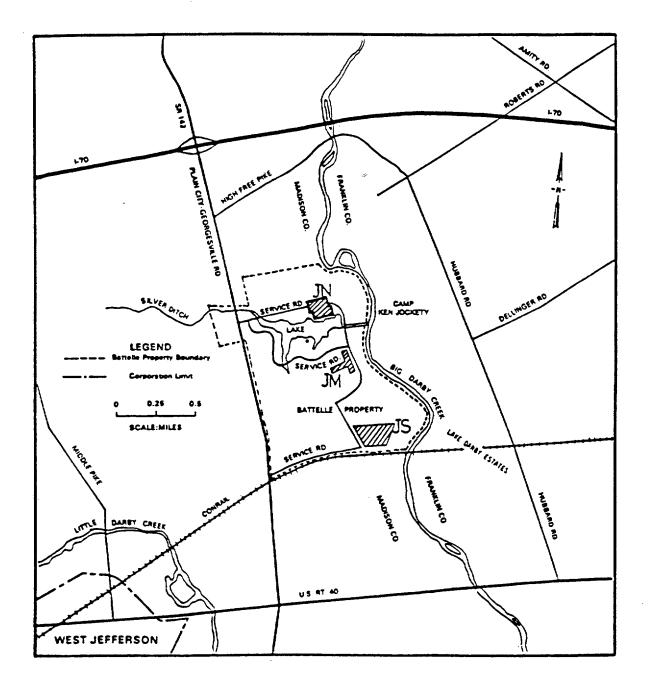


FIGURE 1. MAP OF WEST JEFFERSON SITE AND VICINITY



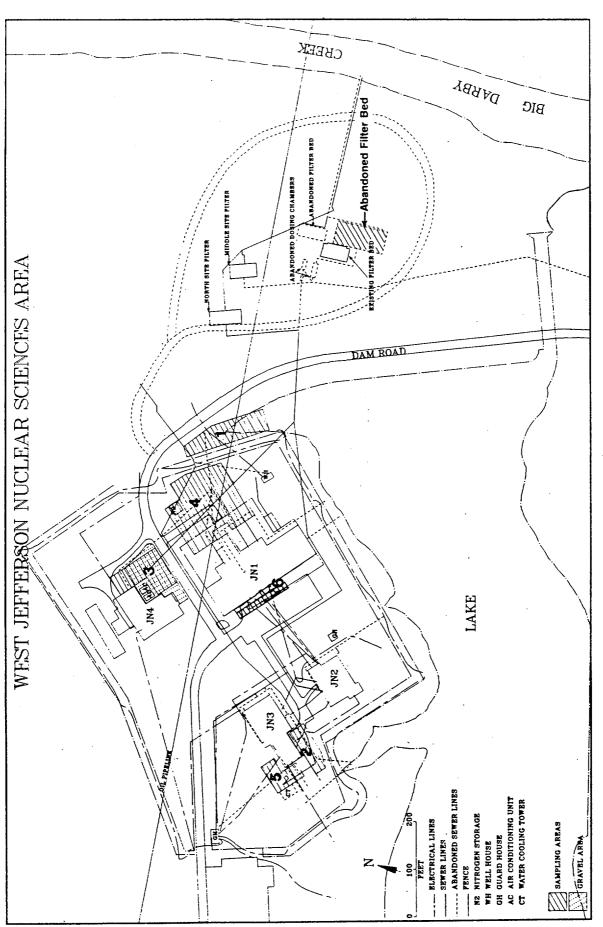


FIGURE 2. SAMPLING AREA MAP

of the subsurface environment will facilitate the decommissioning and decontamination project at West Jefferson north site.

#### 1.2 SCOPE OF WORK

The West Jefferson North Area is located on the eastern border of Madison County where it meets Franklin County in Central Ohio (Figure 1). The site is bounded by Big Darby Creek to the east and the lake to the south and southwest. The Site Characterization program is limited to six specific sampling areas at the JN site and also to the retired filterbeds located on Big Darby Creek flood plain.

The study focuses on the field evaluation of the shallow geology and groundwater flow systems within the artificial fill and shallow glacial till sediments. In addition, the sand and gravel lenses within the glacial till can be significant pathway for groundwater movement due to their relatively high permeability. Therefore, an attempt is made to closely define the location and extent of these lenses within the till.

#### 1.3 PREVIOUS STUDIES AND SOURCES OF DATA

While no previous site characterization studies have been performed at the site, a number of available reports provide general geological information. These include studies performed during the construction of buildings, the dam, and water wells. Boring logs are available from drillings by Burgess and Niple, Limited (1966) for the dam site, from foundation drillings below JN-1 (G. K. Jewell & Associates, 1971) and from water well drilling at JN (Klaer, Fred H. and Associates, 1963). Reports on geology, water resources, soil types, and the climate of Madison county are available with Ohio Department of Natural Resources (ODNR) and United States Geological Survey.

Information from the above sources was used along with information from drilling/coring operations, well installation, and data collection and was used for detailed site characterization of the JN facilities.

#### 1.4 DESCRIPTION OF AREA

### 1.4.1 Geographic and Geologic Setting

Battelle's West Jefferson facility is located about 17 miles west of Columbus and 3 miles north-northeast of the town of West Jefferson in a rural agricultural area. It is at Madison county's eastern border along Big Darby Creek. The land surface is gently sloping except for the steep V-shaped valley of Silver Creek (now dammed and occupied by the lake) and the wide flat bottomed valley of Big Darby Creek. A topographic map of the area is shown in Figure 3.

The JN facilities lie at the northern end of the Battelle West Jefferson area, just north of the lake, approximately 500 ft west of Big Darby Creek valley. Surface elevations within the fenced area of JN are generally between 900 and 910 ft above mean sea level (MSL). The flood plain of Big Darby Creek is at elevations of about 860-870 ft MSL.

### 1.4.2 Land Use

The primary man-made structures located in this area are the dam, the lake, the JN complex of buildings, and the sanitary system filter beds. The dominant land use around the JN facilities is farmland for cultivation of corn and soybeans as shown in the aerial photo of the area (Figure 4). The flood plain of Big Darby Creek is mostly covered with trees and bushes. There are some newer residential developments on the east side of Big Darby Creek valley.

#### 1.4.3 Climate

Climate of the south-central Ohio region may be described as continental-temperate. As such, the region is subject to a wide seasonal range in temperature.

Summers are quite warm with the mean temperature for the months of June, July and August being 73.3°F. Temperatures of 90°F or above are expected for about 15 days during these months. For the months of December,

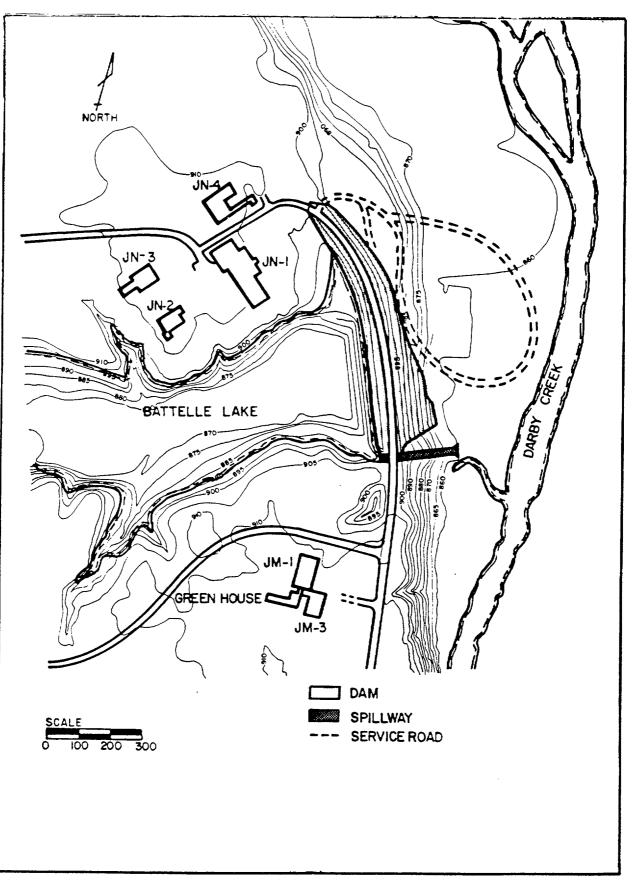


FIGURE 3. SITE TOPOGRAPHIC MAP





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January, and February, the mean temperature is 31.2°F, although the typical number of days per year which have temperatures below 32°F and below 0°F is 122 and 4, respectively.

Precipitation is distributed fairly uniformly during the year, although 60 percent falls during the spring and summer seasons. The annual monthly average rainfall is about 3.2 inches with the greatest recorded rainfall for any 24 hour period being 3.87 inches, in July of 1947.

#### 2.0 GEOLOGY

#### 2.1 INTRODUCTION

A detailed study of the geologic and hydrologic characteristics of the Nuclear Sciences Area at West Jefferson was performed as part of the Site Characterization Plan. Previously existing information in the form of boring logs was used (Figure 5) to develop correlations. In addition, extensive drilling and sampling operations were performed. This section is divided into subsections on General Site Geology and Specific Sampling Area Geology.

#### 2.2 GENERAL SITE GEOLOGY

The Nuclear Science Area at the West Jefferson site is situated on glacial till deposits. The till overlies approximately 500 feet of limestone/dolomite bedrock, and, based on bedrock contours and surface elevations at the site, the maximum possible range of till thickness is estimated to be 40 to 160 feet. Soil up to 6-feet thick has developed at the top of the till. In the floodplain of Big Darby Creek the surficial material is alluvium between 10 to 15 feet deep. Fill materials are also present as a result of construction and related activities.

### 2.2.1 <u>Soil</u>

Four types of surficial soils are mapped at the West Jefferson JN Area (ODNR 19759, USDA 1981): Crosby-Lewisburg, Lewisburg-Celina, Medway, and Miamian (Figure 6). The Medway soil has formed on alluvium and the other types are all formed on till.

Crosby-Lewisburg soils are found in nearly level and gentle slopes. They are poorly-drained to moderately well-drained, showing seasonal wetness and temporary ponding, particularly in level areas. Permeability is slow (0.06 to 0.2 in./hr). Surface soil is slit loam about 8 in. thick. Subsoil is 14 in. thick clay loam and the parent material substratum to about 60 in. depth is glacial till. The depth to till is greater than 22 in. in some areas. In the Unified Soil Classification System (which is based on

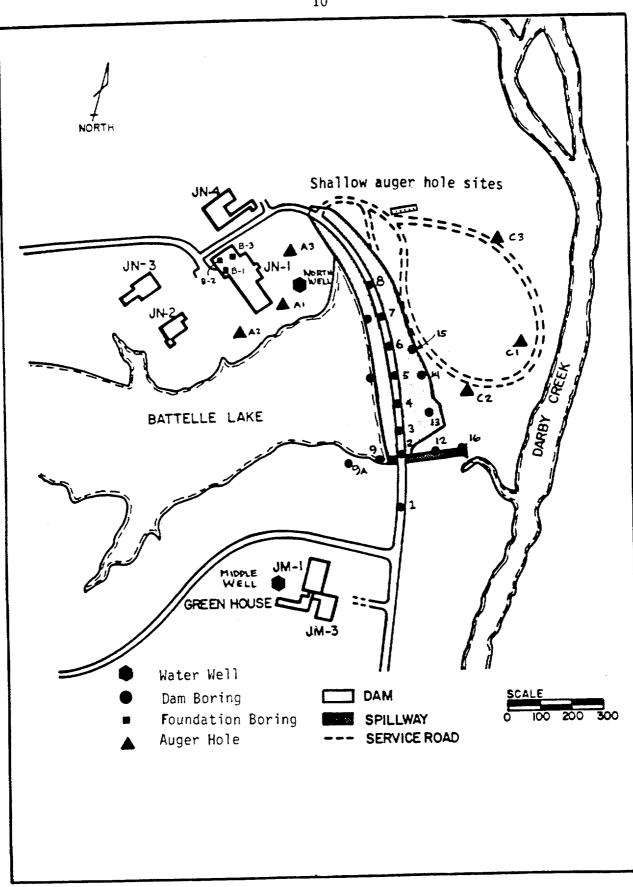
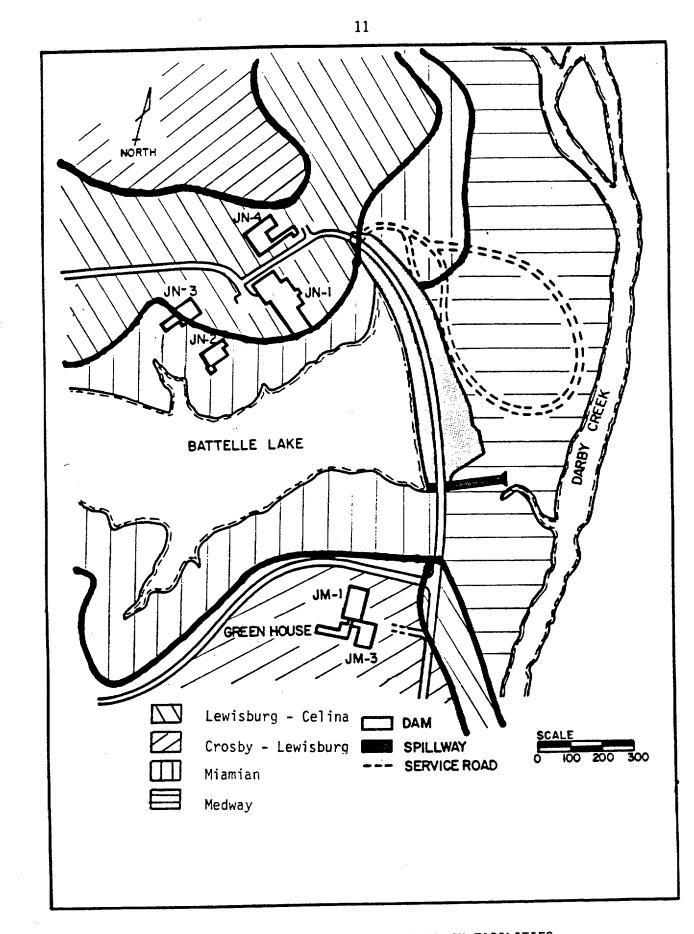


FIGURE 5. PREEXISTING BOREHOLE LOCATIONS



# FIGURE 6. SOIL DISTRIBUTION AROUND JN FACILITIES

engineering characteristics) these soils are classified as CL/ML (CL= low-tomedium plasticity clays, including silty, sandy, and gravelly clays; ML=slightly plastic silt and very fine sand).

Lewisburg-Celina soils are found on gentle slopes and are moderately well drained. Temporary surface wetness occurs. Permeability is moderately slow (0.2 to 0.6 in/hr). The surface layer, silt loam, is about 9 in. thick. Subsoil, clay loam, is 17 in. thick and the substratum is glacial till at a depth of about 40 in. or more. The engineering classification of these soils is CL/ML.

Miamian soil is formed on gently sloping to steep topography. The surface layer, 8 in., is silt loam. The 19-in. thick subsoil is clay loam, and the substratum at approximately 27 in. is glacial till. Permeability is moderately slow (0.2 TO 0.6 in./hr). It is well drained due to the slope. The engineering classification of this soil also is CL/ML.

Medway soil occupies nearly level flood plains and forms on recent alluvium derived from upland soils. Surface soil is about 15 in. thick and is silt loam. The subsoil is 19 in. thick and is mottled silt and clay loam. The substratum is gravelly loam and may be deeper than 24 in. Permeability is moderate (0.6 to 2.0 in/hr), and it is moderately well drained. Organic material content is high. Included in areas mapped as Medway are very poorly drained loam soils in depressions, high water channels, and base of slope breaks. The engineering classification of these soils is ML/CL.

### 2.2.2 <u>Alluvium</u>

Alluvium consisting primarily of stratified silt, fine to medium sand, and clay is found in the flood plain of Big Darby Creek. Very thin layers of fine sand were observed in clay/silt deposits, and the alluvium is generally more well-sorted than the till. Some small gravel is sparsely interspersed in the alluvium, probably due to its abundance in the parent till.

The alluvial deposits are black, gray, red, or brown, and occasionally are mottled. Alternating wetting and drying of these materials occurs as the stream level and water table fluctuates, causing alternating oxidizing (low water) and reducing (high water) environments. Yellow and red colorations are due to oxidation of iron. The black color and a sulfur odor indicate the presence of organic material and reducing conditions.

Although sand layers within the alluvium may be continuous to some degree, they are thin and difficult to trace or correlate over distance.

The permeability of the alluvium is not known, but is estimated to be in the range of  $10^1$  to  $10^{-7}$  cm/sec, with clean sand at the high end and silt at the low end of the range (Freeze and Cherry, 1979).

### 2.2.3 <u>Fill</u>

Fill is found overlying natural soils or parent materials in areas where the West Jefferson site has been affected by construction activities. These areas include the JN building complex, roadways (paved and unpaved), the dam and spillway, spoil areas, the filter bed area, and other disturbed areas. Fill also is found where underground utilities are placed (electrical, telephone, sewer, water, gas, etc.). The depth of fill is highly variable, as are its characteristics.

The source of the fill material are excavation and borrow areas. In most instances the materials are from the site itself, although some types of fill used for specific purposes have been brought in from nearby areas because of their absence or sparsity at the site. Consequently, the gross composition of fill is usually the same as native materials; i.e., mixture of clay, silt, sand, and gravel but can be sorted or selected to consist entirely of a single constituent, such as sand or gravel.

The placement process also introduces great variability, some fill being very carefully compacted at optimal moisture content, some being compacted under less stringent specifications, some being placed and not compacted at all. This results in a range of densities and, consequently, a range of geologic and hydrologic characteristics.

Two types of fill are recognized at the site: general fill and engineered fill. General fill consists of reworked native materials that have been placed to achieve a required grade and compacted to some degree. It consists of a clayey silt with some sand and gravel. The gravel may be derived from till or may be crushed rock brought into the site. The gravel in most of the general fill tends to be larger and more abundant than in the native till. The density may vary widely from uncompacted (as in spoil piles) to compaction as great as native till, where moisture and compactive effort were optimal. Hydrologic characteristics will vary according to the methods of placement.

Engineered fill materials consist of native or imported materials that have been selected and placed to meet engineering specifications. Examples at the site include the subgrades below the foundations of various structures, subgrades of paved roads, the dam embankment, and the sanitary sewer filter beds, drains, and the backfill used for underground pipelines. The type of material is variable depending on use. Strength, moisture content, size gradation, degree of compaction, and hydrologic characteristics all depend on the design of the fill.

### 2.2.4 <u>Till</u>

Borings for the dam investigation and construction of two water wells show bedrock at 791 to 796.5 feet above MSL and till thickness of 63.8 feet to 119 feet. This is within 40-to 160-feet range expected based on published bedrock elevations shown in ODNR (1959) and the surface elevations. None of the borings during current Site Characterization penetrated the bedrock.

The till consists of dense, predominantly non-plastic silt and clay, with minor amounts of sand and gravel in an unstratified mixture. Boulders up to 5 feet in diameter may be present. Outwash deposits of small areal extent are also found within the till as sand and gravel lenses or stringers

The till was deposited directly by ice during two substages (episodes) of the Wisconsin glaciation, the first about 50,000 years ago and the second about 22,000 years ago. A sand and gravel deposit between the upper and lower tills is found in some areas of Madison and Franklin counties (ODNR, 1958 and 1959). These outwash deposits are reported to be as thick as 20 feet and were deposited by meltwater. Other sand and gravel layers are seen in the available logs, but are of limited lateral extent and it is difficult to correlate between borings. Some evidence of weathering (soil formation) indicates an extended ice-free interval before the upper till was deposited. A color change in till from brown to gray denotes the difference

between the upper and lower tills, and this feature is used for developing correlations for Site Characterization.

The till revealed in valley walls of Silver Creek is described by Burgess and Niple (1966) as dense silt and clay with intermixed sand and gravel with "occasional erratic streaks and seams of sand." In one boring at "the Silver Creek area", whose exact location is unknown, a 6-foot thick sand layer was encountered between elevations of 882.5 and 876.5 feet. Its extent is unknown, but it is not observed in any other boreholes at the Nuclear Sciences Area. It is another example of a discontinuous outwash layer within the till.

### 2.2.5 Bedrock

The bedrock in the site area consists mainly of approximately 500 ft. thick limestone and dolomite strata of Silurian and Devonian ages (Bass Islands dolomite and Columbus Limestone, respectively). The strata lie on the east flank of the Cincinnati Arch and dip northeastward about 20 feet per mile. At the site, bedrock is at elevations of 791 to 796.5 MSL, about 113.5 to 119 feet below the surface at JN-1 (elevation about 910 feet MSL).

Prior to glaciation, the area was a relatively flat upland deeply incised by wide, steep-sided valleys. The valleys that cross this surface are the former courses of the Teays River and its tributaries. A tributary is mapped west of the Battelle site but bedrock elevations do not show it to be present at the site.

Weathering of the bedrock occurred when the rocks were exposed at the surface, resulting in open crevices. Some crevices were enlarged by solutioning to openings of 50 feet or more (ODNR, 1959). The bedrock is, therefore, very permeable, yielding up to 400 gallons per minute (ODNR, 1959). The pumping capacities of the bedrock wells at the West Jefferson site were evaluated by Klaer and Associates (1963) and are capable of 250 gpm (north well) and 500 gpm (middle well).

### 2.3 DRILLING/CORING OPERATIONS FOR SITE CHARACTERIZATION

A total of 168 boreholes were drilled as part of the site characterization plan for JN site. All drilling work was performed by John Mathes and Associates, Inc. during 1989 and 1990. The SCP calls for a two stage sampling plan. Stage 1 was the main sampling phase during which 110 boreholes were sampled in Areas 1 through 6 and in the retired filter beds in 1989 (Table 1). These borehole locations are shown in Figures 7 through 12. Stage 2 soil sampling was invoked only if the Stage 1 data do not allow a decision to be made regarding the radiological condition of any of the designated Areas. 20 boreholes were sampled in Area 3 (Figure 9) and in the retired filter beds area (Figure 12) as part of Stage 2 sampling in summer 1990 as per QA-QAP-3.0 (1990). In November 1989, 32 boreholes (Figure 13) were drilled and samples collected for chemical analysis as per quality assurance procedure SC-SP-011 (1989).

Sampling locations were identified and flagged in advance of drilling/coring operations. Procedures used for soil sample collection are described in SC-SP-004. The primary technique for extracting continuous cores was via a 5-foot long split-barrel, 3-inch OD sampler. The sampler held 2-2.5 ft long, 2 inch diameter clear plastic liners into which the core was pushed. The core usually remained intact and this system worked very well except that the core could only be viewed and not touched except on the ends. The geologic materials could be visually identified satisfactorily through the plastic. Few problems were encountered although some holes were offset either to achieve better recovery or to avoid utility liners. For sample locations which were located in asphalt or gravel covered areas, the top asphalt/gravel was removed and stored in plastic bags prior to drilling. If continuous coring did not produce greater than 50 percent recovery after three attempts then 18 inch long split-spoons were driven. The exact locations of all boreholes was later determined by a professional surveyor.

Geologic interpretation of core materials was performed as soon as core and core liners were removed from the auger. An instrument for detecting volatile organics in the boreholes was used, however, no significant organic vapors were detected except near some USTs. Continuous scanning for

	Sampling Area	Number of Boreholes Drilled	Number of Boreholes to 10 Feet	Number of Wells Installed
(1)	Stormsewer outfall	23	. 18	5
(2)	Wash-down between JN-2 and JN-3	7	6	1
(3)	Rear of JN-4 Stage 1 Stage 2	14 10	11 (30 ft. deep)	3
(4)	Rear of JN-1	21	20	1
(5)	Loading Zone of JN-3	8	7	1
(6)	Loading Zone of JN-1	9	8	1
(7)	Retired Filter Bed Stage 1 Stage 2	34 10	34 (10.0 - 14.5 ft. deep	13
(8)	Chemical Sampling Boreholes	32	(variable depth)	3
	Total	168	104	28

# TABLE 1.SUMMARY OF BOREHOLES DRILLED<br/>FOR SITE CHARACTERIZATION

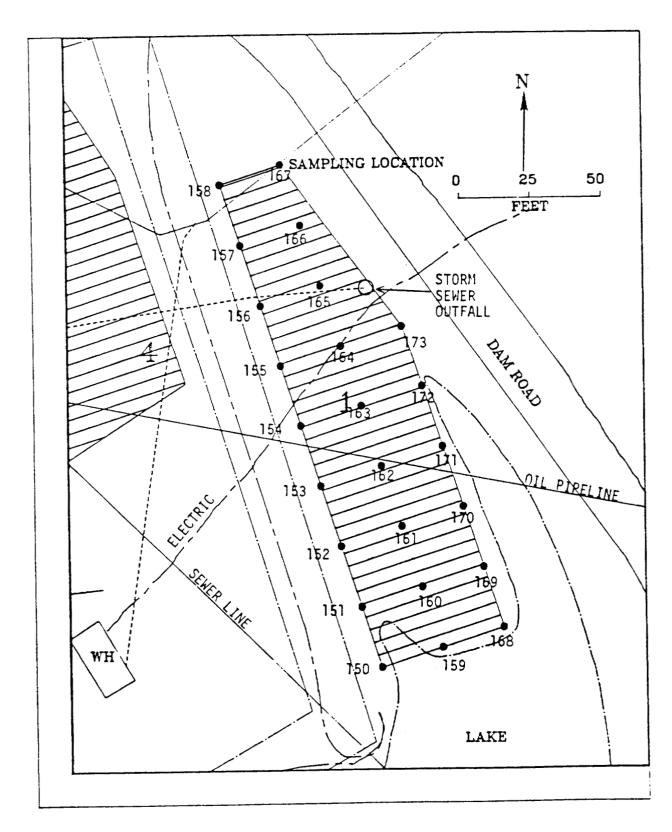
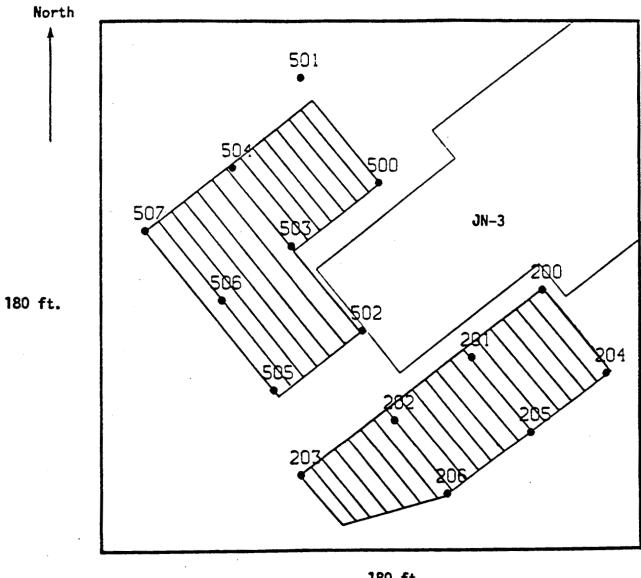


FIGURE 7. BOREHOLE LOCATIONS IN AREA 1



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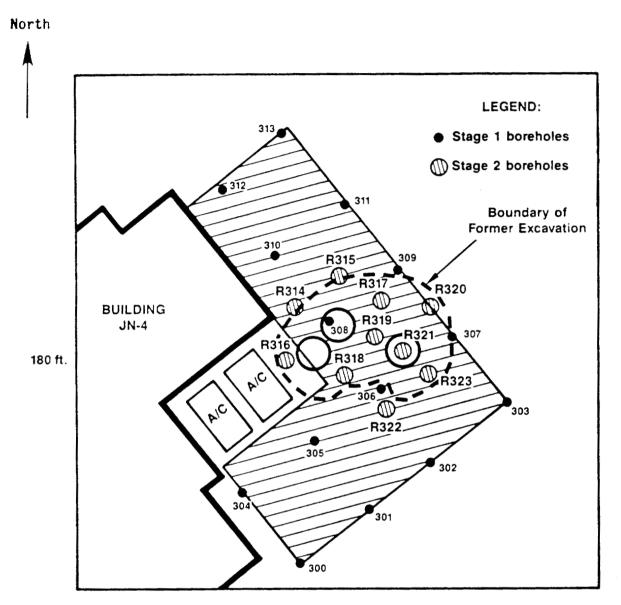
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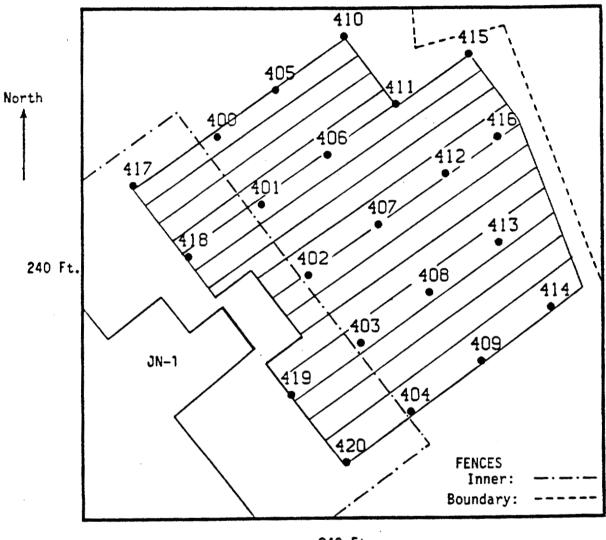
180 ft.

# FIGURE 8. BOREHOLE LOCATIONS IN AREAS 2 AND 5



180 ft.

## FIGURE 9. BOREHOLE LOCATIONS IN AREA 3 FOR STAGES 1 AND 2



240 Ft.

FIGURE 10. BOREHOLE LOCATIONS IN AREA 4

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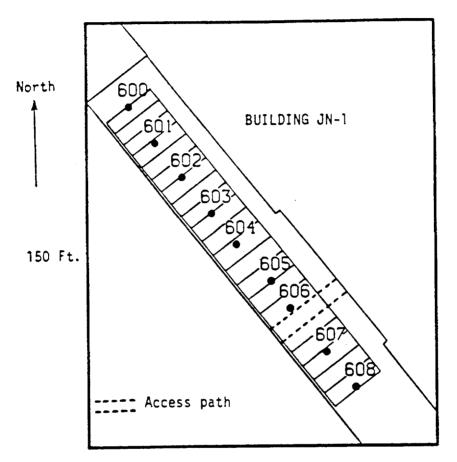
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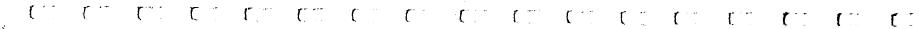
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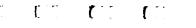
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120 Ft.

# FIGURE 11. BOREHOLE LOCATIONS IN AREA 6





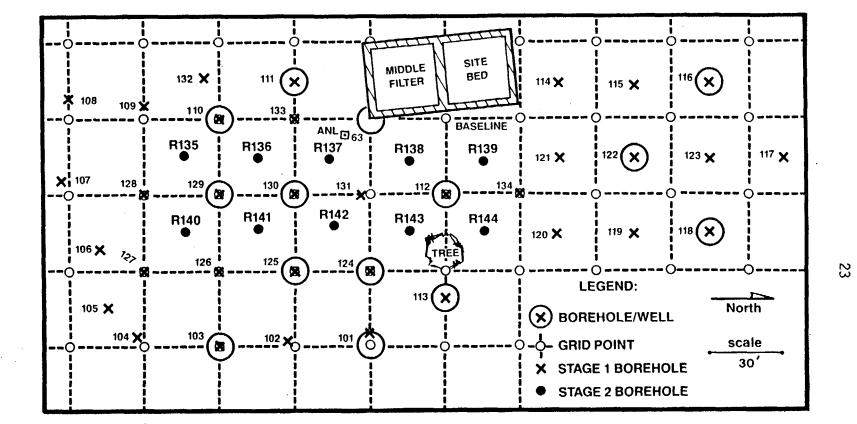


FIGURE 12. BOREHOLE LOCATIONS IN FILTER BEDS AREA

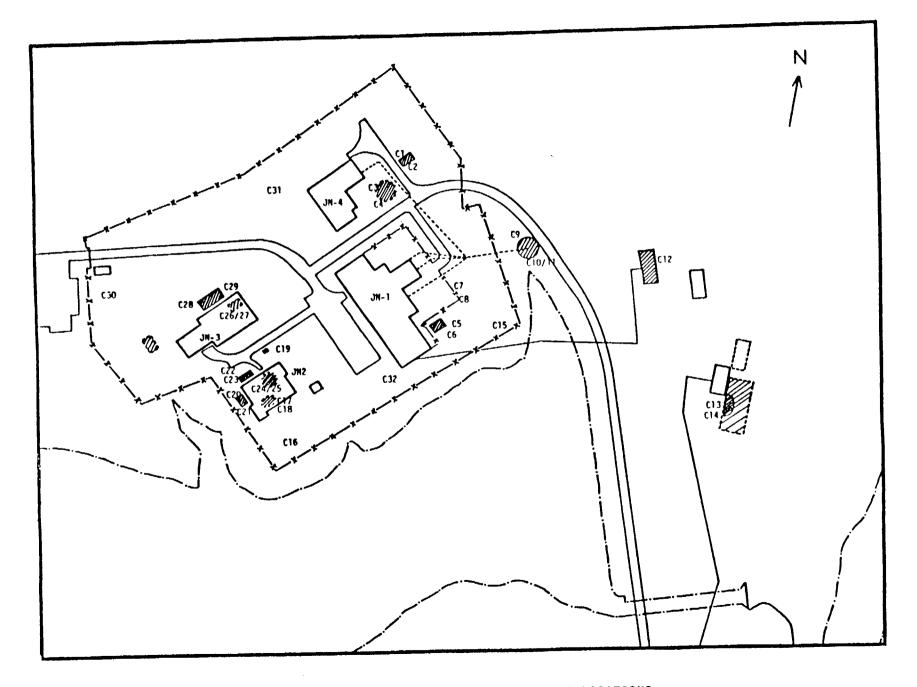


FIGURE 13. CHEMICAL SAMPLING BOREHOLE LOCATIONS

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radioactivity as well as smear samples were taken on every 5 ft section for QA and safety purposes.

#### 2.4 SPECIFIC SAMPLING AREA GEOLOGY

After drilling, boring logs for each sample point were generated and these are included in Appendix A. Representative core logs from each sample area were used to construct a generalized fence diagram for JN area facilities (Figure 14). In general, the shallow geology of these areas is the same as the regional geology for central Ohio. That is, it is dominated by glacial till deposits. The marker within the till for correlation purposes is where the color changes from brown to gray. Specific variations within the sampling areas are due to the presence of sand/gravel outwash deposits within the till and to artificial fill in excavations, and along utility lines. Significant observations from each sample area are discussed below.

#### 2.4.1 Area 1 (Storm Sewer Outfall)

Most of the boreholes in this Area show thick deposits of siltyclay/clayey silt underlain by fine grained sand and silt deposits. These sediments are moderately well sorted and contain variable amounts of cobbles, pebbles and gravel. The general thickness of the fine sand and silt deposits increases from absent in the north (borehole 158) to more than 10 feet in the south. The sandy deposits are underlain, and sometimes intermixed, with unstratified brown till. The depth to top of gray till is between 11 to 15 feet. The sandy and silty layers are most likely of glacial outwash or alluvial origin.

2.4.2 Area 2 (Between JN-2 and JN-3)

The top 10 feet in this Area are dominated by poorly sorted brown fill and till. Only one stringer of clayey sand was observed at about 9.5 feet depth in hole 202. The top of gray till is at 9.5 feet depth.

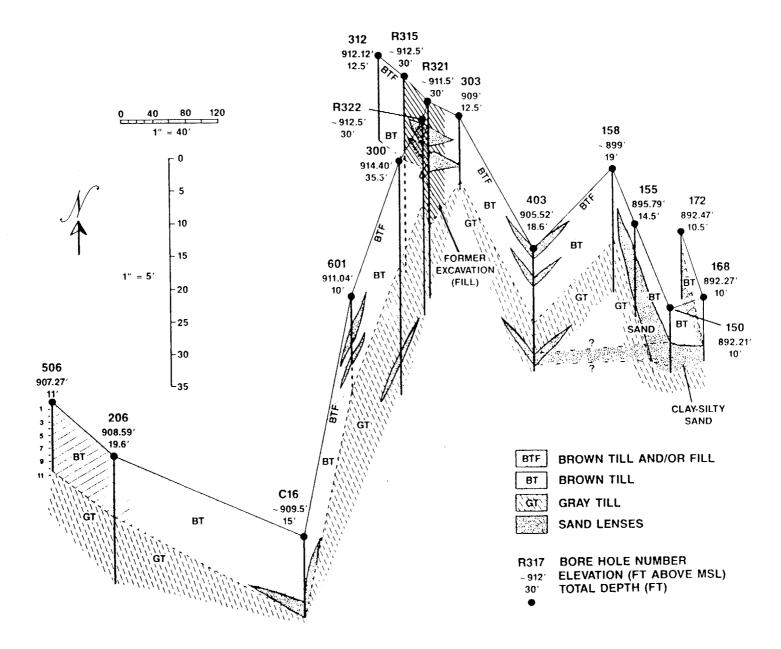


FIGURE 14. FENCE DIAGRAM FOR JN AREA

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#### 2.4.3 Area 3 (Rear of Building JN-4)

This Area includes the site of a former excavation pit which was later filled with loose sediments. The depth to top of gray till in areas outside the excavation is about 13 feet whereas in the deepest part of the old excavation it is as deep as 19 feet. The gray till in this Area is generally very compact with only a few thin sand and gravel lenses.

Sediments above the gray till are a mixture of soft to subcompact sand, silt, clay and gravel. This is mostly brown colored fill. Many sand and gravel rich segments were observed in the fill but they do not appear to be interconnected in all the boreholes. The general softness and medium to coarse grain size increases the possibility of rapid groundwater movement in this Area if an outlet exists.

#### 2.4.4 Area 4 (Rear of JN-1)

This Area is dominated by brown fill and till, subcompact with very few sandy lenses. The gray colored dense till is seen at the bottom of some holes at about a 10 feet depth. The presence of man-made inclusions such as asphalt, crumbled steel, and metallic fines in several holes indicates widespread presence of general fill at shallow depths in this Area. Borehole 403 was drilled to 18.6 ft and encountered a fine sand stringer near the bottom of the hole. The general particle size distribution, color, and texture of this fine sand/silt was identical to the fine sand/silt deposits found in the south end of Area 1.

#### 2.4.5 Area 5 (Loading Area of JN-3)

This Area is also dominated by brown subcompact till underlain by gray till at about 10 feet. These are not obvious sand lenses except the gravel from 2.5-5.0 feet in 503 which is from an abandoned drain.

#### 2.4.6 Area 6 (Loading Area of JN-1)

The top 10 feet are mostly brown fill and till. There is gravel just below surface. Sand lenses are present in 601 and 604 between 5 and 10 feet deep.

#### 2.5 GEOLOGY OF ABANDONED FILTER BED AREA

Characterization of the two retired, remediated filter beds was initiated in 1988, prior to Stage 1 main site characterization. 34 holes numbered 101 to 134 were drilled to a depth of 10 feet each (Figure 12). In July 1990, 10 additional boreholes were sampled as part of the Stage 2 Site Characterization. These boreholes were sampled to include all of the filter bed materials and the permeable sand and gravel layers. The total depth of the boreholes ranged between 10 and 14.5 feet.

Geologically this Area is made up of a variety of deposits including alluvial sands, sand and gravel artificial fills, and filterbed pebbles. Much of the material in the top 10 feet consists of soft, unconsolidated silty, clayey deposits intermixed with sand and gravel lenses. In the holes that penetrate the retired filter bed an unconsolidated layer of brown and black pebbles with white coatings can be seen. This varies in thickness from zero to 2.5 feet and is probably the old filterbed material. Coarse sand and gravel are widespread between 5 and 10 feet deep. These are underlain by brown and red colored till layers. The varied nature of sediments makes it difficult to correlate different layers, especially in the retired filter bed area.

#### 3.0 HYDROLOGY

#### 3.1 SURFACE HYDROLOGY

#### 3.1.1 Surface Water and Drainage

The surface hydrologic features at the site are Big Darby Creek and the lake, which occupies the valley of a tributary to the creek.

The lake was created in 1967 following the completion of the dam. The lake surface elevation is nominally 888 feet above mean sea level, the elevation of the spillway crest. At this elevation, the lake capacity is 275 acre feet. Its maximum depth is 32 feet. JN buildings are about 100 feet north of the shore of the lake and are 20 feet or more above it.

Silver Creek is the tributary dammed to make the lake. It originates approximately 1.5 miles west of the dam and it drains a watershed of 2.1 square miles. The dam is about 400 feet from Silver Creek's junction with Big Darby Creek, and the gradient of the creek is about 75 feet in 1.6 miles.

Big Darby Creek flows from north to south about 400 feet east of the dam and 900 feet east of the JN Area. The only known stream gage on Big Darby Creek is at Darbyville, about 20 miles south of the site and downstream. Records for this gaging station are presented in yearly USGS Water Data Reports (USGS, 1988).

The average discharge over the 63 years of record is 455 cu ft/sec. However, flow ranges widely from very low, when the stream is in pool stage and the only perceptible flow is over riffles, to very high, when the stream is at flood stage. The maximum discharge was 49,000 cu ft/sec on January 22, 1959. The minimum was 1.4 cu ft/sec on September 17, 1931. The gradient of the stream at the site is very low; about 1 ft in the 1.2 mi reach from the north boundary to the southern boundary of the Battelle property (at the Engineering Area).

#### 3.1.2 Precipitation, Evaporation, Infiltration

Climatological summary data were obtained from Ohio Department of Natural Resources (ODNR) for three stations surrounding the site: London, about 12 mi to the southwest; Irwin, about 19 mi to the northwest, and Columbus Airport, about 14 mi to the east. The average of the mean yearly precipitation values for the three stations is 37.96 in. per year. Published monthly precipitation records for these stations were also collected for 1983 through 1990 and can be obtained up to the current date at ODNR. In addition to above weather stations a number of private rain gages are located in the area as part of Central Ohio Rain Network (CORN). One of these, operated by Mr. David Cashell of ODNR is only 3 miles east of West Jefferson. Rain data for this station (Appendix 4) for the period June 18, 1989 to August 1990 was used for comparison with water level data from monitoring wells.

The evapotranspiration rate at the site is not known, however, the calculated evaporation loss from the lake is 18 in. per year for the record dry year (Battelle FEOD FILE). The runoff/infiltration-retention relationship for site soil is not known, but runoff appears to be fairly rapid. The calculated seepage loss from the reservoir is 6 in. per year (Battelle FEOD).

#### 3.2 GROUNDWATER HYDROLOGY

#### 3.2.1 General Description

A reconnaissance of available information was conducted to characterize the groundwater hydrology of the JN Area and its surroundings. This included review of soil borings and well logs available from construction of facilities and well installation and development at the site, as well as more general discussions of groundwater in Madison and Franklin counties.

The groundwater flow in the area of the JN site is controlled by lithology, stratigraphy, topography, precipitation, surface water, and construction at the site.

The hydrogeologic units at the site correspond to the five geologic units described previously: soil, alluvium, fill, till, and bedrock.

3.2.1.1 <u>Soil</u>. Soil hydrologic characteristics are largely the same as those of the parent materials. The soil interface with parent material is gradual, and does not constitute a permeability interface which would affect the flow of groundwater.

3.2.1.2 <u>Alluvium</u>. Alluvium is found on the floodplain of Big Darby Creek and on the floor of the valley occupied by the lake. Alluvium at the creek is 10 to 15 ft thick. It is characterized as relatively impermeable by ODNR. The valley floor of Silver Creek, under the lake, contained about 6 ft of alluvium and soil, the lower 2 to 3 ft of which were sand and gravel (Burgess and Niple, 1966).

The alluvium at the site is mostly fine- to very-fine-grained material derived from soils and glacial till. The permeability of alluvium is in the same range as till, with sand and gravel layers being most permeable.

At Big Darby Creek, alluvium is found a minimum of about 500 feet from JN facilities and direct contamination from them is unlikely. However, the filter beds and sewer outfall are in and on top of the alluvium. Lowconductivity silts and clays would inhibit the flow of water.

3.2.1.3 <u>Fill</u>. Fill, as described earlier is a highly variable material and its hydrologic characteristics are difficult to generalize. However, it is likely that the general fill is more conductive than native materials, and that engineered fills may also be more conductive than till or soil but to a lesser degree. This means that they could provide the most reasonable pathway for water to travel. Flow will follow the fill within the native materials. In order to be a significant pathway, however, the fill around buried pipes, under a floor slab, etc.). Secondly, the contaminant force affecting flow, the movement of water through the fill will be downward into less permeable materials rather than laterally (e.g., a small volume of water slowly leaking from a buried pipe surrounded by fill will quickly travel to the base of the fill and then slowly seep into till rather than being carried along the route of the pipe). If the gradient and volume of fluid are

great enough, lateral flow can occur through fill, and the fill can be a significant pathway.

3.2.1.4 <u>Till</u>. The till is densely compacted, unsorted glacial deposits consisting of silt, clay, sand and gravel, with low to very low hydraulic conductivity. The till's general effect on the groundwater regime is that of limiting recharge to any permeable units within or below it.

Outwash deposits, and well-sorted sand and/or gravelly layers which are found interbedded within the till, are good aquifers if large enough and are often used for water supply wells in other parts of Madison County. In the site area, most outwash occurrences in the till are limited to thin beds (generally 1 to 2 ft thick), that are of limited lateral extent. Some of these beds are dry and some constitute small groundwater zones. Water levels in boreholes may be sustained at a high level as the water from these zones drains into the hole, but as they become depleted, the water levels drop. These deposits may be found anywhere in the till, at any depth.

There does appear to be a thin silty sandy bed that can be traced in the storm sewer out fall area and appears to extend through Area 4. This sandy layer may intersect the foundation of JN1 and does allow a more rapid pathway for water to move than would be possible in the till alone.

3.2.1.5 <u>Bedrock</u>. The limestone/dolomite bedrock is used at the site and in most of the surrounding area for a water supply. The water is obtained in the top few feet of bedrock from weathered crevices. Confined conditions exist in the aquifer because it is confined by overlying low permeability till.

Records on the north and middle well at the Battelle facilities provide site-specific information about the bedrock aquifer. At the north well, close to JN-1, the static water level reported during well construction was 18.5 feet below pump base (elevation 878.5 ft MSL). At the middle well it was 40.92 ft (elevation 869.09 ft MSL). Transmissivity of the limestone is estimated at 9,050 to 16,000 gal per day per foot (Klaer & Associates, 1963).

Although productive, the bedrock aquifer is unlikely to be a pathway due to the intervening thickness of till. By bypassing the till (e.g. by following a well or deep borehole), a significant but unlikely pathway to the bedrock aquifer could be available for groundwater to flow.

#### 3.2.2 Specific Sampling Areas Hydrogeology

As part of the Stage 1 Site Characterization 12 monitoring wells with 2 inch inner diameter PVC screens were installed (Table 2) according to quality assurance procedure SC-SP-004.1 (1989). In addition, three 4 inch diameter stainless steel wells, CO3, CO9 and C16, were installed for chemical sampling. All of the wells were installed using a hollow-stem auger drill. Schematic well construction diagrams for all the wells are included in Appendix B. Two of the wells: #158 and #300 were later damaged by vehicular traffic and their use for data collection had to be discontinued.

3.2.2.1 <u>Water Levels</u>. The water level readings for monitoring wells were taken at regular intervals. These data are listed in Table 3. Well hydrographs were constructed from the water level data (Figures 15 and 16) and these give some information on the nature of flow regimes and extent of water level fluctuations at the site. The well hydrographs also show the precipitation data for the duration of study.

Wells in Area 1 show that water surface in this area is shallow (3-5 feet deep). This is consistent within the thick silt and sand deposits in this Area, but in well #158 the three available values show a deeper water surface (7.6-9 feet). This observation corresponds with the absence of silty sand lenses in well #158.

Well hydrographs in Area 1 also indicate that the dominant flow direction is from the NW to SE wells (towards the lake). This is shown by consistently higher water levels in well #155 and 172 than in well 150 and 168 which are closer to the lake.

Similarly the dominant flow from Areas 3, 4 and 6 is towards the lake. This is shown by hydrographs of the wells in these areas. These are similar to, but consistently higher than those in Area 1. The only exception to above is well 300 (now damaged) which was completed much deeper in the gray till and has a water level much lower than the surrounding wells.

Sampling Area and Well Number	Top of Well Inner Casing Elevation (feet)	Ground Surface Elevation (feet)	Total depth (feet)	Screened Interval Depth (feet)
AREA 1 150	894.46	892.21	10.0	4.8-9.8
155	898.46	895.79	14.5	9.1-14.1
158*	900.85	DAMAGED WELL		
168	894.62	892.27	10.0	4.5-9.5
172	895.00	892.47	10.5	4.5-9.5
AREA 2 206	910.78	908.59	19.6	14.4-19.5
AREA 3 300*	914.21	911.95	35.5	30.3-35.4
306	913.20	911.22	15.0	9.7-14.7
312	914.16	912.12	12.5	6.3-11.4
AREA 4 403	908.37	905.52	18.6	13.6-18.6
AREA 5 506	909.72	907.27	11.0	6.0-11.0
AREA 6 601	913.10	911.04	10.0	4.0-9.0
CHEMICAL SAMPLE CO3**	ING 912.15	≈911	11.4	5.5-11.4
CO9**	895.32	≈892	8.9	3.0-8.9
C16**	911.95	≈909.5	13.6	8.0-13.6

TABLE 2. MONITORING WELL DATA

\* Well damaged \*\* 0.33 Feet Diameter Stainless Steel Well

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	6/19/89		7/5-6/89 10/26-27/89		11/10/89		12/7/89		1/12/90		3/28/90			
le]]	Depth Feet	Elevation Feet	Depth Feet	Elevation Feet	Depth Feet	Elevation Feet	Depth Feet	Elevation Feet	Depth Feet	Elevation Feet	Depth Feet	Elevation Feet	Depth Feet	Elevation Feet
150	3.90	890.56	3.92	890.54			6.00	888.46	6.86	887.60	5.07	889.39	4.11	890.35
155	5.29	893.17	5.60	892.86		** **	6.05	892.41	7.73	890.73	6.58	891.88	6.14	892.32
158	7.61	893.24	7.66	893.19			9.25	891.60						
168	3.68	890.94	3.75	890.87			6.23	888.39	7.01	887.61	5.05	889.57	3.83	890.79
172	3.33	891.67	3.50	891.50			5.23	889.77	6.61	888.39	5.03	889.97	3.86	891.14
206			21.09	889.69	9.39	901.39			13.46	897.32	14.07	896.71	8.30	902.48
300			15.72	898.49					23.34	890.87	23.44	890.76	22.53	891.68
06			5.58	907.62	8.08	905.12			7.93	905.27	6.77	906.43	6.21	906.99
812			9.84	904.32					6.97	907.19	6.49	907.67	6.45	907.71
103					10.86	897.51	10.52	897.85	11.95	896.42	10.78	897.59	10.31	898.06
506					7.50	902.22			7.56	902.16	6.94	902.78	6.93	902.79
501		<b></b>							7.08	906.02	6.29	906.81	6.06	907.04
203						• <b>•</b> ••					6.77	905.38	5.58	906.57
:09											5.43	889.89	4.27	891.05
216											8.63	903.32	8.75	903.20

TABLE 3. WATER LEVEL DATA

β

	5/3 Depth	3/90 Elevation		0/90 Elevation	7/ Depth	3/90 Elevation	8/13/ Depth	90 Elevation	9/4, Depth	/90 Elevation
Well	Feet	Feet	Feet	Feet	Feet	Feet	Feet	Feet	Feet	Feet
150	4.12	890.34	3.41	891.05	4.24	890.22	4.11	890.35	4.20	890.26
155	6.29	892.17	3.95	894.51	6.72	891.74	6.37	892.09	10.13	888.33
158										
168	3.87	890.75	3.24	891.38	3.98	890.64	3.87	890.75	3.94	890.68
172	3.87	891.13	3.02	891.98	4.23	890.77	3.96	891.04	4.30	890.70
206	8.26	902.52	7.50	903.28	7.64	903.14	7.72	903.06	8.51	902.27
300			•-		• -					
306	6.46	906.74	4.95	908.28	6.40	906.8	6.76	906.44	8.19	905.01
312	6.56	907.60	5.36	908.80	6.50	907.66	6.61	907.55	7.43	906.73
403	10.57	897.80	8.50	899.87	10.25	898.12	10.17	898.2	10.99	897.38
506	7.14	902.58	5.63	904.09	7.14	902.58	7.24	902.48	7.74	901.98
601	6.05	907.05	4.06	909.04	6.28	906.82	6.03	907.07	6.48	906.62
C03	5.77	906.38	3.82	908.33	5.71	906.44	5.98	906.17	7.63	904.52
C09	4.29	891.03	3.17	892.15	4.90	890.42	4.52	890.8	5.04	890.28
C16	9.22	902.73	4.72	907.23	9.59	902.36	9.92	902.03	11.04	900.91

### TABLE 3. WATER LEVEL DATA (Continued)

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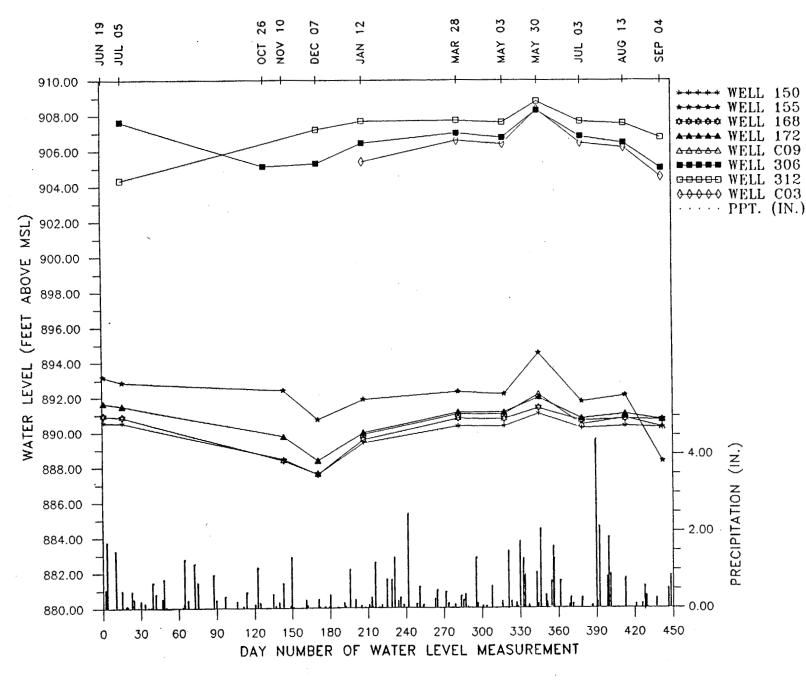
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FIGURE 15. WELL HYDROGRAPHS FOR AREAS 1, AND 3

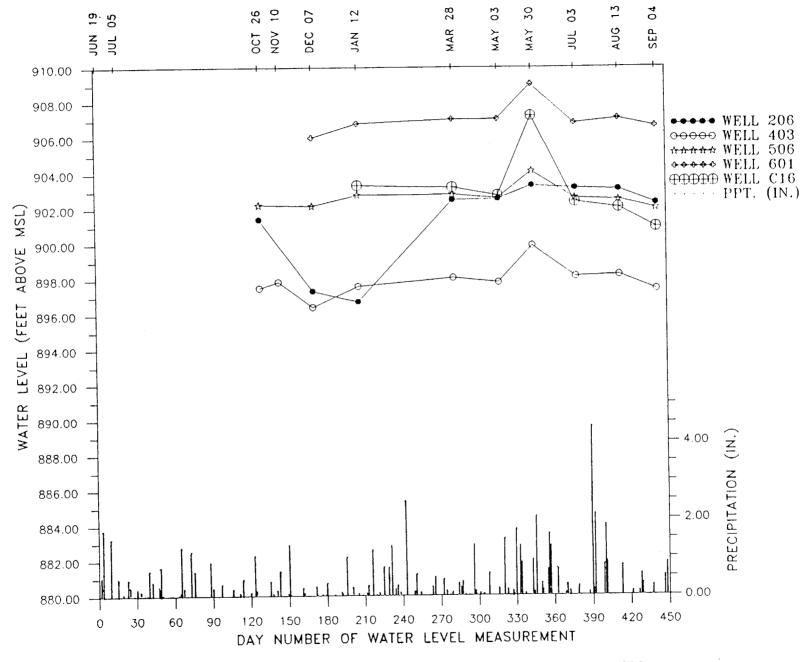


FIGURE 16. WELL HYDROGRAPHS FOR AREAS 2, 4, 5, AND 6 AND WELL C16

It appears that outwash sand layers in Areas 1, 4, and possibly 3 may form a somewhat interconnected aquifer. The sand stringer in Area 4 is found at depth and is confined by gray till, but still connected with the silty sands found in Area 1.

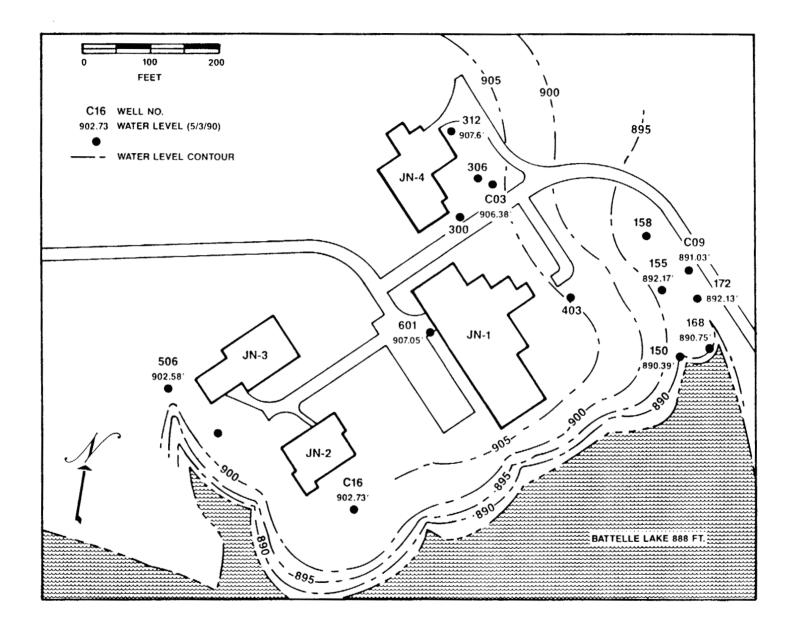
The water levels in shallow monitoring wells were used along with topography and the lake water-level to construct shallow water-table-surface map for JN facilities (Figure 17). This is only an approximate map and is meant to provide only a general idea of the shallow flow regime. As seen in Figure 17, the water table in the area is sloping away from the JN facility buildings. Most of the shallow groundwater flow is towards the lake. However, some of the flow from Area 3 is towards the flood plain of Big Darby Creek. The ultimate discharge for all shallow flow is into the Big Darby Creek. We can make some rough estimates of existing gradients based on the knowledge of water levels and dominant flow directions. For example, for the flow from Areas 3 and 6 towards Area 1 the approximate gradient is 0.04 ft/ft whereas, the gradient between wells 155 and 150 is about 0.016.

In Areas 2 and 5, minimum depth to water observed during drilling was about 5 feet, but only in boreholes 202, 504 and 506. For the remainder of boreholes, the water depth was greater than 10 feet (about 15 feet in borehole 206). Flow from these areas and from C16 appears to be towards the lake as shown by higher water levels in these wells compared to the lake levels.

Vertical flow gradients were determined from general observations of water levels in consecutively deeper wells near each other. The water levels were lower in deeper wells. Therefore the vertical flow gradients appear to be downward and imply that most of the area is a recharge area.

#### 3.2.3 Hydraulic Conductivity Testing

This section summarizes the hydraulic conductivity determination by "slug tests" that were performed in the Stage 1 characterization of the Nuclear Sciences Area of Battelle's West Jefferson Site. The testing was done in all 15 groundwater monitoring wells as part of a comprehensive characterization and in accordance with SC-SP-009 (1989).



### FIGURE 17. WATER TABLE SURFACE MAP FOR JN FACILITIES

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The purpose of a "slug test" is to determine the hydraulic conductivity or transmissivity of an aquifer in the immediate area of a well. The test consists of causing an instantaneous change in the water level of a well by suddenly introducing or removing a known volume of water or solid cylinder, and observing the recovery of the water level with time.

3.2.3.1 <u>Data Analysis</u>. The time versus water-level data from the slug tests was analyzed using two standard methods. The Bouwer and Rice method (1976, 1989) was used for the data, hydrogeology, and well construction dimensions that best defined an unconfined aquifer. The other method (1967), the Copper et. al. method, was used for situations that more closely resembled a confined or semi-confined aquifer. Both methods use an analysis of semilog plots of relative water level change versus time.

In many cases the plotted data clearly matched one method or the other, but in several instances, the data was able to be analyzed using both methods. When the data could be properly analyzed using both methods, the answers were usually within an order of magnitude of each other. The techniques of slug test evaluation can be found in the technical papers cited in SC-SP-009 and the reference section.

3.2.3.2 <u>Results</u>. The results of the analysis fall within the range of values normally associated with the geologic materials present at each well. The results can be grouped into four main categories by the material present, either the near surface brown till/fill material, the confined and unconfined sandy zones, and the dense grey till. Table 4 lists the results for each well by method and the best answer is indicated. The most appropriate hydraulic conductivity value for each well is shown in Figure 18.

These results as well as the boring logs both agree that in general wells 306, 312, 506, CO3, and C16 were completed in the brown till with some wells extending slightly into the grey till. The hydraulic conductivity values are moderate to low, ranging from 2.6 x  $10^{-4}$  cm/sec to 3.0 x  $10^{-7}$  cm/sec.

Wells 150, 155, 168, 172, 300, 403, 601, and CO9 were completed in the sandier materials of higher hydraulic conductivity. Wells 150 and 168 intersect a sandy deposit under unconfined conditions. Wells 155, 172, 403

			CONDUCTIVITY	<del></del>			
	Pourson 8	Rice, 1976	Cooper et al., 1967				
		4- <u></u>		cm/sec			
Well #	ft/sec	cm/sec	ft/sec				
150	* 1.4 × 10 <sup>-5</sup>	4.3 x 10 <sup>-4</sup>	$2.9 \times 10^{-5}$	8.9 x 10 <sup>-4</sup>			
155	$4.2 \times 10^{-6}$	1.3 x 10 <sup>-4</sup>	* 2.0 x 10 <sup>-6</sup>	$6.0 \times 10^{-5}$			
158	* 8.5 x 10 <sup>-8</sup>	2.6 x 10 <sup>-6</sup>	2.1 × 10 <sup>-9</sup>	6.5 x 10 <sup>-8</sup>			
168	* 1.3 × 10 <sup>-4</sup>	$4.1 \times 10^{-3}$	$2.0 \times 10^{-4}$	$6.2 \times 10^{-3}$			
172	$2.8 \times 10^{-5}$	$8.5 \times 10^{-4}$	$* 4.0 \times 10^{-5}$	$1.2 \times 10^{-3}$			
206	* 9.1 x 10 <sup>-9</sup>	$2.8 \times 10^{-7}$	$1.4 \times 10^{-10}$	4.2 × 10 <sup>-9</sup>			
300			* 7.9 x 10 <sup>-4</sup>	2.4 × $10^{-2}$			
306	$1.3 \times 10^{-5}$	$3.8 \times 10^{-4}$	* 9.4 x 10 <sup>-6</sup>	$2.9 \times 10^{-4}$			
312	* 2.7 x 10 <sup>-7</sup>	8.3 x 10 <sup>-6</sup>	9.9 x 10 <sup>-9</sup>	$3.0 \times 10^{-7}$			
403	2.0 x 10 <sup>-5</sup>	5.9 x $10^{-4}$	* 4.0 x $10^{-5}$	$1.2 \times 10^{-3}$			
506	* 6.9 x 10 <sup>-7</sup>	2.1 x 10 <sup>-5</sup>					
601	* 6.6 x 10 <sup>-6</sup>	$2.0 \times 10^{-4}$					
C03	* 1.0 x 10 <sup>-7</sup>	3.1 x 10 <sup>-6</sup>					
C09	* 1.3 × 10 <sup>-7</sup>	3.9 x 10 <sup>-6</sup>					
C16	* 1.3 x 10 <sup>-7</sup>	$3.9 \times 10^{-6}$					

TABLE 4. WEST JEFFERSON HYDRAULIC CONDUCTIVITY RESULTS

\* Indicates best method (results) for the slug test data.

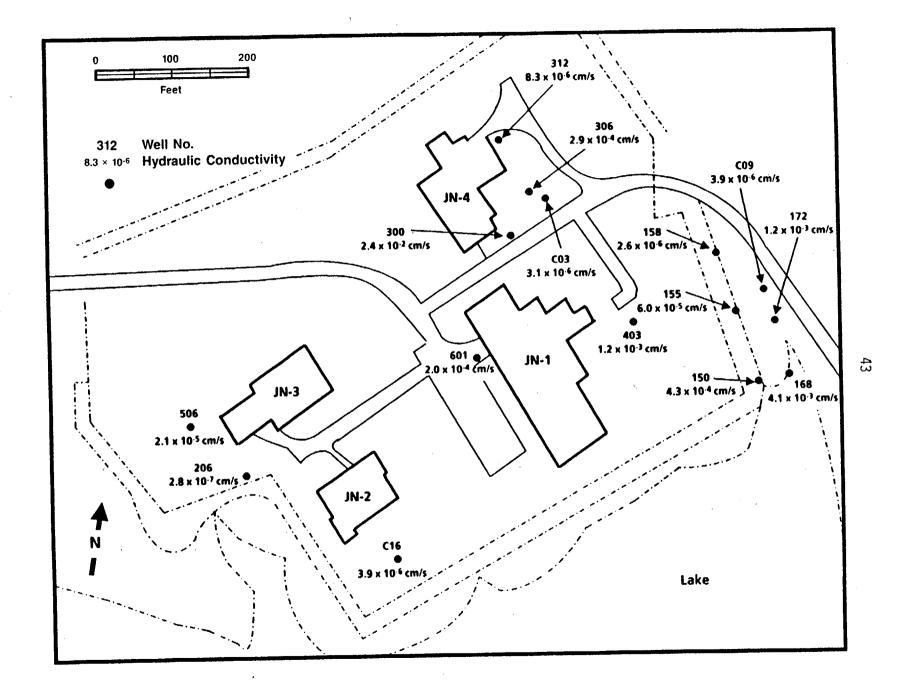


FIGURE 18. HYDRAULIC CONDUCTIVITY DISTRIBUTION

and CO9 also are believed to be completed in this same sandy deposit but are under semi-confined or confined conditions, that is a lower conductivity material overlies the sand material. All of these values fall within a relatively narrow range from  $1.2 \times 10^{-3}$  cm/sec to  $6.0 \times 10^{-5}$  cm/sec. Well 300 may be completed in this same sand, but this is not certain. This well had the highest hydraulic conductivity of all wells tested at  $2.4 \times 10^{-2}$  cm/sec. Well 601 was completed in more conductive material but not the same continuous deposit as the others.

The remaining wells 158 and 206 were completed in dense grey till and have low hydraulic conductivities of 2.6 x  $10^{-6}$  cm/sec and 2.8 x  $10^{-7}$  cm/sec, respectively.

#### 4.0 CONCLUSIONS

A geologic and hydrogeologic characterization of the JN Area and retired filter beds at Battelle's West Jefferson Site was performed. A total of 168 boreholes were drilled as part of site characterization.

A review of the geologic information obtained through drilling indicates that the shallow geology at the site is same as the general geology in central Ohio. Five major geologic materials present at the site are: soil, alluvial deposits, artificial fill, glacial till and limestone bedrock. The shallow geology is dominated by brown and gray colored till. Sand/gravel lenses of limited extent are present within the till. Artificial fill is dominant in Area 3 and in the retired filter beds area. Alluvial deposits are present in the filter beds area.

Hydrologically the lake and the Big Darby Creek are dominant surface features. The lake and Big Darby Creek also serve as the discharge locations for shallow groundwater flow. The hydrologic units at the site correspond to the five geologic units described previously: soil, alluvium, fill, till, and bedrock. Slug tests for permeability determination show that the till deposits have very low hydraulic conductivity, and the silty, sandy lenses within the till have moderate hydraulic conductivity.

#### 5.0 REFERENCES

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SC-SP-004 Collection of Soil Core Samples in Support of Site Soil Characterization Revision 1, 1989.

SC-SP-004.1 Procedure for Well Installation and Well/Borehole Abandonment Revision 1, 1989.

SC-SP-009 Determination of Hydraulic Conductivity using Slug Test Method Revision 0, 1989.

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United States Department of Agriculture Soil Conservation Service in cooperation with Ohio Department of Natural Resources. <u>Soil Survey of Madison</u> <u>County, Ohio</u>. Government Printing Office, issued June, 1981.

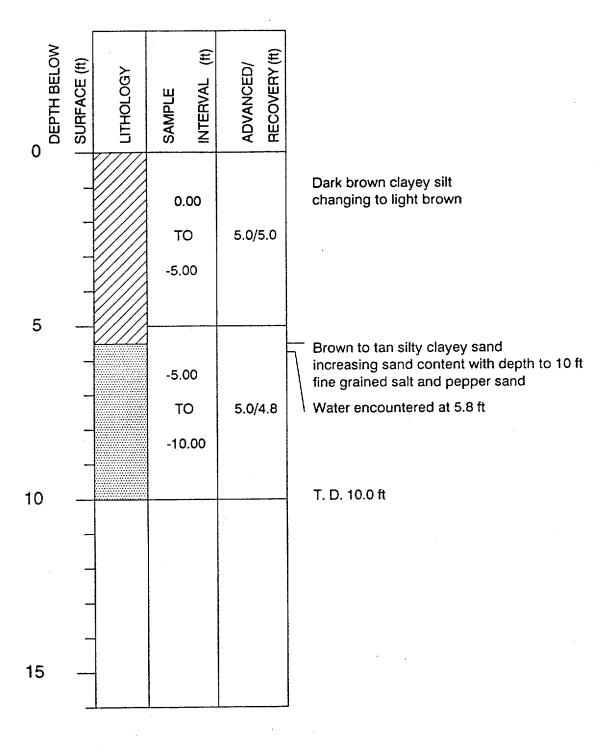
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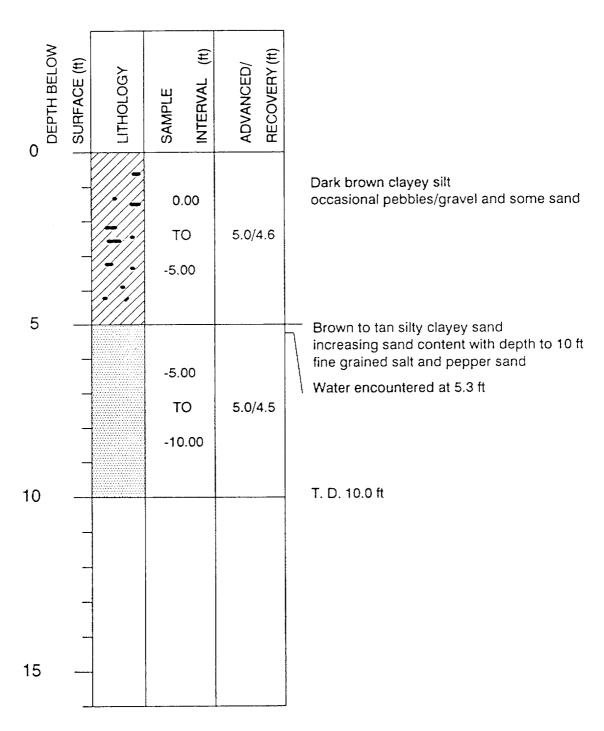
APPENDIX 1

BORING LOGS FOR ALL SAMPLING AREAS

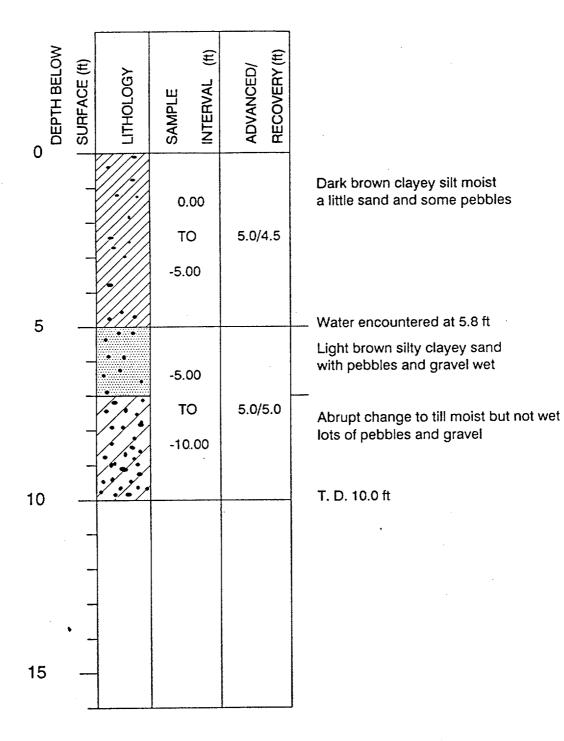
LOCATION: Sample Area 1



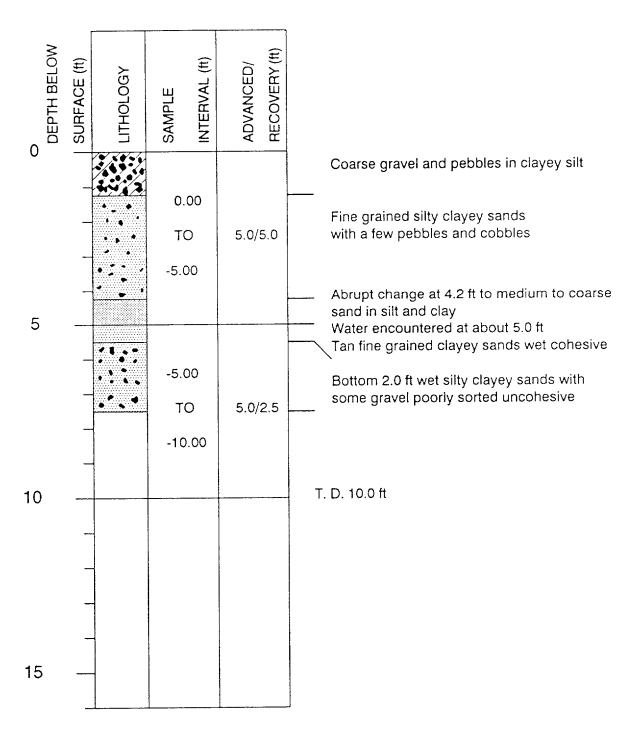
LOCATION: Sample Area 1

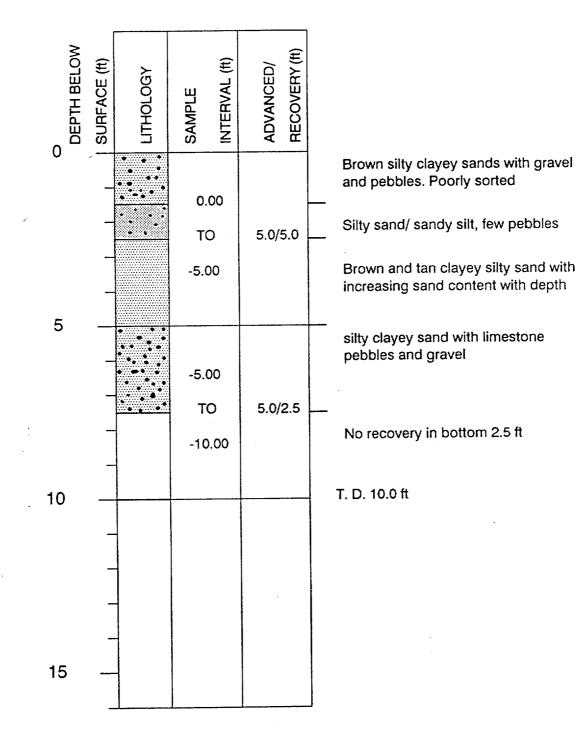


LOCATION: Sample Area 1



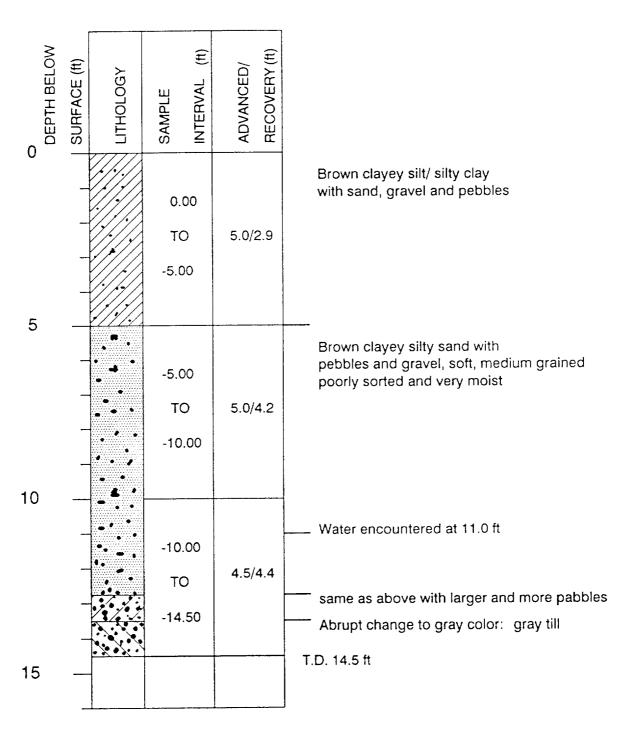
LOCATION: Sample Area 1





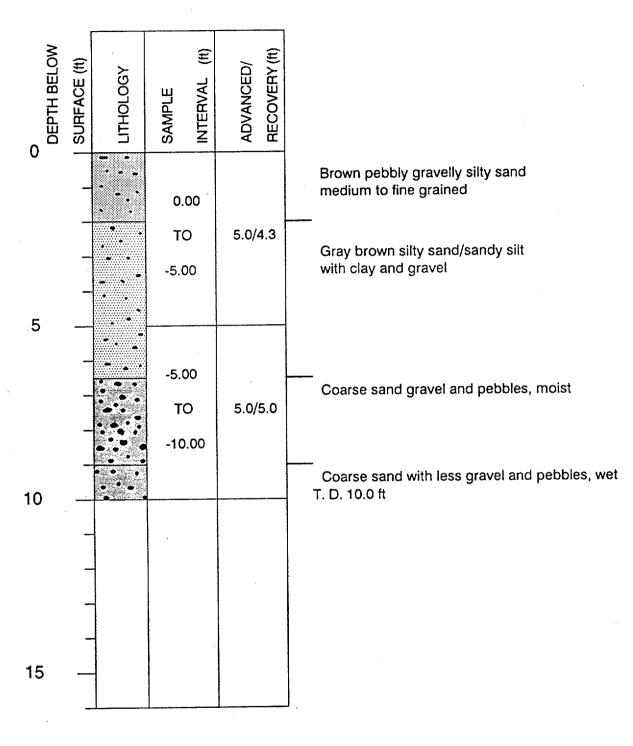
LOCATION: Sample Area 1

27 APRIL 1989



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LOCATION: Sample Area 1



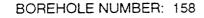
LOCATION: Sample Area 1

28 APRIL 1989

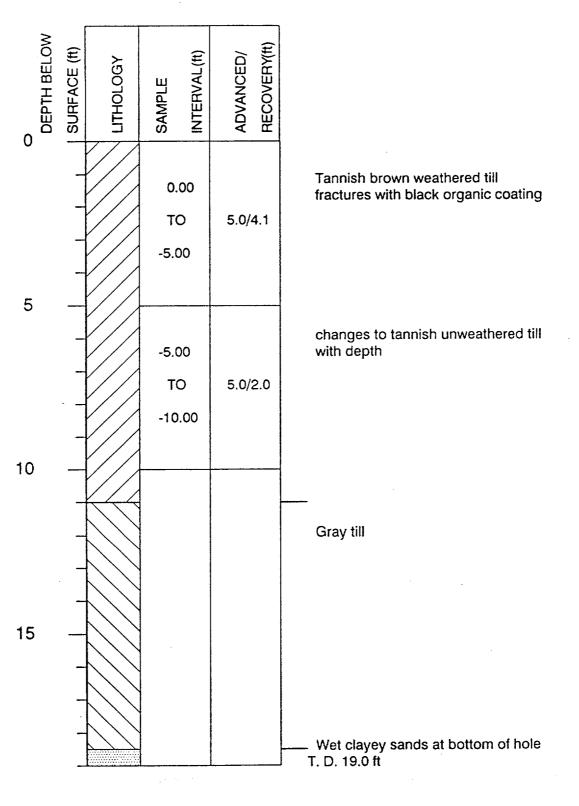
DEPTH BELOW RECOVERY (ft) INTERVAL (ft) SURFACE (ft) ADVANCED/ ΓΙΤΗΟLOGY SAMPLE 0 0.00 5.0/3.2 ТО -5.00 5 -5.00 ТО 5.0/3.5 -10.00 T. D. 10.0 ft 10 15

Brown gravelly pebbly silty clayey sands uncohesive dry

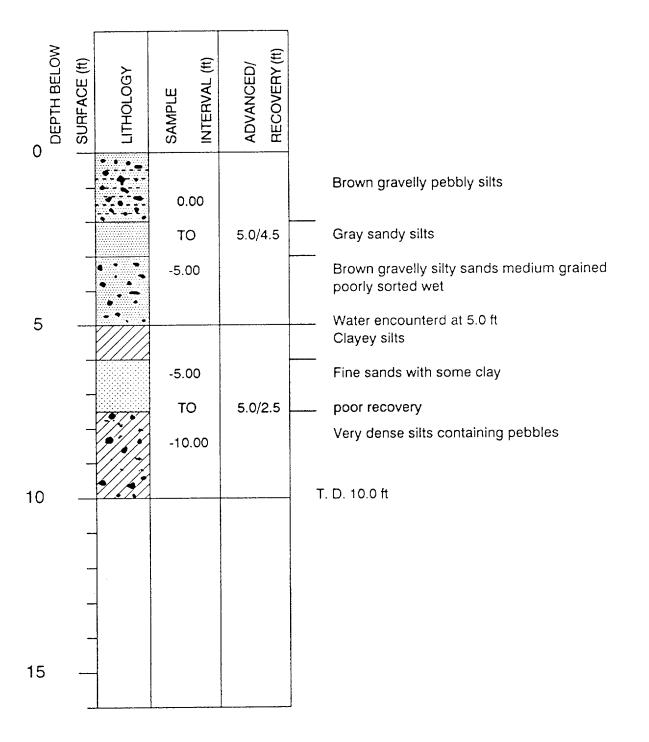
Silty clayey sands with finer grained sand and fewer pebbles and gravel moist

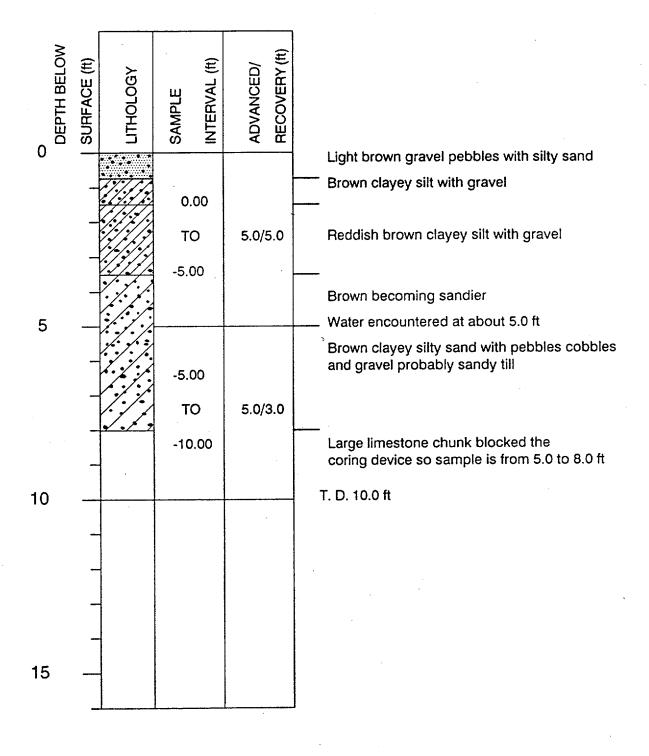


LOCATION: Sample Area 1



LOCATION: Sample Area 1





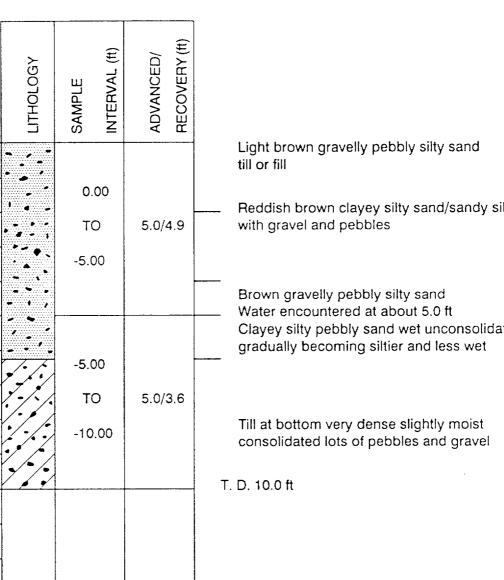
DEPTH BELOW

0

5

SURFACE (ft)

LOCATION: Sample Area 1



10

15

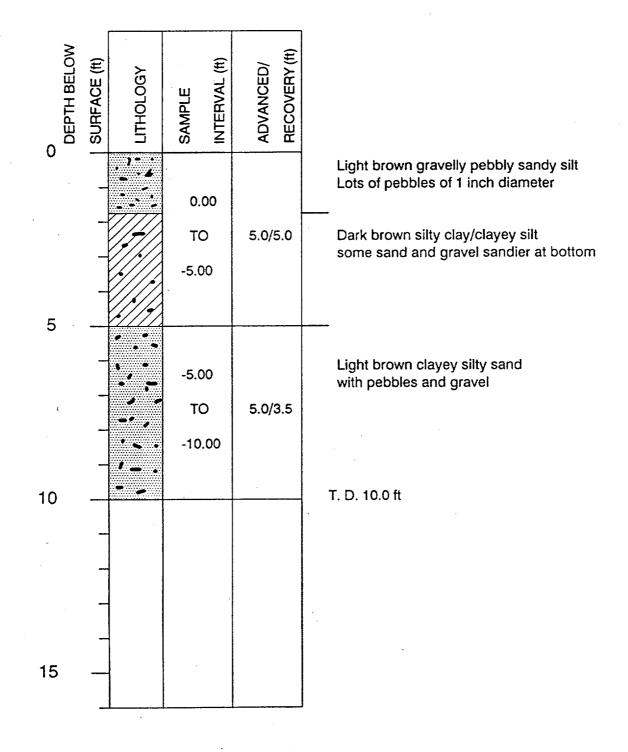
26 APRIL 1989

Reddish brown clayey silty sand/sandy silt

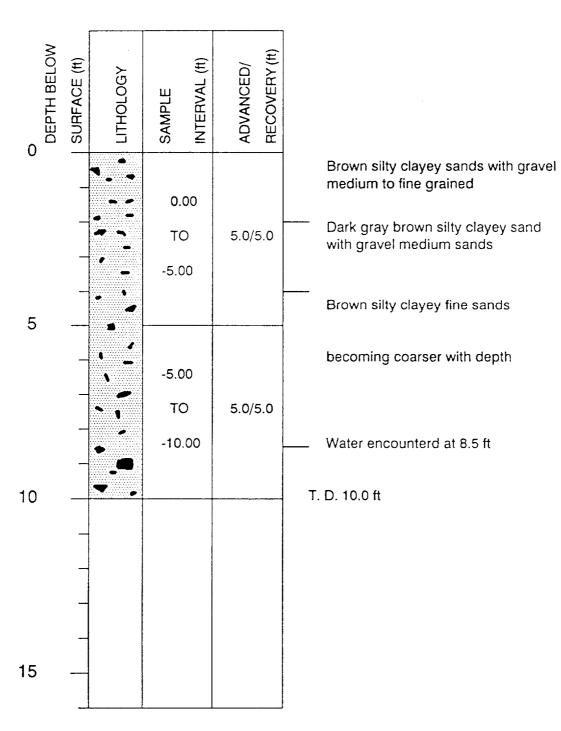
Clayey silty pebbly sand wet unconsolidated

#### BOREHOLE NUMBER: 163B

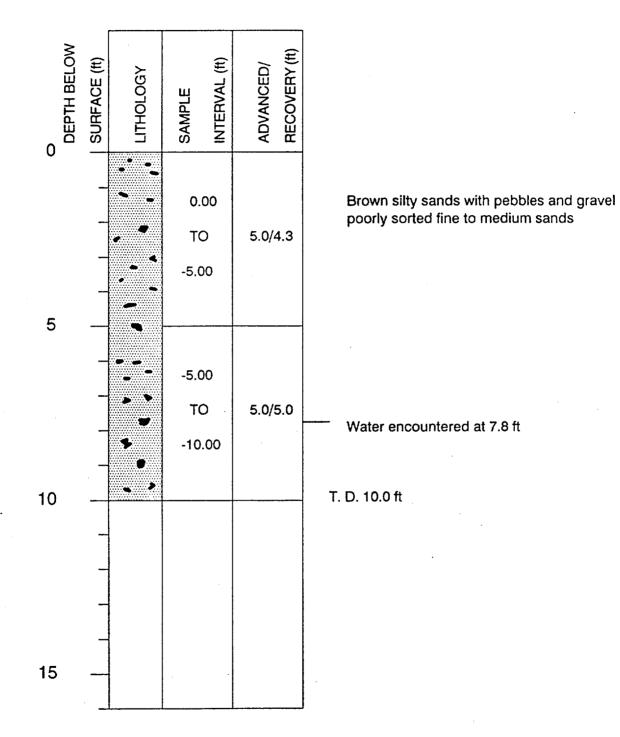
LOCATION: Sample Area 1



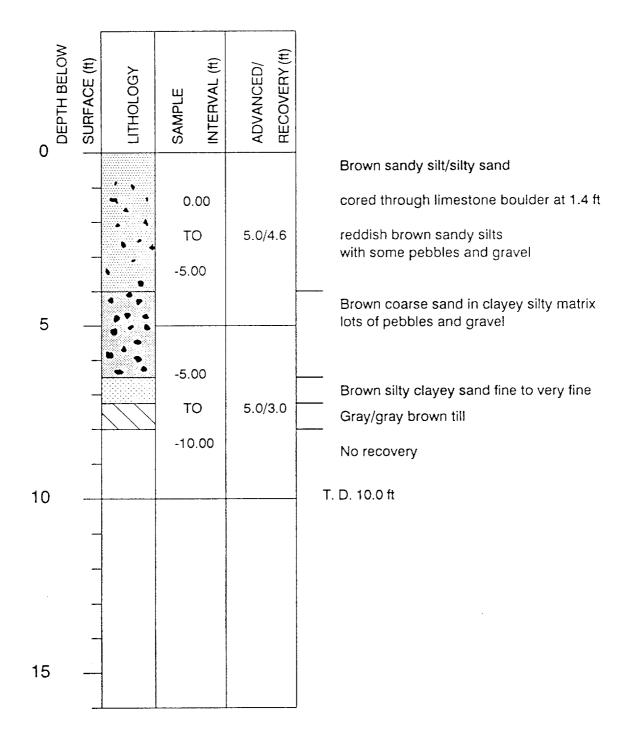
LOCATION: Sample Area 1



1

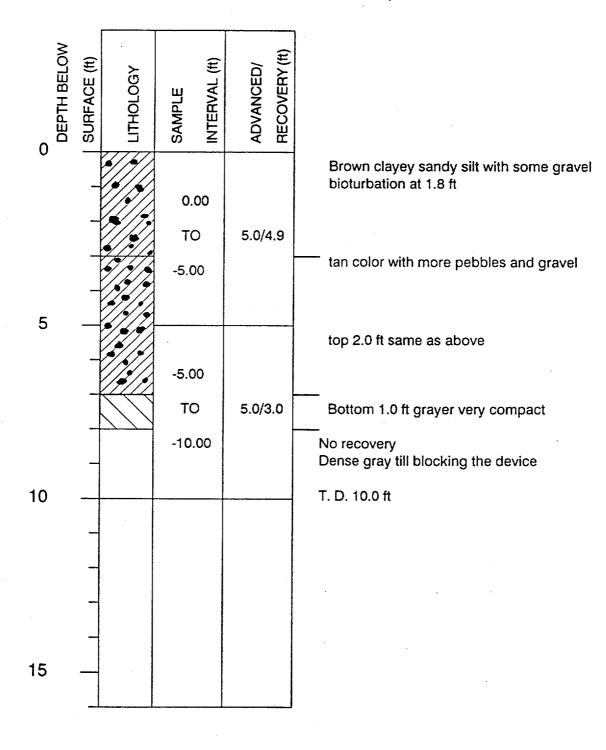


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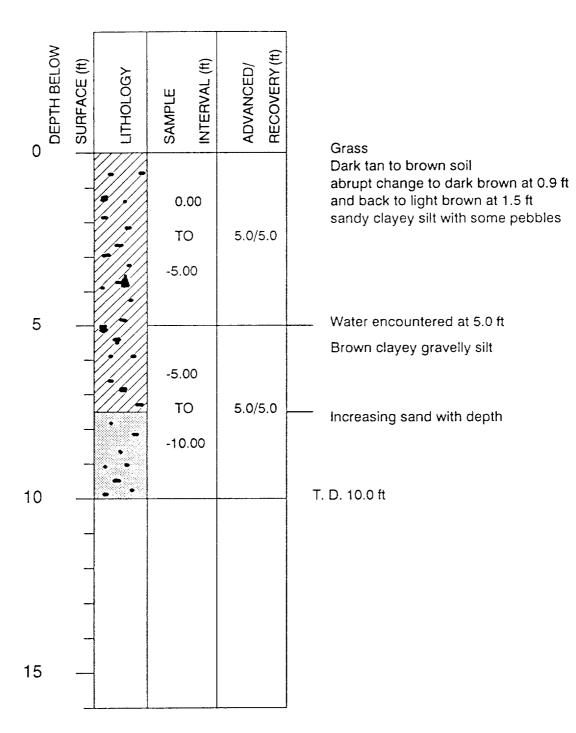
LOCATION: Sample Area 1

28 APRIL 1989



LOCATION: Sample Area 1

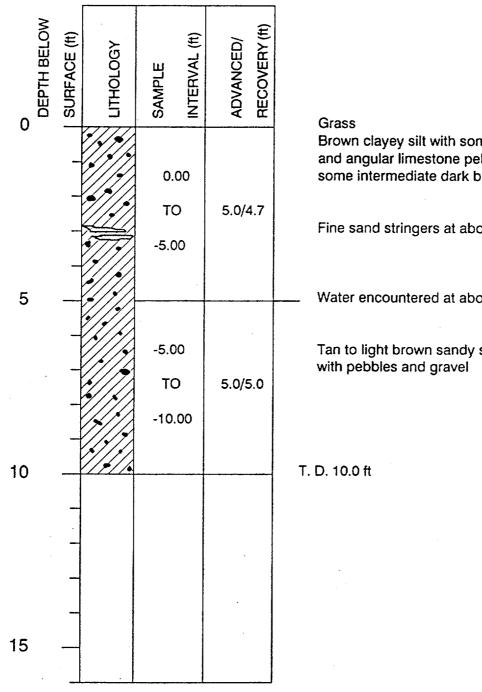
25 APRIL 1989



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LOCATION: Sample Area 1

25 APRIL 1989



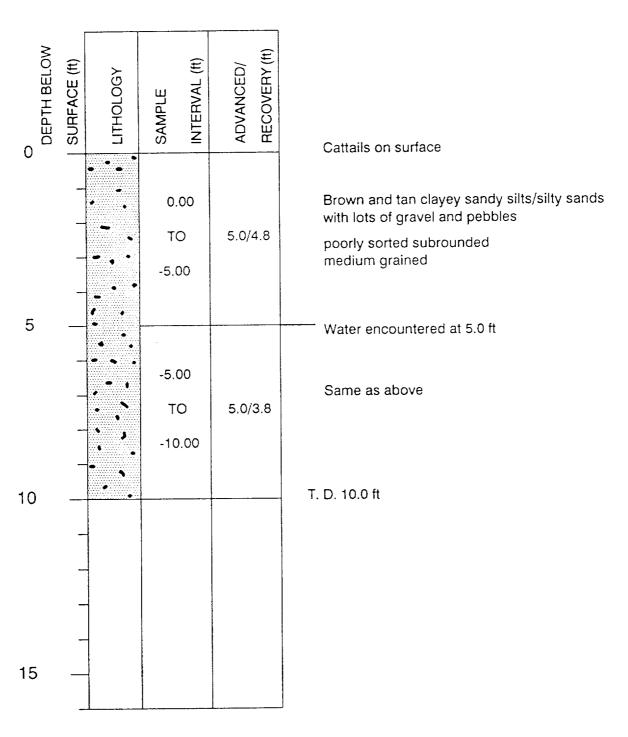
Brown clayey silt with some sand and gravel and angular limestone pebbles some intermediate dark brown zones

Fine sand stringers at about 3 ft., tan

Water encountered at about 5.0 ft

Tan to light brown sandy silty clay

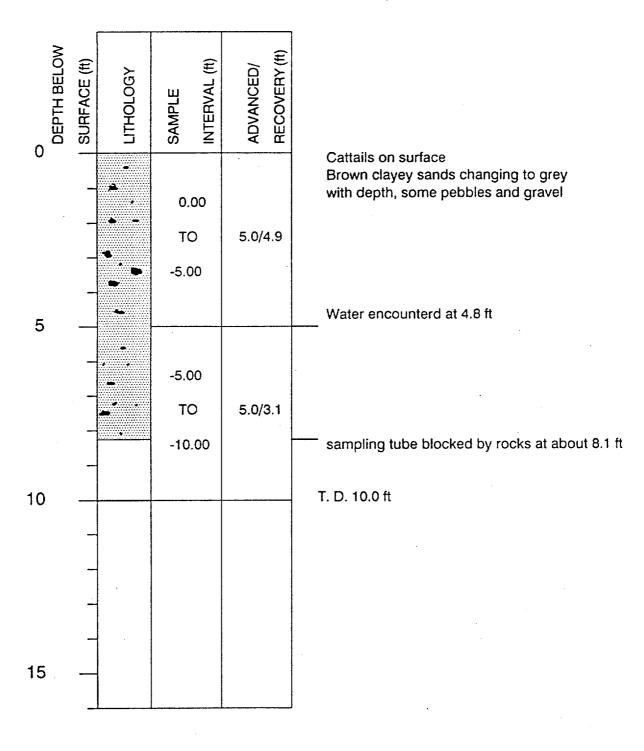
LOCATION: Sample Area 1



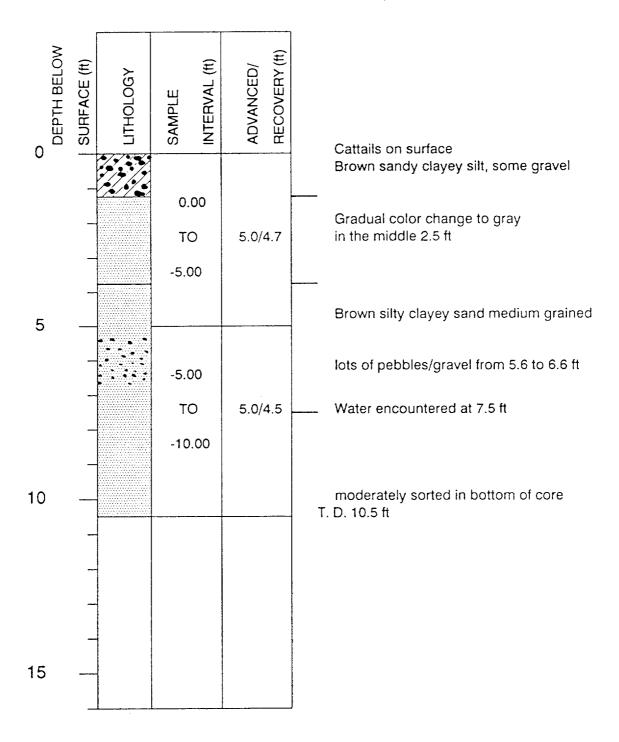
1

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LOCATION: Sample Area 1

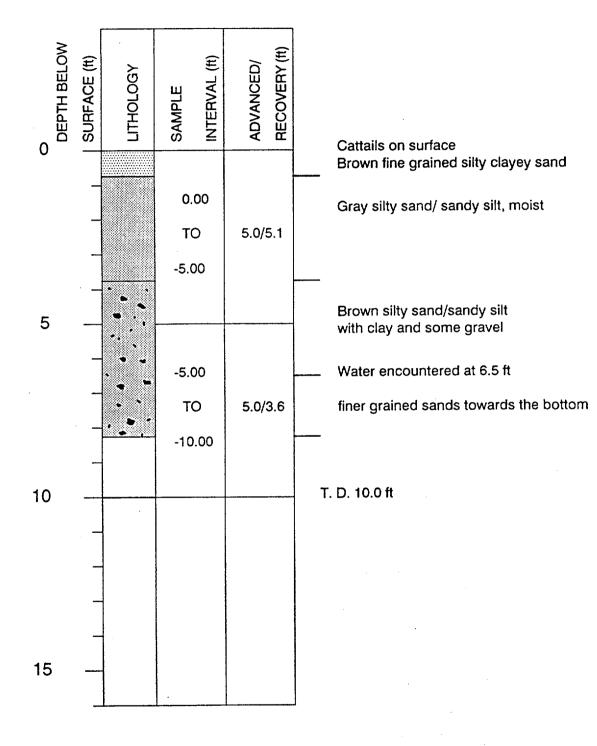


LOCATION: Sample Area 1

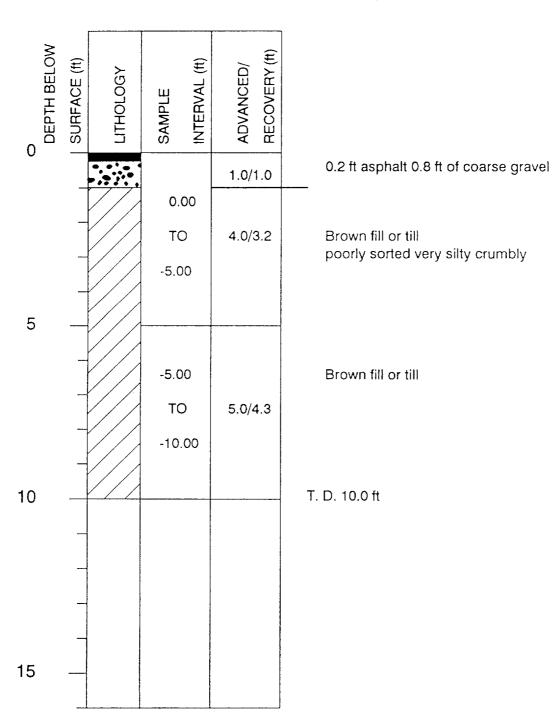


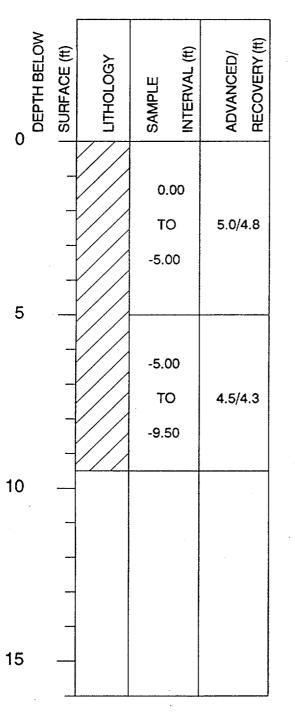
LOCATION: Sample Area 1

27 APRIL 1989



1 1





#### Grass

#### Reddish brown till or fill

.

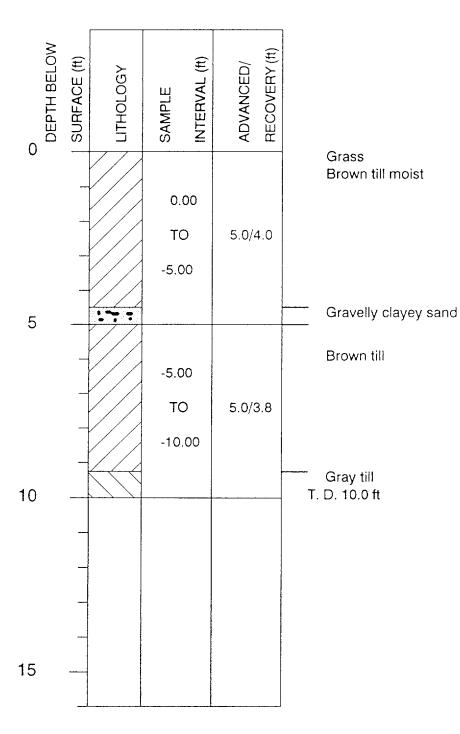
#### brown till

.

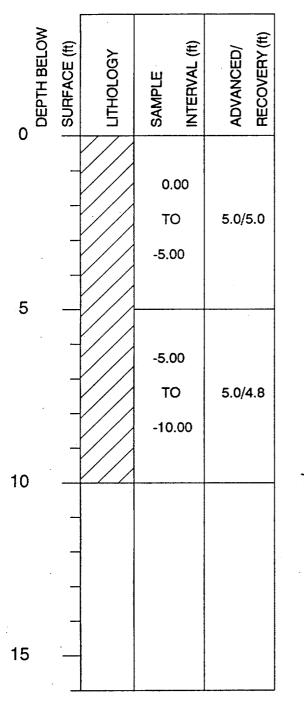
wet from 8.0 to 8.5 ft

T. D. 9.5 ft hit rock or some other obstacle

LOCATION: Sample Area 2



LOCATION: Sample Area 2

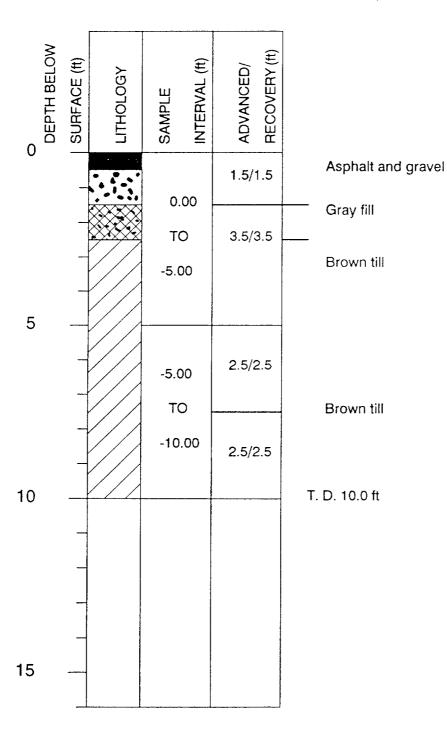


#### Grass

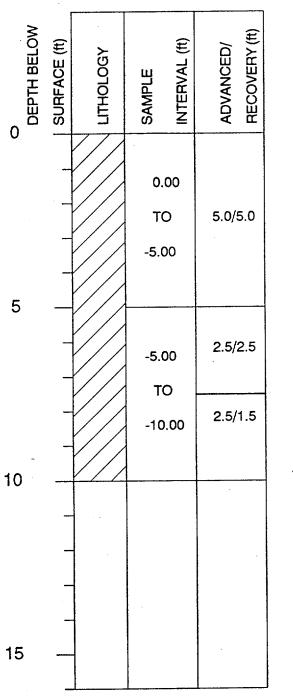
Brown till or fill poorly sorted moist

# Brown till

T. D. 10.0 ft



27 JUNE 1989

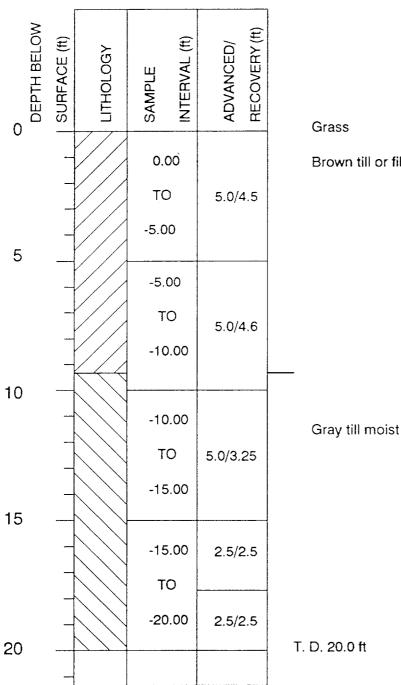


Grass

Brown till or fill

Brown till poorly sorted

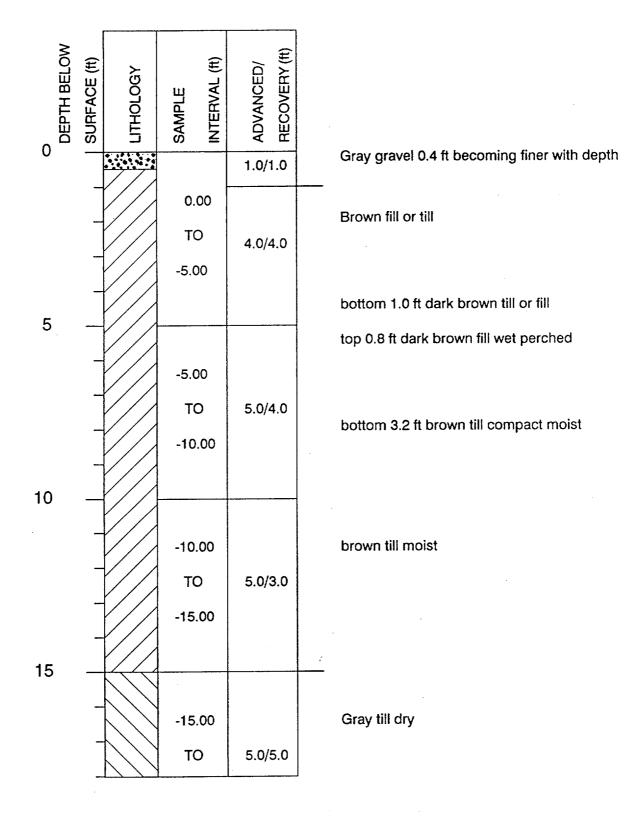
T. D. 10.0 ft



#### Brown till or fill moist

29 JUNE 1989

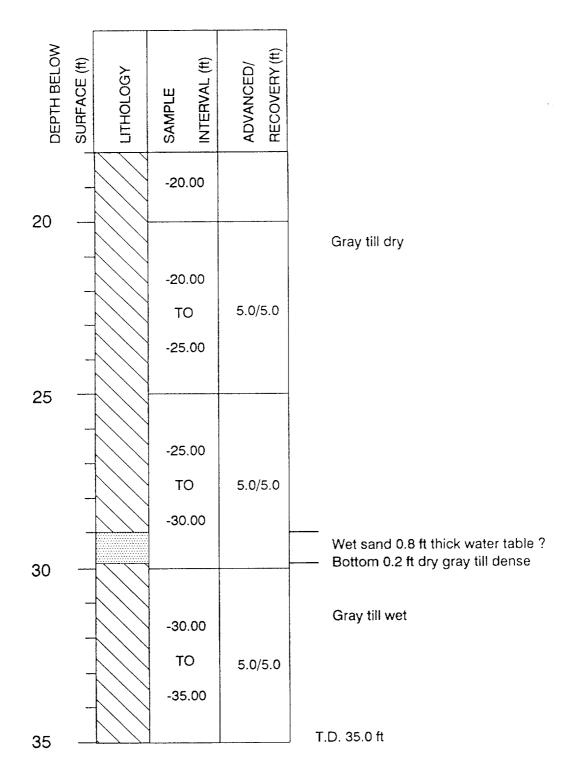
PAGE 1 OF 2



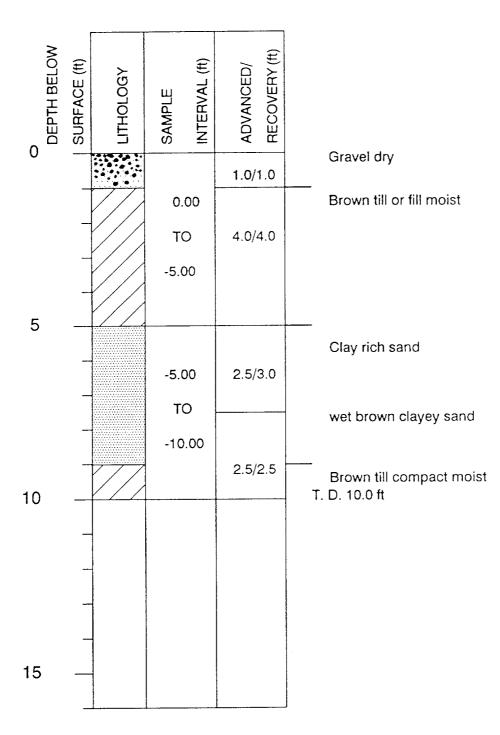
#### PAGE 2 OF 2

BOREHOLE NUMBER: 300

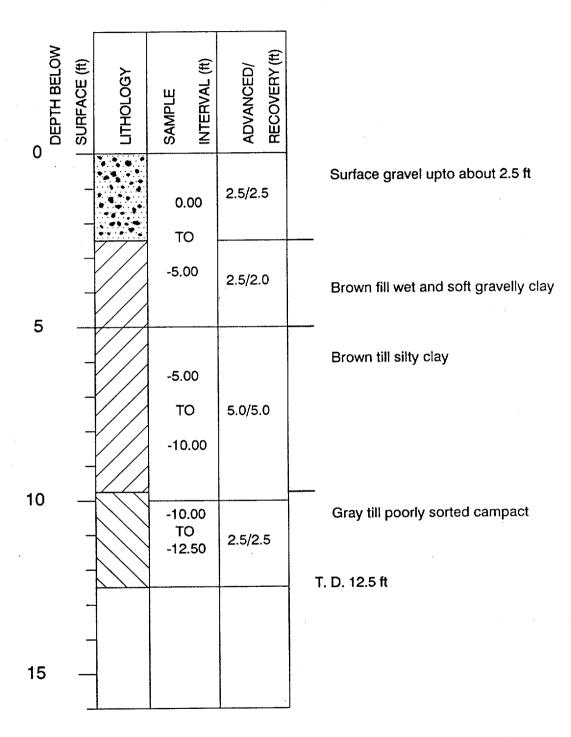
LOCATION: Sample Area 3



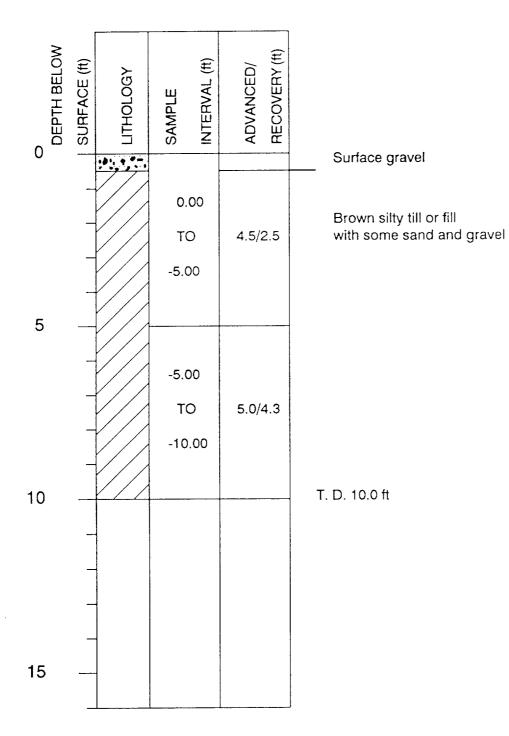
DEPTH BELOW RECOVERY (#) INTERVAL (ft) SURFACE (ft) ADVANCED/ ΠΤΗΟΓΟGΥ SAMPLE 0 Surface gravel 1.5/1.5 0.00 3.5/3.5 то Brown fill or till -5.00 Color change to Gray/grayish brown 5 changing back to brown till with some gravel very compact -5.00 то 5.0/5.0 -10.00 T. D. 10.0 ft 10 15

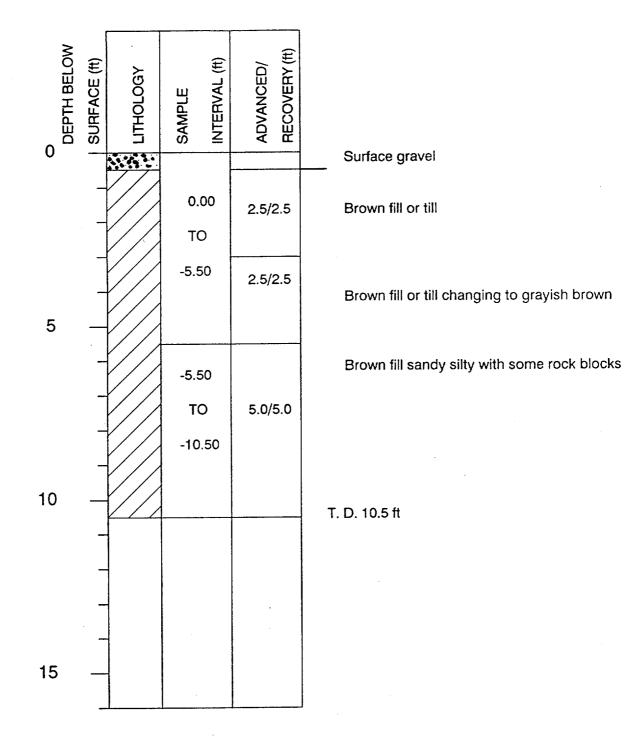


LOCATION: Sample Area 3

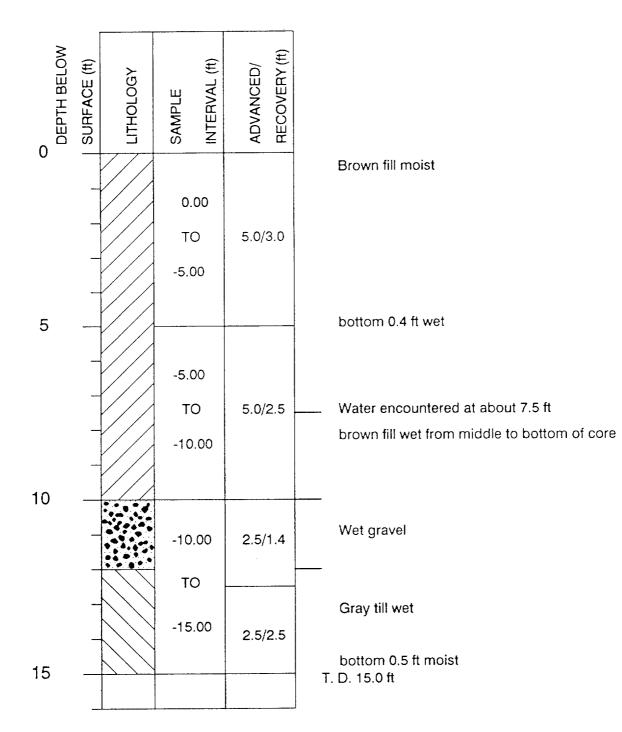


LOCATION: Sample Area 3

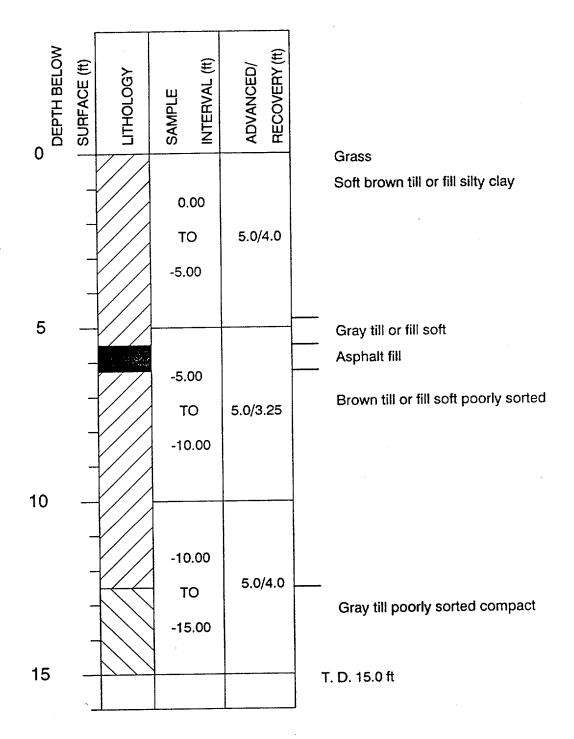




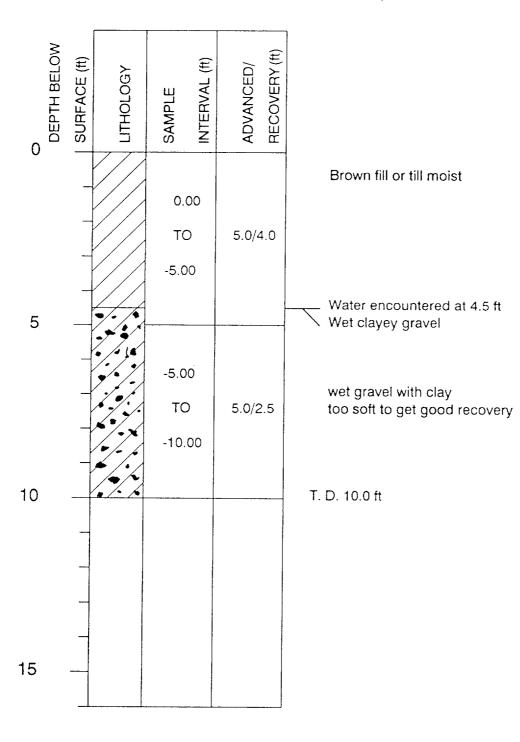
LOCATION: Sample Area 3

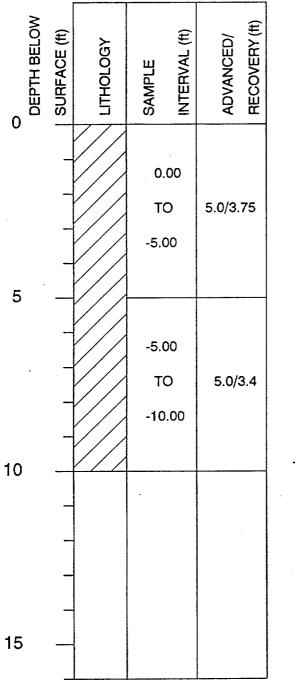


LOCATION: Sample Area 3



LOCATION: Sample Area 3





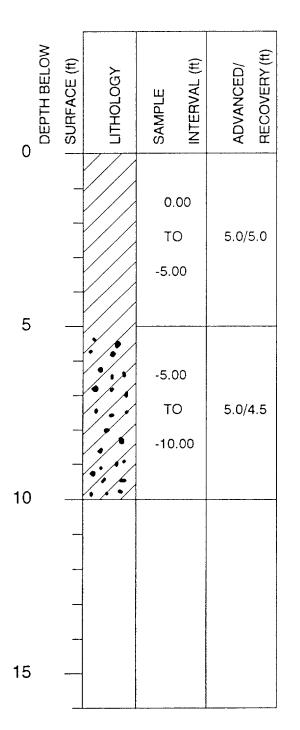
Grass

Brown fill or till silty sandy clay very soft

T. D. 10.0 ft

LOCATION: Sample Area 3

28 JUNE 1989



Grass and gravel Brown fill or till soft silty clay

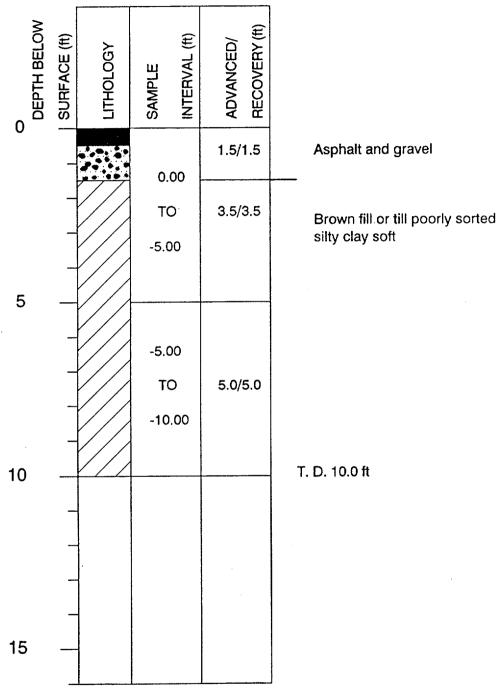
brown fill or till poorly sorted with pebbles and cobbles in a silty clay matrix

becomes very stiff at 9.0 ft

T. D. 10.0 ft

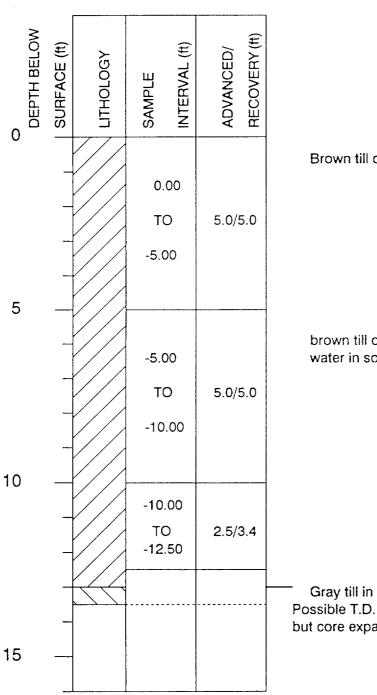
LOCATION: Sample Area 3

28 JUNE 1989



LOCATION: Sample Area 3

29 JUNE 1989



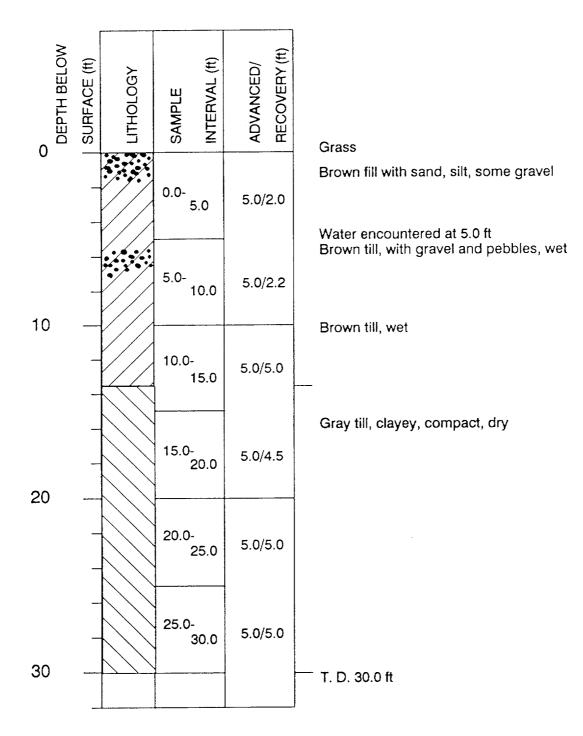
Brown till or fill moist

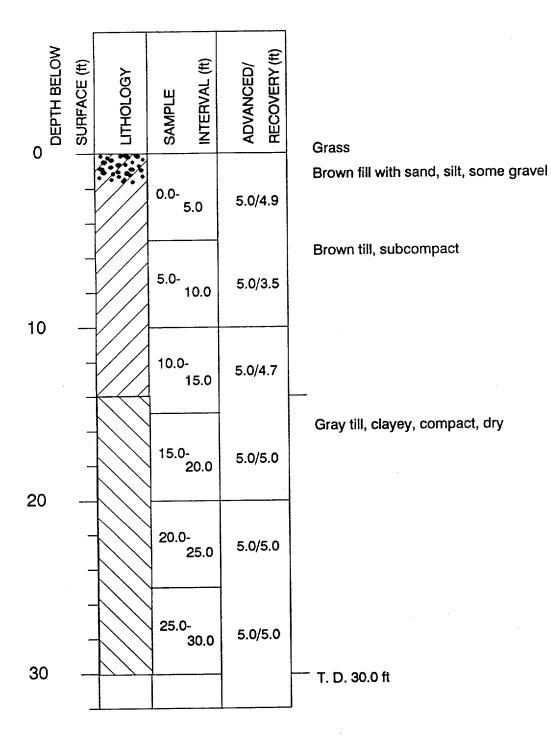
brown till or fill with water in some voids

Gray till in bottom 0.4 ft of barrel Possible T.D. 13.4 ft drilling only to 12.5 ft but core expanded a bit

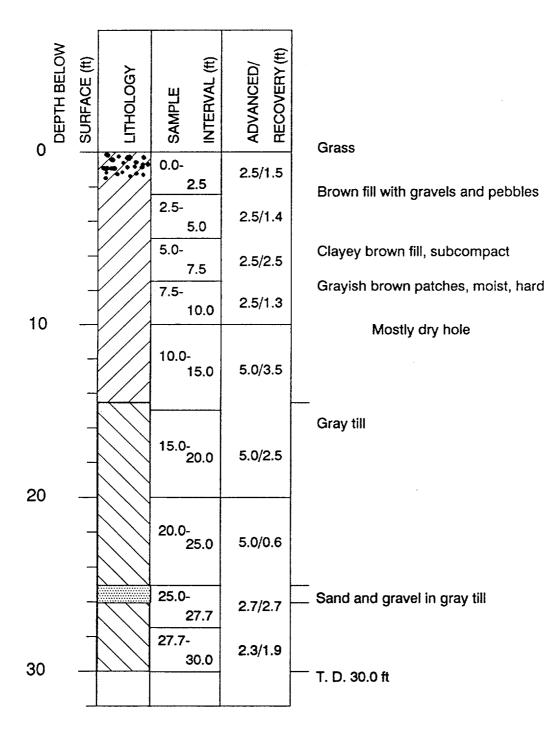
LOCATION: Sample Area 3

DEPTH BELOW RECOVERY (ft) INTERVAL (ft) SURFACE (ft) ADVANCED/ ΓΙΤΗΟLOGY SAMPLE 0 1.5/1.5 Asphalt and gravel 0.00 3.5/3.5 то Brown fill or till poorly sorted silty clay soft -5.00 5 -5.00 4.0/4.0 то brown till or fill with some gray sandy patches -9.00 hit something hard at 9.0 ft T. D. 9.0 ft 10 15

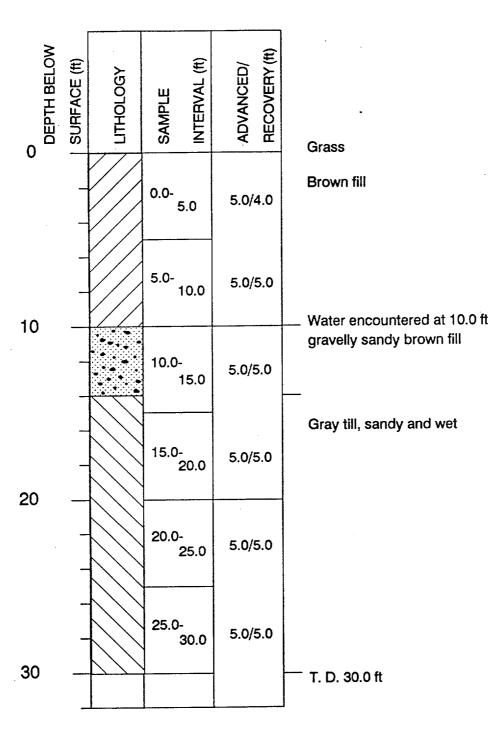


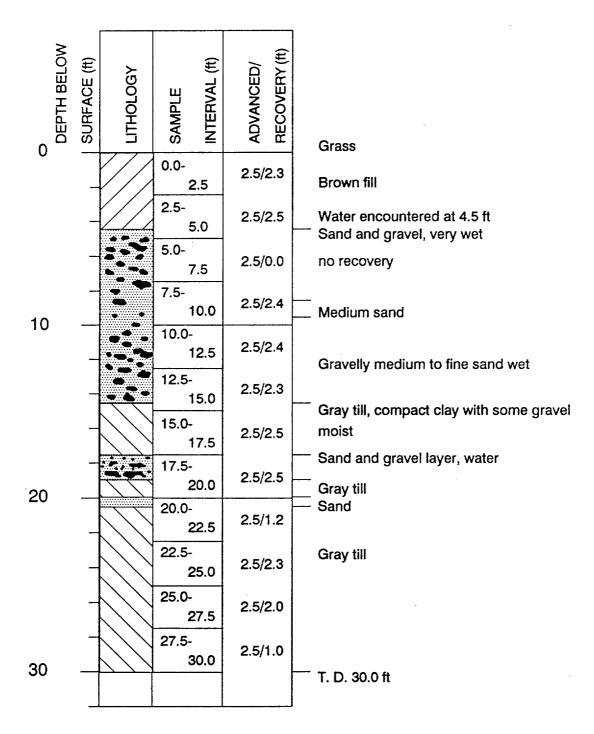


LOCATION: Sample Area 3

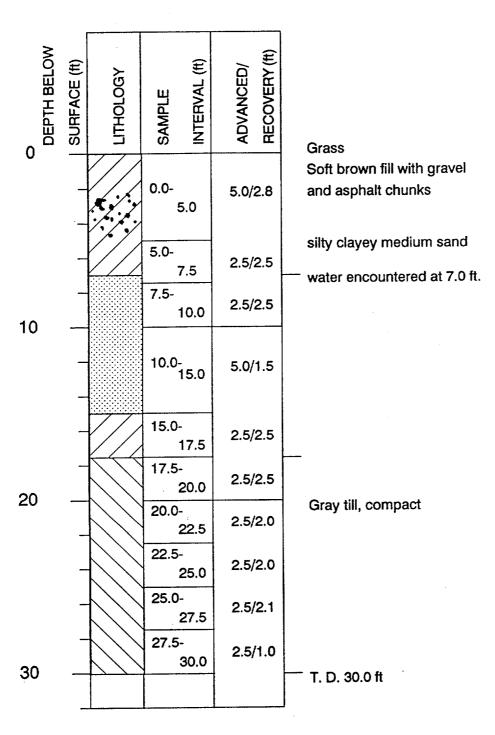


26 JULY 1990

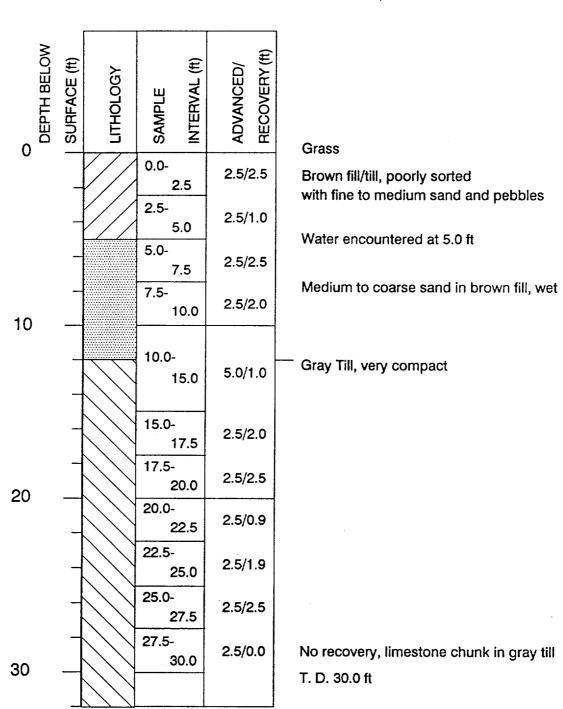




BOREHOLE NUMBER: R319B

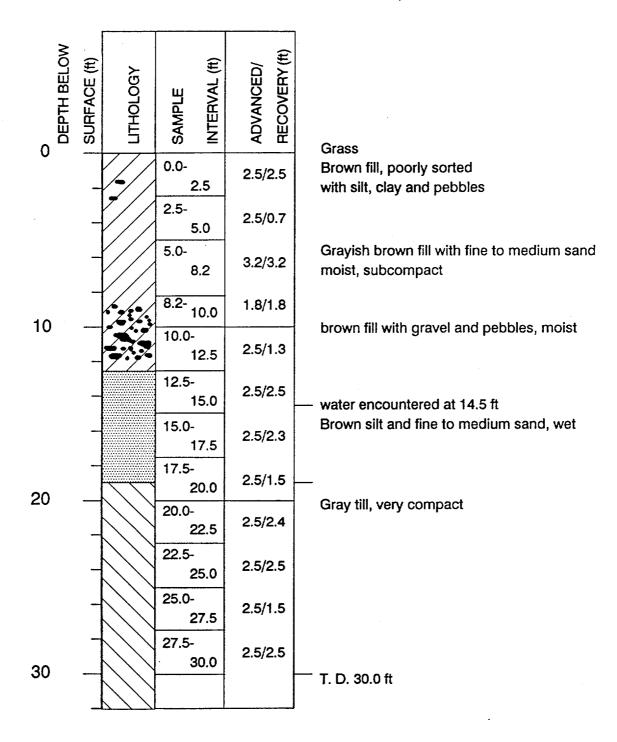


LOCATION: Sample Area 3



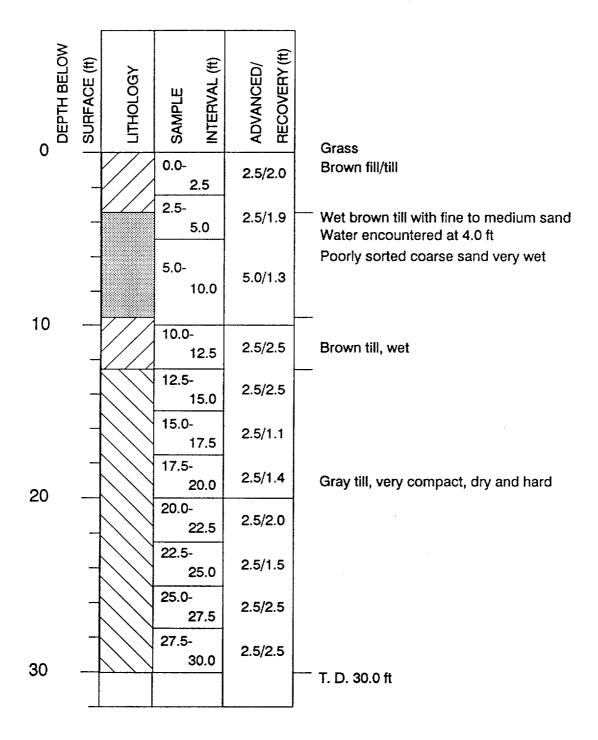
25 JULY 1990

25 JULY 1990

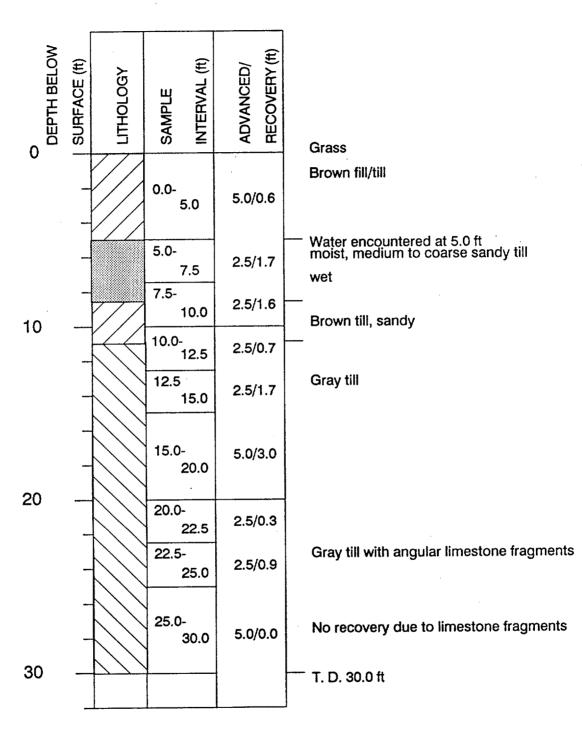


BOREHOLE NUMBER: R322B

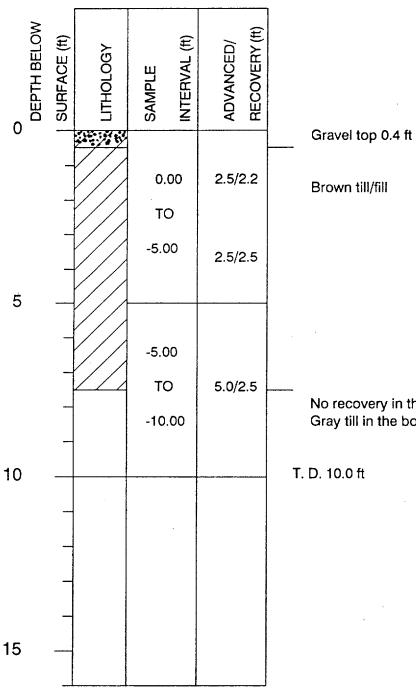
23 JULY 1990



BOREHOLE NUMBER: R323B

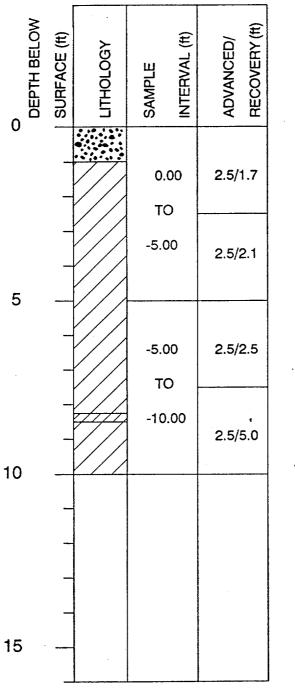


LOCATION: Sample Area 4



No recovery in the bottom 2.5 ft Gray till in the bottom of core device

2 AUGUST 1989



Grass Gravel 1.0 ft

Brown till/fill moist

Brown till/fill

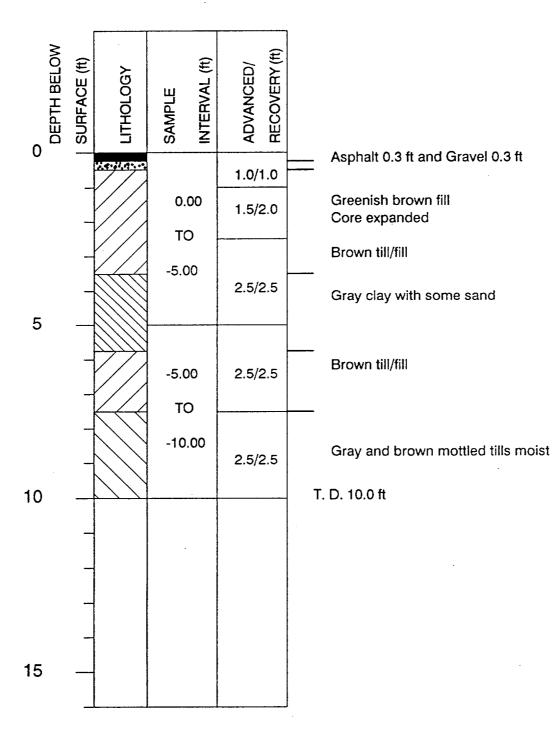
Wet clay and silt rich layer 0.3 ft thick

T. D. 10.0 ft

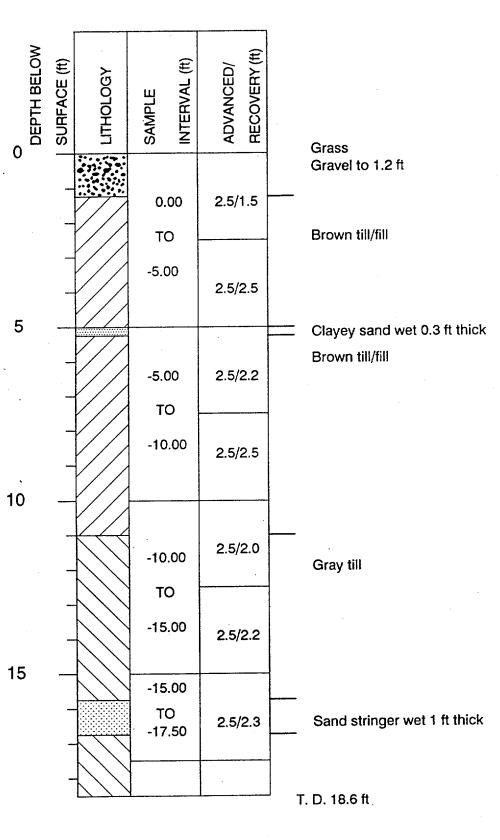
Note: 5.0 ft of core obtained on 2.5 ft of drilling only. No explanation. marked as 7.5 - 8.75 and 8.75 - 10.0.

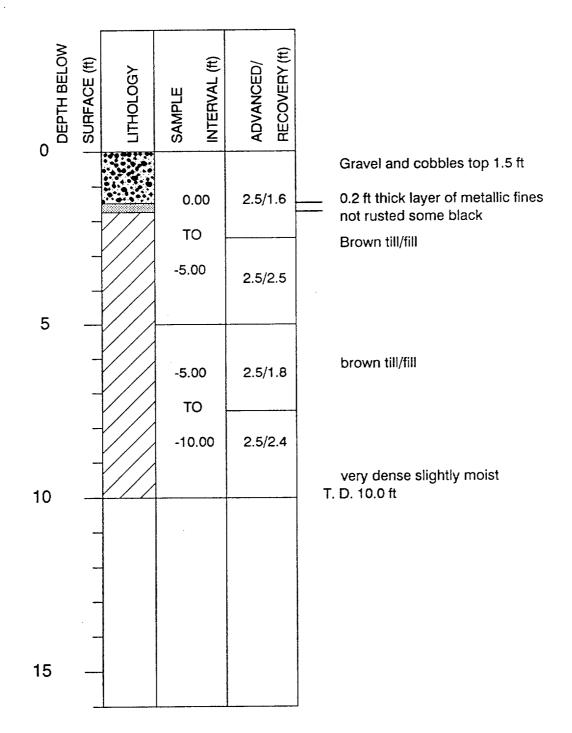
LOCATION: Sample Area 4

2 AUGUST 1989



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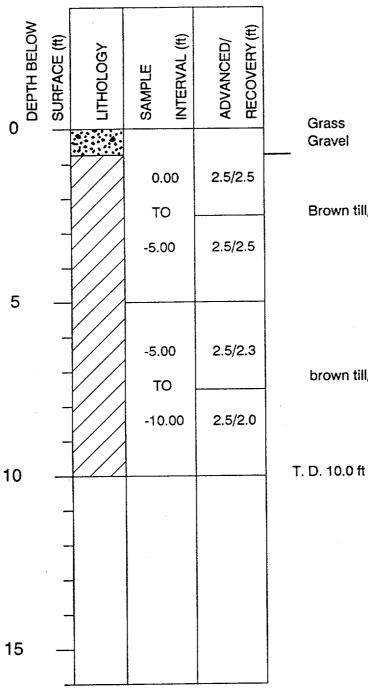




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LOCATION: Sample Area 4

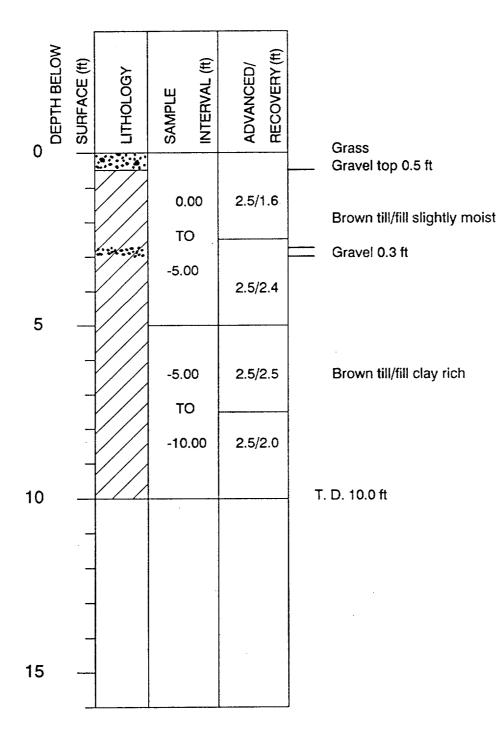
1 AUGUST 1989



Brown till/fill

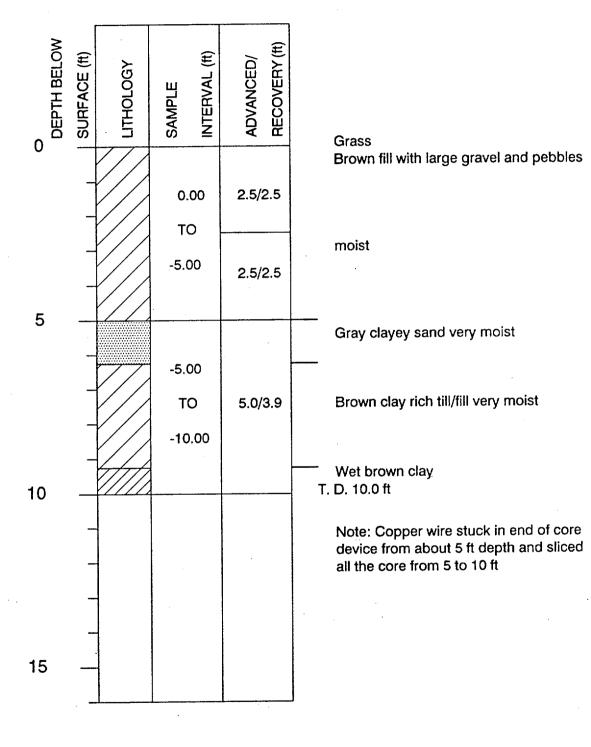
brown till/fill

LOCATION: Sample Area 4

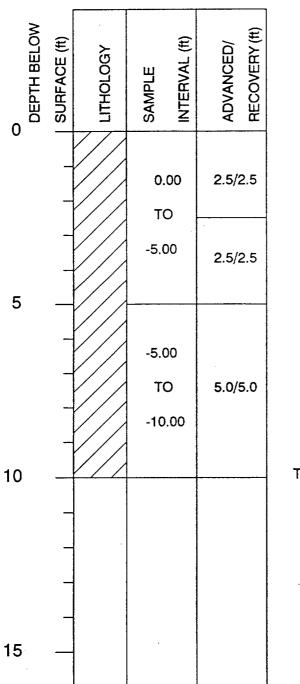


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1 AUGUST 1989



LOCATION: Sample Area 4



Grass

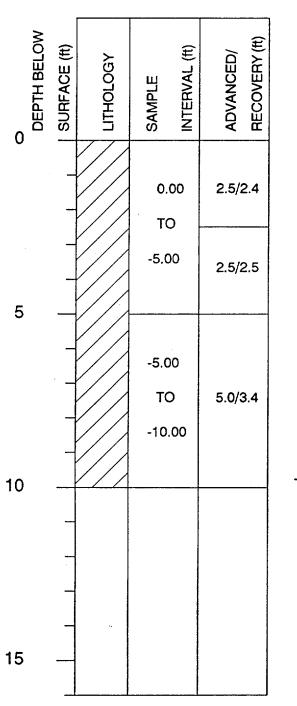
Brown till/fill very gravelly slightly moist

Brown till/fill slightly more moist

### T. D. 10.0 ft

LOCATION: Sample Area 4

1 AUGUST 1989



Grass

# Brown till/fill moist

.

## Brown till/fill

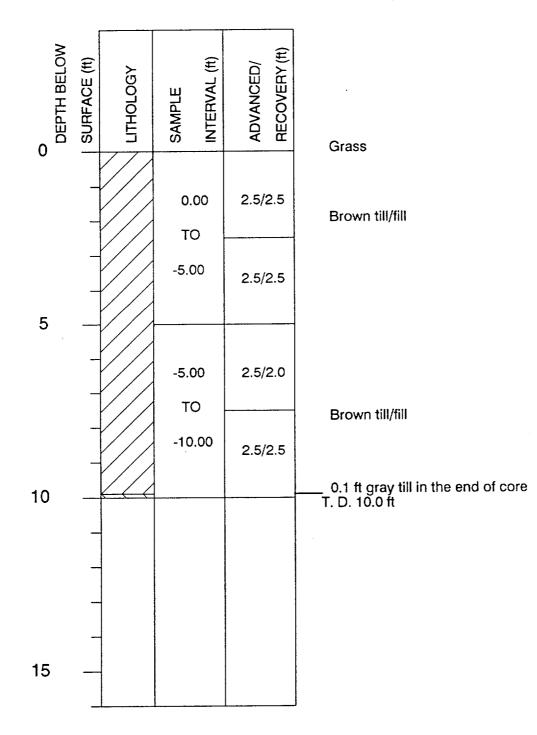
very dense

# T. D. 10.0 ft

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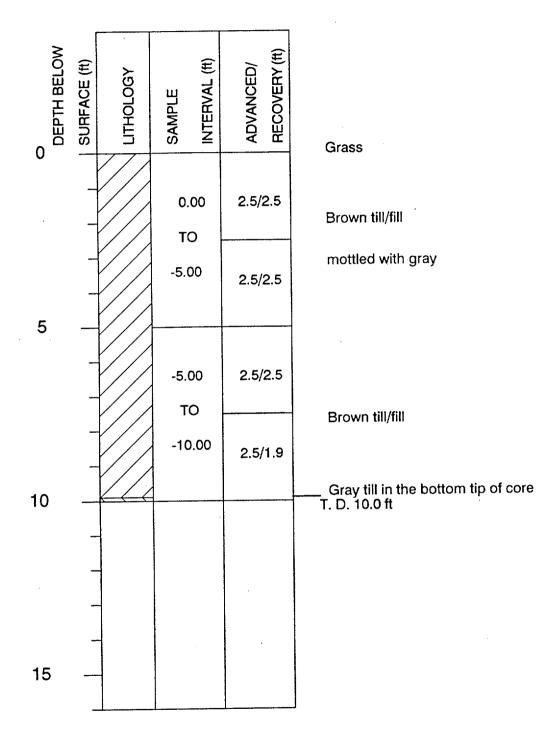
# BOREHOLE NUMBER: 410 LOCATION: Sample Area 4

1 AUGUST 1989



LOCATION: Sample Area 4

1 AUGUST 1989



LOCATION: Sample Area 4

28 JULY 1989

DEPTH BELOW RECOVERY (ft) SURFACE (ft) INTERVAL (#) ADVANCED/ ПТНОСОGY SAMPLE 0 0.00 2.5/2.0 то -5.00 2.5/2.3 5 -5.00 2.5/1.9 то 2.5/1.5 -10.00 10 15

Grass

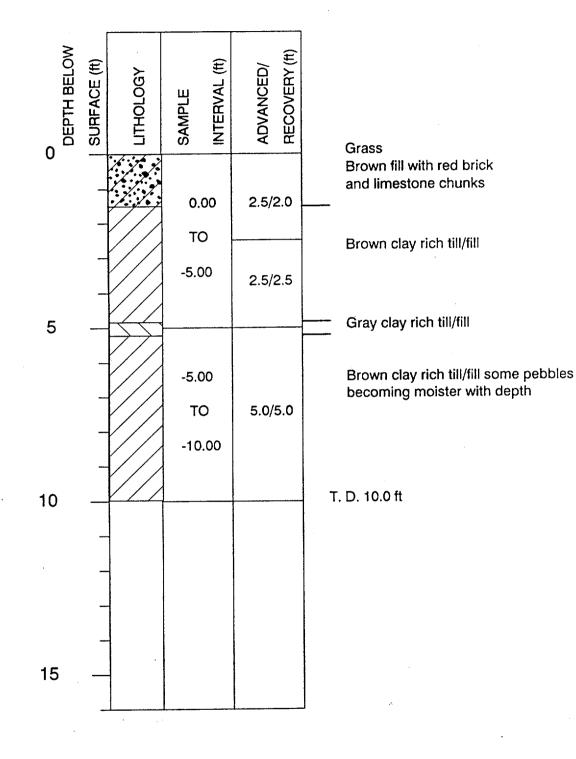
## Brown till/fill weathered

# slightly moist

# brown till/fill with gravel and pebbles

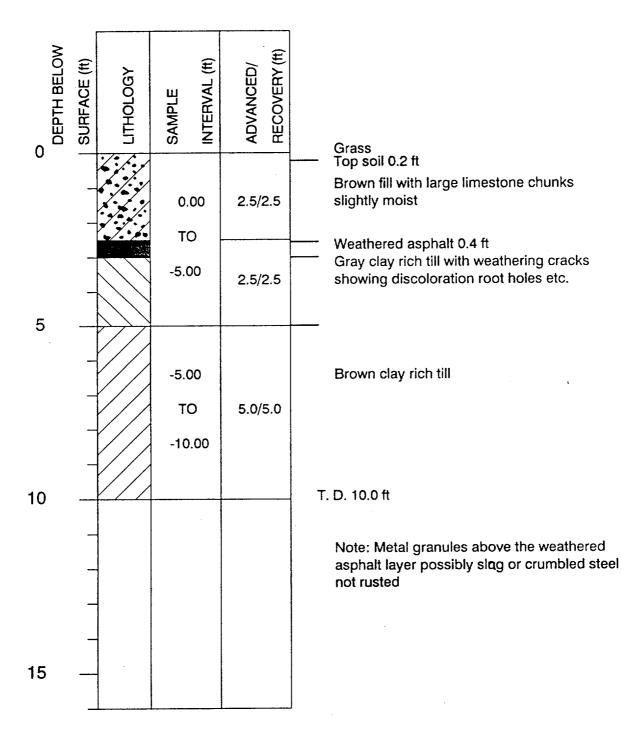
## T. D. 10.0 ft

28 JULY 1989



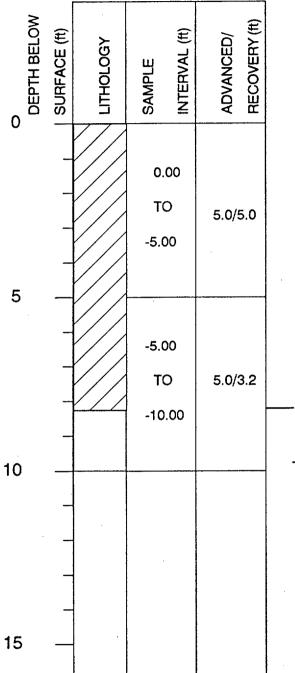
LOCATION: Sample Area 4

28 JULY 1989



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LOCATION: Sample Area 4



Grass

# Brown till/fill barely moist

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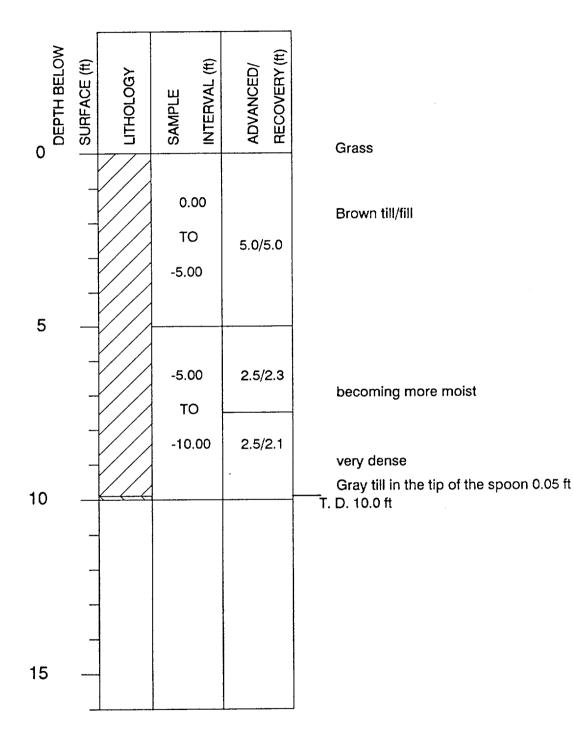
# very dense

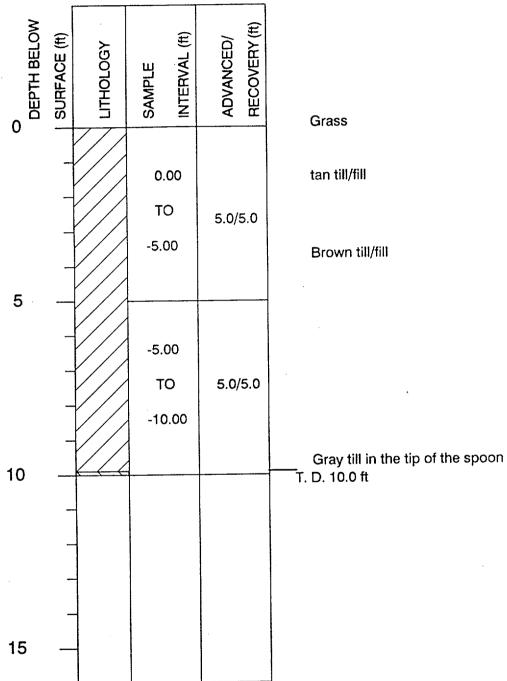
Gray till in the tip of coring device

# T. D. 10.0 ft

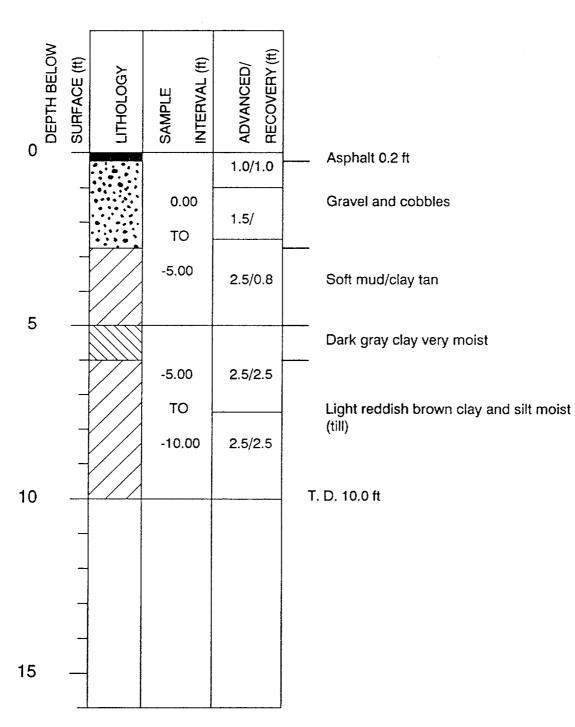
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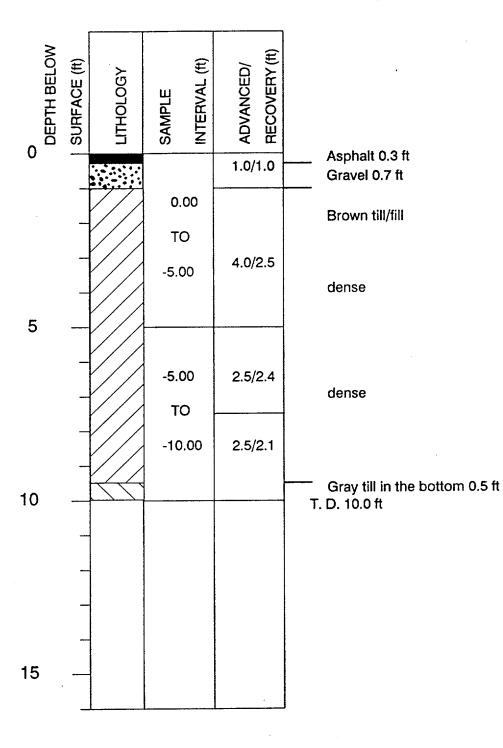




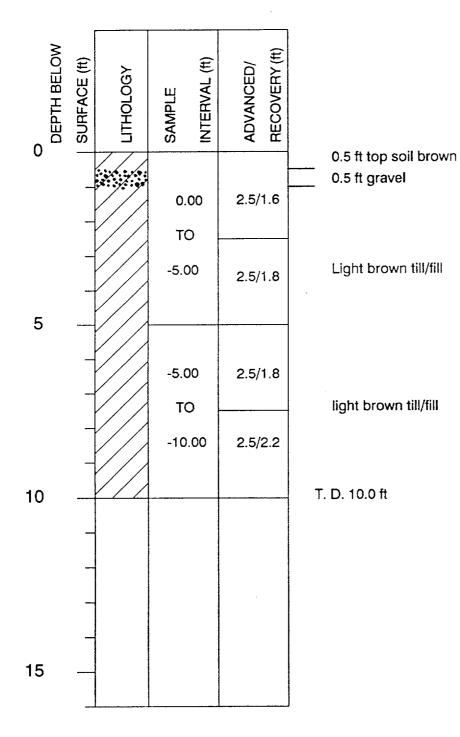
LOCATION: Sample Area 4

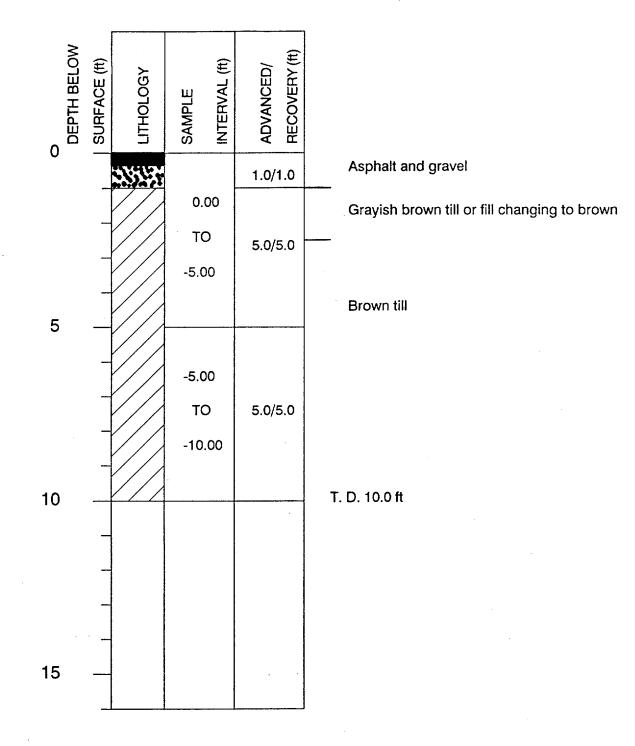


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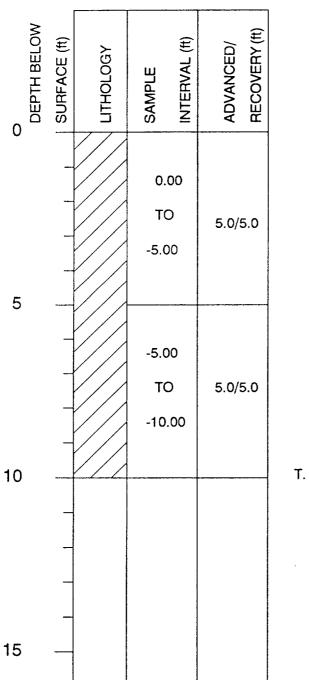
2 AUGUST 1989





LOCATION: Sample Area 5

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#### Grass

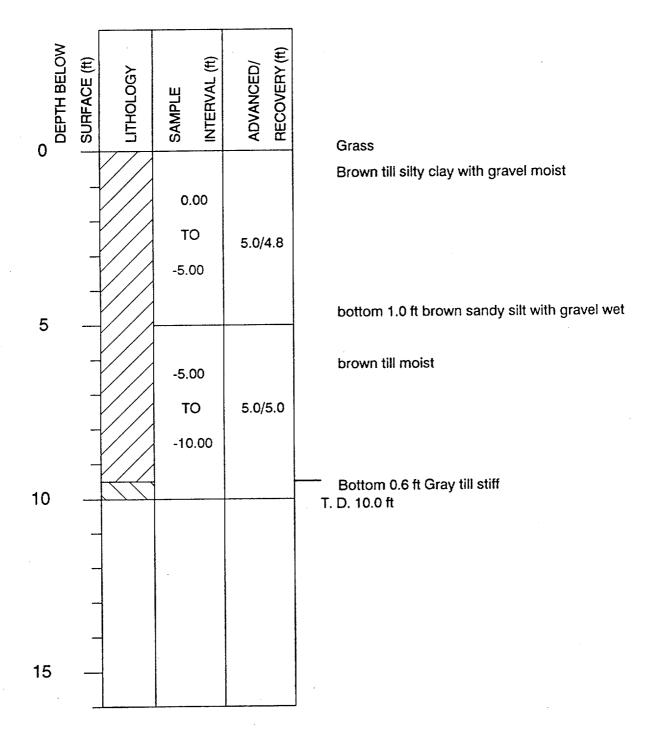
### Brown till silty clay with gravel

# brown till

T. D. 10.0 ft

LOCATION: Sample Area 5





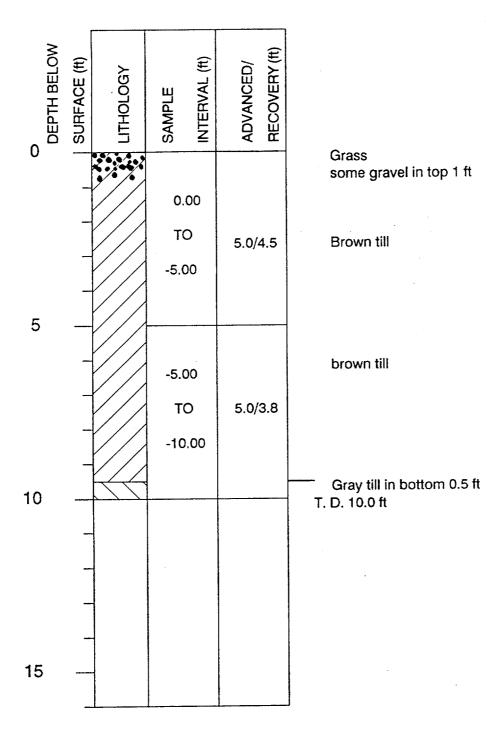
LOCATION: Sample Area 5

24 JULY 1989

DEPTH BELOW RECOVERY (ft) SURFACE (ft) INTERVAL (ft) ADVANCED/ LITHOLOGY SAMPLE 0 Grass Brown fill 2.5/2.5 0.00 то Gravel probably from abandoned drain tiles -5.00 2.5/1.0 poor recovery 5 -5.00 Brown till 5.0/4.3 TO -10.00 bottom 1.0 ft wet 10 T. D. 10.0 ft 15

BOREHOLE NUMBER: 504 LOCATION: Sample Area 5

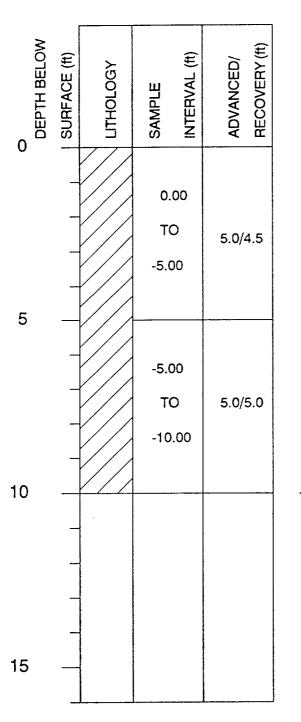
24 JULY 1989



LOCATION: Sample Area 5

24 JULY 1989

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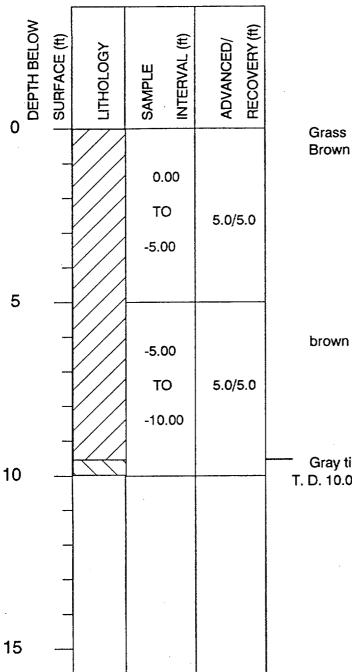
Grass

# Brown till silty clay with some gravel moist some gray patches

brown till wet in top 0.5 ft then moist

sandy silt with gravel at the bottom T. D. 10.0 ft

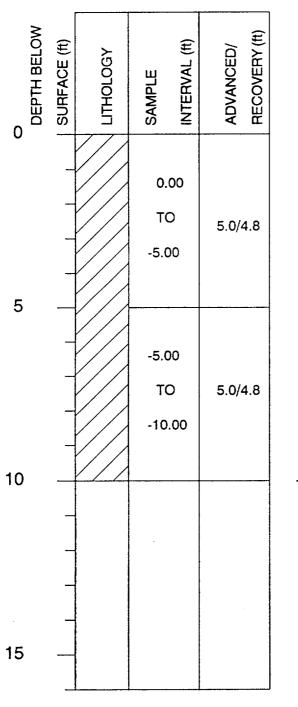
LOCATION: Sample Area 5



Brown till silty clay with gravel moist

brown till

Gray till bottom 0.4 ft silty clay with gravel T. D. 10.0 ft

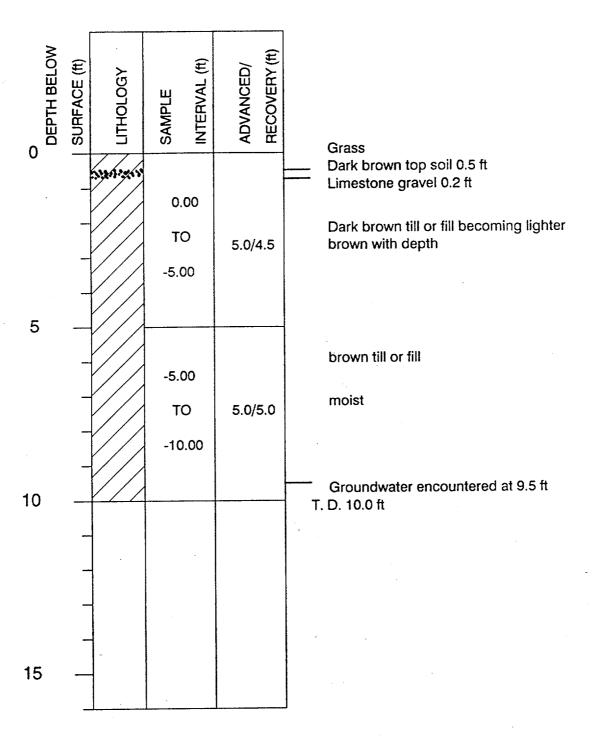


#### Grass Brown till soft clayey silt

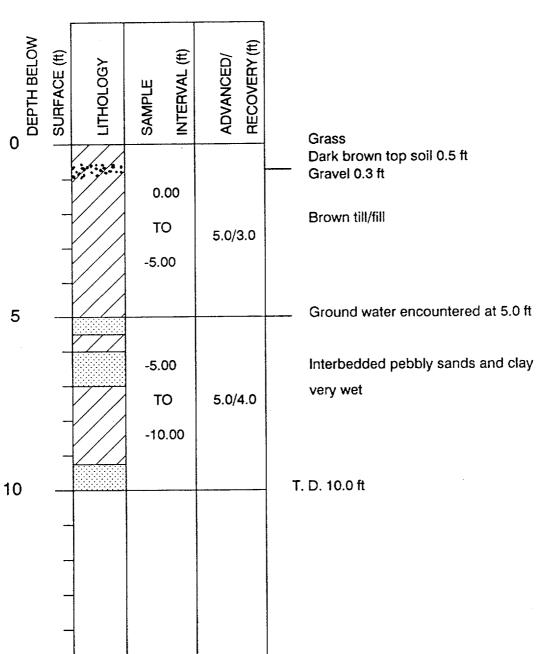
brown till stiff moist

becoming grayish brown at the bottom

#### T. D. 10.0 ft



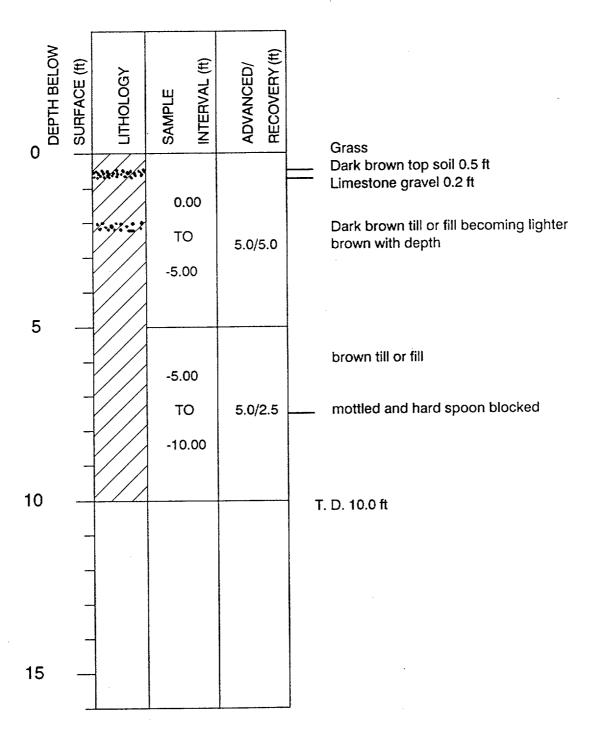
LOCATION: Sample Area 6



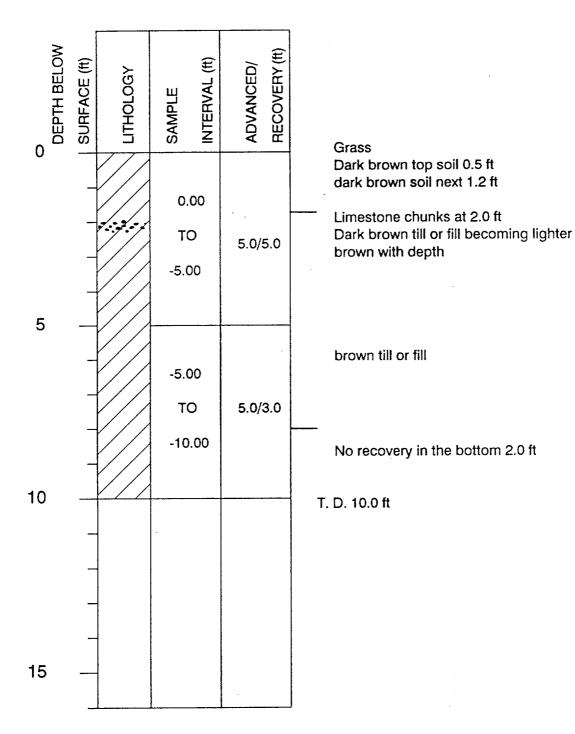
15

26 JULY 1989

LOCATION: Sample Area 6

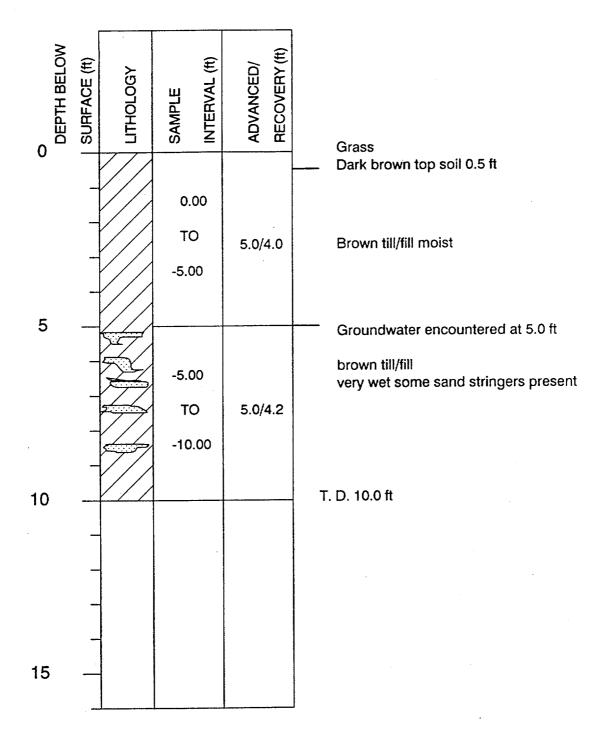


LOCATION: Sample Area 6



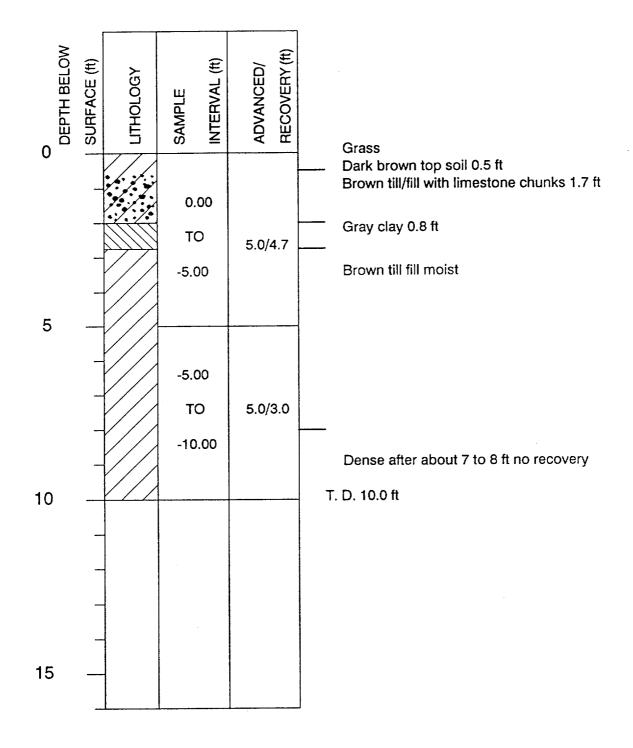
LOCATION: Sample Area 6

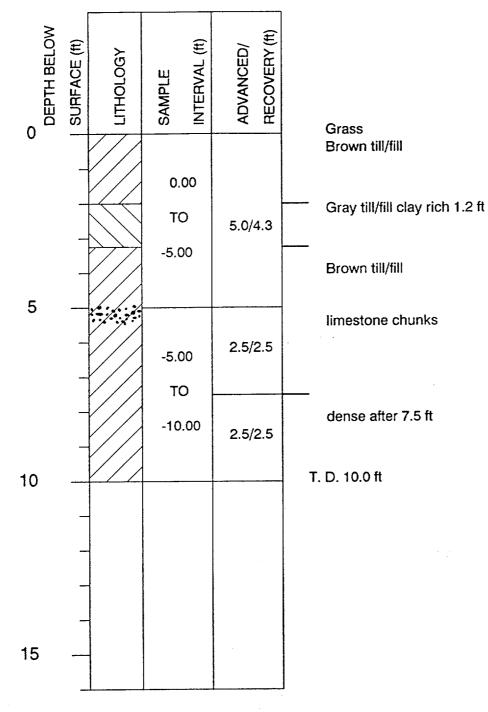
26 JULY 1989

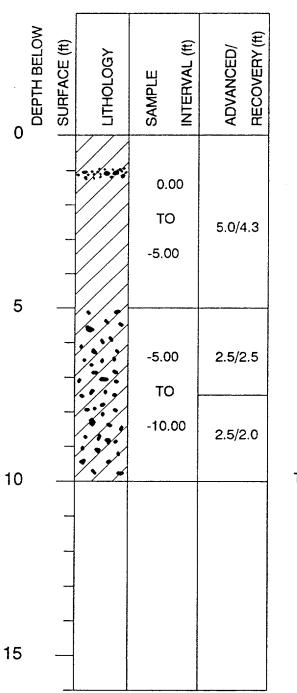


LOCATION: Sample Area 6

26 JULY 1989







Grass

Gravel zone 0.2 ft thick

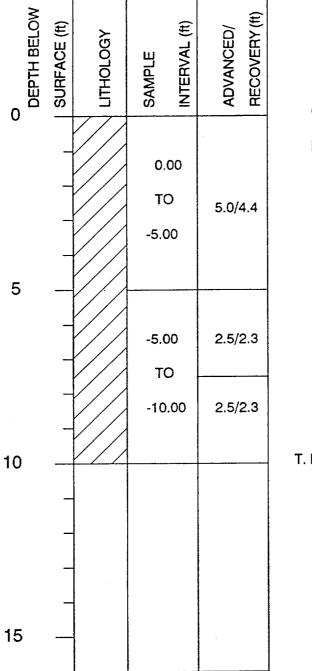
#### Brown till/fill moist

limestone chunks 5.0 to 10.0 ft

#### T. D. 10.0 ft

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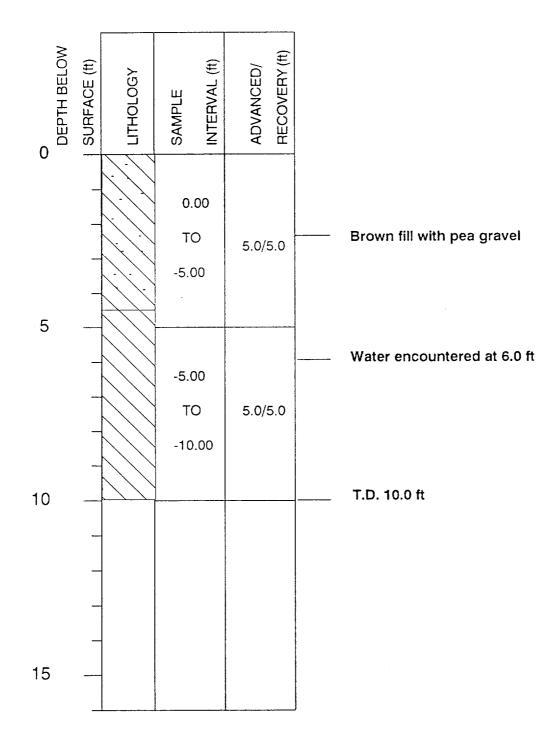
Grass

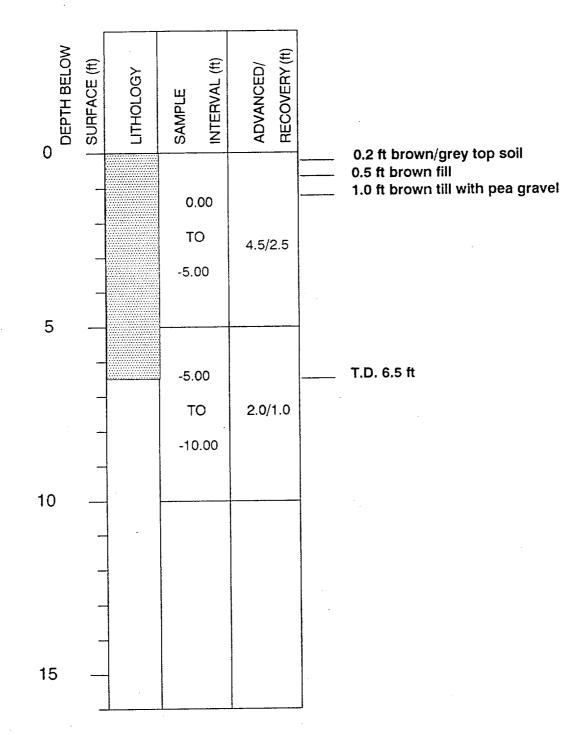
#### Brown till/fill

moist and denser

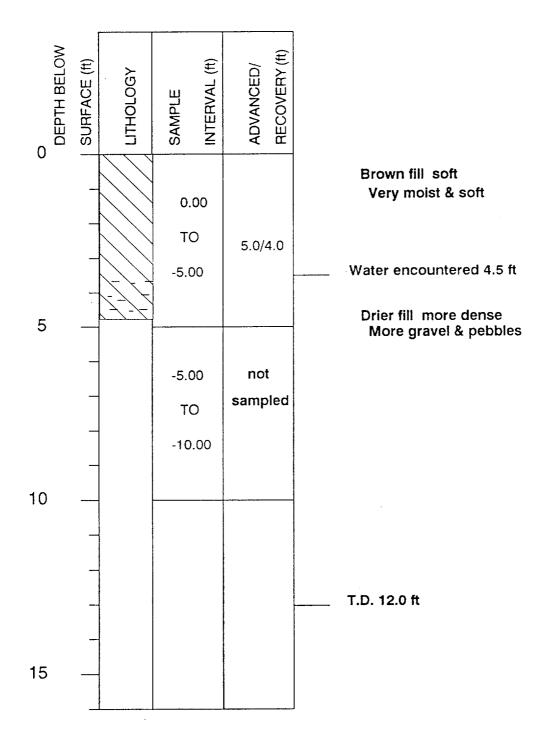
#### brown till/fill

# T. D. 10.0 ft



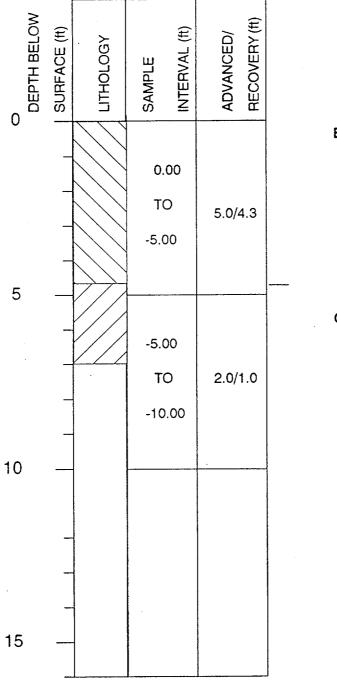


17 November 1989



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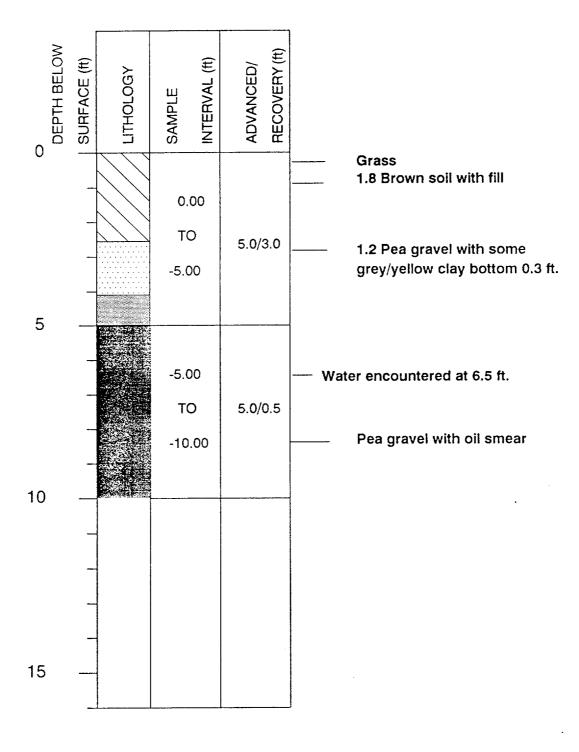
#### 14 November 1989



Brown fill

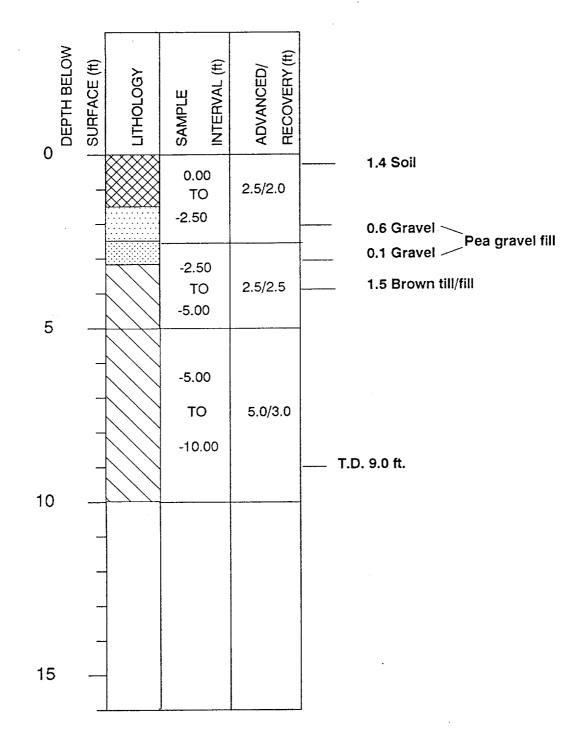
# Grey fill/till moist & soft Not enough volume

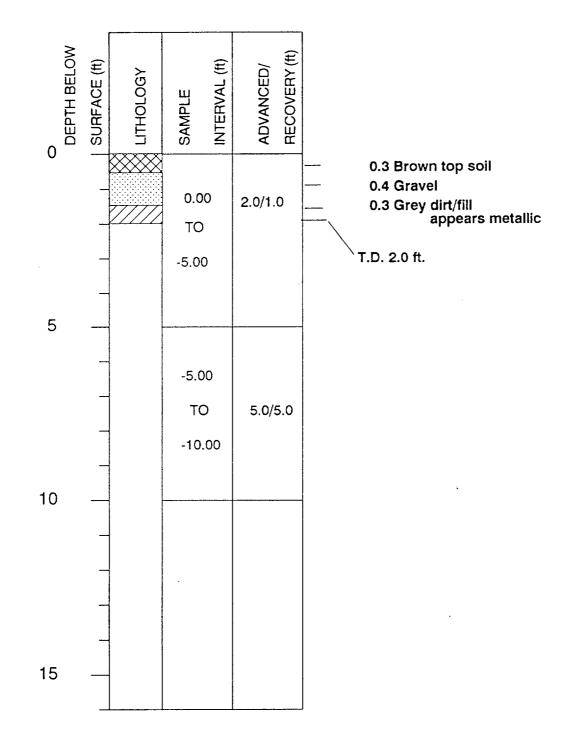
#### 14 November 1989

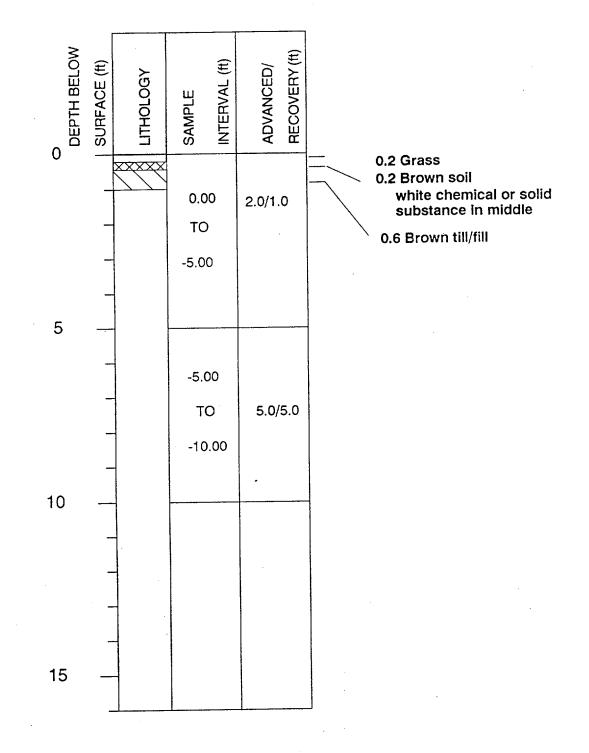


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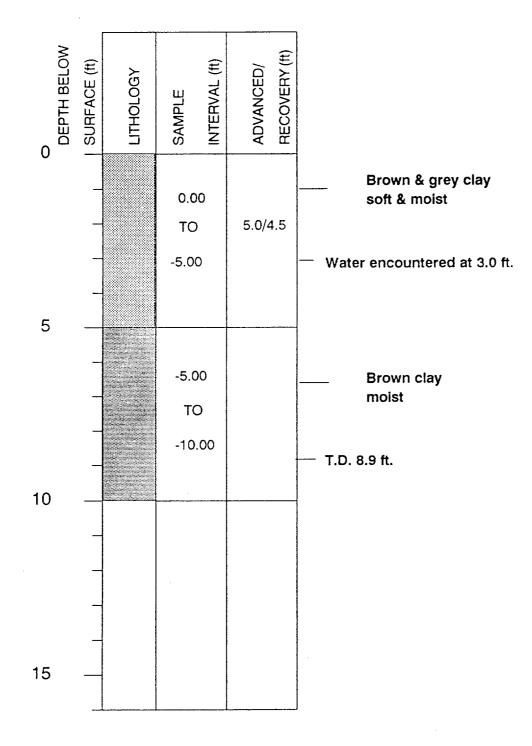
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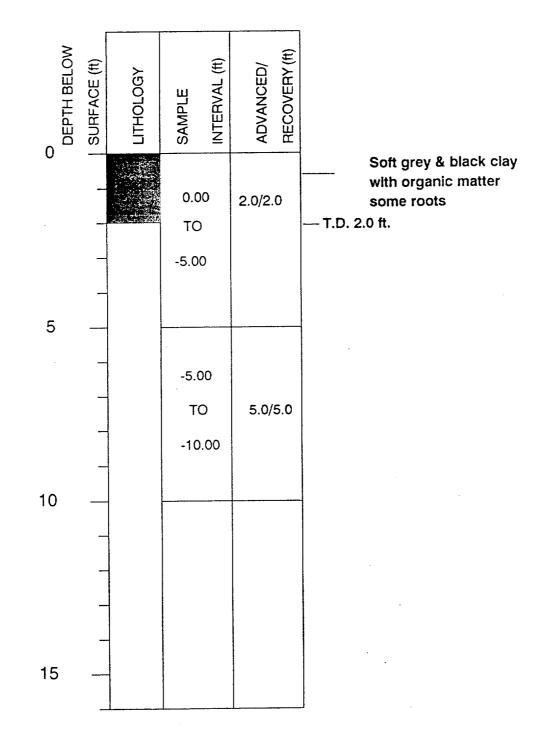




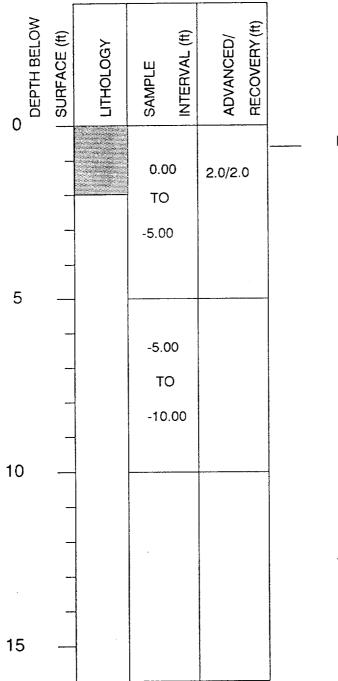


17 November 1989

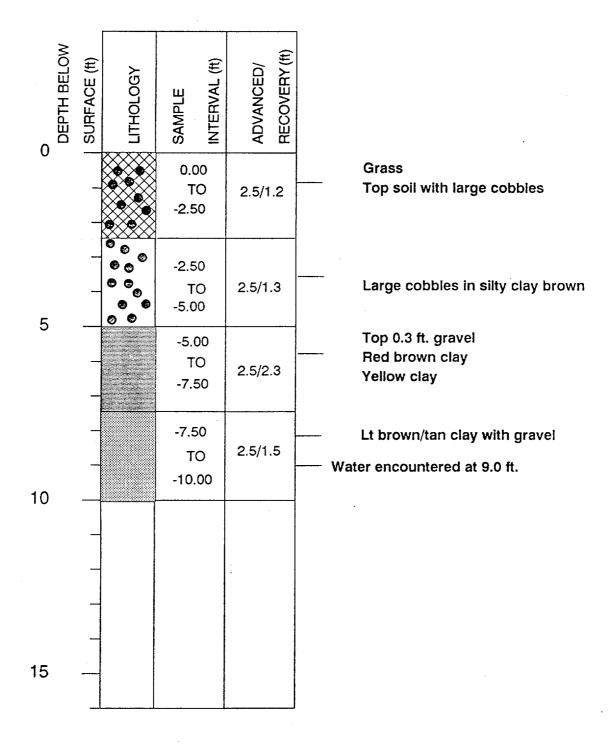




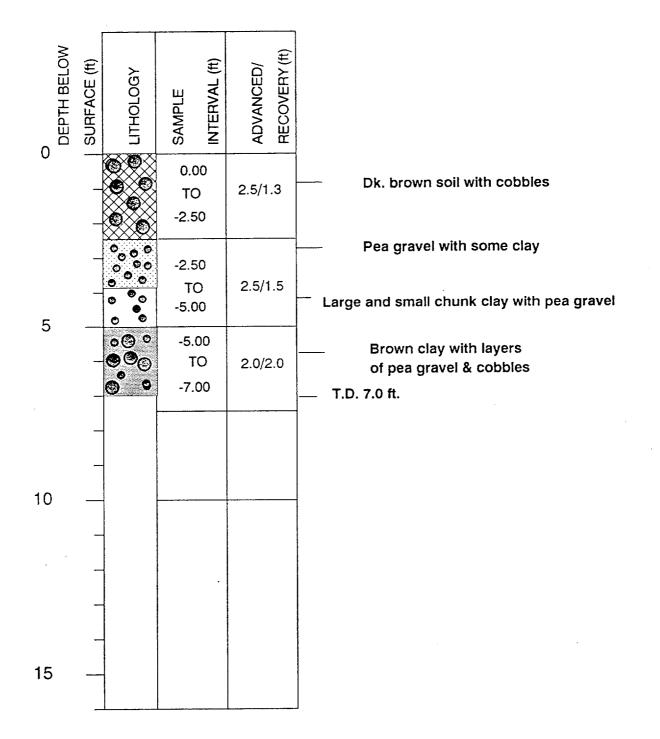
15 November 1989



Brown clay

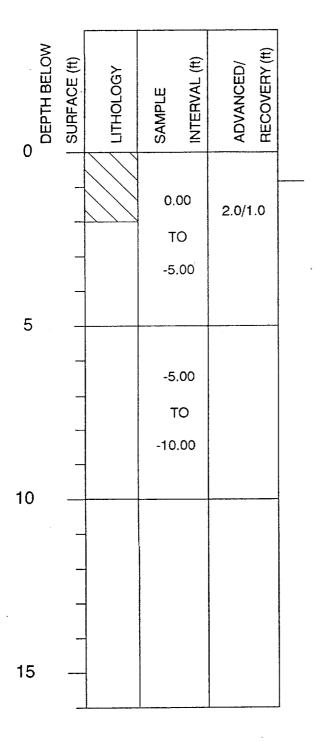


#### 15 November 1989



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#### 15 November 1989

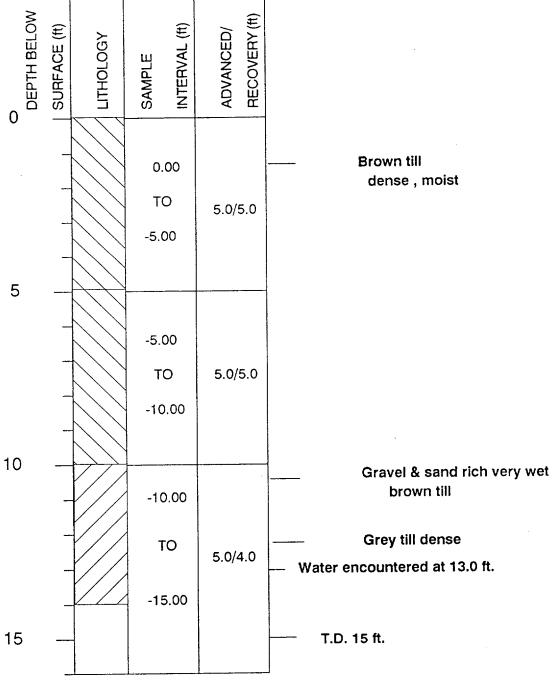


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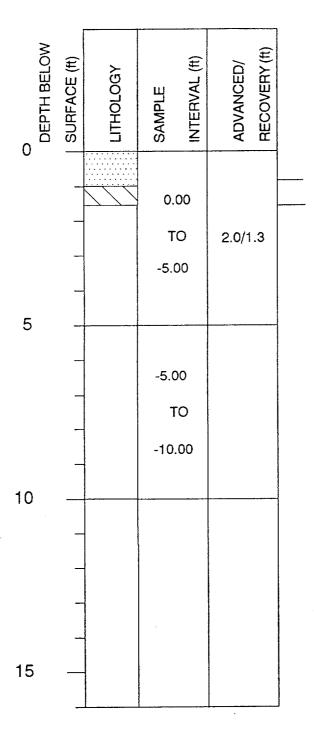
Brown till/fill

#### 15 November 1989



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27 November 1989



1.0 Sand & gravel fill loose 0.3 Brown till dense

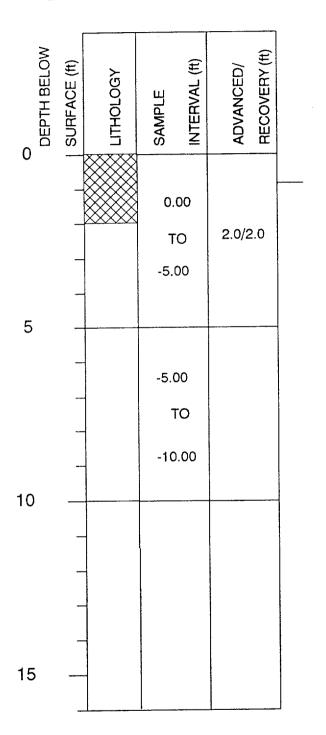
# SUBFLOOR

# 15 November 1989

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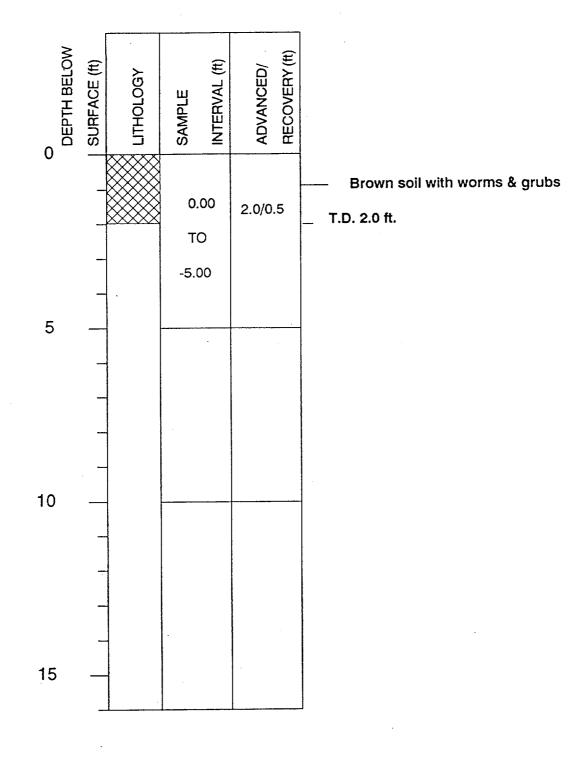
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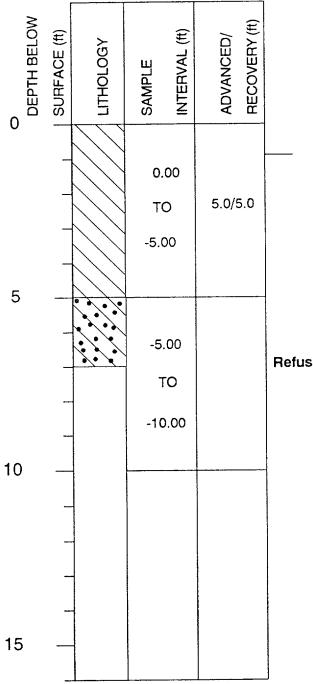
Brown soil

15 November 1989



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#### 15 November 1989

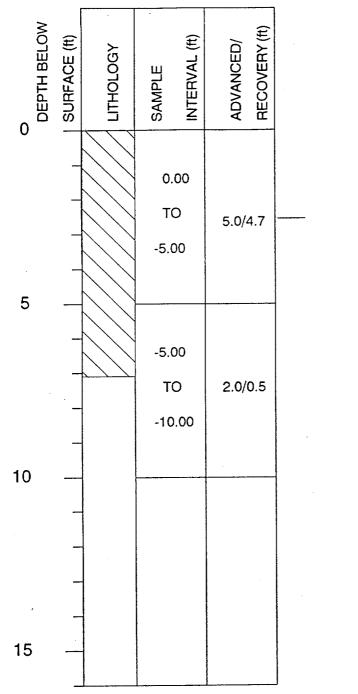


Grass

#### **Brown till**

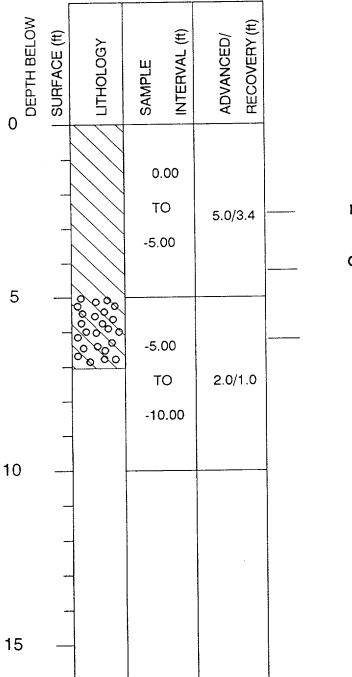
Brown till with gravel & pebbles Refusal at 7.0 ft.

#### 15 November 1989



**Brown till** 

#### 15 November 1989



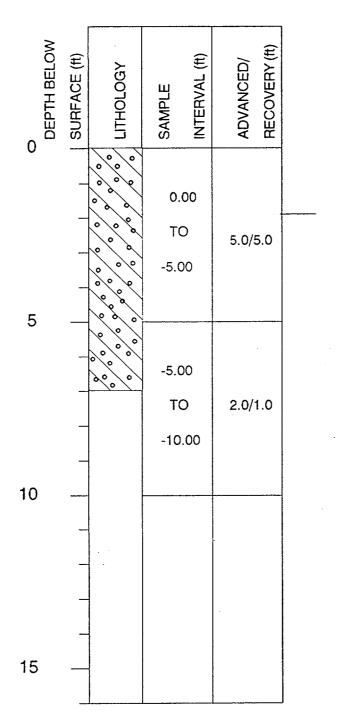
Brown till fill

Gravelly in bottom 0.5 ft.

Brown till with gravel

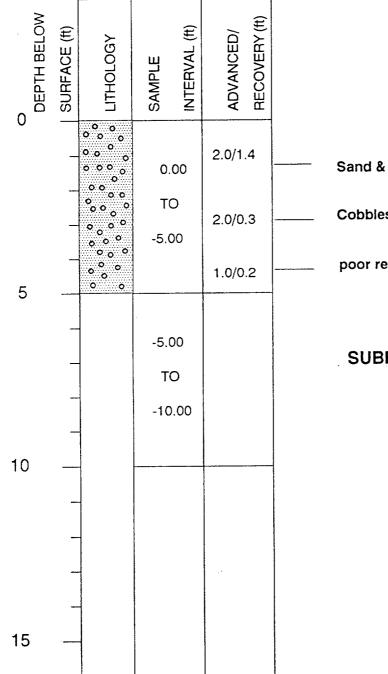
**L** 

#### 15 November 1989



Brown till with gravel

#### 27 November 1989



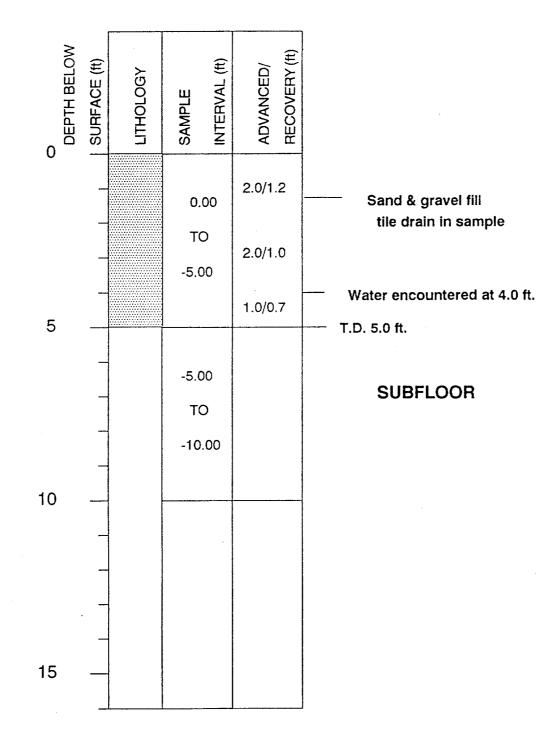
Sand & gravel with cobbles

Cobbles

poor recovery

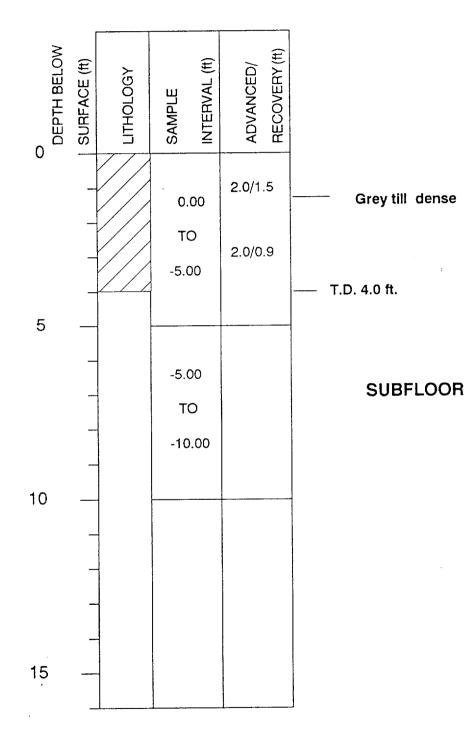
## SUBFLOOR

#### 27 November 1989

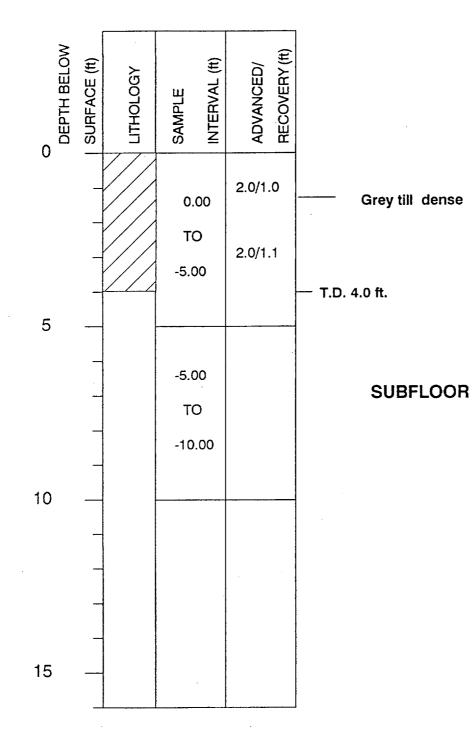


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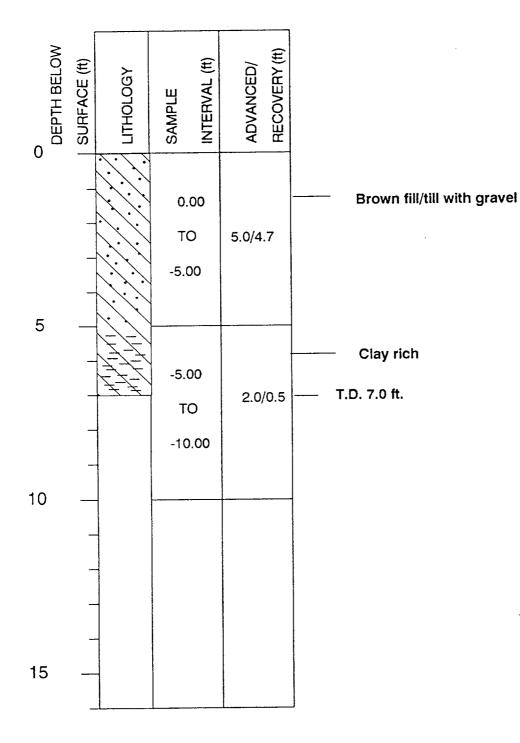
27 November 1989



#### 27 November 1989



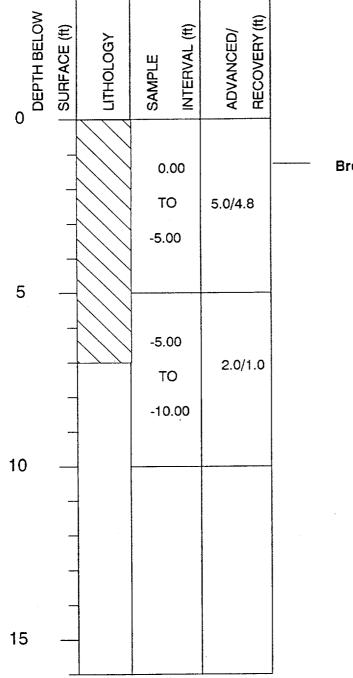
#### 16 November 1989



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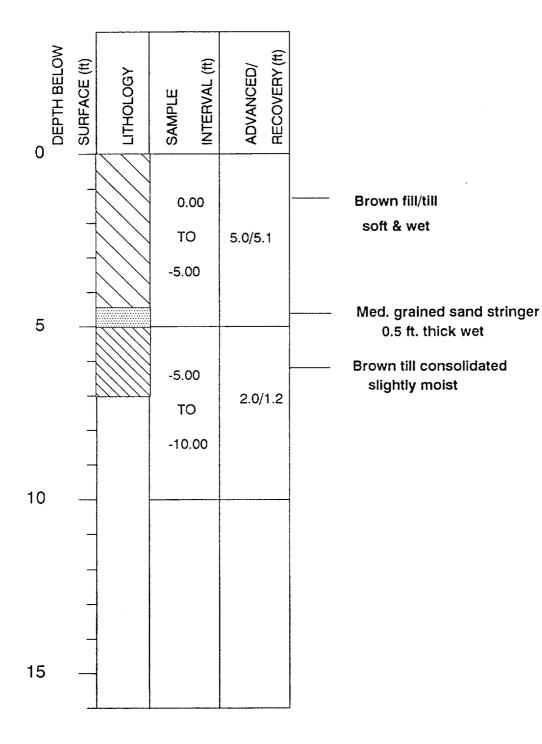
1

#### 16 November 1989



Brown fill/till

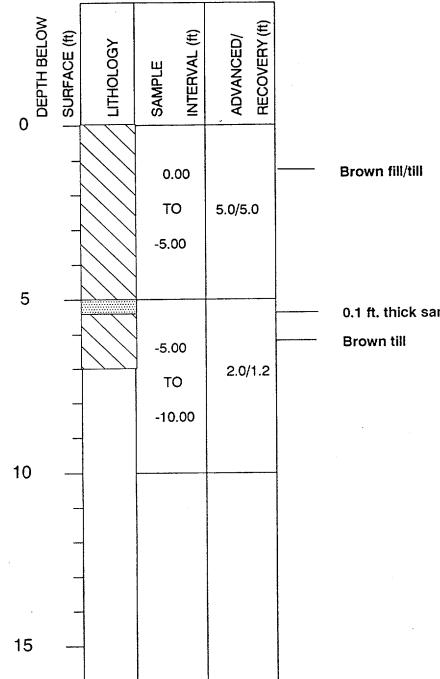
16 November 1989



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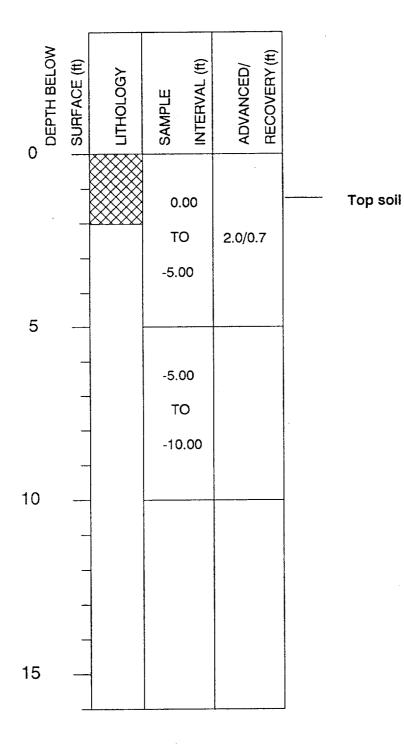
5

#### 16 November 1989



0.1 ft. thick sand stringer

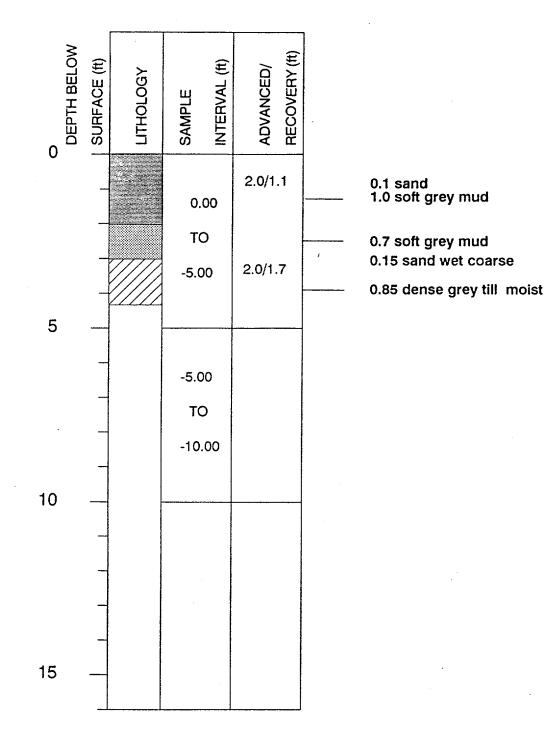
16 November 1989



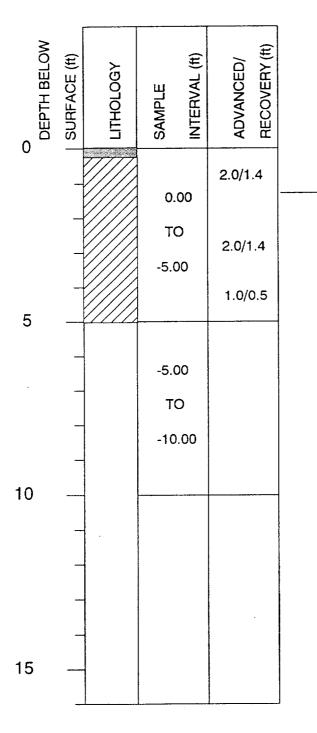
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#### 28 November 1989



27 November 1989



0.1 sand Grey till very dense

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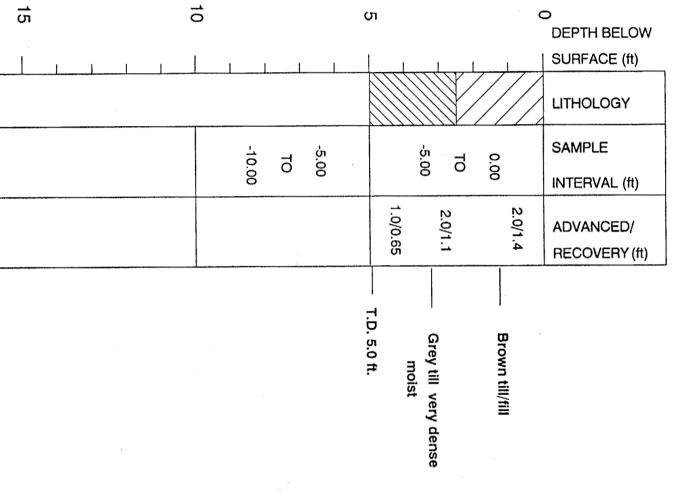
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**BOREHOLE NUMBER: R-3** 

27 November 1989

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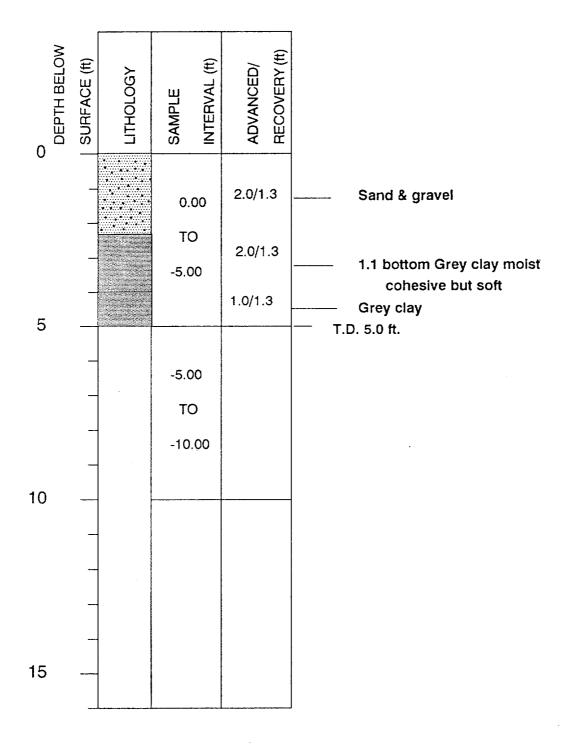
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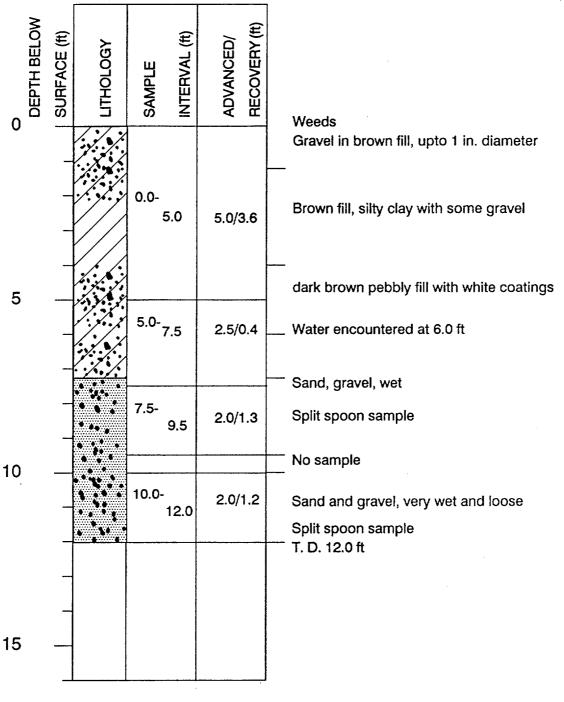
27 November 1989



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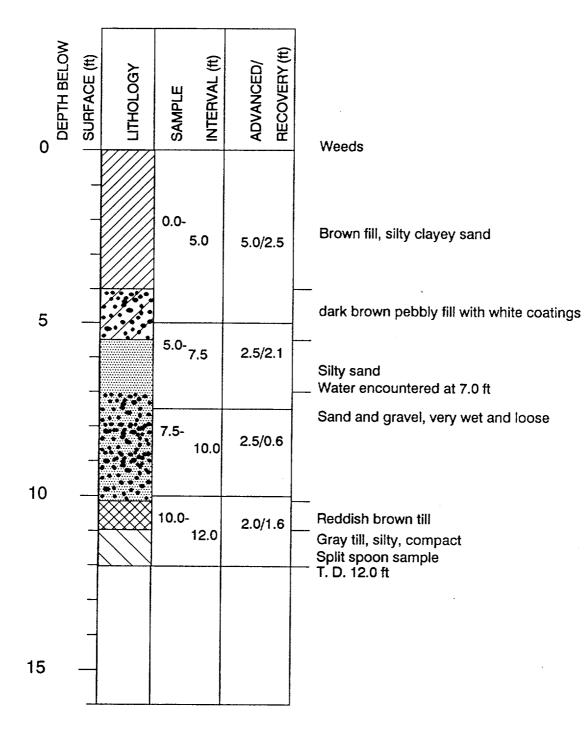
LOCATION: Filter Beds

31 JULY 1990

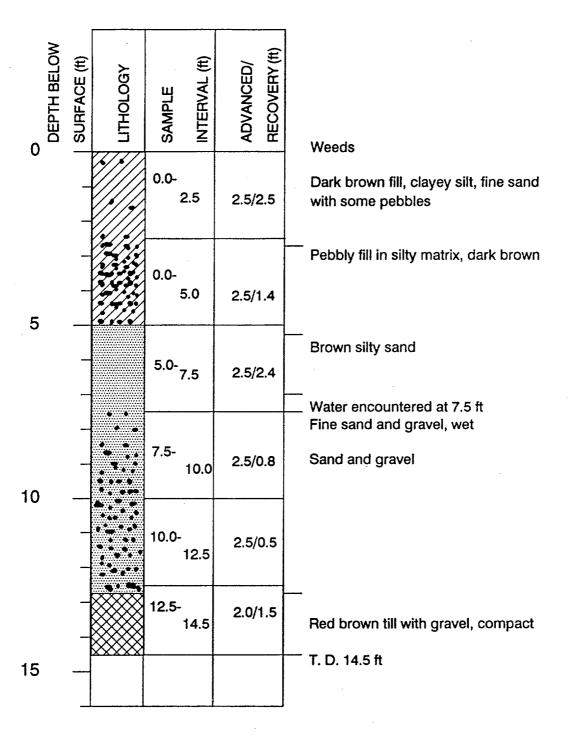


LOCATION: Filter Beds

31 JULY 1990



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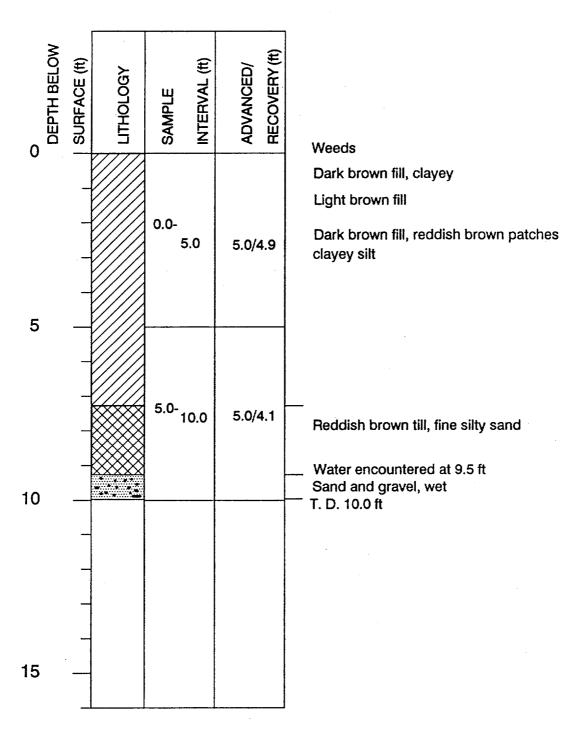
LOCATION: Filter Beds

30 JULY 1990

DEPTH BELOW RECOVERY (#) SURFACE (ft) INTERVAL (ft) ADVANCED/ LITHOLOGY SAMPLE 0 Weeds Brown fill, silt and gravel 0.0-5.0 5.0/4.1 Dark brown clayey silt 5 Pebbles in silty matrix Brown fill, clayey silt <sup>5.0-</sup>10.0 5.0/1.5 Water encountered at 9.0 ft 10 Sand and gravel, very wet 10.0-2.5/0.7 12.5 T. D. 12.5 ft 15

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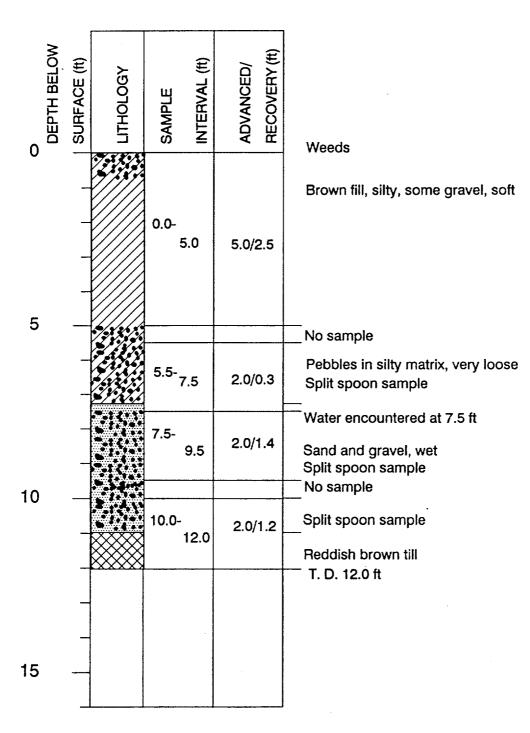


#### BOREHOLE NUMBER: R140B

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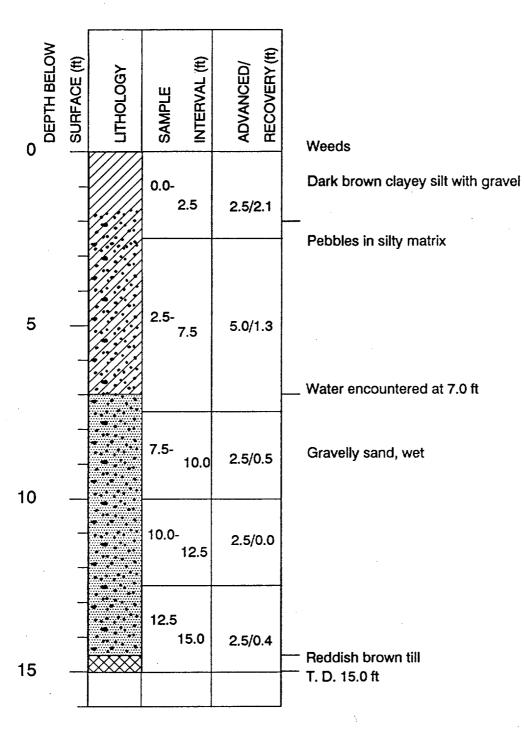
LOCATION: Filter Beds

31 JULY 1990



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30 JULY 1990



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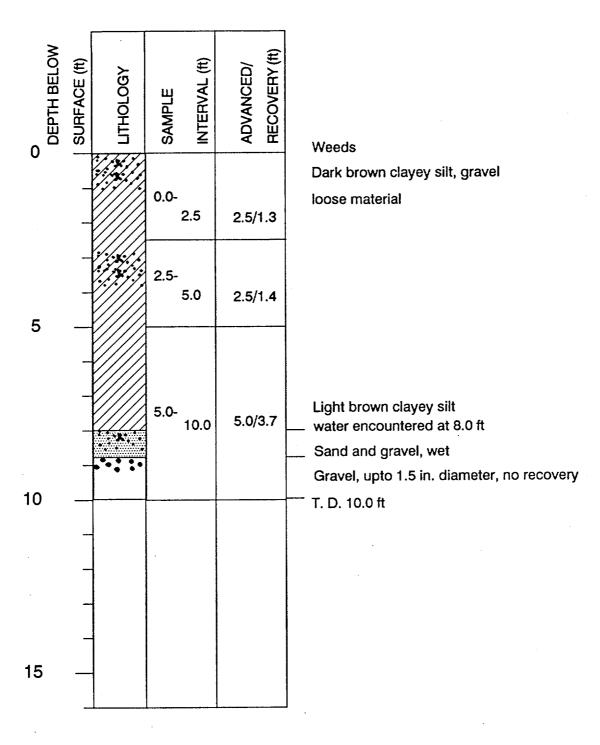
LOCATION: Filter Beds

30 JULY 1990

DEPTH BELOW RECOVERY (ft) ŧ SURFACE (ft) ADVANCED/ **LITHOLOGY** INTERVAL SAMPLE Weeds 0 Dark brown silty fill, gravel 0.0becoming compact and moist with depth 2.5 2.5/1.8 2.5-5.0 2.5/1.5 5 5.0-<sub>7.5</sub> Light brown silty sand 2.5/2.5 Water encountered at 7.0 ft Very coarse sand and gravel, wet 7.5-2.5/1.9 10.0 Compact reddish brown till with gravel 10 T. D. 10.0 ft 15

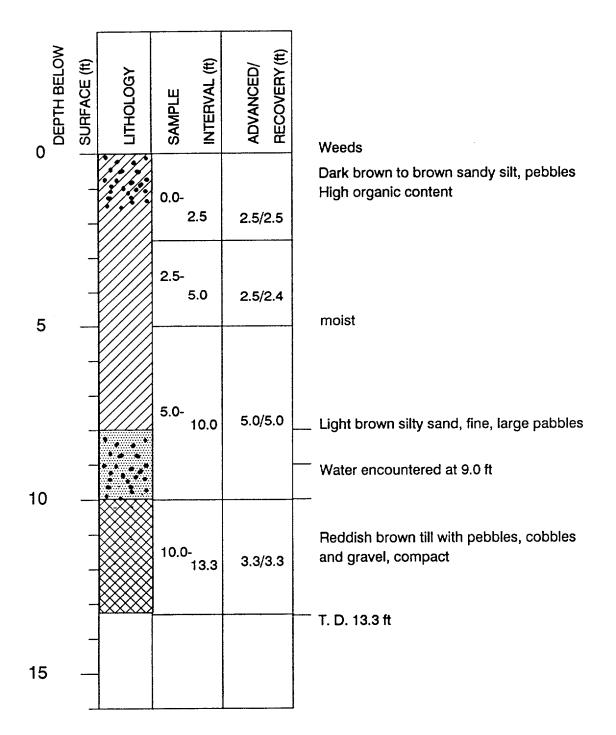
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LOCATION: Filter Beds



LOCATION: Filter Beds

30 JULY 1990



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## APPENDIX 2

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1.

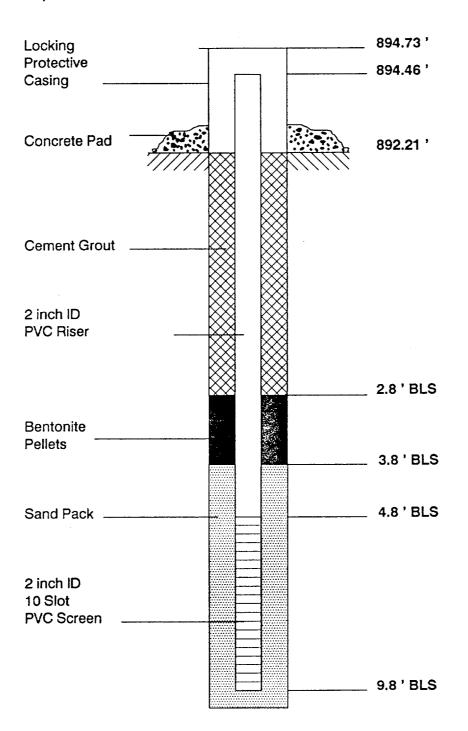
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## MONITORING WELLS CONSTRUCTION DIAGRAMS

Total Depth 10.0'

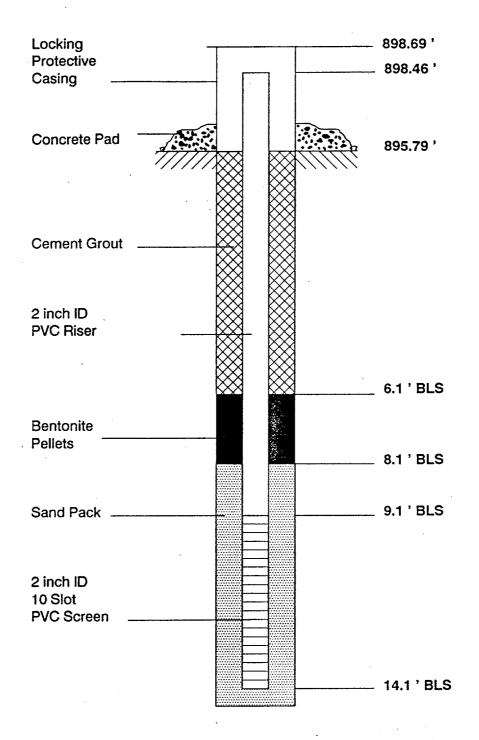
Borehole Diameter 0.7 '



## BOREHOLE NUMBER: 155 LOCATION: Sample Area 1 27 April 1989

Total Depth 14.5 '

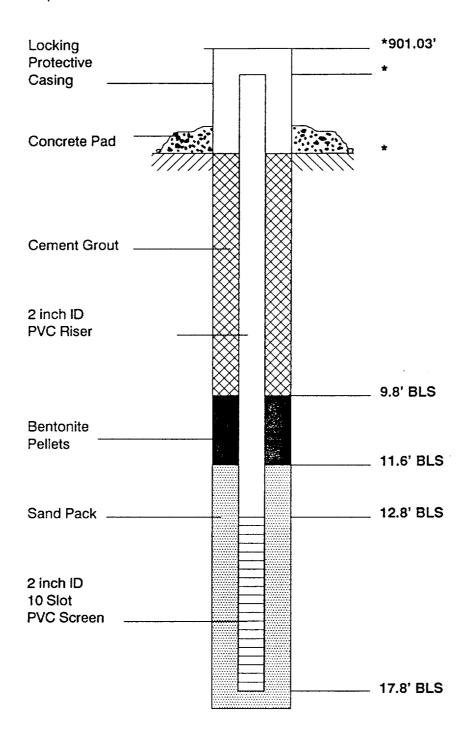
Borehole Diameter 0.7 '



## BOREHOLE NUMBER: 158 LOCATION: Sample Area 1 28 April 1989

Total Depth 19.0'

Borehole Diameter 0.7'

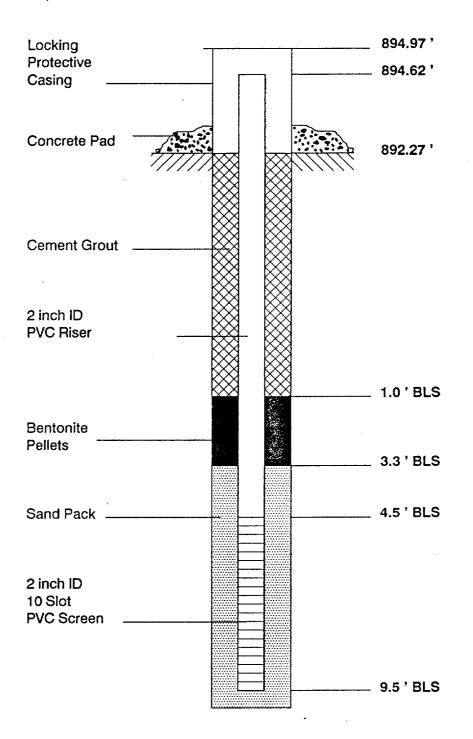


\* The well has been damaged, and the elevations are approximate.

## BOREHOLE NUMBER: 168 LOCATION: Sample Area 1 25 April 1989

Total Depth 10.0 '

Borehole Diameter 0.7 '

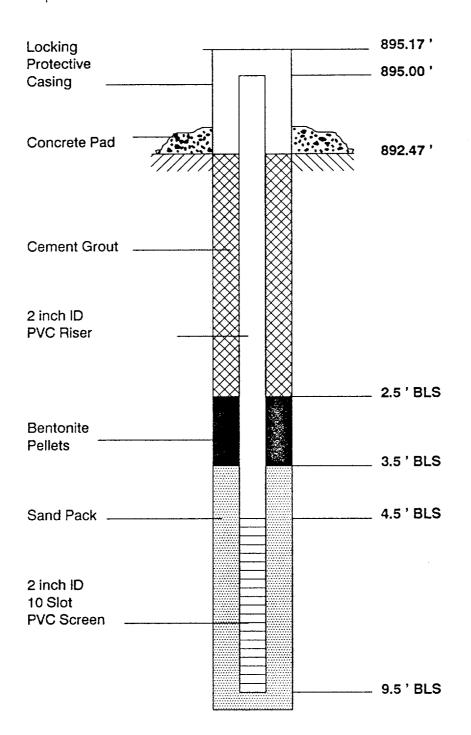


#### BOREHOLE NUMBER: 172 LOCATION: Sample Area 1 27 April 1989

Total Depth 10.5'

Borehole Diameter 0.7 '

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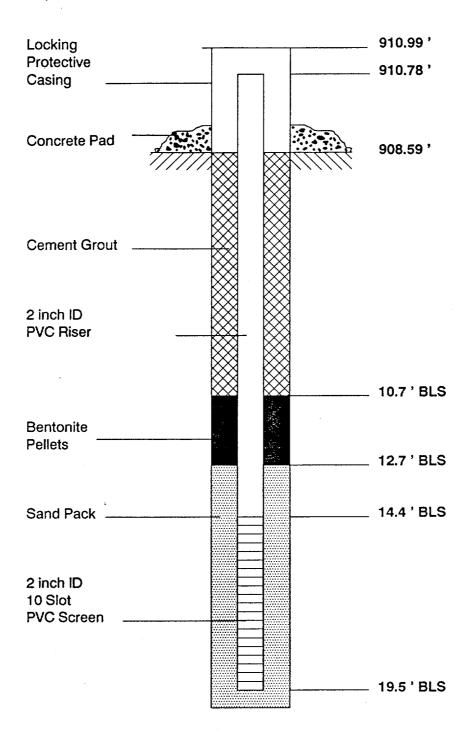
BOREHOLE NUMBER: 206 LOCATION: Sample Area 2 27 June 1989

Total Depth 19.6 '

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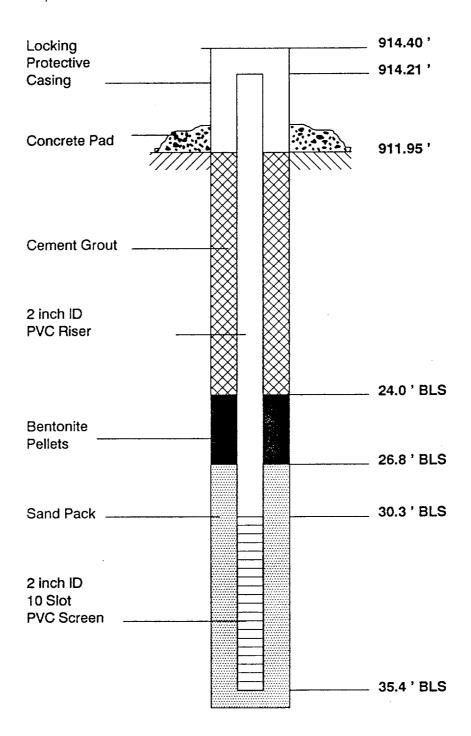
Borehole Diameter 0.7 '



Total Depth 35.5 '

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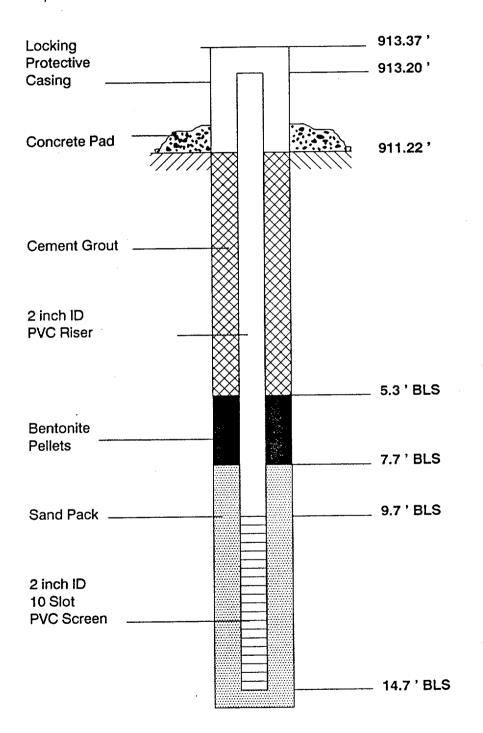
Borehole Diameter 0.7 '



# BOREHOLE NUMBER: 306 LOCATION: Sample Area 3 29 June 1989

Total Depth 15.0'

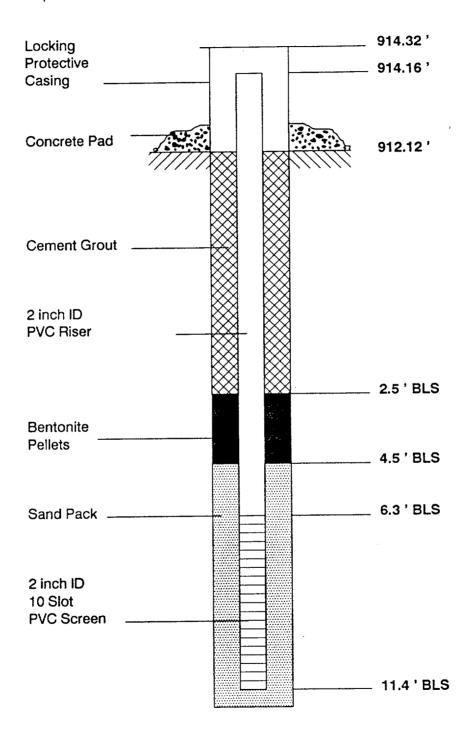
Borehole Diameter 0.6 '



# BOREHOLE NUMBER 312 LOCATION: Sample Area 3 29 June 1989

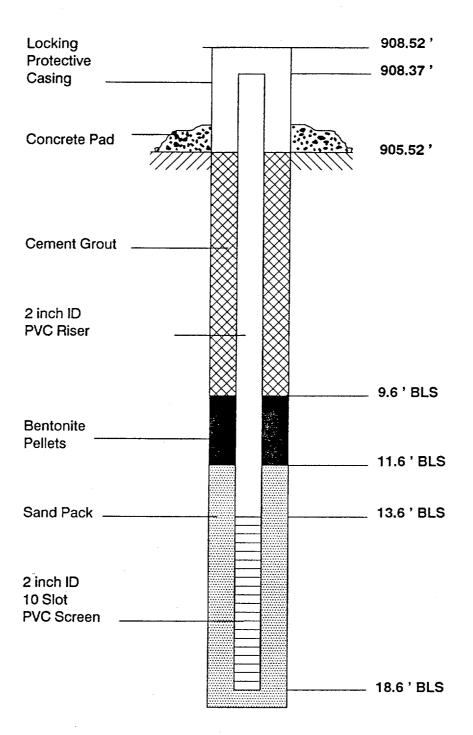
Total Depth 12.5 '

Borehole Diameter 0.6 '



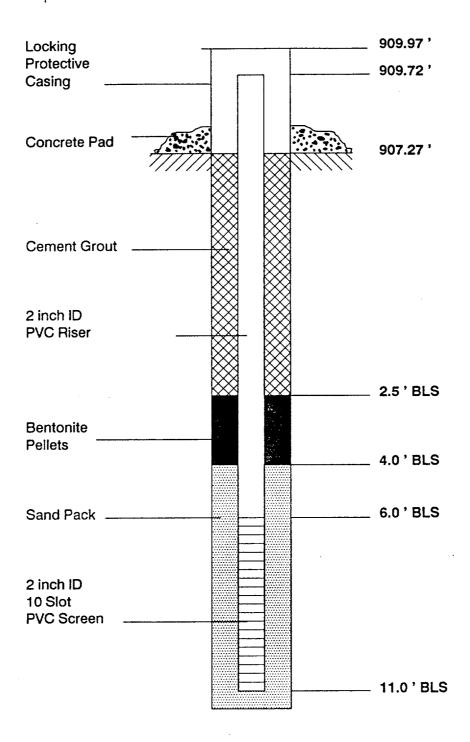
#### BOREHOLE NUMBER: 403 LOCATION: Sample Area 4 03 August 1989

Total Depth 18.6'



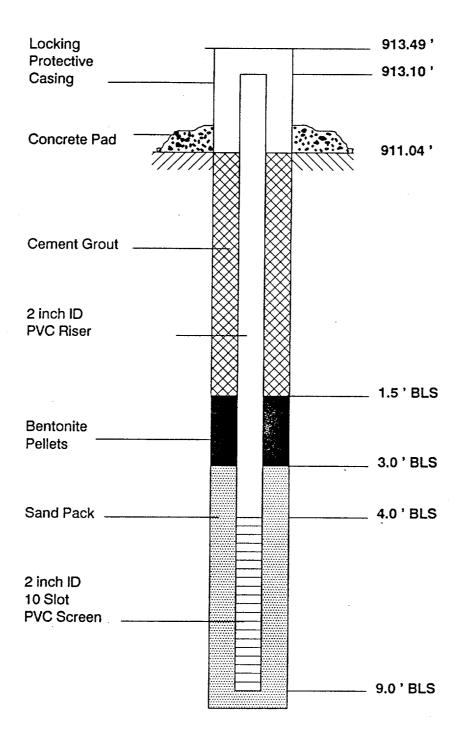
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Total Depth 11.0'



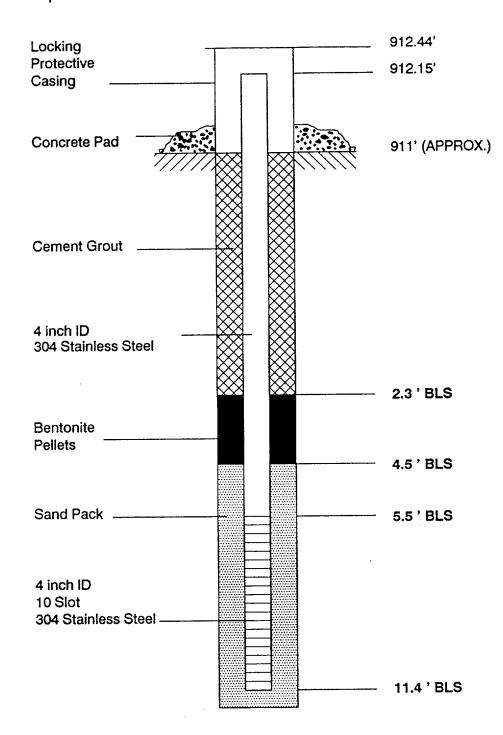
Total Depth 10.0 '

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# BOREHOLE NUMBER: C-03 LOCATION: Behind JN-4 17 November 1989

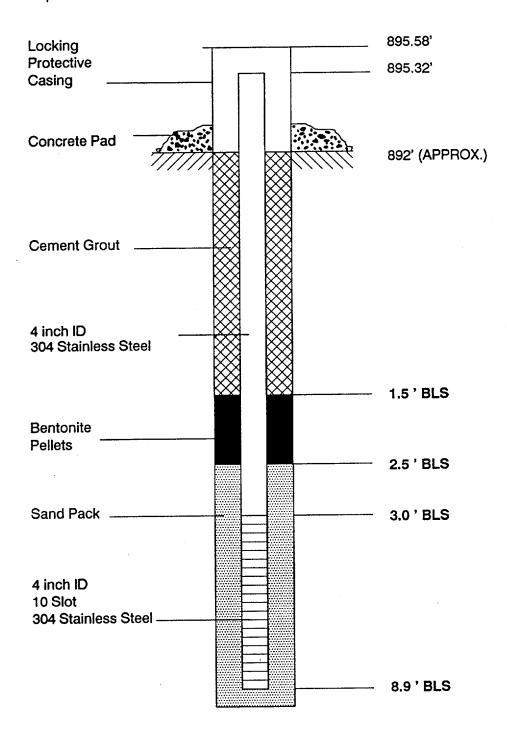
Total Depth 11.4 '



# BOREHOLE NUMBER: C-09 LOCATION: Sewer Outfall 17 November 1989

Total Depth 8.9'

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BOREHOLE NUMBER: C-16 LOCATION: South of JN-2 17 November 1989

912.14' Locking Protective 911.95' Casing Concrete Pad 909.5' (APPROX.) Cement Grout 4 inch ID 304 Stainless Steel 4.5 ' BLS **Bentonite** Peilets 6.5 ' BLS Sand Pack \_ 8.0 ' BLS 4 inch ID 10 Slot 304 Stainless Steel -13.6 ' BLS

Total Depth 13.6'

# APPENDIX 3

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# SLUG TEST DATA

### HERMIT DATA FOR WELL 150 SLUG TEST

Environ	E1000B nental 10 18:	Logg :33	jer	
Unit# O	0476 1	ſest∦	≠ 0	
INPUT 1:	Level	(F)	тос	
Reference Scale fac Offset	tor	29.	.00 .98 .04	
Step# 0	11/10	09:	:40	
Elapsed T 0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333		$\begin{array}{c} 0 \\ 0 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\ 1 \\$	1e .07 .819 .0693 .8774162740633752975310495258 .005258	

Elapsed Time	e Value
Elapsed Time 0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 4.5000 5.5000 6.0000 6.5000 7.0000 7.5000 8.0000 8.5000 9.0000	<pre>Value 0.83 0.78 0.72 0.69 0.64 0.60 0.56 0.53 0.51 0.47 0.44 0.42 0.39 0.36 0.26 0.18 0.14 0.10 0.08 0.06 0.04 0.04 0.03 0.02 0.02 0.02 0.02 0.01 0.01</pre>
9.5000 10.0000	0.01 0.01

#### HERMIT DATA FOR WELL 155 SLUG TEST

			•
Environ	E1000B mental 10 18:	Logger	
Unit# OG	0476 1	ſest# 3	
INPUT 1:	Level	(F) TOC	
Reference Scale fact Offset	tor	0.00 29.98 0.04	
Step# 0	11/10	11:17	
Elapsed Ti 0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333	ime 	8.15 7.65 0.99 3.96 1.13 2.54 1.87 2.17 2.05 2.04 2.04 2.04 2.01	· ·

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Elapsed Time	Value
Elapsed Time 0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 4.0000 5.5000 5.5000 6.0000 5.5000 6.0000 5.5000 7.0000 7.5000 8.0000 9.5000 10.0000 12.0000	Value 1.52 1.50 1.46 1.43 1.40 1.38 1.36 1.33 1.31 1.28 1.26 1.23 1.21 1.20 1.09 0.99 0.90 0.90 0.84 0.77 0.70 0.66 0.61 0.57 0.53 0.50 0.47 0.44 0.41 0.39 0.25 0.21 0.18 0.17 0.15 0.13 0.12
28.0000 30.0000	0.12 0.12

#### HERMIT DATA FOR WELL 158 SLUG TEST

Envi	ronm	1000) enta 0 1	1 Lo	) )	er	
Unit	# 00	476	Tes	st#	4	
INPUT	1:	Leve	1 (1	-)	тос	
Refere Scale Offset	nce fact	or		0. 29. 0.		
Step#	0	11/1	0	12:	14	
$\begin{array}{c} 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0\\ 0.0$	000 033 066 099 133 160 233 260 333 500 833 066 333 500 663 300 166 333 500 666 833			$\begin{array}{c} 3. \\ 3. \\ 3. \\ 1. \\ 2. \\ 1. \\ 1. \\ 2. \\ 1. \\ 1. \\ 1$	14 83 56 36 72 78 61 92 70 93 99 00	

Elapsed Time	Value
0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 4.5000 5.5000 6.0000 5.5000 7.0000 7.5000 8.0000 8.5000 9.0000 9.5000 10.0000 12.0000 12.0000 12.0000 12.0000 22.0000 24.0000 26.0000 32.0000 32.0000 34.0000 36.0	1.94 1.94 1.94 1.94 1.93 1.93 1.93 1.93 1.93 1.92 1.92 1.92 1.92 1.92 1.92 1.92 1.90 1.90 1.89 1.88 1.88 1.87 1.86 1.85 1.84 1.82 1.82 1.81 1.71 1.69 1.67 1.65 1.64 1.55 1.53

# HERMIT DATA FOR WELL 158 (Continued)

Elapsed Time	Value
46.0000	1.53
48.0000	1.51 1.49
52.0000	1.49
54.0000	1.47
56.0000	1.45
58.0000	1.44
60.0000 62.0000	1.43 1.42
64.0000	1.42
66.0000	1.40
68.0000	1.39
70.0000	1.38
72.0000	1.38 1.37
76.0000	1.37
78.0000	1.35
80.0000	1.34
82.0000	1.33
84.0000 86.0000	1.32
88.0000	1.32
90.0000	1.30
92.0000	1.29
94.0000	1.28
96.0000 98.0000	1.27 1.27
100.000	1.27
110.000	1.20
120.000	1.19
130.000	1.15
140.000 150.000	1.11 1.09
END	1.03

#### HERMIT DATA FOR WELL 168 SLUG TEST

SE1000B Environmental 11/10 18	:34	
Unit# 00476 INPUT 1: Level		
Reference Scale factor Offset	0.00	
Step# 0 11/10	10:03	
Elapsed Time 0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333	Value 4.89 0.94 1.67 1.69 1.63 1.52 1.45 1.38 1.33 1.27 1.22 1.03 0.90 0.79 0.69 0.63 0.56 0.51 0.46 0.42 0.38 0.35 0.33 0.29 0.27 0.25 0.23 0.22 0.17 0.14 0.12 0.08 0.07	Elapsed 0.91 1.00 1.08 1.16 1.25 1.33 1.41 1.50 1.58 1.66 1.75 1.83 1.91 2.00 3.50 4.00 4.50 5.00 5.50 6.00 6.50 7.00 7.50 8.00 8.50 9.00 9.50 10.00

apsed Time	Value
	*
0.9167	0.07
1.0000	0.06
1.0833	0.05
1.1667	0.05
1.2500	0.04
1.3333	0.04
1.4166	0.04
1.5000	0.04
1.5833	0.03
1.6667	0.03
1.7500	0.03
1.8333	0.03
1.9167	0.03
2.0000	0.03
2.5000	0.02
3.0000	0.02
3.5000	
	0.01
4.0000	0.01
4.5000	0.01
5.0000	0.00
5.5000	0.00
6.0000	0.00
6.5000	0.00
7.0000	0.01
7.5000	0.01
8.0000	0.01
8.5000	0.01
9.0000	0.01
9.5000	0.01
10.0000	0.01
ID	

### HERMIT DATA FOR WELL 172 SLUG TEST

Elapsed Time	Value
Elapsed Time 0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 5.0000 5.5000 6.0000	Value 0.37 0.34 0.25 0.25 0.23 0.21 0.19 0.17 0.15 0.15 0.15 0.15 0.13 0.12 0.08 0.06 0.05 0.03 0.02 0.02 0.02 0.01
6.0000	0.01
6.5000 7.0000	0.01
7.5000 8.0000 8.5000	0.00 0.00 0.00
9.0000	0.00
9.5000 10.0000	0.00
END	

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#### HERMIT DATA FOR WELL 206 SLUG TEST

# HERMIT DATA FOR WELL 206 (Continued)

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Floored Time	Value
Elapsed Time	Value
*******	
46.0000	1.53
48.0000	1.53
50.0000	1.53 1.53
52.0000	1.53
54.0000	1.53
56.0000	
58.0000	1.53 1.53
60.0000	1.53
62.0000	1.52
64.0000	1.52
66.0000 68.0000	1.52
70.0000	1.52
72.0000	1.52
74.0000	1.52
76.0000	1.52
78.0000	1.52
80.0000	1.52
82.0000	1.52
84.0000	1.52
86.0000	1.51
88.0000	1.51
90.0000	1.51
92.0000	1.51
94.0000	1.51
96.0000	1.51
98.0000	1.51
100.000	1.51
110.000	1.50
120.000	1.50
130.000	1.50
140.000	1.49
END	

END

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#### HERMIT DATA FOR WELL 300 SLUG TEST

Value

0.41 0.40 0.38 0.37

0.37 0.35 0.35 0.34 0.34 0.34 0.33 0.33 0.33

0.32 0.31 0.30 0.28 0.28 0.28 0.28 0.27 0.27 0.27 0.27 0.26 0.26 0.26 0.27 0.26 0.26 0.26 0.25 0.26 0.25 0.25 0.25

SE1000B Environmental 12/19 17		Elapsed Time
U	Fest# 7	0.9167
	(F) TOC	1.0000 1.0833
Reference	0.00	1.1667 1.2500
Scale factor Offset	29.98 0.04	1.3333 1.4166 1.5000
Step# 0 12/19	15:10	1.5833 1.6667
Elapsed Time	Value	1.7500
0.0000	0.86	1.9167
0.0033	5.37	2.0000
0.0066 0.0099	3.43 2.59	2.5000 3.0000
0.0133	4.06	3.5000
0.0166	1.41	4.0000
0.0200	1.42	4.5000
0.0233	1.82	5.0000 5.5000
0.0266 0.0300	$1.50 \\ 1.54$	6.0000
0.0333	1.51	6.5000
0.0500	1.36	7.0000
0.0666	1.24	7.5000
0.0833	1.15	/ 8.0000 / 8.5000
0.1000 0.1166	$1.06 \\ 1.00$	9.0000
0.1333	0.94	9.5000
0.1500	0.89	10.0000
0.1666	0.85	12.0000
0.1833 0.2000	0.81 0.77	14.0000 16.0000
0.2166	0.74	18.0000
0.2333	0.71	20.0000
0.2500	0.69	22.0000
0.2666	0.68	24.0000
0.2833 0.3000	0.66 0.64	26.0000 28.0000
0.3166	0.62	30.0000
0.3333	0.61	32.0000
0.4167	0.55	34.0000
0.5000	0.51	36.0000
0.5833	0.49 0.46	38.0000 40.0000
0.6667 0.7500	0.40	40.0000
0.8333	0.43	44.0000

# HERMIT DATA FOR WELL 300 (Continued)

;

Elapsed Time	Value
46.0000	0.25
48.0000	0.25
50.0000	0.24
52.0000	0.24
54.0000	0.25
56.0000	0.24
58.0000	0.25
60.0000	0.25
62.0000	0.25
64.0000	0.24
66.0000	0.25
68.0000	0.25
70.0000	0.25
72.0000	0.25
74.0000	0.25
76.0000	0.25
78.0000	0.25
80.0000	0.25
82.0000	0.25
84.0000	0.25
86.0000	0.25
FND	

#### HERMIT DATA FOR WELL 306 SLUG TEST

SE1000B Environmental Logger 11/10 18:45				
Unit# 00	476	ſest# 5		
INPUT 1:	Level	(F) TOC		
Reference Scale fact Offset	tor	0.00 29.98 0.04		
Step# 0	11/10	16:01		
Elapsed T 0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333		Value 0.01 2.50 6.28 2.26 0.53 3.71 0.86 1.84 2.44 1.15 2.18 2.46 2.32 2.38 1.55 1.50 1.28 1.28 1.20 1.28 1.20 1.28 1.29 1.03 0.98 0.94		

Value
Value 0.89 0.86 0.83 0.79 0.76 0.72 0.69 0.68 0.65 0.62 0.60 0.58 0.56 0.54 0.31 0.28 0.26 0.24 0.23 0.22 0.21 0.20 0.21 0.20 0.18 0.17 0.16 0.15 0.13 0.17 0.16 0.15 0.13 0.11 0.10

# HERMIT DATA FOR WELL 306 (Continued)

Elapsed Time	Value			
46.0000	0.00			
48.0000	0.09 0.08			
50.0000	0.08			
52.0000	0.09			
54.0000	0.08			
56.0000	0.08			
58.0000	0.09			
60.0000	0.08			
62.0000	0.08			
64.0000	0.08			
66.0000	0.08			
68.0000	0.08			
70.0000	0.08			
72.0000	0.08			
74.0000	0.09			
76.0000	0.08			
78.0000	0.08			
80.0000	0.08			
END	<u>e</u> '			

END

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#### HERMIT DATA FOR WELL 312 SLUG TEST

SE1000B Environmental 12/19 17	Logger :33	Elapsed Time	Value
Unit# 00476	Test# 7	0.9167	1.57
INPUT 2: Level	(F) TOC	1.0000 1.0833 1.1667	1.57 1.56 1.56
Reference Scale factor Offset	0.00 19.98 0.09	1.2500 1.3333 1.4166	1.56 1.56 1.56
	15:10	1.5000 1.5833 1.6667	1.56 1.56 1.56
Elapsed Time	Value	1.7500 1.8333	1.56 1.56
0.0000 0.0033 0.0066	- 0.02 1.98 5.07	1.9167 2.0000 2.5000	1.56 1.55 1.55
0.0099 0.0133 0.0166	2.09 1.53 1.87	3.0000 3.5000 4.0000	1.55 1.54 1.54
0.0200 0.0233 0.0266	1.49 1.81 1.56	4.5000 5.0000 5.5000	1.54 1.54 1.54
0.0300 0.0333	1.69 1.61	6.0000 6.5000 7.0000	1.53 1.53
0.0500 0.0666 0.0833	1.62 1.63 1.62	7.5000 8.0000	1.53 1.53 1.53
0.1000 0.1166 0.1333	$1.60 \\ 1.60 \\ 1.60 $	9.0000 9.5000	1.53 1.54 1.54
0.1500 0.1666 0.1833	$1.60 \\ 1.60 \\ 1.60$	10.0000 12.0000 14.0000	1.56 1.56 1.59
0.2000 0.2166 0.2333	1.60 1.59 1.59	16.0000 18.0000 20.0000	1.60 1.59 1.60
0.2500 0.2666	$1.59 \\ 1.59$	22.0000 24.0000 26.0000	1.60 1.59 1.59
0.2833 0.3000 0.3166	1.58 1.58 1.58	28.0000 30.0000	1.58 1.55
0.3333 0.4167 0.5000	1.58 1.58 1.58	32.0000 34.0000 36.0000	1.56 1.55 1.55
0.5833 0.6667 0.7500	1.58 1.58 1.58	38.0000 40.0000 42.0000	1.56 1.56 1.57
0.8333	1.57	44.0000	1.57

# HERMIT DATA FOR WELL 312 (Continued)

Elapsed Time	Value
46.0000	1.57
48.0000	1.59
50.0000 52.0000	1.57
54.0000	1.55 1.54
56.0000	1.54
58.0000	1.55
60.0000	1.56
62.0000	1.56
64.0000	1.56
66.0000	1.55
68.0000	1.56
70.0000	1.56
72.0000	1.56
74.0000	1.54
76.0000	1.53
78.0000 80.0000	1.53
82.0000	1.52 1.52
84.0000	1.53
86.0000	1.52
END	

END

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#### HERMIT DATA FOR WELL 403 SLUG TEST

SE1000B Environmental Log 11/10 18:47	iger	Elapsed Time	Value
Unit# 00476 Test	:# 5	0.9167 1.0000	1.60 1.57
INPUT 2: Level (F)	TOC	1.0833	1.57 1.55 1.53
Scale factor 19	0.00 0.98 0.09	1.2500 1.3333 1.4166 1.5000	1.50 1.48 1.46 1.44
Step# 0 11/10 16	:01	1.5833 1.6667	1.42
Elapsed Time Val	ue	1.7500	1.38
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	.00 .70 .04 .37 .96 .37 .75 .45 .14 .14 .14 .69 .91 .96 .95 .94 .92 .92 .92 .92 .92 .92 .90 .89 .89 .89 .89 .87 .87 .86 .85 .84 .83 .82 .82 .81 .77 .71 .68 .65 .62	$\begin{array}{c} 1.8333\\ 1.9167\\ 2.0000\\ 2.5000\\ 3.0000\\ 3.5000\\ 4.0000\\ 4.5000\\ 5.5000\\ 6.0000\\ 5.5000\\ 6.0000\\ 6.5000\\ 7.0000\\ 7.5000\\ 8.0000\\ 8.5000\\ 9.0000\\ 9.5000\\ 10.0000\\ 9.5000\\ 10.0000\\ 12.0000\\ 12.0000\\ 12.0000\\ 12.0000\\ 12.0000\\ 24.0000\\ 24.0000\\ 24.0000\\ 24.0000\\ 24.0000\\ 36.0000\\ 38.0000\\ 36.0000\\ 38.0000\\ 40.0000\\ 42.0000\\ 44.0000\\ 44.0000\\ \end{array}$	$\begin{array}{c} 1.36\\ 1.34\\ 1.32\\ 1.23\\ 1.15\\ 1.08\\ 1.01\\ 0.95\\ 0.89\\ 0.85\\ 0.80\\ 0.77\\ 0.74\\ 0.72\\ 0.70\\ 0.68\\ 0.65\\ 0.65\\ 0.65\\ 0.552\\ 0.552\\ 0.552\\ 0.551\\ 0.59\\ 0.49\\ 0.49\\ 0.49\\ 0.50\\ 0.49\\ 0.50\\ 0.55\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.52\\ 0.49\\ 0.50\\ 0.52\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.49\\ 0.50\\ 0.51\\ 0.52\\ 0.52\\ 0.51\\ 0.52\\ 0.52\\ 0.51\\ 0.52\\ 0.51\\ 0.52\\ 0.53\\ 0.52\\ 0.51\\ 0.52\\ 0.53\\ 0.52\\ 0.53\\ 0.55\\ 0.52\\ 0.55\\ 0.55\\ 0.52\\ 0.55\\ 0$

# HERMIT DATA FOR WELL 403 (Continued)

Elapsed Time	Value		
46.0000	0.37		
48.0000	0.35		
50.0000	0.35		
52.0000	0.35		
54.0000	0.34		
56.0000	0.34		
58.0000	0.35		
60.0000	0.35		
62.0000	0.35		
64.0000	0.35		
66.0000	0.35		
68.0000	0.35		
70.0000	0.36		
72.0000	0.37		
74.0000	0.37		
76.0000	0.37		
78.0000	0.37		
80.0000	0.37		

#### HERMIT DATA FOR WELL 506 SLUG TEST

Environ	E1000B mental 19 17:	Logo	jer	
Unit# O	0476	Test	#6	
INPUT 2:	Level	(F)	тос	
Reference Scale fac Offset	tor	19.	.00 .98 .09	
Step# 0	12/19	11:	:32	
Elapsed T	ime	Valu	le	
0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1666 0.1333 0.1500 0.1666 0.2333 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333		1. 1. 1. 1. 1. 1. 1. 1. 1. 1.	61 61 55 47 39 31 25 20 15	

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# HERMIT DATA FOR WELL 506 (Continued)

Elapsed Time	Value
46.0000	0.24
48.0000	0.23
50.0000	0.23
52.0000	0.23
54.0000	0.23
56.0000	
	0.21
58.0000	0.20
60.0000	0.19
62.0000	0.17
64.0000	0.15
66.0000	0.14
68.0000	0.13
70.0000	0.13
72.0000	0.13
74.0000	0.13
76.0000	0.11
78.0000	0.11
80.0000	0.11
82.0000	0.12
84.0000	0.11
86.0000	0.11
88.0000	0.09
90.0000	
	0.09
92.0000	0.08
94.0000	0.08
96.0000	0.09
98.0000	0.10
100.000	0.10
110.000	0.07
120.000	0.07
130.000	0.06
140.000	0.03
END	

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### HERMIT DATA FOR WELL 601 SLUG TEST

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Elapsed Time	Value
0.9167	0.30
1.0000	0.30
1.0833	0.29
1.1667	0.28
1.2500	0.28
1.3333	0.27
1.4166	0.27
1.5000 1.5833	0.26 0.25
1.6667	0.25
1.7500	0.24
1.8333	0.21
1.9167	0.20
2.0000	0.20
2.5000	0.17
3.0000	0.15
3.5000	0.14
4.0000	0.12
4.5000	0.12
5.0000	0.11
5.5000	0.10
6.0000 6.5000	0.10
7.0000	0.09 0.09
7.5000	0.03
8.0000	0.08
8.5000	0.07
9.0000	0.07
9.5000	0.08
10.0000	0.07
12.0000	0.06
14.0000	0.06
16.0000	0.06
18.0000	0.06
20.0000	0.05

### HERMIT DATA FOR WELL CO3 SLUG TEST

SE1000B Environmental Logger 01/16 18:03			
Unit# 00	)476 T	est# 0	
INPUT 1:	Level	(F) TOC	
Reference Scale fact Offset	or	0.00 29.98 0.04	
Step# 0	01/12	14:04	
Elapsed Ti 0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333	me	Value 0.02 0.01 0.04 0.83 1.95 0.94 1.90 2.04 1.90 1.85 1.73 1.63 1.55 1.46 1.37 1.29 1.23 1.16 1.11 1.08 1.04 1.01 1.03 1.00 0.98 0.98 0.99 0.96 0.95 0.92 0.86 0.86 0.85	

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Elapsed Time	Value
0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 4.0000 4.5000 5.5000 6.0000 5.5000 7.0000 7.5000 8.0000 9.0000 9.5000 10.0000 12.0000 12.0000 12.0000 12.0000 22.0000 22.0000 22.0000 23.0000 32.0000 32.0000 34.0000 36.0	0.84 0.83 0.82 0.82 0.82 0.82 0.83 0.81 0.80 0.81 0.81 0.79 0.79 0.79 0.79 0.77 0.77 0.77 0.75 0.72 0.72 0.72 0.72 0.71 0.70

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# HERMIT DATA FOR WELL CO3 (Continued)

Elapsed Time	Value
46.0000	0.69
48.0000	0.70
50.0000 52.0000	0.70 0.69
54.0000	0.69
56.0000	0.69
58.0000	0.69
60.0000	0.69
62.0000 64.0000	0.69
66.0000	0.69 0.69
68.0000	0.69
70.0000	0.69
72.0000	0.69
74.0000 76.0000	0.69 0.69
78.0000	0.69
80.0000	0.69
82.0000	0.68
84.0000	0.67
86.0000 88.0000	0.67 0.67
90.0000	0.67
92.0000	0.67
94.0000	0.66
96.0000	0.68
98.0000 100.000	0.67 0.67
110.000	0.65
120.000	0.65
130.000	0.66
140.000 150.000	0.64
160.000	0.64 0.64
170.000	0.63
180.000	0.62
ND	

#### HERMIT DATA FOR WELL CO9 SLUG TEST

				0	
Environ	E1000B mental 16 17:	Logg 53	er		
Unit# O	0476 T	'est#	2		
INPUT 1:	Level	(F) <sup>-</sup>	тос		
Reference Scale fac Offset	tor	0.0 29.9 0.0	98		
Step# 0	01/16	16:3	33		
Elapsed T	ime	Value			
0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.2333 0.2000 0.2166 0.2333 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333		$\begin{array}{c} 0.0\\ 0.0\\ 0.0\\ 0.0\\ 1.2\\ 0.4\\ 0.1\\ 1.1\\ 1.7\\ 2.0\\ 1.5\\ 1.4\\ 1.2\\ 1.1\\ 1.0\\ 1.0\\ 0.9\\ 0.9\\ 0.9\\ 0.9\\ 0.9\\ 0.9\\ 0.9\\ 0$	010096229490327677814063208765419766	;	

Elapsed Time	Value
0.9167 1.0000	0.84 0.84
1.0833 1.1667	0.83
1.2500 1.3333	0.82
1.4166 1.5000	0.81 0.80
1.5833 1.6667	0.80
1.7500 1.8333	0.79
1.9167	0.79
2.5000 3.0000	0.77
3.5000 4.0000	0.74 0.73
4.5000 5.0000	0.72 0.71
5.5000 6.0000	0.70 0.69
6.5000 7.0000	0.69 0.69
7.5000 8.0000	0.68 0.67
8.5000 9.0000	0.67
9.5000 10.0000	0.65
12.0000 14.0000	0.62
16.0000 18.0000	0.58
20.0000	0.54 0.52
24.0000 26.0000	0.51
28.0000 30.0000	0.48
32.0000 34.0000	0.45
36.0000 38.0000	0.42
40.0000 42.0000	0.39
44.0000	0.37

# HERMIT DATA FOR WELL CO9 (Continued)

Elapsed Time	Value
46.0000	0.36
48.0000	0.35
50.0000	0.34
52.0000	0.34
54.0000	0.33
56.0000	0.32
58.0000	0.31
60.0000	0.31
62.0000 64.0000	0.30
66.0000	0.29
68.0000	0.28
70,0000	0.27 0.262
	0.202

# HERMIT DATA FOR WELL C16 (Continued)

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#### APPENDIX 4

#### PRECIPITATION RECORD

# HERMIT DATA FOR WELL C16 SLUG TEST

SE1000B Environmental Logger 01/11 16:57			
Unit# O	0476 -	Fest#	8
INPUT 1:	Level	(F) 1	гос
Reference Scale fac Offset	tor	0.( 29.9 0.(	98
Step# 0	01/11	11:5	52
Elapsed T 0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1000 0.1666 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.4167 0.5000 0.5833 0.4167 0.5000 0.3833		Value 0.0 1.8 1.6 1.7 0.2 0.8 1.9 1.8 1.8 1.8 1.8 1.8 1.8 1.8 1.8	162089319960887677867767555575533

L\_\_\_\_

Elapsed Time	Value
0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 4.5000 5.0000 5.0000 6.0000 6.5000 7.0000 7.5000 8.0000 8.0000 9.0000 9.0000 9.5000 10.0000 12.0000 14.0000 14.0000 14.0000 14.0000 20.0000 22.0000 24.0000 32.0000 34.0000 36.0000 38.0000 38.0000 40.0000 40.0000 38.0000 30.0000 38.00000 38.00000 38.0000 38.0000 38.0000 38.0000 38.0	$\begin{array}{c} 1.83\\ 1.83\\ 1.83\\ 1.83\\ 1.82\\$

Date	Day No.	Rainfall Inches	Date	Day No.	Rainfall Inches
June 18, 1989	0	0.02	Nov. 2, 1989	137	0.05
June 20, 1989	2	0.48	5 8	140 143	0.13 0.64
21	2 3	1.71	14	149	0.05
28	10	1.49	15	150	1.32
20 10	20.0	16	151	0.02	
July 3, 1989	15	0.45	26	161	0.20
Ĩ 7 Î	19	0.05	27	162	0.06
11	23	0.43			
12	24	0.23	Dec. <u>6</u> , 1989	171	0.22
11 12 16 18	28	0.01	7	172	0.02
18	30	0.18	11	176	0.03
21	33	0.12	14	179	0.02
24 26	36 38	0.01 0.01	15 21	180 186	0.33 0.02
28	40	0.66	26	191	0.02
30	42	0.36	27	192	0.04
50	, _	0.00	31	196	1.00
Aug. 4, 1989	47	0.24	-		
5 7	48	0.74	Jan. 4, 1990	200	0.21
	50	0.02	9	205	0.05
20	63	0.03	12	208	0.01
21	64	0.08	15	211	0.06
22	65	1.27	16	212	0.01
24 30	67 73	0.18	17 20	213 216	0.25 1.18
30	/3	1.16	23	210	0.02
Sept. 1, 1989	75	0.63	25	221	0.02
14	88	0.87	29	225	0.74
16	90	0.19	. – –		
23	97	0.30	Feb. 2, 1990	229	0.73
			4	231	1.30
Oct. 2, 1989	106	0.17	7	234	0.17
7	111	0.04	9	236	0.27
10	114	0.41	11	238	0.08
16	120	0.09	15	242	2.45
19	123	1.05	22	249 251	0.10 0.55
					0.55
20 31	124 135	0.12 0.36	24 27	251 254	

#### PRECIPITATION RECORD

\_\_\_\_

Date	Day No.	Rainfall Inches	Date	Day No.	Rainfall Inches
March 8, 1990 10 17 19 22 24 29 31	263 265 272 274 277 279 284 286	0.23 0.46 0.41 0.12 0.01 0.08 0.31 0.20	July 1, 1990 9 12 13 14 21 22 23	379 387 390 391 392 399 400 401	0.25 0.06 4.39 0.15 2.11 0.81 1.83 0.88
April 1, 1990 3 10 11 14 17 21 29	287 289 296 297 301 304 308 316	0.34 0.02 1.30 0.11 0.05 0.04 0.56 0.17	Aug. 5, 1990 13 18 20 21 22 29	414 422 427 429 430 431 438	0.78 0.1 0.56 0.32 0.01 0.25
May 4, 1990 6 10 11 13 16 17 20 26 27 29	321 323 327 328 330 333 334 337 343 344 346	1.48 0.15 0.11 0.03 1.73 1.29 0.85 0.05 0.92 0.09 2.05	Sept. 7, 1990 9	447 449	0.51 0.85
June 2, 1990 3 7 8 9 14 21 23 24	350 351 355 356 357 362 369 371 372	0.32 0.14 0.68 1.60 1.28 0.70 0.07 0.27 0.10		•	

#### PRECIPITATION RECORD

142-90-48

REPORT

# SITE CHARACTERIZATION

WEST JEFFERSON

NORTH SITE

**GROUNDWATER** 

MONITORING WELL

**HYDRAULIC** 

**CONDUCTIVITY TESTING** 

AND ANALYSIS

JANUARY 31, 1990



WSN Site C 0

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# REPORT

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# SITE CHARACTERIZATION WEST JEFFERSON NORTH SITE GROUNDWATER MONITORING WELL HYDRAULIC CONDUCTIVITY TESTING AND ANALYSIS

January 31, 1990

BATTELLE 505 King Avenue Columbus, Ohio 43201-2693

## TABLE OF CONTENTS

		<u>P</u>	aye
1.0	O INTRODUCTION		1
2.0	O DESCRIPTION OF THE AREA	• • • • • •	1
3.0	.0 HYDRAULIC CONDUCTIVITY FIELD TESTING		3
4.0	O DATA ANALYSIS	• • • • • •	3
5.0	.0 RESULTS		4
6.0	.0 REFERENCES	• • • • • •	6

# LIST OF APPENDICES

AP	PENDIX 1:	BORING LOGS FOR MONITORING WELLS
AP	PENDIX 2:	MONITORING WELL CONSTRUCTION DIAGRAMS
	PENDIX 3:	SLUG TEST DATA

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# LIST OF TABLES

### LIST OF FIGURES

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### REPORT

#### on

SITE CHARACTERIZATION WEST JEFFERSON NORTH SITE GROUNDWATER MONITORING WELL HYDRAULIC CONDUCTIVITY TESTING AND ANALYSIS

January 31, 1990

#### 1.0 INTRODUCTION

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This report summarizes the hydraulic conductivity testing that was performed in the Stage 1 characterization of the Nuclear Sciences Area of Battelle's West Jefferson Site. The testing was done in 15 groundwater monitoring wells that were installed in the summer and fall of 1989 as part of a comprehensive characterization to identify areas of soil which might require remediation in order to release the site for unrestricted use. Figure 1-1 shows the West Jefferson North Site area with the buildings designated "JN". In addition, the well locations are also shown. The results of this work can be used in conjunction with additional information regarding groundwater flow directions and gradients as input to permit accurate pathways analysis of radionuclide transport in the subsurface.

### 2.0 DESCRIPTION OF THE AREA

The land surface in the area generally is gently sloping except for the V-shaped valley of Silver Ditch south of the facility which is now dammed and occupied by a lake. The surface elevations within the fenced area are for the most part between 900 and 910 feet MSL, with the area to the east of the facility sloping downward to the Big Darby Creek flood plain.

The Nuclear Science Area at the West Jefferson Site is situated on glacial till deposits. The till overlies limestone/dolomite bedrock, and, based on bedrock contours and surface elevations at this site, the maximum range of till thickness is estimated to be 40 to 160 feet. The depth to bedrock in the immediate area is estimated to be 100 feet. Soil up to 6-feet thick has developed on the top of the till.

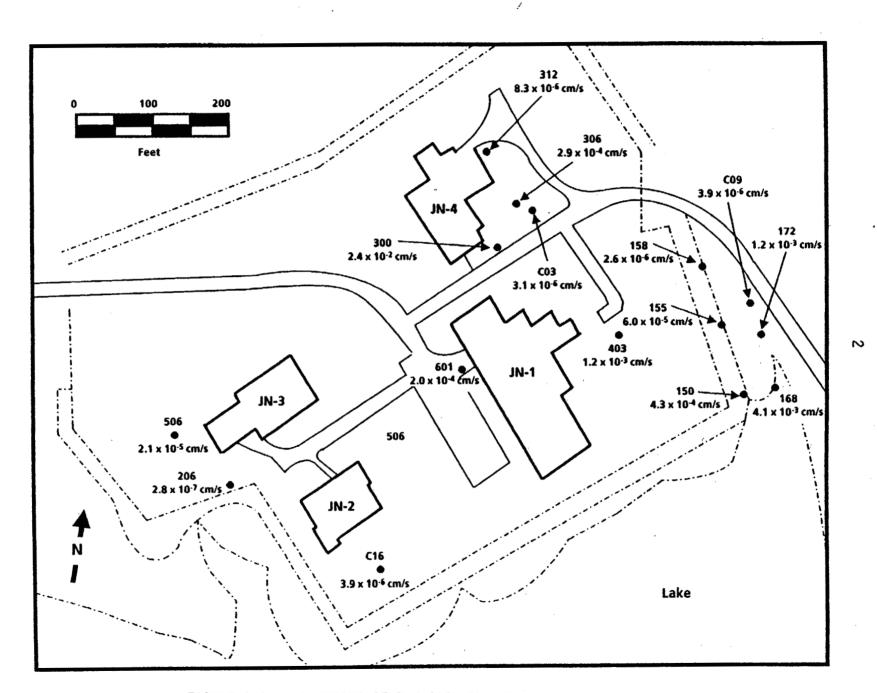


FIGURE 1.1: LOCATIONS OF THE GROUNDWATER MONITORING WELLS

It is in the till deposits that the borings and well installations were performed. The till consists of dense, predominantly non-plastic silt and clay, with minor amounts of sand and gravel in an unstratified mixture. Larger cobbles and boulders have been found in the till. Outwash deposits of small areal extent are also found within the till as sand and gravel lenses or stringers. The till has been subdivided in the boring logs (Appendix 1) by color into a brown till that is generally from 8 to 15-feet thick which overlies a grey till that is believed to extend to bedrock.

#### 3.0 HYDRAULIC CONDUCTIVITY FIELD TESTING

In the fall and winter of 1989, hydraulic conductivity tests, also called slug tests, were performed in 15 monitoring wells at the West Jefferson North Site. Twelve of the wells were constructed of 2-inch diameter PVC with Steel wells **Construction** and three of the wells were 4-inch diameter stainless steel wells **Construction Construction** diagrams are located in Appendix 2.

The wells were slug tested according to Site Characterization Procedure SC-SP-009 using a Hermit 1000 B data logger and pressure transducer. The results of the slug testing are located in Appendix 3.

#### 4.0 DATA ANALYSIS

The time versus water-level data from the slug tests was analyzed using two standard methods. The Bouwer and Rice method was used for the data, hydrogeology, and well construction dimensions that best defined an unconfined aquifer. The other method, the Cooper et. al. method, was used for situations that more closely resembled a confined or semi-confined aquifer. Both methods use an analysis of semi-log plots of relative water level change versus time.

In many cases the plotted data clearly matched one method or the other, but in several instances, the data was able to be analyzed using both methods. When the data could be properly analyzed using both methods, the answers were usually within an order of magnitude of each other. The

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techniques of slug test evaluation can be found in the technical papers cited in SC-SP-009 and the reference section.

## 5.0 <u>RESULTS</u>

The results of the analysis fall within the range of values normally associated with the geologic materials present at each well. The results can be grouped into four main categories by the material present, either the near surface brown till/fill material, the confined and unconfined sandy zones, and the dense grey till. Table 5-1 lists the results for each well by method and the best answer is indicated. The most appropriate hydraulic conductivity value for each well is shown in Figure 1-1.

These results as well as the boring logs both agree that in general wells 306, 312, 506, CO3, and C16 were completed in the brown till with some wells extending slightly into the grey till. The hydraulic conductivity values are moderate to low, ranging from 2.6 x  $10^{-4}$  cm/sec to 3.0 x  $10^{-7}$  cm/sec.

Wells 150, 155, 168, 172, 300, 403, 601, and CO9 were completed in the sandier materials of higher hydraulic conductivity. Wells 150 and 168 intersect a sandy deposit under unconfined conditions. Wells 155, 172, 403 and CO9 also are believed to be completed in this same sandy deposit but are under confined conditions, that is a lower conductivity material overlies the sandy material. All of these valves fall within a relatively narrow range from 1.2 x  $10^{-3}$  cm/sec to 6.0 x  $10^{-5}$  cm/sec. Well 300 may be completed in this same sand, but this is not certain. This well had the highest hydraulic conductivity of all wells tested at 2.4 x  $10^{-2}$  cm/sec. Well 601 was completed in sandier material but not the same continuous deposit as the others.

The remaining wells 158 and 206 were completed in dense grey till and have low hydraulic conductivities of 2.6 x  $10^{-6}$  cm/sec and 2.8 x  $10^{-7}$  cm/sec, respectively.

			HYDRAULIC CONDUCTIVITY					
		Bouwer & R	tice, 1976	<u>Cooper</u> e	Cooper et al., 1967			
1	Well #	ft/sec	cm/sec	ft/sec	cm/sec			
135	150	* 1.4 x 10 <sup>-5</sup>	4.3 x 10 <sup>-4</sup>	2.9 x 10 <sup>-5</sup>	8.9 x 10 <sup>-4</sup>			
20	155	4.2 x 10 <sup>-6</sup>	$1.3 \times 10^{-4}$	19 * 2.0 x 10 <sup>-8</sup>	$6.0 \times 10^{-5}$			
1	158	* 8.5 x 10 <sup>-8</sup>	2.6 x 10 <sup>-8</sup>	2.1 x 10 <sup>-9</sup>	6.5 x 10 <sup>-8</sup>			
1260	168	* 1.3 x 10 <sup>-4</sup>	4.1 x 10 <sup>-3</sup>	$2.0 \times 10^{-4}$	6.2 x 10 <sup>-3</sup>			
400	172	2.8 x 10 <sup>-5</sup>	$8.5 \times 10^{-4}$	* 4.0 $\times$ 10 <sup>-5</sup>	$1.2 \times 10^{-3}$			
8.)	206	* 9.1 x 10 <sup>-9</sup>	2.8 x 10 <sup>-7</sup>	$1.4 \times 10^{-10}$	4.2 x 10 <sup>-9</sup>			
7666	<i>∝</i> 300			* 7.9 x 10 <sup>-4</sup>	$2.4 \times 10^{-2}$			
91	306	$1.3 \times 10^{-5}$	$3.8 \times 10^{-4}$	* 9.4 x 10 <sup>-6</sup>	$2.9 \times 10^{-4}$			
3	312	* 2.7 x $10^{-7}$	8.3 x 10 <sup>-8</sup>	$9.9 \times 10^{-9}$	$3.0 \times 10^{-7}$			
400	403	2.0 x 10 <sup>-5</sup>	$5.9 \times 10^{-4}$	40° * 4.0 x 10 <sup>-5</sup>	$1.2 \times 10^{-3}$			
7	506	* 6.9 x 10 <sup>-7</sup>	2.1 x 10 <sup>-5</sup>					
لمالم	601	* 6.6 × 10 <sup>-6</sup>	$2.0 \times 10^{-4}$					
1	C03	* 1.0 x 10 <sup>-7</sup>	3.1 x 10 <sup>-6</sup>					
1.3	C09	* 1.3 × 10 <sup>-7</sup>	3.9 x 10 <sup>-6</sup>					
1-3	C16	* 1.3 x 10 <sup>-7</sup>	$3.9 \times 10^{-8}$					

TABLE 5.1. WEST JEFFERSON HYDRAULIC CONDUCTIVITY RESULTS

\* Indicates best method (results) for the slug test data.

## 6.0 <u>REFERENCES</u>

Bouwer, H., 1989. "The Bouwer and Rice Slug Test - An Update", <u>Groundwater</u>, <u>27</u> (3), pp. 304-309.

Bouwer, H., and Rice, R. C., 1976. "A Slug Test for Determining Hydraulic Conductivity of Unconfined Aquifers with Completely or Partially Penetrating Wells", <u>Water Resources Research</u>, <u>12</u>, pp. 423-428.

Cooper, H. H. Jr., Bredehoeft, J. D., and Papadopulos, I. S., 1967. "Response of a Finite Diameter Well to an Instantaneous Charge of Water", <u>Water</u> <u>Resources Research</u>, <u>3</u>, pp. 263-269.

# APPENDIX 1

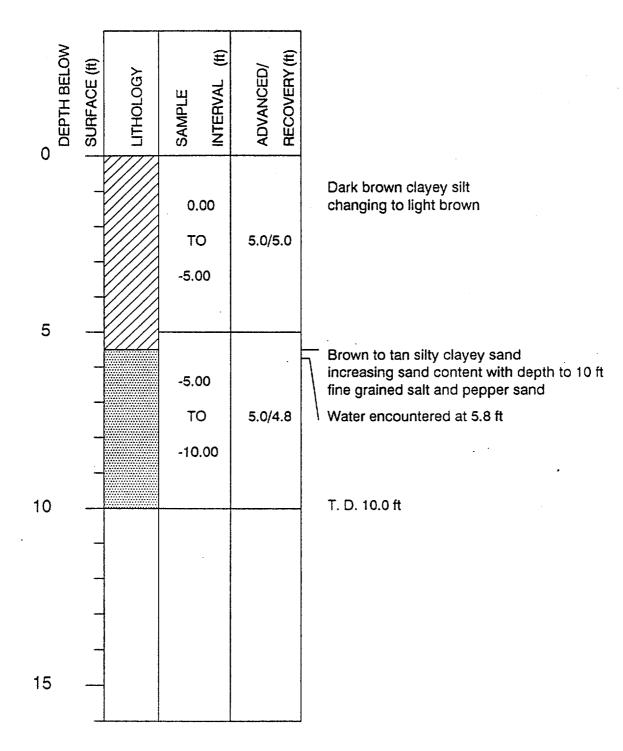
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# BORING LOGS FOR MONITORING WELLS

LOCATION: Sample Area 1

24 APRIL 1989

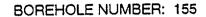


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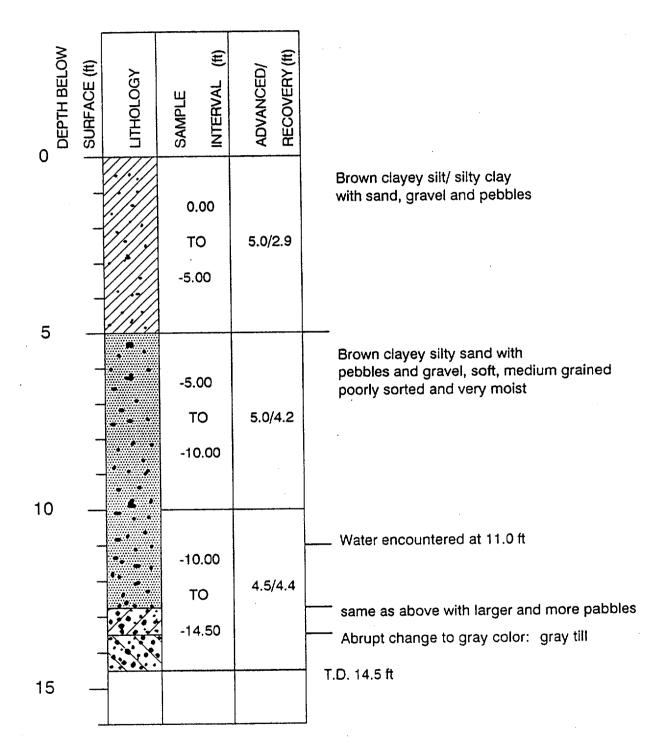
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LOCATION: Sample Area 1

27 APRIL 1989



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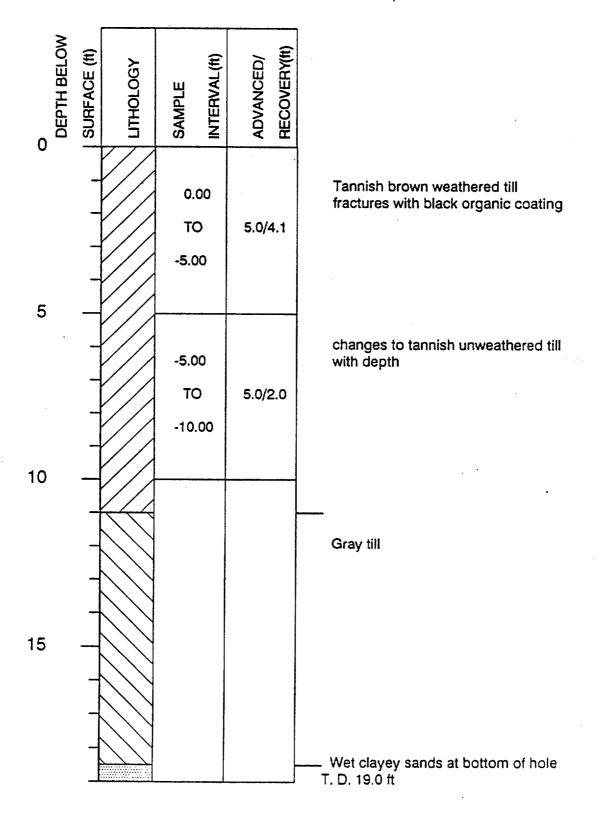
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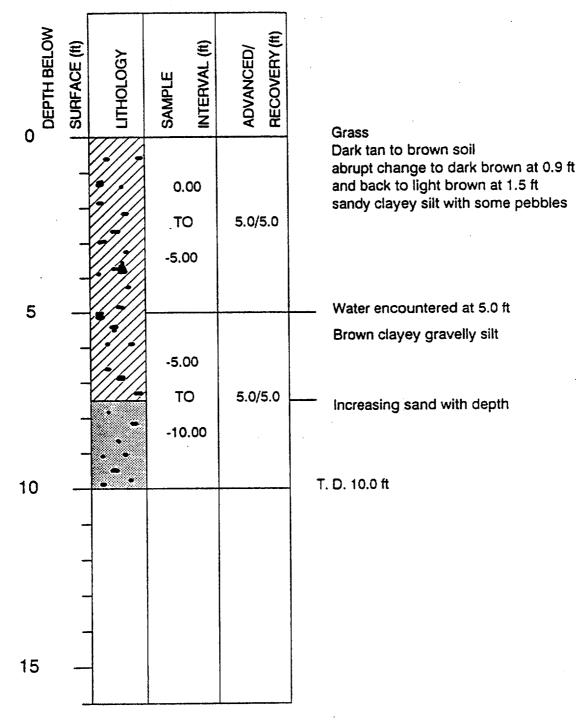
LOCATION: Sample Area 1

28 APRIL 1989



## LOCATION: Sample Area 1

25 APRIL 1989



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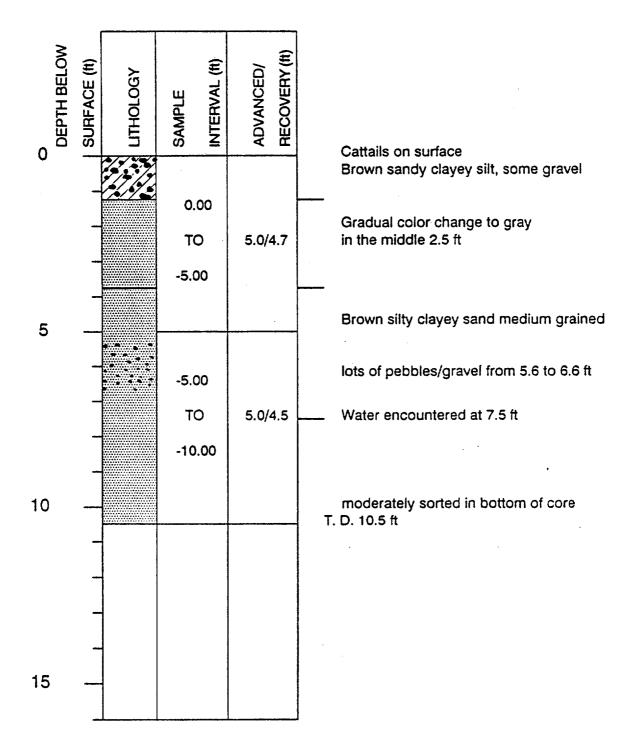
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LOCATION: Sample Area 1

27 APRIL 1989



LOCATION: Sample Area 2

26 JUNE 1989

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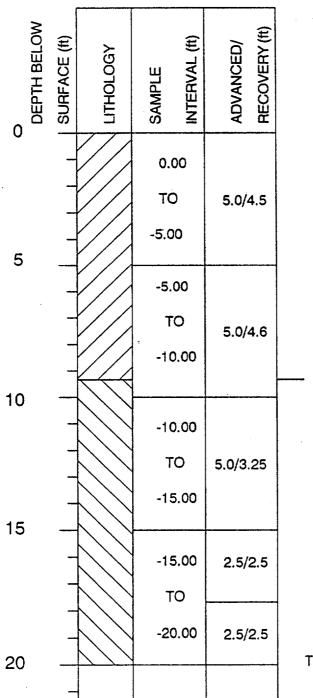
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Grass

Brown till or fill moist

Gray till moist

T. D. 20.0 ft

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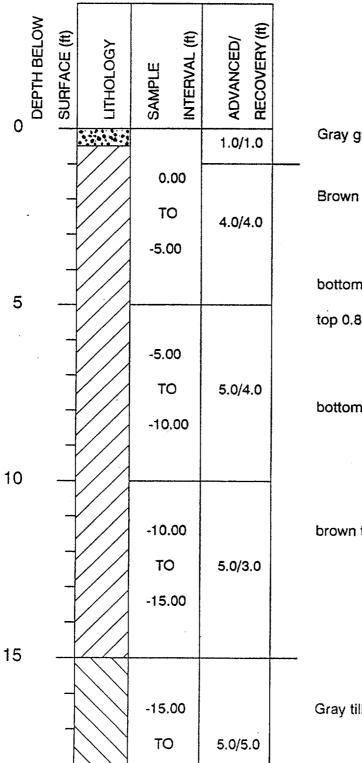
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in the second

LOCATION: Sample Area 3

29 JUNE 1989

PAGE 1 OF 2



Gray gravel 0.4 ft becoming finer with depth

Brown fill or till

bottom 1.0 ft dark brown till or fill

top 0.8 ft dark brown fill wet perched

bottom 3.2 ft brown till compact moist

brown till moist

Gray till dry

### PAGE 2 OF 2

15

1

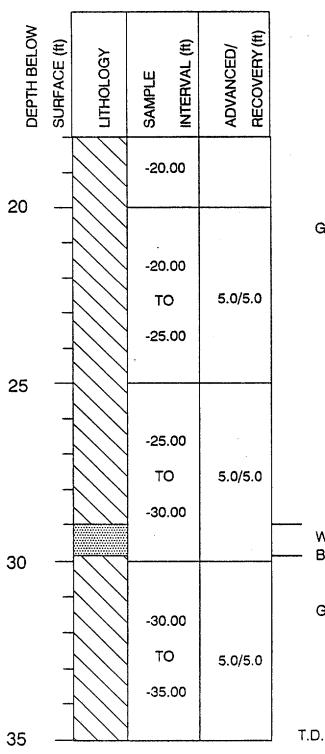
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# BOREHOLE NUMBER: 300

LOCATION: Sample Area 3

29 JUNE 1989



Gray till dry

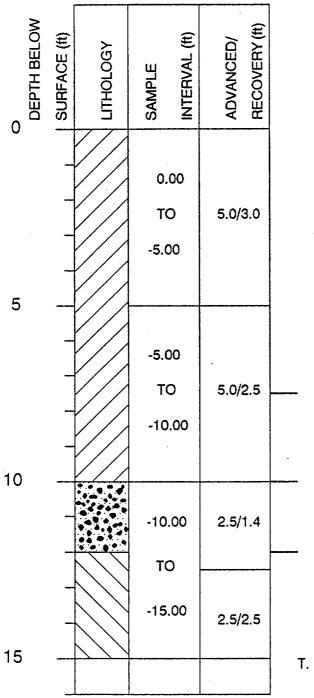
Wet sand 0.8 ft thick water table ? Bottom 0.2 ft dry gray till dense

Gray till wet

T.D. 35.0 ft

1

29 JUNE 1989



Brown fill moist

bottom 0.4 ft wet

Water encountered at about 7.5 ft brown fill wet from middle to bottom of core

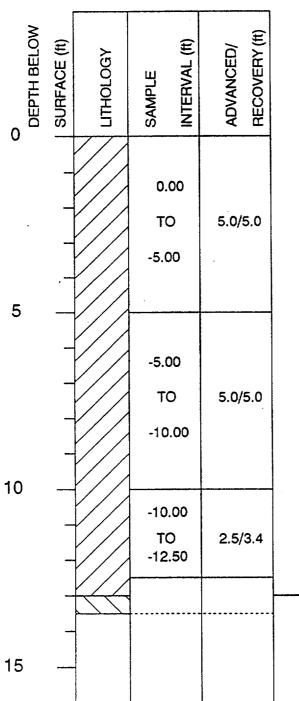
Wet gravel

Gray till wet

bottom 0.5 ft moist T. D. 15.0 ft

LOCATION: Sample Area 3

29 JUNE 1989



Brown till or fill moist

brown till or fill with water in some voids

Gray till in bottom 0.4 ft of barrel Possible T.D. 13.4 ft drilling only to 12.5 ft but core expanded a bit

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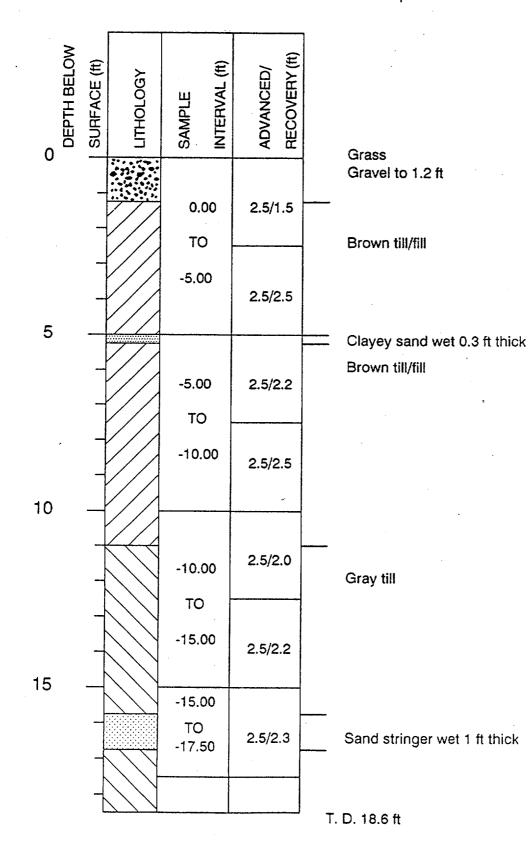
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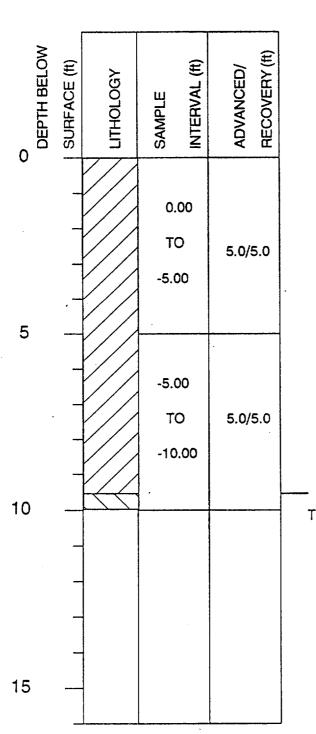
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LOCATION: Sample Area 4

2 AUGUST 1989





Grass Brown till silty clay with gravel moist

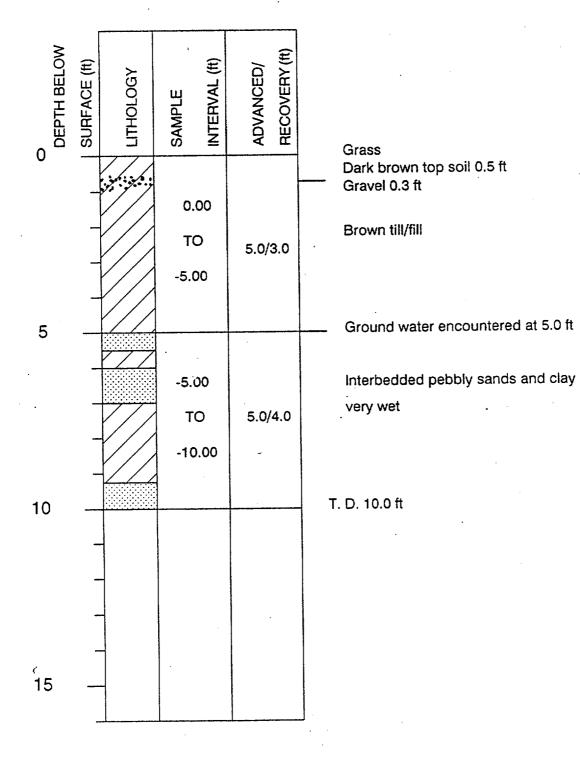
brown till

Gray till bottom 0.4 ft silty clay with gravel T. D. 10.0 ft

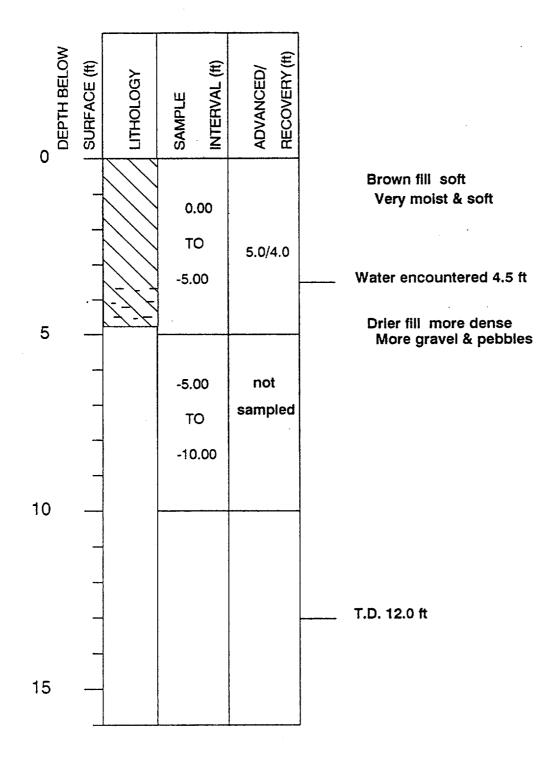
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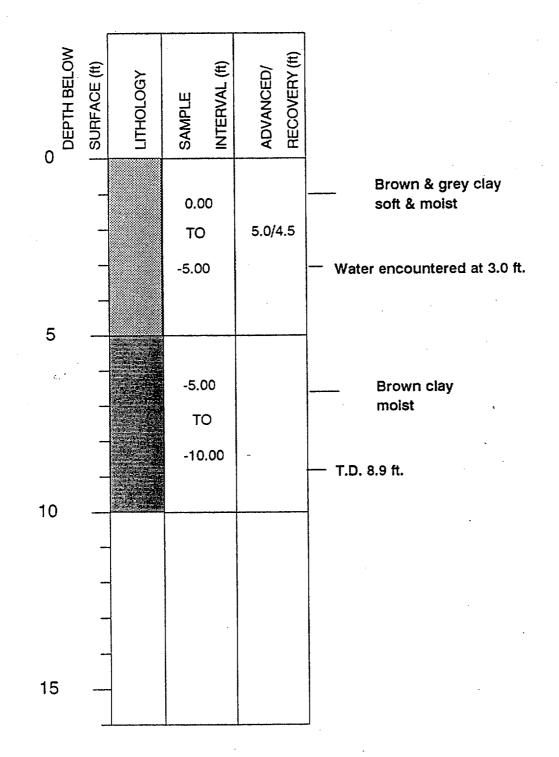
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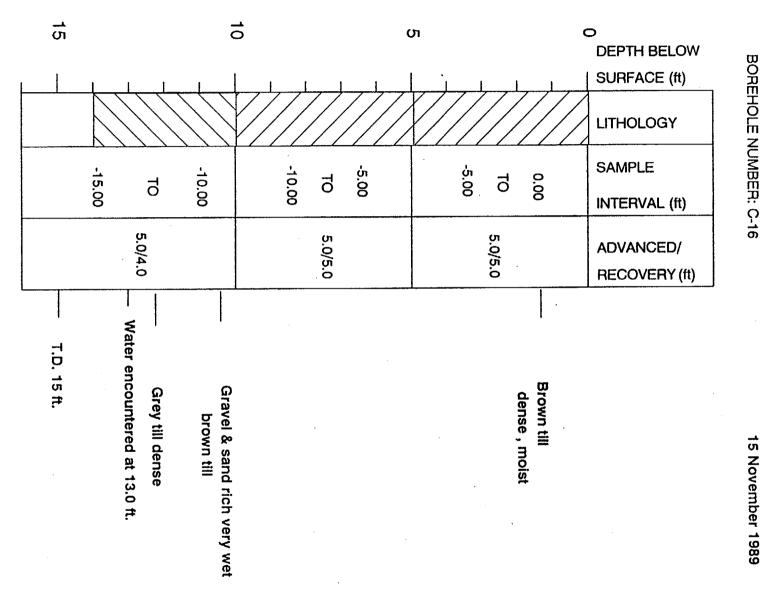


17 November 1989



17 November 1989





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15 November 1989

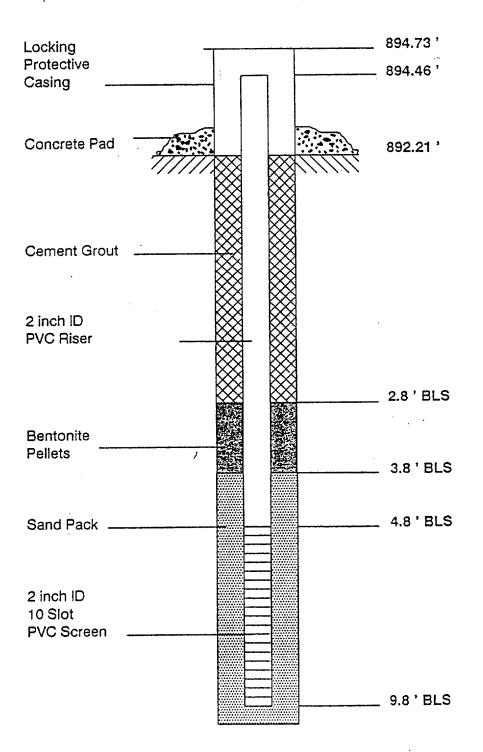
# APPENDIX 2

# MONITORING WELL CONSTRUCTION DIAGRAMS

BOREHOLE NUMBER: 150 LOCATION: Sample Area 1 25 April 1989

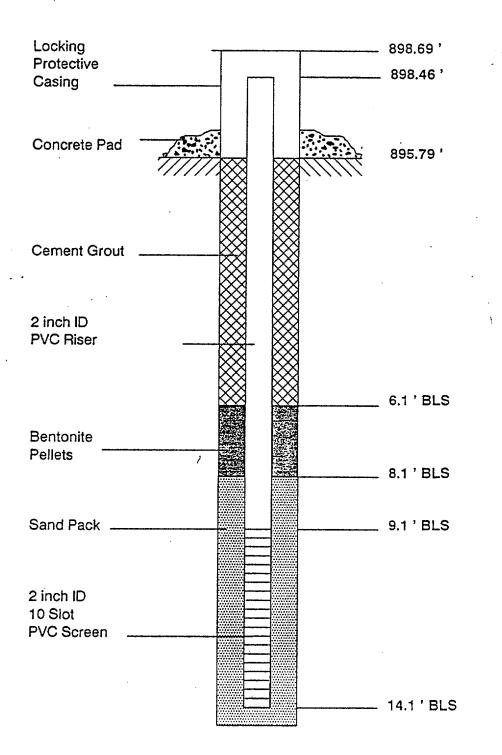
Total Depth 10.0'

Borehole Diameter 0.7 '



Total Depth 14.5'

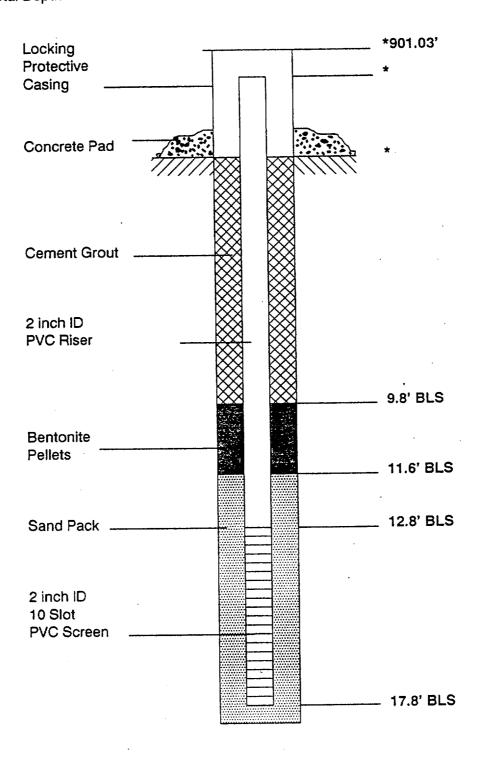
Borehole Diameter 0.7 '



# BOREHOLE NUMBER: 158 LOCATION:Sample Area 1 28 April 1989

Total Depth 19.0'

Borehole Diameter 0.7'

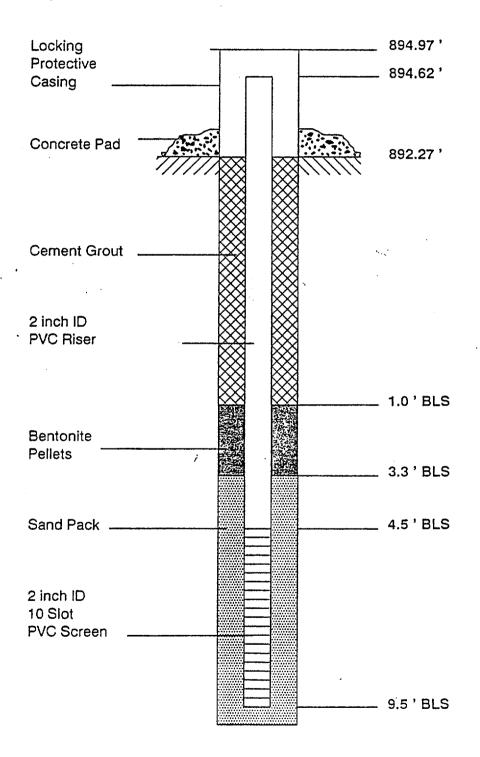


\* The well has been damaged, and the elevations are approximate.

BOREHOLE NUMBER: 168 LOCATION: Sample Area 1 25 April 1989

Total Depth 10.0 '

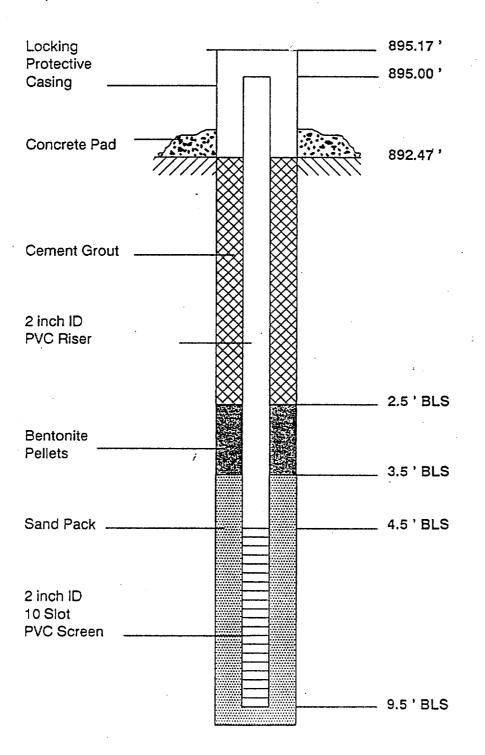
Borehole Diameter 0.7 '



# BOREHOLE NUMBER: 172 LOCATION: Sample Area 1 27 April 1989

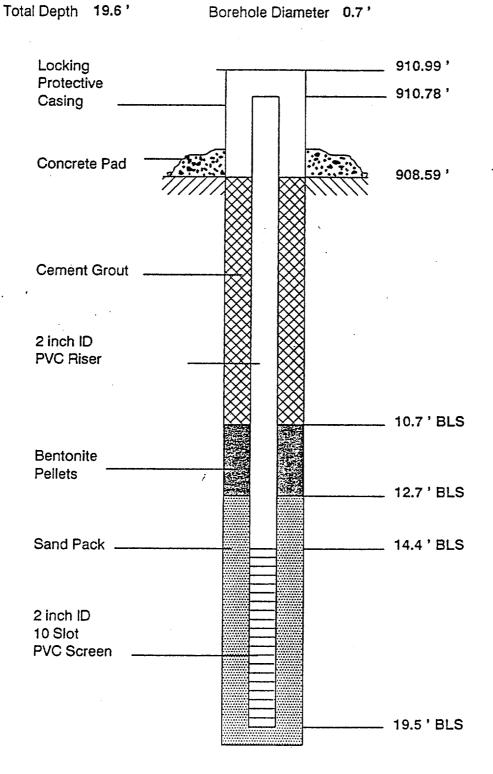
Total Depth 10.5 '

Borehole Diameter 0.7 '



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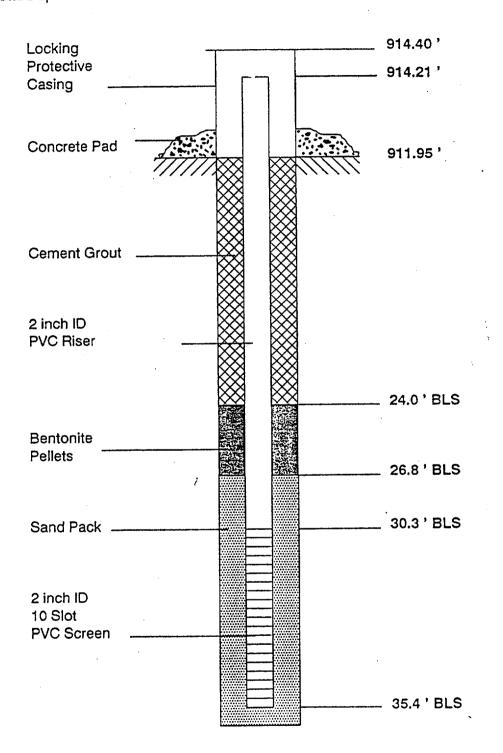
Borehole Diameter 0.7 '



BOREHOLE NUMBER: 300 LOCATION: Sample Area 3 30 June 1989

Total Depth 35.5 '

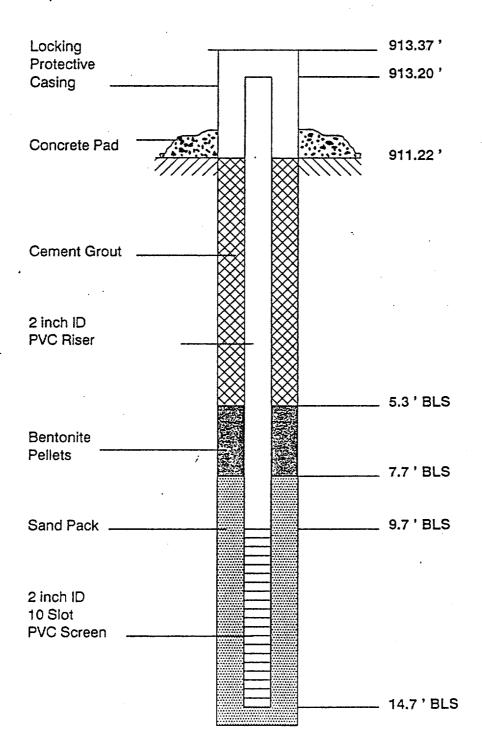
Borehole Diameter 0.7 '



BOREHOLE NUMBER: 306 LOCATION: Sample Area 3 29 June 1989

Total Depth 15.0'

Borehole Diameter 0.6 '



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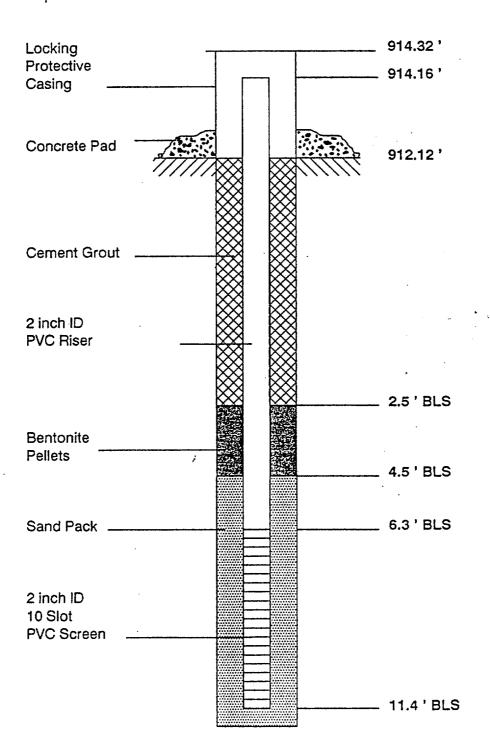
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BOREHOLE NUMBER 312 LOCATION: Sample Area 3 29 June 1989

Total Depth 12.5 '

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Borehole Diameter 0.6 '

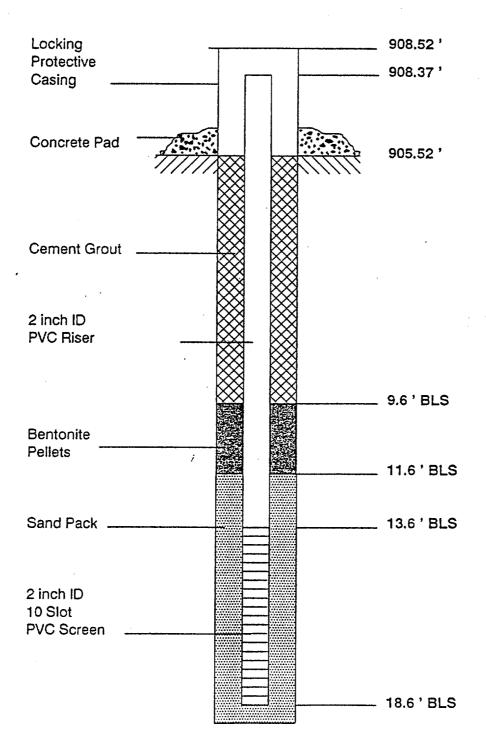


# BOREHOLE NUMBER: 403 LOCATION: Sample Area 4 03 August 1989

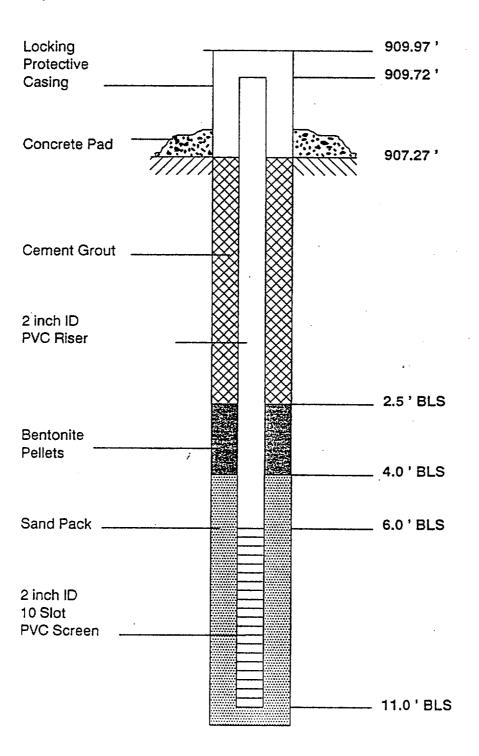
I

Total Depth 18.6 '

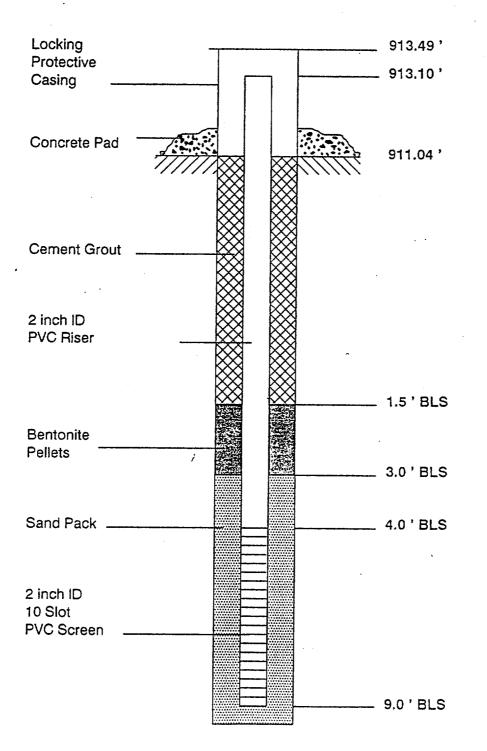
Borehole Diameter 0.8 '



Total Depth 11.0'

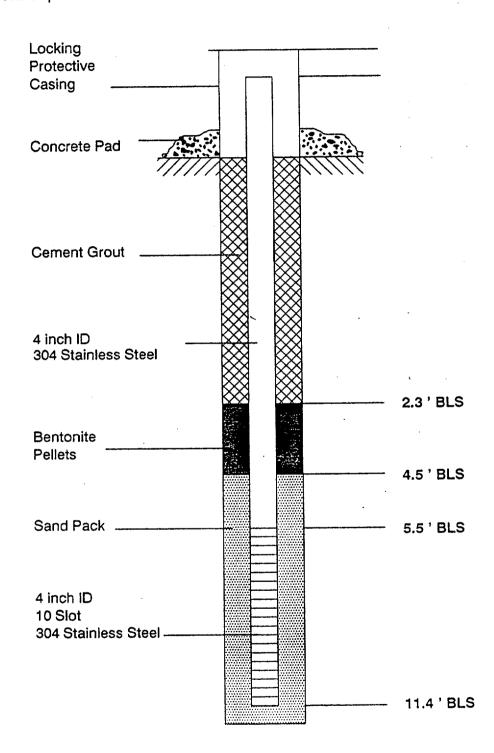


Total Depth 10.0'



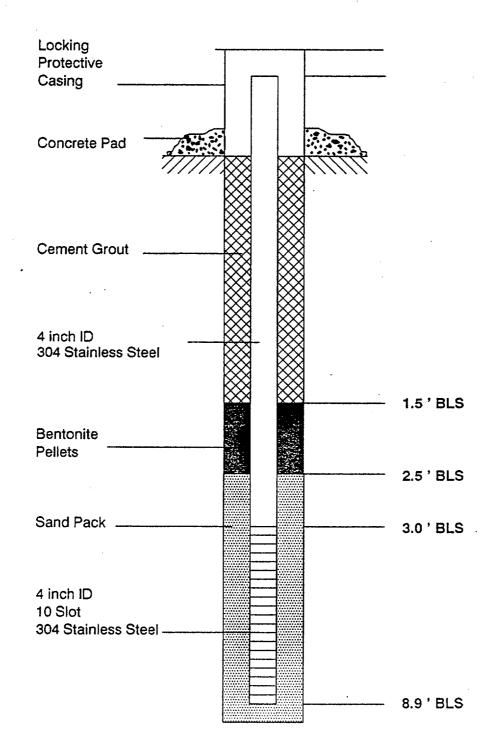
### BOREHOLE NUMBER: C-03 LOCATION: Behind JN-4 17 November 1989

Total Depth 11.4 '



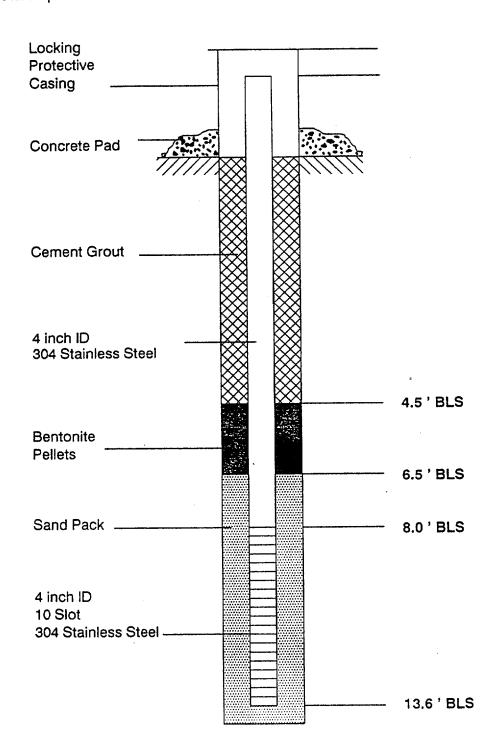
### BOREHOLE NUMBER: C-09 LOCATION: Sewer Outfall 17 November 1989

Total Depth 8.9 '



# BOREHOLE NUMBER: C-16 LOCATION: South of JN-2 17 November 1989

Total Depth 13.6 '



# APPENDIX 3

# SLUG TEST DATA

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# HERMIT DATA FOR WELL 150 SLUG TEST

Environ	E1000B nental 10 18:	Logg 33	er
Unit# 0	0476	ſest#	0
INPUT 1:	Level	(F)	тос
Reference Scale fac Offset	tor	29.	00 98 04
Step# 0	11/10	09:	:40
Elapsed 1 0.0000 0.003 0.0060 0.009 0.013 0.0160 0.020 0.023 0.0260 0.033 0.050 0.066 0.033 0.100 0.1160 0.133 0.150 0.166 0.183 0.200 0.216 0.235 0.260 0.216 0.235 0.250 0.260 0.216 0.235 0.250 0.260 0.235 0.250 0.260 0.285 0.260 0.285 0.285 0.260 0.285 0.285 0.260 0.285 0.285 0.260 0.285 0.260 0.285 0.285 0.260 0.285	            	0 6 1 1 1 1 1 1 1 1 1 1 1 1 1	1e .07 .81 .19 .00 .69 .03 .68 .73 .74 .74 .74 .74 .74 .74 .74 .74

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Elapsed Time	Value
Elapsed Time 0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 5.5000 6.0000 5.5000 6.0000 7.5000 8.0000 8.5000 9.0000 9.5000	Value 0.83 0.78 0.72 0.69 0.64 0.60 0.56 0.53 0.51 0.47 0.44 0.42 0.39 0.36 0.26 0.18 0.14 0.10 0.08 0.06 0.04 0.00 0.04 0.00 0.02 0.02 0.02 0.01 0.01 0.01 0.01 0.01 0.01
10.0000	0.01

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SE1000B Environmental Logg 11/10 18:38	er		
Unit# 00476 Test#	3		
INPUT 1: Level (F)	тос		
Scale factor 29.	00 98 04	Elapsed Time 0.9167 1.0000	Value 1.52 1.50
Step# 0 11/10 11:	17	1.0833	1.46
Elapsed Time Valu	e	1.2500	1.40
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	15 65 99 96 13 54 87 17 05 04 04 01 99 97 95 94 92 91 90 89 87 86 85 84 83 82 80 79 79 73 70 66 62 55	1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 4.5000 5.0000 5.5000 6.0000 5.5000 7.0000 7.5000 8.0000 9.0000 9.5000 10.0000 12.0000 12.0000 12.0000 12.0000 20.0000 20.00000 20.0000 20.0000 20.0000	$\begin{array}{c} 1.38\\ 1.36\\ 1.33\\ 1.31\\ 1.28\\ 1.26\\ 1.23\\ 1.21\\ 1.20\\ 1.09\\ 0.99\\ 0.90\\ 0.84\\ 0.77\\ 0.70\\ 0.66\\ 0.61\\ 0.57\\ 0.53\\ 0.50\\ 0.47\\ 0.44\\ 0.41\\ 0.39\\ 0.36\\ 0.29\\ 0.25\\ 0.21\\ 0.18\\ 0.17\\ 0.15\\ 0.13\\ 0.12\\$

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# HERMIT DATA FOR WELL 158 SLUG TEST

SE1000B Environmental Log 11/10 18:39	gger	Elapsed Time	Value
Unit# 00476 Test	t# 4	0.9167 1.0000	1.94 1.94
INPUT 1: Level (F)	) ТОС	1.0833 1.1667	1.94 1.94
Scale factor 2	0.00 9.98 0.04	1.2500 1.3333 1.4166 1.5000	1.93 1.93 1.93 1.93
Step# 0 11/10 13	2:14	1.5833 1.6667	1.93 1.92
Elapsed Time Va	lue	1.7500	1.92
0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667	3.14 3.83 3.56 1.36 0.72 1.78 2.61 2.49 1.92 1.70 1.93 1.99 2.00 2.01 2.00 1.99 1.99 1.98 1.98 1.98 1.98 1.98 1.98 1.98 1.98 1.97 1.95	$\begin{array}{c} 1.9167\\ 2.0000\\ 2.5000\\ 3.0000\\ 3.5000\\ 4.0000\\ 4.5000\\ 5.5000\\ 5.5000\\ 6.0000\\ 6.5000\\ 7.0000\\ 7.5000\\ 8.0000\\ 9.0000\\ 9.5000\\ 10.0000\\ 9.5000\\ 10.0000\\ 12.0000\\ 14.0000\\ 14.0000\\ 16.0000\\ 18.0000\\ 20.0000\\ 22.0000\\ 24.0000\\ 26.0000\\ 28.0000\\ 30.0000\\ 30.0000\\ 34.0000\\ 36.0000\\ 38.0000\\ 40.0000\\ 38.0000\\ 40.0000\\ 38.0000\\ 40.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0000\\ 30.0000\\ 38.0$	$\begin{array}{c} 1.92\\ 1.92\\ 1.90\\ 1.90\\ 1.90\\ 1.89\\ 1.88\\ 1.87\\ 1.86\\ 1.85\\ 1.84\\ 1.85\\ 1.84\\ 1.82\\ 1.82\\ 1.82\\ 1.82\\ 1.81\\ 1.76\\ 1.74\\ 1.73\\ 1.71\\ 1.69\\ 1.67\\ 1.65\\ 1.64\\ 1.62\\ 1.61\\ 1.60\\ 1.59\\ 1.55\\$
0.3333 0.4167 0.5000 0.5833	1.96 1.96 1.96 1.95	34.0000 36.0000 38.0000	1.60 1.59 1.57

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# HERMIT DATA FOR WELL 158 (Continued)

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Elapsed Time	Valúe
46.0000	1.53
48.0000 50.0000	1.51 1.49
52.0000	1.48
54.0000	1.47
56.0000 58.0000	1.45 1.44
60.0000	1.44
62.0000	1.42
64.0000 66.0000	$1.41 \\ 1.40$
68.0000	1.40
70.0000	1.38
72.0000 74.0000	1.38
76.0000	1.36
78.0000	1.35
80.0000' 82.0000	1.34 1.33
84.0000	1.33
86.0000	1.32
88.0000 90.0000	1.31
92.0000	1.29
94.0000	1.28
96.0000 98.0000	1.27 1.27
100.000	1.26
110.000 120.000	1.21
130.000	1.19
140.000	1.11
150.000	1.09

# HERMIT DATA FOR WELL 168 SLUG TEST

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Elapsed Time	Value
$\begin{array}{c} 0.9167\\ 1.0000\\ 1.0833\\ 1.1667\\ 1.2500\\ 1.3333\\ 1.4166\\ 1.5000\\ 1.5833\\ 1.4166\\ 1.5000\\ 1.5833\\ 1.6667\\ 1.7500\\ 1.8333\\ 1.9167\\ 2.0000\\ 2.5000\\ 3.0000\\ 3.0000\\ 3.5000\\ 4.0000\\ 4.5000\\ 5.5000\\ 5.5000\\ 5.5000\\ 5.5000\\ 6.0000\\ 5.5000\\ 5.5000\\ 8.0000\\ 8.5000\\ 9.0000\\ 9.5000\\ 10.0000\\ \end{array}$	0.07 0.06 0.05 0.04 0.04 0.04 0.03 0.01
END	

HERMIT DATA FOR WELL 172 SLUG TEST

-----0.37

0.34 0.30

0.27 0.25 0.23 0.21 0.19 0.17

0.17 0.15 0.15

0.13 0.12 0.08 0.06 0.05

0.03 0.03 0.02

0.02 0.01 0.01 0.00

0.00 0.00

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SE1000 Environmenta 11/10 18			
Unit# 00476	Test# 2		
INPUT 1: Leve	1 (F) TOC		
Reference Scale factor Offset	0.00 29.98 0.04		
Step# 0 11/1	0 10:28		
Elapsed Time	Value	Elapsed Time	Value
0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333	6.95 -0.04 1.67 1.72 1.72 1.72 1.72 1.65 1.62 1.60 1.58 1.55 1.49 1.42 1.38 1.23 1.20 1.16 1.12 1.08 1.05 1.03 1.00 0.96 0.93 0.91 0.88 0.86 0.75 0.66 0.58 0.51 0.46 0.41	0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 5.5000 6.0000 5.5000 6.0000 5.5000 8.0000 8.5000 9.0000 9.5000 10.0000 END	$\begin{array}{c} 0.37\\ 0.34\\ 0.30\\ 0.27\\ 0.25\\ 0.23\\ 0.21\\ 0.19\\ 0.17\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.15\\ 0.08\\ 0.06\\ 0.05\\ 0.03\\ 0.02\\ 0.02\\ 0.01\\ 0.00\\$

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# HERMIT DATA FOR WELL 206 SLUG TEST

SE1000B Environmental Logger 12/19 17:28	
Unit# 00476	
INPUT 1: Level (F) TOO	2
Reference0.00Scale factor29.98Offset0.04	
Step# 0 12/19 11:32	
Elapsed TimeValue0.00000.990.00334.490.00664.740.00990.340.0133- 0.320.01662.600.02003.140.02331.220.02660.560.03001.920.03332.450.05001.850.06661.600.08331.550.10001.560.11661.580.13331.600.15001.600.16661.600.18331.590.20001.590.21661.590.23331.590.25001.580.30001.580.31661.580.31661.580.31661.580.31661.570.50001.570.58331.570.66671.570.75001.570.83331.57	

Elapsed Time	Value
Elapsed Time 0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000 3.0000 3.5000 4.0000 4.5000 5.5000 6.0000 5.5000 6.0000 5.5000 8.0000 9.5000 10.0000 12.0000 10.0000 12.000	Value 1.56 1.56 1.56 1.56 1.56 1.56 1.56 1.56 1.56 1.56 1.55

# HERMIT DATA FOR WELL 206 (Continued)

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Elapsed Time	Value
46.0000	1.53
48.0000	1.53
50.0000	1.53
52.0000	1.53
54.0000	1.53
56.0000 58.0000	1.53 1.53
60.0000	1.53
62.0000	1.52
64.0000	1.52
66.0000	1.52
68.0000	1.52
70.0000	1.52
72.0000	1.52
74.0000 76.0000	1.52
78.0000	1.52
80.0000	1.52
82.0000	1.52
84.0000	1.52
86.0000	1.51
88.0000	1.51
90.0000 92.0000	1.51
94.0000	1.51
96.0000	1.51
98.0000	1.51
100.000	1.51
110.000	1.50
120.000	1.50
130.000 140.000	1.50 1.49
140.000	1.49

### HERMIT DATA FOR WELL 300 SLUG TEST

SE10 Environmen 12/19		gger
Unit# 0047	6 Test	t# 7
INPUT 1: Le	evel (F)	) TOC
Reference Scale factor Offset	· 29	0.00 9.98 0.04
Step# 0 12	2/19 1	5:10
Elapsed Time	e Va	lue
0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333		0.86 5.37 3.43 2.59 4.06 1.41 1.42 1.82 1.50 1.54 1.51 1.36 1.24 1.15 1.06 1.00 0.94 0.89 0.85 0.81 0.77 0.74 0.71 0.69 0.68 0.66 0.64 0.62 0.61 0.55 0.51 0.49 0.43

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# HERMIT DATA FOR WELL 300 (Continued)

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Elapsed Time	Value
***	
46.0000	0.25
48.0000	0.25
50.0000	0.24
52.0000	0.24
54.0000	0.25
56.0000	0.24
58.0000	0.25
60.0000	0.25
62.0000	0.25
64.0000	+ · +
	0.24
66.0000	0.25
68.0000	0.25
70.0000	0.25
72.0000	0.25
74.0000	0.25
76.0000	0.25
78.0000	0.25
80.0000	0.25
82.0000 <sup>,</sup>	0.25
84.0000	0.25
86.0000	0.25
CUD	

#### HERMIT DATA FOR WELL 306 SLUG TEST

$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	SE1000B Environmental 11/10 18		Elapsed Time	Value
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	Unit# 00476	°est# 5		0.89
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	INPUT 1: Level	(F) TOC	1.0833	0.83
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	Scale factor	29.98	1.2500 1.3333 1.4166	0.76 0.72 0.69
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	Step# 0 11/10	16:01	1.5833	0.65
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	Elapsed Time	Value	1.7500	0.60
0.28331.3426.00000.100.30001.3228.00000.100.31661.3030.00000.100.33331.2832.00000.130.41671.2134.00000.110.50001.1436.00000.11	0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667	2.50 6.28 2.26 0.53 3.71 0.86 1.84 2.44 1.15 2.18 2.46 2.32 2.38 1.53 1.55 1.55 1.55 1.55 1.55 1.55 1.55	$\begin{array}{c} 1.8333\\ 1.9167\\ 2.0000\\ 2.5000\\ 3.0000\\ 3.5000\\ 4.0000\\ 4.5000\\ 5.0000\\ 5.5000\\ 6.0000\\ 5.5000\\ 6.0000\\ 6.5000\\ 7.0000\\ 7.5000\\ 8.0000\\ 8.5000\\ 9.0000\\ 9.5000\\ 10.0000\\ 9.5000\\ 10.0000\\ 12.0000\\ 14.0000\\ 14.0000\\ 16.0000\\ 18.0000\\ 20.0000\\ 22.0000\\ 24.0000\\ 26.0000\\ 28.0000\\ 30.0000\\ 32.0000\\ 34.0000\\ 36.0000\\ 38.0000\\ 40.0000\\ \end{array}$	0.56 0.54 0.38 0.34 0.31 0.28 0.26 0.24 0.23 0.22 0.21 0.20 0.18 0.17 0.17 0.16 0.15 0.13 0.12 0.11 0.10 0.11 0.10 0.11 0.10 0.13 0.11 0.08

# HERMIT DATA FOR WELL 306 (Continued)

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Elapsed Time	Value
46.0000	0.09
48.0000	0.08
50.0000	0.08
52.0000	0.09
54.0000	0.08
56.0000	0.08
58.0000	0.09
60.0000	0.08
62.0000	0.08
64.0000	0.08
66.0000	0.08
68.0000	0.08
70.0000	0.08
72.0000	0.08
74.0000	0.09
76.0000	0.08
78.0000	0.08
80.0000	0.08
END	

#### HERMIT DATA FOR WELL 312 SLUG TEST

Envi	ironn	1000B mental .9 17:		jer	
Unit	t# 00	476 -	Γest#	7 ₹	
INPUT	2:	Level	(F)	тос	
Refere Scale Offset	ence fact t	or	0 19 0	.00 .98 .09	
Step#	0	12/19	15:	10	
0.0 0.0 0.0 0.0 0.0	0000 0033 0056 0099 0133 0166	me 	- 0. 1. 5. 2. 1. 1.	.02 .98 .07 .09 .53 .87	
0.0 0.0 0.0 0.0 0.0 0.0	)200 )233 )266 )300 )333 )500 )666 )833 1000 1166			.49 .81 .56 .69 .61 .62 .63 .62 .63 .62 .60	. /
	1333 1500 1666 1833 2000 2166 2333 2500 2666 2833 3000 3166 3333 1167 5000 5833 5667 7500			.60 .60 .60 .60 .59 .59 .59 .58 .58 .58 .58 .58 .58 .58 .58 .58 .58	
	3333			57	

Elapsed Time	Value
0.9167	1.57
1.0000	1.57
1.0833	1.56
1.1667	1.56
1.2500	1.56
1.3333	1.56
1.4166 1.5000	1.56
1.5833	1.50
1.6667	1.56
1.7500	1.56
1.8333	1.56
1.9167	1.56
2.0000 2.5000 3.0000	1.55
3.0000	1.55
3.5000	1.54
4.0000	1.54
4.5000	1.54
5.0000 5.5000	1.54
6.0000	1.54 1.53
6.5000	1.53
7.0000	1.53
7.5000	1.53
8.0000 8.5000	1.53 1.53
9.0000	1.53
9.5000	1.54
10.0000	1.56
12.0000	1.56
14.0000	1.59
16.0000 18.0000	1.60 1.59
20.0000	1.60
22.0000	1.60
24.0000	1.59
26.0000	1.59
28.0000	1.58
30.0000 32.0000	1.55
34.0000	1.50
36.0000	1.55
38.0000	1.56
40.0000	1.56
42.0000	1.57
44.0000	1.57

# HERMIT DATA FOR WELL 312 (Continued)

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Elapsed Time	Value
46 0000	
46.0000 48.0000	1.57
	1.59
50.0000	1.57
52.0000	1.55
54.0000	1.54
56.0000	1.54
58.0000	1.55
60.0000	1.56
62.0000	1.56
64.0000	1.56
66.0000	1.55
68.0000	1.56
70.0000	1.56
72.0000	1.56
74.0000	1.54
76.0000	1.53
78.0000	1.53
80.0000 <sup>,</sup>	1.52
82.0000	1.52
84.0000	1.53
86.0000	1.52
END	

#### HERMIT DATA FOR WELL 403 SLUG TEST

SE1000B Environmental Logger 11/10 18:47	Elapsed Time	Value
Environmental Logger 11/10 18:47 Unit# 00476 Test# 5 INPUT 2: Level (F) TOC Reference 0.00 Scale factor 19.98 Offset 0.09 Step# 0 11/10 16:01 Elapsed Time Value 0.0000 0.00 0.0033 2.70 0.0066 7.04 0.0099 7.37 0.0133 5.96 0.0166 4.37 0.0200 - 0.75 0.0233 3.45 0.0266 2.14 0.0300 1.14 0.0333 2.69	$\begin{array}{c} 0.9167\\ 1.0000\\ 1.0833\\ 1.1667\\ 1.2500\\ 1.3333\\ 1.4166\\ 1.5000\\ 1.5833\\ 1.6667\\ 1.7500\\ 1.8333\\ 1.6667\\ 1.7500\\ 1.8333\\ 1.9167\\ 2.0000\\ 2.5000\\ 3.0000\\ 3.5000\\ 4.0000\\ 4.5000\\ 5.5000\\ 6.0000\\ 6.5000\\ \end{array}$	$\begin{array}{c} 1.60\\ 1.57\\ 1.55\\ 1.53\\ 1.50\\ 1.48\\ 1.46\\ 1.44\\ 1.42\\ 1.40\\ 1.38\\ 1.36\\ 1.34\\ 1.32\\ 1.23\\ 1.15\\ 1.08\\ 1.01\\ 0.95\\ 0.89\\ 0.85\\ 0.80\\ 0.77\end{array}$
$\begin{array}{cccccccccccccccccccccccccccccccccccc$	7.0000 7.5000 8.0000 8.5000 9.0000 9.5000 10.0000 12.0000 14.0000 16.0000 20.0000 22.0000 24.0000 26.0000 28.0000 30.0000 32.0000 34.0000 34.0000 38.0000 40.0000 42.0000 42.0000	0.74 0.72 0.68 0.65 0.65 0.59 0.55 0.52 0.51 0.49 0.49 0.49 0.49 0.50 0.49 0.50 0.51 0.52 0.51 0.52 0.49 0.50 0.51 0.52 0.49 0.52 0.51 0.52 0.52 0.53 0.53 0.50 0.53 0.50 0.52 0.50 0.52 0.50 0.52 0.50 0.52 0.50 0.50 0.50 0.51 0.52 0.50 0.52 0.50 0.52 0.50 0.50 0.50 0.50 0.52 0.50

# HERMIT DATA FOR WELL 403 (Continued)

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Elapsed Time	Value
46.0000	0.37
48.0000	0.35
50.0000	0.35
52.0000	0.35
54.0000	0.34
56.0000	0.34
58.0000	0.35
60.0000	0.35
62.0000	0.35
64.0000	0.35
66.0000	0.35
68.0000	0.35
70.0000	0.36
72.0000	0.37
74.0000	0.37
76.0000	0.37
78.0000	0.37
80.0000.	0.37
THE	

# HERMIT DATA FOR WELL 506 SLUG TEST

Environ	E1000B mental 19 17:	Logger 30	
Unit# 0	0476 T	est# 6	
INPUT 2:	Level	(F) TOC	
Reference Scale fac Offset	tor	0.00 19.98 0.09	
Step# 0	12/19	11:32	
Elapsed T 0.0000 0.0033 0.0066 0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500 0.1666 0.1833 0.2000 0.2166 0.2833 0.2500 0.2666 0.3333 0.2500 0.2666 0.3333 0.2500 0.2666 0.3333 0.2500 0.2666 0.3333 0.2500 0.2667 0.5833 0.6667 0.8333 0.6667 0.8333 0.8		Value 5.65 4.34 1.58 1.49 2.10 1.42 1.80 1.61 1.62 1.00 1.15 1.20 1.05 1.03 1.03 1.02 1.00	

# HERMIT DATA FOR WELL 506 (Continued)

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Elapsed Time	Value
46.0000	0.24
48.0000 50.0000	0.23 0.23
52.0000	0.23
54.0000 56.0000	0.22 0.21
58.0000	0.21
60.0000	0.19
62.0000 64.0000	0.17 0.15
66.0000	0.14
68.0000 70.0000	0.13 0.13
72.0000	0.13
74.0000 76.0000	0.13 0.11
78.0000	0.11
80.0000	0.11
82.0000 84.0000	0.12
86.0000	0.11
88.0000 90.0000	0.09 0.09
92.0000	0.08
94.0000	0.08 0.09
98.0000	0.10
100.000 110.000	0.10 0.07
120.000	0.07
130.000 140.000	0.06
170.000	0.03

#### HERMIT DATA FOR WELL 601 SLUG TEST

Value 0.30

0.30 0.29 0.28

0.28 0.27 0.27 0.26 0.25 0.24 0.22

0.21 0.20 0.20 0.17 0.15 0.14 0.12 0.12 0.12 0.11 0.10 0.09 0.09 0.08 0.08 0.07

0.07 0.08 0.07 0.06 0.06 0.06 0.06 0.05

#### HERMIT DATA FOR WELL CO3 SLUG TEST

SE1000B Environmental Logger 01/16 18:03 Elaps Unit# 00476 Test# 0 INPUT 1: Level (F) TOC Reference 0.00 Scale factor 29.98 Offset 0.04 Step# 0 01/12 14:04 Elapsed Time Value -----0.0000 0.02 0.0033 0.01 0.0066 0.04 0.0099 0.83 0.0133 1.95 0.0166 0.94 0.0200 1.90 0.0233 2.04 0.0266 1.95 0.0300 1.90 0.0333 1.85 0.0500 1.73 0.0666 1.63 0.0833 1.55 0.1000 1.46 0.1166 1.37 0.1333 1.29 0.1500 1.23 0.1666 1.16 0.1833 1.11 0.2000 1.08 0.2166 1.04 0.2333 1.03 0.2500 1.00 0.2666 0.98 0.2833 0.98 0.3000 0.99 0.3166 0.96 0.3333 0.95 0.4167 0.92 0.5000 0.89 0.5833 0.86 0.6667 0.86 0.7500 0.86 0.8333 0.85

lapsed Time	Value
0.0167	
0.9167	0.84
1.0000	0.84
1.0833	0.83
1.1667	0.82
1.2500	0.82
1.3333	0.82
1.4166	0.83
1.5000	0.81
1.5833	0.82
1.6667	0.80
1.7500	0.80
1.8333	0.81
1.9167	0.81
2 0000	0.01
2.0000	0.79
2.0000 2.5000 3.0000 3.5000	0.79
3.0000	0.78
3.5000	0.77
4.0000	0.77
4.5000	0.77
5.0000	0.76
5.5000	0.73
6.0000	0.75
6.5000	0.75
7.0000	0.75
7.5000	0.75
8.0000	0.75
8.5000	0.74
9.0000	0.74
9.5000	0.74
10.0000	0.74
12.0000	0.73
14.0000	0.74
16.0000	0.75
18.0000	0.73
20.0000	
22.0000	0.74 0.72
24.0000	0.72
26.0000	0.72
28.0000	0.72
30.0000	0.71
22 0000	0.72
32.0000	0.71
34.0000	0.71
36.0000	0.70
38.0000	0.70
40.0000	0.70
42.0000	0.69
44.0000	0.70

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# HERMIT DATA FOR WELL CO3 (Continued)

Elapsed Time	Value
46.0000	0.69
48.0000 50.0000	0.70 0.70
52.0000	0.70
54.0000	0.69
56.0000	0.69
58.0000	0.69
60.0000	0.69
62.0000 64.0000	0.69
66.0000	0.69
68.0000	0.69
70.0000	0.69
72.0000 74.0000	0.69 0.69
76.0000	0.69
78.0000	0.68
80.0000	0.69
82.0000	0.68
84.0000 86.0000	0.67 0.67
88.0000	0.67
90.0000	0.67
92.0000	0.67
94.0000 96.0000	0.66 0.68
98.0000	0.67
100.000	0.67
110.000	0.65
120.000	0.65
130.000 140.000	0.66 0.64
150.000	0.64
160.000	0.64
170.000	0.63
180.000	0.62

#### HERMIT DATA FOR WELL CO9 SLUG TEST

Value

0.84

0.84

0.83

0.82

0.82

0.81

0.81

0.80

0.80

0.79

0.79

0.79

0.79

0.79

0.77

0.75

0.74

0.73

0.72

0.71

0.70

0.69

0.69

0.69

0.68

0.67

0.67

0.66

0.65

0.65

0.62

0.60

0.58

0.56

0.54

0.52

0.51

0.49

0.48

0.46

0.45

0.43

0.42

0.41

0.39

0.38

0.37

SE1000B Environmental Logger 01/16 17:53 Elapsed Time Unit# 00476 Test# 2 0.9167 1.0000 INPUT 1: Level (F) TOC 1.0833 1.1667 Reference 0.00 1.2500 Scale factor 29.98 1.3333 Offset 0.04 1.4166 1.5000 Step# 0 01/16 16:33 1.5833 1.6667 Elapsed Time Value 1.7500 1.8333 0.0000 0.00 1.9167 0.0033 2.0000 0.01 2.5000 0.0066 0.00 0.0099 0.39 3.0000 0.0133 1.26 3.5000 4.0000 0.0166 1.22 0.0200 4.5000 0.49 0.14 0.0233 5.0000 0.0266 1.19 5.5000 0.0300 6.0000 1.70 0.0333 2.03 6.5000 7.0000 0.0500 1.72 7.5000 0.0666 1.67 0.0833 1.56 8.0000 0.1000 8.5000 1.47 0.1166 9.0000 1.37 0.1333 9.5000 1.28 0.1500 1.21 10.0000 0.1666 1.14 12.0000 0.1833 14.0000 1.10 0.2000 16.0000 1.06 0.2166 1.03 18.0000 0.2333 1.02 20.0000 0.2500 1.00 22.0000 0.2666 0.98 24.0000 0.2833 0.97 26.0000 0.3000 0.96 28.0000 0.3166 0.95 30.0000 0.3333 32.0000 0.94 0.4167 0.91 34.0000 0.5000 0.89 36.0000 0.5833 0.87 38.0000 0.6667 0.86 40.0000 0.7500 0.86 42.0000 0.8333 0.85 44.0000

# HERMIT DATA FOR WELL CO9 (Continued)

Elapsed Time	Value
46.0000	0.36
48.0000	0.35
50.0000	0.34
52.0000	0.34
54.0000	0.33
56.0000	0.32
58.0000	0.31
60.0000	0.31
62.0000	0.30
64.0000	0.29
66.0000	0.28
68.0000	0.27
70.0000	0.262
END	

#### HERMIT DATA FOR WELL C16 SLUG TEST

Value

1.83 1.83

1.83 1.82 1.82 1.82

1.82 1.82 1.82

1.82 1.82 1.82

1.82 1.82 1.82 1.81

1.82 1.81 1.81

1.81

1.81 1.81

1.80 1.80 1.80 1.81

1.80 1.80 1.80

1.81 1.80 1.79

1.79 1.78 1.77 1.78

1.77 1.77 1.77

1.76 1.76 1.75

1.75 1.74 1.73

1.74

1.73

SE100 Environment 01/11	OB al Logger 16:57	Elapsed Time
Unit# 00476 INPUT 1: Leve Reference Scale factor Offset Step# 0 01/2 Elapsed Time 0.0000 0.0033 0.0066	el (F) TOC 0.00 29.98 0.04	0.9167 1.0000 1.0833 1.1667 1.2500 1.3333 1.4166 1.5000 1.5833 1.6667 1.7500 1.8333 1.9167 2.0000 2.5000
0.0099 0.0133 0.0166 0.0200 0.0233 0.0266 0.0300 0.0333 0.0500 0.0666 0.0833 0.1000 0.1166 0.1333 0.1500	1.70 - 0.28 0.89 1.93 1.91 1.89 1.89 1.86 1.90 1.88 1.88 1.87 1.86 1.87 1.87	$\begin{array}{c} 2.5000\\ 3.0000\\ 3.5000\\ 4.0000\\ 4.5000\\ 5.0000\\ 5.5000\\ 6.0000\\ 6.5000\\ 7.0000\\ 7.5000\\ 8.0000\\ 8.5000\\ 9.0000\\ 9.5000\\ 10.0000\end{array}$
0.1666 0.1833 0.2000 0.2166 0.2333 0.2500 0.2666 0.2833 0.3000 0.3166 0.3333 0.4167 0.5000 0.5833 0.6667 0.7500 0.8333	1.88 1.86 1.87 1.87 1.86 1.86 1.86 1.86 1.86 1.86 1.85 1.85 1.85 1.85 1.85 1.83 1.83	$12.0000 \\ 14.0000 \\ 16.0000 \\ 18.0000 \\ 20.0000 \\ 22.0000 \\ 24.0000 \\ 26.0000 \\ 28.0000 \\ 30.0000 \\ 32.0000 \\ 34.0000 \\ 36.0000 \\ 38.0000 \\ 40.0000 \\ 42.0000 \\ 44.0000 \\ 44.0000 \\ 0$

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# HERMIT DATA FOR WELL C16 (Continued)

Elapsed Time	Value
46.0000 48.0000	1.73
50.0000	1.72
54.0000 56.0000	1.71
58.0000 60.0000	1.70 1.70
62.0000 64.0000	1.70
66.0000 68.0000	1.69 1.69
70.0000 72.0000	1.68
74.0000 76.0000	1.68 1.67 1.67
78.0000 80.0000 82.0000	1.67
84.0000 86.0000	1.67
88.0000 90.0000	1.66
92.0000 94.0000	1.65 1.65
96.0000 98.0000	1.64 1.64
100.000 110.000	1.65
120.000 130.000 140.000	1.59 1.57 1.56
150.000	1.56 1.55 1.53
170.000 180.000	1.52
190.000 200.000	1.50
210.000 220.000	1.46 1.47
230.000 240.000	1.45 1.43
250.000 260.000	1.42
270.000 280.000	1.40 1.39
290.000	1.38

SC-SP-004.1 Revision 1

#### QUALITY ASSURANCE DOCUMENT

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# PROCEDURE FOR WELL INSTALLATION AND WELL/BOREHOLE ABANDONMENT

BATTELLE 505 King Avenue Columbus, Ohio 43201

フ 代F 108.00 S. Nelson

September 5, 1989 Date

APPROVED BY APPROVED BY J. Ster home Michael 922 *]* ک Date Date

APPROVED BY . APPROVED BY hclin.d 22/89 С <u>9)29/25</u> Date Date

SC-SP-004.1 Revision 1 September 29, 1989 Page i of i

#### DOCUMENT REVISION RECORD

Title	Procedure for Well Installation and	No.	<u>SC-SP</u>	<u>SC-SP-004.1</u> i of		
	Well/Borehole Abandonment	Page	i	of	i	

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INDEX OF PAGE REVISIONS

Page No.	i	ii	iii				
Rev. No.	1						

Page No.	1	2	3	4	5	6	7	8	9	10
Rev. No.	1	1	1	1	1	1	1	1		

Page No.					
Rev. No.					

Page No.					
Rev. No.					

Page No.					
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#### **REVISION RECORD**

Rev. No.	00	01	02	03	04	05	06	07	08	09
Date		9/29/21								
Approval		Part								

SC-SP-004.1 Revision 1 September 29, 1989 Page 1 of 8

#### PROCEDURE FOR WELL INSTALLATION AND WELL/BOREHOLE ABANDONMENT

#### 1.0 Scope

This document describes the procedure for installing wells in boreholes and for abandoning wells or boreholes.

#### 2.0 Purpose

The purpose of this procedure is to provide a method for performing well installations in support of drilling/coring operations. The procedure is primarily in support of soil characterization work in the Nuclear Sciences Area of Battelle's West Jefferson Site; however, it may be applied to other locations where soil characterization work is being performed. The major reason for installing wells is to obtain information from subsequent measurements relating to water levels and hydraulic conductivity. A method for abandoning wells/boreholes is also provided for restoration of the land surface and safety purposes.

#### 3.0 <u>References</u>

3.1 Ohio Administrative Code 3745-9-10, Water Well Standards and Waivers.

#### 4.0 General

#### 4.1 Materials

- 4.1.1 Pipe casing, 2-inch Schedule 40 PVC, with suitable flushthreaded fittings. All connections will be flush-joint threaded.
- 4.1.2 Screen, 2-inch diameter PVC having 0.010-inch slots. The screen will be capped at the bottom.
- 4.1.3 Rounded sand or gravel, washed and bagged, with a grain-size distribution (U.S. Sieve Size) compatible with the screen and formation.

SC-SP-004.1 Revision 1 September 29, 1989 Page 2 of 8

- 4.1.4 Bentonite, granulated or pelletized.
- 4.1.5 Cement grout; nominally 74 percent Portland Class A cement, 24 percent Pozzolan cement, and 2 percent bentonite.

#### 5.0 <u>Responsibilities</u>

- 5.1 The hydrogeologist shall be responsible for the placement of wells, i.e. for determining which boreholes are to have a well installed.
- 5.2 The drilling subcontractor shall be suitably qualified in the installation of wells, as determined previously (during bid selection) by the hydrogeologist responsible for bid selection.
- 5.3 The Technical Project Manager shall determine which method to be used for each borehole.

#### 6.0 Procedure

- 6.1 Well Casing Initial Installation
  - 6.1.1 Place the screen and casing into the borehole.
    - NOTE: If borehole walls are found to be prone to slumping during well drilling, the hollow stem auger can be used as a temporary casing through which screens and casing can be run into the borehole.
  - 6.1.2 Place the sand/gravel pack (Step 4.1.3) into the casing to fill the well from the bottom of the borehole to 1 foot above the top of the screen. NOTE: If the water table is close to the land surface, the field hydrogeologist will reduce this quantity of sand/gravel pack above the screen so that no surface runoff will seep into the wells.
  - 6.1.3 Tremie bentonite (Step 4.1.4) above the sand/gravel pack, to a minimum thickness of 3 feet.
  - 6.1.4 Tremie-grout cement grout (Step 4.1.5) from above the bentonite seal to the land surface.

SC-SP-004.1 Revision 1 September 29, 1989 Page 3 of 8

- 6.2 Completion of Well
  - 6.2.1 METHOD 1
    - Casing Flush or Below the Land Surface (See Figure 1):
    - 6.2.1.1 Set the casing 2 to 3 inches below land surface, using cement.
    - 6.2.1.2 Complete the assembly with a protective steel casing, equipped with a locking lid.
    - 6.2.1.3 Install protective housing consisting of a castiron valve box assembly centered in a 3-footdiameter concrete pad sloped away from the valve box.
    - 6.2.1.4 Maintain free drainage away from the well within the valve box.
    - 6.2.1.5 Install a screw-type stainless steel cap with Teflon or Viton O-ring to prevent infiltration of surface water.
    - 6.2.1.6 Maintain a minimum of 1 foot of clearance between the casing top and the bottom of the valve box lid.
  - 6.2.2 METHOD 2

Above-Ground Surface Completion (See Figure 2):

- 6.2.2.1 Extend the well pipe approximately 2 feet above land surface.
- 6.2.2.2 If the well is located near a depression, lake, or creek with a history of flooding, install this extension (riser) higher than the flood stage.
- 6.2.2.3 Provide an aboveground stainless steel endplug or casing cap.
- 6.2.2.4 Shield the above-ground pipe with a steel casing placed over the PVC pipe.

SC-SP-004.1 Revision 1 September 29, 1989 Page 4 of 8

- 6.2.2.5 Seat all wells of this type in a 2-foot diameter by 4-inch thick concrete surface pad.
- 6.2.2.6 Slope the pad away from the well casing.
- 6.2.2.7 Install a lockable cap or lid on the steel casing.
- 6.2.2.8 If necessary (as determined by the Technical Project Manager), install 3-inch diameter steel guardposts for additional protection.
- 6.2.2.9 Install these guard posts about 5 feet high, radially from each wellhead, and recessed approximately 2 feet into the ground.
- 6.2.2.10 Paint the protective steel guard posts and clearly number the well on the lid exterior.
- 6.2.3 Provide locks for both flush and above ground well assemblies. Turn over lock kevs to the Technical Project Manager following completion of the field sampling.
- 6.2.4 Develop all groundwater monitoring wells after installation.Prior to development, monitor water levels (to the nearest 0.01 inch) with respect to an established survey point at the top of the well casing.
- 6.2.5 Details of the well installation, including exact measurements, wi11 be filled Well out the on Construction/Completion Report Sheet (DDO-125).

## 6.3 Well/Borehole Abandonment

6.3.1 Seal wells/boreholes according to the recommended procedure (Reference 4.3), using material impervious to migration of water in the hole or within the hole (i.e. grout).

SC-SP-004.1 Revision 1 September 29, 1989 Page 5 of 8

## 7.0 Records

7.1 The QA records generated by the implementation of this procedure are completed and approved form DDO-125 and copies of quality affecting and relevant information entered in Laboratory Record Books.

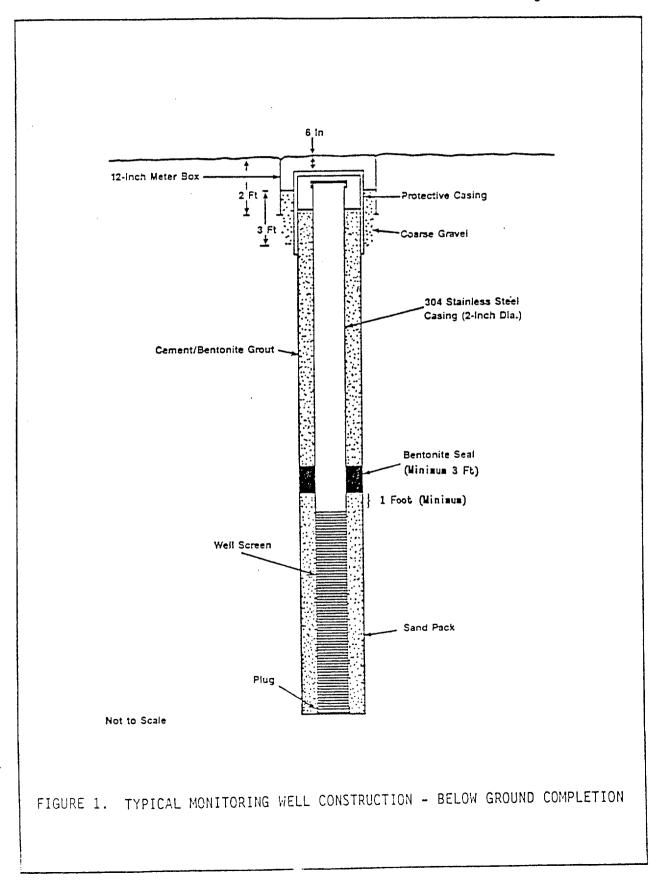
## 8.0 Figures and Forms Referenced in This Procedure

- 8.1 Figures
  - 8.1.1 Figure 1, Typical Monitoring Wall Construction Below Ground Completion
  - 8.1.2 Figure 2, Typical Wall Construction Above Ground Completion

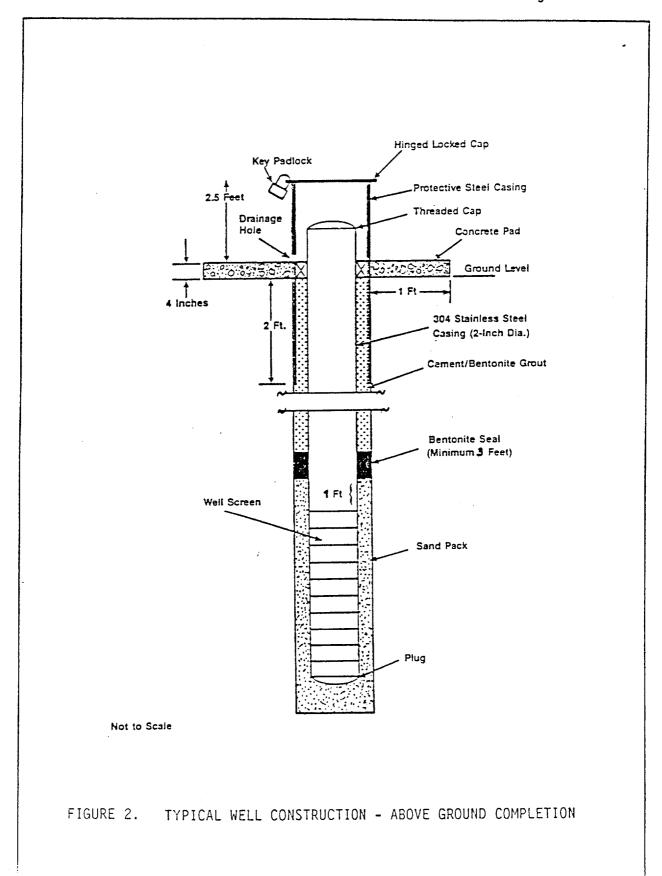
## 8.2 Forms

8.2.1 DDO-125, Well Construction/Completion Report Sheet

SC-SP-004.1 Revision 1 September 29, 1989 Page 6 of 8



SC-SP-004.1 Revision 1 September 29, 1989 Page 7 of 8



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SC-SP-004.1 Revision 1 September 29, 1989 Page 8 of 8

Name of Site:	<u>Summary of Well Constr</u>	
/ell ID Number:	Well Cap Elevation:	ft
Orilling Location: (Use Coordinates if Available) 777	Ground Surface Elevation:ft.	
Total Depth:ft.	Conductor Hole Dia.: to	ft.
Drilling Contractor:		
Type of Rig:	Surface Conductor	
Date Installation Completed:	Type:           ID:in. CD:           ID:in. CD:           to	in. ft.
<u>(Y Y M M D D)</u>	E Borehole Dia.:i	n.
Dbservations of Monitoring Zone and Further Information:	to to	ft.
	Filler Material:to	
		ft.
· · · · · · · · · · · · · · · · · · ·	Casing (with screen) Material:	
	Material: ID:in. 00: to	in. ft.
	7 A	
······································	Seal Material:	
	Seal Material: to to	ft.
	Screen Haterial:	
	ID:in. OD: to	in. ft.
	Slot Size:	
	Pack	
	Type: Size: to	ft.
Estered by:		
Entered by:,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, ,, , , _, ,, ,, ,, ,, , ,, , , , , , , , , , , , , , , , , , , ,	Approved by:, (Cate)	

J. Tholen

EM-QAP-2.0 Revision 0

## QUALITY ASSURANCE PLAN

for

#### BATTELLE GROUNDWATER MONITORING

Battelle 505 King Avenue Columbus, Ohio 43201

Prepared by

E.R. Swindall

<u>June 18, 1990</u> Date

APPROVED BY

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7/27/20 Michael J. Henhouse 8/2/90 Date M. J. Stenhouse Date

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EM-QAP-2.0 Revision O August 17, 1990 Page i of i

## DOCUMENT REVISION RECORD

Title	Quality Assurance Plan for	No.	EM-Q/	AP-2.0	<del>-</del>
	Battelle Groundwater Monitoring	Page	i	of	i

## INDEX OF PAGE REVISIONS

Page No.	i	ii	iii							
Rev. No.	0									
Page No.	1	2	3	4	5	6	7	8	9	10
Rev. No.	0	0	0	0	0	0	0	0	0	0
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Page No.							[			1
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## **REVISION RECORD**

Rev. No.	00	01	02	03	04	05	06	07	08	09
Date	8-17-90									
Approval	KSR									

DDO-009

Rev. No.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 1 of 19

#### BATTELLE GROUNDWATER MONITORING

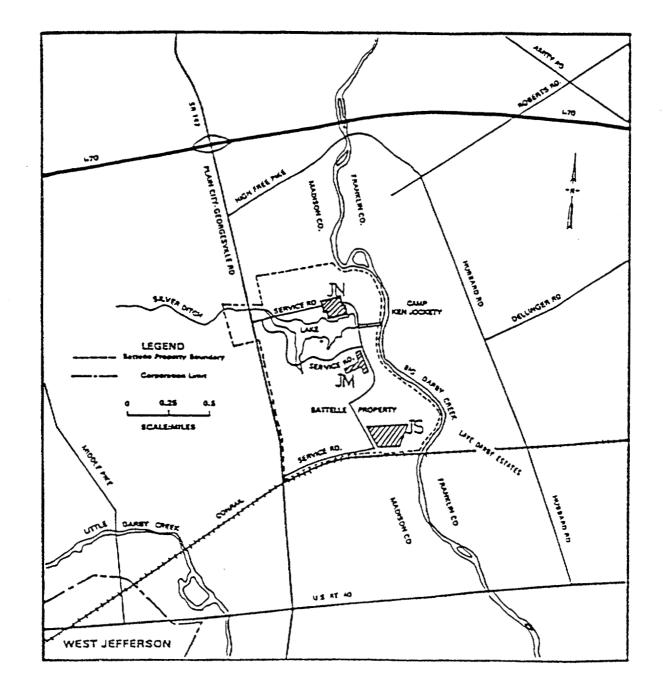
#### INTRODUCTION

This plan specifically describes the quality assurance (QA) program for conducting environmental groundwater monitoring at the Battelle West Jefferson site (Figures 1 & 2) during the two phases of the Battelle Columbus Laboratories Decommissioning Project (BCLDP): surveillance and maintenance (S&M), and decontamination and decommissioning (D&D). It will be adopted as a subtier document under the Battelle Nuclear Quality Assurance (NQA) manual for the Decommissioning and Decontamination (DDO) Group. QA procedures and documents have been and will be developed and revised to provide the necessary planning, control, documentation, and safety for all activities associated with this effort.

The groundwater monitoring program is designed and will be implemented in accordance with monitoring requirements of 40 CFR Part 264, Subpart F, and 40 CFR Part 265, Subpart F. Monitoring for radionuclides shall be in accordance with DOE Orders in the 5400 series. The Task Leader of the Environmental Monitoring Group will coordinate this effort. This plan shall be reviewed annually and updated every three years until the contract expires. This plan was developed to be responsive to the requirements of DOE Order 5400.1, Chapter III, "General Environmental Protection Program", paragraph 4.a "<u>Special Program Planning Requirements</u>, Groundwater Protection Management Program, and the requirements of the groundwater monitoring plan pursuant to Chapter IV, paragraph 9 of DOE Order 5400.1, and all applicable DOE Orders of the 5400 series in addition to ANSI/ASME NQA-1 as listed below. Each applicable criteria is discussed in the sections that follow.

Organization (1)(2) Quality Assurance Program (4) Procurement Document Control (5) Instructions, Procedures, and Drawings (6) Document Control (7) Control of Purchased Items and Services Identification and Control of Items (8)(10)Inspection (11)Test Control (12)Control of Measuring and Test Equipment (13)Handling, Storage, and Shipping Inspection, Test and Operating Status (14) (15)Control of Nonconforming Items (16)Corrective Action (17)Records (18)Audits

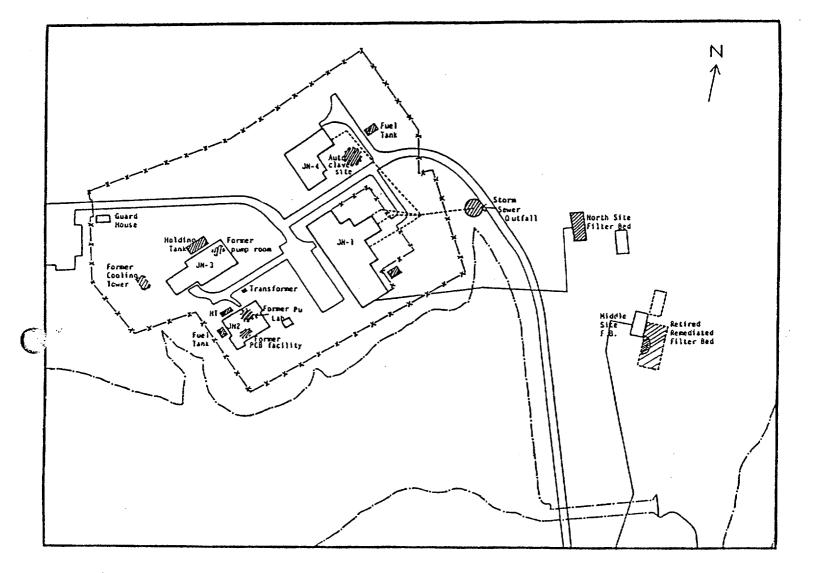
EM-QAP-2.0 Revision 0 August 17, 1990 Page 2 of 19



# FIGURE 1. LOCAL VICINITY MAP OF WEST JEFFERSON SITE

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EM-QAP-2.0 Revision 0 August 17, 1990 Page 3 of 19



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FIGURE 2. NUCLEAR SCIENCES AREA

EM-QAP-2.0 Revision 0 August 17, 1990 Page 4 of 19

Since no specific designs are being developed, and no special processes will be conducted during the sampling program, NQA-1 requirements (3) Design Control, and (9) Control of Processes and are not applicable to this project and are not addressed in this plan.

#### 1.0 Organization

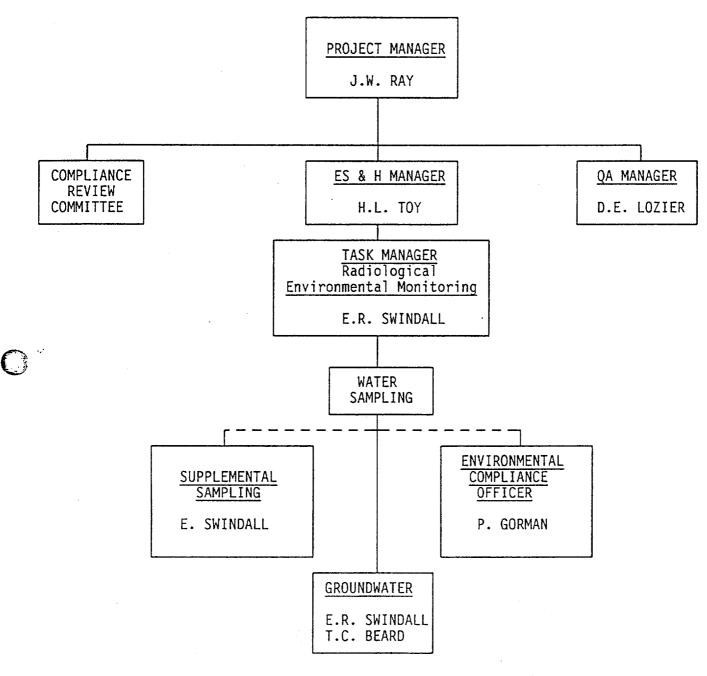
The organizational structure for Groundwater Monitoring is shown in Figure 3. The Environment, Health and Safety (ES&H) Manager, the QA Manager, and the Compliance Review Committee report directly to the BCLDP Project Manager. The Task Manager of the Radiological Environmental Monitoring Group reports directly to the ES&H Manager and is responsible for the day to day monitoring activities, equipment calibration, and review and evaluation of data generated. The Environmental Compliance Officer will assist in interpretation of data for compliance purposes.

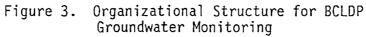
#### 2.0 Program

Battelle has done limited groundwater monitoring for radionuclides since the early 1970s. The area of concern has been an underground aquifer running in the vicinity of the nuclear fuel storage pool for the West Jefferson, JN-1 facility. Two monitoring wells were installed at the time the pool was put in. One well is a sump that collects condensate from the pool liner, while the other collects water from the aquifer on the down gradient side of the pool. Samples have been collected on a monthly basis. The samples have been analyzed for gross alpha and gross beta emitters, fission products and activation products. There has been no indication that the aquifer has become contaminated from the pool. This sampling schedule is expected to continue for at least three to six months after the pool is emptied of all water to assure that there are no leaks from the groundwater to the inside sump which would imply a leak in the pool liner that could yet allow transfer of radioactivity to the groundwater system.

Additional radiological groundwater monitoring is done at a former supply well (JN) for the West Jefferson Nuclear Sciences Area and from

EM-QAP-2.0 Revision 0 August 17, 1990 Page 5 of 19





EM-QAP-2.0 Revision O August 17, 1990 Page 6 of 19

existing supply wells before the water is treated in any form. Samples that have been collected annually over the last five years have undergone the same analysis for gross alpha and gross beta emitters, fission products, and activation products. These wells continue to be sampled on a routine basis with additional chemical analysis of the JN well sample planned for volatile organic compounds. The Summary from the Interim Report on Site Characterization, West Jefferson North Site, Stage 1 Sampling and Analysis, Chemical Sampling Summary Report, December 22, 1989, has been modified to represent the current status of results and is presented here as the basis for the additional analysis.

#### Summary

A sampling and analysis program for chemical contaminants was performed in November 1989, at Battelle's Nuclear Sciences Area, West Jefferson Site, Ohio. A total of 32 sampling locations provided 29 soil and 3 groundwater samples for chemical analysis. During drilling operations, and the subsequent collection of soil cores, some hydrocarbon contamination of soil, assumed to be fuel oil, was observed around the three fuel storage tanks on site. Subsequent analysis of soil samples collected in these locations confirmed the presence of oil at levels of about 1300 ppm (JN-1), 1200-1500 ppm (JN-2), and 25-50 ppm (JN-4). Further characterization is underway. The EP Toxicity test for metals showed no concentrations above the RCRA limit of 1 ppm for most soil samples analyzed. PCBs were found in only one soil sample, taken close to the on-site transformer beside building JN-2, but at a ppb concentration, well below the action limit of 50 ppm. The only other contaminants, found at ppb concentrations in a few soil samples, were several volatile organic compounds, with acetone predominant. While the concentrations of these compounds are lowlevel, some additional sampling of soil in the storm-sewer outfall area is recommended in conjunction with the additional (Stage 2) sampling proposed for radiological purposes. No contamination was found in the groundwater samples collected.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 7 of 19

In addition to the three chemical sampling wells, twelve shallow wells were installed around the West Jefferson North site as part of the site characterization and will be used for monitoring radionuclides. (See Figure 4.) A full hydrogeological study is expected to be completed in June, 1990, that will allow for the reduction of the number of radiological monitoring wells to six. The need for additional chemical monitoring wells in the vicinity of the fuel storage tanks is being evaluated and appropriate monitoring will be instituted.

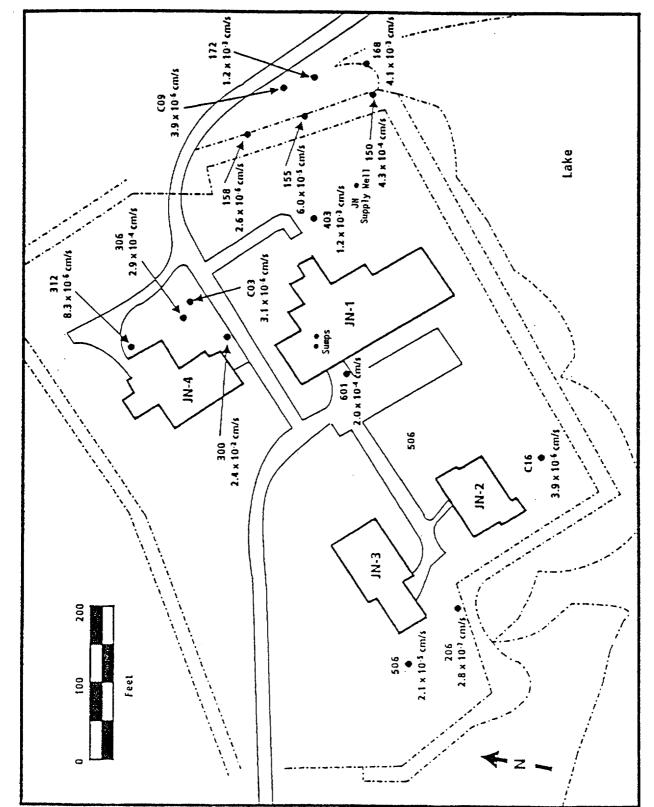
Three of the shallow radiological monitoring wells, also installed as part of the site characterization, are located in and near two former filter beds that still shows traces of radioactive materials. The Summary mentioned above suggests the need for installing chemical wells in this area to identify any pockets of chemicals that may exist from a build up of chemicals that may have gotten into the drain over the life of the filter bed. Radiological monitoring will be performed on an annual basis using existing wells with routine chemical monitoring being added when the wells are completed.

Liquid effluents at Battelle's King Avenue site are discharged into the city sanitary sewer system. The discharge points are currently monitored under the Environmental Monitoring program. All electrical transformers are housed within the facility with secondary containment dikes. There has been no known waste disposal or treatment on site. As a result of these conditions, no groundwater sampling is planned for the King Avenue site.

The budgetary resources for this program are from the BCLDP operating funds which will have to be increased or another source found for installing additional wells and covering the cost of routine chemical sampling.

Supplemental environmental monitoring, done in support of groundwater monitoring, includes the routine sampling or monitoring of effluents from, and the collection of routine samples of surface water, soil, and biota in the environs of a facility. The data and information collected are assessed to determine the impact of the operations in the facility on the environs and persons present in the environs, and to provide guidance for adjusting the operations if the impact is inappropriate.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 8 of 19



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FIGURE 4. LOCATIONS OF GROUNDWATER MONITORING WELLS

EM-QAP-2.0 Revision 0 August 17, 1990 Page 9 of 19

<u>Objective:</u> 1. Ensure that appropriate media samples are analyzed and data correctly interpreted to determine the impact of elements, compounds and radionuclides to groundwater. The annual objective is the reporting and assessment of all data culminating in a formal report to DOE.

> Maintain appropriate instrumentation and equipment in good repair and calibration to effectively collect and assess all sampled media.

Work Statement: Maintain the schedule of routine sampling, monitoring and analytical activities as provided in Appendix A and evaluate and perform additional sampling as determined necessary to adequately characterize the impact of operations on the environment.

#### 4.0 Procurement

Procurement of items affecting quality of sampling shall be controlled through documents QA-AP-4.1. Procurements will be usually limited to replacement of worn or defective equipment from approved vendors.

#### 5.0 Instructions, Procedures, and Drawings

Besides the application of ANSI/ASME NQA-1, the following groundwater monitoring procedure shall be used for routine monitoring, along with other procedures listed that may be used for supplemental sampling to identify potential hazards. Procedures and dates in parentheses indicate replacement procedures and the anticipated dates for completion of the replacement. Nonroutine sampling shall be handled through work instruction as outlined in QA-AP-5.2.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 10 of 19

Procedure	Revision	Date	Title
√ SC-SP-012	Rev. 0.1	05-30-90	Collection of Groundwater Samples in Support of Site Groundwater Charac- terization
NS-NS-10.1 (EM-SP-009)	Rev. O	07-07-81 (08-31-90)	Procedure for the Collection of Environmen- tal Hazardous Chemical Samples
√ EM-SP-002	Rev. 0.1	06-11-90	Procedure for the Collec- tion of Environmental Radiological Water Sam- ples
NS-NS-11.1 (EM-SP-010)	Rev. O	07-07-81 (08-31-90)	Procedure for the Collection of Environmen- tal Hazardous Chemical Water Samples
√ NS-NS-12 (EM-SP-003)	Rev. 1	07-31-90 (08-31-90)	Procedure for the Collection of Environmen- tal Radiological Soil Samples
√ NS-NS-13 (EM-SP-004)	Rev. 1	05-07-84 (08-31-90)	Procedure for the Collection of Environmen- tal Vegetation Samples - Annual Grass
SC-SP-006	Rev. O	01-29-90	Sampling of Sediment and Sludge in Ponds, Streams, Sumps and Closures

## 6.0 Document Control

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The following Project and QA program personnel or their designated alternates shall have the authority to approve quality documents:

EM-QAP-2.0 Revision 0 August 17, 1990 Page 11 of 19

ES&H Manager

Task Manager

Health Physics Supervisor

• QA Manager

Radiochem. Lab. Manager

All documents shall be controlled by the review, approval and issue of process document QA-AP-6.1. Document revision and operation under temporary procedure changes shall be accomplished as specified in QA-AP-6.1. Documents shall be controlled by distribution of a document index containing the current document revision, and by staff responsibility for possessing current documents.

#### 7.0 Control of Purchased Items and Services

All project purchases other than routine supplies shall be reviewed by the QA Manager to determine if the purchase is a quality item based on the evaluation process described in QA-AP-4.1 and QA-AP-2.1. The QA organization personnel shall evaluate objective evidence of quality furnished by subcontractors to determine their suitability for placement on the approved suppliers list. Selection of quality-affecting subcontractors shall be made from records of past performance, incorporation on an approved suppliers list, and/or site visit evaluations, if necessary, as controlled by document QA-AP-7.1. The Project and QA organization may perform on-site surveys of the proposed subcontractor for acceptance on the approved suppliers list.

Procurements from time of order placement to receipt at Battelle shall be controlled by methods in QA-AP-7.2. The QA organization shall participate in the examination of all purchased quality items and services to determine their compliance to specifications of the purchase order. Approved inspection plans for items shall be drawn up in advance as specified in QA-AP-7.2 and QA-QP-10.1.

### 8.0 Identification and Control of Items

All specimens, samples or any items quality-related to the program shall be identified by an affixed identification designation and/or in documents

EM-QAP-2.0 Revision 0 August 17, 1990 Page 12 of 19

traceable to the items. Routine samples shall use the appropriate sample identification found in appendix A, along with the date and time of sample collection. These samples are further identified in traceable documents and procedures. Non-routine sample identification shall be spell out in work instructions and identified in traceable documentation.

#### 10.0 Inspections

Inspections for items shall be conducted in accordance with the requirements of QA-AP-10.1. The acceptance of items shall be documented and approved by the Task Manager or by higher management, as necessary and appropriate.

Spot surveillance of activities by observation by the QA Manager or his designee to assess their conformance with requirements and approved procedures. Any discrepancies noted shall be resolved with the Task Manager. Any noncompliance reports (NCRs), deficiency notices (DNs), and corrective action reports (CARs) shall be prepared, processed, and resolved in accordance with Sections 15 and 16 of this plan.

#### 11.0 Test Control

Analytical testing activities will be performed to collect data from the groundwater samples. Radioanalytical activities performed at Battelle will be controlled, documented and evaluated under procedure EL-AP-1.0 and its associated testing procedures.

#### 12.0 Control of Measuring and Test Equipment

- 12.1 The following items of systems are quality-affecting but not directly data generating, and requiring calibration.
  - 12.1.1 Teflon 1-1/2 inch bailer
  - 12.1.2 Composite Water Sampling System
  - 12.1.3 Radioanalytical Lab Counting Equipment
  - 12.1.4 Chemical Analytical Lab

from approved vendors list

EM-QAP-2.0 Revision 0 August 17, 1990 Page 13 of 19

- 12.2 The equipment indicated in Section 12.1.3 shall be calibrated to a standard traceable to the National Institute of Standards and Technology (NIST), formerly NBS.
- 12.3 The re-calibration time sequence for the calibrated equipment should not exceed one year unless justification is documented.

#### 13.0 Handling, Storage, and Shipping

The handling of all specimens, samples, and quality-related items shall be performed under controlled conditions predetermined to prevent damage, loss, minimize deterioration and assure safety. The storage of all specimens, samples, and quality-related items shall be implemented under controlled conditions predetermined to prevent damage, loss, minimize deterioration, and assure safety. The cleaning of all specimens, samples, and quality-related items shall be implemented under controlled conditions predetermined to prevent damage, loss, minimize deterioration, and assure safety. The packaging and shipping of all sample materials shall be implemented under controlled conditions predetermined to minimize loss, damage, and minimize deterioration.

#### 14.0 Inspection, Test, and Operating Status

The status of inspection and test activities shall be documented in records traceable to the items and the items tagged or identified where possible to assure the tests or inspections are performed. All items not meeting the inspection or test specifications or allowable limits shall be marked and/or separated from the approved items to prevent their inadvertent use, transport or disposal. All samples containing contaminant radioactivity will be identified with appropriate radioactivity identification tag and segregated from uncontaminated material. All samples containing contaminant levels of chemicals will be identified with a chemical contamination tag and segregated from uncontaminated material.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 14 of 19

#### 15.0 Control of Nonconforming Items

All items or operations not meeting specifications or performed in accordance with approved procedures shall be documented on a nonconformance report. The nonconformance report shall be processed in accordance with procedure QA-AP-15.1.

#### 16.0 Corrective Actions

All proposed corrective actions generated to resolve conditions adverse to quality shall be submitted to the NQA Manager for approval of the adequacy and time schedule of the action. Corrective actions shall conform to the requirements of procedure QA-AP-16.1. The cause of the adverse condition shall be determined, if possible, and corrective actions taken to preclude its recurrence. Follow-up action shall be taken by the program Technical Manager and Q.A. Manager to verify implementation and effectiveness of the corrective action.

#### 17.0 Quality Assurance Records

Records which furnish documentary evidence of quality shall be specified, prepared, and maintained.

Specified records include, but are not limited to the following:

- a. Maps identifying sampling locations
- b. Sampler Record Book
- c. Sample inventory
- d. Technical procedures and data sheets
- e. Calculation and analyses records
- f. Reports
- g. Q.A. Surveillance and Audit Records
- h. Program correspondence

Records shall be made part of the BCLDP record management system and subject to all the requirements and restrictions of the system.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 15 of 19

## 18.0 Audits

Audits shall be planned and implemented in accordance with procedure QA-AP-18.1. A pre-program audit is not required as this is a continuation of an existing program. Periodic audits will be conducted over the life of the program.

EM-QAP-2.0 Revision 0 August 17, 1990 Page 16 of 19

APPENDIX A

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EM-QAP-2.0 Revision 0 August 17, 1990 Page 17 of 19

#### A-1

## ROUTINE ENVIRONMENTAL GROUNDWATER MONITORING SCHEDULE

This monitoring schedule describes the well identification, general location, sampling frequency, and the measurements to be made. Radiological measurements are to consist of gross alpha and gross beta-gamma measurements with isotopic analysis to be performed on samples that are five times the counting background.

Chemical sampling is to consist of any combination of the following parameters and analytical methods or other approved methods that will produce the same sensitivity or better.

#### Analytical Methods

Volatile organic compounds	 SW-846 Method 8240 (GCMS)
Semi-volatile compounds	 SW-846 Method 8270 (GCMS)
Oil and grease	 SW-846 Method 423.1 (gravimetric)
Metals - EP Toxicity	 SW-846 Several Methods
PCBs	 SW-846 Method 8080 (GCMS)
рН	 SW-846 Method 150.1 (electrometric)

Sample collection frequency is to be annually (A) unless otherwise specified.

A - Annually M - Monthly

EM-QAP-2.0 Revision 0 August 17, 1990 Page 18 of 19

## A-2

## GROUNDWATER SAMPLING -WEST JEFFERSON LOCATION

	North Area West Jeff	erson Site	
WELL #	Location	Frequency	Measurements
150	Storm Sewer Outfall	Α	$\alpha$ , $\beta$ (Diss/susp)
155	Storm Sewer Outfall	A	$\alpha$ , $\beta$ (Diss/susp)
158	Storm Sewer Outfall	A	$\alpha$ , $\beta$ (Diss/susp)
168	Storm Sewer Outfall	A	$\alpha$ , $\beta$ (Diss/susp)
172	Storm Sewer Outfall	A	$\alpha$ , $\beta$ (Diss/susp)
206	South of JN-3	А	$\alpha$ , $\beta$ (Diss/susp)
300	Southeast of JN-4	А	$\alpha$ , $\beta$ (Diss/susp)
306	East of JN-4	A	$\alpha$ , $\beta$ (Diss/susp)
312	Northeast of JN-4	А	$\alpha$ , $\beta$ (Diss/susp)
403	East of JN-1	А	$\alpha$ , $\beta$ (Diss/susp)
506	West of JN-3	A	$\alpha$ , $\beta$ (Diss/susp)
601	West of JN-1	A	$\alpha$ , $\beta$ (Diss/susp)
C03	East of JN-4	A	$\alpha$ , $\beta$ (Diss/susp)
C09	Storm Sewer Outfall	A	$\alpha$ , $\beta$ (Diss/susp)
C16	Southeast of JN-2	A	$\alpha$ , $\beta$ (Diss/susp)
		<b>D</b> 1 4	
Well #	Remediated Filter		Mangunamente
$\frac{ne11}{\pi}$ 101		Frequency	<u>Measurements</u>
101	East Side of Filter Bed	А	$lpha,\ eta$ (Diss/susp), and Chemical Sampling
104	Southeast of Filter Bed	А	$lpha,\ oldsymbol{eta}$ (Diss/susp), and Chemical Sampling
110	West Side of Filter Bed	A	$\alpha$ , $\beta$ (Diss/susp), and Chemical Sampling

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EM-QAP-2.0 Revision O August 17, 1990 Page 19 of 19

	Supply We	<u>11s</u>	
Well ID	Location	Frequency	Measurements
JN	Nuclear Science Area (Supply Well-Inactive)	A M	$lpha,\ eta$ (Diss/susp), and Chemical Sampling
ЈМ	West Jefferson Middle Area (Supply Well- Active)	А	$\alpha$ , $\beta$ (Diss/susp)
JM-1	West Jefferson Middle Area (Facility Well)	A	$\alpha$ , $\beta$ (Diss/susp)

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## FRED H. KLAER, JR. & ASSOCIATES

CONSULTING GROUND-WATER GEOLOGISTS AND HYDROLOGISTS forthe the

16 LELAND AVENUE COLUMBUS 14, OHIO

PHONE AMHERST 8-3316 April 2, 1963

Mr. C. T. Greenidge Battelle Memorial Institute 505 King Avenue Columbus 1, Ohio

Dear Mr. Greenidge:

#### West Jefferson, Ohio.

In accordance with our discussion on March 14, Mr. Don Kyser of this office met Mr. Glen Williams at the West Jefferson plant of Battelle Memorial Institute on March 15.

The non-pumping or static level in the south well (Layne No.1) was measured as 42.1' below the pump hase. At the time the well was drilled in September 1954, the static level was reported by Layne as 41 feet. The apparent difference in static level is insignificant and suggests that there has been no serious regional decline in water levels during the past 9 years. It was not possible to measure the pumping level, because of the danger of flooding the basement. I believe this should be done, however, by making arrangements to waste water through several outlets within the building, if this can be worked out. (73)

The static water level in the north well was measured as 18.17 feet below the pump base. The static water level in April 1955, as reported by Layne was 18'5". This also shows that there has been no significant change in water levels in this area. The pumping level was measured as 39.22 feet below the pump base after 3 to 4 minutes of pumping. This is not particularly significant because of the short period of pumping.

At the present time both wells operate automatically for such short periods of time that it is impossible to get any true value for pumping level. It may be possible to make arrangements to waste water in some way, so that the pumps could operate for one-half to one hour without shutting off. This I believe should be investigated. It is obvious, however, that you are not fully utilizing the capacities of these wells.

We have plotted up the data on the south well provided to you in a letter report from Burgess and Niple, dated September 16, 1954. This indicates that the transmissibility of the limestone aquifer is about 16,500 gallons per day per foot. Assuming a coefficient of storage of 0.0001, which is reasonable for limestone aquifers, we estimate that this well could be pumped continuously at 250 gpm. for a period of a year without recharge, without lowering the pumping level below the pump intake.

Using the values assumed above, we have computed the cone of influence of the south well pumping continuously at 250 gallons per minute without recharge, a copy of which is attached. This indicates that pumping the south well continuously will lower the water level in the north well, about 3700 feet away, about 2.5 feet in 24 hrs., 6.4 feet in 10 days, 11.4 feet in 180 days and about 12.6 feet in one year. At the present time your pumping schedule is so infrequent that one well probably does not affect the other.

It is believed that the wells of West Jefferson are about 10,000 feet from the south well. The computed cone of influence graph shows that unless the pumping from the West Jefferson wells now reported as 200 - 300 gpm. increases considerably, it is unlikely that your wells will be seriously affected.

This diagram can be used to determine proper spacing for additional wells, depending on the rates and expected duration of pumping.

We will be glad to discuss this with you at your convenience.

Sincerely yours,

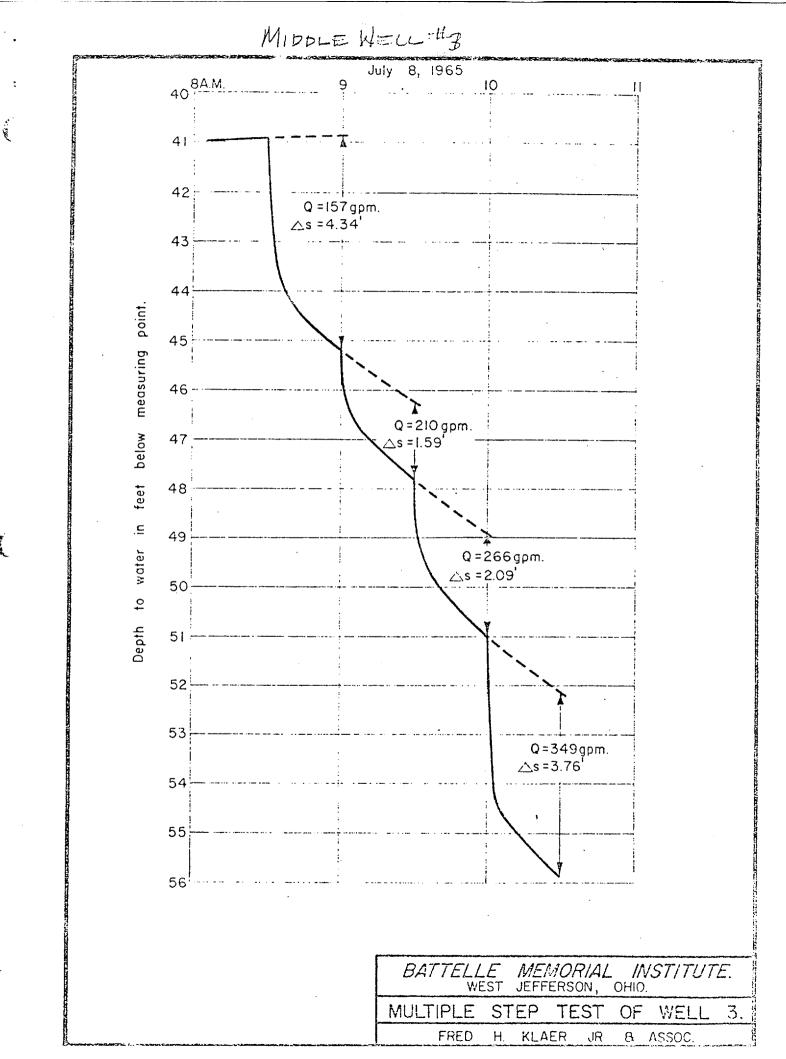
FRED H. KLAER, JR. & ASSOCIATES

Fred H. Klaerfr.

Fred H. Klaer, Jr. Consulting Ground-Water Geologist and Hydrologist.

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(FORMERLY ROBERT C. SMITH & ASSOCIATES)

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FRED H. KLAER, JR. & ASSOCIATES CONSULTING GROUND WATER GEOLOGISTS AND HYDROLOGISTS

> 16 LELAND AVENUE COLUMBE 5, OHIO 43214 P.O. BOX 3496 PHONE AMALERST 8-3316

July 13, 1965.

Eattelle Memorial Institute 505 King Avenue Colubus, Ohio 43201

Attention: Mr. Daun Peterseim

#### Well 3, West Jefferson, Ohio.

Gentlemen,

In accordance with your instructions, we have followed closely the drilling of Well 3 near your proposed greenhouse site at your West Jefferson, Ohio, plant. The well was completed and tested during the week of July 6, 1965.

The driller's log of the well is reported as follows:

0 - 3	topsoil
3 - 5	light brown clay
5 - 48	brown clay and gravel
48 - 54	gravel
54 - 115	light brown clay and gravel
115 - 119	gravel
119 - 149	brown lime rock
149 - 152	red clay
152 - 161'9"	brown lime rock
118'6"	pipe in hole

Driller reports crevice in limestone at 141 feet. Static water level, June 8, 1965, 40.92 feet below top of casing.

On June 8, 1965, a multiple step drawdown test was run to determine at what depths water was entering the well and to determine the well losses within the pumping well. The well was pumped at rates of 157, 210, 266, and 349 gpm. for periods of 30 minutes at each rate. Prior to the step test, the static water level was at a depth of 40.92 feet below the top of the casing. At the end of the test, the pumping level was at a depth of 55.87 feet. A hydrograph of the multiple step test is shown in an attached figure.

# MIDDLE KIELL #3

## FRED H. KLAER, JR. & ASSOCIATES CONSULTING GROUND-WATER GEOLOGISTS AND HYDROLOGISTS

During the step test, the pumping level at a rate of 349 gpm. was at a depth of 55.87 feet below the top of the casing. Since the casing extended to a depth of 118 feet, 6 inches, it was not possible to lower the water level below the casing to show where the water enters the well. Since the driller reported a crevice at a depth of 141 feet, we must assume that the major part of the water enters the well below a depth of 118 feet and probably at about 141 feet.

The well loss factor was computed to be 8.92 and the actual well loss at a pumping rate of 349 gpm. was 5.36 feet. This is not unreasonable for a limestone well and we believe the well is acceptable.

Some difficulty was encountered originally in developing the well to pump clear water. It was necessary to spend several days in developing. It was planned to use acid treatment to clear up the well. However, no acid was used in the well and the capacity was increased by surging and pumping. At a rate of 349 gpm. the water pumped was completely clear.

On June 9, 1965, a constant rate test was run. The well was pumped at a constant rate of 349 gpm. from 8:15 a.m. until 2:45 p.m., a total of 6-1/2 hours. The static water level at the start of the test was at a depth of 41.22 feet below the top of the casing and the pumping leve' at the end of the test was 62.75 feet, giving a total drawdown of 21.53 feet. The apparent specific capacity was about 16.2 gpm. per foot of drawdown.

The transmissibility of the limestone aquifer was determined as 9050 gallons per day per foot. This is somewhat lower than that computed from a pumping test on the south well of about 16,500 gpd. per ft., reported to Mr. Greenidge in our letter of April 2, 1963, based on a pumping test run by Burgess and Niple in September 1954.

Using a value of transmissibility of 9050 gpd. per ft. and a storage coefficient of 0.0001 (which is reasonable for the limestone aquifer in this area), the computed drawdown of a well pumping continuously at a rate of 500 gpm. for one year from ground water storage will be about 69.1 feet, plus 11 feet of well loss or a total of 80.1 feet. The total available drawdown will be the difference between the static water level of about 45 feet and the point where the water enters the well, say at 140 feet, giving a total drawdown of 95 feet. It is our opinion, therefore, that the safe capacity of the well is about 500 gpm. If it is necessary to pump the well at this rate the bottom of the pump intake should be set at a depth of 140 feet.

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# MIDDLE HELL #1

## FRED H. KLAER, JR. & ASSOCIATES CONSULTING GROUND-WATER GEOLOGISTS AND HYDROLOGISTS

We will be glad to discuss the results of these tests you in more detail at your convenience.

Very truly yours,

FRED H. KLAER, JR. AND ASSOCIATES

Fight. Klass.

Fred H. Klaer, Jr. Consulting Ground-Water Geologist and Hydrologist.

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: Lyne-Ohio Co.

	South Well House THE LAYNE OHIO COMPANY	E Jefferson Lugo
	COLUMBUS, OHIO LOG OF TEST WELL No.	malisan Cr.
Battella Institute	Vest Jefferson City	Ohio

......

3000 ft. east of Georgesville Road; north of ravine

Location.....

. For

Date Started.

......Date Finished...

## **FORMATIONS**

DEPTH TO	DEPTH TO	THICK-	STATIC	1-HOUR B.	AILING TEST	HOW FAR DID	
TOP OF STRATUM	BOTTOM OF STRATUM	NESS OF STRATUM	WATER LEVEL	AVERAGE G. P. M.	DRAWDOWN FEET	FORMATION HEAVE	FORMATION FOUND REMARKS
0*	12*	12"					Top soll
12	33	21		-		-	Blue clay
-33	38	5	24*7"	-			Dirty gravel; e little water
38	60	22	-				blue cley
60	68	8	24.7"			-	Dirty gravel
68	88	20	-			-	Red clay
38	99	11	~				Gray clay
99	103	٤.	41*	103: 41	12" pipe i	n hole	Gravel
103	106	3	41.	Pumped 2	50 g.p.m.	t 68°6"	Totton Timus Pouls
106	111	5		<u>pursjin</u>	5 LGVCi BI	ter 12 hrs.	Gray "
111	115 120	14 5				-	Limestone; layers of clay Gray limestone
<u>115</u> 120 <u>130</u>	130 138	10	41*				Ligestone and clay Oray limestone

Toledo Rogers

DRILLER

	THE	LA	YNE	OHIO	COMPANY
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COLUMBUS, OHIO 12" LOG OF TEST WELL No. 2

For Battelle Memorial Institute

City West Jefferson State

Ohio

Location

3-16-55 Date Started

Date Finished 4-8-55

## FORMATIONS

DEPTH TO TOP OF STRATUM	DEPTH TO BOTTOM OF STRATUM	THICK- NESS OF STRATUM	STATIC WATER LEVEL	1-HOUR BAILING TEST		HOW FAR DID	
				AVERAGE G. P. M.	DRAWDOWN FEET	FORMATION HEAVE	FORMATION FOUND REMARKS
01	61	61					Yellow clay
6	7	1					Sand and gravel
7	32	25					_Blue clay
32	34	2					Sloppy sand
34	80	46					Clay and gravel
80	98	18					Brown clay
98	102	<u>4</u>	2015"	411- <sup>-</sup>			Red clay and soft rock
102	115	13					Brown limestone
115 118	118 119	3					Brown lime and clay Break
119 122	122 123	3	1815"				Brown limestone Break
123 128	128 130	52					Brown lime Blue shale

Andy Ego DRILLER

MATERIAL: 107' of 12" pipe (extending 1'6" above ground) PERMANENT LOG SHOWING WELL FORMATIONS AND CONSTRUCTION AND PUMPING (SKETCH OF LOCATION ON BACK OF THIS LOG SHEET) EQUIPMENT WILL BE PREPARED AS SOON AS PUMPING EQUIPMENT IS INSTALLED.

5-G P-897

