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TABLE 1

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GEOMETRIC DATA FOR VERTICAL EMPLACEMENT OPTION

	-	Analyzed	Conceptual Design (ft)		
Drift Dimension					
Width	16.0 ^a	(4.88m)	16.0		
Height	22.0 ^a	(6.71m)	22.0		
Radius of Roof Arch	9.0 ^b	(2.74m)	9.5 ^e		
Container Borehole Dimensions					
Depth	25.00 ^{&}	(7.62m)	25.0		
Diameter	2.42 [£]	(0.74m)	2.42		
Panel Dimensions					
Waste Standoff from					
Access Drift Wall	77.5 ^ª	(23.62m)	92.5		
Access Drift Width	21.0 ²	(6.40m)	21.0		
Emplacement Drift Spacing	112.0 ^c	(34.14m)	26.0		
Barrier Pillar Width	63.0 ^a	(19.20m)	63.0		
Panel Width	1400.0 ^a	(426.72m)	1400.0		

Source references: ^aMansure and Stinebaugh (1985). ^bParsons, Brinckerhoff, Quade and Douglas (1985). ^cMansure and Ortiz (1984). ^dMacDougall (1986) - Spent Fuel Emplacement ^eParsons, Brinckerhoff, Quade and Douglas (1986).

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GEOMETRIC DATA FOR HORIZONTAL EMPLACEMENT OPTION

	-	Analyzed	Conceptual Design ^d	
Drift Dimension				
Width	18.0 ^a	(5.49m)	23.0	
Height	13.0 ^a	(3.96m)	13.0	
Radius of Roof Arch	10.2 ^b	(3.11m)	14.0	
Container Borehole Dimensions				
Waste Standoff from Emplacement				
Drift Centerline	117.50 ^a	(35.81m)	145.5 ^e 4 4	
Length	682.00 ^a	(207.87m)	363.0 131	
Diameter	2.75 ^a	(0.84m)	2.5	
Panel Dimensions				
Panel Width	1400.0 ^a	(426.72m)	1400.0	
Panel Depth	985.0 ^C	(300.23m)	748.0	

^aMansure and Stinebaugh (1985). ^bParsons, Brinckerhoff, Quade and Douglas (1985). ^cMansure and Ortiz (1984). ^dMacDougall (1986) - Spent Fuel Emplacement ^eParsons, Brinckerhoff, Quade and Douglas (1986).

DATA FOR THERMAL AND THERMAL/MECHANICAL ANALYSES OF EMPLACEMENT DRIFTS

Property		Value
Rock Mass		
Specific Gravity ^a		2.34 g/cc
Young's Modulus [®]		15.1 GPa
Poisson's Ratio ^a		0.2
Thermal Conductivity ^a		
(25 to 100 deg. C temp range)		2.07 W/m·K
Thermal Capacitance ^a Thermal Expansion ^a (*10 ⁶)		2.25 J/cm ³ K
(25 to 200 deg. C temp range)		10.7C* ⁻¹
Horiz./Vert. In situ Stress ^b		0,55
Ground Surface Temperature ^C	•	16.0°C •
Temperature Gradient ^d		0.0239°C/m
Rock Matrix ^a		0.0237 0/2
Unconfined Compressive		
Strength of Rock		75.4 MPa
Tensile Strength		-9.0 MPa
Angle of Internal Friction		29.2*
Joints ^a		67.6
Joint Cohesion		1.0 MPa
Joint Coefficient of Friction		0.8 (38.7*)
Joint Angle		90° (Vertical)
(Frequently Assumed Value)		

References:

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^aNimick et al. (1984). ^bBauer, Holland and Parrish (1985) ^cEglinton and Dreicer (1984). ^dSass and Lachenbruch (1982).

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NORMALIZED COEFFICIENTS FOR THE POWER DECAY FUNCTION FOR PWR AND BWR SPENT FUEL MIX

Normalized S	trength, a _i	
$t^a = 0 yr$	t = 8.55 yr	Time Exponent, b ₁ (yr ⁻¹)
0.03120	0.15602	0.00135
0.13920	0.59787	0.01914
0.04920	0.15227	0.05188
0.78270	0.09384	0.43768
	t ^a <u>- 0 yr</u> 0.03120 0.13920 0.04920	0.031200.156020.139200.597870.049200.15227

^aTime, t, is given with respect to time of removal of spent fuel from the reactor.

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RESULTS OF PRELIMINARY ANALYSES OF THE HORIZONTAL EMPLACEMENT OPTION--DATA FOR 100 YR AFTER EMPLACEMENT

	Number of	Free T	emp. at	Total Initi	al Stress
Description	Sources in	Surface Pan	el Center	Horizontal	Vertical
of Model	<u>Half Model</u>	<u>Condition</u>	(*C)p	(MPa)	<u>(MPa)</u>
Single Panel	35	None .	35.89	12.21	5.81
		Isothermal Only	35.89	12.49	5.52
		Free (1,000 m ^a)	35.89	11.90	5.07
		Free (2,000 m)	35.89	11.86	5.07
					•
Extended Panel	12	None	34.11	13.38	4.27
		Isothermal Only	34.11	13.83	3.38
		Free (1,000 m)	34.11	12.77	3.30
Three Panels	35 + 10	None	35.95	13.94	4.08
		Isothermal Only	35.95	14.40	3.63
		Free (1,000 m)	35.95	13.25	3.25
		Free (2,000 m)	35.95	13.05	3.24

^aThe lengths specified here refer to the extent of the boundary elements used to model the ground surface above the repository, measured from the repository centerline. ^bAmbient Temperature = 23.2°C

-31-

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THERMAL ANALYSES PERFORMED USING THE DOT COMPUTER CODE

Time (Yr)	Time Steps Number of (Yr) Steps		Time Increment For Power Decay Function (Yr)
VERTICAL EMPL	ACEMENT UNVE	NTILATED	
0-10 10-35 35-100	0.005 0.010 0.065	2,000 2,500 1,000	0.10 0.35 1.00
	VEN	TILATED	
0-1 1-10 10-35 35-100	0.0010 0.0025 0.0100 0.0500	1,000 3,600 2,500 1,300	$\begin{array}{c} 0.1 \\ 1.0 \\ 3.5 \\ 1.0 \end{array}$
HORIZONTAL EN	PLACEMENT		
• 0-10		NTILATED	0.10
10-35 35-100	0.010 0.025 0.065	1,000 1,000 1,000	0.10 0.35 1.00
	VEN	FILATED	
0-10 10-35 35-100	0.0100 0.0250 0.0650	1,000 1,000 1,000	0.10 0.35 1.00

^aThe power decay function describing the strength of the heat sources has been tabulated for various times. The strength at any particular time between the tabulation times is determined by interpolating linearly within the appropriate time increment.

- 32 -

TABLE 7

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RESULTS OF THERMAL ANALYSES OF UNVENTILATED VERTICAL EMPLACEMENT DRIFT USING ALTERNATIVE EFFECTIVE THERMAL CONDUCTIVITIES

Effective Thermal	Drift Perimeter Temperatures 10 Yr After Waste Emplacement					
Conductivity W/m.K	Crown (°C)	Midfloor (°C)	Midwall (°C)			
25	72.2	77.0	74.7			
50	73.4	75.9	74.7			
100	- 73.9	75.2	74.6			

^a The tabulated values of temperature were obtained by performing analyses, using the DOT code. For each analysis 2,000 time steps of 0.005 yr were used to reach the total simulation time of 10 yr.

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RESULTS OF BOUNDARY-ELEMENT ANALYSES OF THE VERTICAL EMPLACEMENT DRIFT

Time (Yr)	٥	5	10	20	35	50	100	150	200
^a Crown Displace ment (cm)	-0.078	-0.804	-0.172	-0.041	0.611	1.487	3.528	4.794	5.742
Midfloor (CA) Displacement (CA)	0.507	1.132	0.244	-0.218	-1.102	-2.095	-4.229	-5.474	-5.394
^b Total Vertical Closure (ca)	0.429	0.328	0.072	-0.259	-0.491	-0.603	-0.701	-0.680	-0.652
Midwall Displacement	0.115	0.244	0.381	0.546	0.655	0.701	0.718	0.677	0.645
Midwall Closure (cm)	0.230	0.488	0.762	1.092	1.310	1.402	1.436	1.355	1.290
Crown Stress (MPa)	5.757	15.790	25.100	35.840	44.800	48.500	50.210	48.240	46.330
Midfloor Stress (MPa)	0.730	5.184	11.130	18.410	23.150	25.310	26.250	25.070	23.950
Midwall Stress (MPa)	8.453	8.648	5.873	2.163	0.307	-1.507	-2.340	-1.981	-1.559
Crown Temp. (C)	23.0	46.1	60.7	78.8	92.3	99.0	104.0	102.5	100.8
Midfloor Temperature (C)	23.0	75.4	91.3	104.2	112.9	116.1	114.3	109.9	105.9
Midwall Temperature (C)	23.0	58.6	73.6	90.6	102.1	107.2	109.0	106.1	103.8

The following supplemental information is provided to clarify the meaning of various response measures.

^aBoundary element normal displacement.

^bClosure is negative if the dimension increases.

RESULTS OF FINITE-ELEMENT ANALYSES OF THE VERTICAL EMPLACEMENT DRIFT

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Emplacement Type and		Unventil	ated Drif	t		Ventilat	ed Drift	
Location	t=0 yr	10 yr	35 yr	100 yr	t=0 yr	10 yr	35 yr	100 yr
^a Crown Dis- placement (cm)	-0.191	2.330	5.826	10.382	-0.191	1.411	2.482	2.960
Midfloor Dis- placement (cm)	0.235	2.341	5.299	0.9332	0.235	1.739	2.730	3.225
^b Total Vertical Closure (cm)	0.426	0.011	-0.527	-1.050	0.426	0.328	0.248	0.265
Midwall Dis- placement (cm) Midwall	-0.113	-0.399	-0.687	-0.791	-0.113	-0.187	-0.218	-0.204
Closure (CE)	0.226	0.800	1.374	1.582	0.226	0.374	0.436	0.408
c _{Crown} Stress (MPa)	5.75	28.48	47.78	54.28	5.75	11.23	10 50	11 60
Midfloor							12.56	11.60
Stress (MPa) Midwall	0.80	10.38	23.13	28.30	0.80	0.26	2.76	3.55
Stress (MPa)	8.16	4.99	-1.24	-3.84	8.16	6.01	5.05	5.78
Crown Temp. (C) Midfloor	23.0	73.4	101.8	108.5	23.0	23.0	23.0	23.0
Temp (C) Midwall	23.0	75.9	103.6	109.3	23.0	23.0	23.0	23.0
Temp (C)	23.0	74.7	102.8	109.0	23.0	23.0	23.0	23.0
d Factors of Safe	ty:							
Crown	7.2	1.9	1.3	1.2	7.2	4.0	3.6	3.9
Midfloor Midwall	43.7 5.1	4.1 7.6	2.1 60.8	1.8 11.4	43.7 5.1	83.2 6.6	13.7 7.8	11.0 6.9

The following supplemental information is provided to clarify the meaning of various response measures.

^aDisplacements are considered positive if in the positive coordinate direction.

^bClosure is negative if the dimension increases.

^CStresses are extrapolated to the nodal points using computed values at the element gauss points and assuming a linear variation of stress within the element. Compressive stresses are positive.

d These are the stress/strength ratios for the rock mass.

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RESULTS OF BOUNDARY-KLEMENT ANALYSES OF THE HORIZONTAL EMPLACEMENT DRIFT

Time (Yr)	0	5	10	20	35	50	100	150	200
^a Crown Displace- ment (cm)	0.149 -	0.225	0.896	1.152	2.093	2.698	4.126	5.068	5.772
Midfloor Displacement (cm)	0.317	0.032	-0.796	-1.264	-2.341	-3.002	-4.433	-5.326	-5.990
^h Total Closure (cm)	0.465	0.257	0.100	-0.112	-0.248	-0.304	-0.307	-0.258	-0.218
Midwall Displacement (cm)	0.036	0.135	0.207	0.304	0.354	0.389	0.385	0.355	0.329
Midvall Closure (cm)	0.072	0.270	0.414	0.608	0.788	0.778	0.770	0.710	0.658
Crown Stress (MPa)	3.600	9.647	14.380	21.430	26.790	29.720	32.120	31.40	30.390
Midfloor Stress (MPa)	-0.749	3.385	6.564	11.130	14.410	16.070	17.080	16.350	15.570
Midwall Stress (MPa)	12.640	7.448	3.704	-1.059	-3.691	-4.497	-3.400	-1.628	-0.111
Crown Temp. (C)	23.0	23.3	25.0	30.6	39.0	45.8	58.9	64.5	67.2
Midfloor Temperature (C)	23.0	23.3	25.0	30.7	39.0	45.8	59.0	54.5	67.2
Midwall Temperature (C)	23.0	23.3	25.1	30.8	39.1	46.0	59.0	64.5	67.2

The following supplemental information is provided to clarify the meaning of various response measures.

^aBoundary element normal displacement.

^bClosure is negative if the dimension increases.

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RESULTS OF FINITE-ELEMENT ANALYSES OF THE HORIZONTAL EMPLACEMENT DRIFT

Emplacement Type and	Unventilated Drift				Ventilated Drift			
Location	t=0 yr	10 yr	35 yr	100 yr	t=0 yr	10 yr	35 yr	100 yr
^a Crown Dis-								
placement (cm) Midfloor Dis-	-0.184	1.410	3.910	7.251	-0.184	1.419	3.912	7.097
placement (cm)	0.273	1.478	3.638	6.899	0.273	1.499	3.621	6.683
^b Total Vertical Closure (cm)	0.457	0.068	-0.272	-0.352	0.457	0.080	-0.291	-0.414
Midwall Dis-								
placement (cm) Midwall	-0.036	-0.224	-0.397	-0.453	-0.036	-0.222	-0,399	-0.459
Closure (Cn)	0.073	0.448	0.794	0.906	0.073	0.444	0.798	0.918
Crown								
Stress (MPa) Midfloor	3.81	15.52	28.82	36.15	3.81	16.43	27.23	30.88
Stress (MPa) Midwall	-0.71	7.23	15.55	19.37	-0.71	7.60	14.86	17.21
(Stress (MPa)	11.84	2.58	-4.63	- 5.17	11.84	3.24	-5.78	-8.81
Crown Temp-								•
erature (C)	23.0	25.0	38.7	58.3	30.0	30.0	30.0	30.0
Midfloor Temp (C) Midwall	23.0	25.0	38.7	58.3	30.0	30.0	30.0	30.0
Temp (C)	23.0	25.0	38.7	58.3	30.0	30.0	30.0	30.0
d Factors of Safet	.							
Crown	10.4	3.0	1.9	1.6	10.4	2.9	1.9	1.8
Midfloor Midwall	51.6 3.7	5.8 14.4	3.0 8.1	2.5 7.3	51.6 3.7	5.6 11.6	3.1 6.3	2.7 3.9
				•••			~.~	

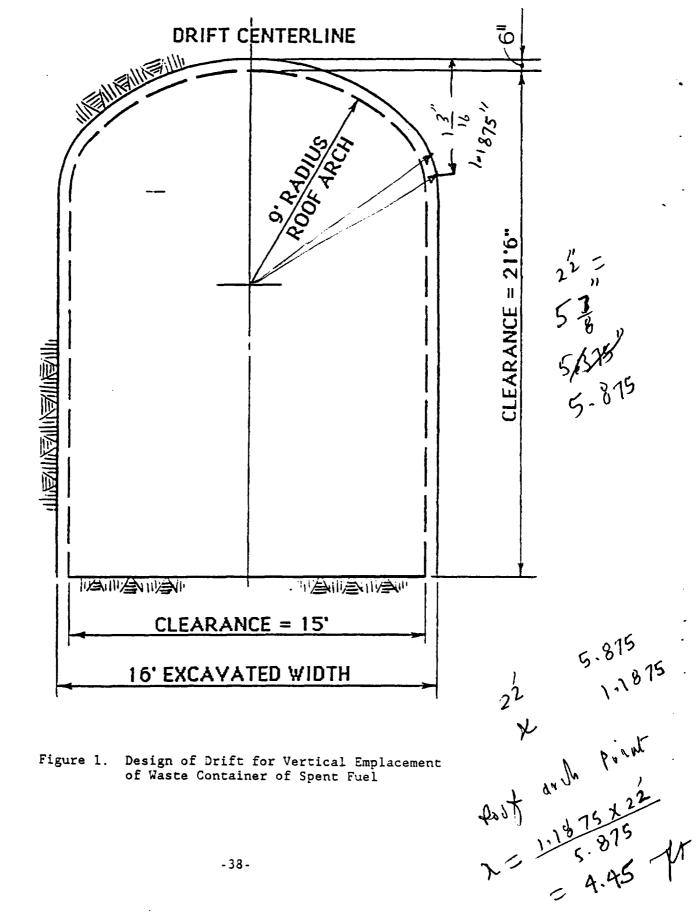
The following supplemental information is provided to clarify the meaning of various response measures.

^aDisplacements are considered positive if in the positive coordinate direction.

^bClosure is negative if the dimension increases.

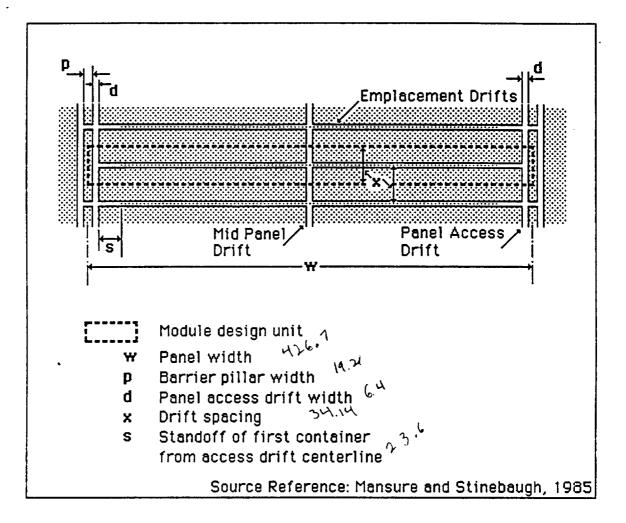
^CStresses are extrapolated to the nodal points using computed values at the element gauss points and assuming a linear variation of stress within the element. Compressive stresses are positive.

d These are the strength/stress ratios for the rock mass.



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Figure 2. Design Module of Vertical Emplacement Panels

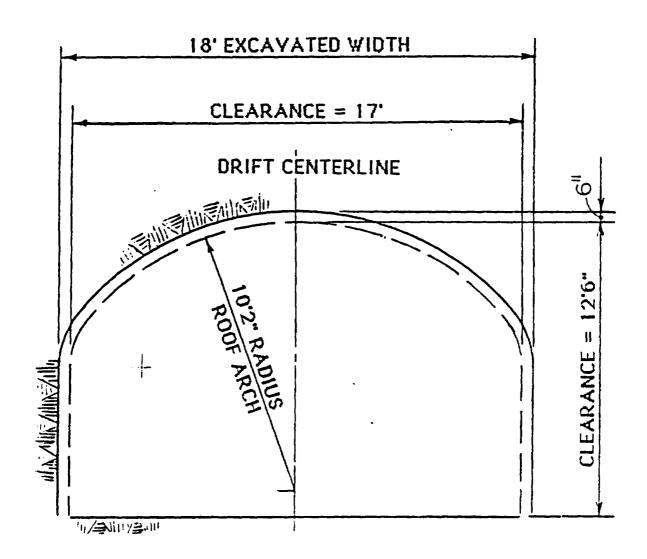


Figure 3. Design of Drift for Horizontal Emplacement of Waste Container of Spent Fuel

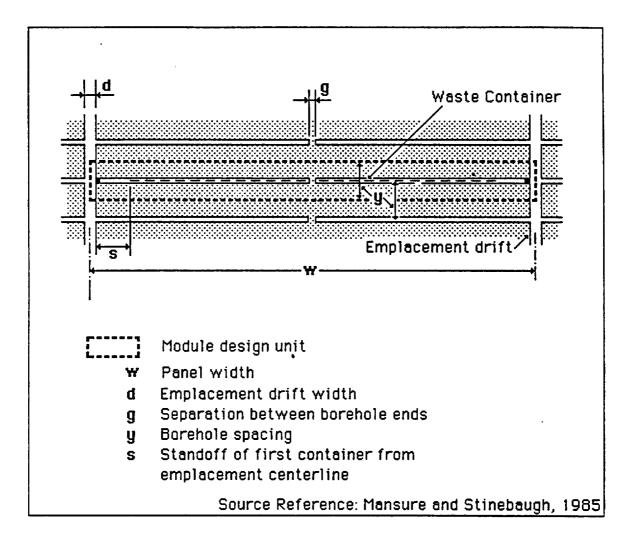
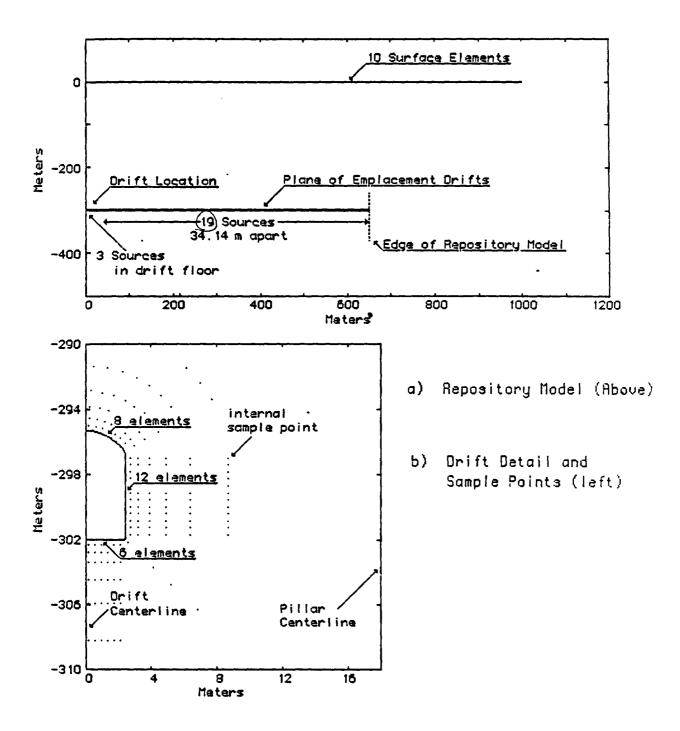
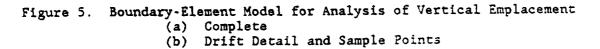
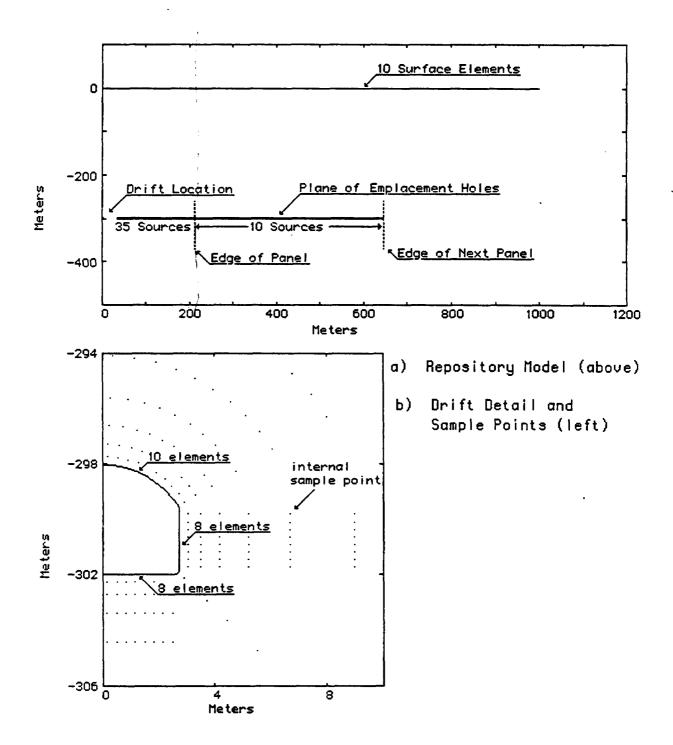


Figure 4. Design Module of Horizontal Emplacement Panels





-42-



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Figure 6. Boundary-Element Model for Analysis of Horizontal Emplacement (a) Complete (b) Drift Detail and Sample Points

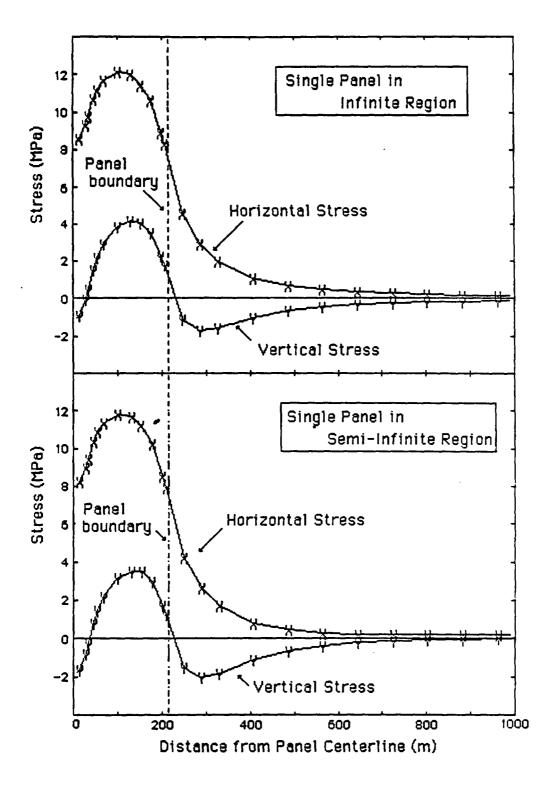


Figure 7. Thermally Induced Stresses in the Plane of a Single Panel 100 Yr After Waste Emplacement in Horizontal Boreholes

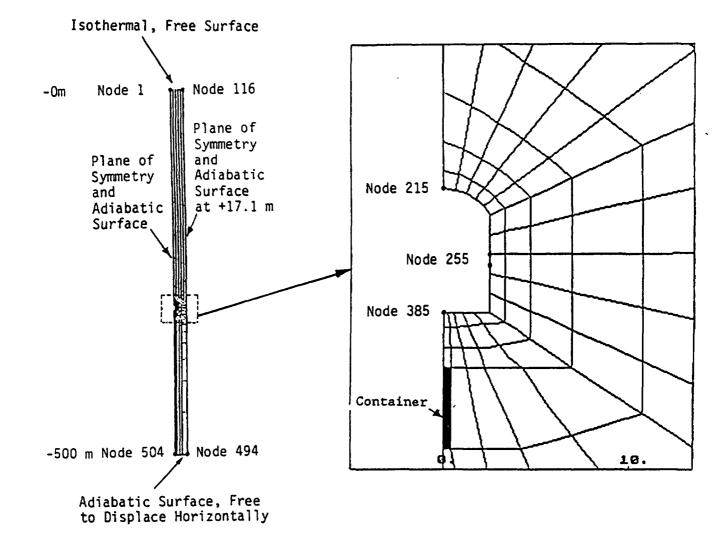


Figure 8. Finite-Element Model for Analysis of Vertical Emplacement (a) Complete Mesh

(b) Details of Mesh around Drift

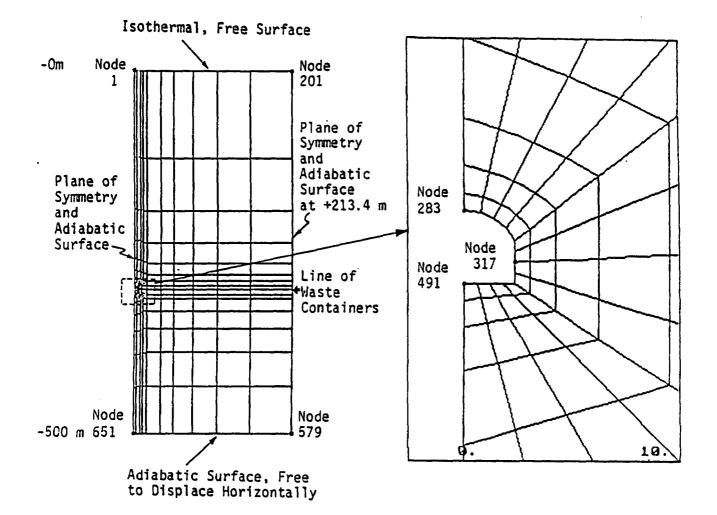


Figure 9. Finite-Element Model for Analysis of Horizontal Emplacement (a) Complete Mesh(b) Details of Mesh around Drift

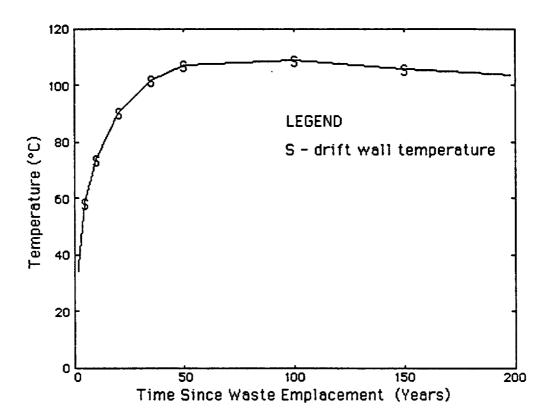


Figure 10. Boundary-Element Predictions of Wall Temperatures of the Vertical Emplacement Drift - Unventilated Drift

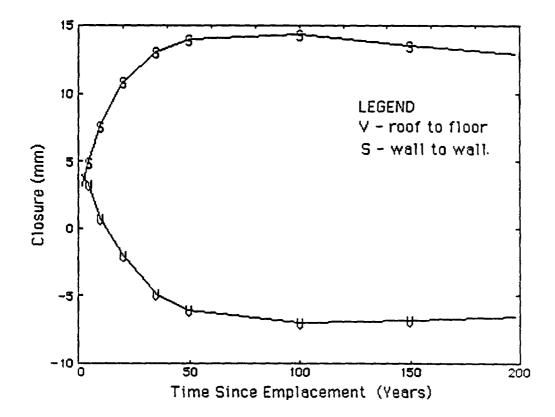


Figure 11. Boundary-Element Predictions of Vertical and Horizontal Closures of the Vertical Emplacement Drift - Unventilated Drift

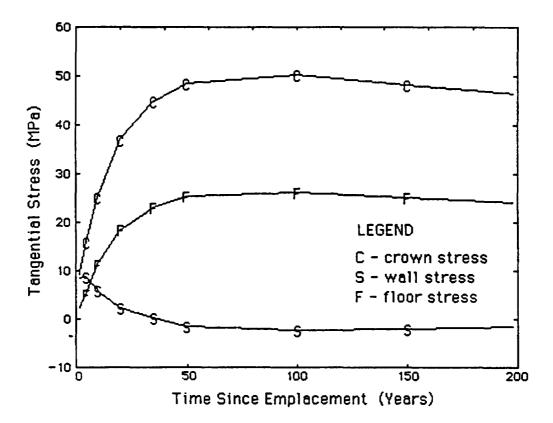


Figure 12. Boundary-Element Predictions of Tangential Stresses at Selected Points Around the Vertical Emplacement Drift - Unventilated Drift

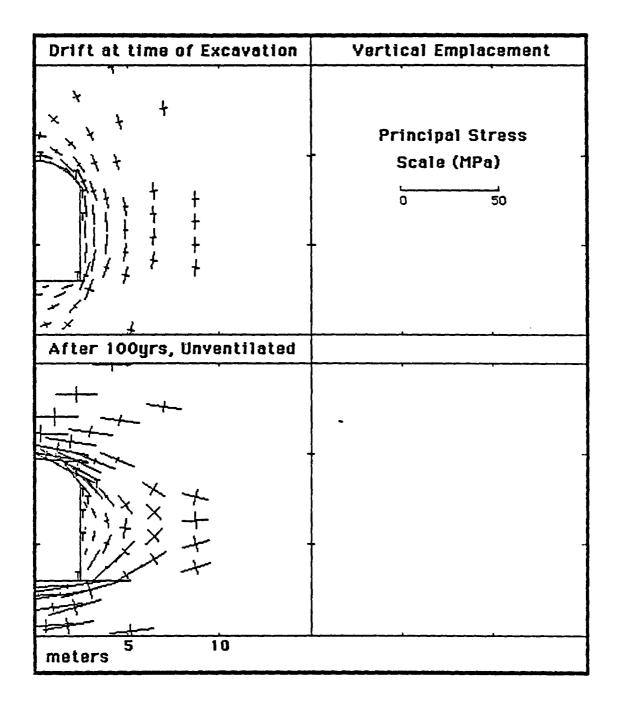


Figure 13. Boundary-Element Predictions of Principal Stresses in the Vicinity of the Vertical Emplacement Drift -Unventilated Drift

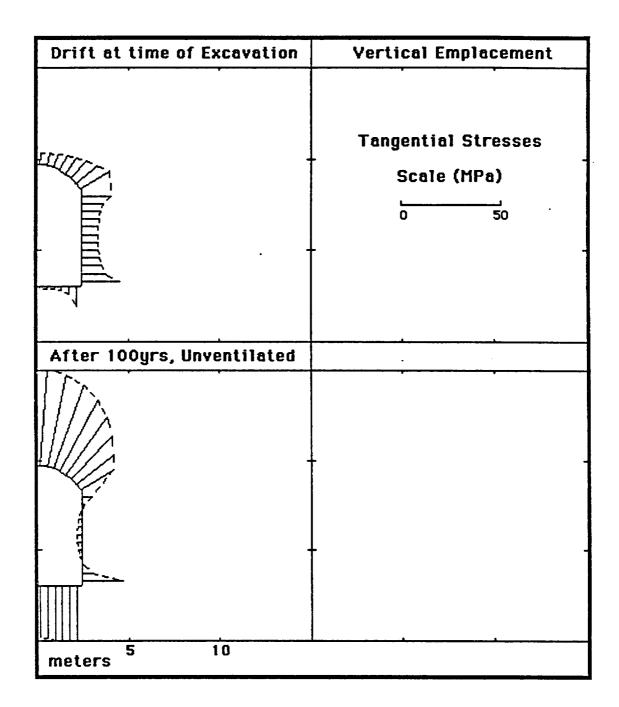


Figure 14. Boundary-Element Predictions of Tangential Stress Distribution Around the Vertical Emplacement Drift - Unventilated Drift

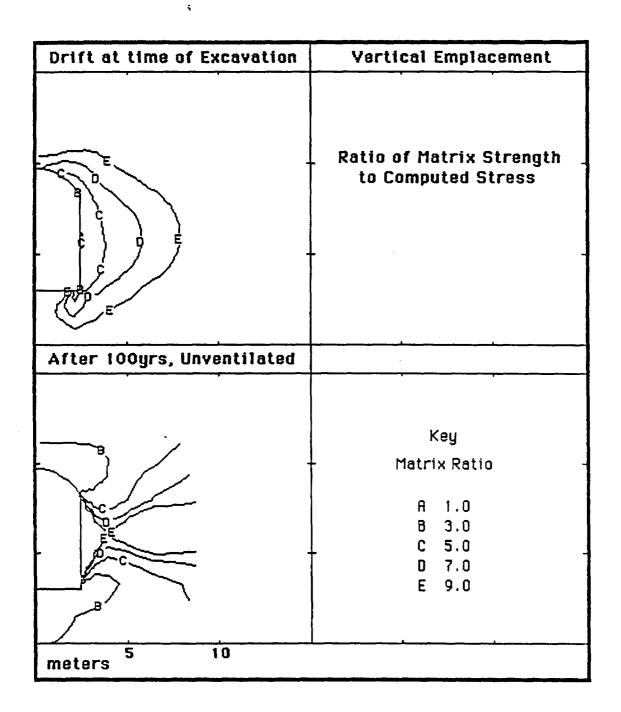


Figure 15. Boundary-Element Predictions of the Ratio Between the Matrix Strength and Stress Around the Vertical Emplacement Drift - Unventilated Drift

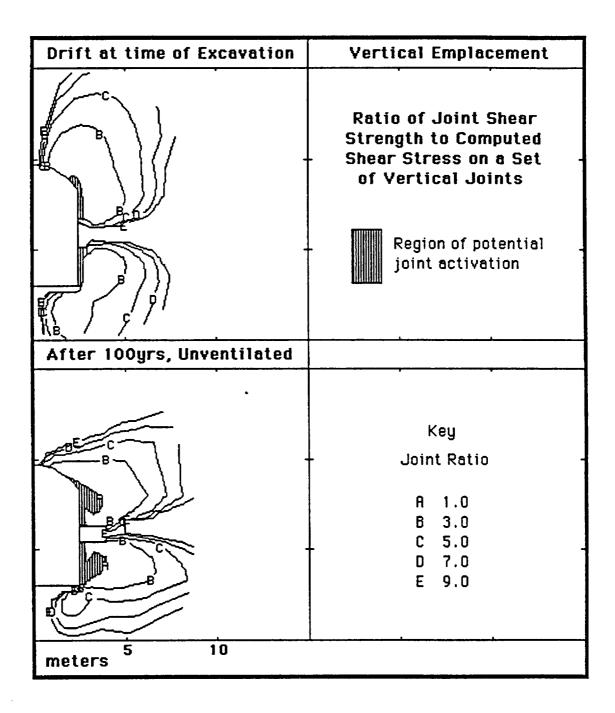


Figure 16. Boundary-Element Predictions of the Ratio Between the Joint Strength and Stress Around the Vertical Emplacement Drift - Unventilated Drift

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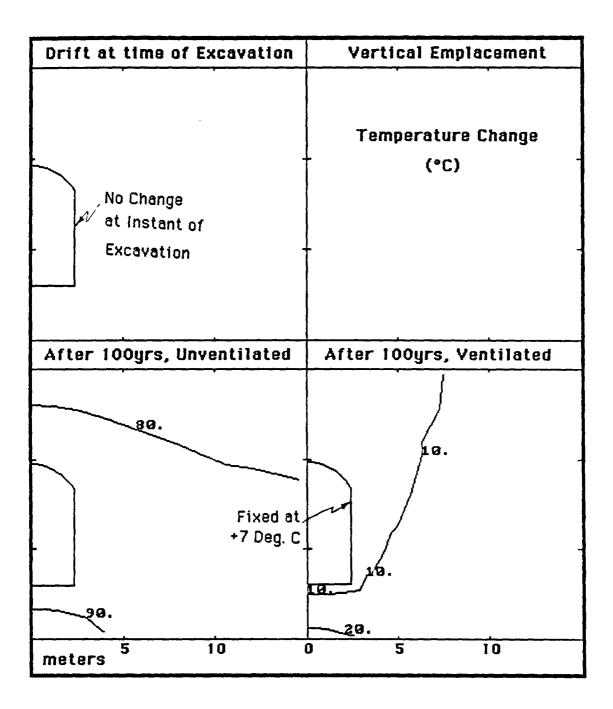


Figure 17. Finite-Element Predictions of the Temperature Change in the Vicinity of the Vertical Emplacement Drift 100 Yr After Waste Emplacement

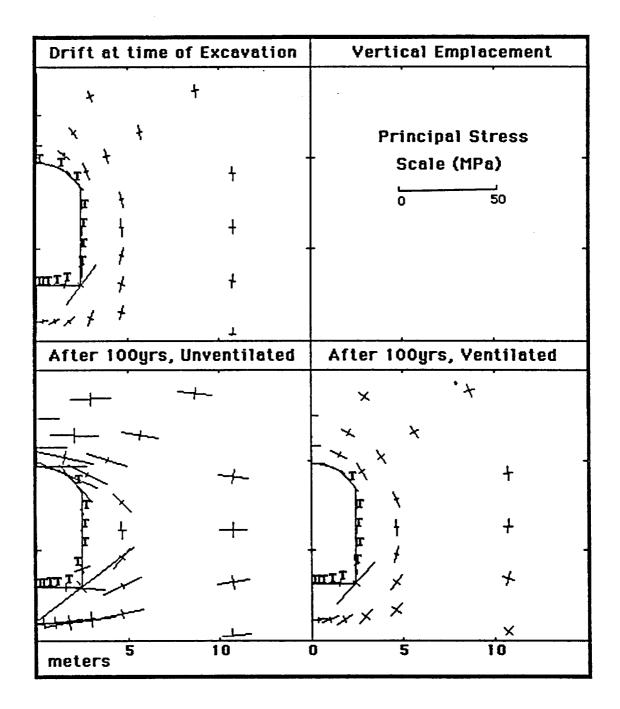


Figure 18. Finite-Element Predictions of the Principal Stresses in the Vicinity of the Vertical Emplacement Drift

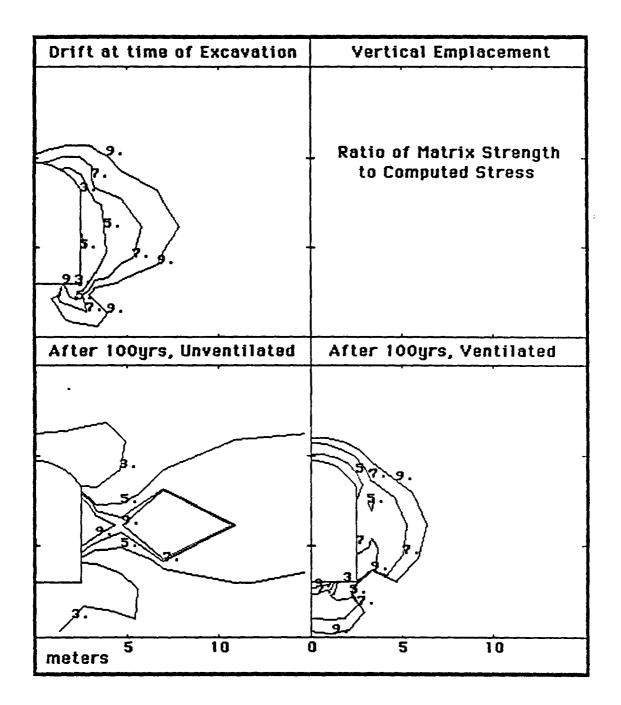


Figure 19. Finite-Element Predictions of the Ratio Between Matrix Strength and Stress Around the Vertical Emplacement Drift

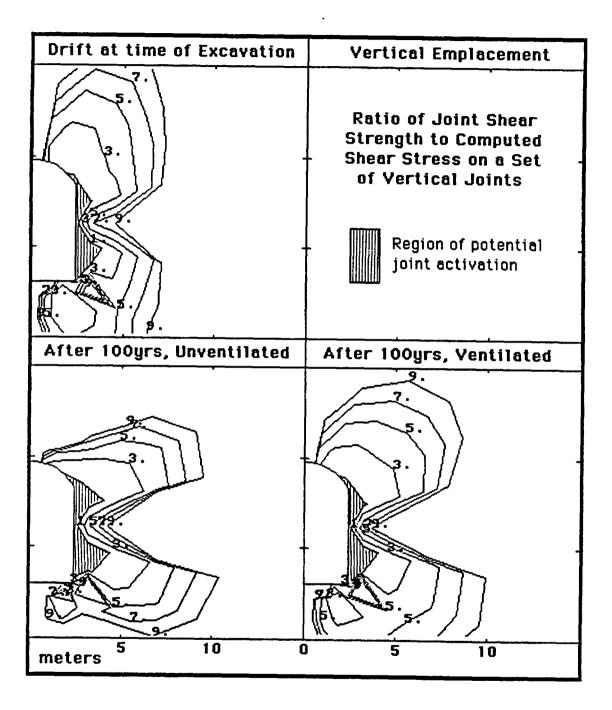


Figure 20. Finite-Element Predictions of the Ratio Between Joint Strength and Stress Around the Vertical Emplacement Drift

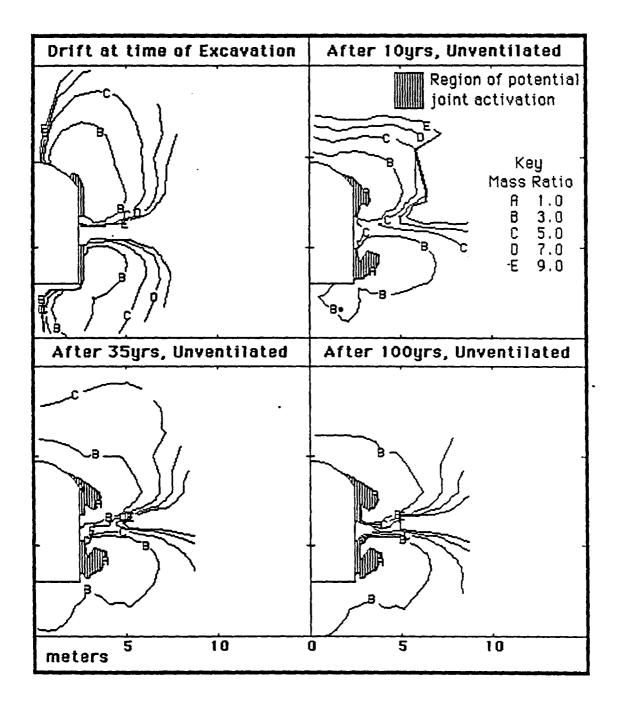


Figure 21. Boundary-Element Predictions of the Ratio Between Joint Strength and Stress Around an Unventilated Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement

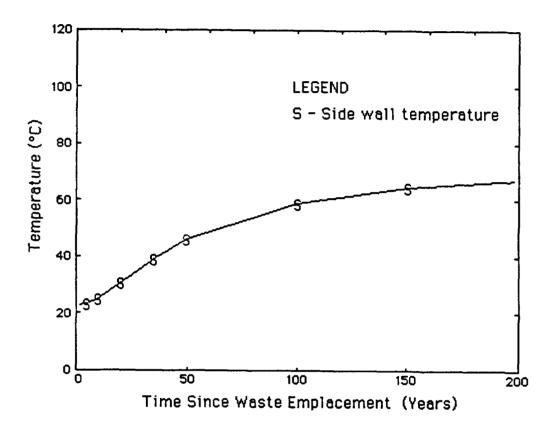


Figure 22. Boundary-Element Predictions of Wall Temperatures of the Horizontal Emplacement Drift - Unventilated Drift

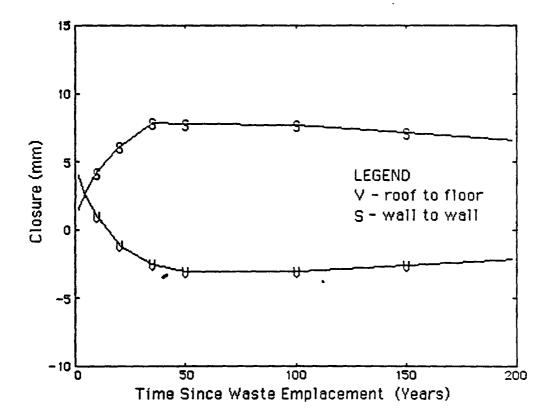


Figure 23. Boundary-Element Predictions of Vertical and Horizontal Closures of the Horizontal Emplacement Drift - Unventilated Drift

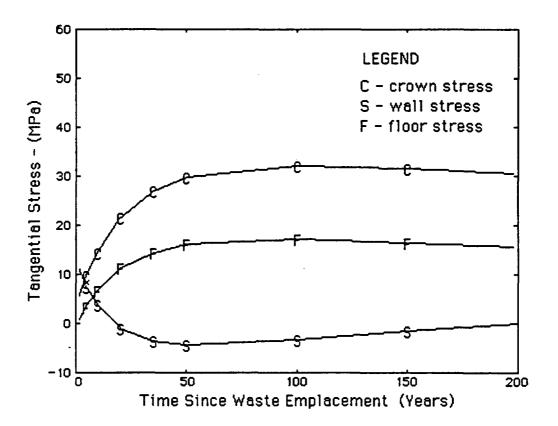


Figure 24. Boundary-Element Predictions of Tangential Stresses at Selected Points Around the Horizontal Emplacement Drift - Unventilated Drift

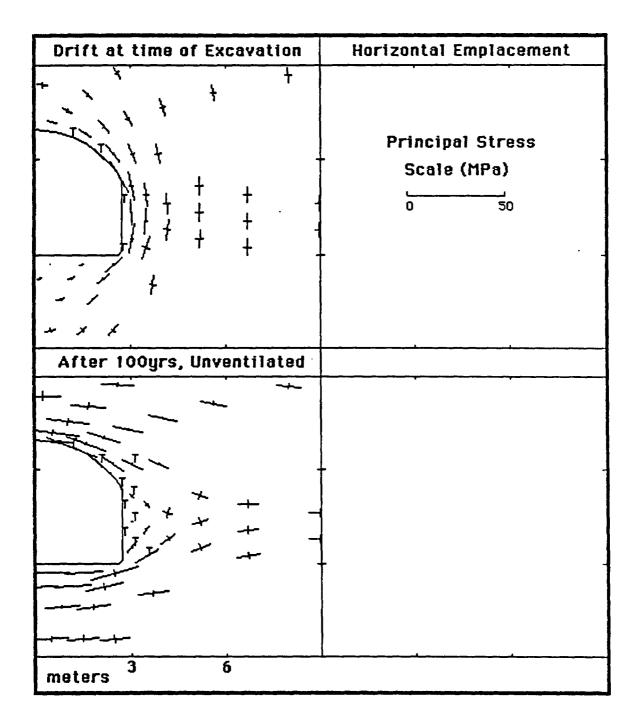


Figure 25. Boundary-Element Predictions of Principal Stresses in the Vicinity of the Horizontal Emplacement Drift - Unventilated Drift

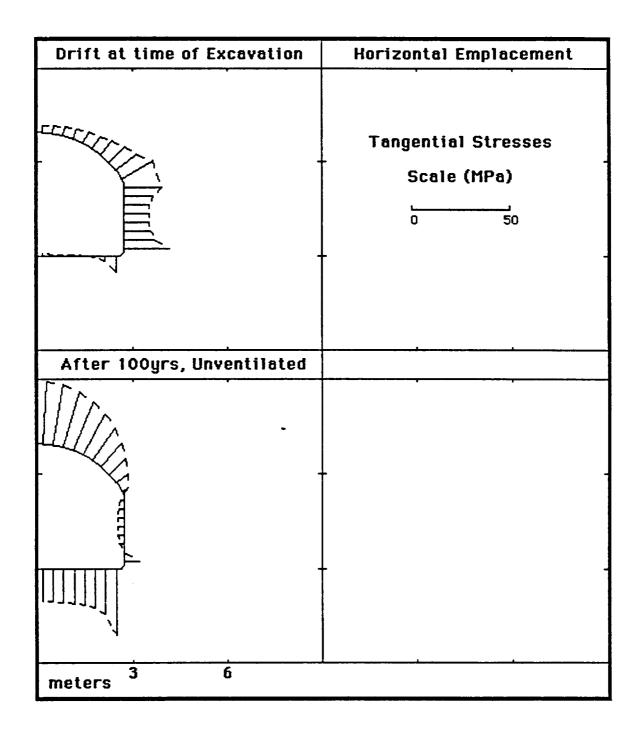


Figure 26. Boundary-Element Predictions of Tangential Stress Distribution Around the Horizontal Emplacement Drift - Unventilated Drift

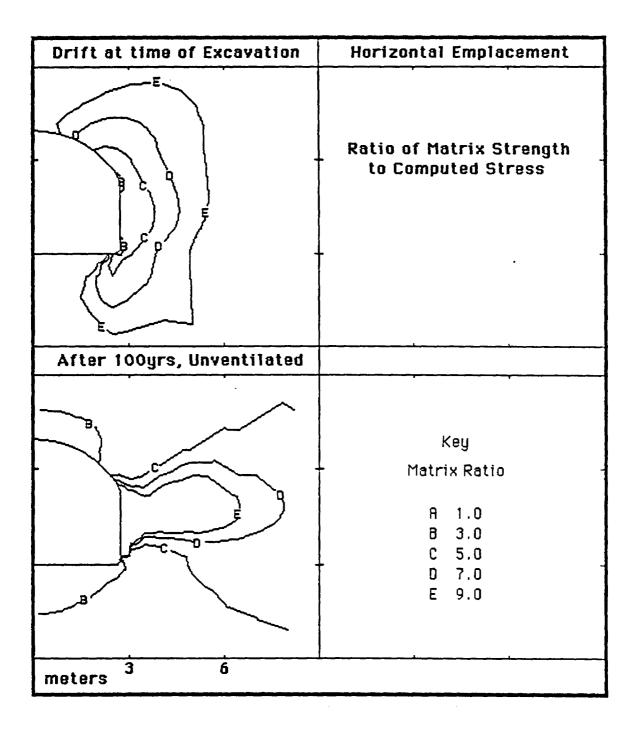


Figure 27. Boundary-Element Predictions of the Ratio Between the Matrix Strength and Stress Around the Horizontal Emplacement Drift - Unventilated Drift

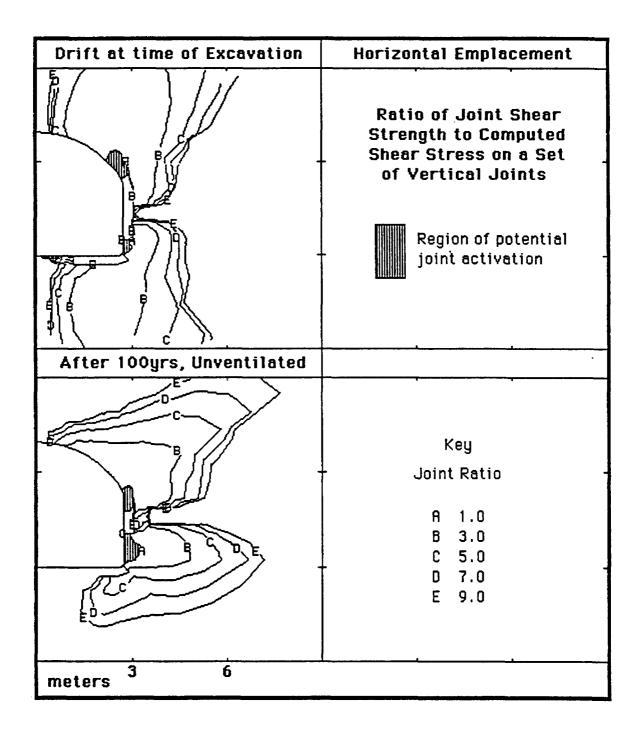


Figure 28. Boundary-Element Predictions of the Ratio Between the Joint Strength and Stress Around the Horizontal Emplacement Drift - Unventilated Drift

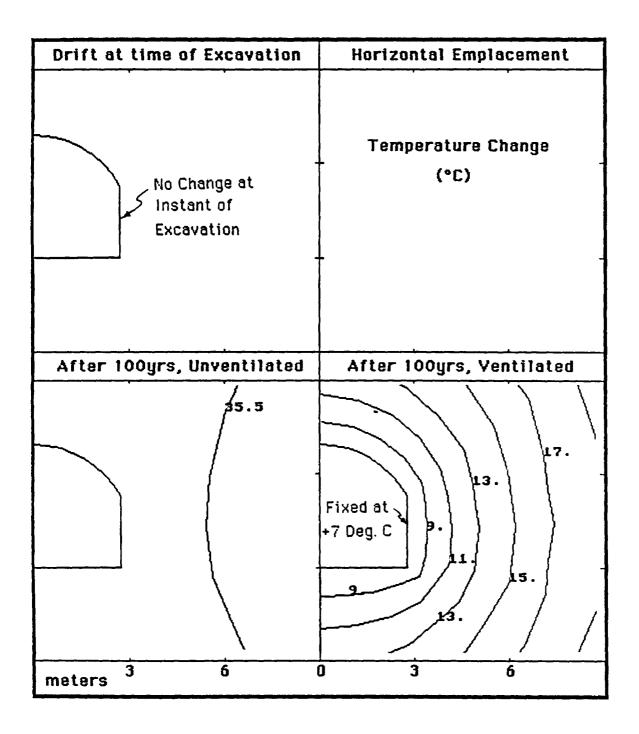


Figure 29. Finite-Element Predictions of the Temperature Changes in the Vicinity of the Horizontal Emplacement Drift 100 Yr After Waste Emplacement

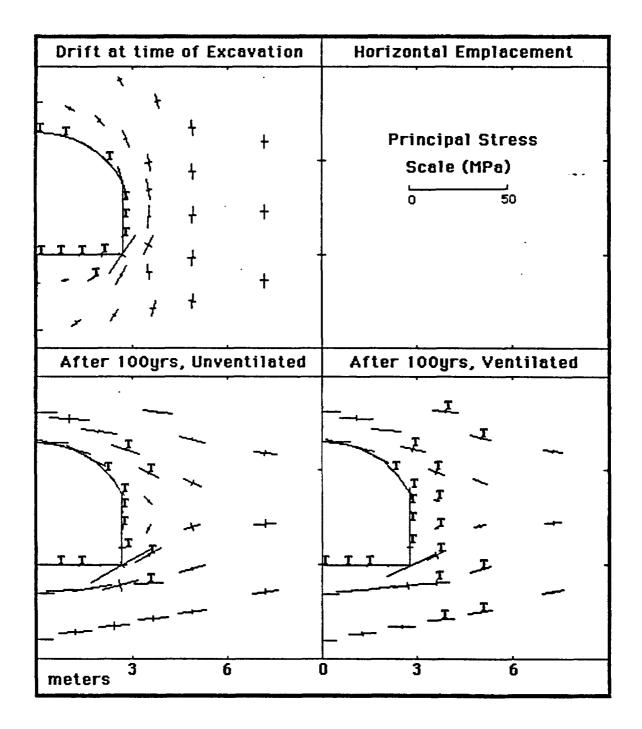


Figure 30. Finite-Element Predictions of the Principal Stresses in the Vicinity of the Horizontal Emplacement Drift

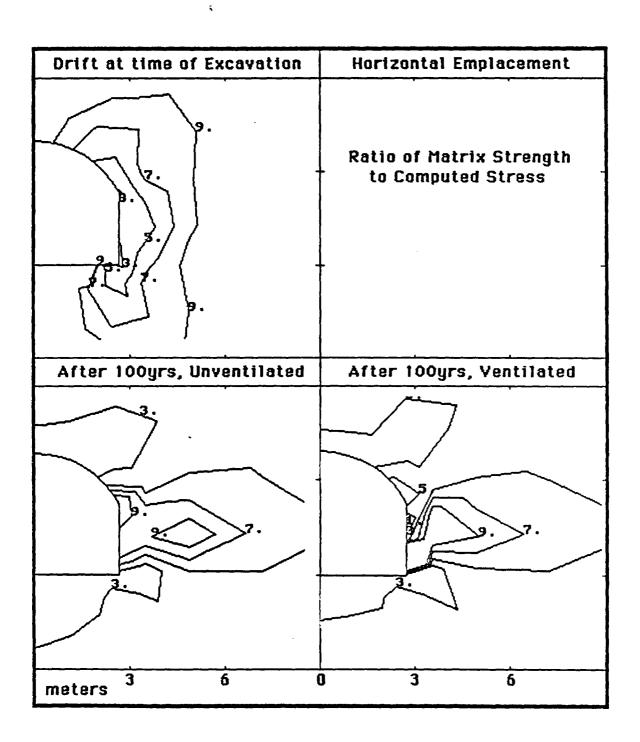


Figure 31. Finite-Element Predictions of the Ratio Between Matrix Strength and Stress Around the Horizontal Emplacement Drift

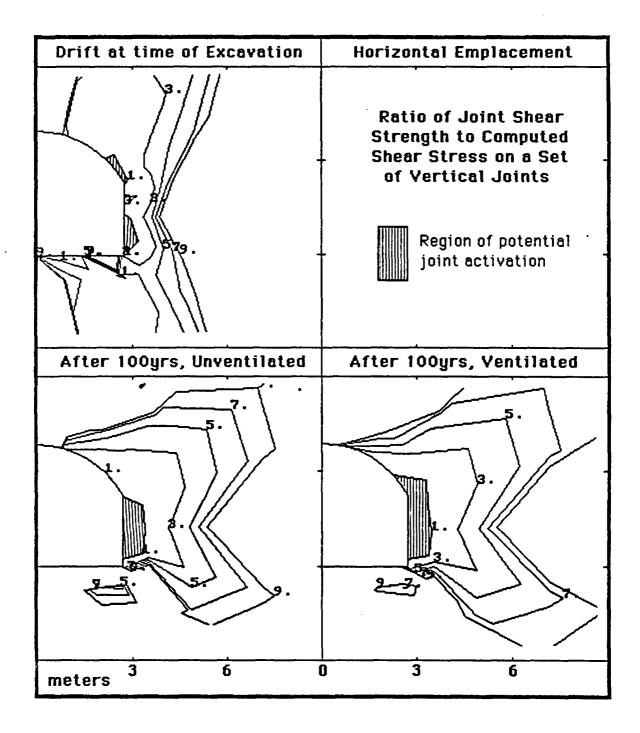


Figure 32. Finite-Element Predictions of the Ratio Between Joint Strength and Stress Around the Horizontal Emplacement Drift

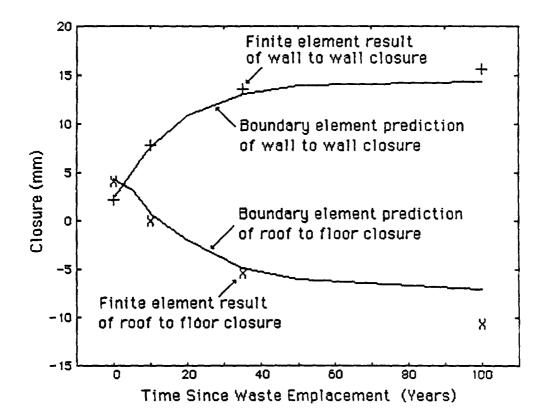


Figure 33. Comparison of Closure Histories For the Drift with No Ventilation, Computed Using Boundary-Element and Finite-Element Models: Vertical Emplacement

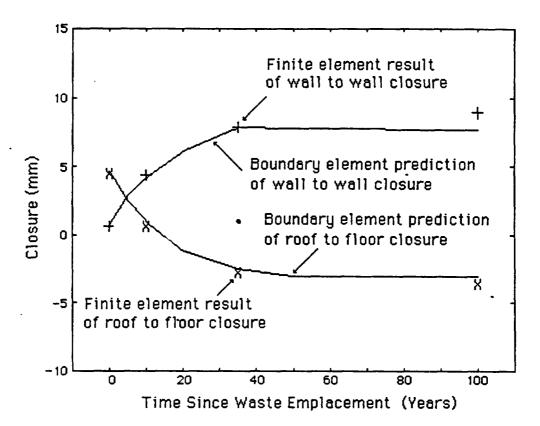


Figure 34. Comparison of Closure Histories for the Drift With No Ventilation, Computed Using Boundary-Element and Finite-Element Models: Horizontal Emplacement

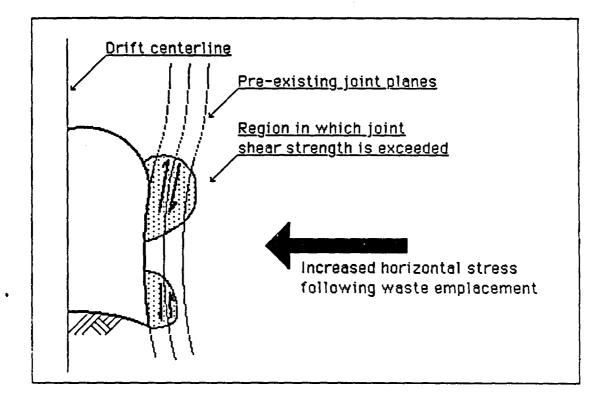


Figure 35. Temperature Histories for Point in the Interior of the Rock Mass

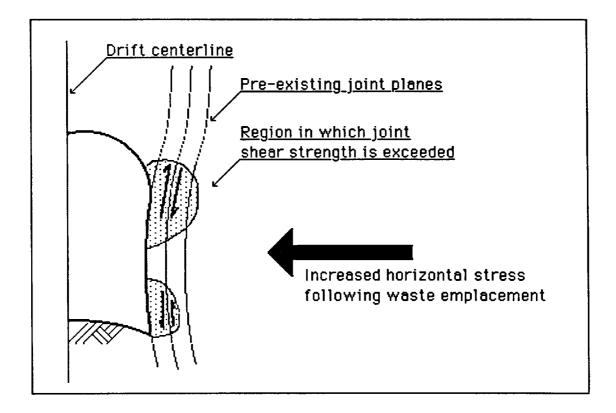
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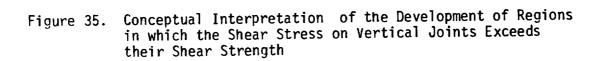
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REFERENCE THERMAL AND THERMAL/MECHANICAL ANALYSES OF DRIFTS FOR VERTICAL AND HORIZONTAL EMPLACEMENT OF NUCLEAR WASTE IN A REPOSITORY IN TUFF

C.M. St. John J.F.T. Agapito and Associates, Inc.

Please replace pages 72, 73, and 74 in your copy of this report with the attached new pages. Thank you.





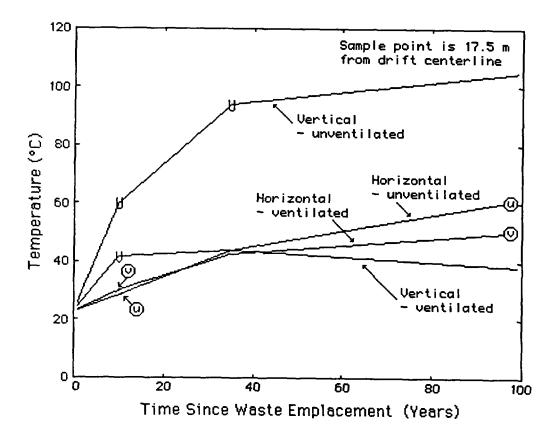
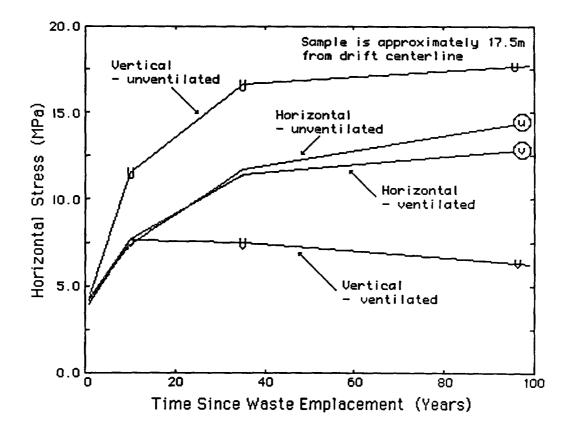
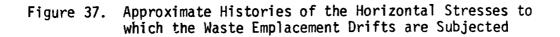


Figure 36. Temperature Histories for Point in the Interior of the Rock Mass



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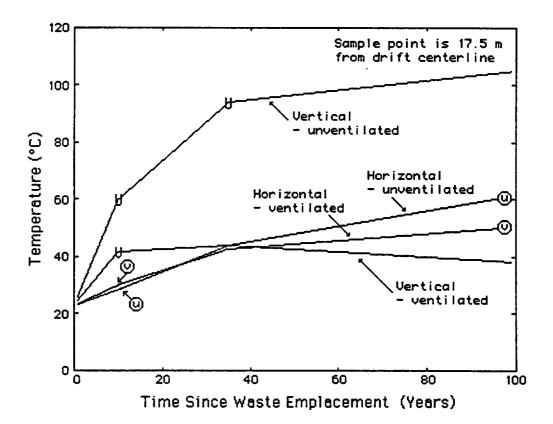


Figure 36. Approximate Histories of the Horizontal Stresses to which the Waste Emplacement Drifts are Subjected

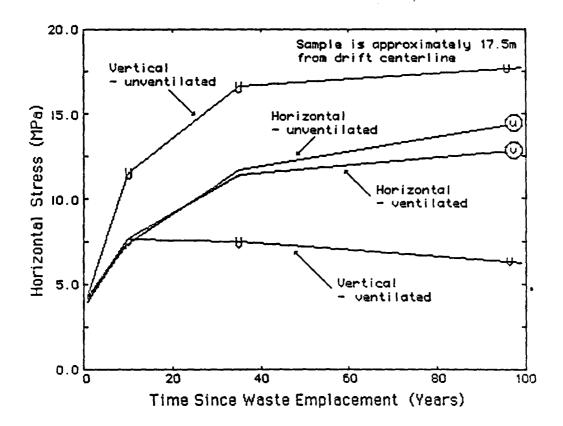


Figure 37. Conceptual Interpretation of the Development of Regions in which the Shear Stress on Vertical Joints Exceeds their Shear Strength

APPENDIX A

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BOUNDARY-ELEMENT ANALYSES OF VERTICAL EMPLACEMENT

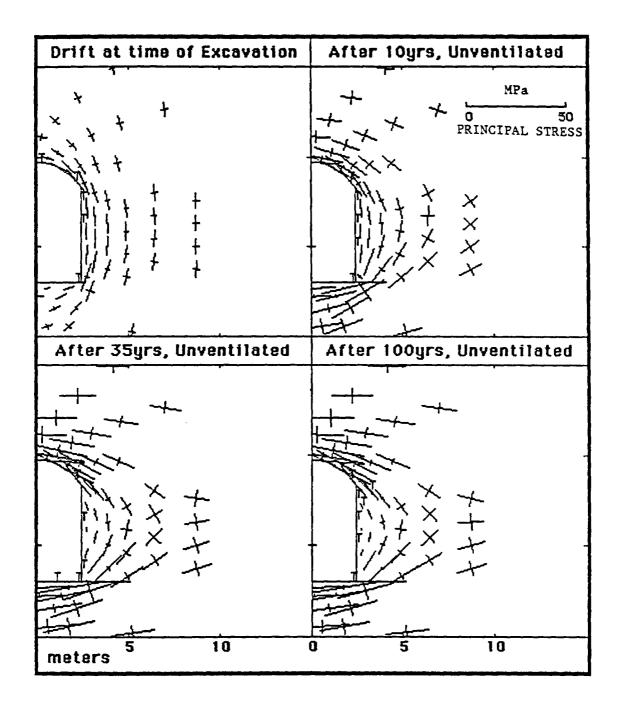


Figure A.1 Boundary-Element Predictions of Principal Stresses in the Vicinity of the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Drift

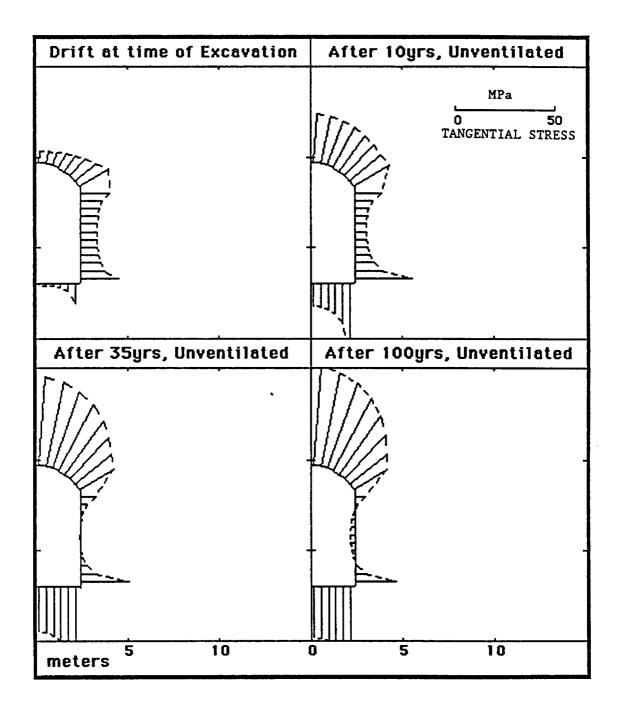


Figure A.2 Boundary-Element Predictions of Tangential Stress Distribution Around the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Unventilated Drift

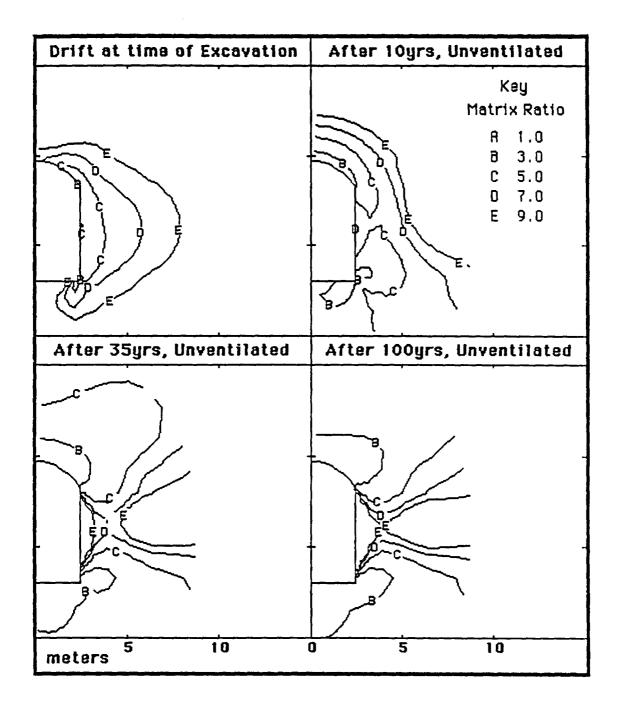


Figure A.3 Boundary-Element Predictions of the Ratio Between the Matrix Strength and Stress Around the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Drift

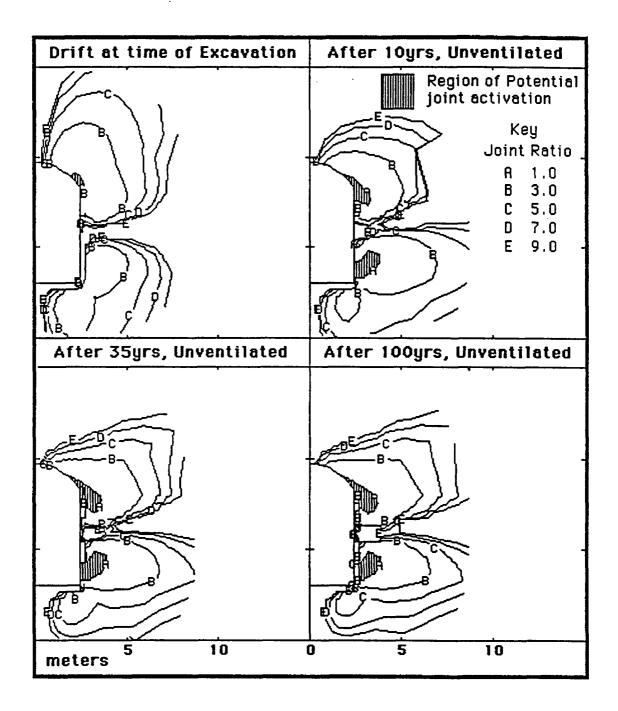


Figure A.4 Boundary-Element Predictions of the Ratio Between the Joint Strength and Stress Around the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Unventilated Drift

APPENDIX B

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FINITE-ELEMENT ANALYSES OF VERTICAL EMPLACEMENT

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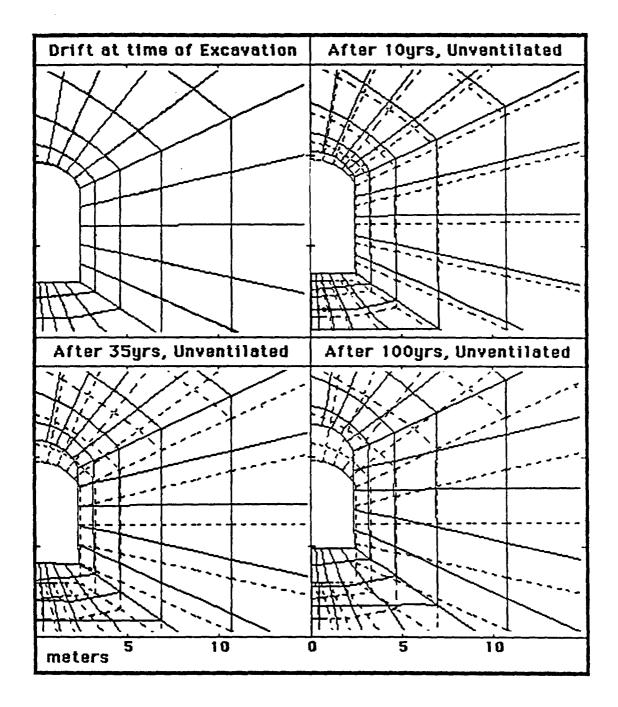


Figure B.1a Finite-Element Prediction of Deformations in the Vicinity of the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Case. (The displacement scale is magnified 20 times relative to the geometric scale. Deformed rock is shown with the solid lines. The deformations are measured relative to the pre-excavation state.)

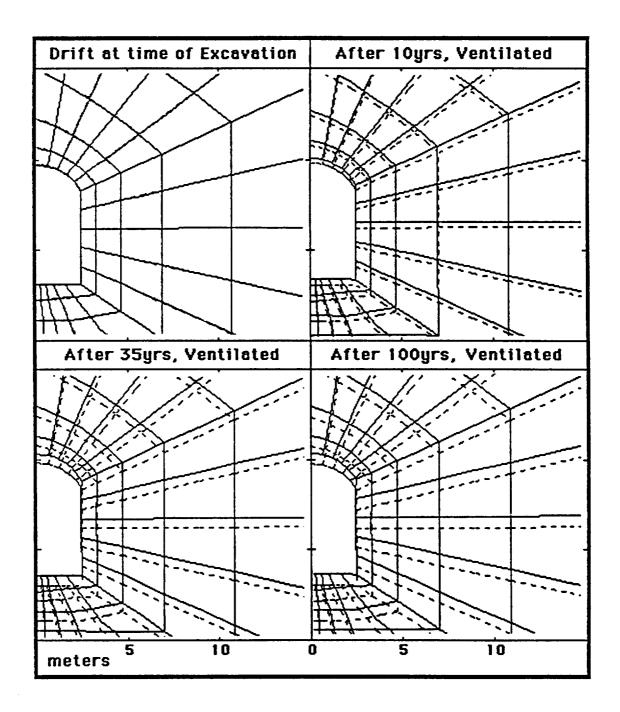


Figure B.1b Finite-Element Prediction of Deformations in the Vicinity of the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Ventilated Case. (The displacement scale is magnified 20 times relative to the geometric scale. Deformed rock is shown with the solid lines. The deformations are measured relative to the pre-excavation state.)

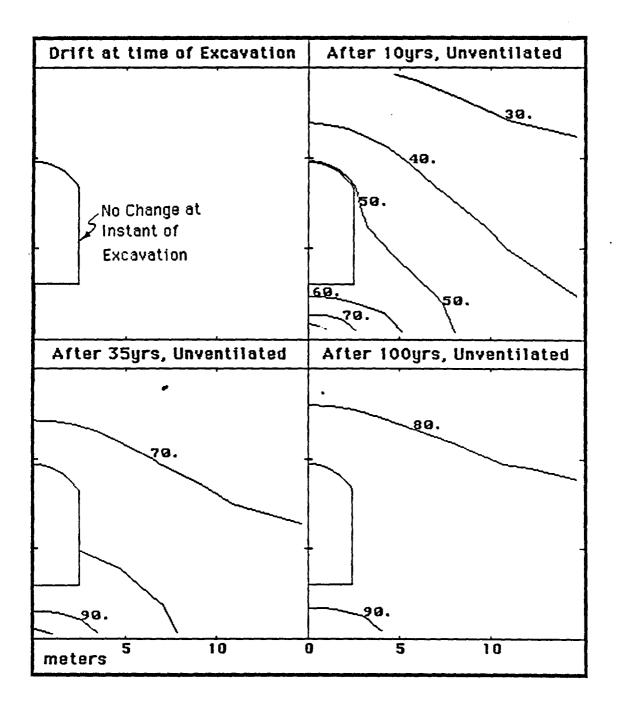


Figure B.2a Finite-Element Predictions of the Temperature Changes (°C) in the Vicinity of the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Case

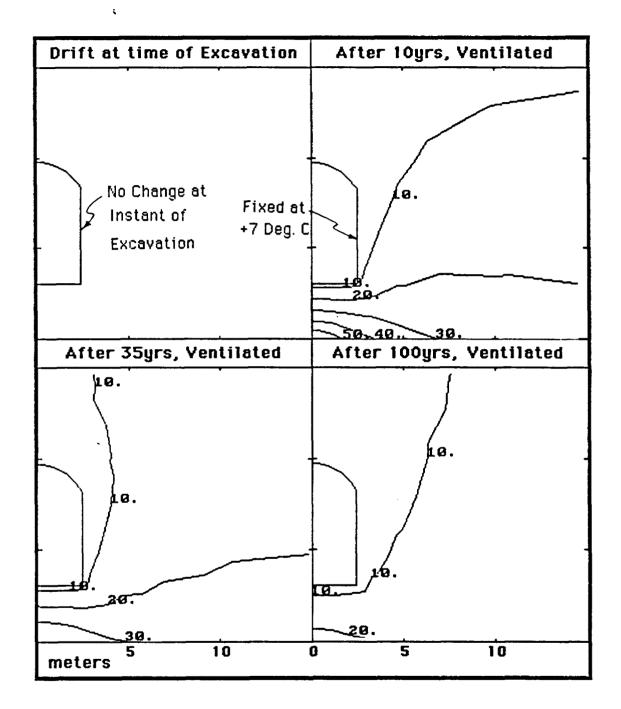


Figure B.2b Finite-Element Predictions of the Temperature Changes (°C) in the Vicinity of the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Ventilated Case

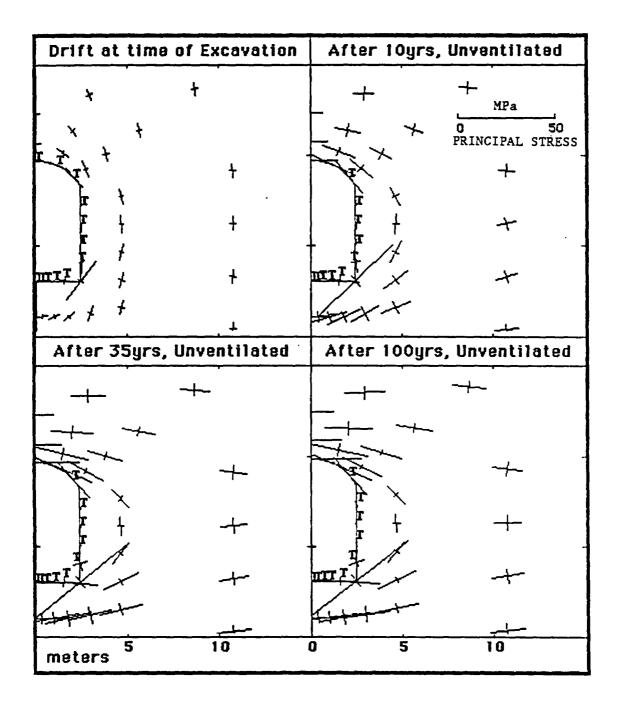


Figure B.3a Finite-Element Predictions of the Principal Stresses in the Vicinity of the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Case

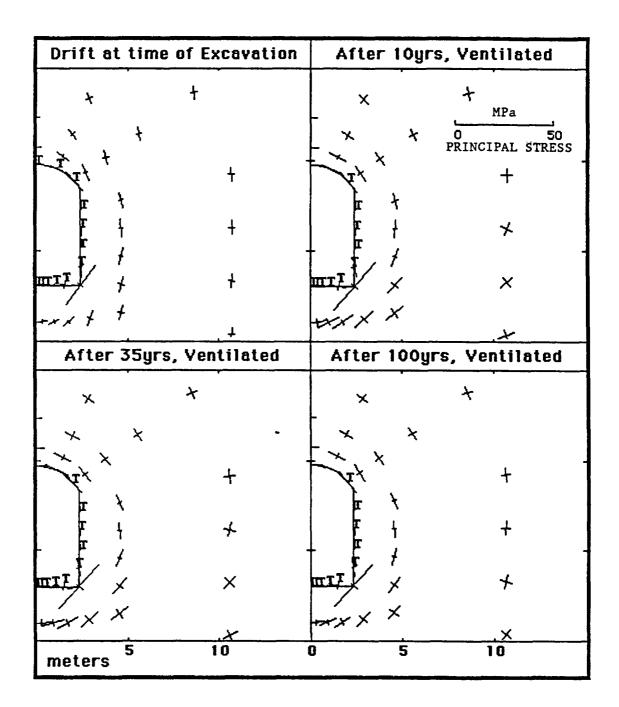


Figure B.3b Finite-Element Predictions of the Principal Stresses in the Vicinity of the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Ventilated Case

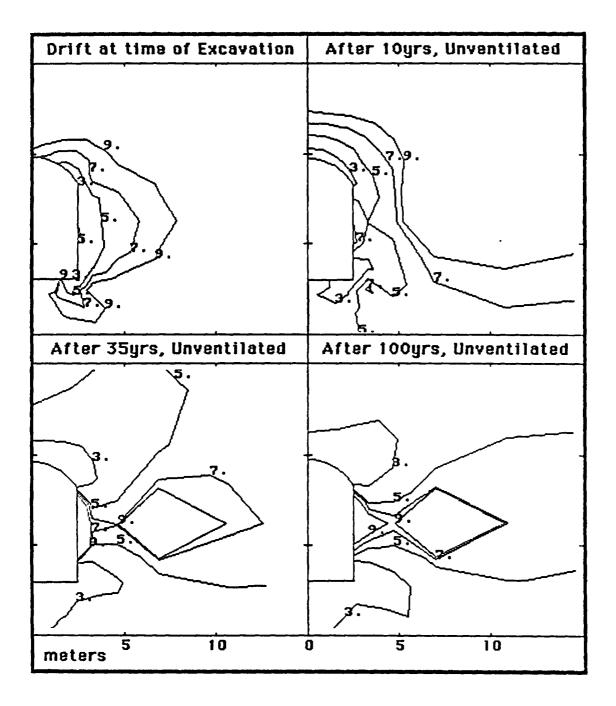


Figure B.4a Finite-Element Predictions of the Ratio Between Matrix Strength and Stress Around the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Unventilated Case

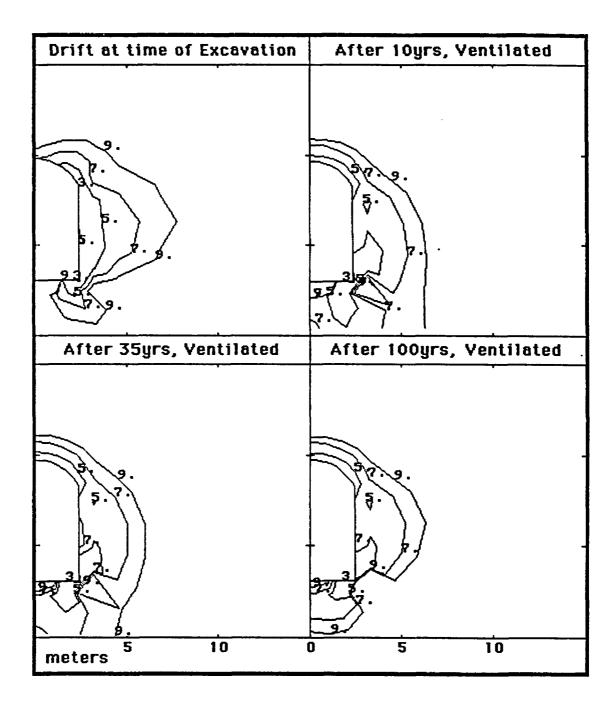


Figure B.4b Finite-Element Predictions of the Ratio Between Matrix Strength and Stress Around the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Ventilated Case

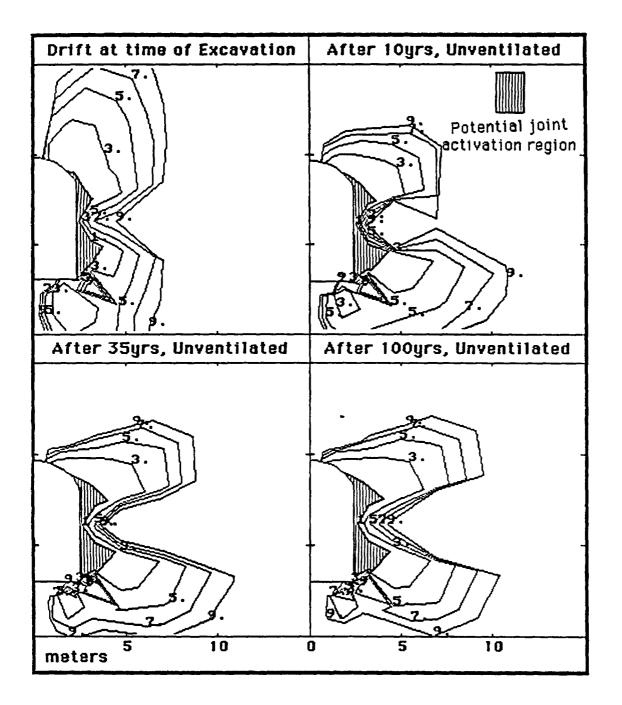


Figure B.5a Finite-Element Predictions of the Ratio Between Joint Strength and Stress Around the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Unventilated Case

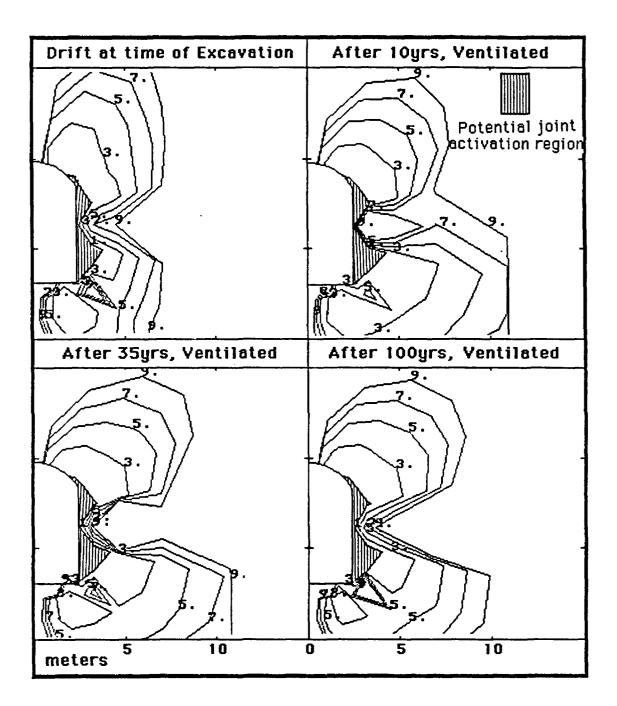


Figure B.5b Finite-Element Predictions of the Ratio Between Joint Strength and Stress Around the Vertical Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Ventilated Case



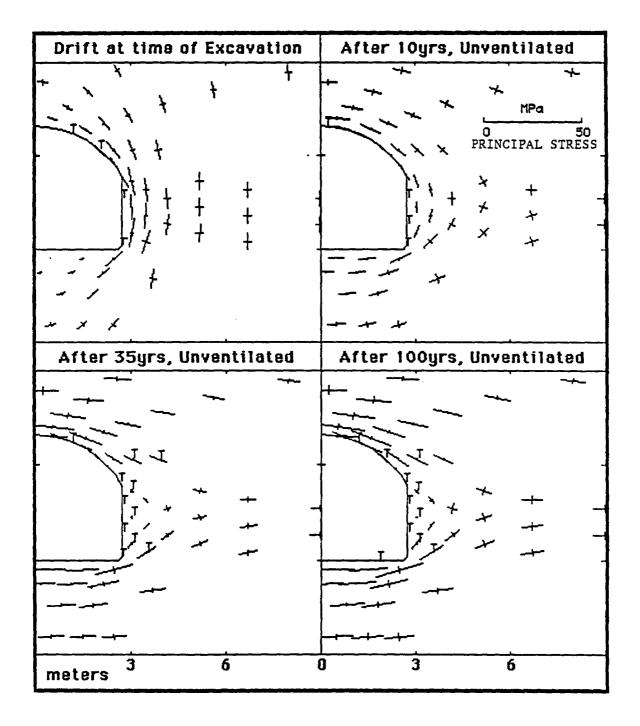
APPENDIX C

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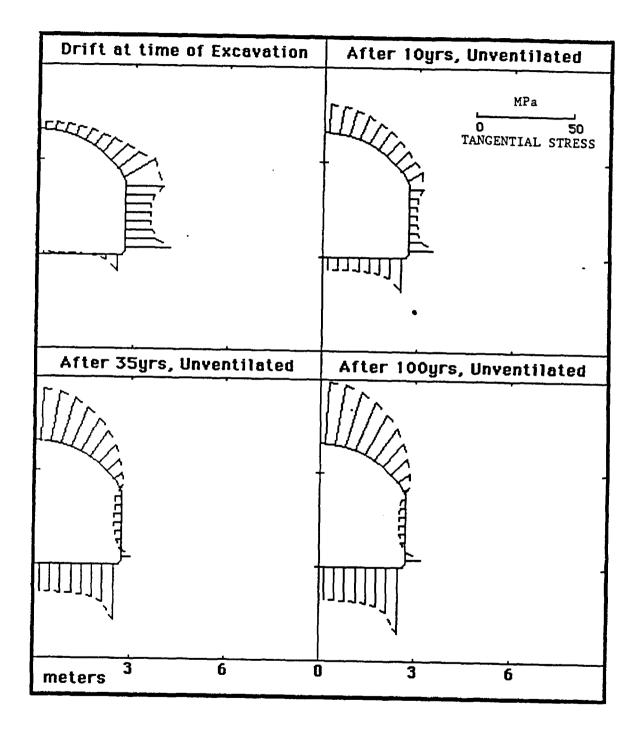
BOUNDARY-ELEMENT ANALYSES OF HORIZONTAL EMPLACEMENT

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Figure C.1 Boundary-Element Predictions of Principal Stresses in the Vicinity of the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Unventilated Drift



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Figure C.2 Boundary-Element Predictions of Tangential Stress Distribution Around the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Drift

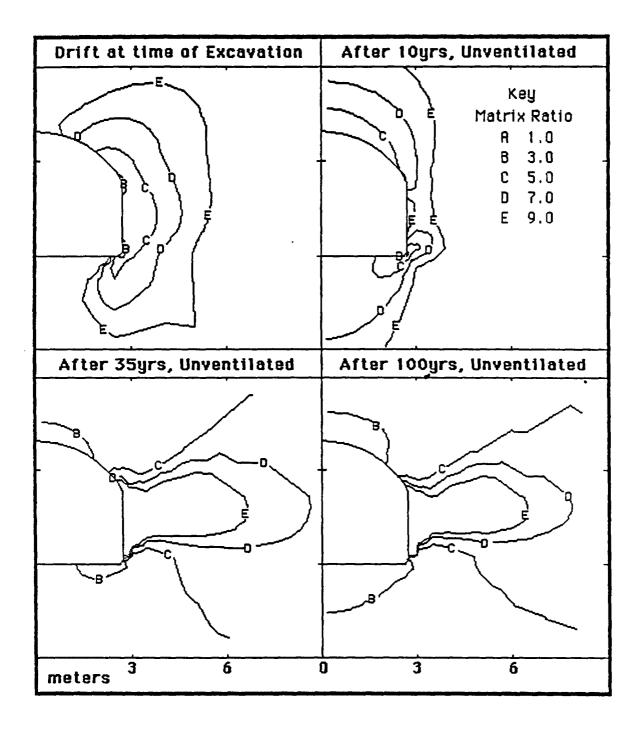


Figure C.3 Boundary-Element Predictions of the Ratio Between the Matrix Strength and Stress Around the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Drift

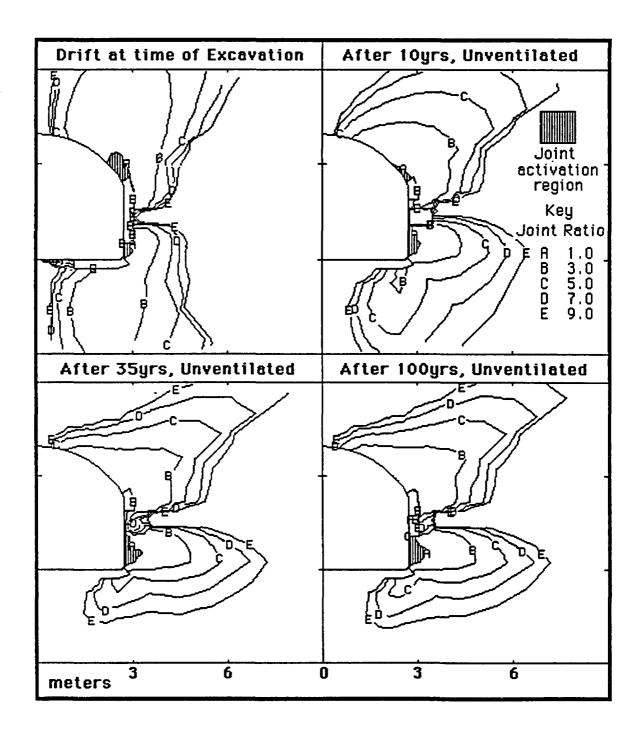


Figure C.4 Boundary-Element Predictions of the Ratio Between the Joint Strength and Stress Around the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Drift

APPENDIX D

FINITE-ELEMENT ANALYSES OF HORIZONTAL EMPLACEMENT

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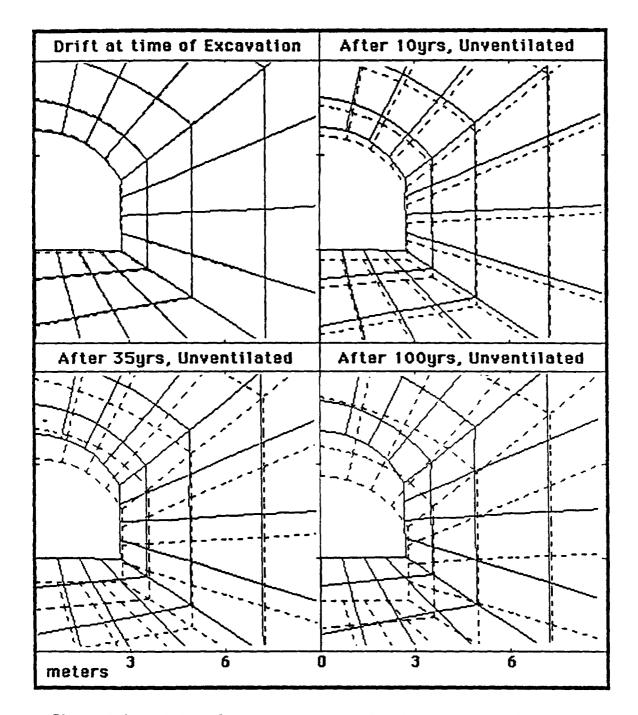
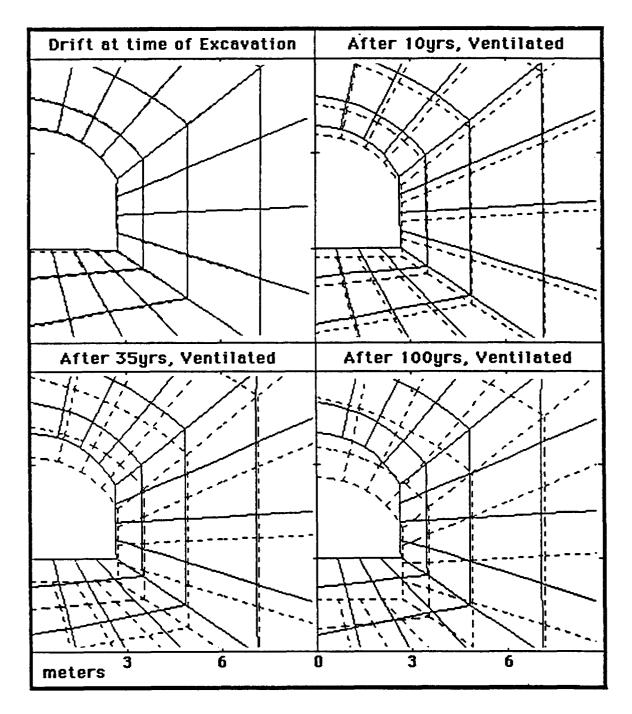


Figure D.1a Finite-Element Prediction of Deformations in the Vicinity of the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Case. (The displacement scale is magnified 20 times relative to the geometric scale. Deformed rock is shown with the solid lines. The deformations are measured relative to the pre-excavation state.)



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Figure D.1b Finite-Element Prediction of Deformations in the Vicinity of the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Ventilated Case. (The displacement scale is magnified 20 times relative to the geometric scale. Deformed rock is shown with the solid lines. The deformations are measured relative to the pre-excavation state.)

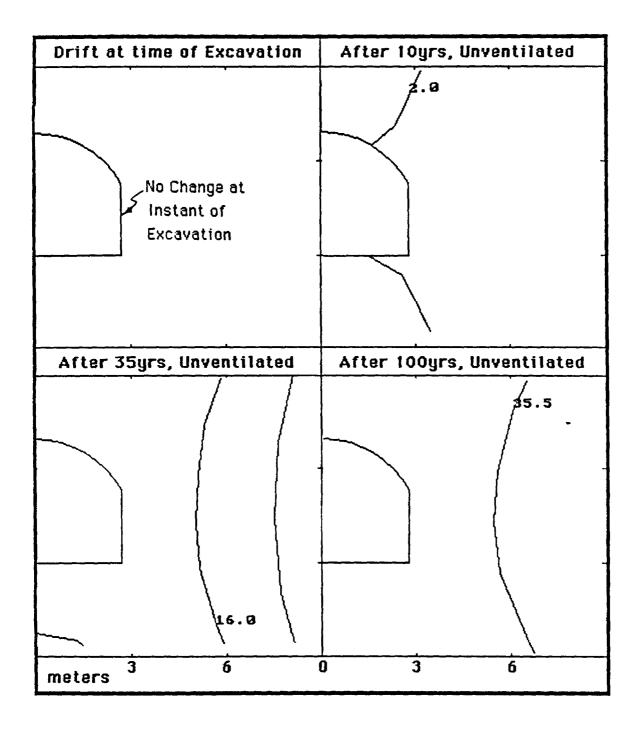


Figure D.2a Finite-Element Predictions of the Temperature Changes (°C) in the Vicinity of the Drift for the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Unventilated Case

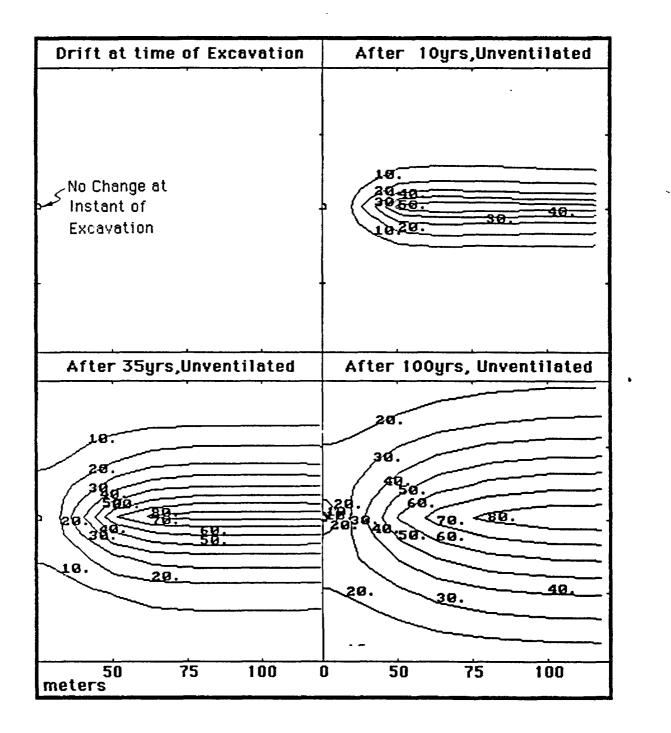


Figure D.2b Finite-Element Predictions of the Temperature Changes (°C) in the Far-Field of the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Case

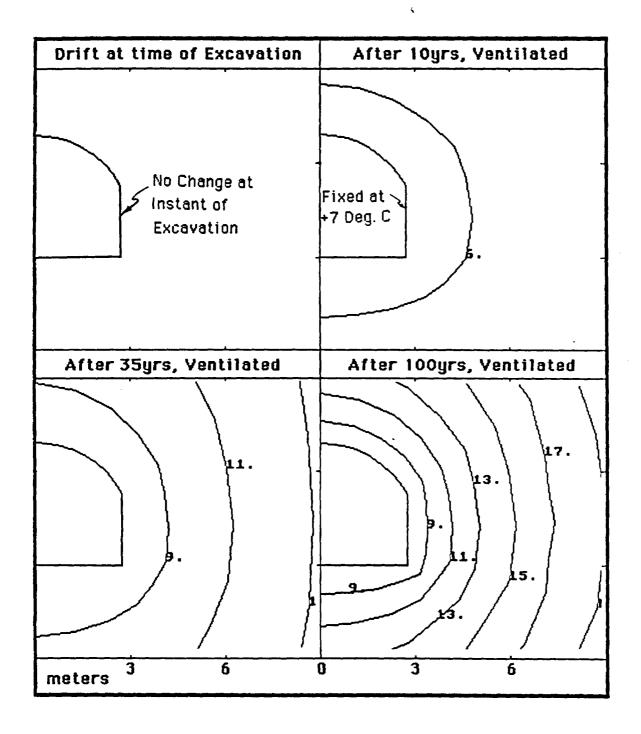
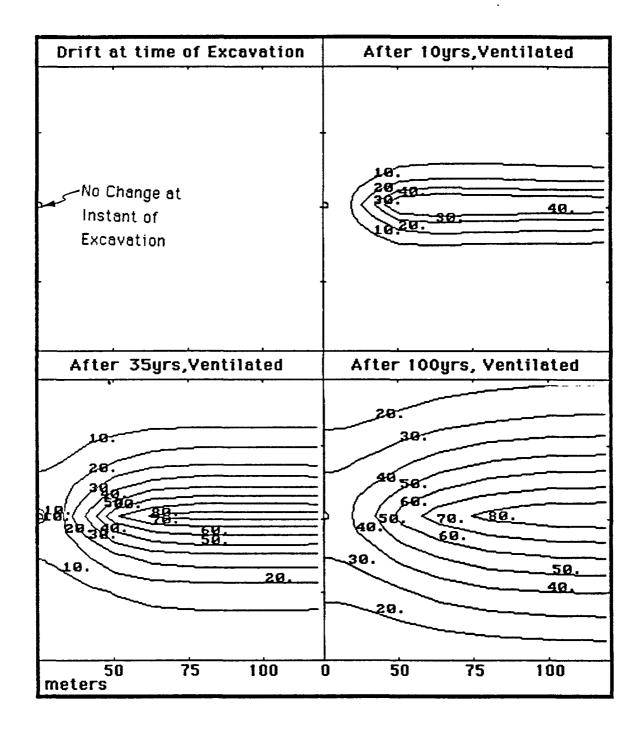


Figure D.2c Finite-Element Predictions of the Temperature Changes (°C) in the Vicinity of the Drift for the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement -Ventilated Case



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Figure D.2d Finite-Element Predictions of the Temperature Changes (°C) in the Far-Field of the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Ventilated Case

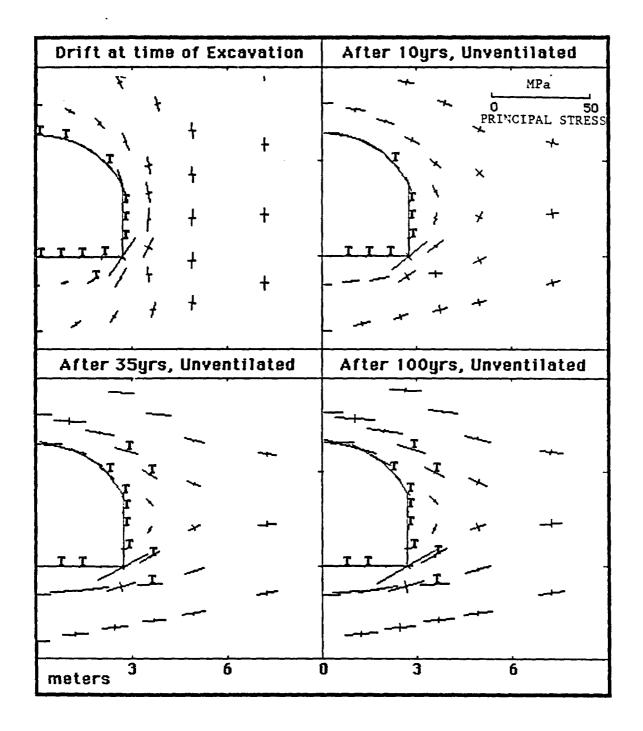


Figure D.3a Finite-Element Predictions of the Principal Stresses in the Vicinity of the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Case

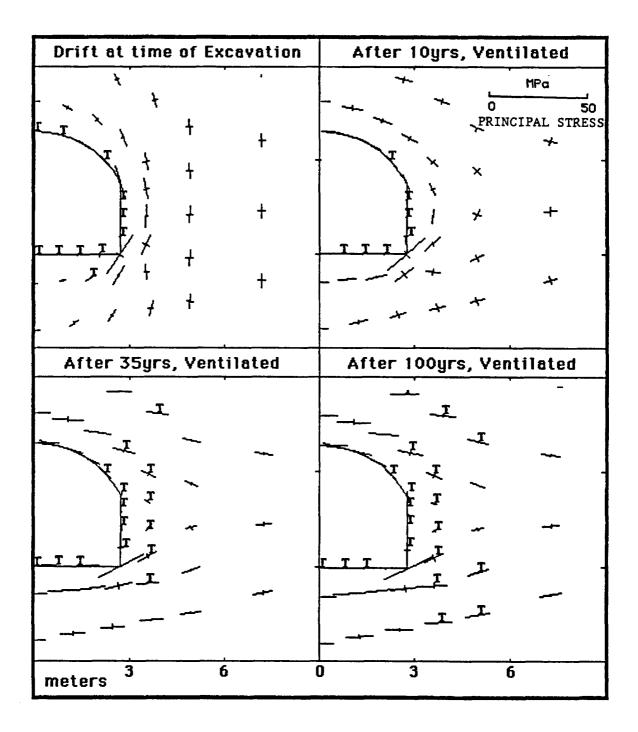


Figure D.3b Finite-Element Predictions of the Principal Stresses in the Vicinity of the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Ventilated Case

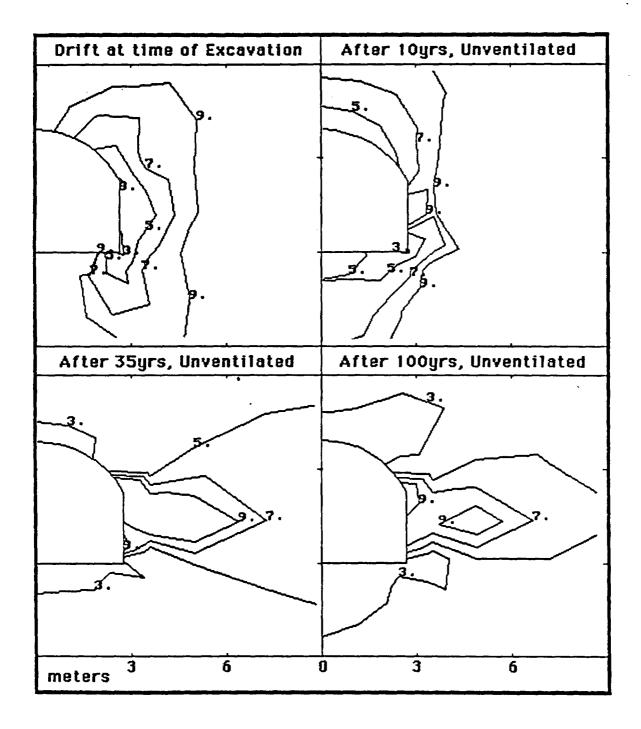


Figure D.4a Finite-Element Predictions of the Ratio Between Matrix Strength and Stress Around the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Case

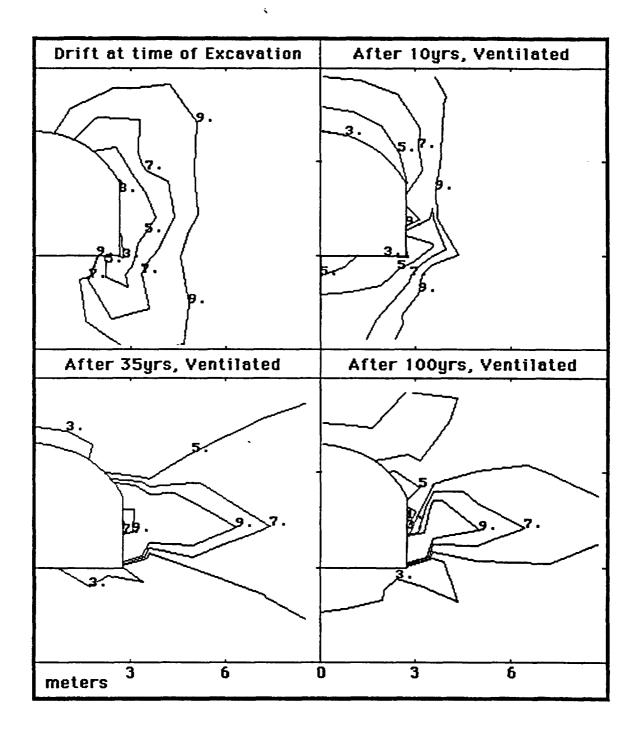


Figure D.4b Finite-Element Predictions of the Ratio Between Matrix Strength and Stress Around the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Ventilated Case

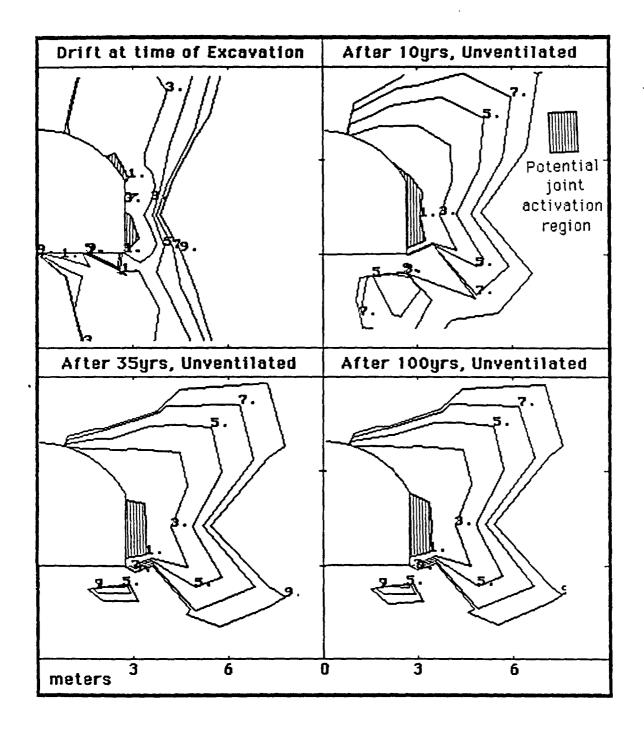


Figure D.5a Finite-Element Predictions of the Ratio Between Joint Strength and Stress Around the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Unventilated Case

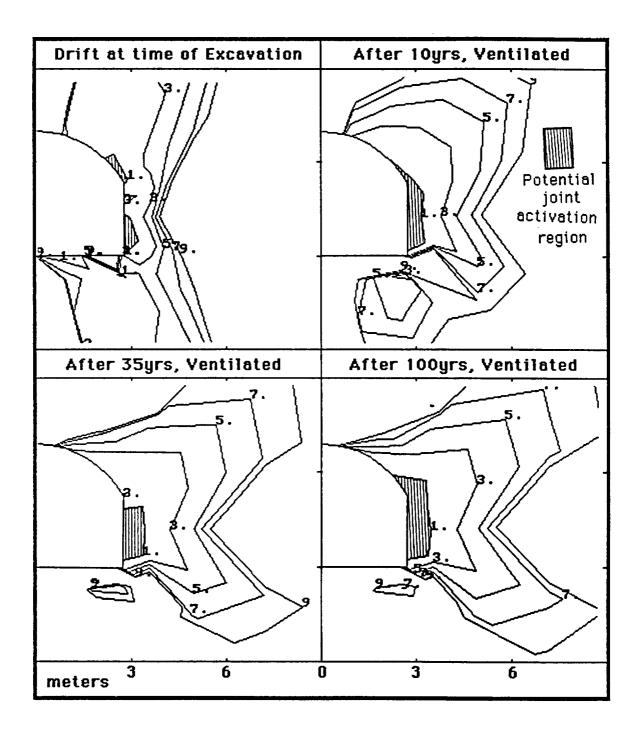


Figure D.5b Finite-Element Predictions of the Ratio Between Joint Strength and Stress Around the Horizontal Emplacement Drift, at Times up to 100 Yr After Waste Emplacement - Ventilated Case

APPENDIX E

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MEMO: MANSURE, A.J.,

"ALLOWABLE THERMAL LOADING AS A FUNCTION OF WASTE AGE"

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Sandia National Laboratories

Albuquerque, New Mexico 87185

date: 2/13/1985

to: Hill, 6311

arthin Marsure A.J. Mansure, 6314 from:

subject: Allowable Thermal Loading as a Function of Waste Age

INTRODUCTION:

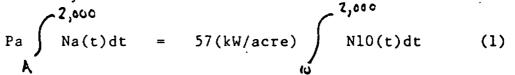
During the unit evaluation calculations were done to determine the allowable thermal loading and demonstrate that this loading did not have any adverse far-field effects (Johnstone, 1984). Based on that study 57 kW/acre has been used as a baseline for calculations and for design of the underground facility layout. The unit evaluation assumed 10 year out of the reactor spent fuel. Thermal decay curves used during the unit evaluation were taken from Kissner (1978).

The criteria used to determine the allowable thermal loading in the unit evaluation was that the drift floor temperature should not exceed 100oC for vertical emplacement. That criteria was not based upon any firm requirement for ventilation or retrieval. In addition the vertical emplacement floor temperature is dependent upon such things as drift thermal loading (kilowatts per meter of drift or the output of the canisters divided by the spacing between the boreholes) and upon the standoff between the canister and the drift floor. Further, the same allowable thermal load was used for horizontal and vertical emplacement although horizontal emplacement drift temperatures are expected to be much lower than vertical emplacement drift temperatures because of larger standoffs between the waste and the drift for horizontal emplacement. Thus the allowable thermal load is being reevaluated.

This memorandum considers the effect of waste age on allowable thermal loading. O'Brien and Shirley (1984) analyzed both constant initial areal power densities and "constant borehole spacings" for wastes of different ages. They found that "constant borehole spacing" gave nearly the same areal energy deposition for waste ages 5 to 30 years, where as, constant initial areal power density resulted in greatly different areal energy densities. They thus recommended emplacement at "constant borehole spacing". (Their recommendation is for fixed package size, ie. number of assemblies per package.)

BASIS FOR COMPARING THERMOMECHANICAL EFFECTS OF WASTE OF DIFFERENT AGES

It has been assumed that far-field thermomechanical effects are the determining factor for the allowable thermal load. Surface uplift can be used as an indicator of thermomechanical effects. Surface uplift peaks at about two thousand years (see Figure 1, Brandshaug, 1983). Thus for waste that is not 10 years out of the reactor to produce the same thermomechanical effects as ten year old SF, it should be emplaced at an initial areal power density that results in approximately the same accumulated areal energy deposition through 2000 years. The allowable initial power density as a function of waste age (for a given burnup) is thus determined by



where Pa is the initial power density of waste of age A at emplacement, Na is the normalized power function for waste of age A, and N10 is the normalized power function for waste of age 10 years.

These integrals were evaluated numerically by fitting each pair of data points with an exponential so the normalized energy between any two times is given by

(N1-N2)*(T2-T1)/ln(N1/N2)

The accuracy of this approach for evaluating the integrals was assessed by comparing to a trapezoidal integration (which would systematically lead to an over estimation). The trapezoidal integration was 1.1% higher.

The assumption that the integrals should be evaluated through 2000 years was checked by also evaluating them through 1000 years. When the initial power density was calculated with that assumption, the allowable power density was 100.7% or less of the initial power density determined using 2000 years.

ALLOWABLE THERMAL LOADING

The data for determining the allowable initial power density was taken from the GR (DOE, 1984). That data is not the same as used in the unit evaluation which came from Kissner (1978). In general the data in the GR does not decay as fast as the data used in the unit evaluation (see Table 1). Therefore, the right hand side of equation 1 was evaluated using the data from Kissner (1978) to insure the amount of energy deposited in 2000 years was no more than that used in the unit evaluation.

Using the data in Table 1 the allowable initial power density Pa was calculated using equation 1. Values determined are summarized in Table 2 and Figure 2.

Most of the increased allowable thermal loading for younger

waste in Figure 2 is due to the higher thermal output per MTU of younger waste. The point made by Figure 2 is not to emplace younger waste so as to deposit more energy per acre, but to compensate for the higher initial outputs of younger waste so as to achieve the same energy density. O'Brien and Shirley (1984) suggest that the way to achieve almost constant energy density is "constant borehole spacing". That of course assumes several other variables such as the number of assemblies per canister are constant. A better way to express that same concept is constant assemblies per acre rather than "constant borehole spacing."

The number of assemblies per acre can be determined from the allowable initial thermal loading according to

ASSMB*(MTU/assmb)*P(A) Na(t)dt = (57kW/acre) N10(t)dt

where ASSMB is the number of assemblies allowed per acre, (MTU/assmb) is the number of MTU per assembly (=.4614 for PWR and =.1833 for BWR, O'Brien, 1985), and P(A) is the power per MTU at age A.

Table 2 gives the number of assemblies per acre allowed for average age and burnup spent fuel (average burnups are 33,000 MWD/MTU for PWR and 27,5000 MWD/MTU for BWR, O'Brien, 1985). In contrast to allowable initial power density, the number of assemblies per acre is relatively constant and decreases with waste age at emplacement. This is because the younger waste over time produces more energy and so has to be spread out farther. This measure of allowable thermal loading may also be reasonable for high burnup fuel (Mansure, 1985).

	Unit	GR data				F C C C C C C C C C C	
Year	Evaluation *	PW **	R *	BWF **	*		
5		1798		1380			
5 6 7 8 9		1534		1193			
7		1375		1079			
8		1270		1004			
		1196		951.5			
10	1	1140	1	911.3	1		
16		949.2	.833	772.7	.848		
18		907.6	.796	741.5	.814		
20	.75	870.5	.764	713.5	.783		
25 30	.681	790.7 723.1	.694 .634	651.9 598.9	.715		
40	.622 .525	612.2	.537	510.6	.657 .56		
50	.449	524.8	.46	440.1	.483		
60	.387	454.8	.399	383.5	.405		
70		398.5	.35	337.7	.371		
80	.301	352.9	.31	300.5	.33		
90		315.8	.277	270.2	.297		
100		285.6	.251	245.5	.27		
110	.238						
200	.137	160.1	.14	142.1	.156		
300	.108	126.4	.111	113.5	.125		
400 500	.0919 .0806	107.5 93.79	<pre>.0943 .0823</pre>	97.19 85.03	107 .0933		
600	.0711	37.12	.0025	\$7. 05	.0933		
700	.0633						
800	.0569						
900	.0514						
1000	.0466	54.71	.048	49.9	.0547		
2000	.0247	29.18	.025	26.81	.029		

Table 1. Data used to evaluate allowable initial power density

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* Data normalized to 10 year old output.
** Watts/MTU

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Table 2. ALLOWABLE LOADING

••••••	-kW/acı	re- 1	-assemblies/acre-			
YEAR	PWR	BWR	PWR	BWR		
5	84.6	73.8	102	291		
6	72.8	64.3	103	294		
7	65.8	58.5	104	296		
6 7 8 9	61.2	54.8	104	298		
9	58.0	52.3	105	300		
10	55.6	50.3	106	301		
11	53.6	48.7	106	303		
12	52.1	47.5	107	304		
13	50.9	46.4	108	306		
14	49.8	45.6	108	308		
15	48.9	44.8	109	309		
16	48.0	44.1	110	311		
17	47.2	43.4	110	313		
18	46.5	42.8	. 111	314		
19	45.8	42.2	111	316		
20	45.1	41.6	112	317		
21	44.4	41.0	113	319		
22	43.8	40.5	113	320		
23	43.2	39.9	114	322	•	
24	42.5	39.4	114	323		
25	41.9	38.8	115	324		
26	41.3	38.3	115	326		
27	40.7	37.8	115	327		
• 28	40.1	37.3	116	328		
29	39.5	36.7	116	329 .		
30	39.0	36.2	117	330		

E-6

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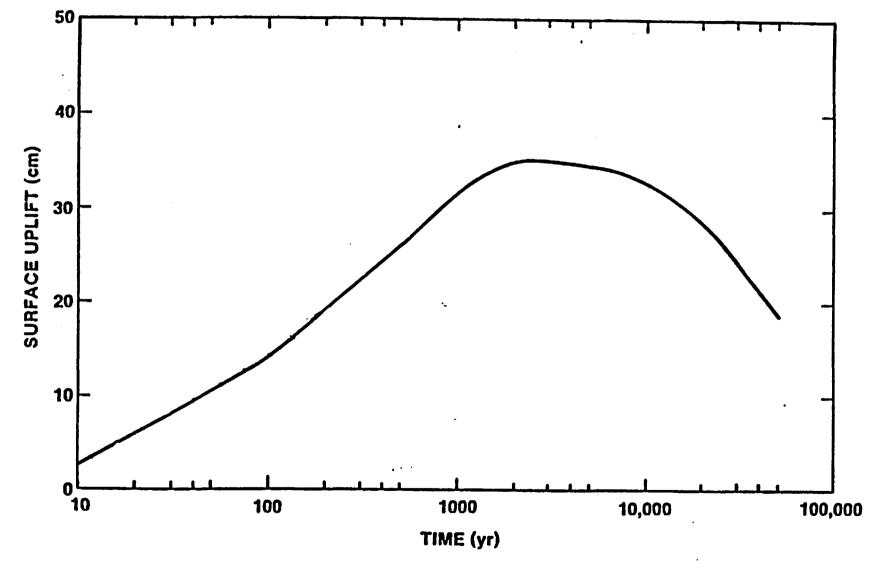
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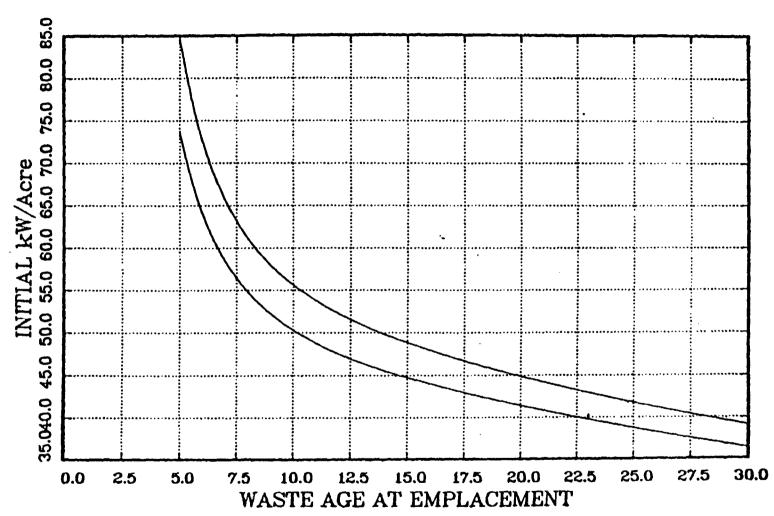
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Figure 1. Surface Uplift for 10 Year Old SF Emplaced in the Topopah Spring Member at 57 kW/acre (Brandshaug, 1983)

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ALLOWABLE THERMAL LOADING

Figure 2. Allowable Initial Power Density

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APPENDIX F

MEMO: MANSURE, A.J. AND R.E. STINEBAUGH, "MEMORANDUM OF RECORD OF INSTRUCTIONS FOR THERMAL DESIGN ANALYSIS AND PERFORMANCE ASSESSMENT LAYOUT."

DEC 11 1986

Sandia National Laboratories P.O. Box 5800 Albuquerque, NM 87185

Date: 4/18/1985

To: R. Hill, 6311

From: A.J. Mansure and R.E. Stinebaugh, 6314 AJM RES

Subject: Memorandum of Record of Instructions for Thermal Design, Analysis, and Performance Assessment of Layout, Version I

This memo establishes reference design parameters to be used in thermal, thermomechanical, and hydrologic calculations. Such parameters include both underground facility design criteria parameters (canister diameter) and the results of the design process (drift dimensions).

Parameters that establish the reference design are presented in this memo in three sections:

 section one contains underground facility design criteria parameters (this section will be superceded by the Functional Design Criteria when it is published). Many of the numbers in this section such as borehole diameter are important to calculations.

2) section two contains guidance to the underground facility design A&E. This section does not contain parameters that are to be used in calculations. It is included in this memo for completeness of documentation of the reference design.

3) section three contains layout dimensions established by Parsons Brinkerhoff. These dimensions constitute the current and reference design and should be used in calculations.

1. Design Criteria Parameters

The following criteria are to be used establishing borehole spacings and the layout of the underground facility to ensure thermal conditions are satisfactory.

1.1 Temperature:

- borehole wall temperature for spent fuel is not to exceed 220 deg. centigrade for all times
- temperature one meter from the borehole wall is not to exceed 200 deg. centigrade for all times
- horizontal emplacement drift rock wall temperature (long borehole case) is not to significantly exceed 50 deg. centigrade at 50 years for spent fuel
- vertical emplacement access drift rock wall temperature (panel access drift) is not to significantly exceed 50 deg. centigrade at 50 years for spent fuel

1.2 Waste Package, Equipment, and Facility Dimensions:

- 1.2.1 Spent Fuel
 - canister diameter 26" canister length 15 ft.
- 1.2.1.1 horizontal borehole
 - diameter 33"
 - length approximately 682 ft liner id 31"

 - liner thickness .5"
 - dolly length 16.5 ft.
 - unencumbered drift dimensions (dimensions dictated by equipment clearences are included) see attached Figure 1.
 - minimum borehole spacing for emplacement 8 ft.

1.2.1.2 vertical borehole

- diameter 29" (counter bored above canister)
- counter bore diameter 34"
- depth 25 ft.
- bottom 14 ft. (not lined)
- unencumbered drift dimensions (dimensions dicated by equipment clearences are included) see attached Figure 2
- minimum borehole spacing for mining and emplacement 7.5 ft.

1.2.2 DHLW & WVHLW

- canister diameter 26" canister length 10 ft.

1.2.2.1 horizontal borehole

- diameter 33"
- maximum length approximately 682 ft.
- dolly length 11.5 ft. liner id 31"
- liner thickness .5"
- unencumbered drift dimensions (dimensions dictated by equipment clearences are included) see attached
- minimum borehole spacing for emplacement 8 ft.

1.2.2.2 vertical borehole

- diameter 29"
- depth 20 ft.
- unencumbered drift dimensions (dimensions dictated by equipment clearences are included) see attached Figure 4
- minimum borehole spacing for mining and emplacement 5 ft.

1.3 High burnup fuel emplaced at equivalent assemblies per acre as average burnup fuel (Mansure, 1985a).

1.4 Overall thermal loading (assemblies per acre or initial kilowatts per acre) for SF is to be equivalent to 57 kW/acre - 10

year old waste (Johnstone, 1984) as described in "Allowable Thermal Loading as a Function of Waste Age" (Mansure, 1985b). Overall thermal loading for DHLW ---TBD---?

1.5 SF thermal decay characteristics should be based on the decay functions given in "Thermal Decay Curves for PWR and BWR SF Waste" (Mansure, 1985c). DHLW thermal decay characteristics should be based on the decay functions given by Peters (1983).

1.6 Rock properties: project baseline rock properties are given in "Recommended Matrix and Rock-Mass Bulk, Mechanical, and Thermal Properties for Thermomechanical Stratigraphy of Yucca Mountain" (Nimick, et. al. 1984). Thermal conductivity and heat capacity used do not have to agree with values in that report but should be traceable to that document and a clear argument should be developed as to how properties used result in a conservative design.

1.7 SF canister initial thermal outputs, age, and number per year should be as given in O'Brien (1985) case II.

1.8 Unless otherwise justified pillar space between drifts that are not continuously maintained (access drifts, ie. P/B panel drifts) should be about four times the drift width.

1.9 Determination (calculation) of thermal loading should be based upon P/B module drawings (see attached Figures 5 & 6).

2. Underground Facility Design Guidance

The above criteria result in the following design guidance for spent fuel. Should these design guidance be inconsistent with the criteria and good design practice SNL should be advised.

2.1 For horizontal emplacement a 115' standoff will achieve the 50 deg. centigrade at 50 years objective for both PWR and BWR independent of canister output, is for all waste ages and borehole spacings, as long as allowable loadings (Mansure, 1985b) are adhered to. Standoff used for horizontal emplacement should be about this distance.

2.2 For horizontal emplacement the hottest borehole wall conditions result from 8.55 year old PWR canisters. This is the youngest aged for which PWR can have 6 assemblies per canister and still meet the 3.4 kW/canister loading limit presently being assumed for design of the canister (O'Brien, 1985). For this age PWR and canister output, the borehole wall and rock temperature at 1 meter calculated are 212 and 168 deg. centigrade. The borehole spacing used in this calculation is 36 meters. This borehole spacing is based upon the above criteria and the standoff in 2.1. Thus for horizontal emplacement the present criteria, especially the canister output and the dolly length, result in thermal conditions that automatically satisfy 1.1, borehole wall and 1 meter temperature, if 1.4, 1.9 and 2.1 are adhered to. Therefore adequate data and analysis exist to proceed with the horizontal layout using 1.9 to establish borehole spacing.

2.3 Vertical emplacement is much more complex thermally than horizontal emplacement. For vertical emplacement it is possible to violate borehole wall temperature. Adequate analysis have not been done to delineate how to establish vertical emplacement borehole and drift spacing.

2.4 Vertical emplacement drift temperature calculations have shown the following:

- For the same standoff between the waste and the panel access drifts, waste of varying age emplaced at the allowable thermal loading (Mansure, 1985b) results in essentially the same drift temperature at 50 years.
- For the same standoff between the waste and the panel access drifts, waste emplaced at the same loading but with different borehole and emplacement drift spacings results in essentially the same drift temperature at 50 years.

Based upon these two facts, it is reasonable to use the same standoff (actual standoff will vary slightly to keep number of boreholes an integer) for all waste ages and to determine this standoff prior to determining borehole spacing. This is convenient since borehole spacing is dependent upon drift spacing.

2.5 For Vertical emplacement, panel access drift temperature is dependent not only upon the standoff between the waste and the drift, but also the pillar between the drifts. If the pillar is very small, then the standoff must be bigger to compensate. The * tradeoff is not one to one, but if the the sum of twice the standoff, twice the drift width and the width of the pillar is constant, then the temperature only varies a few degrees. Minimizing the pillar width does not necessarily result in the minimum mining. Recommended dimensions are about 23.75m for both the standoff and the pillar width. This makes the pillar about 4 times the drift width for a 6.25m drift and makes the pillar width equal to the standoff.

3. Reference Layout Dimensions

Current layout dimensions that result from the above criteria and guidance have been established by P/B. These are summarized below and should be used in design analysis and performance assessment.

3.1 Excavated dimensions for spent fuel

3.1.1 - horizontal emplacement

- standoff 102 ft.
- panel width 1400 ft.
- number of canisters per borehole 35
- emplacement drift width 18 ft.
- emplacement drift height 13 ft.
- alcove face to face distance 31 ft. (a equipment criteria number)

3.1.2 - vertical emplacement

- standoff 77.5 ft. - barrier pillar width 63 ft. - mid panel drift width 16 ft. - mid panel drift height 22 ft... - emplacement drift width 16 ft. - emplacement drift height 22 ft. - access drift width 21 ft. - access drift height 14 ft. - panel width 1400 ft. 3.2 Excavated dimensions for DHLW and WVHLW 3.2.1 - horizontal emplacement - standoff -TBD- (10 to 102 ft.) - module width varies (up to 700 ft.) - number of canisters per borehole -TBD-- emplacement drift width 26 ft. * - emplacement drift height 13 ft. 3.2.2 - vertical emplacement (double row of boreholes with 7' between rows and 5' between boreholes in a row) - standoff -TBDbarrier pillar width 40 ft. *
emplacement drift width 16 ft. - emplacement drift height 18 ft. - access drift width 21 ft. - access drift height 13 ft.

- panel width varies (up to 700 ft.)

* Subject to thermostructural review.

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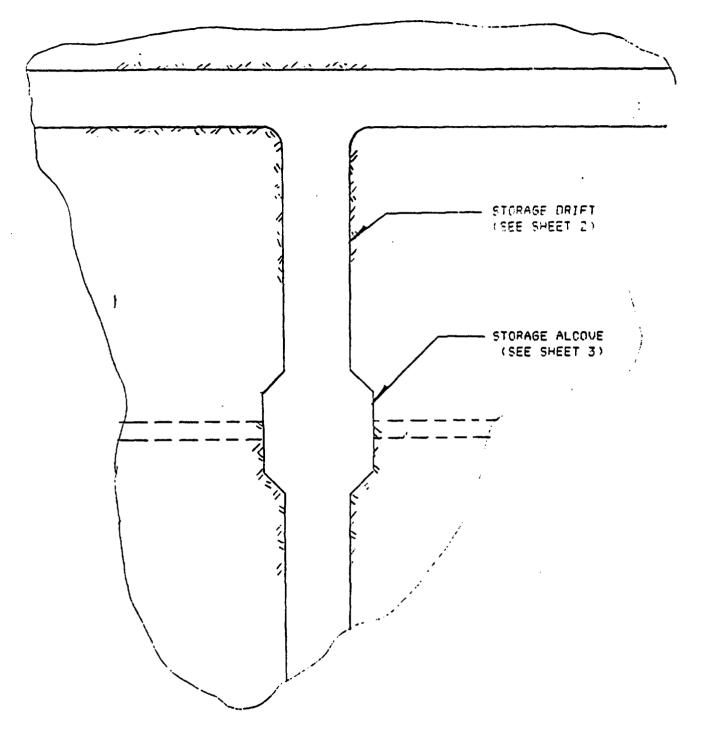
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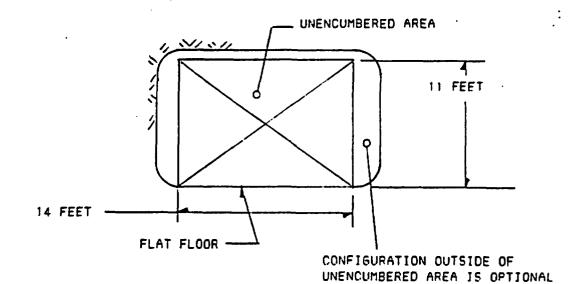
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MINING REQUIREMENTS FOR HORIZONTAL EMPLACEMENT OF SPENT FUEL

FIGURE 1 SHEET 1 OF 3



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FIGURE 1 SHEET 2 OF 3

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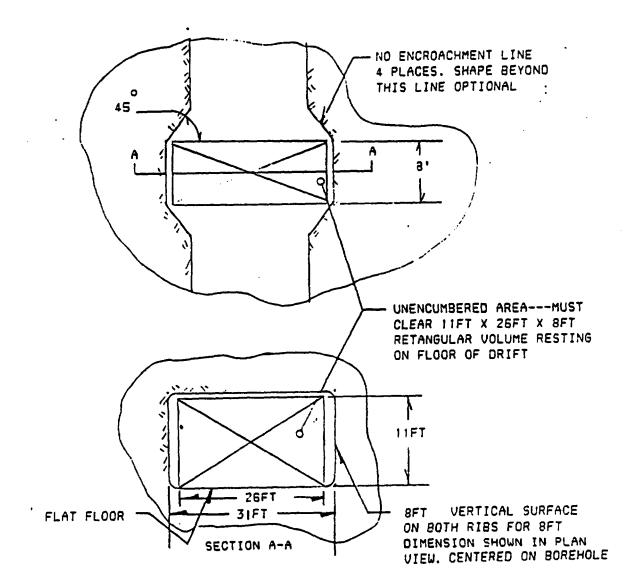
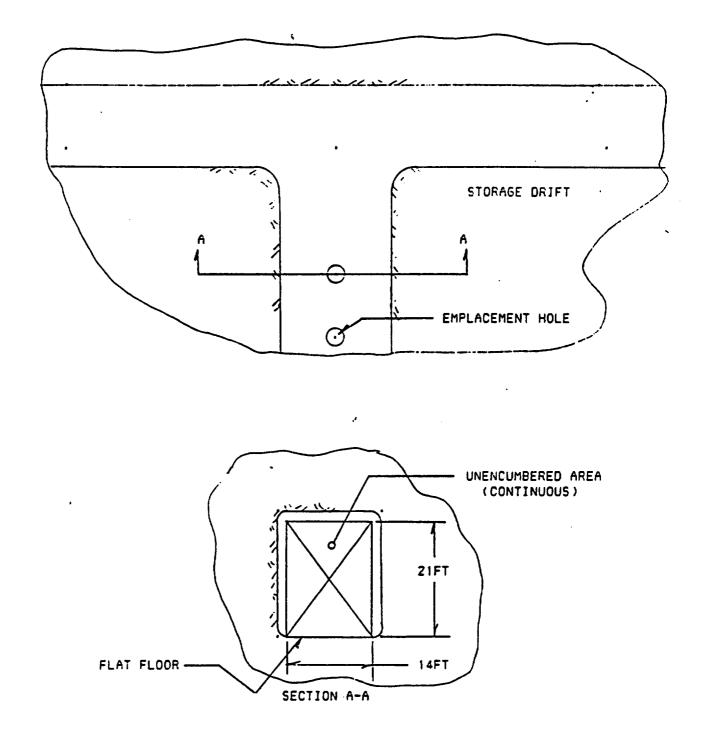
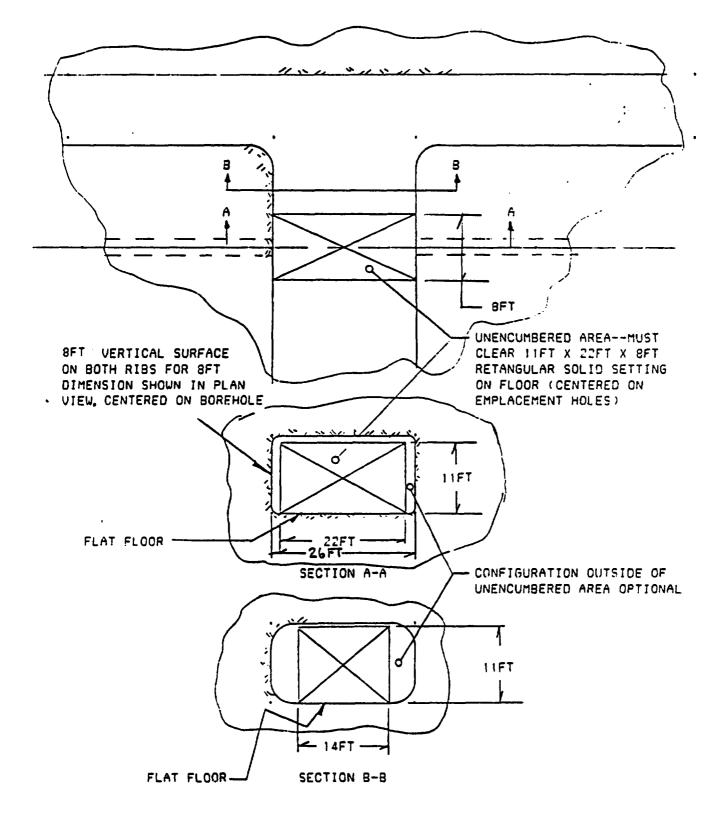


FIGURE 1 SHEET 3 OF 3

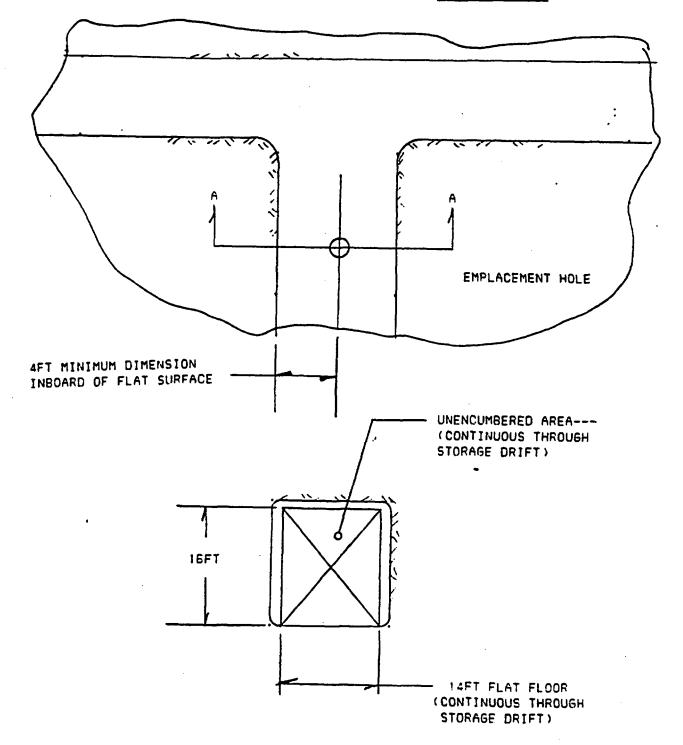
MINING REQUIREMENTS FOR VERTICAL EMPLACEMENT OF SPENT FUEL





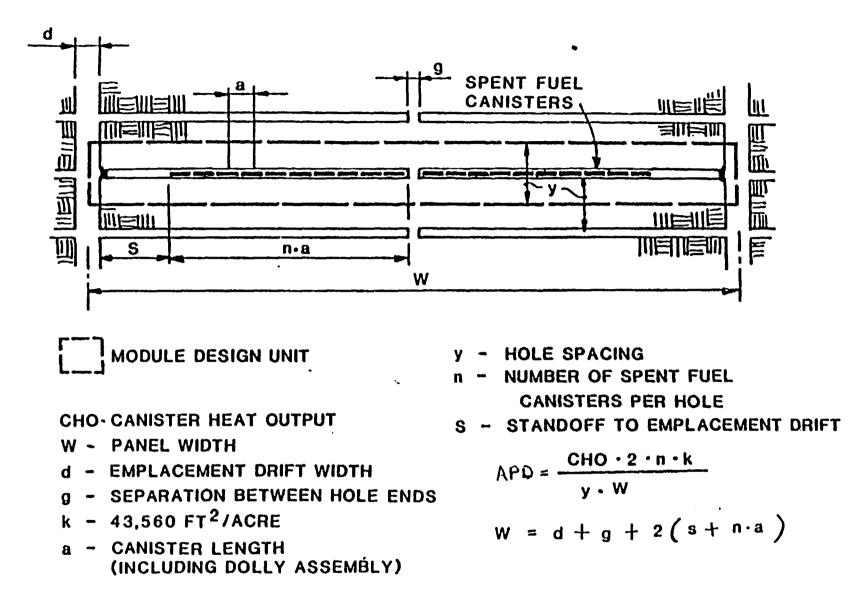
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FIGURE 3



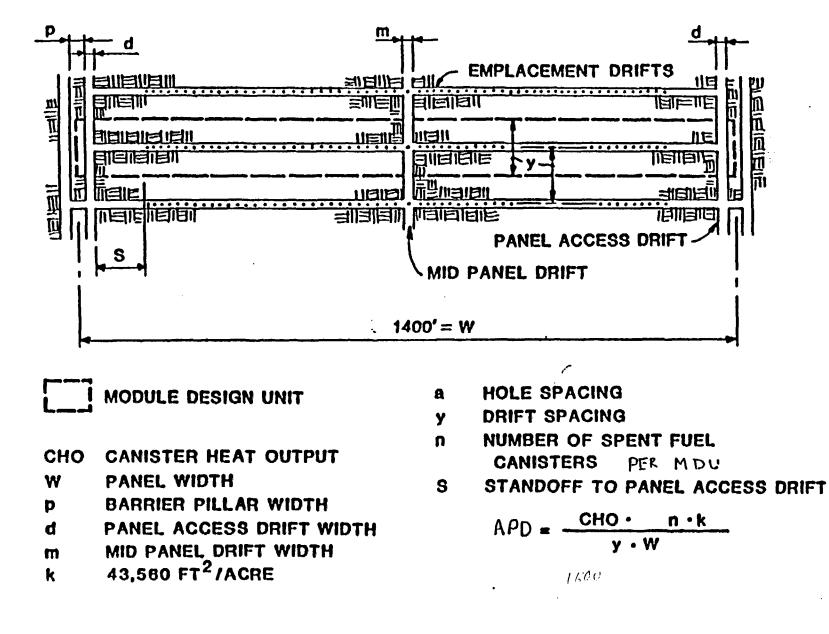
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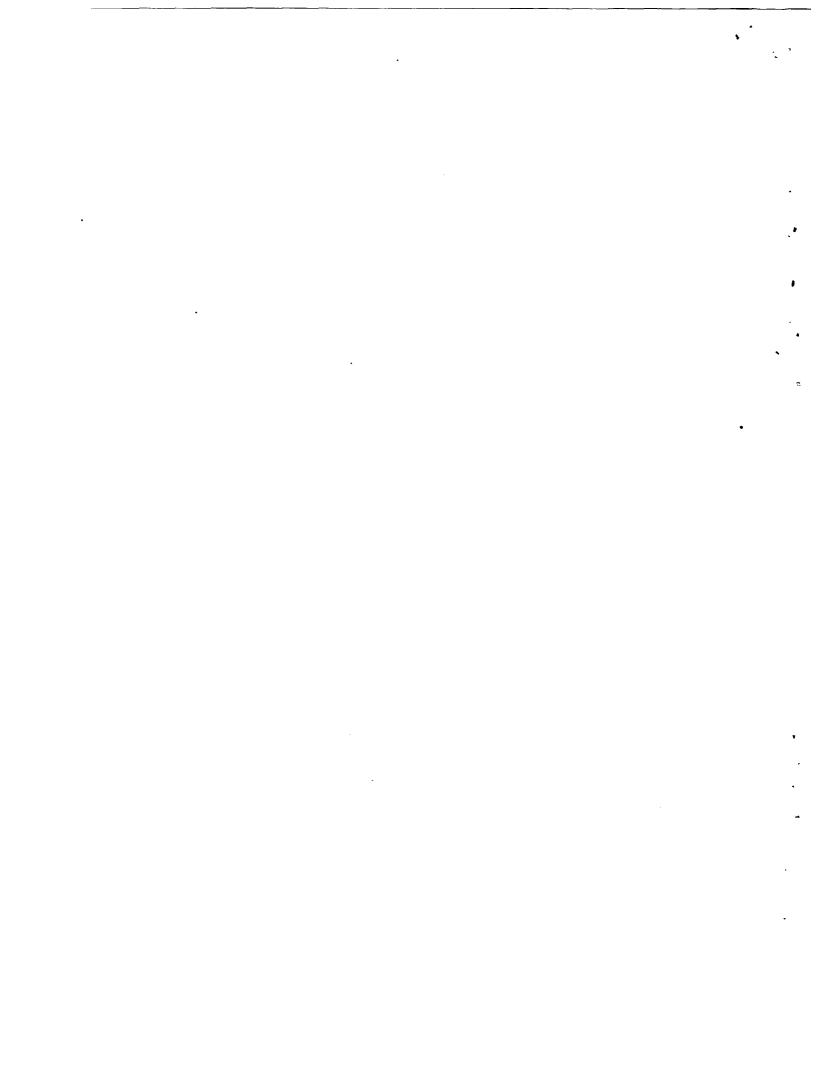


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APPENDIX G

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COMPARISON OF STUDY DATA WITH NNWSI REFERENCE INFORMATION BASE

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Material Property	Value Used	RIB Value ^a	RIB Reference	
Density	2.34 g/cc	2.34 g/cc	1/2/1/5/1-3	
Thermal Conductivity	2.07 W/mK	2.07 W/mk (25-100°C)	1/3/1/6/1-5	
Heat Capacity	2.25 J/cm ³ K	³ 2.25 J/cm ³ ⋅K (25-100°C)	1/3/1/6/1-5	
Coefficient of Thermal Expansion	10.7*10 ⁻⁶ K ⁻¹	10.7 (25-200°C)	1/3/1/6/1-5	
Elastic Modulus	15.1 GPa	15.1 GPa	1/3/1/7/1-6	
Poisson's Ratio	0.2	0.2	1/3/1/78/1-6	
Uniaxial Strength (Matrix)	75.4 MPa	75.4 MPa	1/3/1/7/1-6	
Tensile Strength (Matrix)	-9.0 MPa	-9.0 MPa	1/3/1/7/1-6	
Friction Angle (Matrix)	29.2• •	29.2*	1/3/1/7/1-6	
Cohesion (Joint)	1.0 MPa	1.0 MPa	1/3/1/8/1-2	
Friction Coefficient (Joint)	0.8	0.8	1/3/1/8/1-2	

FIGURE G.1							
MATERIAL	PROPERTY	DATA					

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Reference: Zeuch and Eatough, 1986 - TS2 Data, 80% Saturated.

Geometric Parameter	Value	Used	RIB Value (ft)	RIB Reference
Drift Dimension				•
Width	16.0 ²	(4.88m)	16.0 ^d	2/2/1/1-17
Height	22.0 ^a	(6.71m)	22.0 ^d	2/2/1/1-17
Radius of Roof Arch	9.0 ^b	(2.74m)	9.5 ^e	••••
Container Borehole Dimens	ions			
Depth	25.00 ^a	(7.62m)	25.00 ^d	2/2/1/1-17
Diameter	2.42 ^a	(0.74m)	2.42 ^d	2/2/1/1-17
Panel Dimensions	•		•	
Waste Standoff from				
Access Drift Wall	77.5 ⁸	(23.62m)	77.5 ^d	2/2/1/1-17
Access Drift Width	21.0 ^a	(6.40m)	21.0 ^d	2/2/1/1-17
Emplacement Drift			_	
Spacing	112.0 ^c	(34.14m)	112.0 ^d	2/2/1/1-17
Barrier Pillar Width	63.0 ^a	(19.20m)	63.0 ^d	2/2/1/1-17
Panel Width	1400.0 ^a	(426.72m)	1400.0 ^d	2/2/1/1-17

FIGURE G.2a GEOMETRIC DATA FOR VERTICAL EMPLACEMENT OPTION

Source references: ^aMansure and Stinebaugh (1985). ^bParsons, Brinckerhoff, Quade and Douglas (1985). ^cMansure and Ortiz (1984). ^dMansure and Stinebaugh (1985). ^eParsons, Brinckerhoff, Quade and Douglas (1985).

FIGURE G.2b

GEOMETRIC DATA FOR VERTICAL EMPLACEMENT OPTION

Geometric Parameter	Valu	e Used	RIB Value (ft)	RIB Reference
Emplacement Drift Dimensi	on			
Width	18.0 ^a	(5.49m)	18.0 ^d	2/2/1/1-15
Height	13.0 ^a	(3.96m)	13.0 ^d	2/2/1/1-15
Radius of Roof Arch	10.2 ^b .	(3.11m)	9.5 ^e	••••••••
Container Borehole Dimens	ions			
Waste Standoff From				•
Emplacement Drift	117.50 ^a	(35.81m)	117.50 ^d	2/2/1/1-15
Length	682.00 ^a	(207.87m))	682.00 ^d	2/2/1/1-15
Diameter	2.75 ^a	(0.84m)	2.75 ^d	2/2/1/1-15
Panel Dimensions			•	
· · · ·	1400.00 ^a	(426.72m)	1400.00 ^d	2/2/1/1-15
Panel Width	985.0 ^C	(300.23m)	748.0 ^e	, , , -,

Parsons, Brinckerhoff, Quade and Douglas (1985).

^CMansure and Ortiz (1984). ^dMansure and Stinebaugh (1985). ^eParsons, Brinckerhoff, Quade and Douglas (1985).

APPENDIX H

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U.S. Department of Energy Forrestal Building
Washington, DC 20585

Eric Anderson Mountain West Research-Southwest, Inc. 398 South Mill Avenue, Suite 300 Tempe, AZ 85281

H-2

J. H. Anttonen Deputy Assistant Manager for Commercial Nuclear Waste Basalt Waste Isolation Project Office U.S. Department of Energy P.O. Box 550 Richland, WA 99352

Timothy G. Barbour Science Applications International Corporation 1626 Cole Boulevard, Suite 270 Golden, CO 80401

E. P. Binnall Field Systems Group Leader Building 50B/4235 Lawrence Berkeley Laboratory Berkeley, CA 94720

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Flo Butler Los Alamos Technical Associates 1650 Trinity Drive Los Alamos, New Mexico 87544

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Forrestal Building
Washington, DC 20585 B. W. Church
Director
Health Physics Division
U.S. Department of Energy
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Las Vegas, NV 89114

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John Fordham Desert Research Institute Water Resources Center P.O. Box 60220 Reno, NV 89506

Judy Foremaster (5) City of Caliente P.O. Box 158 Caliente, NV 89008 D. L. Fraser
General Manager
Reynolds Electrical & Engineering Co., Inc.
P.O. Box 14400
Mail Stop 555
Las Vegas, NV 89114-4400

M. W. Frei (RW-231)
Office of Civilian Radioactive Waste Management
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B. G. Gale (RW-223)
Office of Civilian Radioactive Waste Management
U.S. Department of Energy Forrestal Building
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R. W. Gale (RW-40)
Office of Civilian Radioactive Waste Management
U.S. Department of Energy
Forrestal Building
Washington, DC 20585

V. M. Glanzman U.S. Geological Survey 913 Federal Center P.O. Box 25046 Denver, CO 80225

Vincent Gong Technical Project Officer for NNWSI Reynolds Electrical & Engineering Co., Inc. P.O. Box 14400 Mail Stop 615 Las Vegas, NV 89114-4400

A. E. Gurrola General Manager Energy Support Division Holmes & Narver, Inc. P.O. Box 14340 Mail Stop 580 Las Vegas, NV 89114 R. Harig
Parsons Brinkerhoff Quade & Douglas, Inc.
1625 Van Ness Avenue
San Francisco, CA 94109-3678

Roger Hart Itasca Consulting Group, Inc. P.O. Box 14806 Minneapolis, Minnesota 55414

T. Hay Executive Assistant Office of the Governor State of Nevada Capitol Complex Carson City, NV 89710

L. R. Hayes (3) Technical Project Officer for NNWSI U.S. Geological Survey 421 Federal Center P.O. Box 25046 Denver, CO 80225

W. M. Hewitt Program Manager Roy F. Weston, Inc. 955 L'Enfant Plaza, Southwest Suite 800 Washington, DC 20024

T. H. Isaacs (RW-22)
Office of Civilian Radioactive Waste Management
U.S. Department of Energy
Forrestal Building
Washington, DC 20585

\$

Allen Jelacic (RW-233) Office of Civilian Radioactive Waste Management U.S. Department of Energy Forrestal Building Washington, DC 20585 C. H. Johnson Technical Program Manager Nuclear Waste Project Office State of Nevada Evergreen Center, Suite 252 1802 North Carson Street Carson City, NV 89701

S. H. Kale (RW-20) Office of Civilian Radioactive Waste Management U.S. Department of Energy Forrestal Building Washington, DC 20585

B. J. King (2)
Librarian
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Richland, WA 99352

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J. P. Knight (RW-24) Office of Civilian Radioactive Waste Management U.S. Department of Energy Forrestal Building Washington, DC 20585

٦

T. P. Longo (RW-25) Office of Geologic Repositories U.S. Department of Energy Forrestal Building Washington, DC 20585

R. R. Loux, Jr. (3) Executive Director Nuclear Waste Project Office State of Nevada Evergreen Center, Suite 252 1802 North Carson Street Carson City, NV 89701 S. A. Mann Manager Crystalline Rock Project Office U.S. Department of Energy 9800 South Cass Avenue Argonne, IL 60439

Dr. Martin Mifflin Desert Research Institute Water Resources Center 2505 Chandler Avenue Suite 1 Las Vegas, NV 89120

D. F. Miller Director Office of Public Affairs U.S. Department of Energy P.O. Box 14100 Las Vegas, NV 89114

R. Lindsay Mundell U.S. Bureau of Mines Denver Federal Center P.O. Box 25086 Building 20 Denver, Colorado 80225

S. D. Murphy Technical Project Officer for NNWSI Fenix & Scisson, Inc. P.O. Box 15408 Mail Stop 514 Las Vegas, NV 89114

J. O. Neff Manager Salt Repository Project Office U.S. Department of Energy 505 King Avenue Columbus, OH 43201

D. C. Newton (RW-23) Engineering & Licensing Division Office of Geologic Repositories U.S. Department of Energy Forrestal Building Washington, DC 20585

a state of the second second

N. A. Norman Project Manager Bechtel National Inc. P.O. Box 3965 San Francisco, CA 94119

D. T. Oakley (4) Technical Project Officer for NNWSI Los Alamos National Laboratory P.O. Box 1663 Mail Stop F-619 Los Alamos, NM 87545

O. L. Olson Manager Basalt Waste Isolation Project Office Richland Operations Office U.S. Department of Energy P.O. Box 550 Richland, WA 99352

Gerald Parker (RW-241) Office of Civilian Radioactive Waste Management U.S. Department of Energy Forrestal Building Washington, DC 20585

David K. Parrish RE/SPEC Inc. 3815 Eubank, N.E. Albuquerque, NM 87191

J. P. Pedalino Technical Project Officer for NNWSI Holmes & Narver, Inc. P.O. Box 14340 Mail Stop 605 Las Vegas, NV 89114

P. T. Prestholt NRC Site Representative 1050 East Flamingo Road Suite 319 Las Vegas, NV 89109 W. J. Purcell (RW-20)
Associate Director
Office of Civilian Radioactive
Waste Management
U.S. Department of Energy
Forrestal Building
Washington, DC 20585

L. D. Ramspott (3)
Technical Project Officer for NNWSI
Lawrence Livermore National
Laboratory
P. O. Box 808
Mail Stop L-204
Livermore, CA 94550

J. R. Rollo
Deputy Assistant Director for Engineering Geology
U.S. Geological Survey
106 National Center
12201 Sunrise Valley Drive
Reston, VA 22092

B. C. Rusche (RW-1)
Director
Office of Civilian Radioactive
Waste Management
U.S. Department of Energy
Forrestal Building
Washington, DC 20585

J. E. Shaheen (RW-44)
Outreach Programs
Office of Policy, Integration and Outreach
U.S. Department of Energy
Forrestal Building
Washington, DC 20585

Dr. Madan M. Singh President Engineers International, Inc. 98 East Naperville Road Westmont, IL 60559-1595 M. E. Spaeth
Technical Project Officer for NNWSI
Science Applications
International Corporation
101 Convention Center Drive
Suite 407
Las Vegas, NV 89109

. i - 2

Christopher M. St. John J.F.T. Agapito Associates, Inc. 27520 Hawthorne Blvd., Suite 295 Rolling Hills Estates, CA 90274

Ralph Stein (RW-23) Office of Civilian Radioactive Waste Management U.S. Department of Energy Forrestal Building Washington, DC 20585

K. Street, Jr.
Lawrence Livermore National Laboratory
P. O. Box 808
Mail Stop L-209
Livermore, CA 94550

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W. S. Twenhofel
Consultant
Science Applications International Corp.
820 Estes Street
Lakewood, CO 89215

D. L. Vieth (4) Director Waste Management Project Office U.S. Department of Energy P.O. Box 14100 Las Vegas, NV 89114

J.S. Wright Technical Project Officer for NNWSI Waste Technology Services Division Westinghouse Electric Corporation Nevada Operations P.O. Box 708 Mail Stop 703 Mercury, NV 89023