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UNITED STATES OF AMERICA OFFICE OF THE SECRETARY

NUCLEAR REGULATORY COMMISSION RULEMAKINGS AND
BEFORE THE ATOMIC SAFETY AND LICENSING BOARDICATIONS STAFF

In the Matter of :

PRIVATE FUEL STORAGE : Docket No. 72-22

L.L.C. : ASLPB No. 97-732-02-ISFSI

· -----x

Washington, D.C.

Friday, March 15, 2002

Deposition of

JAMES K. MITCHELL

a witness, called for examination by counsel for Private Fuel Storage, pursuant to notice and agreement of counsel, beginning at approximately 8:30 a.m., at the law offices of Shaw Pittman, 2300 N Street, NW., Washington, D.C., before Barbara A. Huber of Beta Reporting & Videography Services, notary public in and for the District of Columbia, when were present on behalf of the respective parties:

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## NUCLEAR REGULATORY COMMISSION

Docket No. 72-22	Official Exh. No. PFS Ex. 228
In the matter of Private F	uel Storage
Staff	IDENTIFIED
Applicant	RECEIVED
Intervenor	REJECTED
Other	WITHDRAWN
DATE JUNE 17, 2002	Witness Mitchell
Clerk	

_	2
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22	Anwar E.Z. Wissa . Steven Bartlett

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## PROCEEDINGS

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JAMES K. MITCHELL

was called as a witness and, having been

first duly sworn, was examined and testified

as follows:

EXAMINATION BY COUNSEL FOR PRIVATE

FUEL STORAGE

BY MR. TRAVIESO-DIAZ:

- Q Good morning, Dr. Mitchell.
- 11 A Morning.
- Q Would you please state your full
- 13 | name for the record?
- 14 A James K. Mitchell.
- 15 Q My name is Matias Travieso-Diaz.
- 16 I'm an attorney representing PFS in this
- 17 | proceeding. I will asking you some
- 18 questions today about what has been called
- 19 | the Utah Contention QQ, which is now part of
- what has become unified Contention L/QQ.
- 21 But we get underway, let me ask you.
- Have you testified prior to today

in any legal proceedings?

A I have.

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Q So you are somewhat familiar with the process?

A I am.

Q Have you given depositions before?

A Yes.

Q In any event, just to refresh your memory as to the most important thing, if there is a question that I ask you that you don't understand, is unclear, poorly formulated or whatever, could you please ask me to fix it in whichever way, refresh it, ask it again, or whatever? Would you please do that?

A I will.

Q Are you familiar with Contention QQ?

A Generally, yes.

Q What is the basis for your familiarity?

A I was asked to look at the issues

- relating to the use of soil cement and formulate some opinions.
  - Q Were you involved in the process that led to the filing of Contention QQ?
  - A Yes.

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- Q When were you first involved with that process?
  - A Oh, that was last spring, April or May of 2001.
- 10 | Q How did you become involved?
- A I was called by the folks in Utah
  who are working with the attorney general's
  office there.
- Q Who called you? •
- 15 A The first contact I believe was
  16 Steve Bartlett.
- Q What did Mr. Bartlett talk to you about?
  - A He talked to me about the project in general, and the fact that they were proposing to use soil cement, and that he felt that they wanted someone to work with

- 1 them who was familiar with soil cement.
- 2 Q So at that point, you were engaged 3 to do that work?
  - A After some decision about what my involvement would be and so on.
  - Q Who else did you talk to in addition to Mr. Bartlett?
  - A I talked to Farhang Ostadon; and at some point, Connie Nakahara and Denise Chancellor.
  - Q I'm going to ask you -- I'm sure your lawyer will, as well -- not to disclose to me anything of your conversations with either Connie or Denise, since they are counsel.

So apart from whatever conversations you had with counsel for Utah, what was the nature of your disclosure?

MS. CURRAN: Just to interrupt you for a minute. We would also consider any conversation with another expert in which a lawyer was present, subject to that

8 1 privilege. So we would instruct you not to 2 discuss those conversations. MR. TRAVIESO-DIAZ: Well, we'll take it as it comes, but the question that I 5 need to ask him is sufficiently general. 6 it shouldn't be a problem. 7 BY MR. TRAVIESO-DIAZ: 8 What was the nature of your 9 conversation with Dr. Ostadon at that point? 10 Well, I can't remember Α 11 specifically whether it was with Dr. Ostadon 12 or Dr. Bartlett, because they were both 1.3 involved, but it was generally related to 14 the proposal for uses of soil cement, and the issues that they felt might be relevant. 15 16 So, essentially, they suggested to you issues that they thought might be 17 18 relevant? 19 Α They did. 20 Did you raise issues on your own? 21 I believe I did. 22 We'll get to those in a minute. Q

1	I believe Contention QQ was filed
2	in May of 2001, almost a year ago?
3	A Yes.
4	Q What has been your involvement
5	with that contention since that time?
6	A None, until around the first of
7	this year. In fact, I've had no involvement
8	with the contention itself, other than
9	having been sent the I don't know what
10	you call it, the decisions that were made
11	about it, its admissibility and so on.
12	Q So they sent you various filings
13	that were made and decisions made by the
1 4	board, and so on?
15	A That's correct. That was sometime
16	after the first of the year.
17	Q I take it you had no other
18	involvement up to the beginning of the year
19	with the rest of the case, either?
20	A No. I have not.
21	Q Since the beginning of the year,
22	what has been your involvement?

A My involvement has been essentially nil, until about a week ago when I started to review the current information, and when I was sent deposition transcripts and so forth.

Q Would it be fair to say that from the beginning of the year until about a week

Q Would it be fair to say that from the beginning of the year until about a week ago, all that was happening was they were sending you materials for your information as to what was going on?

A I had a brief meeting with Denise and Connie Nakahara sometime in mid January, when I was in Salt Lake City for other reasons. That was simply to talk about schedule and availability and do what I'm doing here today.

- Q What documents have you prepared in connection with this case?
  - A What documents have I prepared?
  - O Yes.
- A I prepared a declaration, I believe, last spring. That's the only

- document, other than making some notes as I

  went along.

  So you have prepared no other
  - Q So you have prepared no other documents, written material since that time?
    - A For submission?
    - Q Yes. Or for any other purpose?
  - A No. Only the notes that I write down maybe in a phone call or as I read something.
    - Q Is it your understanding that you're going to be a witness on behalf of the state with respect to Contention QQ?
    - A Yes.

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- Q What aspects of Contention QQ is it your understanding you're going to testify about?
- 17 A Soil cement.
  - Q Is that it?
- 19 A Yes.
- Q Since this was a year ago, and not to test your memory, I'm going to give you at this point what has been marked as

Exhibit 1. I will describe what it is for 1 the record. 2 3 Exhibit 1, which was introduced in 4 earlier depositions in this round, is 5 called, Joint submittal of united 6 geotechnical contention, Utah L and Utah 00. 7 It's dated January 16, 2002. It's the cover 8 page. It's signed by Denise Chancellor, 9 Assistant Attorney General. 10 I understand that starting on the 11 fourth page, which is actually number 12 page 1, there is what I believe is the 13 description of what now is become --14 Α I don't have a page 1. 15 MS. CURRAN: Page 1 is this one. 16 Wait a minute his copy is --17 MR. TRAVIESO-DIAZ: Is it 18 defective? 19 MS. CURRAN: I think so. 20 THE WITNESS: I don't know.

MS. CURRAN: Let me take a look.

MR. TRAVIESO-DIAZ:

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Let's go off

1	the record a second.
2	(Discussion off the record)
3	BY MR. TRAVIESO-DIAZ:
4	Q As I was saying, this document is
5	what the parties have agreed is the text of
6	Contention L/QQ.
7	Have you seen this document
8	before?
9	A Yes, I have seen it, just
10	recently.
11	Q Was that part of the documents you
12	started reviewing last week?
13	A Yes.
14	Q Now, my understanding is what used
15	to be Contention QQ has now become .
16	subsection C-3-B, C, D, and E, on page 2
17	and 3, and subsection D on page 3, 4, and 5.
18	Actually, going on to the very first line of
19	page 6.
2 0	Does that agree with your
21	understanding?
22	A I don't know the details of how

they consolidated the two contentions.

Q I ask you to keep that document handy because we're going to be referring to it quite frequently.

Since you became involved -again, I take it in this case early this
year -- apart from counsel for the state,
who else have you spoken to?

A Since I became involved in the case?

Q Well, okay. All right. If I recall what you said a moment ago, you essentially provided a declaration in May last year?

A That's correct.

Q Then essentially had no involvement until January, when you had a brief meeting with Denise Chancellor and Connie Nakahara?

A That's correct.

Q You really had no further involvement until about a week or so when

you began preparing for this deposition?

A Yes.

Q Now, my question is: Since

January of this year, apart from counsel for
the state of Utah, who else have you spoken
to in connection with this case?

A I've spoken with Steve Bartlett, and Farhang Ostadon.

Q What was the nature of your conversations, these more recent conversations, you had with Mr. Bartlett?

A They were discussing the depositions, I believe, of Paul Trudeau; and some points that they were they thought were relevant to this particular case with soil cement.

Q What did they tell you, as far as you can recall?

MS. CHANCELLOR: This is Denise Chancellor. I would like to caution Dr. Mitchell that I was on that phone conversation, too.

1	THE WITNESS: Oh, you were.	
2	That's correct.	
3	So that is something that I don't	
4	respond to; is that correct?	
5	MR. TRAVIESO-DIAZ: Well, you tell	
6	me.	
7	Are you instructing the witness	
8	not to tell me what Dr. Bartlett told you?	
9	MS. CURRAN: Yes.	
10	MR. TRAVIESO-DIAZ: On the basis	
11	that it was part of a conversation with	
12	counsel for the state?	
13	THE WITNESS: Yes. Thank you for	
14	refreshing my memory, but Denise Chancellor	
15	set up that call.	
16	BY MR. TRAVIESO-DIAZ:	
17	Q So it was a conference call?	
18	A Yes, it was.	
19	Q Apart from that conference call,	
2 0	have you had any other conversations with	
21	anyone else?	
22	A I don't recall any conversations,	

1 no.

Q What materials, what written documents did you review in preparing for this deposition today?

Trudeau's deposition transcript. I've reviewed or read through these contentions. I refreshed my memory of some past documents on this, looked at a couple of things in the SAR. I looked at the ACI document on soil cement, sort of the state-of-the-art document. I looked at my prior declaration. I'm trying to think. My memory's bad. It was those kinds of documents, all related to this particular issue.

MR. TRAVIESO-DIAZ: All right.

Let me show you and give you a document

that's going to be marked by the reporter as

Exhibit 36.

(Deposition Exhibit No. 36 was marked for identification.)

BY MR. TRAVIESO-DIAZ:

(202) 638-2400

Q Dr. Mitchell, I have introduced
and marked as Exhibit 36, a document
entitled, Declaration of Dr. James K.

Mitchell, dated May 15, 2001.

Is this the declaration that you were referring to a moment ago as preparing at the time Contention QQ was filed?

- A It is.
- Q Did you prepare this document?
- 10 A Yes.

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- Q I see also that attached to this
  document -- so I take it that's your
  signature on page 5?
- 14 A Yes.
  - Q Attached to that signature page, there is a two-page document dated

    April 2001, which, as I read it, appears to be a summary of your qualifications and experience; is that correct?
    - A Yes.
- Q You prepared that, as well?
- 22 A Yes. I don't know if it's been

edited any since I submitted some
information or not. But it looks pretty
much like what I've been using.

Q I would like you to take a moment, given that you prepared this document a year ago, and review the declaration first. Then we'll talk about the qualifications later.

But starting with the declaration itself, to see if based on what you have learned, if anything, over last year there is any change you want to make, or any addition, correction or modification to what you say in the declaration.

A There is.

Q Could you tell us what there is?

A There may be more than this, if I read it very, very fairly. But in paragraph 11, in my subsequent review the past few days, I realize that I misinterpreted the nature of the cross-section under the pads.

Q Could you explain?

A When I prepared this, I had misunderstood, as I indicated, the cross-section; and had been under the impression that the cast themselves would be sitting directly on the layer of soil cement in the pad area.

I start talking in here about the general characteristics of Portland cement concrete and asphalt concrete, pavement structures; and the similarity of what is being proposed at the site here with pavements, and the fact that there was no structural layer present.

In fact, I was wrong. There certainly is. There is a three-foot thick reinforced concrete structural layer that's between the casts from the -- the bottom of the casts and the soil cement. So that changes what the loading and stress conditions will be in the soil cement.

Q I was going to later ask you questions, because I had the sense that you

made that misunderstanding.

What change would you propose to make to paragraph 11 to make it correct, based to your understanding?

A I would suspect that freeze/thaw will probably not be a major issue at that depth, at least to the same extent that it would be if the material were exposed. The second sentence about the tensile strength is still valid.

It's still true that in heavy-duty pavements with soil cement bases there's usually a structural asphalt concrete or Portland cement concrete pavement layer of 16 inches, but that's not relevant in this case. Because we're looking at a 36-inch thick reinforced concrete layer. So we would need to strike the sentence that this structural layer is absent in the applicant's proposed design.

The bending stresses and their consequences will be much less in the actual

	<u> </u>
1	design than I had been presuming. It would
2	mean that the last sentence of this
3	paragraph perhaps would be modified some. I
4	still have not found and maybe the
5	information is there in the SAR or other
6	calculations I have not seen a I've
7	not personally seen the calculation of the
8	magnitude of the bending stresses at the
9	bottom of the soil cement layer.
10	Q Not to put words in your mouth,
11	but how would you propose to reword the last
12	sentence of paragraph 11?
13	A I would say that I have not seen
14	the results of calculations of the stresses
15	in the treated soil cement layer beneath the
16	reinforced concrete pad.
17	Q You mean the bending stresses?
18	A The bending stresses would be the
19	most critical in this case. I think wes

most critical in this case, I think, yes.

MR. TRAVIESO-DIAZ: Would you read the last part of his answer?

(The reporter read the record as

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1		requested.)	23
2		MR. TRAVIESO-DIAZ: Would you take	
3	a moment t	•o go off the record?	
4		(Discussion off the record)	
5		BY MR. TRAVIESO-DIAZ:	
6	Q	Dr. Mitchell, apart from those	
7	changes tl	nat you're suggesting in	
8	paragraphi	11, is there any other part of	
9	your decli	aration that you think it may be	
10	appropriat	se to change, based on what you	
11	know now?:		
12		Take a couple of minutes to look	
13	through i!	z.	
14	A	Paragraph 14 perhaps needs some	
15	modificate	ion, simply to indicate that .	
16	placement	of the pads, the reinforced	
17	concrete :	pads themselves, will have the	
18	affect of	causing some changes in moisture	
19	content o	f the underlying soil.	
20	Q	Where will you make that change?	
21	A	Well, let's see. The first	
22	sentence	might say, placing the reinforced	

concrete storage pads and the soil cement layer beneath them will lead to an increase in the water content, the partly saturated silty clay and clay silt soils beneath them.

- Q Any other changes?
- A I think that's all right.
- Q Very good. Before I ask you to look at the declaration, you were telling me that attached to it is a statement of your qualifications and experience; is that right?

Would you turn to that?

- A This one?
- O Yes.
- 15 A Yes.

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- Q It's clear that you have quite an extensive record here. Rather than my characterizing it for you, could you in your own words describe to me what you believe to be your areas of expertise?
  - A Well, my overall area of expertise is geotechnical engineering. Within it, in

my research, my teaching, and my consulting activities, I have focused on soil stabilization and ground improvement, and on the engineering properties of soils, on aspects of geoenvironmental engineering, and more recently on geotechnical earthquake engineering.

Q You don't consider yourself in an expert in what I think is called structural engineering; that is to say, the design of structures?

A I am not.

Q How about, for lack of a better word, the specialized analysis that go with the design such as soil structure interaction? Are you an expert in that?

A I am not expert on the analysis of soil structure interaction. I work with experts in those areas as I appropriate.

Q Like Dr. Ostadon, for example?

A Yes.

Q Now, a moment ago when you said

that you have become or you are an expert in geotechnical earthquake engineering.

Could you explain to me what that means?

A In this case, I will choose to limit the area within that overall category of geotechnical earthquake engineering to issues related to soil liquefaction and ground modification and improvement so as to increase their resistance to damaging affects of seismic loading.

Q What work have you done on that last category, ground improvements to better the resistance of the ground to earthquake?

A I have worked both in research and in consulting on the appropriate treatments to attain the levels of improvement that are needed to resist liquefaction. I've worked on specific designs for improving the foundations of existing dams that are in seismic areas. It's that general area that I've been working in.

Q We will talk about this probably more later.

But just to get an idea, what kind of techniques or designs have you been involved with for improving the designs or foundations to resist the earthquake loads?

A Things ranging from densification by vibratory techniques, densification by compaction grouting or injection grouting, the construction of structural fills to replace potentially liquefiable sand materials.

Q Have you ever had occasion to use soil cement as an application for helping foundations or soils resist earthquake loads?

A Well, there is a very significant case that's referred to in documentation for this project we're talking about here, in South Africa, the Coberg nuclear power station, where they replaced about an eight-meter thick potentially liquefiable

layer of sand -- it was quite a clean

sand -- with the same material treated with

cement.

- Q Were you involved in this case?
- A I was.

- Q Do you think that's a case that the application is analogous to the one that's been proposed here?
  - A No.
  - Q Why not?
- A It's not analogous to this

  particular case because the soils there were
  loose, saturated sands that were potentially
  liquefiable. The soils at the PFS site in
  Skull Valley are plastic, fine grain
  materials that I don't believe would be
  susceptible to liquefaction.
- Q Well, apart from the different constituency of the soil, the soil material, how did the application at the South Africa plant differ from the application that is proposed here?

A The one in South Africa involved a large excavation. As I recall, it was about 24 meters deep below the ground surface, and the removal of a very thick layer of loose sand, and treatment with cement and replacement and recompaction.

Q Was it with treated with cement, or did they make a cement soil mixture?

A They mixed cement with the sand.

Q Isn't that what they're proposing to do here at PFS, as well?

A They're proposing mixing cement with the soil, but the soil type is totally different.

Q Perhaps it's an ignorant question, but what difference would it make in terms of the nature of the application whether you have one type of soil or another? Wouldn't you be doing the same thing?

A The South African issue was potential liquefaction of the sand which would lead to loss of foundation support.

The application in the PFS case, it's shallow. It's treatment of a fine grain soil at a location where liquefaction is not an issue. The purpose of the treatment is to provide a stronger material; and, if I understand correctly, is also to be able to use the upper two feet of material that otherwise would have to be wasted.

Q Well, which of the two do you consider more significant or difficult technical problem: To solve the liquefaction concern in the South Africa plant, or to straining the soils at PFS?

A I wonder if that isn't, in some ways, an apples and oranges kind of comparison. Because each project had different challenges. I can't say that one is more difficult or critical than the other.

Q All right. Go back to your qualifications, if you would for a second.

Since this was prepared on April

of last year, would you take a look at this statement of qualifications and tell me whether you have done any additional work since the time this statement was prepared that would be relevant to the issues in Contention OO?

A I don't think that there are any projects that are directly relevant to this one that I've been working on recently.

- Q Now, you said a moment ago that apart from your declaration and some notes that you wrote to yourself, you haven't prepared any written materials relating to this contention in this litigation; is that correct?
  - A To the best of my knowledge, yes.
- Q Have you performed any tests, any soil examinations, or any physical inspections with respect to the soils at PFS?
  - A I have not.
- Q Have you been to the PFS site at

32 1 all? 2 Α No. I have not. Do you anticipate doing any 3 written analysis or performing any tests of 4 the type that are normally performed on soil 5 or otherwise, between now and maybe in the 6 next month, when the hearing will take place in this proceeding? 9 I have no plans to do so. 10 Q Let's go back to Exhibit 1, which 11 is the text of Contention QQ. 12 personally had no input in the preparation of this document that's Exhibit 1; is that 13 14 correct? 15 Α This is the combined L and 00? 16 0 Correct. 17 Α No, I had nothing to do with that. 18 0 All right. 19 MS. CURRAN: Hold on just a 20 minute. 21 (Counsel conferred with witness) 22 BY MR. TRAVIESO-DIAZ:

Q Please go back on the record and explain what you were talking to counsel with.

A Well, she was asking if I had involvement in the preparation of QQ. My involvement was the preparation of a declaration. I believe that there were drafts of QQ that I looked at the relevant pieces of back in -- a year ago time frame. So I did have involvement in that. But when it became the combined L and QQ, I don't recall having done anything with that.

Q That was precisely my question, whether you had anything to do with the new version, which was submitted in January of this year.

I thought you said "no"?

A I did, but --

Q I'm sorry. Let me see if we can get this way. I can clarify my question.

You testified that you provided input that ultimately formed the basis for

- what became Contention QQ that was filed in May of 2001?
  - A That's correct.

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- Q Now, had you had any input in the process, whatever it may have been, that led to the preparation and the filing of Exhibit 1, the document that was filed in January of this year?
  - A No.
- Q That was what I though you said.
- Now, in that case, will you turn to Exhibit 1? I ask you to review section C, D, and E for a moment.
  - A Little C, D, and E, is that what you're asking?
    - Q I'm sorry. Big C, D, and E, starting on page 2 and going all the way through the end.
      - What I'm going to ask you, so that you may do this very quickly, is to confirm for me that, as you understand it, you're going to be testifying only with respect to

- the matters raised on subsection C, big C, that goes from page 2 to page 3?
- A That is my understanding, yes.
- Q So we can limit what we need to talk about.
  - A Yes.

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- Q Now, turn to section C, big C.

  Look at subparagraph three that begins on page 2 and goes on to page 3.
  - Would it be correct to say that on paragraph C-3, with the exception of the first and the last subparagraph, which are A and E, that the rest of them relate to or address the use of soil cement?
  - A Yeah. That's correct.
- Q So to put it differently, you don't expect that you will be testifying with respect to C-3-A or C-3-E; is that correct?
- 20 A I have some opinion on C-3-E.
  21 That's, again, related to the soil and
  22 cement.

-		36
1	Q So you expect to be testifying	
2	with respect to C-3-E, as well?	
3	A As it relates to the Youngs	
4	modulus of the soil cement, yes. As to its	
5	impact on the impact forces, no, that's	
6	that part of the analysis is not something	
7	I've been involved in. But I have some	
8	understanding of the modulus of	
9	cement-treated soil.	
10	Q Let's then turn to paragraph C-3-B	
11	on page 2 of Exhibit 1.	
12	Do you have that before you?	
13	A I do.	
14	Q Were you involved in providing	
15	facts or information that form the basis for	
16	the formulation of paragraph C-3-B?	
17	A I was not involved in the	
18	formulation of that.	
19	Q Do you expect that you will be	
20	offering testimony with respect to the	
21	matters addressed in this paragraph?	

A I believe that the dynamic

analyses that are needed there are more in the area of expertise of Dr. Ostadon.

Q As I understand paragraph C-3-B, it raises the potential lack of case history present and the potential lack of site specific testing about or relating to the use of cement-treated soil at PFS or soil cement?

A Yes. That's what it says.

Q Those two areas, will you be addressing them?

A Well, if the earthquake loadings are determined in terms of stress and strength requirements for the soil cement, I might be testifying according to the -- relative to that. But as for determining what those loadings would be, that's not my area.

Q I need you to clarify a little bit more for the record what you mean, after the "if." Explain to me.

A Well, what I mean is that I could

potentially testify as to whether I believe the soil cement will have properties adequate to resist the earthquake loading.

Q State the definition, if you will, as of today, of the earthquake loadings that will imparted upon the soils of PFS?

A My in-depth knowledge of what the specific loads will be is very limited. I understand that the seismic design is based on a maximum horizontal and vertical acceleration of .7 g. But that has to be translated into what the actual loads will be that are transferred into the foundation. I don't know those values.

Q Are you aware of whether PFS has done calculations to try to determine exactly that?

A I believe they have. But I haven't reviewed any of them in detail.

Q So that I can understand the extent of your knowledge, you have not reviewed the calculations so you don't have,

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today, a current knowledge of what the calculated loads on the soil will be; is that correct?

A That is correct.

Q Without knowing what those loads will be, you wouldn't know whether the proposed soil cement design will be sufficient to meet those loads? Is that your view?

A That's my view, yes. I know -let me add that -- that design strength
values have been indicated in the PFS
application that they are intending to
obtain. But what I do not know is whether
the particular soil cement that they're
planning to use will provide them.

Q Let me ask the question this way then.

Is it your lack of current understanding or knowledge, if I can use the term, based on the fact that you don't know whether those numbers are appropriate for

use, or is it based on your not being able to tell whether soil cement can achieve those values?

A I am unable to comment on whether the numbers are appropriate because I've not been involved in that dynamic analysis.

It's not an area I have expertise in. I would suspect that a proper formulation of soil cement could attain the values that are proposed for the design. But I've seen no data for the soils and soil cement mixtures for that site that demonstrate it.

Q We'll talk about this a little bit more later, but just so I understand the limitations of your current knowledge -- you're saying that -- I suspect that you give me a number, say to pick one, 250 PSI, that it is possible to achieve soil cement of that strength; but you don't know, sitting here today, whether, in fact, the methodology that PFS intends to use will indeed resolve in soil with that

characteristic? Is that what you're saying?

A That's I think a reasonable way to put it. It's certainly possible to obtain a strength of 250 PSI. But to date, I have seen the results of -- I have not seen the results of any tests that show me that for this soil.

Q Again, I'm jumping way ahead, but we my go back to this. I provided to the state at their request earlier this week some preliminary test results of the program that PFS is conducting.

Have you seen those?

A Yes.

Q So you have seen those test results?

A I have.

Q Going back --

A That is, if it's the same set of results -- you know, there may -- I don't know how many sets of results there are, but I have seen one set of results.

Q We'll talk about that a little bit more in a moment.

We were addressing or discussing paragraphs C-3-B?

A Yes.

Q You were explaining to me that the portion of the paragraph that related to site specific testing, I believe?

A Uh-huh.

Q I was about to ask you to clarify for me whether you will be addressing the concern about the lack of case history present.

Will you be addressing that, as well, or is that for others?

A Well, I'm personally unaware of any case history precedent for this particular kind of application. It may be there but I, personally, don't have knowledge of a nuclear waste canister storage area involving the use of cement-treated soil.

Q Well, so where you're defining this particular application, you're referring to the use of soil cement to, if you will, improve the performance of the foundations underneath storage casts for nuclear waste?

A Yes. I know of no other case exactly like this.

Q Well, as an experienced engineer, what conclusion or what importance do you attribute to the fact that soil cement has never been tried to be used before in connection with waste storage casts for nuclear facilities? What's the significance of that?

A I'm not sure there is any significance. We're always finding new applications for our materials. I don't see anything inherently wrong with the basic concept that's being proposed here.

Q Let me ask you to look at a document that you referred to as among of

the things that you reviewed. Correct me if
I'm wrong. But I believe we're talking as
the same document. This was previously
introduced as Exhibit 23 in earlier
deposition in this case.

For the record, this document is entitled, ACI 230.1R-90. The title is, State-of-the-art report on soil cement, reported by ACI committee 230. It bears the caption on the top, Reapproved 1997. There was earlier testimony that this document, in fact, was released in 1998.

Is this a document that you reviewed?

A I have the original version of this, which I believe was 1991, something like that. I have not had access to the reapproved version, which you have just handed me here.

Q Tell me, are you familiar with this document; not this particular exhibit, but the state-of-the-art report on soil

cement?

A This one, yes.

Q Well, tell me about what is this document? What is the intent of the document? What is it done for?

A I think the intent of the document is to describe what soil cement is, how it can be used, how you construct it, how you design the mixtures, what are the general ranges of engineering properties. It provides a good overview coverage of the subject for both people who are trying to learn initially about it, and for people who want to have some reference information with values for different properties.

Q Were you involved with the preparation of this document originally?

A I was not involved with the preparation of the document, although there are some figures in it that came from publications that I was involved with in years past.

Q Is this considered a standard reference to work in the field of soil cement?

A I don't know for sure. But I guess it certainly could be considered a standard reference.

Q Do you use it yourself?

myself because, for years, my own notes and other reference materials have formed the basis for the classes that I taught on stabilization with cement, and those types of things. So, basically, the information that's organized very nicely here is what I've had in my own files.

Q So, essentially, this reflects information that you, yourself, have developed?

A Well, there's some information in here that I've developed. But it reflects what a lot of people have developed. I think it reflects a very good coverage of

the subject.

Q Do you think it would be appropriate for somebody seeking to design a soil cement program to use this as a reference to go by?

A Oh, I think so, yeah.

Q Let me show you what has again been previously marked as Exhibit 21 in this proceeding.

For the record, Exhibit 21 is a portion of the safety analysis report for the PFS facility. It comprises section 2.6.411 and goes to the end of section 2.8.

Is this one of the documents that you were saying that you reviewed last week in preparation for this deposition?

A Yes.

Q If you will turn your attention to page 26117.

A Okay.

Q Look at the last paragraph on the

page, that starts with the words "the design."

A Yes.

Q Concentrating just on the first sentence, it says, The design placement, testing, and performance of soil cement is a well established technology.

Would you agree with that?

A I would.

Q Now, the paragraph goes on to say that PFS will develop site specific procedures to implement the recommendations presented in ACI 1998.

If you go back to the references on the back or even to the second line of this paragraph, it appears to refer to the same document that we're talking a moment about before, the state-of-the art report on soil cement; is that correct?

A I believe so.

Q Would it be appropriate for PFS to follow the guidance in the state-of-the-art

report on soil cement in developing the site specific procedures for mixed portion and testing, construction and quality control?

A I'd have to go back and look at the guidelines in some detail to be sure whether you would follow them exactly in all respects. But I think that there's good guidance there, yes. The same kind of guidance are available through the Portland Cement Association publications and elsewhere.

Q Is it, in fact, your understanding that the state-of-the-art report on soil cement references all the publications, such as the Portland cement standard that you talked about?

A I think it does. References the AFTM standards that are often used.

Q Those would be the standards that you would expect somebody designing or constructing soil cement probably would be follow; is that correct?

- A Yes, I believe so. Yes, I would.
- Q As long as we are on that

  page, 26117, would you take a look at the

  discussion we just began looking? Take a

  second to read that, or more than a second.

  I would like you to take a look at that

  paragraph on page 26117, and then three

  paragraphs with bullets that go on

  page 26118 and 119. Take a second to review

  those. Let me know when you're finished.
  - A Okay.

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Q Before we go into the specifics of this bullets, maybe it would be good for the record if we talked about your understanding of what PFS intends to do with soil cement.

Are you sufficiently familiar with what you understand to be their intents, so you can describe it for us?

- A Do you want me to describe it?
- Q Yes. If you could describe your understanding of what they're trying to do.
  - A They're using it in two ways, my

understanding. They're planning to put a roughly two-foot thick layer of soil cement beneath the cast storage pads, over this large area. The cement will be of uniform thickness, essentially, between the storage -- or beneath a storage pad, but between the storage pads, which is a distance of five feet, I think, in one direction, ten in the other. Correct me if I'm wrong. Whatever. 35? In the other.

The soil cement thickness will be greater. It's going to extend up to within -- I think it s eight inches of the ground surface. That upper eight inches between the pads is going to be crushed rock or gravel for transport of vehicles, if I remember correctly.

The other application of the soil cement is around the cast transfer building. It's going to be thicker. I think it's five feet thick there. It's extending out. It's 240 in one direction, and 360, or

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1	something of that sort in the other.	52
2	Q Feet?	
3	A Feet. Those are feet, yes.	
4	That's my understanding of the use of soil	
5	cement, the intended use of soil cement.	
6	Q A couple of clarifying questions.	
7	With respect to the pad and	
8	placement area or the cast and placement	
9	area, is it your understanding that the	
10	storage casts will sit on a reinforced	
11	concrete pad?	
12	A Yes. This was my area of	
13	misunderstanding when I did the declaration.	
14	Q Three feet thick?	
15	A Yes, a three-foot thick reinforced	
16	concrete mat.	
17	Q That mat, in turn, will be sitting	
18	on the two feet of soil cement that you	
19	described a moment ago?	
20	A That's my present understanding,	
21	yes.	
22	Q The intent is to use different	

strengths of mixes, if you will, of soil cement underneath the pad, as opposed to elsewhere; is that correct?

A Yes.

Q So what is your understanding of the compressive strength, if you will, of soil cement that they want to use underneath the pads, as opposed to in other areas?

A My understanding if -- is that the strength of the treated soil beneath the pads is low. Is 40 PSI right, the right number? That the soil cement surrounding the building is stronger, 250 PSI.

Q The soil cement around the pads is also stronger?

A I don't remember on that but I -- I don't remember whether it's still the 40 or whether it's 250.

Q With that background, let's turn to the first bullet on page 26118.

In that paragraph, with the first bullet that is entitled, soil/cement mix and

procedure development, the first paragraph of that entire section says, The sliding forces due to design bases ground motion will be resisted by bond between the base and sides of the foundation and the soil cement, and by passive resistance of the soil cement acting against the vertical side of the foundation. The soil cement mix will be designed and constructed to exceed the minimum shear resistance requirements.

Do you have any reason to believe that this approach as a technical proposition will not be successful if done properly?

A I don't have any reason to believe that it wouldn't be successful, no.

Q It goes on to say that there be direct shear testing conducted to replicate the soil conditions and to confirm the adequate shear resisting and other strength requirements will be provided by the final soil cement mix.

Do you see that?

A Uh-huh.

- Q Assuming that, in fact, such testing is conducted appropriately, do you have any reason doubt that, in fact, it will be possible to do demonstrated by testing that the shear strength requirements will be met?
- A I would anticipate that it should be possible to meet those requirements. But in my opinion, it's necessary that it be demonstrated by testing. To this point, I have seen no results to indicate that it's been achieved.
- Q Of course. I guess the proof is in the pudding?
  - A Yeah.
- Q So your present doubt is not whether it can be done, but whether PFS has demonstrated that what they intend to do specifically will meet the objective that they're describing here; is that right?

1 A I agree.

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- Q That will have to be done through a testing program?
  - A Yes.
  - Q Now, let's go to the second bullet. The second bullet starts on the bottom of page 2118 and go to the top of page 2119. It appears to talk about the actual techniques that will be used to construct the soil cement.
  - Is that how you read that paragraph?
- 13 A Yes.
- Q Now, is the approach to

  construction rate described here, as far as

  your experience tells you, is this the way

  you would do it, or is this an appropriate

  way to do it, as far as constructing the

  soil cement itself?
  - A Excuse me. I'm reading it.
- 21 O Go ahead.
- MR. TRAVIESO-DIAZ: I'll ask you

1 to read that question back to him. 2 THE WITNESS: That seems a reasonable construction procedure for me. 3 BY MR. TRAVIESO-DIAZ: 4 5 Q Thank you. Then let's go back to the last bullet that is entitled, soil 6 cement and in situ clay interface. That one appears to describe the construction 8 9 technique to make sure that there is a good bonding between the soil cement and the 10 11 underlying clay. 12 Is that how you read the 13 paragraph? 14 There are two aspects of it.

A There are two aspects of it. The first is whether the bond that's necessary will be achieved. That can be determined by testing.

Q Correct.

A Should be determined by testing.

Construction techniques are proposed -well, they're not specified here. But they
proposed to develop them such that that

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interface bond is indeed obtained.

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- Does this look to you a reasonable approach?
  - It's a reasonable approach. think that the first step would be to demonstrate that the bond could be obtained. In my opinion, there is no reason that couldn't be done now.
  - That, again, would have to be done Q or could be done by testing?
    - That's correct.
  - Would it make any difference whether that demonstration is done now, or at some future time?
    - Well, in the unlikely event that you couldn't obtain the bond that you have designed your project to have, yes, it would make a difference.
  - Sure. It would be a terrible waste of time?
- Yeah.
- 22 Q But putting that aside, does it

1	make any difference when you test it, as
2	long as you can demonstrate that you can
3	achieve the bond?
4	MS. CURRAN: Clarification.
5	Difference to what?
6	MR. TRAVIESO-DIAZ: If I
7	understand his testimony, he testified that
8	if instead of doing it now you do it at the
9	end of the process, you risk failing to
10	demonstrate that you'll be able to achieve
11	the bond and therefore you have to do
12	something else.
13	THE WITNESS: That's correct.
14	BY MR. TRAVIESO-DIAZ:
15	Q By the same token, if you do that
16	today and you demonstrated that the bond
17	cannot be achieved, then you have to do
18	something else.

Is there any the significance to reaching the determination either way, whether it is done today or whether it is done say just before construction starts?

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A Seems to me to be prudent that you would want to demonstrate these things before the final design is completed and the appropriate licensing to go ahead with the project is issued.

Q Do you know what they intend to test for the achieving of the bond, the ability to achieve such a bond, at any point in the near future?

A Do I know if --

Q What their intent is, as far as you know, with respect this, to this performing this kind of a test?

A No. I don't.

Q Assuming that, in fact, they deferred making this demonstration until the point when they were ready to start construction and at that point they realize that, my God, we cannot achieve this bond, what would happen then? Would they have to come up with a different alternative approach?

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A Well, I would assume so. But I know also -- and I'm no authority in this area -- but I know that when you're dealing with nuclear facilities, things have to be pretty well demonstrated and proven before you can go too far in your process.

Q The part I was trying to get to actually was this.

Do you know of any regulatory requirement, in terms of the licensing of this facility, that would make it required for PFS to perform this test that we're talking about before they get a license to build the facility?

MS. CURRAN: Objection. You're asking for a legal conclusion.

MR. TRAVIESO-DIAZ: I'm asking if he knows there is both regulations, regulatory guidance that the technical people use all the time. I just want to know whether he knows of any.

THE WITNESS: I do not know the

legal and licensing requirements in this
area.

BY MR. TRAVIESO-DIAZ:

Q All right.

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MS. CURRAN: Matias, just let me know when is a good time to take a break.

MR. TRAVIESO-DIAZ: No, no, no, no. no. You to tell me. The witness is the one

9 who's not going to suffer here. If this is
10 a good time now --

MS. CURRAN: No. If you're --

MR. TRAVIESO-DIAZ: This is a good

point, if you want to take a break.

THE WITNESS: I wouldn't mind.

15 (Recess)

BY MR. TRAVIESO-DIAZ:

Q Dr. Mitchell, we were talking
about Exhibit 1, the list of contentions.

A Exhibit 1? This one?

MS. CURRAN: No. It's this here.

21 It's open. That's it.

THE WITNESS: Thank you.

## BY MR. TRAVIESO-DIAZ:

- Q Before the break, we were talking about subparagraph C-3-B on page 2

  Exhibit 1?
  - A Yes.

- Q We were beginning to talk about site specific testing. Do you see those words there?
  - A Yes, I do.
- Q Could you tell us what your understanding is of site specific testing as is used here?
- A My understanding of this is that we have not the yet seen evidence that the soil from the site mixed with the cement that's proposed for use at the site will produce the material that is being relied upon in the design. That's from the formulation of the material standpoint.

Subsequently, I suspect there will be need to demonstrated in the field, perhaps through a test pad, that the

proposed material can be produced using the field construction procedures.

Q So you interpret the site specific testing as meaning two different sets of tests: First test that, if you will, will qualify the material for the use that you want to give it and ensure that it meets the design characteristics; and the second phase in which then you demonstrate that the construction techniques that you intend to use are suitable? Is that how you do it?

A Yes.

Q So you have seen neither of those so far?

A That's correct. I have not seen it.

Q Is it your understanding that PFS has formulated a program to do the first of the two sets of tests that you talked about; that is to say, to qualify the material for use under the site conditions?

A My understanding is that they

1 have, yes.

- Q Do you know whether PFS is, at this point in time, conducting such a test program?
- A It's my understanding that they have indeed had some tests done by AGEC in Salt Lake City.
  - Q Have you reviewed any documentation that reflects the test program that PFS is conducting?
- A Yes. As we noted earlier, I was sent some test results quite recently.
  - Q But I was actually asking about have you seen any documentation that defines the type of tests that are to be conducted and the manner in which they're going to be performed?
  - A There is such a document that I scanned briefly, that provides a scope of work. ESSOW, I think is what it was called.
  - Q I'm going to hand you a document that has already been identified in this

1 | proceeding as Exhibit 14.

For the record, this document is dated January 31, 2001, entitled,
Engineering services scope of work for laboratory testing of soil/cement mixes.
The document number is ESSOW
number 05996.02-G01030.

Is this the document that you said that you reviewed?

A It appears to be, yes.

Q I want to state for the record that Stone & Webster and PFS believe this document contains confidential information.

Therefore, depending on the extent of which we discuss it, we may have to have the reporter bind the pages of this discussion and this document separately, so it's confidential and shall be protected, and that only the parties have access to it.

But let's just talk about it for a moment before we decide whether we need to do that not.

You said you reviewed this document.

Is it your understanding that this document defines the portion of the test program that PFS intends to carry out that satisfies or that will carry out the first of the two sets of tests that you were talking about a moment ago?

A This proposes or describes a series of tests that will give the basic information needed to support some but not all of the design proposals.

Q Let me just ask you a couple questions about the document. Again, given that it is confidential, I'll see if we can talk about generally without into too much detail. But we'll go, of course, to whatever detail we need to.

You will take to section 20, which is applicable documents. It starts on page 1 and goes through page 3, after the cover page.

Do you see that?

A Okay.

Q Now, section 20 starts by saying that, All work shall be performed in accordance with the latest version of the following regulatory requirements, codes, and standards.

Do you see that?

A Yes.

Q Would you review the listing of codes and that appears in section 2.0 and tell me whether you believe that those are the appropriate standards to use in the performance of the tests that are described in this ESSOW, which is sometimes pronounced "ESSOW" I think?

A I am unable to state relative to the USNRC, because I'm not familiar in detail with all of their requirements. So I can only address the American Society for Testing Materials list, which is on page 2, I guess. They look correct. I've not gone

back to my detailed listing of ASTM

standards and checked them off one by one.

But these seem to be the correct ones for the test program as described.

Q Let's turn now then to section three. Why don't you look at section 3.2 that starts on page 5?

Do you have that section?

A Yes.

Q Would you take a look at the listing or the description of the tests that starts on page 5 and goes to the end of page 7, and tell me whether you believe that these are tests that are appropriate to be conducted in the process of qualifying soil cement for an application?

A Yes.

Q Would you take a moment to look at the description of the manner in which this test are specified to be conducted on those two pages?

Tell me whether you believe that

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the description of the manner in which the tests are to be conducted conforms to your expectation on how this test ought to be done.

A They seem to be consistent with what is the current state of practice.

Q You mentioned a moment ago there may be some additional tests that you would like to see or you expect to see conducted.

Could you tell me what those tests are, beyond the ones listed in this document?

A It's the test to provide the information to establish that the sliding resistance between the different layers is sufficient.

Q So you expect that there would have to be a test or should be a test that, in fact, demonstrates that the sliding resistance that is called for in the design can, in fact, be achieved?

A Yes.

(202) 638-2400

Q You don't see that here?

A No. This would be the resistance between the concrete pad and the underlying soil cement and between the soil cement and the underlying Bonneville clay.

Q Do you remember that you mentioned that you looked at the transcript of Mr. Trudeau's deposition last week?

A It was yesterday. It was just about as fast as I could turn the pages because I just received it.

Q You may not have an answer to this.

But do you recall whether

Mr. Trudeau testified that, in fact, is an

intent to define and carry out such a test

as the one that you mentioned, at some time

in the future?

A I recall seeing that in one or the other of the documents that I received. It described -- or it mentioned a test program that going to be done at Dr. Wissa's

laboratory at Ardaman & Associates.

- Q So as far as you are concerned, that is a test that will have to be done?
  - A Yes.

Q Any other tests that you believe should be done that are not included in what was marked as Exhibit 14?

A There is one property that
evidentially is very important to their
analysis. That's the modulus of the
cement-treated soil that's going to be
placed beneath the pads, which has an upper
bound of 75,000 PSI, according to their
analysis. I don't see that indicated here,
or how it's to be deduced from the data that
they will be obtaining.

Q Again, that's something that you expect should be done or you believe that it should be done, and it's not among the tests listed here?

A Yes. What I would say is a determination of that modulus is extremely

-	important, as i interpret the requirements
2	for their tip-over impact analysis.
3	Q Again, in reviewing the transcript
4	of Mr. Trudeau deposition, do you remember
5	whether he referred to any intent of
6	performing such a test?
7	A I don't remember.
8	Q Any other tests that you think
9	should be conducted, beyond those that are
10	listed on Exhibit 14?
11	A I don't think of any. There is
12	one no, I think that's okay.
13	Q Now, I have heard the term used
14	index property tests.
15	Does that term mean anything to
16	vou?

Α Yes.

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What does that mean?

These are the kind of tests that A you do to get a basic characterization for the soil. Includes primarily the moisture contents, the Atterberg limits, and the

74 1 particle gradation as determined by 2 sym-analysis and hydrometer analysis. 3 Would you take a look back at Exhibit 14, starting on page 5? 4 5 Would you confirm for me what I believe you just said, that the tests listed 6 on page 5, starting with 3.22 and going to the next page of 3.27, are the tests that 8 you were referring to as the index property 9 10 tests? 11 Α The index property tests would 12 be 3.22 through 3.26. I don't think that moisture density tests would regularly be 13 14 referred to as an index test. 15 0 That would be a separate test? 16 Α That's a separate test. These are 17 all separate tests. 18 0 Yes. But I mean in the category 19

of index properties would be the first five that are listed here from 3.22 to 3.26?

> Α Yes.

Q All right. Do you know whether

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1	PFS or PFS contractors have performed the	5
2	tests listed in 3.22, 3, 4, 5, and 6?	
3	A They have done some of these, yes.	
4	Q You said that you reviewed some	
5	test results?	
6	A Yes.	
7	Q Would those be among the ones that	
8	you reviewed?	
9	A Yes.	
10	MR. TRAVIESO-DIAZ: Let me mark as	
11	an Exhibit 37, I believe.	
12	(Deposition Exhibit No. 37 was	
13	marked for identification.)	
14	BY MR. TRAVIESO-DIAZ:	
15	Q We are short we may have to	
16	surrender one of the two. Keep the one that	
17	is the marked and then you can pass the	
18	other one.	
19	Exhibit 37, I will identify for	
20	the record as being a letter dated	
21	March 27, 2001, from Applied Geotechnical	
22	Engineering Consultants, Inc., that goes by	

76 the initials AGEC, to Stone & Webster. 1 Ιt 2 consists of a cover letter and a number of tables and figures; is that right? 3 That's correct. Α 4 5 Is it your understanding that this 6 document reports the results of some index 7 property tests? It does. Α 9 MR. TRAVIESO-DIAZ: Let me now 10 mark as Exhibit 38, another document. 11 (Deposition Exhibit No. 38 was 12 marked for identification.) 13 BY MR. TRAVIESO-DIAZ: 14 0 Exhibit 38 is another letter dated December 13, 2001, from AGEC to Stone & 15 Webster and one attached page appears to 16 17 contain a table. 18 Would you understand this to be 19 also another set of index property tests 20 results? 21 It is a set of Atterberg limit 22 test results, which is one of the index

property tests that are listed in the other.

- Q Have you reviewed the test results shown on Exhibit 37 and 38?
  - A I have looked at them, yes.
- Q Did you have any concerns or comments with respect to these test results?
- A These test results refer to the shallow material which in prior documents had been called an eolian silt. It seems to me that the material that's identified by these test results is not well is described as an eolian silt. It's a much finer grain and more plastic material than that term silt would imply to me.
- Q Well, first of all, is this a question of characterization, or it, in your mind, more of a concern as to the actual nature of the property, of the material?

It's a question of what we call it, or more as to what properties it has?

A Well, the question that I was raising was what you call it. The important

thing is what you have. This defines, I think rather clearly, what you have, which is a finer grain material that has a reasonably high natural moisture content, and exhibits a fair amount of plasticity.

Q As a person experienced in reviewing test results, would these result of phase one be sufficient to proceed to subsequent phases in the testing?

Can you use these test results to prepare yourself to do the other phases of the tests?

A Oh, yes. Yes, it provides guidance for that.

Q Tell me what the phases that you expect that you will see take place in this testing program will be; in other words, in which order will you be doing the various tests?

A I would now be moving into the moisture density relationships for the material, and begin to do some tests adding

what I think are reasonable values of cement to get the properties that are required.

- Q Then what tests would you do?
- A Well, they're listed in this program. This is the moisture density test, the wet/dry, freeze/thaw, the strength tests, and the permeability tests. The splitting tensile strength test is a rapid, indirect way to measure the tensile strength that seems to provide pretty good results.

So that suite of tests I think should provide the information that is needed. Except for the bonding between layers, of course.

- Q Which will be a separate test that is not listed here; is that right?
  - A Yes.

- Q Now, with respect to moisture density tests, have you reviewed any results of tests conducted to date on moisture density?
- A I have seen some results.

8.0 MR. TRAVIESO-DIAZ: Let me mark 1 this as Exhibit 39. 2 3 (Deposition Exhibit No. 39 was marked for identification.) 5 BY MR. TRAVIESO-DIAZ: Let me describe this document for 6 0 the record. Exhibit 39 is a set of figures 7 without a cover page, bearing the heading, 8 Applied Geotechnical Engineering 10 Consultants, Inc. It has figures with 11 captions such as moisture content, percent 12 of dry weight, gradation of moisture density 13 relationship. It looks to be a set of figures of tests performed on different 14 15 samples. 16 Is that your understanding? 17 Α That's what I see, except that 18 there is no data on the gradation sections 19 of each of these pages. 20 All right. If you reviewed 21 Mr. Trudeau's deposition, you will probably 22 remember or you may remember that he

indicated that he had received by fax a set of preliminary results on moisture content tests.

Do you remember that?

A I think so, yes.

Q Well, in any event, I will represent for the record that Mr. Trudeau testified to that effect. The record will be whatever it is.

Assuming that, in fact, these are .
moisture content tests, is there any information, comment, or reaction by you to these results? Does it mean anything to you?

A I am very surprise at the low densities that resulted in these tests.

Q Give me an example so that we --

A Well, they seem to be typically less than 90 pounds per cubic foot. I think I even saw one that was less than that, one that's 77.5 maximum density, with optimum water contents that are really quite high.

But that's the one thing that jumped out at me most significant.

Q If you were given these test results to evaluate, would you draw any conclusion from that?

A I would wonder what is it that is making these densities so low and the moisture contents so high. There's one with a maximum dry density of 70 pounds per cubic foot. May be it's 71. It's the next to the last sheet. The last sheet is only 71, as well. Those values, to me, are surprisingly low. The cause, I don't know.

Q Could you draw any inference or relationship, perhaps not based on this alone, but could you draw any inference or relationship between these test results, assuming they are correct, and the properties of the soil?

A I don't know, based on these results by themselves, whether this is a soil composition issue or a test procedure

83 issue. That needs to be determined, I 1 think. 2 3 So you would, if presented with 0 these test results, look further just to try 4 5 to figure out why --6 Try to figure out why those values 7 are so low. 8 You have not reviewed any other 9 test results? 10 I think that's the extent of it. Α 11 Actually, I am going to put on the record now -- although we may talk about 12 this a little later -- so to complete the 13 14 package, something that I'm going to call Exhibit. I don't know if you have reviewed 15 16 this or not, but you can tell us. 17 Α I'll tell you. 18 (Deposition Exhibit No. 40 was 19 marked for identification.) 20 BY MR. TRAVIESO-DIAZ: 21 Exhibit 40, for the record, is --22 Α Excuse me. I have seen this one

before. I forgot.

Q Let's just say on the record what this is. The document that the witness referred to and is marked Exhibit 40 is a letter from Applied Technology and Engineering Consultants, Inc., to Stone & Webster, dated October 31st, 2001. It appears to attach a one-page table. We will be discussing this a little bit later.

But what is your understanding of what this document conveys?

A It indicates the amount of water soluble sulfates in parts per million that was determined for samples from shallow depth, zero to two feet in general, with one exception: In the laboratory.

Q Again, we'll save that for discussion in a moment. But for the moment, if I could ask you to turn, if you could find it, to your declaration that was marked as Exhibit 36.

A Yes.

Let's take a look first at 0 paragraph 12. Let me just direct you specifically to the sentence that I want you to focus on.

Midway the paragraph, at about the sixth line, there is a sentence that starts, The amounts of cement that are proposed to be added.

Do you see the sentence?

Α I do.

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Now, if I understand your discussion in this paragraph 12, you are expressing a concern that the amount of cement in the mix, so to speak, that PFS intends to use may not be sufficient to qualify the mix as a true soil cement? Is that your concern?

> Α That was the concern, yes.

Do you still believe that to be a 0 concern?

Α If their intent is to produce soil cement in the strict definition of the word,

then that would be a concern. If their intent is to produce a material that has certain values of strength and stiffness and it can be demonstrated that it will have sufficient durability for the particular application that they're talking about, then it's not.

Q Now, we talked earlier about the, if you will, the design intent or specification that the soil cement to be used around the canister transfer building have a strength of 250 PSI.

Would the material having that strength be properly classified as soil cement?

A It might or it might not. It would depend on whether it satisfied the wet/dry and freeze/thaw durability test.

Q So the importance as to whether you call it soil cement is whether it shows durability by being subjected or passing the freeze/thaw, dry/wet test?

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- A That's the usual definition of a soil cement.
  - Q So that is one of the tests that
    PFS intends to conduct at this point?
    - A That's correct.
    - Q You have not seen any test results with respect to that phase of the tests, have you?
      - A I have not.
    - Q Assuming that, in fact, the test showed that the material that is specified to have a strength of 250 PSI meets these durability tests, would it qualify, in your mind, as to true soil cement?
      - A Yes.
  - Q If it did, would that resolve the concern that you are expressing here in paragraph 12?
- A Yes.

Q Now, the other application that I remember you described was for the mixture -- let's call it a moisture for the

moment -- underneath the storage pads.

That, I believe you said, is expected to have a much lower strength, something on the order of 40 PSI or so?

A I believe that's PFS is proposing for that material.

Q You would not call that soil cement?

A If the material had that low of strength and at the same time satisfied the durability tests -- I'm not sure whether there is a minimum strength requirement for soil cement.

Q What do you need to have in material that you would call soil cement in accordance with the discussion that we're having relating to the paragraph 12?

Why do we need to have that material pass this durability test?

A PFS, in the documents that I was reading, was referring to the material as soil cement. My response at that point was

if they intend to have soil cement, then it must satisfy the durability requirements.

My understanding now, in the later submissions, is that for the material under the pad, the durability requirements will not be as great, as severe, and they don't need such a high strength. So I believe and what -- I mean now it's being referred to as a cement-treated material. I have no argument with that.

Q Explain, if you will, for the record why is it important that soil cement pass the durability tests?

A I think in this instance where we are facing exposure to some rather hash climactic conditions over long period of time, the durability of the soil cement that surrounds the canister -- cast transfer building is important.

Q Because over a period of time, exposure to the elements would cause it to degrade its property?

90 1 My response to that is that's Α 2 important, yes. 3 Isn't it true that if PFS performs 4 durability tests as specified in Exhibit 14 that demonstrate that the mix that they 5 propose to use passes or survives these 6 durability tests, that that mixture would be 7 8 qualified, in your opinion, as true soil 9 cement? 10 Yes. 11 If it doesn't, therefore it 12 doesn't qualify as such? 13 Α It would not. But that's independent of whether 14 the mixture that they intend to use achieves 15 16 the strength that is specified? 17 Α Yes. 18 You testified earlier that you see 19 no problem with the ability to get the 250 PSI mix as such? 20 21 Α My opinion is that it should be

possible, but I would like to see it

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demonstrated.

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- Q Also you would like to see demonstrated that in addition to having 250 PSI, it meets the durability test?
  - A That's correct.
- Q Let's move to paragraph 13 in your declaration.

It starts with, It is not surprising that no site specific testing has been done to date to obtain the strength and durability properties of the cement-treated soil.

Do you see that?

- A I see that. But what I heard I don't believe is what I said.
- 16 Q Did I misread it?
  - A I believe you said it is not surprising. It's an important distinction.

    Because I said it is surprising.
- Q If I did that, it was a Freudian slip, as they call it.

What I'm asking you, actually,

with respect to this first paragraph, would you like to qualify or modify that paragraph based on the results that you have seen so far?

A Well, the first sentence still holds. So far there has been no information that I have seen about strength and durability. I stand by the next sentence, that we need test data using the actual site soil and cement.

The third sentence about the chemistry of the surficial soils, we now have a little bit of information about the sulfates, which was the exhibit that you last distributed, Number 40. So that sentence would now require some qualification for it to be true today.

The next one, there can be salts and evaporites. So it's still valid. Then in this paragraph, I talk about sulfates and the possible formation of ettrinite. PFS seems to be obtaining information that sheds

some light on that issue.

Q Would it be fair to say that with respect to the concerns expressed on paragraph 13, at least in the first half of the paragraph, what you're saying is, as of today, is that they are conducting a test program, but they have not shown me yet the result that I want to see with respect to durability and strength?

A That's correct.

Q So it's not that you don't believe they're doing it, it's that they have not shown you yet results that show durability and strength?

A Yes. This program that is now being undertaken, as I read it, is intended to provide that information.

Q But that information is not available now; that's your point?

A That's my point.

MS. CURRAN: Excuse me one second.

(Counsel conferred with witness)

THE WITNESS: All right.

## BY MR. TRAVIESO-DIAZ:

Q As long as you have the exhibit in front of you, let's talk about it briefly. Exhibit 40, that refers to the sulfate testing.

Have you reviewed the results of the testing that has been conducted so far with respect to sulfates, the existence of sulfates on the soils at the site?

A I've looked at the information in table one of this exhibit.

Q What intelligence, if you will, did you derive from reviewing that table?

A It indicates that for a reasonable number of samples of the shallow material, which would be this material called the eolian silt, that the sulfate content is less than say 500 parts per million. It shows for one deeper sample a depth of two to four feet, however, that the sulfate content is quite high, 13,800 parts per

million. That's one sample from a very large area. So it's by no meanings necessarily indicative of what its like everywhere at that depth.

What that is if you were making soil cement out of the surficial material, at least in the short-term, I wouldn't consider it to be a significant problem from sulfates. But now I would worry a little bit because of the soluble sulfate that is below it, and whether that could become an issue at some future time. Again, there's only one data point for the deeper material.

Q Understanding that this is only one set of results, assuming that the other results of future tests were comparable to the ones that this first sample takes, would that lead you to believe that the use of the top layer, the eolian soil, for making soil cement would not in itself present a problem with respect to sulfate attack or sulfate presence?

Α Based on the chemistry indicated here for the surficial material, I would not expect it to be a problem. But I'm a little bit concerned and would want to think a bit more what this high value beneath it means in the long-term.

Going back again to paragraph 13, to the concern that you express with respect to sulfates, starting with the sentence that says, Of most concern?

> Α Uh-huh.

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I just want to clarify how this paragraph plays or interacts with the testimony that you just gave.

It is my understanding that with respect to the ability to create suitable soil cement, if they used just the top layer and if the rest of the test show the same amount of sulfates in that soil, that this paragraph would not present really a problem?

That's what I'm trying to get to.

A What I'm trying to say is that the material with the amount of sulfate being less than 500 parts per million should be okay. However, if there is prolonged exposure and upward migration of sulfates from that high sulfate-bearing layer -- and whether it's a layer or not I don't know, because we only have one data point -- conceivably could be a problem in the future.

Now, you mentioned the word migration. I suspect we'll be talking a little bit about that in a minute.

But is that the problem in general that you refer to in the next paragraph, 14, of your declaration?

A Yes. Well, no, no. Excuse me.

That is not the problem in the next

paragraph. The next paragraph has to do

with just the general affect of subsequent

increases in moisture content in the

subgrade soil beneath the soil cement in the

pad.

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I'm sorry. That was a very poorly-worded question.

What I meant to say was: Your concern about sulfates would be migration of sulfates from the layer beneath to the soil cement?

> Α Yes.

That, in that respect, I was analyzing in my mind to the potential migration of moisture.

> Α Right. Both are possible.

All right. Now, assuming again Q that the test showed that the eolian or superficial -- the top two feet of the soil had low sulfate content and that there may be some higher sulfate beneath, you expressed the concern that maybe the sulfate below will migrate over?

> Α Yes.

Wouldn't you expect that to happen Q already if, in fact, that was a viable or

likely mechanism?

I mean, this top layer has been covered with a layer for some millions of years; isn't that right?

A Right. But the placement of a cover over that layer's going to change the moisture regime. This has been just an open, exposed to the atmosphere soil surface. Now it's going to be paved.

Q So you're saying that the sulfate might migrate over with the moisture?

A Because the moisture content will probably increase beneath the pad.

Paragraph 14 was not dealing with sulfates.

It was simply dealing with the whole issue of changed moisture.

Q I recognize that I'm way above my area of knowledge here. So let me try to see if I understand this a little bit.

You're saying that the reason that sulfates have not found their way, assuming that they're in fact there, in higher

amounts in the lower amounts than on top to the higher level is because the higher level was, if you will, exposed, didn't have a cap on it?

A I don't know why they haven't thought the whole thing through. I can't give at this point, without thinking and analyzing a little bit of the exact mechanism and causes. However, all we know is that in the surficial layer sulfate content is reasonably low. But at one point below that, it's very high. There may be some gypsum, for example, in that lower layer. I don't know.

Q What would the significance be of gypsum in the lower layer?

A Gypsum is calcium sulfate.

Q Well, what I was trying to understand -- and, again, I have to admit that I'm totally illiterate when it comes to this -- is that it would me that if sulfates are going to move up with the moisture and

you have no obstacle to the movement, that they would migrate upwards, as you have no surface that prevents them from going up.

What am I missing in that

What am I missing in that scenario?

A The permeability of the upper part of the Bonneville clay, which is the layer beneath the two feet of eolian silt, may be so low as to prevent any significant transfer. Again, I don't know. I don't know this profile, in terms of the details that would be needed to evaluate that.

Q Let me try one more time, just see if I understand it. This goes back to college.

Is it your understanding that this is a fairly arid area where this site is located?

A Yes.

Q Wouldn't you expect that any moisture that tends to move will migrate upwards and evaporate?

1	A There will be continuing	102
2	evaporation and there have been continuing	
3	infiltration after rains. The net result	
4	though of all of this is that the moisture	
5	content in that upper two feet is reasonably	
6	high. It's twenty to thirty percent, as I	
7	recall from the data that are here.	
8	Q Based on the first set of samples?	
9	A Yeah.	
10	Q Is there any other observation	
11	that you can make?	
12	Is this actual, at this point in	
13	time, a concern or a potential concern?	
14	A I would say it would be more a	
15	potential concern.	
16	Q There is not enough information	
17	for it to	
18	A Not enough information.	
19	Q There is not enough information to	
20	progress from potential concern to concern;	
21	is that right?	
22	A That's correct.	

(Counsel conferred with witness)

THE WITNESS: Well, I can clarify what I mean by paragraph 14, but maybe he's going to ask about that.

BY MR. TRAVIESO-DIAZ:

Q I was going to ask you about paragraph 14. Not now, but in a moment, if I may. That way my intent. Why don't we talk about paragraph 14, since we are about to go back and back-fill, as they say.

Tell me what your concerns are on paragraph 14, as you re-read it early this morning.

My concern there is that by paving over the current ground surface, you will change the moisture regime and experience in these kinds of areas as shown, that when you do that, you begin to accumulate water beneath the paved area. That could have some consequences on the engineering properties of the clay layer that's beneath

1 the treated soil cement.

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Q So, essentially, let me see if I can express my thought coherently -- that the presence of a concrete seal or pad above the soil cement will cause water to accumulate on the soil cement layer, or on the top of the clay below it?

Just clear it up for me first.

A I think it's would be concentrated more in the underlying clay. It changes the internal stress conditions within that material.

Q Why don't you tell for the record what are the physical mechanisms that will drive the moisture to accumulate on the clay below, immediately below the cement-treated soil layer?

A Any upward migrating moisture can no longer evaporate once you've sealed the surface. So it stays there. Now, if you have a relatively shallow water table, that can lead to a significant change in the

moisture conditions beneath the barrier layer.

Here, you have a very deep water table. It's 125 feet, or something of that sort, which is way down there. So there would perhaps not be a continuing source. It may be that a new equilibrium will establish in a reasonably short time. However, there are cases in these kinds of climates where after they've paved them and gone back and looked. They found that moisture has accumulated beneath them.

Q Where would that moisture migrate from in order to reach that layer?

That's what I'm trying to establish.

A Probably it's going to be a redistribution of what's already in the soil. There's not going to be significant percolation from above any more, because you've sealed it off. It's going to have to come from what's already there

redistributing itself.

Q . What I'm trying to understand when you say redistributing there, you mean the layers of clay, if you will, that are adjacent to that area, the moisture that's already on those areas --

A Beneath.

Q Beneath. Are you saying that moisture coming all the way from the ground water, or from the area just beneath the clay?

A The area beneath the clay has some moisture profile. I don't know what it is.

I just don't know what it is, as a function of depth. But where it has been evaporating has set up one water pressure condition.

Now we're going to change it. The zone from which there is evaporation and what we call suction or tension in the poor water is now going to be cut off. There is going to be a change in the overall distribution of all of the water that's

1	beneath the surface now to some equilibrium	107
2	distribution under the new pressure	
3	conditions.	
4	Q You expect that that equilibrium	
5	will be achieved at some point in time after	
6	the soil cement that they that the soil	
7	cement is constructed and the pad is placed	
8	on top of it?	
9	A Yes.	
10	Q How long a time do you think it	
11	will take for that redistribution to take	
12	place?	
13	A Probably years.	
14	Q What would the consequence be of	
15	moisture accumulating in that top layer of	
16	the clay that is underneath the cemented	
17	soil pad, the cement?	
18	A It could make it somewhat more	
19	compressible.	
20	Q What would a change in the	

compressibility of the clay resulting over

changing the compressibilities?

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1	A It could cause somewhat increased	
2	settlements.	
3	Q Do you know whether PFS has	
4	performed consolidation tests on the soils	
5	in that layer, that area?	
6	A I don't know if they have. But	
7	I've seen the layer referred to as a stiff	
8	clay. Beyond that, I don't know what the	
9	stress history is in there at this site.	
10	Q Understanding that you have not	
11	reviewed these tests, but assuming that the	
12	two sets of such tests were conducted, one	
13	in which the soil was tested on its natural	
14	condition and another one in which the tests	
15	were performed on inundated samples, and	
16	that the second test change indicated a	
17	little change in compressibility when you	
18	inundated the sample.	
19	Would that tell you that	
20	compressibility is not a serious problem for	
21	these soils?	

A I think that it would be a good

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1	indicater, yes.	109
2	Q Now, what would be the consequence	
3	of increased settlement of this clay layer?	
4	A If it is uniform, across the area,	
5	very little consequence.	
6	Q Do you have any reason to believe	
7	that it would not be uniform?	
8	A This is a very large area, is it	
9	not? Is it 50 acres? Something of that	
10	sort. I would anticipate that the	
11	consequences, assuming that the layer	
12	thicknesses were the same and all of that	
13	across the site, probably not be too great,	
14	except perhaps at the edge.	
15	Q Now, tell me again what is the	
16	physical mechanism by which this moisture	
17	will migrate.	
18	A It migrates in response to changes	
19	in the water pressure distribution beneath	
20	the covered area.	
21	Q Would the creation or the	
22	temperature gradient or difference in	

110 1 temperature between one layer of soil or 2 another cause it to migrate, as well? 3 It could. 4 Would it migrate from the warmer 5 area to the cooler area? 6 My expectation is that, at least in the partly saturated zones, it would go 7 8 from warm to cool. I don't know how familiar you are 9 10 with the PFS project. 11 But is it your understanding that the waste storage casts that are going to be 12 placed at this site contain high levels of 13 14 radioactive spent fuel? 15 It's my understanding. 16 Are you aware that high levels of 0 radioactive spent fuel, in addition to being 17 radioactively hot is also temperature hot? 18 19 Α Yes. 20 Subject to check again, are you aware that, in fact, there at the base of 21 22 the cast on top of the pad, there is a

111 1 temperature differential of a hundred degrees, fifty to a hundred degrees above 2 3 ambient temperature because of the heat 4 released by the spent fuel? I did not know what the 5 temperature increase would be. 6 7 Assuming that, in fact, there is a fifty to a hundred degree Fahrenheit and 8 assuming that there is a fifty to a hundred 9 10 degree Fahrenheit temperature differential at the top of the pad, would you expect that 11 that heat that is being released on top of 12 the pad will migrate down through the 13 14 concrete pad?

A Yes.

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Q Would it also migrate down through the soil cement below?

A To some extent.

Q That heat, would it reach also the top layer of the clay that is underneath?

A I don't know what the distribution would be. I would assume that PFS has made

112 some thermal studies that would tell us. 1 If, in fact, there was some heat 2 that was being moved downwards by the 3 mechanism that we just described, then would 4 that heat tend to move the moisture away the 5 top layer or towards the top layer? 6 7 I would expect it to move it away. I'd be very interested in seeing the thermal 8 results of this. It's an interesting issue. 9 10 Of course, this is not something 0 that you have analyzed to date? 11 12 I have not analyzed. But I have, Α in the past, done both experimental and 13 theoretical research on the heat flow around 14 15 buried things. 16 This mechanism that I described to 17 you is one that you have reason to believe 18 its possible, or at least it's --19 Well, the heat transfer and the 20 temperature. Oh, yes. 21 Now, let's go back to 22 paragraph 14. Because I think in addition

to talking about the moisture migration, you talk about some other problem that you see as potentially happening.

A Okay.

Q I'm referring you to the very end of paragraph 14, on page 5.

Can you explain for the record what your concern is with respect to the paragraph that starts with the words, "if care"?

A Yes. What I'm concerned about here is that if you go out with heavy equipment, start excavating and moving all ground with the equipment, you can cause a lot of disturbance of the subgrade soil. I would expect, particularly in a material like this, that could cause some loss in strength and support capacity of the material. So the concern is that the mixing and placement of the soil cement be done with a minimum of disturbance to the underlying material.

Q Now, what would you do to minimize the disturbance to the underlying material in the process of building the soil cement layer?

A For the treated soil layer, one way -- again, I'm not presuming to propose a construction procedure, a design -- but if you used a mix-in-place procedure with equipment, if there is such equipment that's capable of going two feet -- and that's pushing the envelope I think there -- you would resolve that problem.

I think I read in something as I was skimming through, that there is a plan to use equipment that has an extendable boom that will enable you to get out into the areas without disturbing them. I suspect you could start in one place and work your way across.

Q So this is a mix that you have seen used in other jobs?

A The what?

1	Q Are these the types of techniques	5
2	that you have seen in other jobs where there	
3	is a desire not to disturb the soil in the	
4	process of placing soil cement?	
5	A Well, I can't put my finger on	
6	specific examples. But I'm just thinking of	
7	the conventional procedures that are used	
8	for doing this. Those are ways to approach	
9	it.	
10	Q But I take it, what I'm trying to	
11	get to, these are things that are within the	
12	state-of-the-art?	
13	A Yes, I guess.	
14	Q In skimming through Mr. Trudeau's	
15	deposition, do you recall whether he had	
16	discussed what PFS is considering in doing	
17	to address this issue?	
18	A In something I saw it. It could	
19	have well be there.	
20	Q But, essentially, if I can presume	
21	to boil down your concern and tell me if	
22	I'm wrong your concern is that if care is	

1	not taken not to disturb the top layer of
2	the soil in the process of putting soil
3	cement, you could do injury, if you will, to
4	the soil; is that right?
5	A To the subgrade material beneath
6	the soil cement or the treated soil, yes.
7	Q What would the nature of the
8	injury or damage be?
9	A It would be disturbing the
10	material that I believe, from what I've
11	read, is fairly stiff. It may be sensitive
12	disturbance in the sense that it would loose
13	strength, become mushier, so to speak, and
14	not provide as good subgrade support.
15	Q That's a concern that should be
16	addressed in developing the construction
17	procedures, if you will?
18	A I think so.
19	(Counsel conferred with witness)
20	BY MR. TRAVIESO-DIAZ:
21	Q Let me go back a moment to
22	sulfates. Because I know we have some

things that are pending with sulfates.

Assuming there is a presence of sulfates either in the top layer -- which the test have been shown so far -- or in the layer below, are there treatment methods that can be use to address the presence of sulfates?

A There are some things that you can do if sulfates are likely to be an issue.

One of these is to use sulfate-resistant cement. Another is to increase the treatment level. You know, those are probably the two main approaches.

There are some chemical approaches that have been proposed or suggested for dealing with sulfates in the case of lime-treated soils. But that's a little bit different issue. I think it's sort of experimental at this point. So I think that the more logical way here would be either to use more cement, or to use sulfate-resisting cement.

1 Q Now, let's talk about the first.

When you mean sulfate-resistant cement, meaning a mixture that includes some additive that will make the soil cement result be more likely to resist the affects of sulfate?

reactions, you need to have a source of alumina. There's two sources. Three sources, I suppose. One is in the cement itself, just because of the formulation of cement. Another is in compounds that might be present in the soil itself. The third is from the breakdown of clay that may be present in the soil during the hydration processes. Because when you break down the clay, then you will liberate alumina.

The main cementing material in this case though is the cement itself. So anything that comes from breaking down the soil probably will be not too significant. It's the aluminum that's in the cement that

1	could be the biggest problem in this case.
2	The way you minimize that is to use
3	sulfate-resisting cement.
4	Q That's what I was going to ask
5	you. You anticipated my question.
6	There are different types of
7	cement mixes that can specify that will have
8	say less aluminum contact and, therefore,
9	they will be less subject to sulfate attack?
10	A That is correct.
11	Q So that would be a question of
12	coming up with a proper chemical proposition
1,3	with the cement that you use?
14	A Yes.
15	Q Is that right?
16	A Uh-huh.
17	Q That's something that somebody
18	aware of the problem can come up with a, if
19	you will, a cement recipe that will minimize
20	its affects?
21	A Yeah. Right. Type four cement, I
22	believe, is sulfate